## Garvey & Strathmore Apartments Drainage Study 7849 Garvey Avenue

Rosemead, CA 91770

### Date Prepared:

August 11, 2022

#### Prepared for:

Green Park Property, LLC Attn.: Cindy Lau 120 E. Valley Blvd. San Gabriel, CA 91776 (626) 307-0062

#### Prepared By:



#### Declaration of Responsible Charge:

I hereby declare that I am the engineer of work for this project, that I have exercised responsible charge over the design of the project as defined in section 6703 of the business and professions code, and that the design is consistent with current standards. I understand that the check of the project drawings and specifications by the City of Rosemead is confined to a review only and does not relieve me, as an engineer of work, of my responsibilities for project design.

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Patric T. de Boer Registration Expires

RCE 83583 3-31-2023



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## Site & Project Description

This drainage study has been prepared for the proposed project located at 7849 Garvey Avenue in the City of Rosemead. The project site is located 0.65 south of Interstate 10 at the northwest corner of the intersection of Garvey Avenue and Strathmore Avenue. See figure No.1 for a Vicinity Map.

The site is currently occupied by a paved parking lot and three commercial buildings. No existing storm drains or inlets are known to exist on the project site.

The proposed project involves the demolition and removal of all existing improvements and structures. A seven story apartment with an integrated parking garage will be constructed. The single building will cover the majority of the site.

## Methodology

This drainage report has been prepared in accordance with current County of Los Angeles regulations and procedures.

The analyses of the existing and proposed conditions were performed using HydroCalc (Version 1.02) to calculate runoff flowrates and volumes. Given the area, length of flow path, average slope, design storm depth, imperviousness, and soil type, HydroCalc generates a hydrograph for the existing and proposed conditions. Soil and rainfall inputs were determined using the GIS data provided by maps the Los Angeles Flood Control District.

This report analyzes the flow generated by the 25, 50 and 100-year storm events for storm drain sizing and flood control purposes.

The following references were used in this report

- (1) <u>Handbook of Hydraulics</u>, E.F. Brater & H.W. King, 6<sup>th</sup> Ed., 1976.
- (2) Los Angeles County Department of Public Works Hydrology Manual, 2006

## Existing Conditions

The existing 1.21 acre site is 100% impervious and underlain by soil type # 7. The site accepts offsite runoff from the alley and the properties west of the site. The offsite areas surface flow to the site and confluence with the on-site flow before draining to Strathmore Avenue. The entire site and the offsite areas drain east via surface flow to the gutter on Strathmore Ave., thence south to the existing curb inlet, and ultimately to the existing 24" pipe on the public storm drain system. This point is referred to as Discharge Point # 1 in this report.

## Proposed Conditions

The proposed improvements consist of a seven-story apartment building, hardscape and on-site storm drain system. The proposed development will grade the entire site but will keep the same discharge point as the existing conditions. The proposed site will be 100% impervious. The majority of the site will be covered by the building.

Catch basins will be installed along the west end of the building and the westerly alley to accept the offsite runoff generated from the westerly properties and bypass via pipe flow to a connection point to the 24" storm drain in Strathmore Avenue. The roof drains of the building will drain directly to storm drain pipes in the ground level parking lot that will be connected to a proposed 8'x16' Modular Wetland System vault for treatment. Following treatment, the stormwater will drain out via pipe flow to a proposed storm drain pipe that will connect to the existing 24" RCP pipe on Strathmore. This point is referred to as Discharge Point # 1 in this report.

### Existing HydroCalc Analysis

The existing condition was modeled as a single on-site basin and three off-site basins, referred to as E-1, O-1, O-2 and O-3 in this report. Below is a summary of the HydroCalc calculations for the existing conditions.

Basin #	Area	Soil	Imperv.	$I_{25}$	$I_{50}$	$I_{100}$	Q <sub>25</sub>	$Q_{50}$	$Q_{100}$
	(ac)	Туре	(%)	(in/hr)	(in/hr)	(in/hr)	(cfs)	(cfs)	(cfs)
E-1	1.21	7	100	3.25	3.70	4.15	3.54	4.03	4.52
O-1	0.36	7	100	3.25	3.70	4.15	1.05	1.20	1.35
O-2	0.17	7	100	3.25	3.70	4.15	0.50	0.57	0.64
O-3	0.29	7	45	3.25	3.70	4.15	0.85	0.97	1.08
				Total (	Combine	d Flow	5.93	6.76	7.58

The output data sheets from HydroCalc can be found at the end of this report.

## Proposed HydroCalc Analysis

The proposed site was modeled as a single on-site basin and three offsite basins, referred to as P-1, O-1, O-2 and O-3 in this report.

Basin #	Area (ac)	Soil Type	Imperv.	$I_{25}$ (in/hr)	$I_{50}$ (in/hr)	$I_{100}$ (in/hr)	Q <sub>25</sub> (cfs)	Q <sub>50</sub> (cfs)	Q <sub>100</sub> (cfs)
D 1	\ /	1 ypc		( ' /		( ' /		\ /	
P-1	1.21	/	100	3.25	3.70	4.15	3.54	4.03	4.52
O-1	0.36	7	100	3.25	3.70	4.15	1.05	1.20	1.35
O-2	0.17	7	100	3.25	3.70	4.15	0.50	0.57	0.64
O-3	0.29	7	45	3.25	3.70	4.15	0.85	0.97	1.08
				Total	Combine	d Flow	5.93	6.76	7.58

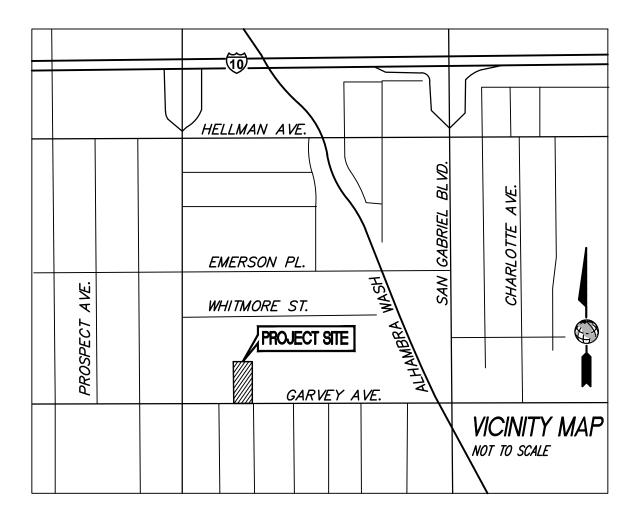
Below is a summary of the HydroCalc calculations for the proposed conditions.

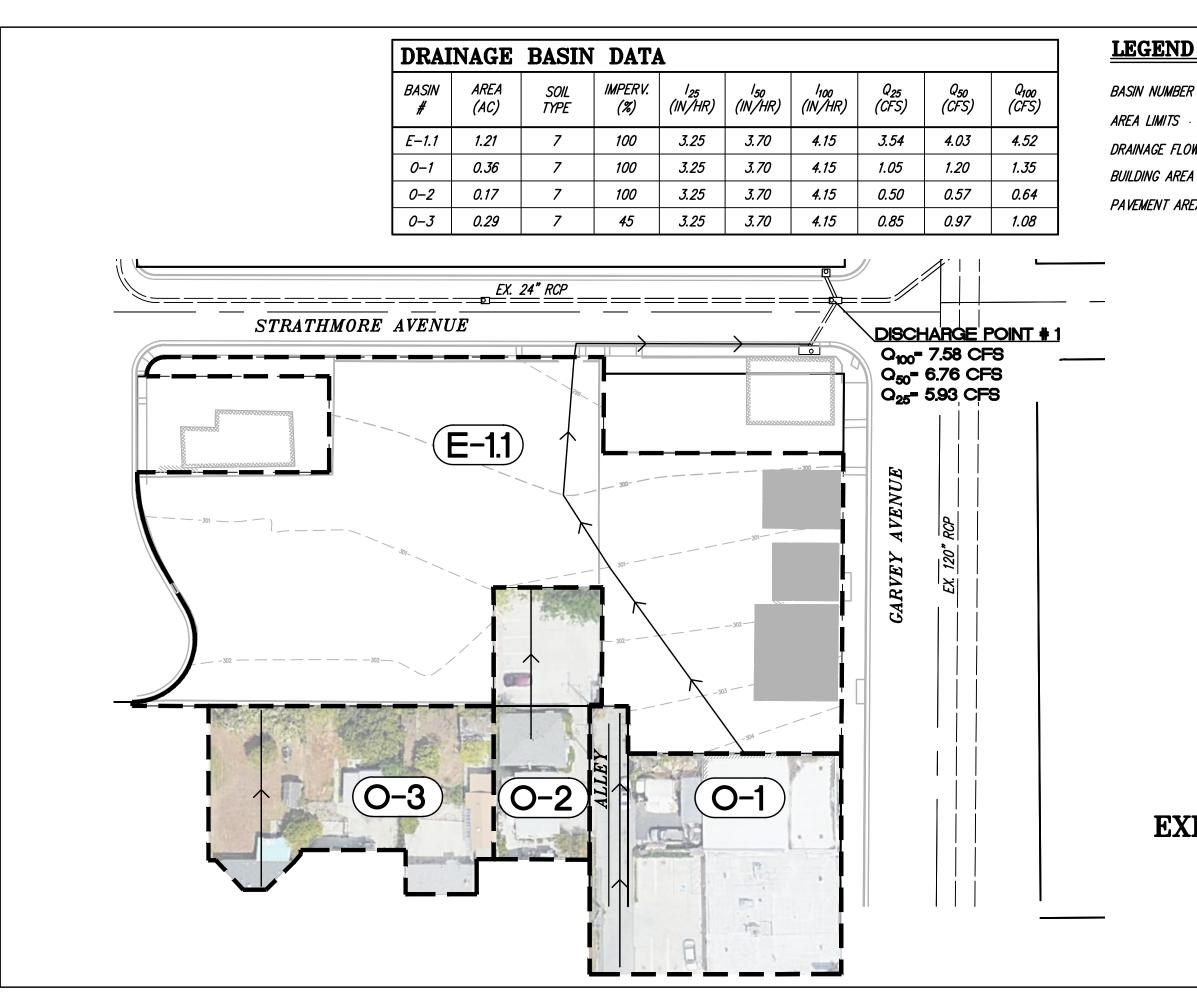
The output data sheets from HydroCalc can be found at the end of this report.

### Results and Conclusions

The proposed improvements result in no change to the peak runoff flowrate for the 25, 50 and 100-year storm events. This is because the site is 100 % impervious for both the existing and proposed conditions. The onsite flowpath was modified, but the time of concentration in both the existing and proposed conditions is below the 5.0 minute minimum allowed by the Rational Method. The offsite flowpath is modified in that runoff from the site will discharge directly to a new connection to the 24" storm drain in Strathmore Ave, rather than flowing to the existing curb inlet first.

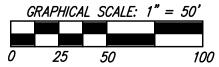
It is the opinion of Omega Engineering Consultants that the project will not cause adverse effects to the downstream drainage facilities or receiving waters. A separate LID Plan has been prepared to discuss the water quality impacts of the proposed development.





NUMBER	
AGE FLOW PATH	
NG AREA	
ENT AREA ······	





# **STRATHMORE EXISTING HYDROLOGY EXHIBIT**



EXHIBIT

	DRAI	NAGE	BASIN	DAT	A						<u>LEGEN</u>	<u>ID</u>				
	BASIN #	AREA (AC)	SOIL TYPE	IMPERV. (%)	<sub>25</sub> (IN/HR)	<sub>50</sub> (IN/HR)	<sub>100</sub> (IN/HR)	Q <sub>25</sub> (CFS)	Q <sub>50</sub> (CFS)	Q100 (CFS)		BER				#
	P-1.1	1.21	7	100	3.25	3.70	4.15	3.54	4.03	4.52		LOW PATH				
	0-1	0.36	7	100	3.25	3.70	4.15	1.05	1.20	1.35		REA			/	
	0–2	0.17	7	100	3.25	3.70	4.15	0.50	0.57	0.64	PAVEMENT	AREA			_	
	0–3	0.29	7	45	3.25	3.70	4.15	0.85	0.97	1.08						
		FY	24" RCP							L		PE DAT	A			
STRATHMORE	AVENU	0 =							IARGE F	<u>POINT # 1</u>	 PIPE #	DIAMETER (INCHES)	SLOPE (%)	DEP TH /DIA	V <sub>100</sub> (FPS)	Q100 (CFS)
	N					20000000000000000000000000000000000000	0	Q <sub>100</sub> =	7.58 CF	·S		12	0.5	0.73	3.68	2.26
			6			00000000		Q <sub>25</sub> =	6.76 CF 5.93 CF	5 5	2	12	0.5	0.73	3.68	2.26
				! I		6 6 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8					3	12	0.5	0.52	3.27	1.35
											4	12	0.5	0.34	2.72	0.64
	0			•				UE			5	12	0.5	0.45	3.15	1.08
SD				10				AVENUE			6	18	0.5	0.83	4.83	7.58
	SD -3		P−1.1)	S S		<b>D-1</b> )		GARVEY A	<u> </u>		ROOF DRAI	STI Ropos	M DRAIN SY PAIN SYSTEM PHICAL SCA 25 50 RATH	STEM THAT ON STRATI	ULTIMATEL HMORE AVE 100	Y DRAINS TNUE.
					B	Te I	-								ME	GA



#### CONDUIT SIZING CALCULATIONS

The following chart details the sizing parameters and for conduits that convey runoff on the site.

K'= Discharge factor
n= Mannings coefficient
d=diameter of conduit (ft)
Q= Discharge
s=Minimum Pipe Slope (ft/ft)
D=depth of flow
C <sub>a</sub> = Flow factor
A=Cross sectional area of flow
V=Velocity

- (Q\*n)/(d<sup>8/3</sup>\*s<sup>1/2</sup>) =
- 0.013 for PVC & HDPE =
- per chart =
- based off portions of basins tributary to outlet =
- per chart =
- From table 7-4 See right =
- From table 7-14 See right =
- $C_a * d^2$ =
- Q/A =

#### **Pipe Flow**

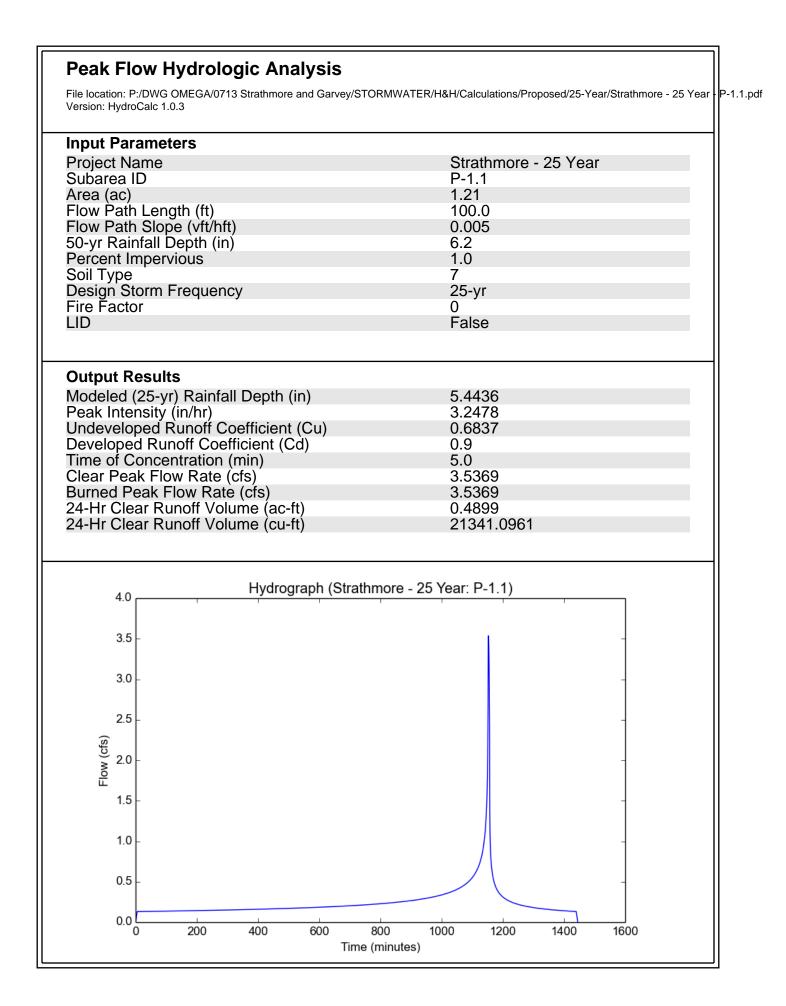
Pipe	Tributary Areas	Q (cfs)	S (%)	d (in)	К'	D/d	C <sub>a</sub>	A (sf)	V (fps)
1	Half of proposed building roof	2.26	0.5	12	0.4155	0.73	0.614	0.614	3.68
2	Half of proposed building roof	2.26	0.5	12	0.4155	0.73	0.614	0.614	3.68
3	Basin O-1	1.35	0.5	12	0.2482	0.52	0.413	0.413	3.27
4	Basin O-2	0.64	0.5	12	0.1177	0.34	0.236	0.236	2.72
5	Basin O-3	1.08	0.5	12	0.1986	0.45	0.343	0.343	3.15
6	Entire proposed building roof and offsite basns	7.58	0.5	18	0.4727	0.83	0.697	1.568	4.83

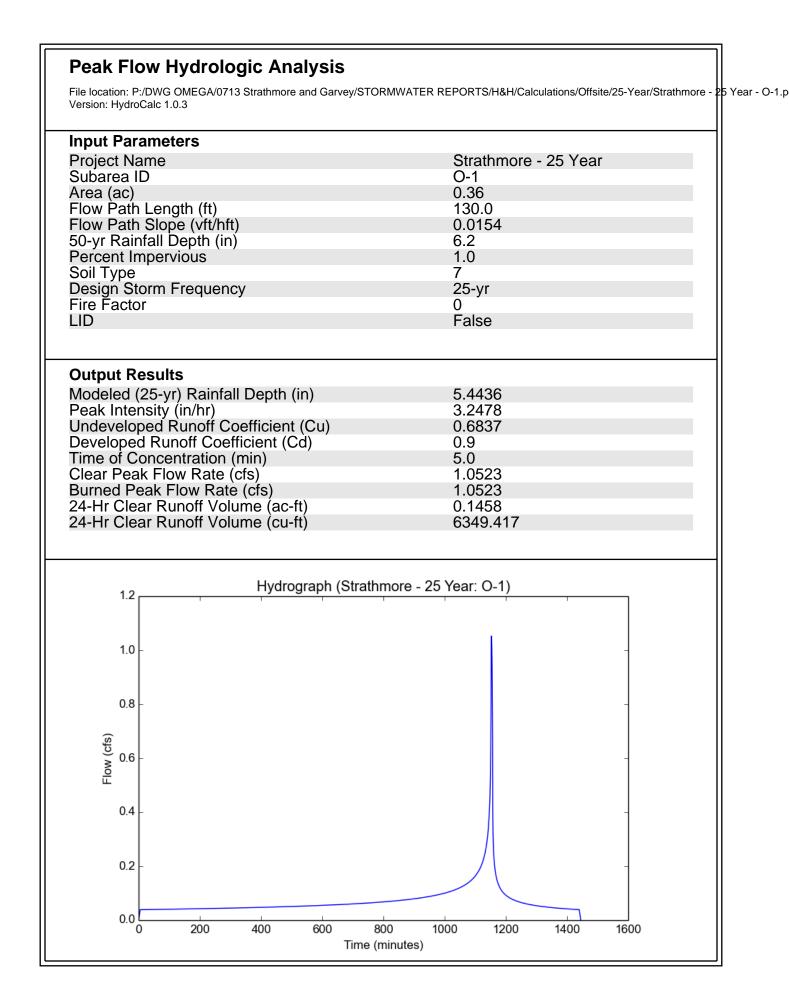
t di	depth of ameter	of water of chan	$\frac{1}{nel} = \frac{1}{2}$	$\frac{D}{d}$ and $C$	uit Flo $Z_a = th$	e tabula	ated va	lue. T	hen a =	$= C_a d^a$
$\frac{D}{d}$	.00	.01	.02	.03	.04	.05	.06	.07	.08	.09
.0	.0000	.0013	.0037	.0069	.0105	.0147	.0192	.0242	.0294	.035
.1	.0409	.0470	.0534	.0600	.0668	.0739	.0811	.0885	.0961	.1039
.2	.1118	.1199	.1281	.1365	.1449	.1535	.1623	.1711	.1800	.1890
.3	.1982	.2074	.2167	.2260	.2355	.2450	.2546	.2642	.2739	.2836
.4	.2934	.3032	.3130	.3229	.3328	.3428	.3527	.3627	.3727	.3827
.5	.393	.403	.413	.423	.433	.443	.453	.462	.472	.482
.6	.492	.502	.512	.521	.531	.540	.550	.559	.569	.578
.7	.587	.596	.605	.614	.623	.632	.640	.649	.657	.666
.8	.674	.681	.689	.697	.704	.712	.719	.725	.732	.738
.9	.745	.750	.756	.761	.766	.771	.775	.779	.782	.784

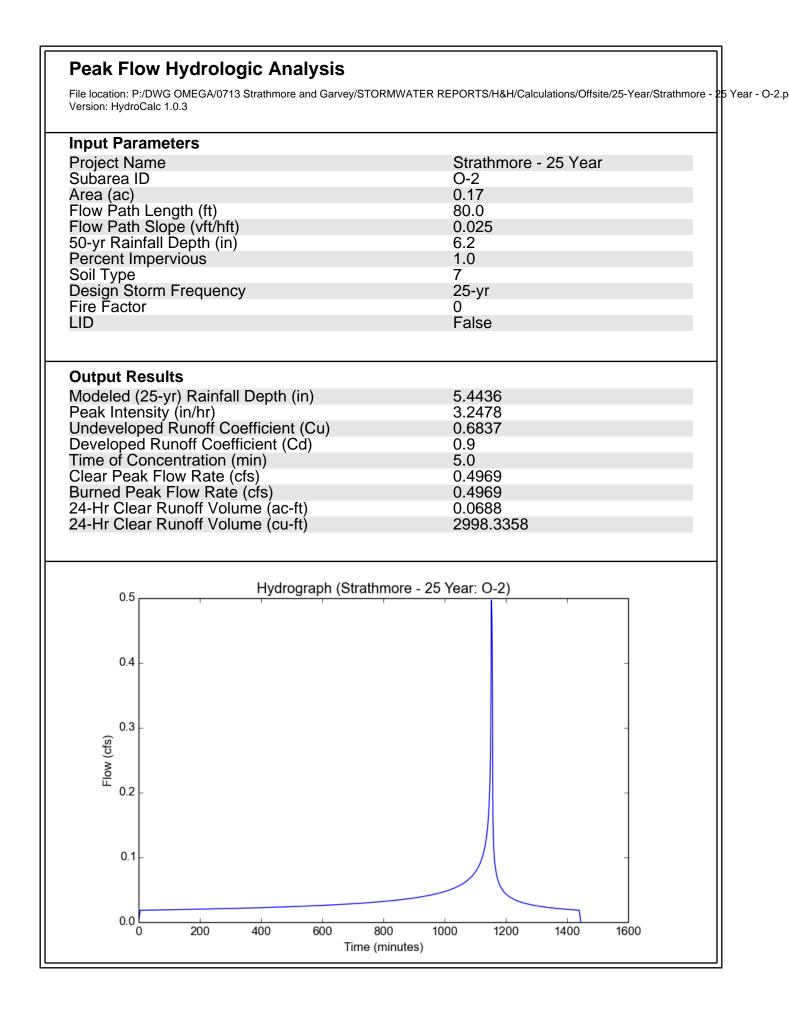
# Table 7-14. Values of K' for Circular Channels in the Formula $Q = \frac{K'}{n} d^{\frac{5}{5}} d^{\frac{5}{5}}$

$\frac{D}{d}$	.00	.01	.02	.03	.04	.05	.06	.07	.08	.09
.0		.00007	.00031	.00074	.00138	.00222	.00328	.00455	.00604	.00775
.1	.00967	.0118	.0142	.0167	.0195	.0225	.0257	.0291	.0327	.0366
.2	.0406	.0448	.0492	.0537	.0585	.0634	.0686	.0738	.0793 -	.0849
.3	.0907	.0966	.1027	.1089	.1153	.1218	.1284	.1352 -	.1420	.1490
.4	.1561	.1633	.1705	.1779	.1854	.1929	.2005	.2082	.2160	.2238
.5	.232	.239	.247	.255	.263	.271	.279	.287	.295	.303
.6	.311	.319	.327	.335	.343	.350	.358	.366	.373	.380
.7	.388	.395	.402	.409	.416	.422	.429	.435	.441	.447
.8	.453	.458	.463	.468	.473	.477	.481	.485	.488	.491
.9	.494	.496	.497	.498	.498	.498	.496	.494	.489	.483
1.0	.463	16.	327	Sing	1753		- service	121	1	

#### **Peak Flow Hydrologic Analysis** File location: P:/DWG OMEGA/0713 Strathmore and Garvey/STORMWATER/H&H/Calculations/25-Year/Strathmore - 25 Year - E-1.1.pdf Version: HydroCalc 1.0.3 **Input Parameters Project Name** Strathmore - 25 Year Subarea ID E-1.1 Area (ac) 1.21 Flow Path Length (ft) 235.0 Flow Path Slope (vft/hft) 0.0241 50-yr Rainfall Depth (in) 6.2 Percent Impervious 1.0 Soil Type 7 **Design Storm Frequency** 25-yr Fire Factor 0 LID False **Output Results** Modeled (25-yr) Rainfall Depth (in) 5.4436 Peak Intensity (in/hr) 3.2478 Undeveloped Runoff Coefficient (Cu) 0.6837 Developed Runoff Coefficient (Cd) 0.9 Time of Concentration (min) 5.0 Clear Peak Flow Rate (cfs) 3.5369 Burned Peak Flow Rate (cfs) 3.5369 24-Hr Clear Runoff Volume (ac-ft) 0.4899 24-Hr Clear Runoff Volume (cu-ft) 21341.0961 Hydrograph (Strathmore - 25 Year: E-1.1) 4.0 3.5 3.0 2.5 Flow (cfs) 2.0 1.5 1.0 0.5 0.0 200 400 600 800 1000 1200 0 1400 1600 Time (minutes)

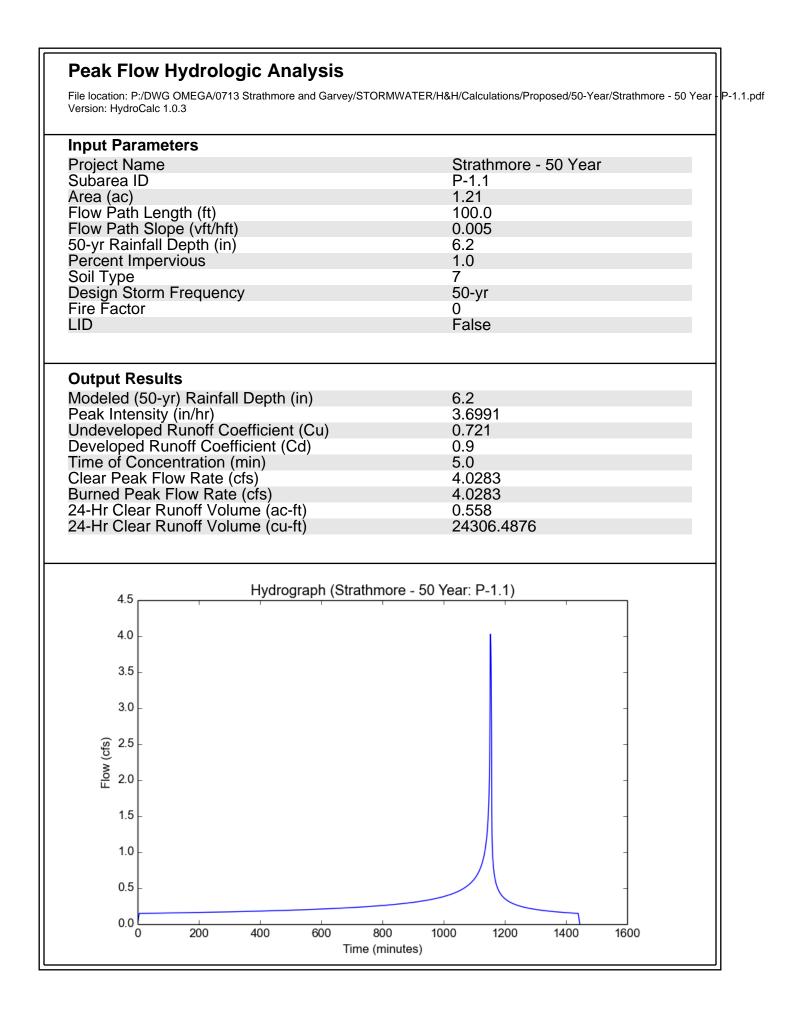


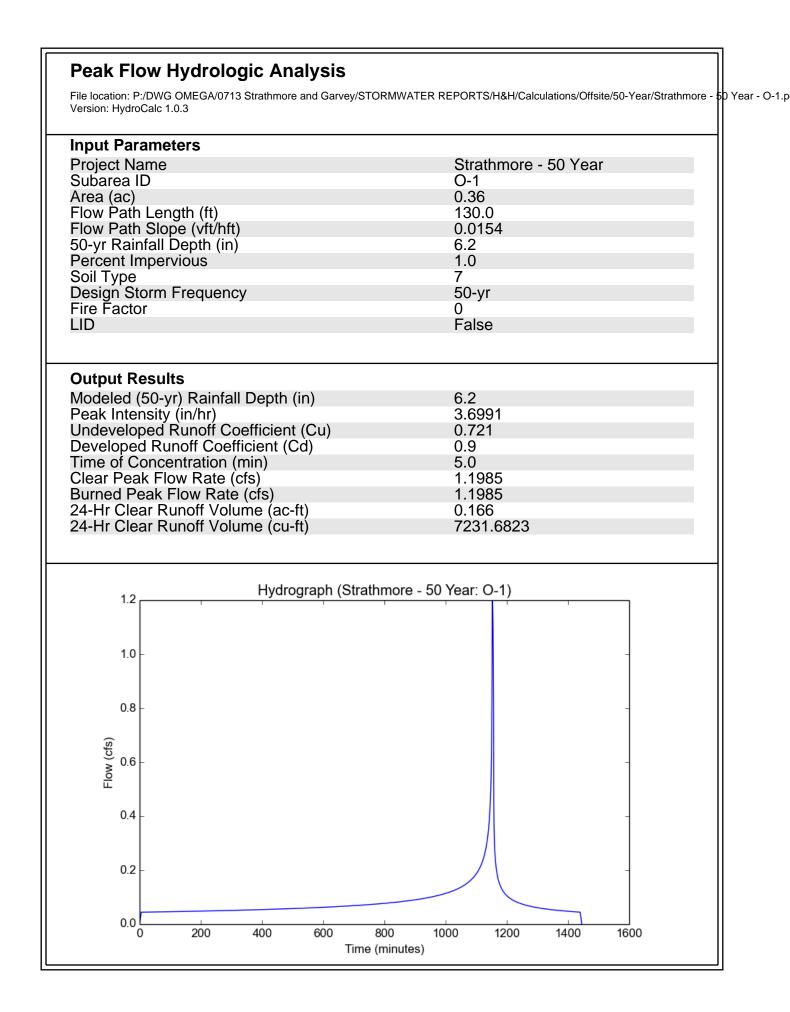


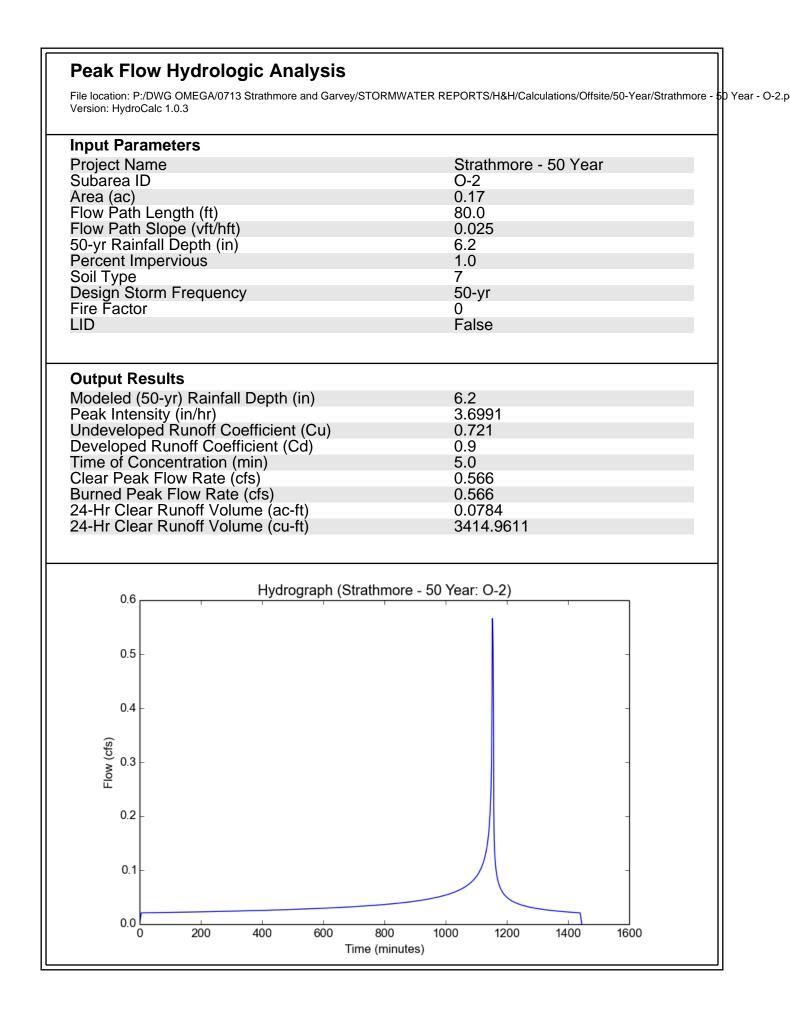


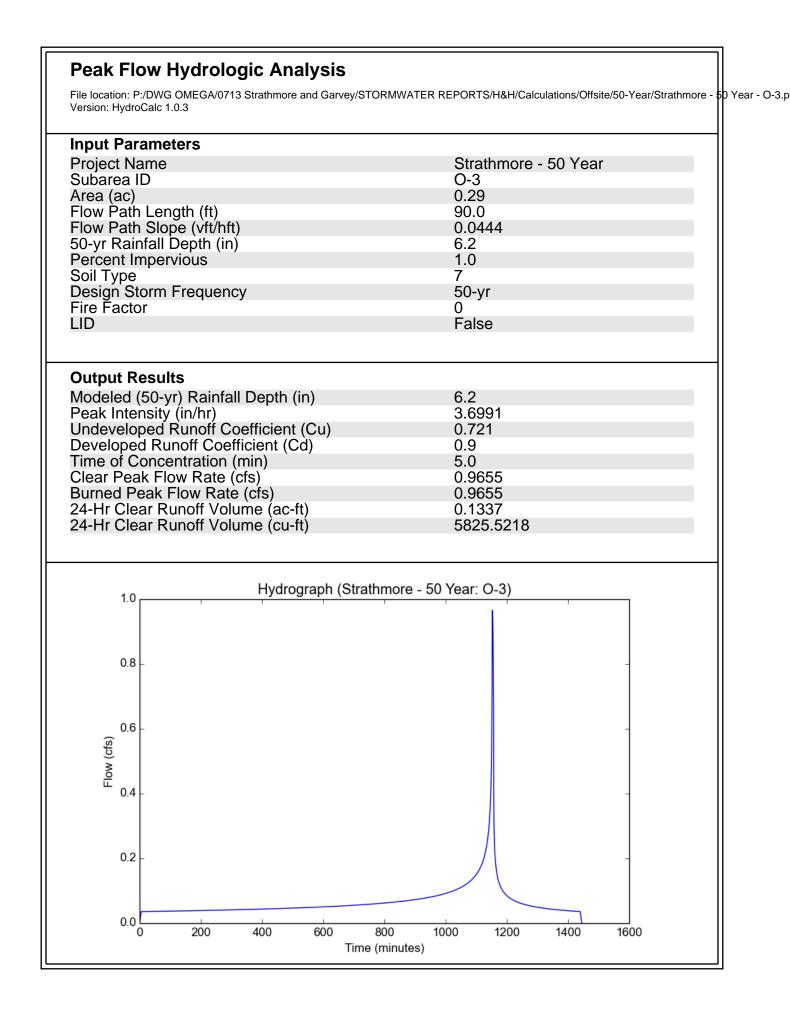
e location: P:/DWG OMEGA/0713 Strathmore and Garvey/STORM ersion: HydroCalc 1.0.3	/WATER REPORTS/H&H/Calculations/Offsite/25-Year/Strathmore
put Parameters	
roject Name	Strathmore - 25 Year
ubarea ID	0-3
rea (ac)	0.29
low Path Length (ft)	90.0
low Path Slope (vft/hft)	0.0444
0-yr Rainfall Depth (in)	6.2
ercent Impervious	1.0
oil Type	7
esign Storm Frequency	25-yr
ire Factor	0
	False
-	
utput Results	
lodeled (25-yr) Rainfall Depth (in)	5.4436
eak Intensity (in/hr)	3.2478
eak Intensity (in/hr) ndeveloped Runoff Coefficient (Cu)	0.6837
eveloped Runoff Coefficient (Cd)	0.9
ime of Concentration (min)	5.0
lear Peak Flow Rate (cfs)	0.8477
urned Peak Flow Rate (cfs)	0.8477
4-Hr Clear Runoff Volume (ac-ft)	0.1174
4-Hr Clear Runoff Volume (cu-ft)	5114.8082
	0111.0002
0.9 Hydrograph (Strathm	ore - 25 Year: O-3)
0.8 -	-
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<u>ي</u> 0.5	
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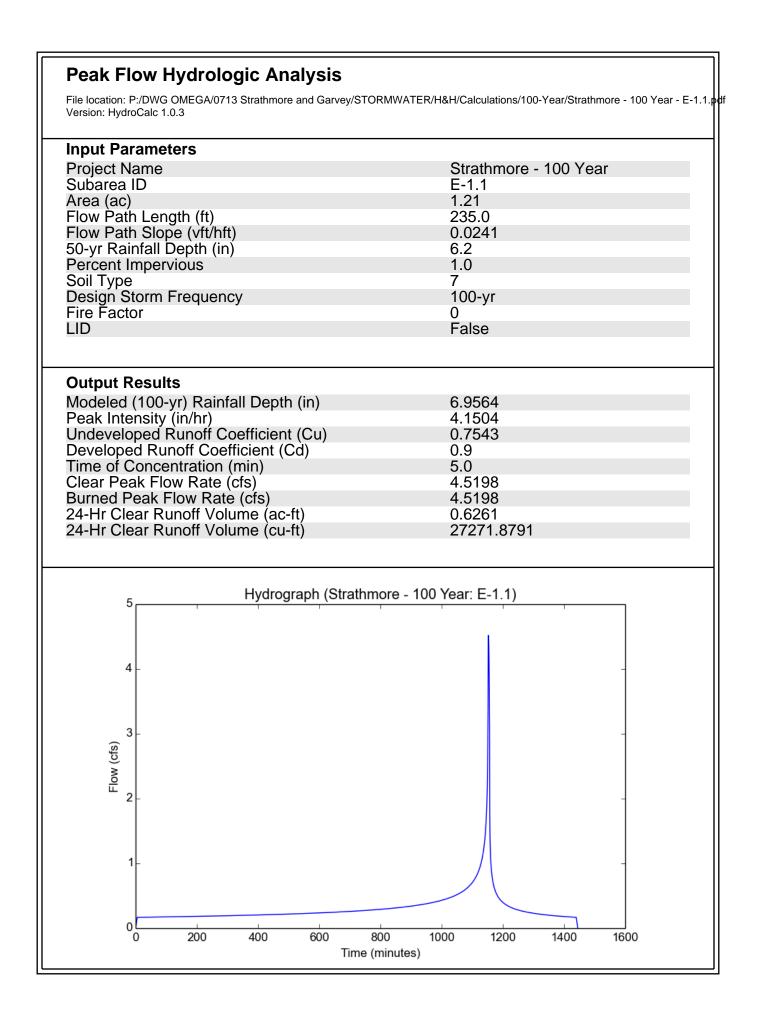
#### **Peak Flow Hydrologic Analysis** File location: P:/DWG OMEGA/0713 Strathmore and Garvey/STORMWATER/H&H/Calculations/50-Year/Strathmore - 50 Year - E-1.1.pdf Version: HydroCalc 1.0.3 **Input Parameters Project Name** Strathmore - 50 Year Subarea ID E-1.1 Area (ac) 1.21 Flow Path Length (ft) 235.0 Flow Path Slope (vft/hft) 0.0241 50-yr Rainfall Depth (in) 6.2 Percent Impervious 1.0 Soil Type 7 **Design Storm Frequency** 50-yr Fire Factor 0 LID False **Output Results** Modeled (50-yr) Rainfall Depth (in) 6.2 Peak Intensity (in/hr) 3.6991 Undeveloped Runoff Coefficient (Cu) 0.721 Developed Runoff Coefficient (Cd) 0.9 Time of Concentration (min) 5.0 Clear Peak Flow Rate (cfs) 4.0283 Burned Peak Flow Rate (cfs) 4.0283 24-Hr Clear Runoff Volume (ac-ft) 0.558 24-Hr Clear Runoff Volume (cu-ft) 24306.4876 Hydrograph (Strathmore - 50 Year: E-1.1) 4.5 4.0 3.5 3.0 Flow (cfs) 2.5 2.0 1.5 1.0 0.5 0.0 200 400 600 800 1000 1200 0 1400 1600 Time (minutes)

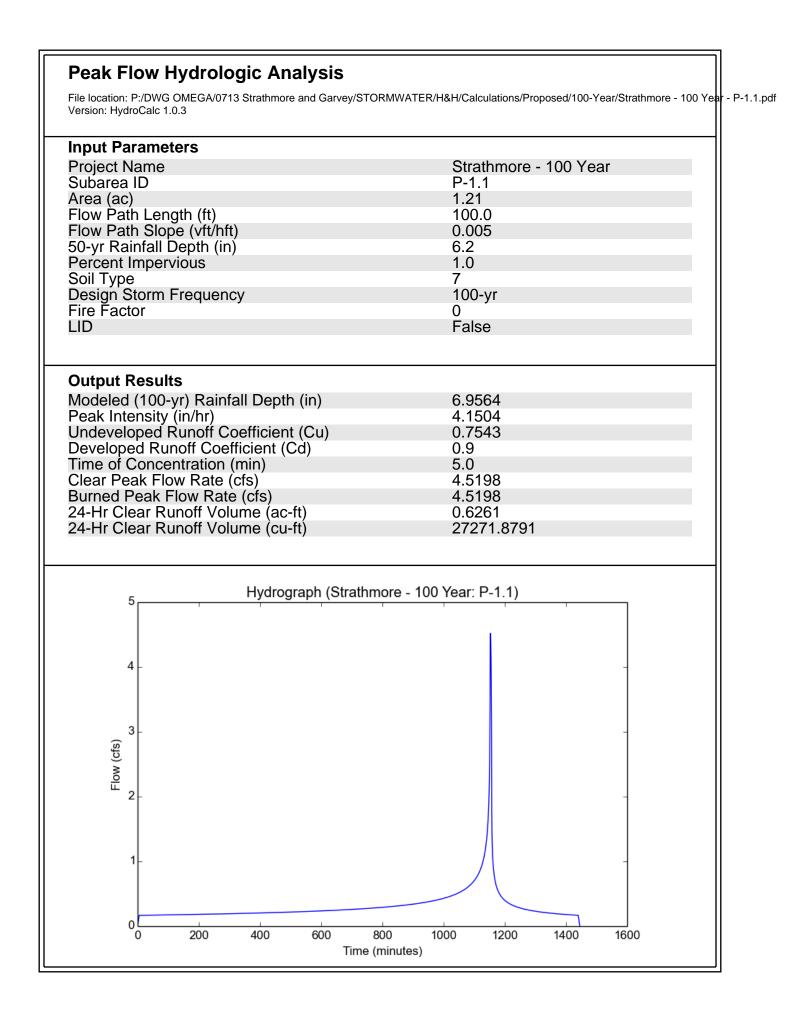


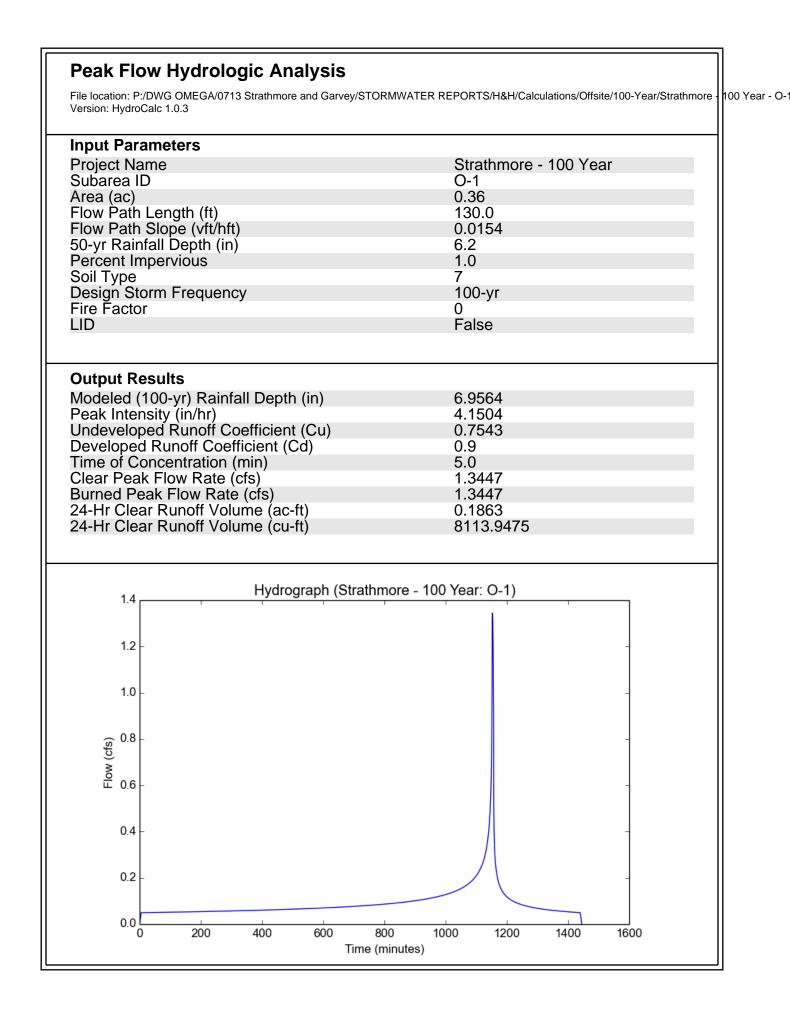


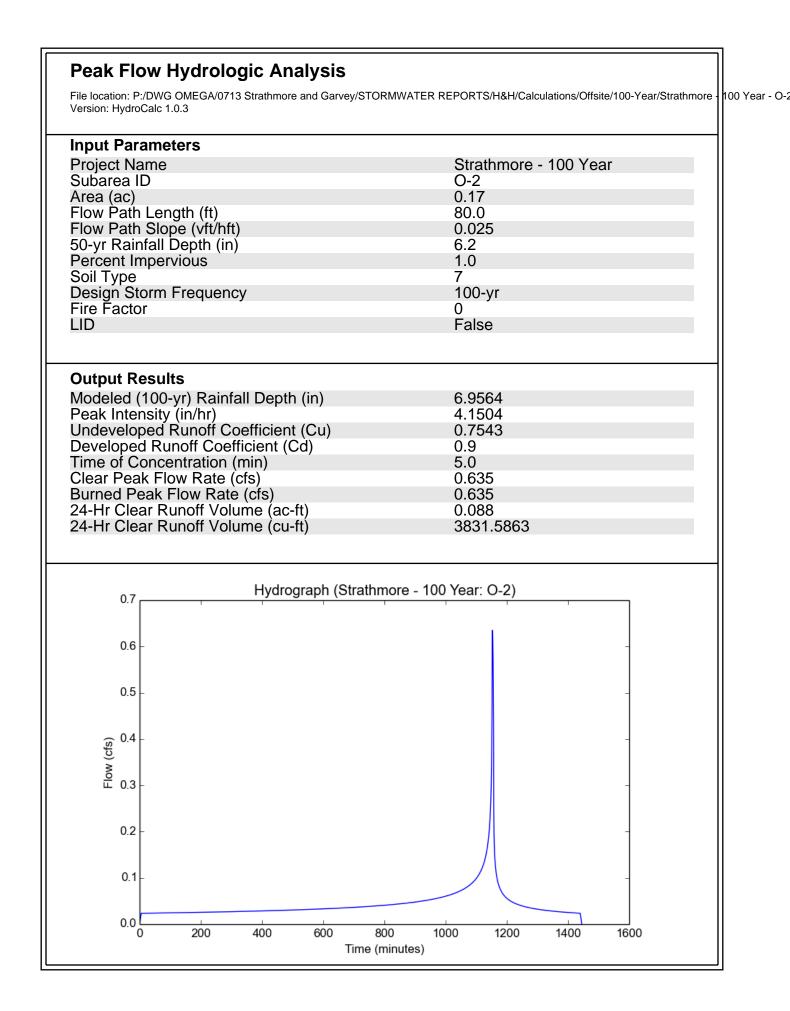


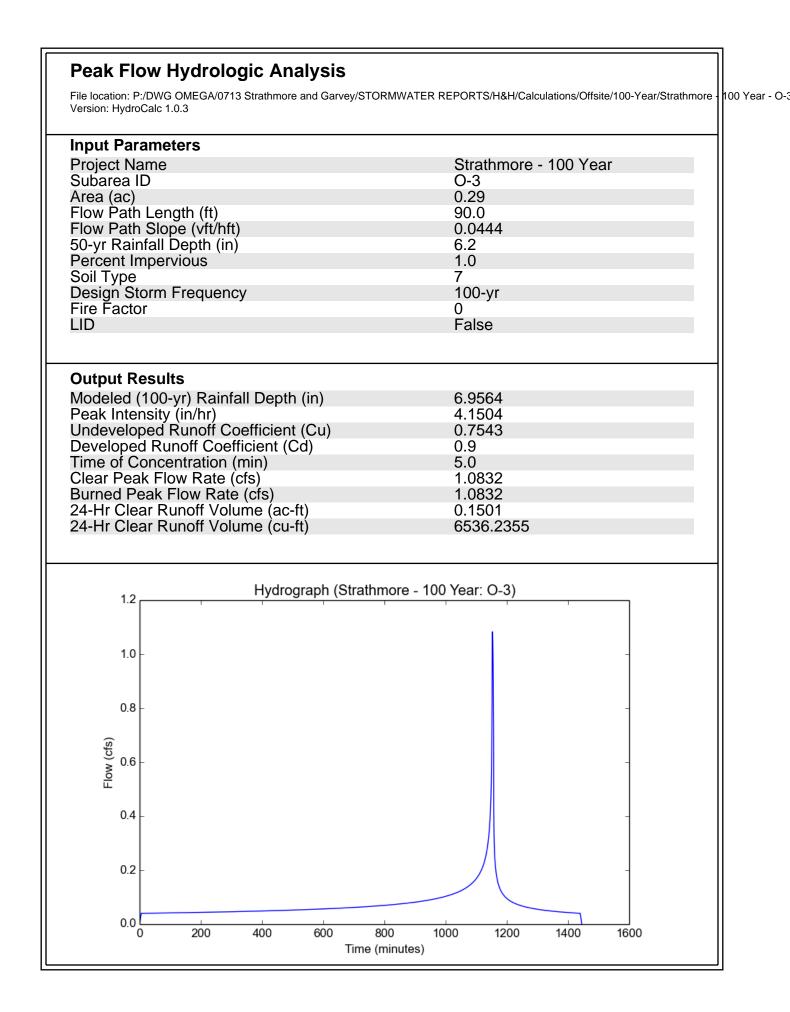




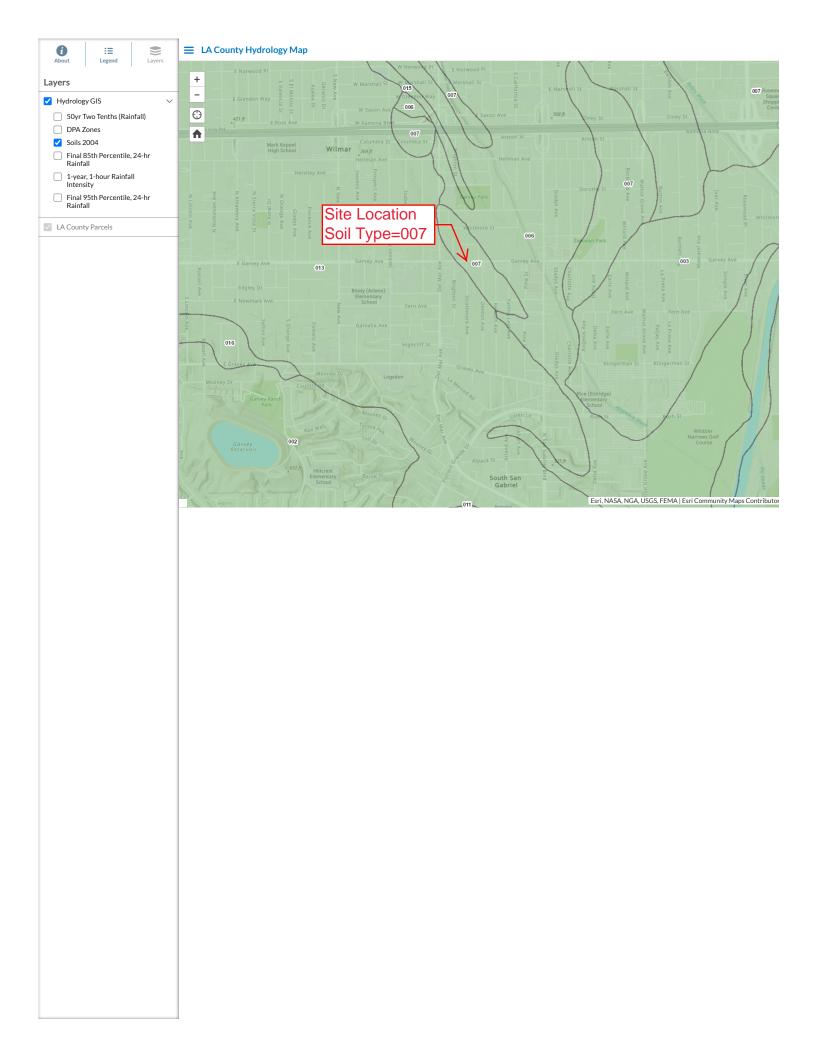








Appendix 1



Appendix 2

