Project Plans



PEOPLE'S SELF HELP HOUSING

GREENFIELD, CALIFORNIA

| | GREENFIELD | , CALIFORNIA | |
|---|--|---|---|
| APPLICABLE CODES: 2019 Building Standards Administrative Code, Part 1, CBSC 2019 California Building Code (CBC) Part 2, CBSC (2006 IBC & California Amendments) 2019 California Electrical Code (CEC) Part 3, CBSC (2005 NEC & California Amendments) 2019 California Mechanical Code (CMC) Part 4, CBSC (2006 UMC & California Amendments) 2019 California Plumbing Code (CPC), Part 5 CBSC (2006 UPC & California Amendments) 2019 California Energy Code, Part 6 CBSC 2019 California Fire Code, Part 9 CBSC (2006 IFC & California Amendments) 2019 California Fire Code, Part 9 CBSC (2006 IFC & California Amendments) 2019 California Referenced Standards, Part 12, CBSC Title 19 C.C.R., Public Safety, SFM Regulations NFPA 13, Automatic Sprinkler System, 2010 edition NFPA 72, Nat'l Fire Alarm Code, (Ca Amended) 2010 Edition (See UL Standard 1971 for "Visual Devices) City of Greenfield Municipal Code (Current Edition) | OWNER / APPLICANT: Peoples' Self-Help Housing Corp. 3533 Empleo Street San Luis Obispo, California 93401 P: 805-540-2465 F: 805-544-1901 Efrain Lopez (owner) Sheryl Flores (Applicant): sherylf@pshhc.org ARCHITECT: THE PAUL DAVIS PARTNERSHIP, LLP Attn: Paul W. Davis, AIA, 286 Eldorado Street | PROJECT INFORMATION ZONING: APN: 009-082-013-000 PARCEL SIZE: 198,400 SF /4.6 ACRES MAX. LOT COVERAGE ALLOWED: PROVIDED: 49,372 SF (25% OF PARCEL) - SEE A1.1 FOR EACH HOUSE LOT FLOOR AREA RATIO PROVIDED: 59,572 SF (30% OF PARCEL) MAX. BUILDING HEIGHT ALLOWED: PROVIDED: 15'-26' (SINGLE STORY & TWO STORIES) PARKING REQUIREMENT: SINGLE FAMILY HOMES (2-COVERED) | A0.1 TITLE SHEET, ABBREVIATIONS, & PROJECT INFORMATION CIVIL SHEET 1 CIVIL COVER SHEET SHEET 2 DEMOLITION PLAN SHEET 3 PROPOSED SUBDIVISION LAYOUT SHEET 4 PROPOSED SUBDIVISION LAYOUT SHEET 5 PROPOSED SUBDIVISION UTILITY LAYOUT SHEET 6 PROPOSED SUBDIVISION UTILITY LAYOUT SHEET 7 PROPOSED SUBDIVISION STREET LAYOUT |
| Should any condition develop that is not covered by the approved plans and specifications such that the finished work will not comply with sitle 24, a change order detailing and specifying the required work shall be submitted to and approved prior to proceeding with the work. The intent of the plans and specifications is to construct this work in accordance with the california building standards code, titles 19 and 24, california code of regulations, should any conditions develop not covered by the approved plans and specifications wherein the finished work will not comply with title 24, california code of regulations, a change order detailing and specifications wherein the finished work will not comply with title 24, california code of regulations, a change order detailing and specifying the required work shall be submitted to and approved by the owner before proceeding with the work. | Monterey, CA 93940 P: 831-373-2784 ext. 207/ 206 F: 373-7459 paulw@pauldavispartnership.com ENGINEER & MONTEREY BAY ENGINEERS, INC. Attn: Steven C. Wilson 607 Charles Ave., Suite B Seaside. CA 93955 P: 831-899-7879 F: 831-899-7879 | PROVIDED: 20x20' 2-CAR GARAGE PER HOUSE CONSTRUCTION TYPE: V-B FIRE SPRINKLER: NFPA 13R BUILDING AREA BREAKDOWN: SINGLE STORY HOUSE: 1,557 SF @ 20 = 31,140 FIRST FLOOR 1,126 SF GARAGE 431 SF TWO-STORY HOUSE: 1,777 SF @ 16 = 28,432 FIRST FLOOR 587 SF SECOND FLOOR 757 SF GARAGE 433 SF 59,572 SF | ARCHITECTURAL A1.1 SITE PLAN A1.2 SITE DETAILS A2.1 FLOOR PLANS - SINGLE STORY A2.2 FLOOR PLANS - TWO STORY A3.0 PROPOSED EXTERIOR MATERIALS & COLORS A3.1A EXTERIOR ELEVATIONS - SINGLE STORY A3.1B EXTERIOR ELEVATIONS - SINGLE STORY A3.2C EXTERIOR ELEVATIONS - SINGLE STORY A3.2B EXTERIOR ELEVATIONS - TWO STORY A3.2B EXTERIOR ELEVATIONS - TWO STORY A3.2C EXTERIOR ELEVATIONS - TWO STORY |
| ABBREVIATIONS | SYMBOLS | VICINITY MAP | |
| PERPENDICULAR DET. DETAIL H.M. HOLLOW METAL OFF. OFFICE | DETAIL NUMBER S. S. STANLESS STEAL S. STANLESS STEAL S. STANLESS STEAL S. S. STANLESS STEAL S. S. STANLESS STEAL S. S. STANLESS STEAL S. STANLESS STEAL S. STANLESS STEAL S. STANLESS STEAL S. STANDARD S. SECTION KEY SECTION NUMBER S. SECTION NUMBER SECTION NUMBER S. SECTION NUMBER S. SECTION NUMBER SECTION NUMBER S. SECTION | PROJECT LOCATION | |
| C.B. CATCH BASIN F.H.W.S. FLAT HEAD WOOD SCREW M.C. MEDICINE CABINET RESIL. RESILLENT C.C.B.R. CLOSED CELL BACKER ROD FLASH. FLASHING M.H. MAN HOLE R.H.M.S. ROUND HEAD M.H. MAN HOLE R.H.M.S. ROUND HEAD M.H. MAN HOLE R.H.M.S. ROUND HEAD M.H. MECHANICAL R.H.W.S. ROUND HEAD M.H. MEMBANIE R.M. ROOM M.H. MEMBANIE R.M. ROOM M.H. MEMBANIE R.M. ROOM M.H. MEZZ. MEZZANINE R.O. ROUGH OPIC M.H. MECHANICAL R.H.W. ROOM M.H. MEMBANIE R.M. ROOM M.H. MINIMUM R.S. RESAWN | FAD METAL SCREW FAD WOOD WOOD FOR MUNDOW FOR LEADER FOR WILL FOR W | | |

Project / Owner:

People's Self Help Housing

296 APPLE AVENUE GREENFIELD, CA

A.P.N.: 109-082-013-000





The Paul Davis Partnership, LLP 286 Eldorado Street Monterey, CA 93940 (831) 373-2784 FAX (831) 373-7459 EMAIL: info@pauldavispartnership.com



Drawn By: A

Drawing Date: 7/12/2

Project Number: 2107

Revisions:

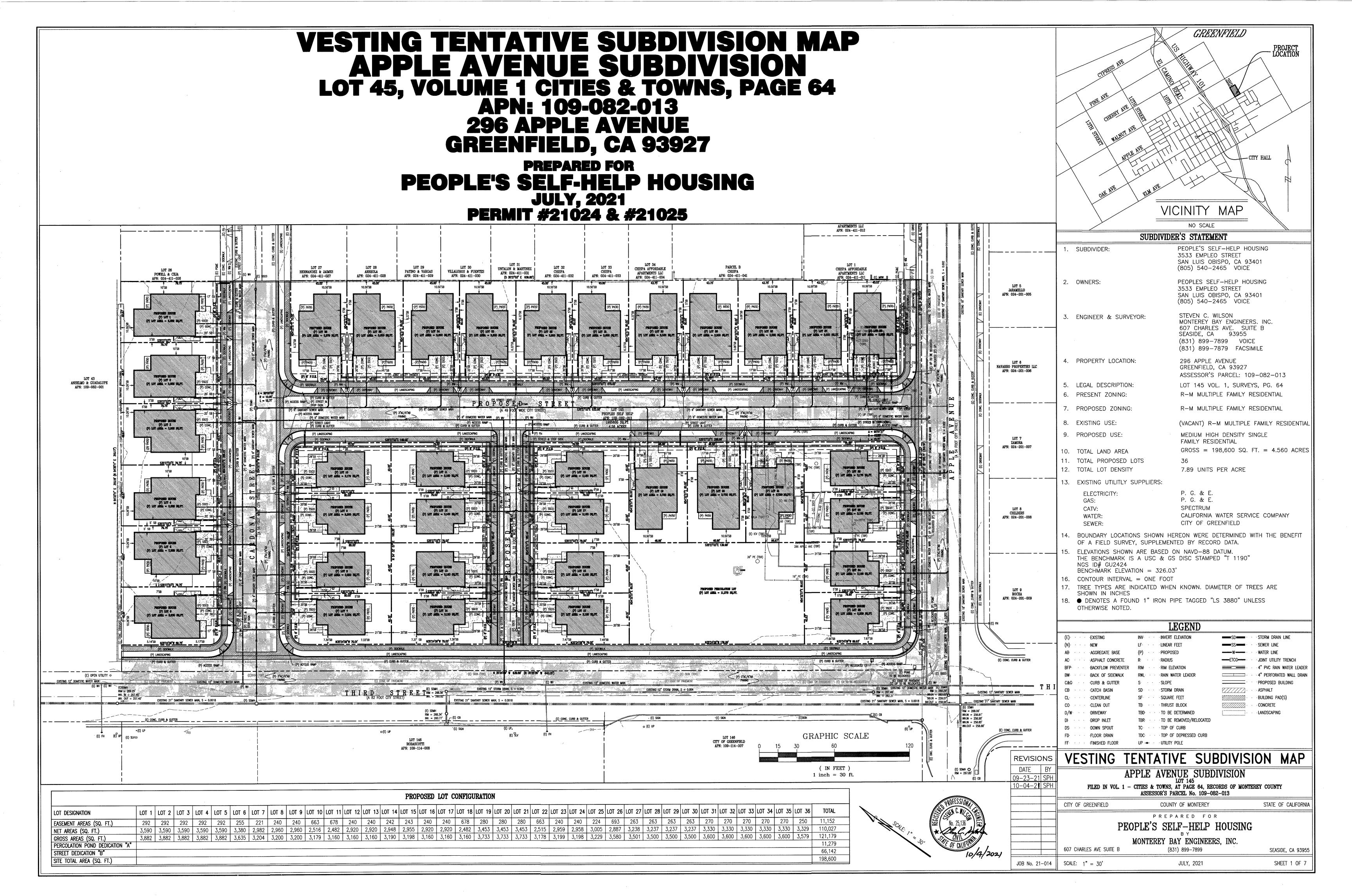
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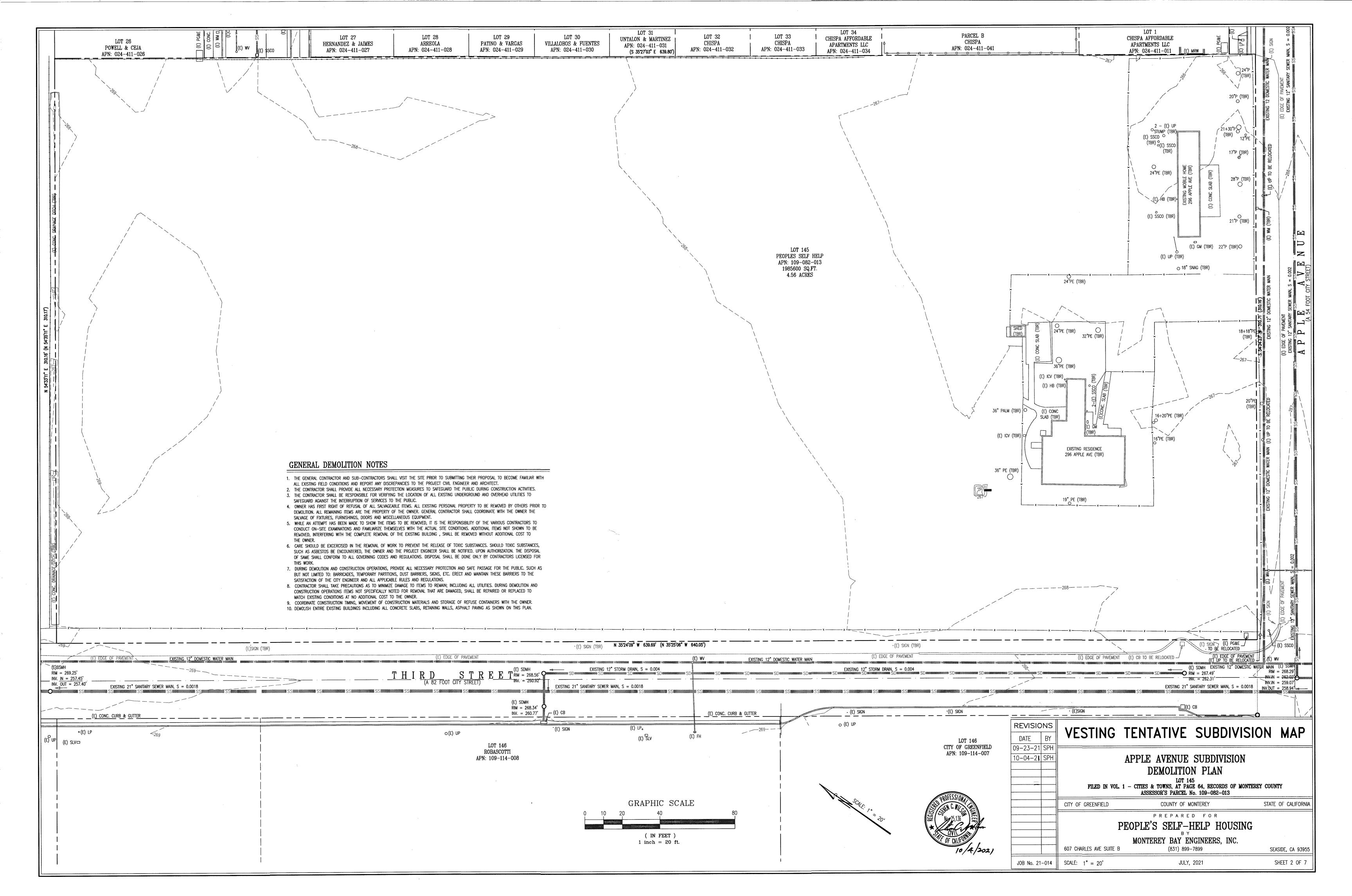


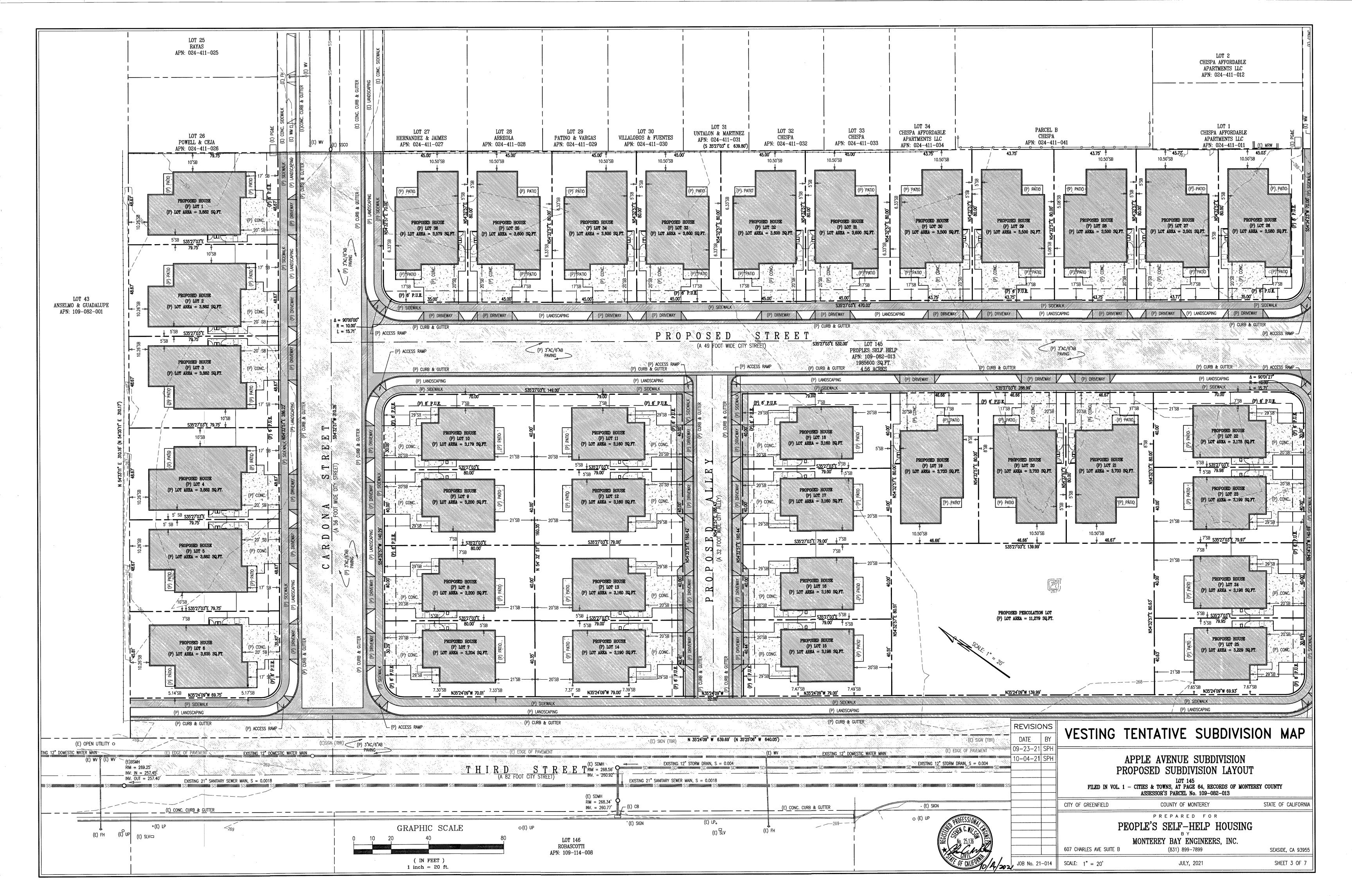
TITLE SHEET
ABBREVIATIONS
PROJECT INFO.

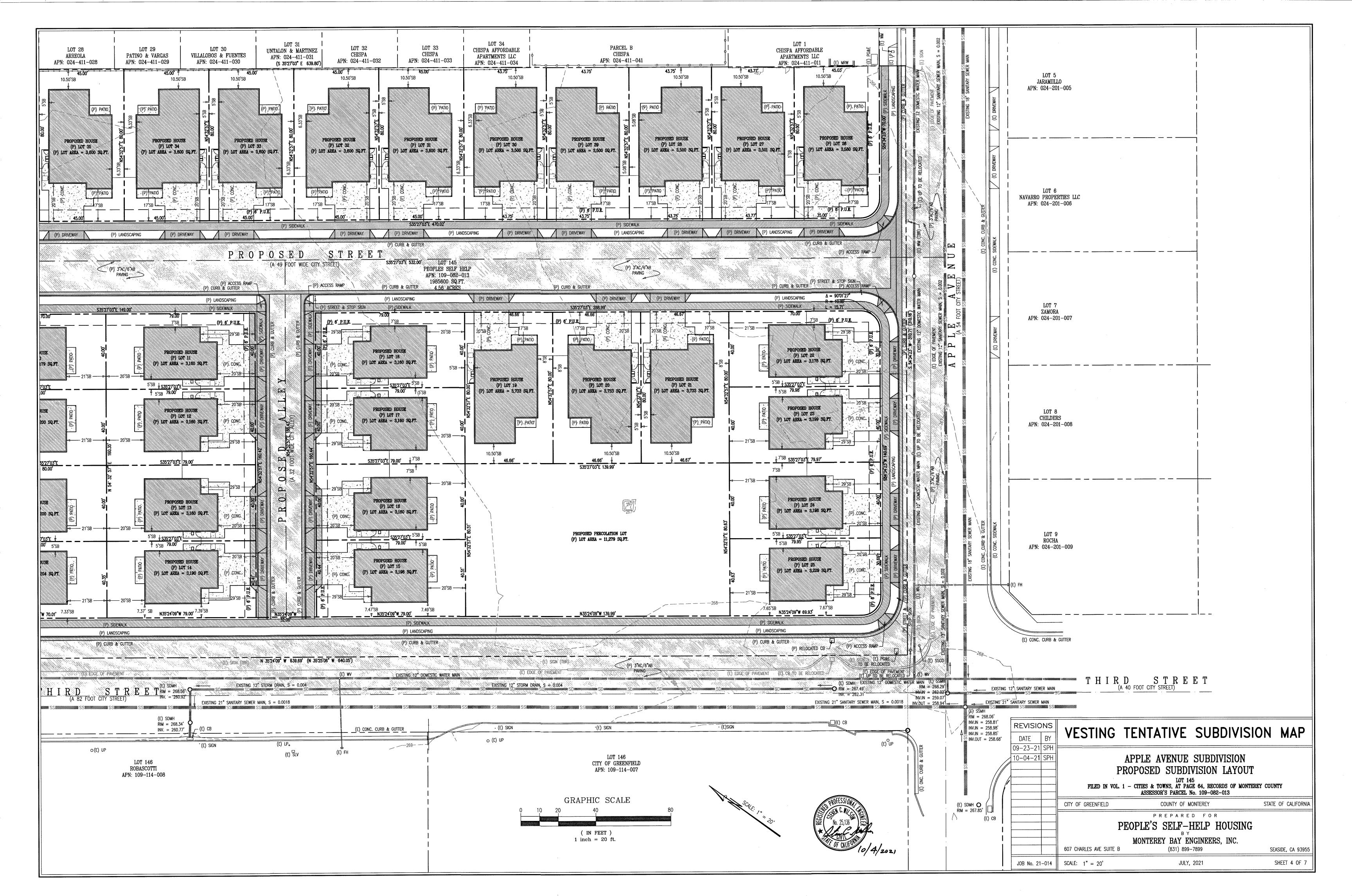
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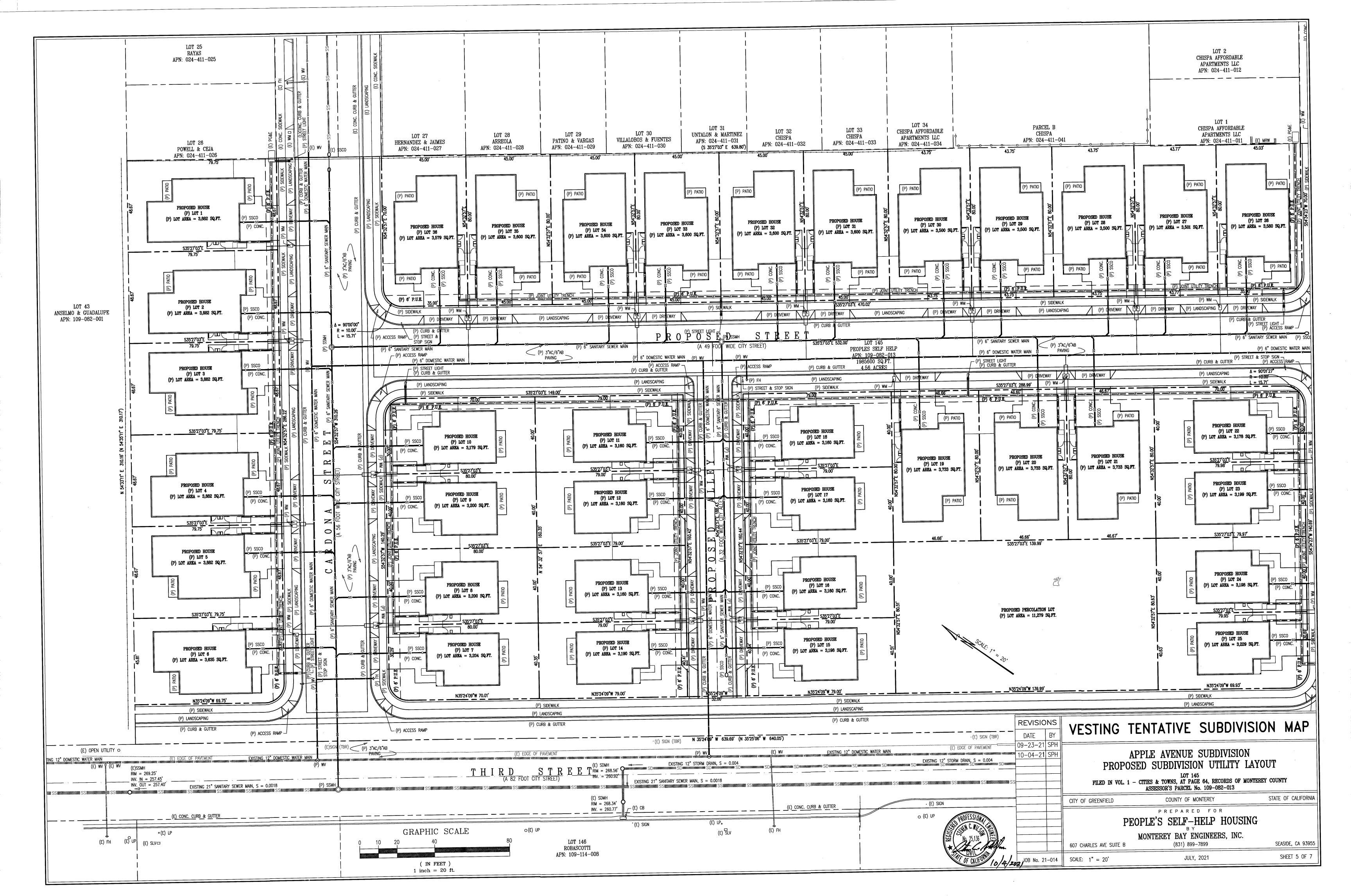
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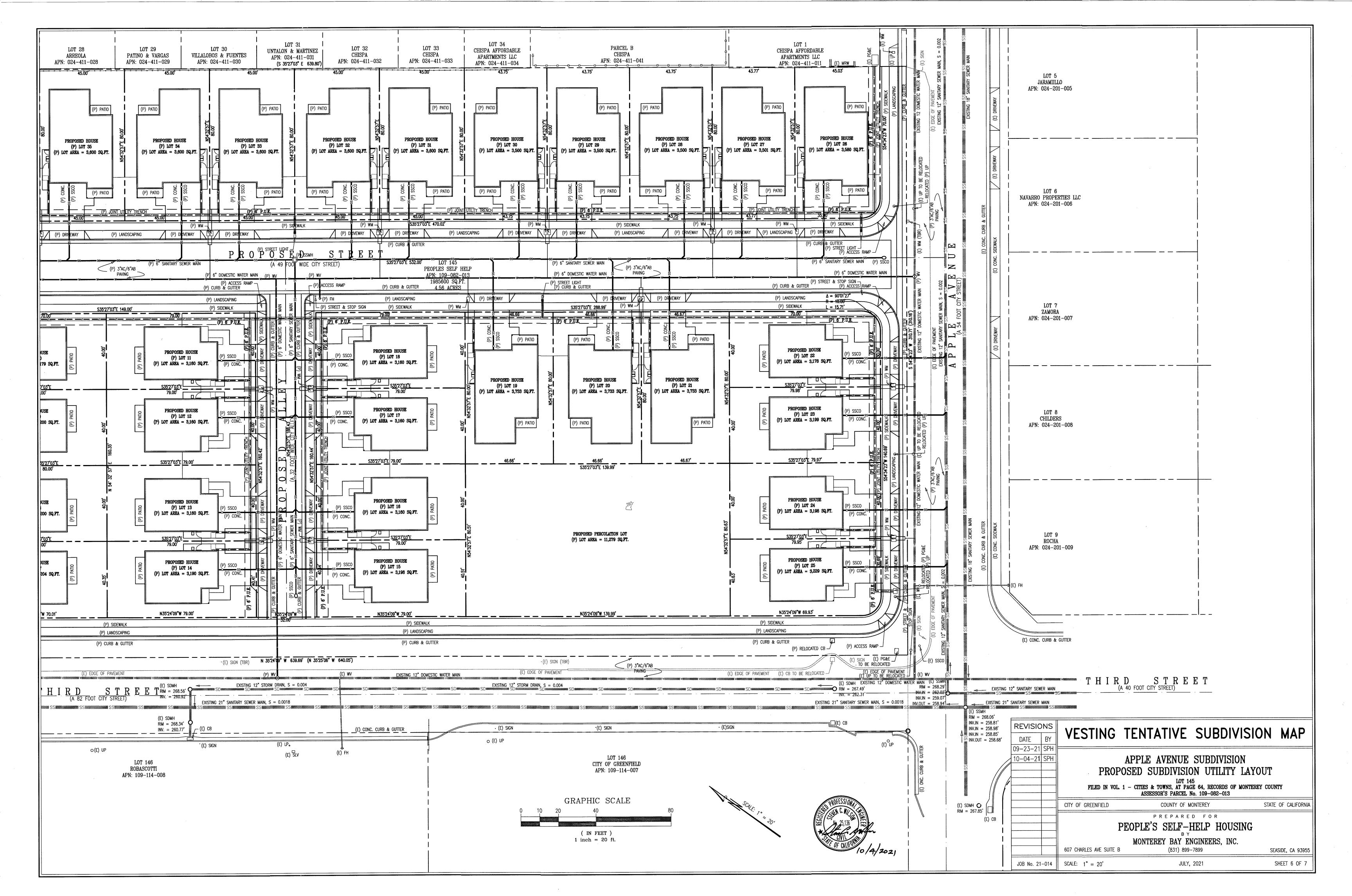




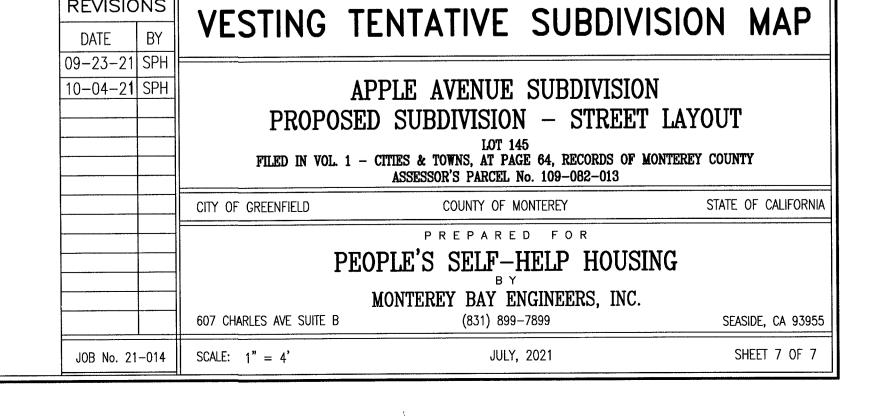


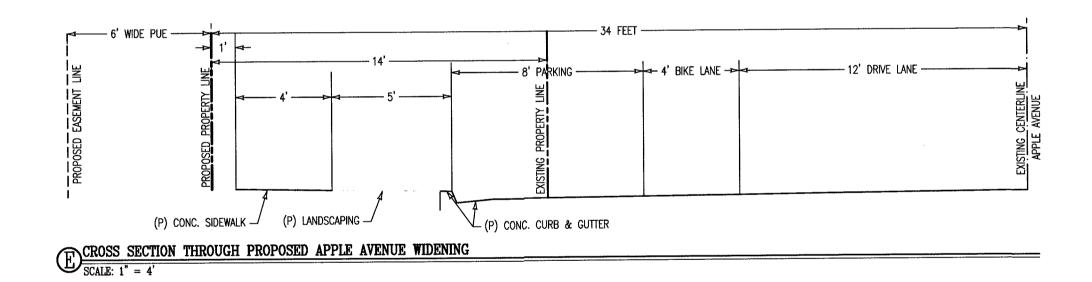


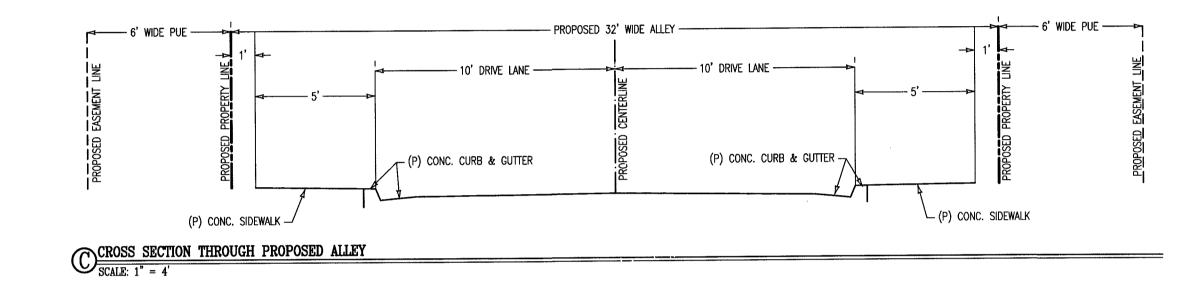


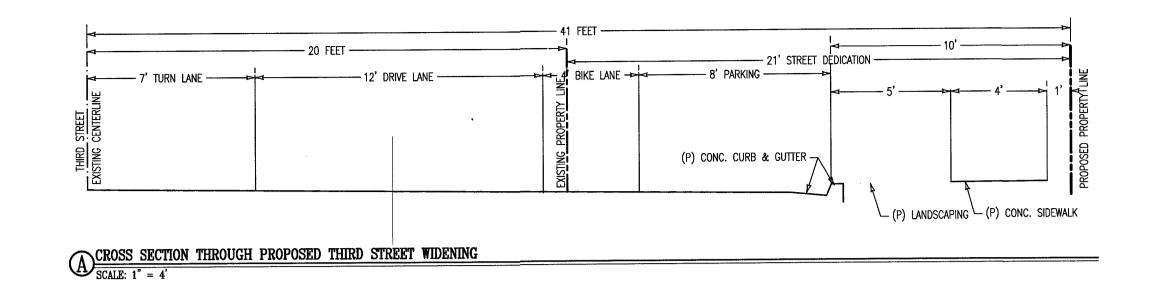


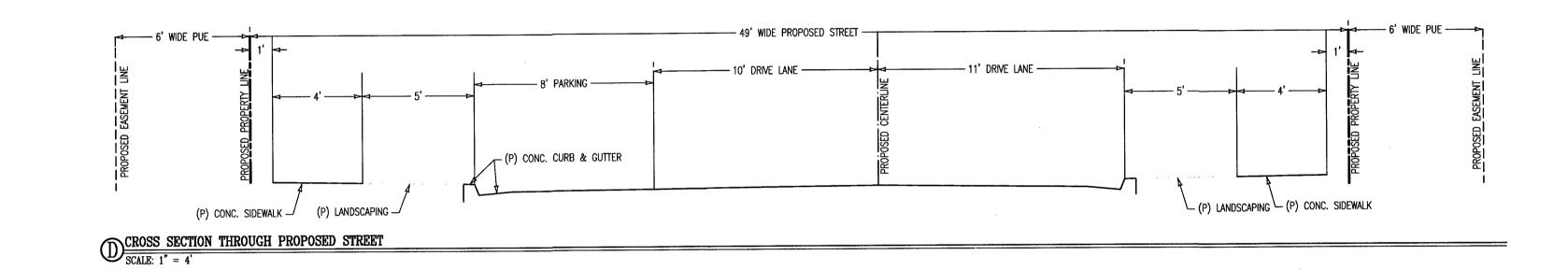


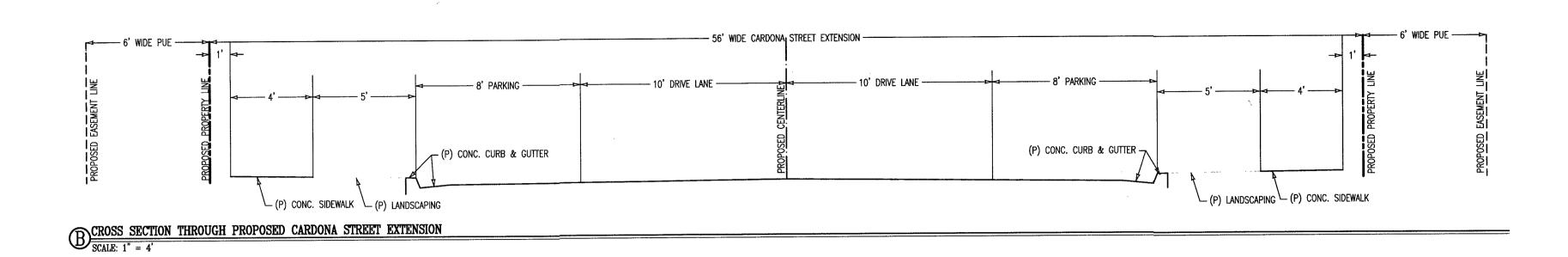












LOT NUMBER

DESIGN ANALYSIS

PLANNED DEVELOPMENT IS PROPOSED WITH PD - SPECIFIC DEVELOPMENT STANDARDS FOR MINIMUM LOT SIZE, MINIMUM LOT WIDTH AND DEPTH, BUILDING SETBACKS (FRONT, SIDE, STREET-SIDE, AND REAR), AND ANY OTHER PERTINENT STANDARDS THAT REQUIRE THE PROPOSED PLANNED DEVELOPMENT.

ZONING: R-M (MULTI-FAMILY RESIDENTIAL (7 to 15 DU/AC)

PARCEL: 109-082-013-000 SITE AREA: 4.6 AC (198,400 SF)

MINIMUM DENSITY: 7 DU/AC

MAX. DENSITY:

15 DU/AC LOT COVERAGE: 60% REQUIRED - SEE TABLE BELOW People's Self Help Housing

296 APPLE AVENUE

GREENFIELD, CA

Project / Owner:

A.P.N.: 109-082-013-000

| | | | LOTS & | BUILDIN | G TYPE | S BREA | KDOW | N | | | | |
|---|--------------|---|-------------|---------------|---------------------------------|------------------------------------|------------------------|-----------------------------|-----------------|-----------------|---------------------------------------|----------|
| Т | SIZE (SF) | Н | OUSE TYPE | COVERED PATIO | HOUSE AREA (SF) 1st Floor | HOUSE AREA (SF) 2nd Floor | GARAGE AREA (SF) | TOTAL FLOOR AREA (SF) | LOT COVERAGE | | | |
| | 3,882 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 41.9 | | | |
| | 3,882 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 41.9 | | | |
| | 3,882 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 41.9 | | | |
| | 3,882 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 41.9 | | | |
| | 3,882 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 41.9 | - | TLIE | |
| | 3,635 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 44.8 | | THE | |
| | 3,204 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 32.8 | PAU | | |
| | 3,200 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 32.9 | | | 7 T T |
| | 3,200 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 32.9 | PART | NEKS. | HI |
|) | 3,179 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 33.1 | - | TS & PLAN | |
| | 3,160 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 33.3 | AKCIIILC | 15 G FLAI | VI VIL I |
| 2 | 3,160 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 33.3 | | _ = _ | |
| 3 | 3,160 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 33.3 | | | |
| Ļ | 3,190 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 33.0 | | | |
| ; | 3,198 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 32.9 | The Paul D | avis Partnership, | LLP |
| ; | 3,160 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 33.3 | 286 1 | Eldorado Street | |
| , | 3,160 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 33.3 | | terey, CA 93940 '84 FAX (831) 373- | 7450 |
| 3 | 3,160 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 33.3 | | oauldavispartnersi | |
|) | 3,733 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 43.6 | 1 ' | , | , |
|) | 3,733 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 43.6 | - | | |
| | 3,733 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 43.6 | - | | |
| 2 | 3,178 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 33.1 | | | |
| 3 | 3,199 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 32.9 | - | | |
| ļ | 3,198 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 32.9 | | | |
| 5 | 3,229 | Α | 3BR/2 STORY | 32 | 587 | 757 | 433 | 1,777 | 32.6 | | | |
| 3 | 3,580 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 45.4 | | | |
| 7 | 3,501 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 46.5 | | | |
| 3 | 3,500 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 46.5 | | | |
|) | 3,500 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 46.5 | | | |
|) | 3,500 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 46.5 | | | |
| | 3,600 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 45.2 | | | |
| 2 | 3,600 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 45.2 | | _ | |
| 3 | 3,600 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 45.2 | | • | |
| ļ | 3,600 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 45.2 | | | |
| 5 | 3,600 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 45.2 | Drawn By: | AC | |
| 6 | 3,579 | В | 3BR/1 STORY | 70 | 1,126 | | 431 | 1,557 | 45.5 | 1 | | |
| | | | | 1,912 | 31,912 | 12,112 | 15,548 | 59,572 | | Drawing Date: | 7/12/2021 | |
| | | ı | | ı | ı | <u> </u> | 1 | ı | ı | Project Number: | 2107 | |

VIS HIP VERS

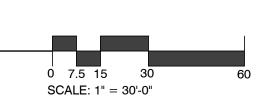
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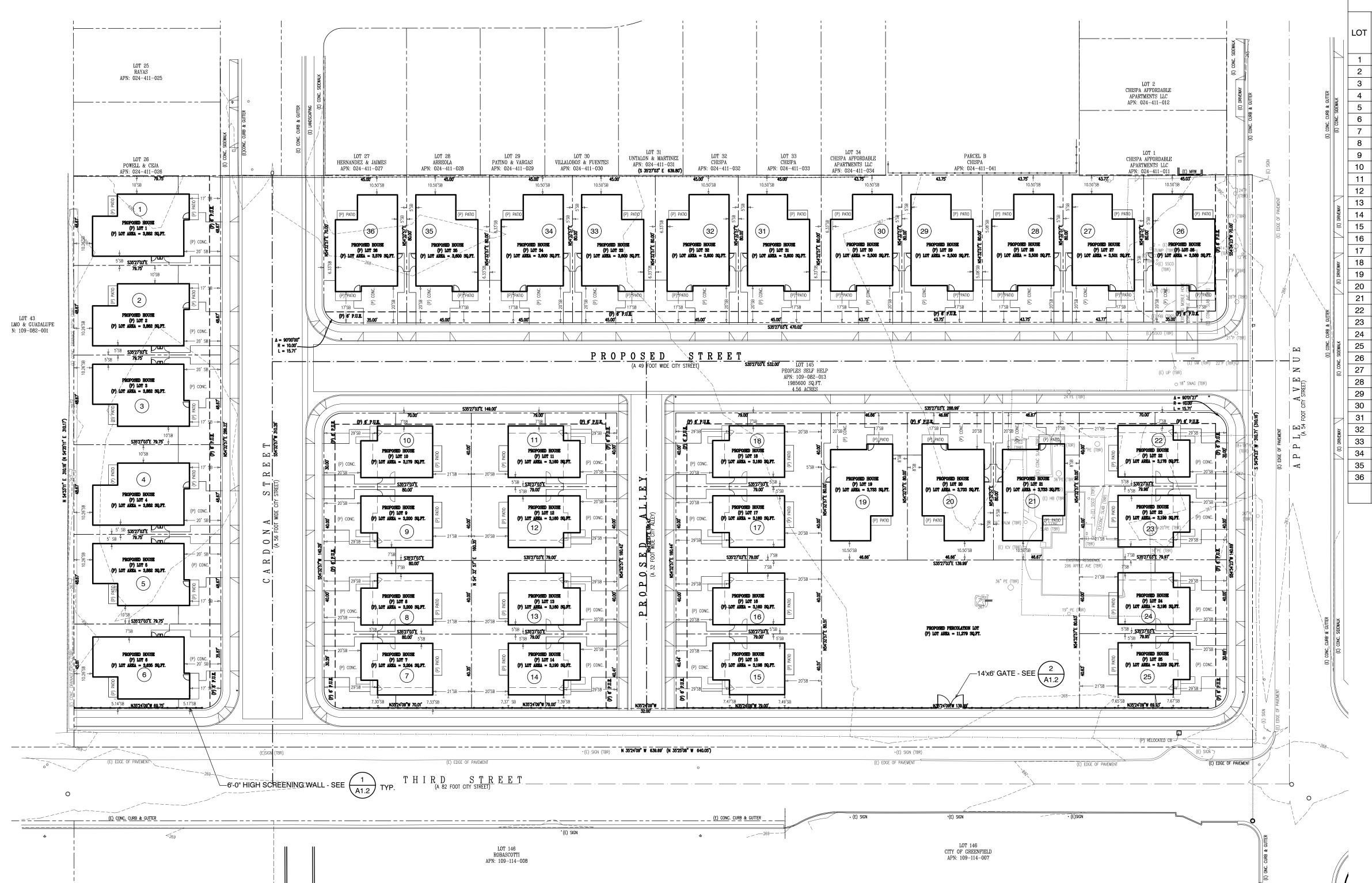
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Sheet Title: SITE PLAN

Sheet Number:









The Paul Davis Partnership, LLP 286 Eldorado Street Monterey, CA 93940 (831) 373-2784 FAX (831) 373-7459 EMAIL: info@pauldavispartnership.com

Drawn By: Drawing Date:

Project Number: 2107

Revisions:

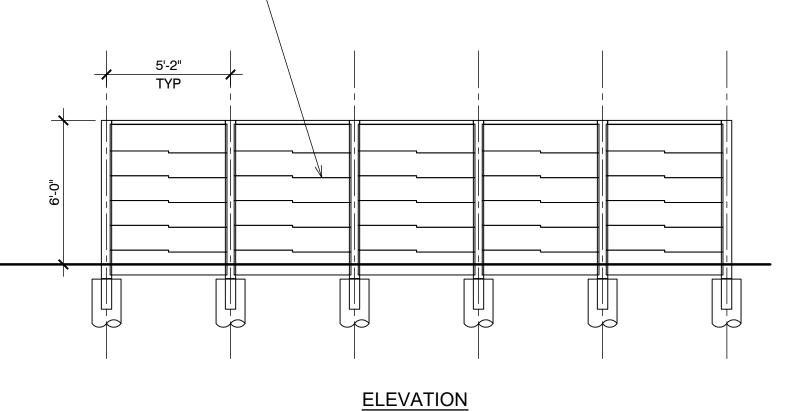
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Sheet Title: SITE DETAILS

Sheet Number:

296 APPLE AVENUE GREENFIELD, CA

CONCRETE PANEL FENCE -5'-2" TYP



SCREENING CONCRETE WALL ELEVATION AND SECTION NO SCALE

POST BEYOND

FINISHED GRADE -

DBL. #3 TIES @ T&B OF— POST AS SHOWN

1'-6"

SECTION - A

#3 TIES @ 12" o.c. -

GENERAL NOTES

INSTALLATION

1. ALL DIMENSIONS ARE TYPICAL AND MAY BE VARIED AT

THE RECOMMENDATION O F THE MANUFACTURER AT

2. HOG RINGS SHALL BE GALVANIZED OR ALUMINUM ALLOY

3. THE TENSION WIRE SHALL BE EITHER NO. 7 GAUGE STEEL WIRE GALVANIZED AT THE RATE PF 0.7 PER SQ. FT. MIN. OR ALUM. WIRE OF ALLOY ALCLAD 5056-H38 OR EQUAL

WITH A WIRE DIAMETER OF 0.1875 OR LARGER

5. FENCE FABRIC SHALL BE WOVEN IN 1-3/4 INCH MESH

FROM N0.9 GAGE STEEL BLACK CONFORMING TO THE

"STANDARD SPECIFICATION FOR ZINC-COATED STEEL

WOVEN INTO THE FENCE FABRIC AFTER ERECTION.

BLACK (ASTM A392). BLACK OR BROWN SLATS SHALL BE

SLATS SHALL BE PRIVACY LINK OR APPROVED EQUAL AND

4. ALL TUBULAR POST TO HAVE METAL CAP

SHALL BE BOTTOM LOCKING SLATS

14'-0" ___TIE WIRES ___STRETCHER — METAL CAP @ 12" o.c. BARS — Π [] PLUNGER BAR LATCH w/ 4x POST DIA. PADLOCK VARIES PER 4x POST DIA. 4x POST DIA. LENGTH OF PLUNGER CATCH

1. THE CONTRACTOR SHALL PROVIDE A SUITABLE METHOD TO VISUALLY ASSURE OBTAINING 2 INCHES CLEARANCE BETWEEN POST AND BOTTOM OF CONCRETE.

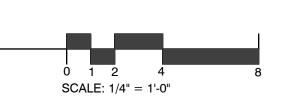
2. STRETCHER BARS FOR GATES ARE REQUIRED ON BOTH SIDES OF EACH GATE. STRETCHER BARS (SIZE 3/16"x3/4") MIN. ANCHOR WITH BANDS NOT EXCEEDING 12" SPACING OR OTHER APPROVED ANCHOR.

> CHAIN LINK FENCE GATE DETAIL SCALE: N.T.S.

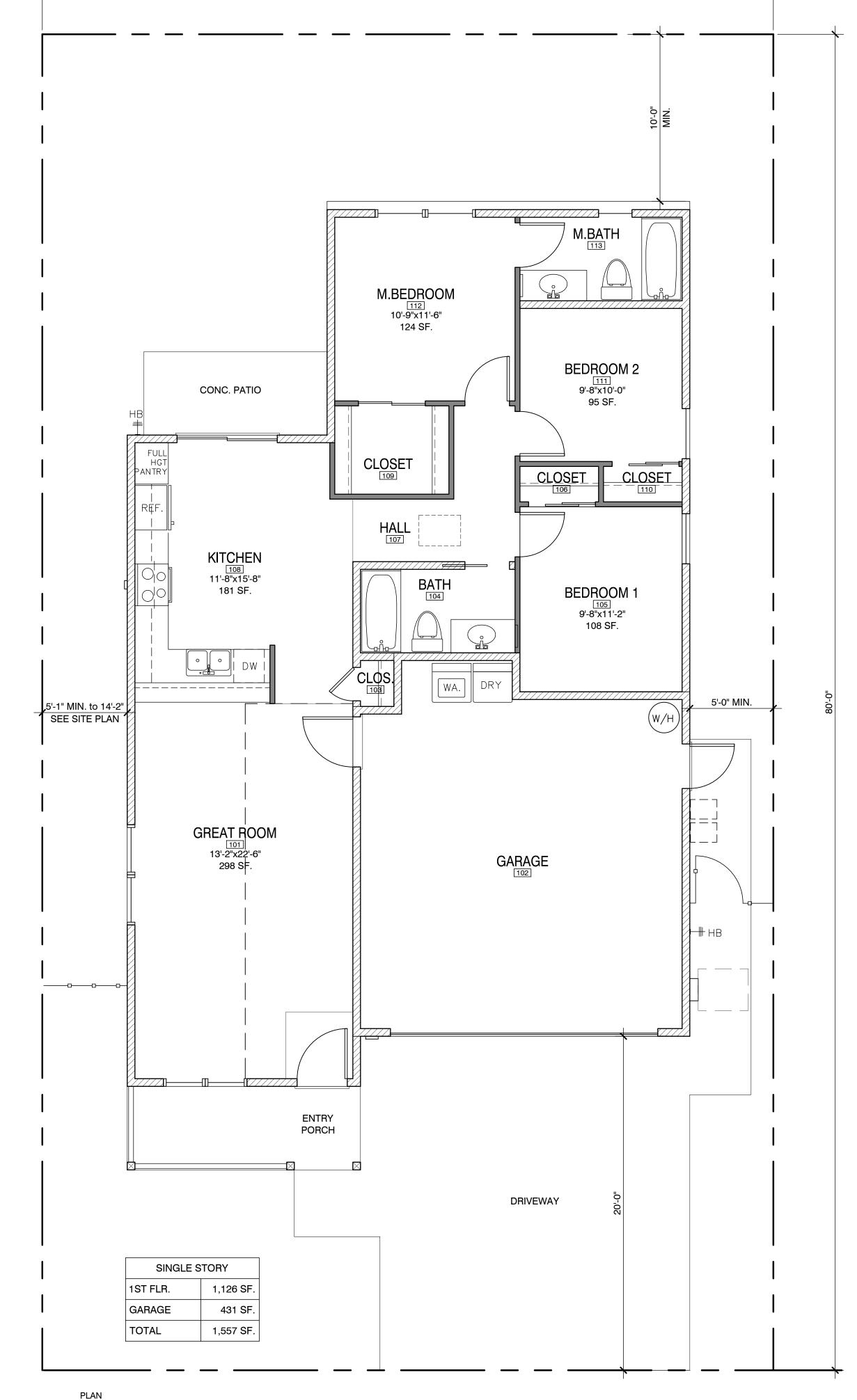
CHAINLINK FENCE GATE DETAIL

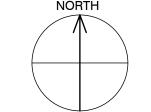
WA. DRY





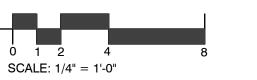
43'-9" TO 52'-10"





FLOOR PLAN - SINGLE-STORY

SCALE: 1/4" = 1'-0"



Project / Owner:

People's Self Help Housing

296 APPLE AVENUE GREENFIELD, CA

A.P.N.: 109-082-013-000





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PROPOSED FLOOR PLAN -SINGLE STORY

Sheet Number:







Drawing Date: 7/12/

Project Number: 2107

Revisions:

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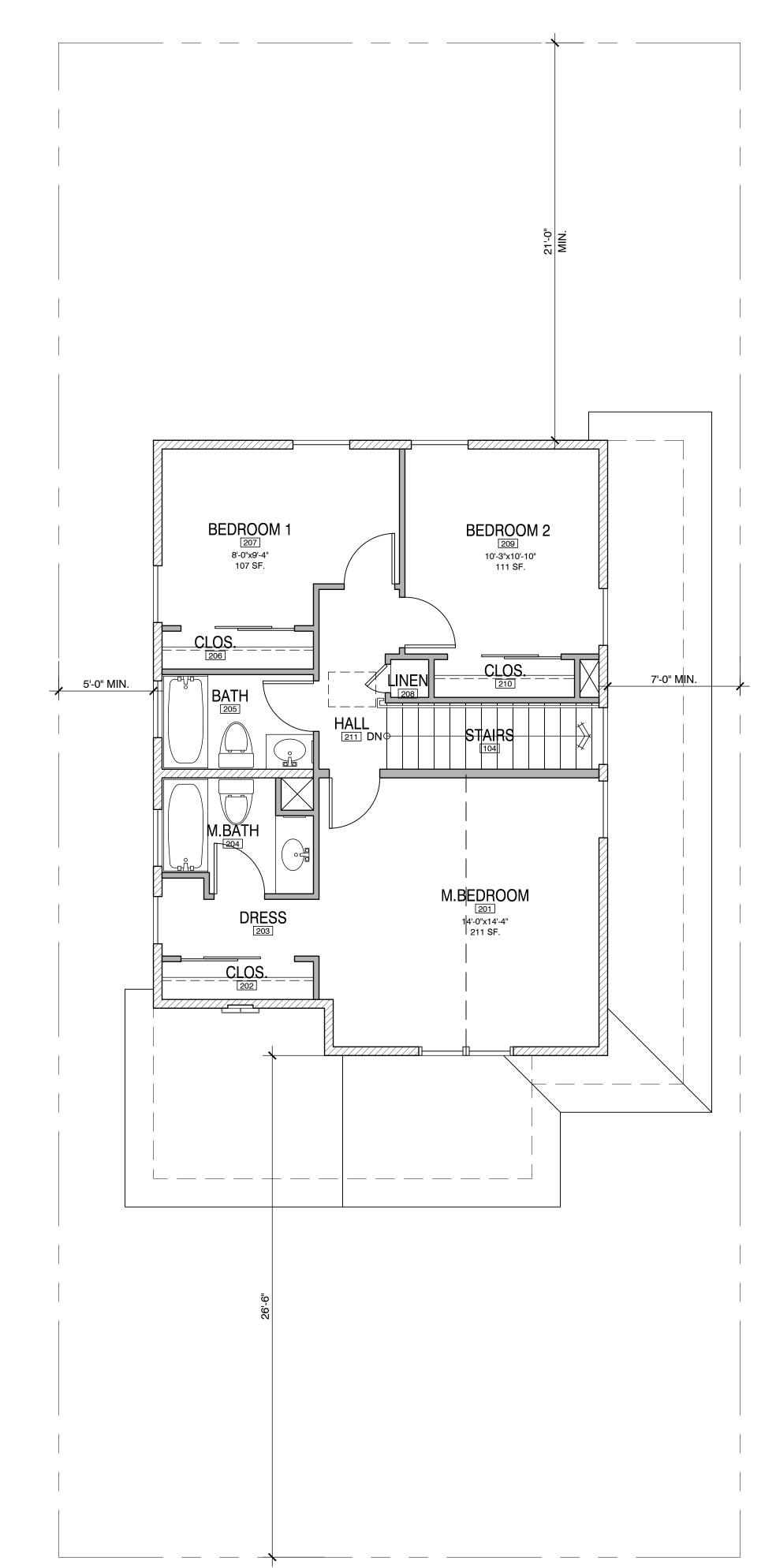


PROPOSED FLOOR PLANS -TWO-STORY

Sheet Number:

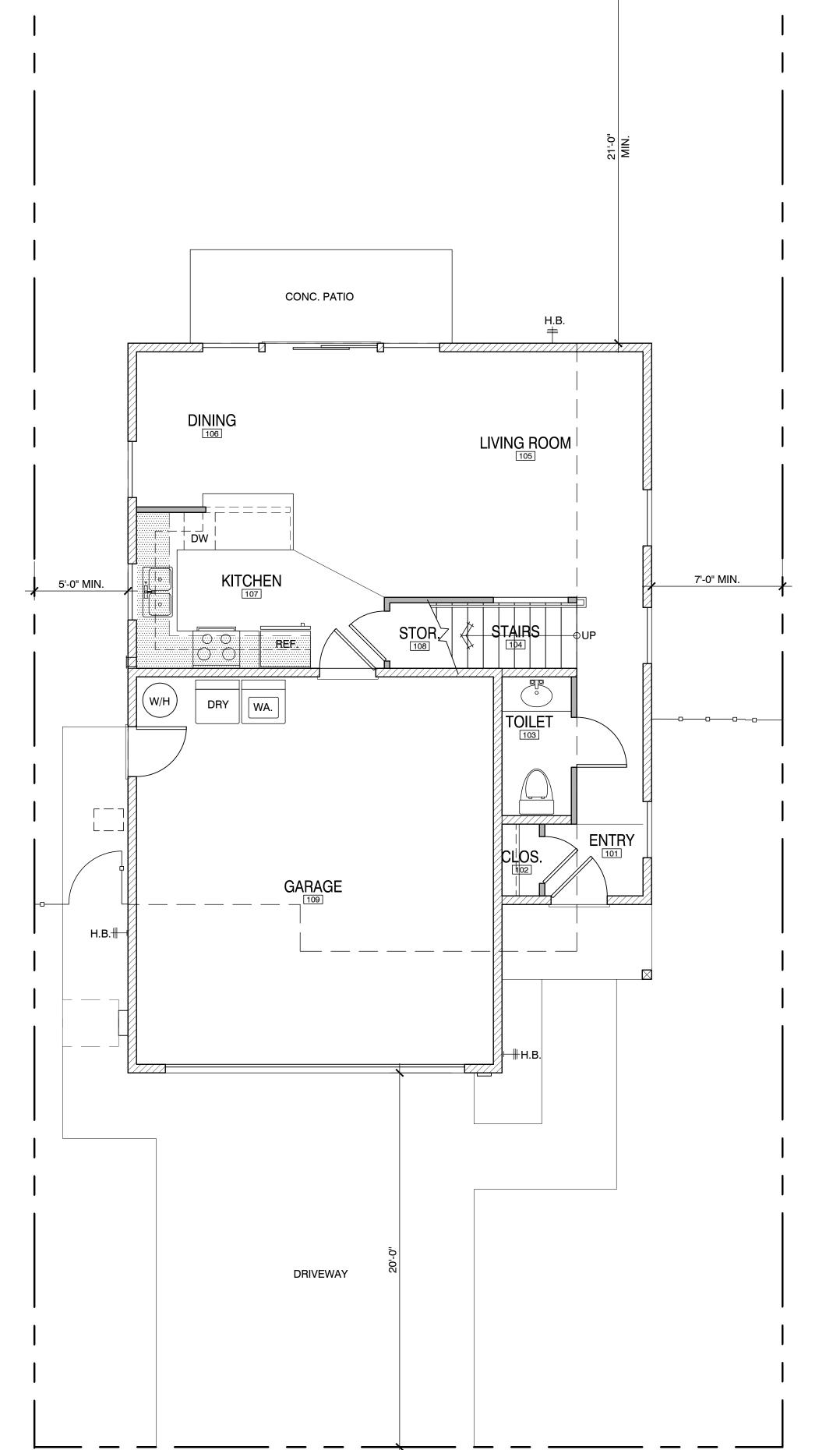
SCALE: 1/4" = 1'-0"





| TWO-ST | ORY |
|----------|-----------|
| 1ST FLR. | 587 SF. |
| 2ND FLR. | 757 SF. |
| GARAGE | 433 SF. |
| TOTAL | 1,777 SF. |





People's Self Help Housing

296 APPLE AVENUE GREENFIELD, CA

A.P.N.: 109-082-013-000





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Project Number:

Revisions:

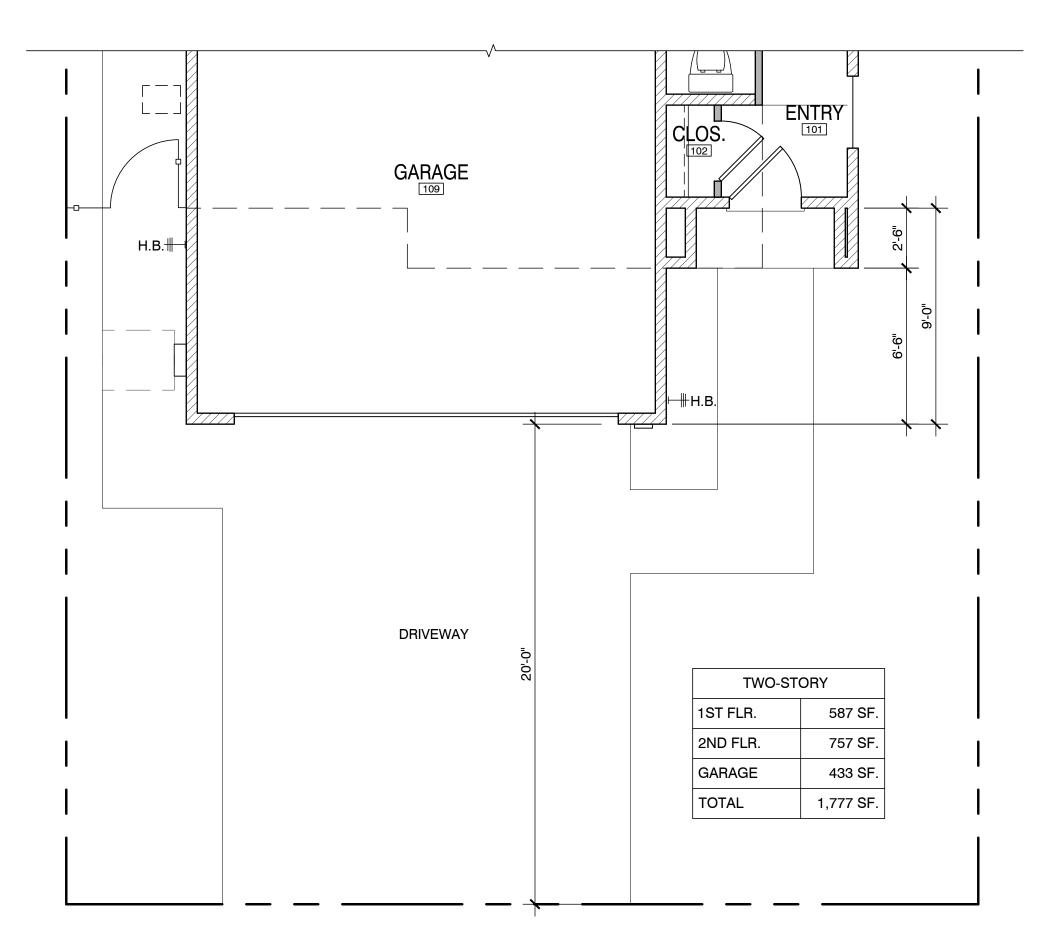
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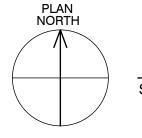


PROPOSED FLOOR PLANS -TWO-STORY

Sheet Number:

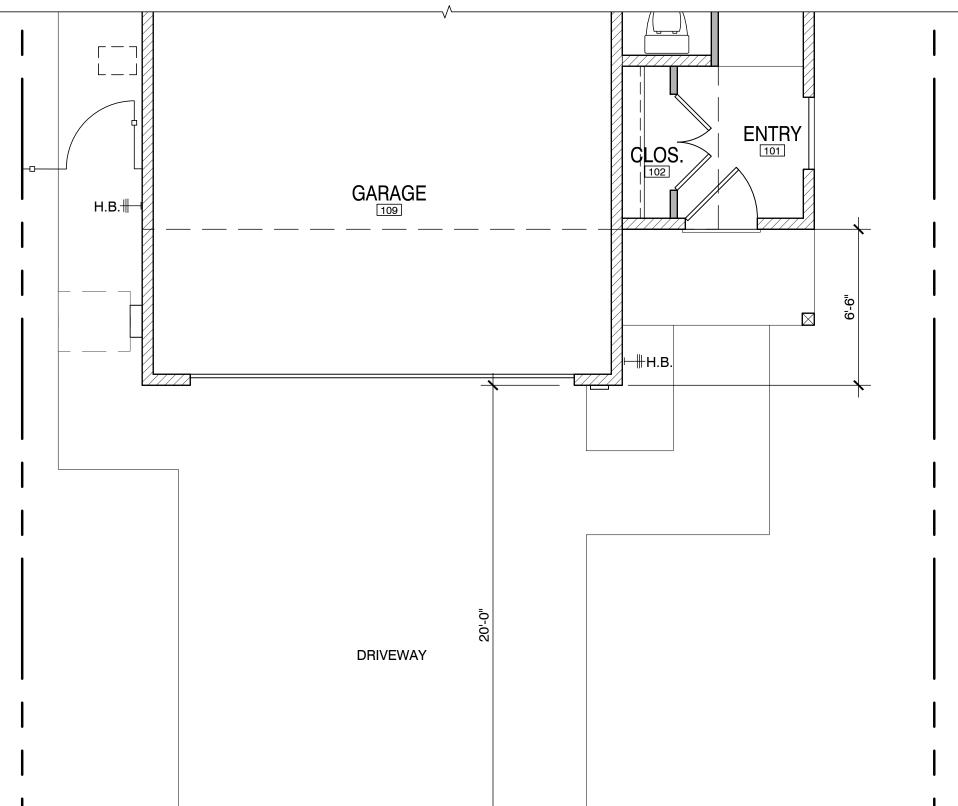
SCALE: 1/4" = 1'-0"

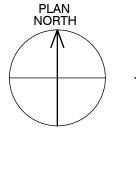


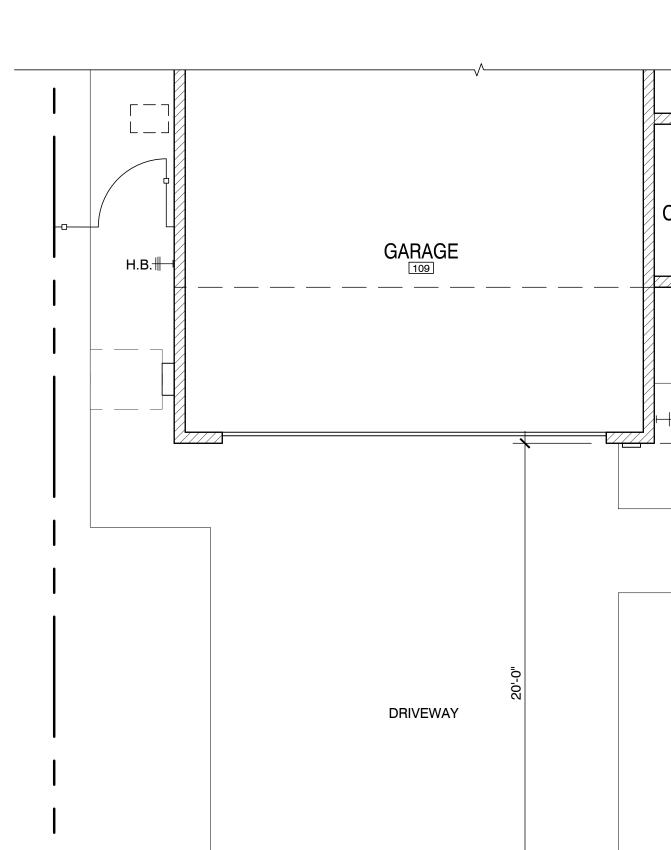


FIRST FLOOR PLAN - TWO-STORY @ ALT. B

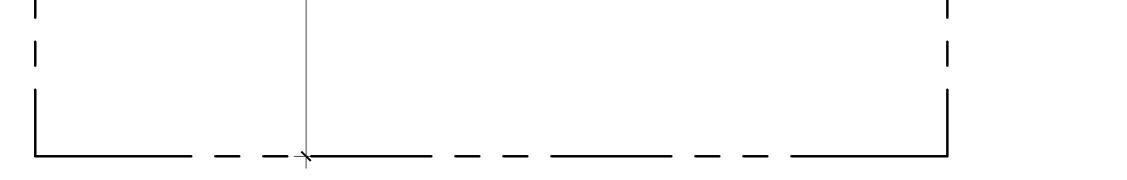
SCALE: 1/4" = 1'-0"







| TWO-ST | ORY |
|----------|-----------|
| 1ST FLR. | 607 SF. |
| 2ND FLR. | 780 SF. |
| GARAGE | 433 SF. |
| TOTAL | 1,820 SF. |



M.BEDROOM

201 14'-0"x14'-4" 255 SF.

M.BATH

SCALE: 1/4" = 1'-0"



Project Number: 2107

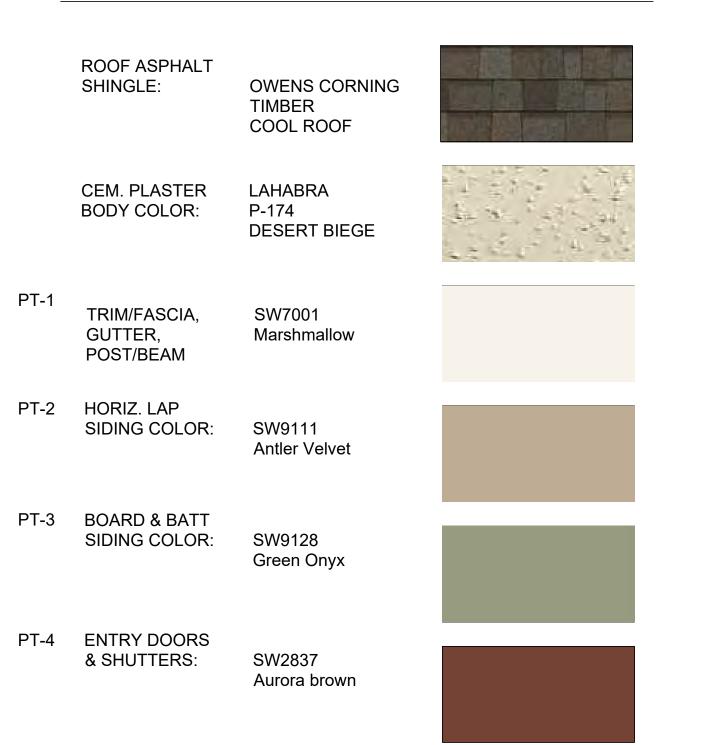
Revisions:



PROPOSED EXTERIOR MATERIALS & COLORS

Sheet Number:

Exterior Material / Color - Scheme # 1



Exterior Material / Color - Scheme # 2

| | ROOF ASPHALT SHINGLE: | OWENS CORNING FOREST BROWN COOL ROOF | |
|------|--------------------------------------|--|--|
| | CEM. PLASTER BODY COLOR: | LAHABRA X-504 BLUE GRAY | |
| PT-1 | TRIM/FASCIA, GUTTER, POST/BEAM | SW7001 Marshmallow | |
| PT-5 | HORIZ. LAP SIDING COLOR: | SW6255 Morning fog | |
| PT-6 | BOARD & BATT SIDING COLOR: | SW6256 Serious gray | |
| PT-7 | ENTRY DOORS & SHUTTERS: | SW6655 Adventure orange | |

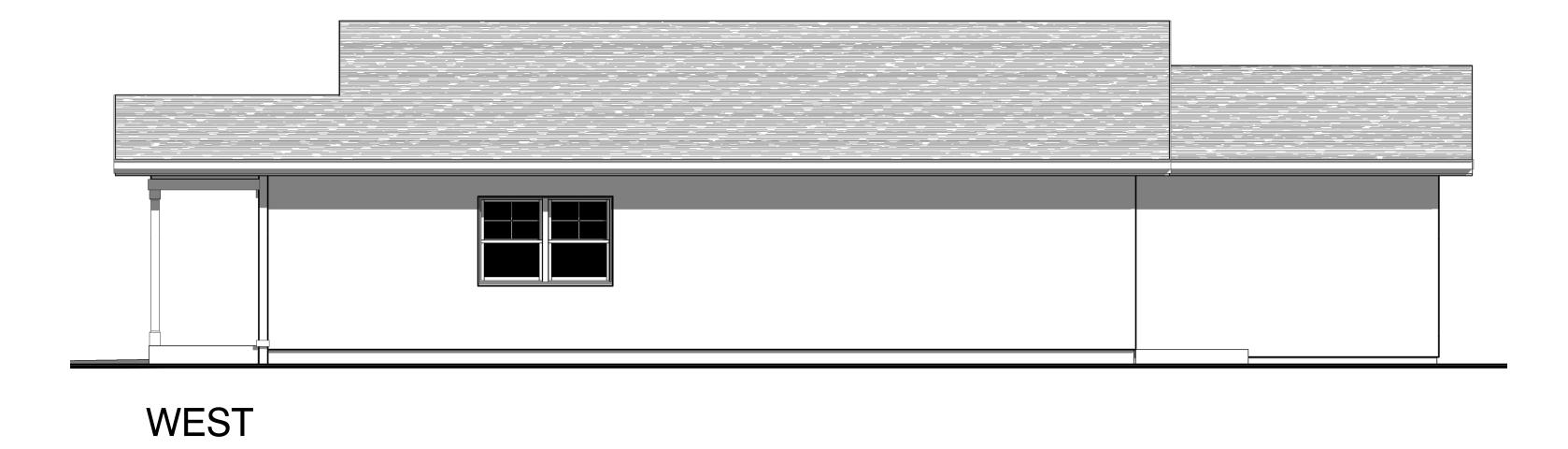
ROOF ASPHALT OWENS CORNING SHINGLE: MOUNTAIN SIDE COOL ROOF LAHABRA X-696 SOUTHERN MOSS CEM. PLASTER BODY COLOR: SW7001 TRIM/FASCIA, GUTTER, Marshmallow POST/BEAM PT-8 HORIZ. LAP SIDING COLOR: SW7713 Tawny tan PT-6 BOARD & BATT SIDING COLOR: SW7715 Pottery Urn PT-7 ENTRY DOORS & SHUTTERS: SW6165 Connected Gray

Exterior Material / Color - Scheme # 3



SCALE: 1/4" = 1'-0"

SOUTH





Class A rated minimum 30-year High Definition asphalt shingle Cool Roof on underlayment per CRC R905.1. Packaging for roof materials shall bear manufacturer's and approved testing agency's labels for field inspection.

FLASHING: 24 GA. Galvanized Sheet Metal, paint all sides prior to installation

and a second coat after installation

General - At a minimum, provide a minimum of one layer of Tyvek StuccoWrap water resistive barrier complying with CRC 703.2. and shall be attached to the studs or sheathing, with flashing as described in the manufacturer's installation instructions, in such a manner as to provide a continuous water-resistive barrier behind the exterior wall veneer/covering.

CEMENT PLASTER: Minimum thickness to be 7/8", three coats on of Grade "D" 60-minute paper with expanded metal lath on Tyvek StuccoWrap on wood sheathing. A minimum 26 GA galvanized metal weep screed with minimum 3-1/2" attachment flange at or below foundation plate is required at 4" minimum above earth and 2"

minimum above finish paving. Comply with CBC chapters 2507 and 2512.

> All exterior wall and surfaces, gutters, downspouts, flashing, trim and exposed concrete foundations shall be painted. 2. Paint all roof jacks, roof caps, dampers and flues to match roof color.

3. Verify head heights of windows to align with the doors.

People's Self Help

Housing 296 APPLE AVENUE GREENFIELD, CA

A.P.N.: 109-082-013-000

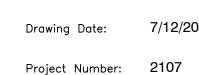




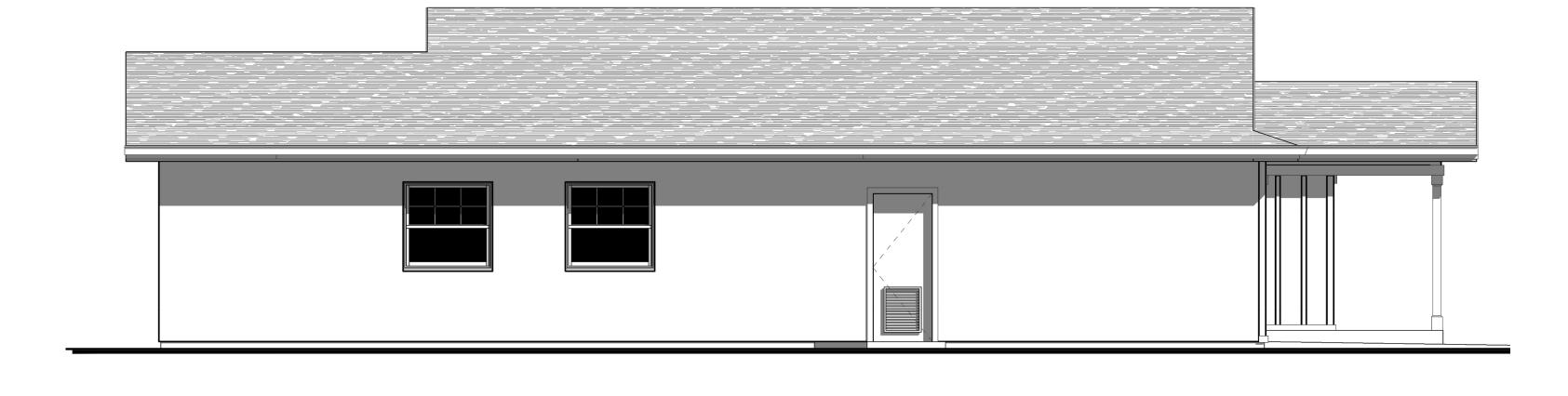
The Paul Davis Partnership, LLP 286 Eldorado Street Monterey, CA 93940 (831) 373-2784 FAX (831) 373-7459 EMAIL: info@pauldavispartnership.com



Drawn By:



Revisions:



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Sheet Title: PROPOSED **EXTERIOR ELEVATIONS-**SINGLE-STORY

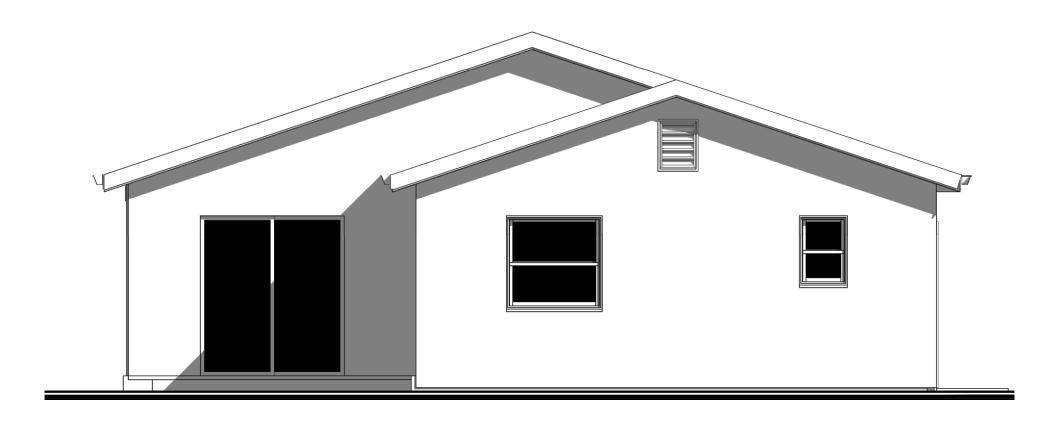
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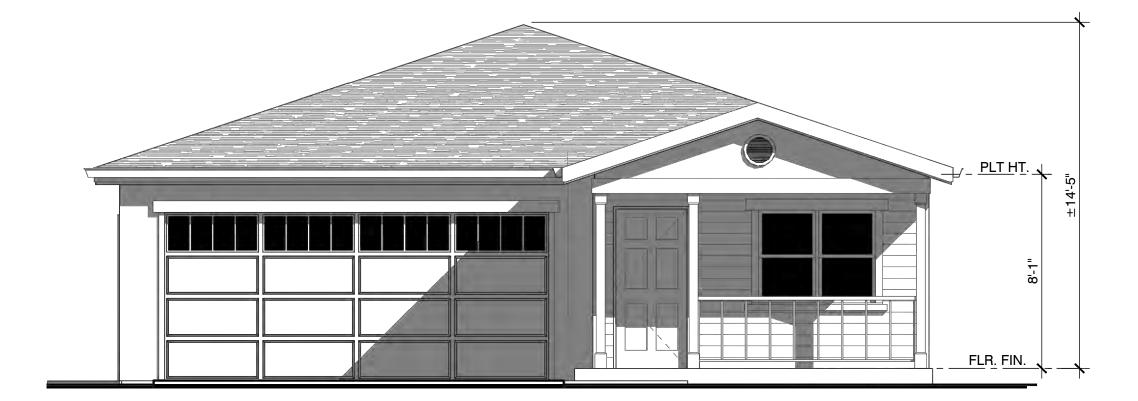
PERSPECTIVE

PROPOSED EXTERIOR ELEVATION - SINGLE-STORY

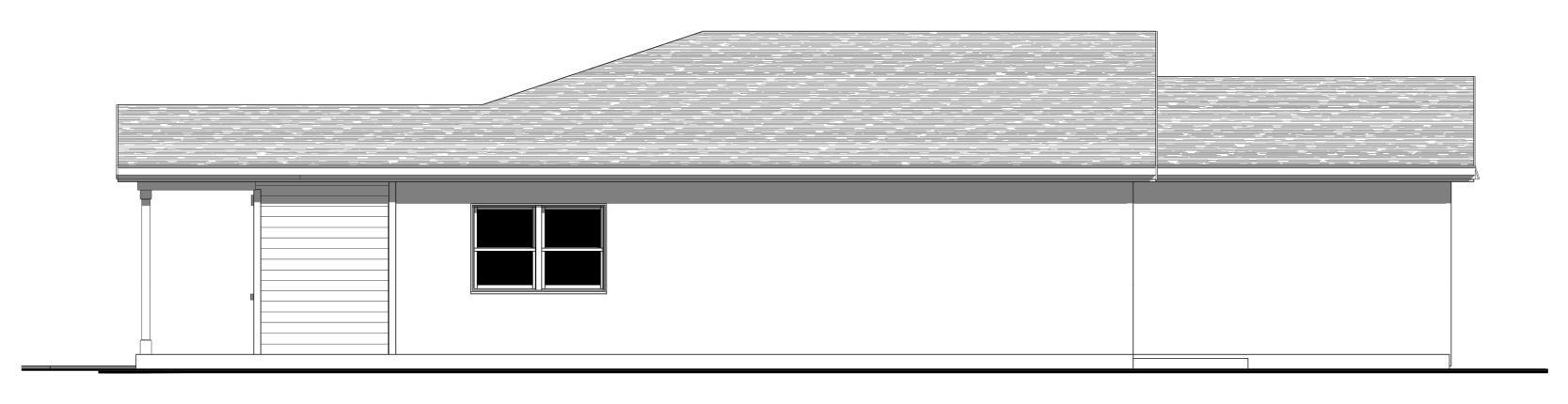
EAST

A3.1A

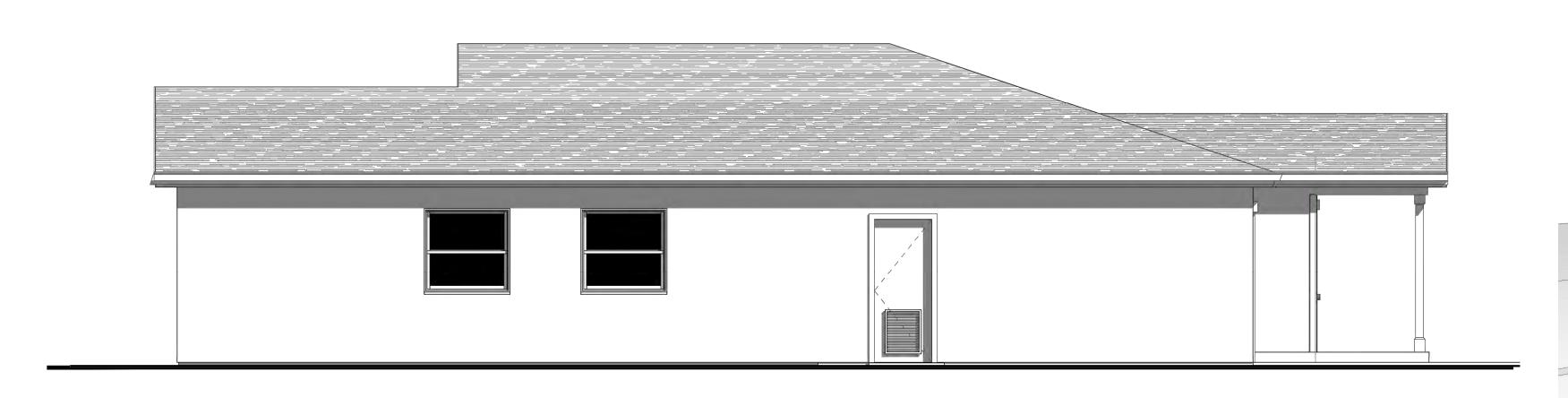




NORTH SOUTH

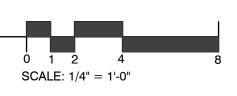


WEST



EAST

PROPOSED EXTERIOR ELEVATION - SINGLE-STORY



EXTERIOR FINISH SCHEDULE

Class A rated minimum 30-year High Definition asphalt shingle Cool Roof on underlayment per CRC R905.1. Packaging for roof materials shall bear manufacturer's and approved testing agency's labels for field inspection.

FLASHING: 24 GA. Galvanized Sheet Metal, paint all sides prior to installation

and a second coat after installation

General - At a minimum, provide a minimum of one layer of Tyvek StuccoWrap water resistive barrier complying with CRC 703.2. and shall be attached to the studs or sheathing, with flashing as described in the manufacturer's installation instructions, in such a manner as to provide a continuous water-resistive barrier behind the exterior wall veneer/covering.

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- 1. All exterior wall and surfaces, gutters, downspouts, flashing, trim and exposed concrete foundations shall be painted.
- 2. Paint all roof jacks, roof caps, dampers and flues to match roof color.
- 3. Verify head heights of windows to align with the doors.

People's Self Help Housing

296 APPLE AVENUE GREENFIELD, CA

A.P.N.: 109-082-013-000





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Project Number:

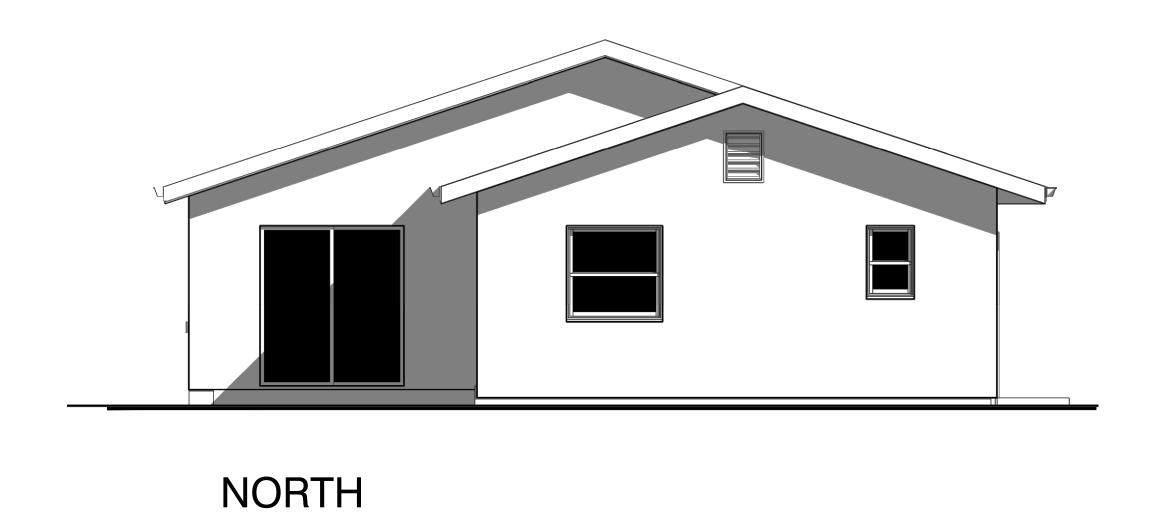
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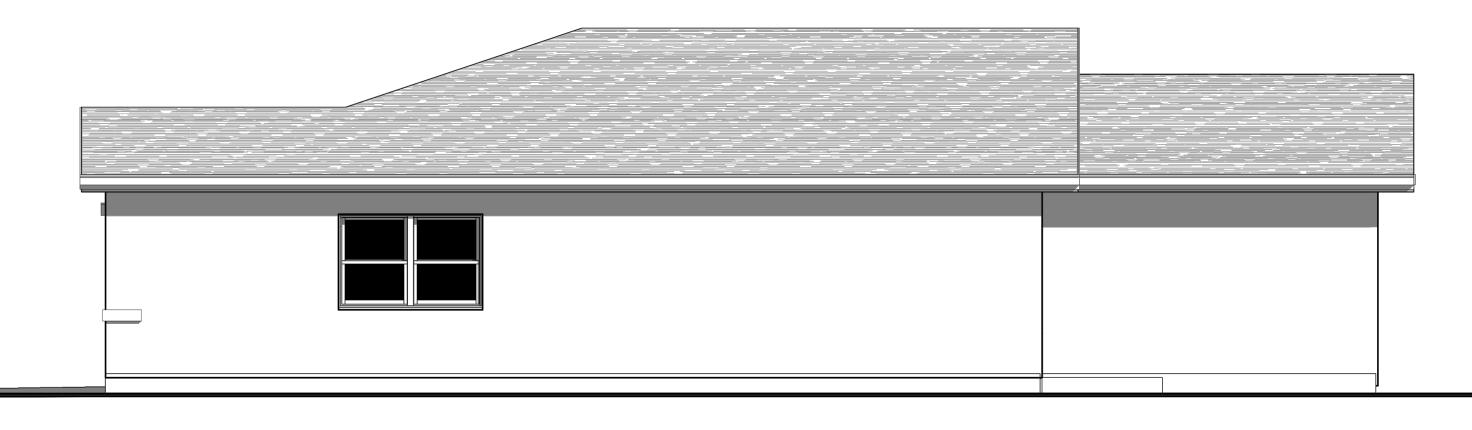
PROPOSED EXTERIOR ELEVATIONS-SINGLE-STORY

PERSPECTIVE

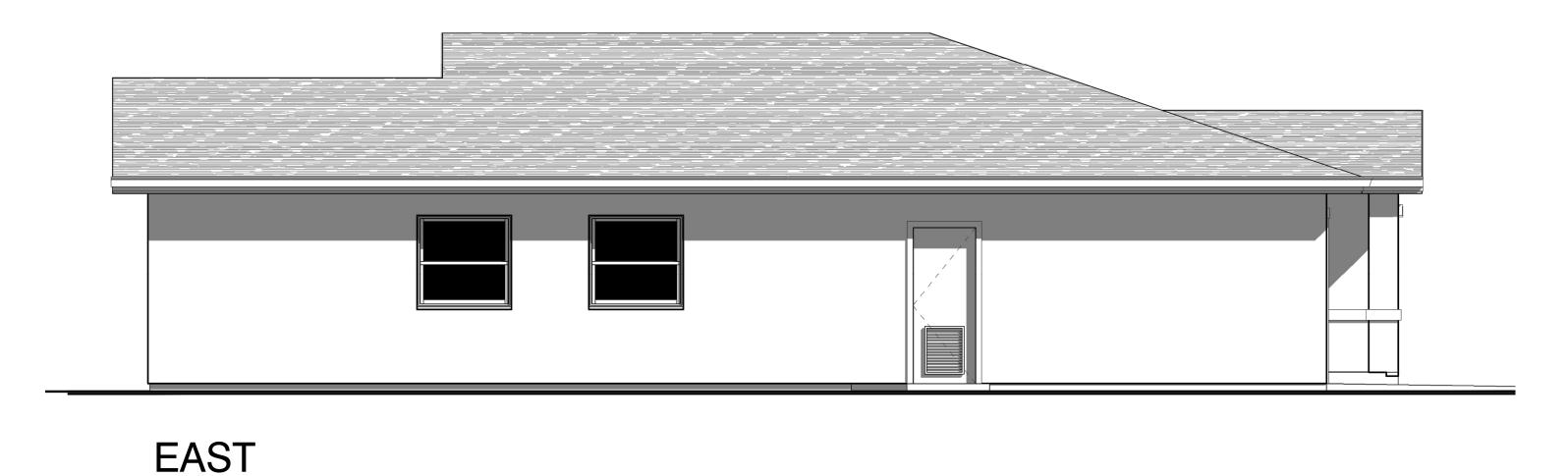




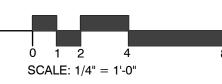
SOUTH



WEST



PROPOSED EXTERIOR ELEVATION - SINGLE-STORY



EXTERIOR FINISH SCHEDULE

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Drawn By: Drawing Date:

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Sheet Title: PROPOSED **EXTERIOR ELEVATIONS-**SINGLE-STORY

PERSPECTIVE

Sheet Number:

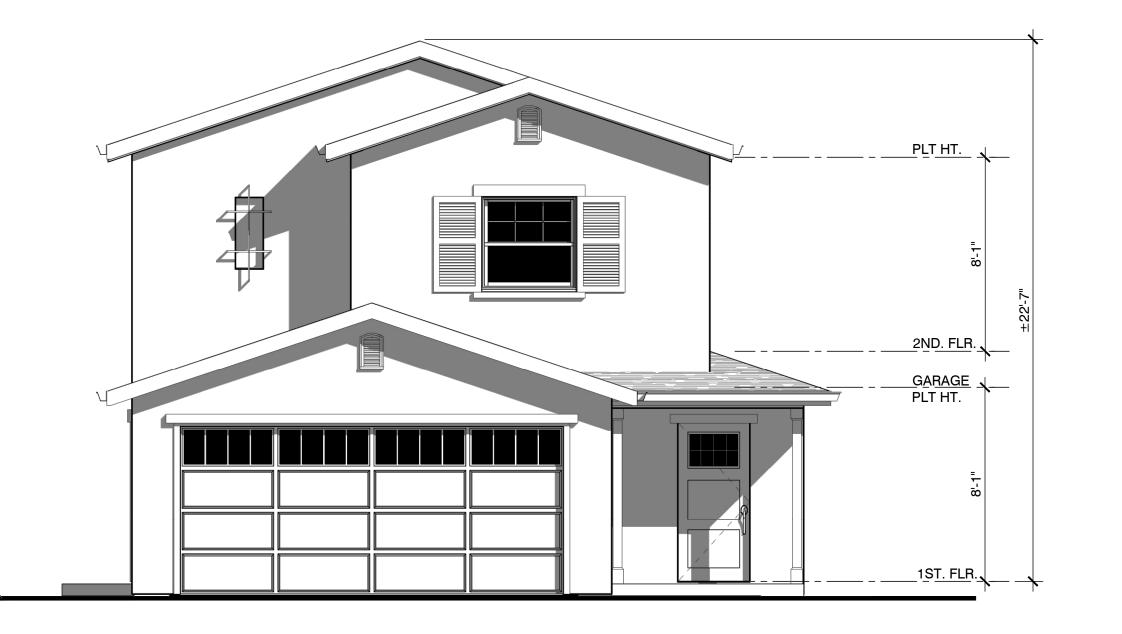
Housing

296 APPLE AVENUE GREENFIELD, CA

A.P.N.: 109-082-013-000

People's Self Help

WEST



EXTERIOR FINISH SCHEDULE

Class A rated minimum 30-year High Definition asphalt shingle Cool Roof on underlayment per CRC R905.1. Packaging for roof materials shall bear manufacturer's and approved testing agency's labels for field inspection.

FLASHING: 24 GA. Galvanized Sheet Metal, paint all sides prior to installation and a second coat after installation

General - At a minimum, provide a minimum of one layer of Tyvek StuccoWrap water resistive barrier complying with CRC 703.2. and shall be attached to the studs or sheathing, with flashing as described in the manufacturer's installation instructions, in such a manner as to provide a continuous water-resistive barrier behind the exterior wall veneer/covering.

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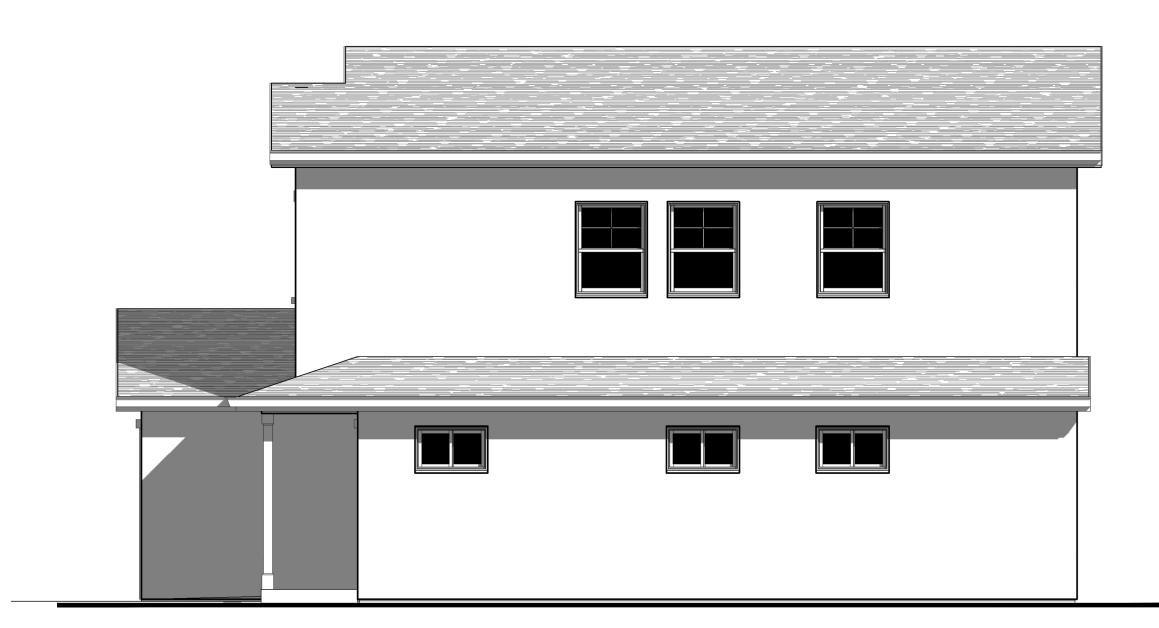
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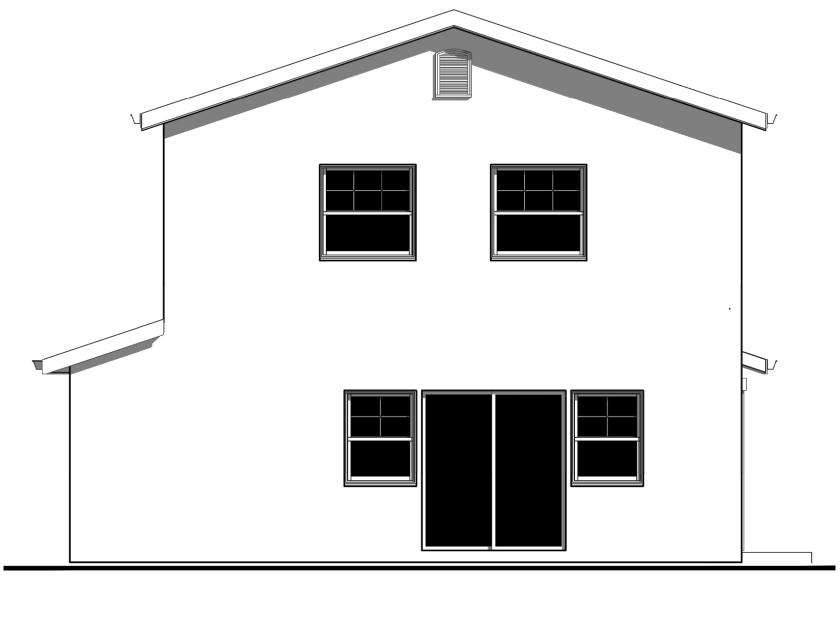




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SOUTH







Project Number:

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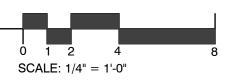
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Sheet Title: PROPOSED EXTERIOR ELEVATIONS-TWO-STORY

EAST NORTH

PROPOSED EXTERIOR ELEVATION - TWO-STORY



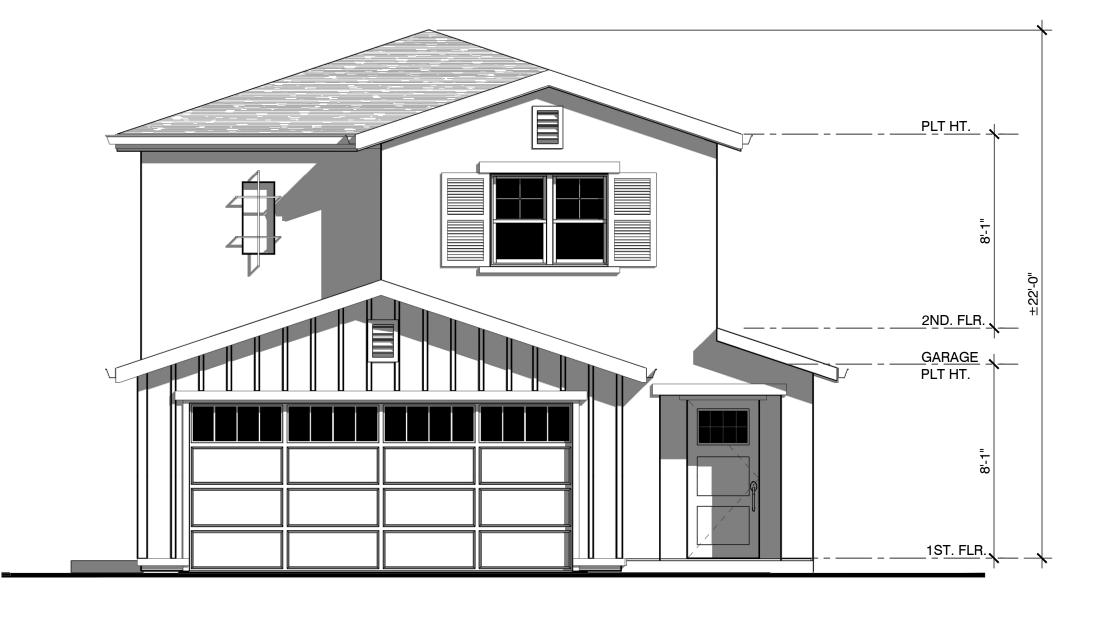
PERSPECTIVE

Housing

296 APPLE AVENUE GREENFIELD, CA

A.P.N.: 109-082-013-000

People's Self Help



EXTERIOR FINISH SCHEDULE

ROOF: Class A rated minimum 30-year High Definition asphalt shingle Cool Roof on underlayment per CRC R905.1. Packaging for roof materials shall bear manufacturer's and approved testing agency's labels for field inspection.

FLASHING: 24 GA. Galvanized Sheet Metal, paint **all** sides prior to installation and a second coat after installation

ALLS: General - At a minimum, provide a minimum of one layer of Tyvek StuccoWrap water resistive barrier complying with CRC 703.2. and shall be attached to the studs or sheathing, with flashing as described in the manufacturer's installation instructions, in such a manner as to provide a continuous water-resistive barrier behind

CEMENT PLASTER: Minimum thickness to be 7/8", three coats on of Grade "D" 60-minute paper with expanded metal lath on Tyvek StuccoWrap on wood sheathing. A minimum 26 GA galvanized metal weep screed with minimum 3-1/2" attachment flange at or below foundation plate is required at 4" minimum above earth and 2" minimum above finish paving. Comply with CBC chapters 2507 and 2512.

the exterior wall veneer/covering.

NOTES:

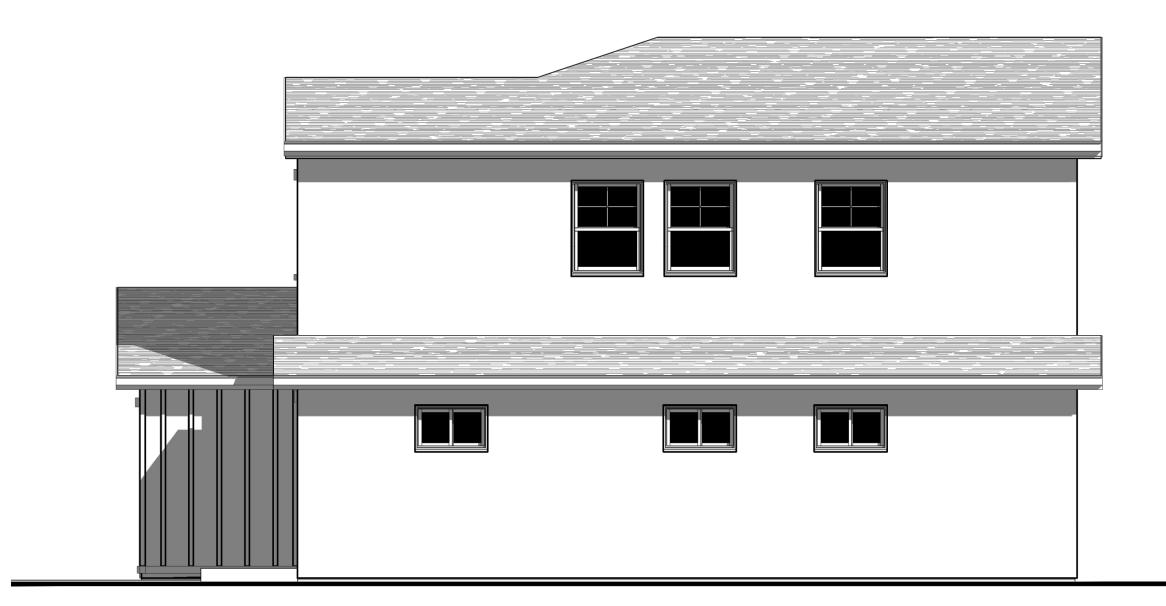
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WEST





Sheet Title: PROPOSED

Revisions:

PROPOSED EXTERIOR ELEVATIONS-TWO-STORY

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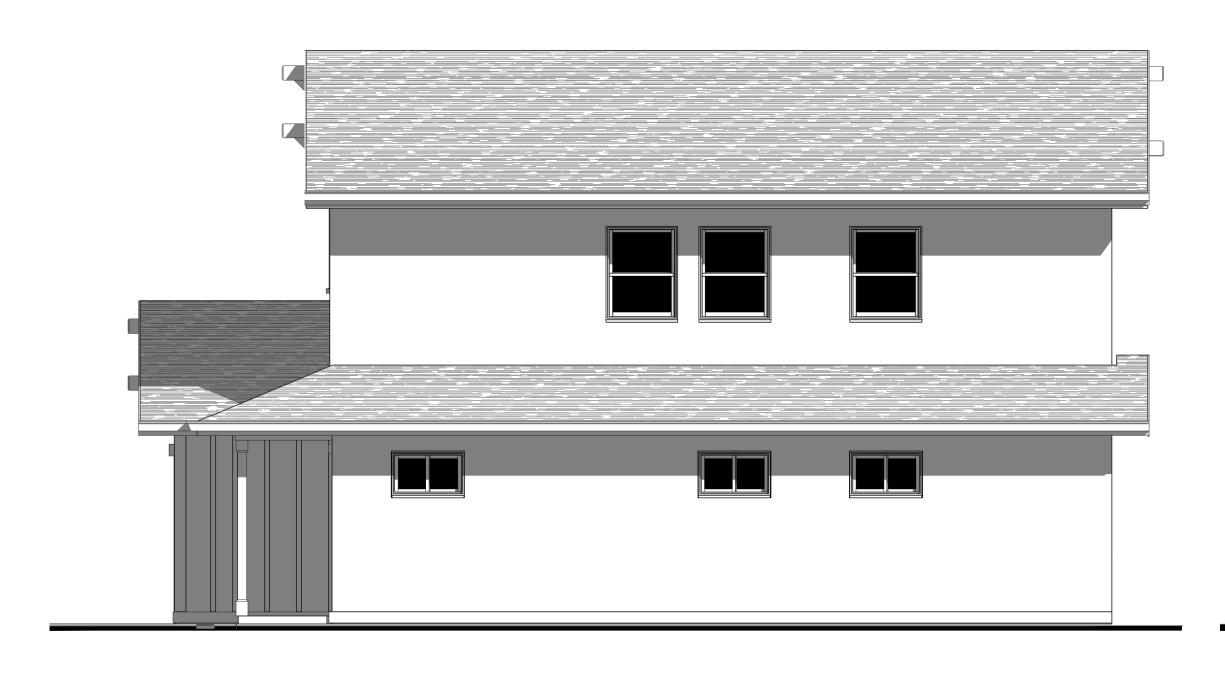
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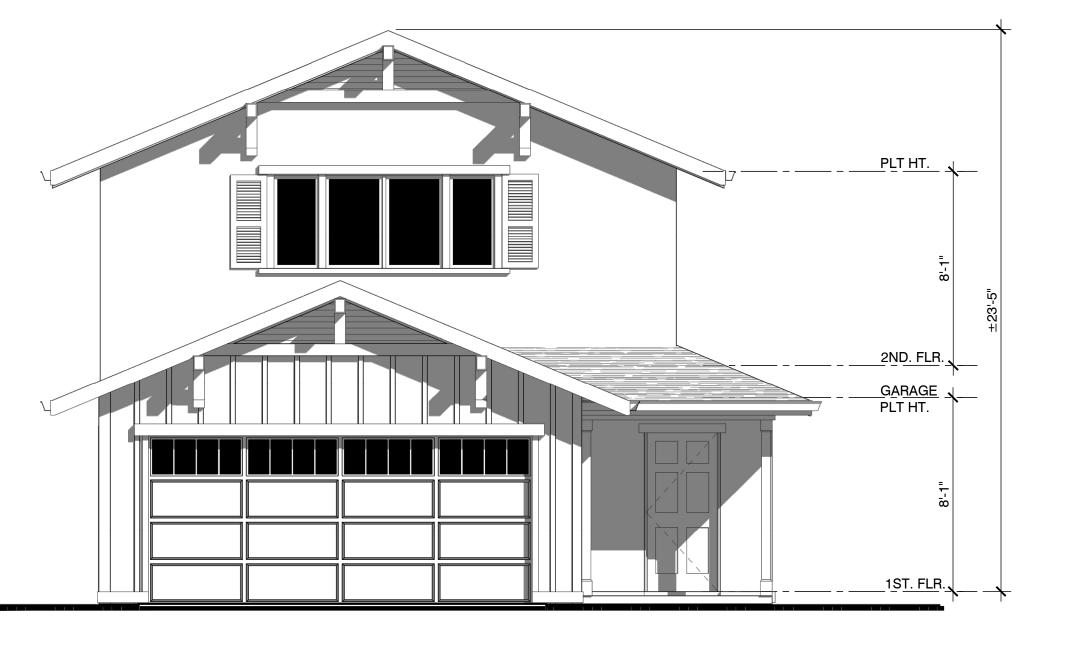
EAST

PROPOSED EXTERIOR ELEVATION - TWO-STORY

0 1 2 4 8 SCALE: 1/4" = 1'-0" PERSPECTIVE

Sheet Number





EXTERIOR FINISH SCHEDULE

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Housing 296 APPLE AVENUE

People's Self Help

GREENFIELD, CA

A.P.N.: 109-082-013-000



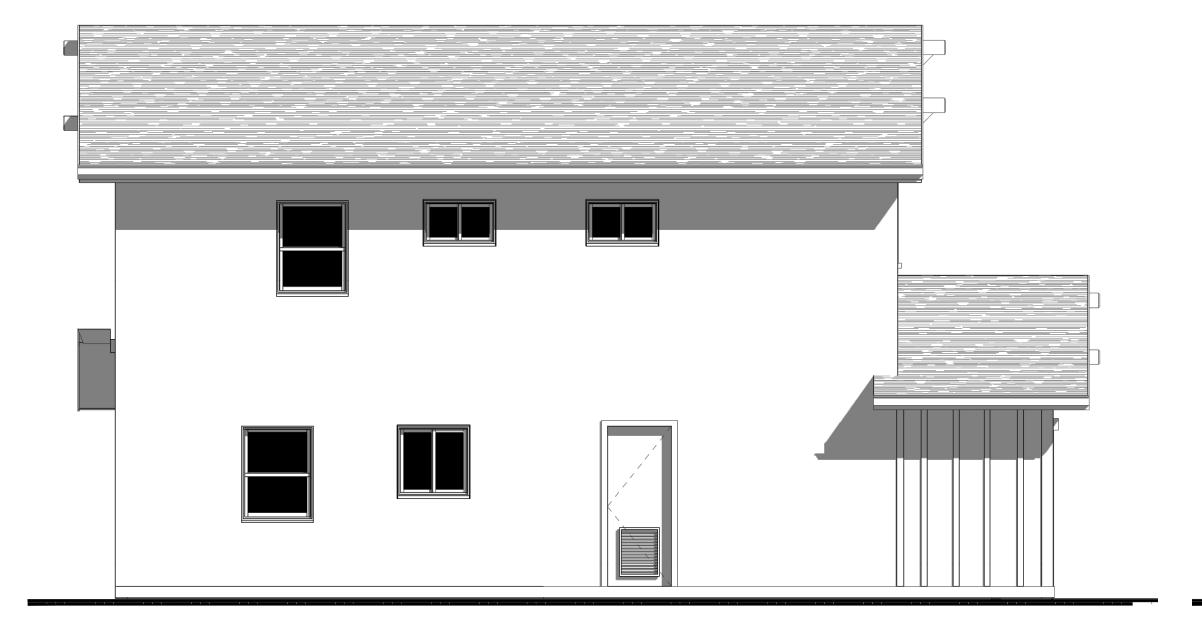


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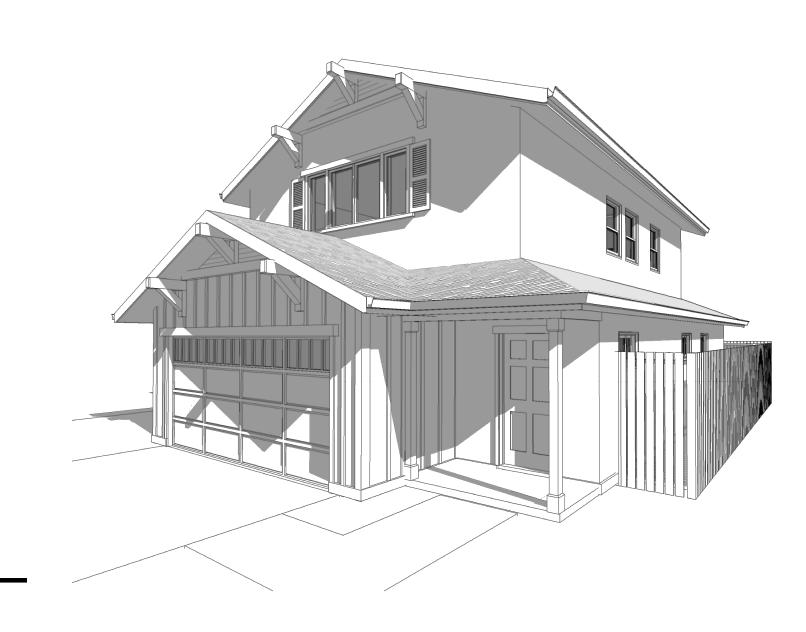
WEST



NORTH







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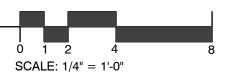


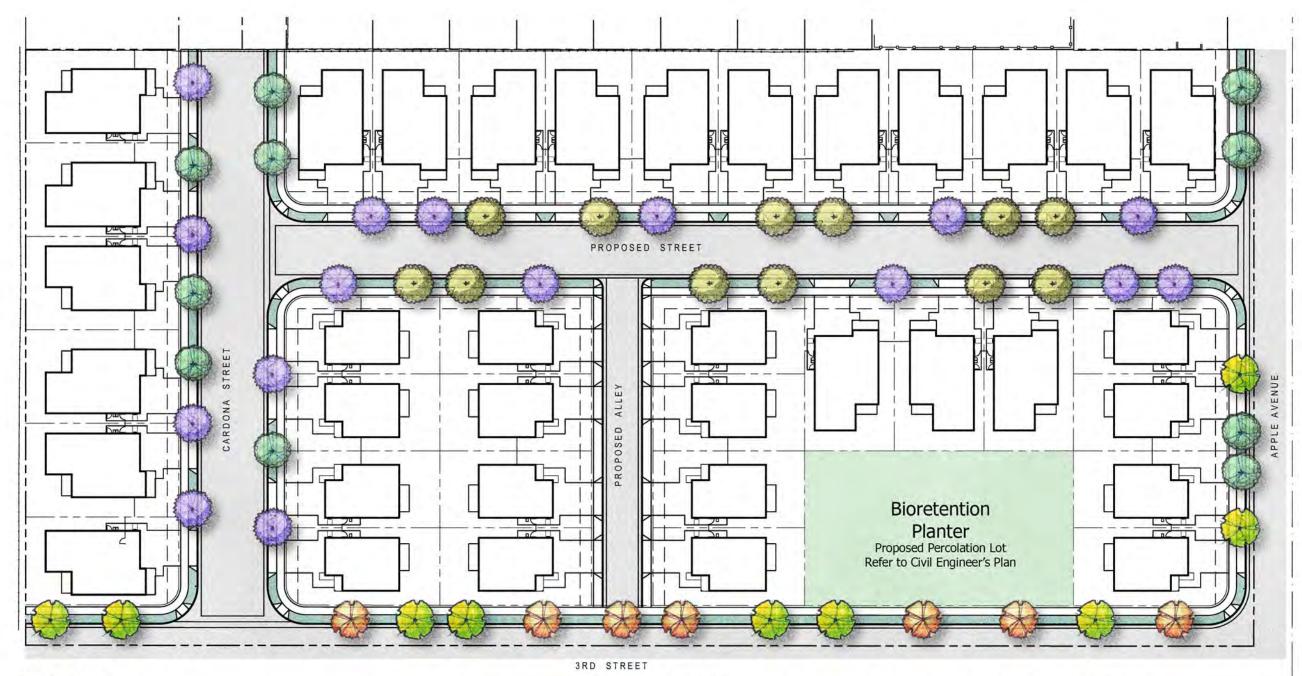
PROPOSED
EXTERIOR
ELEVATIONSTWO-STORY

PERSPECTIVE

EAST

PROPOSED EXTERIOR ELEVATION - TWO-STORY





Bioretention Planters

ABBREV SIZE BOTANICAL NAME / COMMON NAME RATING

ZONE A - BASIN BOTTOM GROUND COVER

F 30" OC 1G CAREX TUMULICOLA / BERKELEY SEDGE L
G 36" OC 1G JUNCUS PATENS / COMMON RUSH L

ZONE B - BASIN SIDE SUPE GROUND COVER

H 60" OC 1G BACCHARIS PILULARIS 'PIGEON POINT' / PROSTRATE COYOTE BRUSH VL
I 36" OC 1G LEYMUS CONDENSATUS 'CANYON PRINCE' / CANYON PRINCE WILD RYE
J 48" OC 1G MUHLENBERGIA RIGENS / DEER GRASS L

Street Trees - Per City of Greenfield "City Street Trees" List

GLEDITSIA TRIACANTHOS 'SHADEMASTER'/SHADEMASTER HONEYLOCUST

KOELREUTERIA PANICULATA / GOLDENRAIN TREE

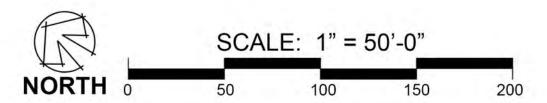
LAGERSTROEMIA INDICA 'TUSCARORA' / CRAPE MYRTLE (CORAL PINK)

PLATANUS ACERIFOLIA 'BLOODGOOD' / LONDON PLANE TREE

PYRUS KAWAKAMI / EVERGREEN PEAR



JBLA Job #21-125 October 8, 2021

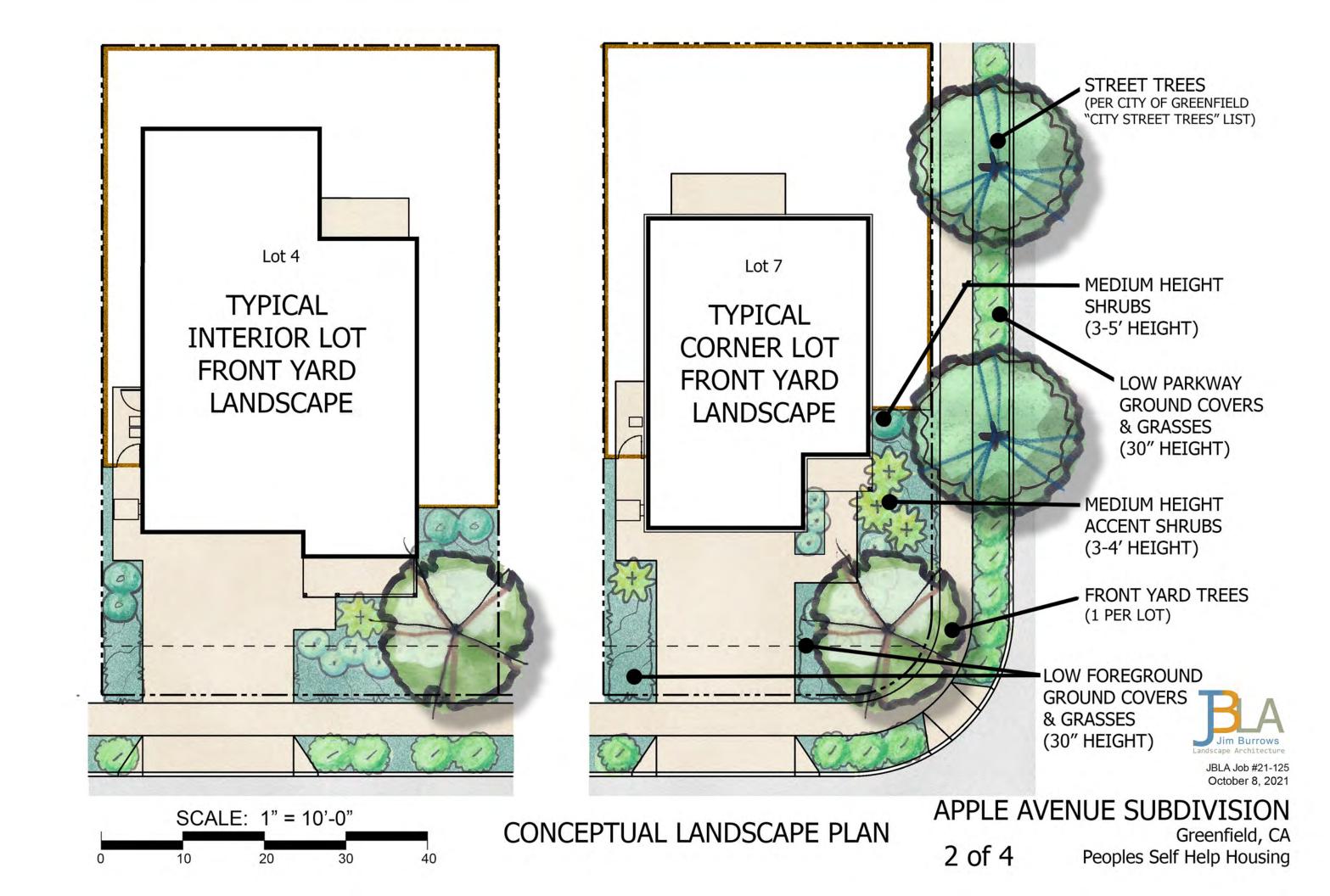


CONCEPTUAL STREET TREE PLAN
With Bioretention Planter Notes

APPLE AVENUE SUBDIVISION
Greenfield, CA

1 of 4

Peoples Self Help Housing



Plant List - 36 Homes, Greenfield, CA - Sunset Zone 14

| ABE | BREV | SIZE | | UCOL: |
|--------|------------------|----------------------|--|-------------|
| | | | City of Greenfield "City Street Trees" List | , , , , , , |
| | Trees mus | t be plant edwood | ted 2-1/2' to 3' behind sidewalk. Trees must be planted with root guards and b stakes per planting details. Trees to be standard form, 8' minimum height, 1" | raced |
| | TRI 'SM' | 15G 15G | GLEDITSIA TRIACANTHOS 'SHADEMASTER'/SHADEMASTER HONEYLOCUS KOELREUTERIA PANICULATA / GOLDENRAIN TREE | T L M |
| LAG | IND 'T' | 15G | LAGERSTROEMIA INDICA 'TUSCARORA' / CRAPE MYRTLE (CORAL PINK) | L |
| PLA | ACE 'B' | 15G | PLATANUS ACERIFOLIA 'BLOODGOOD' / LONDON PLANE TREE | M |
| PYR | KAW | 15G | PYRUS KAWAKAMI / EVERGREEN PEAR | М |
| | nt Yard T | rees | | |
| ARB | | 15G | ARBUTUS 'MARINA' / 'MARINA' ARBUTUS (STD.) | L |
| | TAS | 15G | CHITALPA TASHKENTENSIS / CHITALPA (MULTI-TRUNK) | L |
| | PAN | 15G | KOELREUTERIA PANICULATA / GOLDENRAIN TREE | M |
| | IND 'T' | 15G | LAGERSTROEMIA INDICA 'TUSCARORA' / CRAPE MYRTLE (CORAL PINK) | L |
| PIS | | 15G | PISTACIA CHINENSIS / CHINESE PISTACHE | M |
| PRU | CER 'KV' | 15G | PRUNUS CERASIFERA 'KRAUTER VESUVIUS' / PURPLE-LEAF PLUM | L |
| | | | <u>ubs</u> (3-5' height) | - 5.5 |
| | DEN 'HM' | | ARCTOSTAPHYLOS DENSIFLORA 'HOWARD MCMINN' / MANZANITA | VL |
| CAL | CHI | 5G 5G | CALLISTEMON 'LITTLE JOHN' / DWARF BOTTLEBRUSH LOROPETALUM CHINENSIS 'PURPLE MAJESTY' / RED FRINGE FLOWER | L |
| | CAL 'EC' | 5G | RHAMNUS CALIFORNICA 'EVE CASE' / COFFEEBERRY | Ĺ |
| | OFF 'TB' | 5G | ROSMARINUS OFFICINALIS 'TUSCAN BLUE' / ROSEMARY | ī |
| | MIC 'HL' | 5G | SALVIA MICROPHYLLA 'HOT LIPS' / HOT LIPS SAGE | L |
| Med | dium Hei | ght Acc | ent Shrubs (3-4' height, in groupings of one to three) | |
| BOU | | 5G | BOUGAINVILLEA 'ROSENKA' / BOUGAINVILLEA | L |
| | TEN 'YW' | | PHORMIUM TENAX 'YELLOW WAVE' / DWARF NEW ZEALAND FLAX | L |
| 0.00 | FLO 'I' | 5G | ROSA FLORIBUNDA 'ICEBERG' / ICEBERG ROSE | М |
| YUC | GLO 'W' | 5G | YUCCA GLORIOSA 'WALBRISTAR' / BRIGHT STAR YUCCA | L |
| | | ound Gr 1G | cound Covers and Grasses LAVANDULA ANGUSTIFOLIA 'MUNSTEAD' / MUNSTEAD LAVENDER | 4 |
| A B | 36" OC 60" OC | 1G | MYOPORUM PARVIFOLIUM 'PUTAH CREEK' / TRAILING MYOPORUM | L |
| C | 36" OC | 1G | PENSTEMON 'MARGARITA BOP' / MARGARITA BOP PENSTEMON | Ĺ |
| D | 36" OC | 1G | SALVIA 'DARA'S CHOICE / DARA'S CHOICE SAGE | ī |
| Lov | v Parkwa | y Groun | nd Covers and Grasses | |
| E | 42" OC | 1G | BOUTELOUA GRACILIS 'BLONDE AMBITION' / BLUE GRAMMA GRASS | L |
| F | 36" OC | 1G | FESTUCA MAIREI / ATLAS FESCUE | L |
| G | 48" OC | 1G | LANTANA 'NEW GOLD' / 'NEW GOLD' LANTANA | L |
| Н | 36" OC 36" OC | 1G | LAVANDULA ANGUSTIFOLIA 'MUNSTEAD' / MUNSTEAD LAVENDER | L |
| 1 | | 1G | LOMANDRA LONGIFOLIA 'BREEZE' / DWARF MAT RUSH | _ |
| MUL | | ROUND C | COVER AND PLANTER AREAS WITH 3" MINIMUM LAYER 'WALK-ON' BARK. | |
| | END | | | |
| VL | | | ATER USE | |
| L | | WATER U | | |
| M | = MEDIU | JM WATE WATER U | | |
| H G | = GALL | | JOE | |
| В | = BOX | 0110 | | |
| 00 | | ENITED OF | 101001 | |

OC = ON-CENTER SPACING STD = STANDARD FORMS

DRB = DEEP ROOT BARRIER, AS REQUIRED PER PLANTING DETAIL SHEET.

*WATER-USE EVALUATION OF PLANT MATERIALS

WATER USE OF PROPOSED PLANTS HAVE BEEN EVALUATED USING THE "WATER USE CLASSIFICATION OF LANDSCAPE SPECIES" (WUCOLS IV, UNIVERSITY OF CALIFORNIA COOPERATIVE EXTENSION.)

TREES













Chitalpa tashkentensis

Koelreuteria paniculata

Lagerstroemia 'Tuscarora'

Pistachia chinensis

Prunus Krauter Vesuvius

SHRUBS













Rhamnus Eve Case

Arctostaphylos Howard McMinn Bougainvillea 'Rosenka'



Salvia Hot Lips



Yucca gloriosa Bright Star

GROUND COVERS





Rosmarinus Tuscan Blue









Bouteloua 'Blonde Ambition' Festuca mairei













JBLA Job #21-125 October 8, 2021

APPLE AVENUE SUBDIVISION

3 of 4

Greenfield, CA Peoples Self Help Housing

| Hydrozone # Planting Plant Factor Irrigation Efficiency (IE)* ETAF (PF) Landscape Area (Sq. Ft.) ETAF x Area Estimated Total Water Use (ETWU)** EED EED EE | | The second second | | cienct Land | | | | 6.6 |
|--|--|---------------------------------|--------------------------|--------------------|----------------------|-------------------|---------------|--------------------|
| Solect your city: Greenfield Project name or address: Greenfield Interior Lot | This workshe | | | | | | Documentation | Package, |
| Select your city: Reference Evapotranspiration (ETo): 49.5 Landscape Area Sector Type: Residential | | On | e worksheet co | mplete for point o | orconnection | (water meter).* | | 20A)F 5 |
| Reference Evapotranspiration (ETo): 49.5 Landscape Area Sector Type: Residential | | | Constitution of the last | | | | | C 1 10 C E |
| California Water Efficient Landscape Worksheet | | The second second second second | | | | | 5753500 | |
| Reference Evapotranspiration (ET a) | Reference Evapotra | nspiration (£10): | 49,5 | | Landscape | Area Sector Type: | , A | esidential |
| Plant Factor Property Plant Property Property | | | California W | ater Efficient L | andscape | Worksheet | | |
| Description | Reference Evapotranspira | ation (ET _o) | 49.5 | 1 | Project Type | Resid | ential | 0.55 |
| Low Water Use 0.2 Bubbler 0.77 0.26 0 0 0 0 0 0 0 0 0 | | | | | | | ETAF x Area | - A1 |
| Low Water Use 0.2 Drip 0.81 0.25 636 157 4819 | Regular Landscape A | reas | | | | | | |
| Very Low Water Use | Low Water Use Trees | 0.2 | Bubbler | 0.77 | 0,26 | | 0 | 0 |
| High Water Use 0.8 Overhead 0.75 1.07 0 0 0 | Low Water Use | 0.2 | Drip | 0.81 | 0.25 | 636 | 157 | 4819 |
| Average Total Total | Very Low Water Use | 0.1 | Drip | 0.81 | 0.12 | | 0 | 0 |
| Average Total Total | High Water Use | 0.8 | Overhead | 0.75 | 1.07 | | 0 | 0 |
| Average ETAF for Regular Landscape Areas : In Compliance Special Landscape Areas 0 | | | | | Average | Total | Total | |
| Special Landscape Areas SLA-1 | | 1 | | | 0.25 | 636 | 157 | |
| Special Landscape Areas SLA-1 | | | | | | | | |
| Total Company Compan | | 1 | | Average | ETAF for | Regular Lands | cape Areas : | In Compliance |
| Total Landscape Area 636 | | eas | | | | | | |
| Total Landscape Area 636 Statewide ETAF 0.25 ETWU Total 4,819 Maximum Allowed Water Allowance (MAWA)* 10,735 ETAF Calculations Regular Landscape Areas Average ETAF for Regular Landscape Percentage of MAWA Total ETAF x Area 157 Areas must be 0.55 or below for residential areas, and 0.45 or below for non-residential areas, All Landscape Areas 0.45 Non-Residential Total ETAF x Area 157 0.55 Residential Total Area 636 0.81 Drip | SLA-1 | | | | - | | - 2 | 0 |
| Statewide ETAF 0.25 | | | | | Totals | 0 | 0 | |
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| Maximum Allowed Water Allowance (MAWA) | | | | | | Sta | tewide ETAF | 0.25 |
| Maximum Allowed Water Allowance (MAWA) | | | | | | | ETWILL Tetal | 4.940 |
| ETAF Calculations Average ETAF for Regular Landscape Percentage of MAWA Regular Landscape Areas Average ETAF for Regular Landscape Percentage of MAWA Total ETAF x Area 157 Areas must be 0.55 or below for residential areas, and 0.45 or below for non-residential areas. 45% All Landscape Areas 0.25 Non-Residential Total ETAF x Area 157 0.55 Residential Total Area 636 0.81 Drip | | | | Manufact | um Allena | Water Alleres | | 27.71 |
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| Total ETAF x Area 157 0.55 Residential Total Area 636 0.81 Drip | | | | 0.45 | Non-Resid | lential | | |
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| | - 011 | e worksneer co | inprese an panie | // COMICCION | (water meter). | | ZONE 1 |
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| Reference Evapotrar | The second second second | 49.5 | | And the second second | Area Sector Type: | | esidential |
| Reference Evaporrar | ispiration (E10). | 45.5 | | Lanuscape | Area sector type. | | Coldential |
| | | California W | ater Efficient L | andscape | Worksheet | | |
| Reference Evapotranspira | ation (ET _o) | 49.5 | | Project Type | Resid | ential | 0.55 |
| Hydrozone # / Planting Description | Plant Factor (PF) | Irrigation Method [®] | Irrigation Efficiency (IE) | ETAF (PF/ IE) | Landscape Area (Sq. Ft.) | ETAF x Area | Estimated Total Water Use (ETWU) |
| Regular Landscape Ai | reas | | | | | | |
| Low Water Use Trees | 0.2 | Bubbler | 0.77 | 0.26 | | 0 | 0 |
| Low Water Use | 0.2 | Drip | 0.81 | 0.25 | 1.016 | 251 | 7699 |
| Very Low Water Use | 0.1 | Drip | 0.81 | 0.12 | | 0 | 0 |
| High Water Use | 0.8 | Overhead | 0.75 | 1.07 | | 0 | .0 |
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TYPICAL INTERIOR FRONT YARD LANDSCAPE TYPICAL CORNER FRONT YARD LANDSCAPE WATER EFFICIENT LANDSCAPE ORDINANCE (WELO) WORKSHEETS

Water Conservation Notes

The following water conservation techniques shall be employed in this Project:

- Planting and irrigation design shall conform to the "Model Water Efficient Landscape Ordinance" (MWELO).
- Water conserving plants, defined as "Low" in the "Water Use Classification of Landscape Species" (WUCOLS IV, University of California Cooperative Extension), shall be utilized in 95% of the total planting area.
- Irrigation system shall be separated into distinct hydrozones based on plant material types, exposure and orientation.
- Soil amendments and mulch shall be utilized to improve water holding capacity of soil.
- Automatic irrigation system shall utilize "Smart Controller" technology with water budgeting feature to adjust water application based on soil moisture and/or local weather data.
- Recommendations shall be given for annual irrigation schedule at project completion.
- Lawn is not used.

Statement of Water Conserving Irrigation Design

The following principles of irrigation design are utilized to conserve water and improve the efficiency of the irrigation system:

- All irrigation shall be drip or dripline emitters. No overhead spray heads will be used.
- Irrigation hydrozone application shall be adjusted according to water needs and weather.
- Irrigation system master valve shall be used.
- Irrigation system "Smart controller" with water budgeting feature shall be used.
- · Irrigation system flow sensor shall be used.
- Irrigation system of rain shut-off device connected to irrigation controller shall be used.

To maintain the irrigation efficiency intended in the design, the irrigation system shall be tested and maintained on a monthly basis by maintenance staff.



JBLA Job #21-125 October 8, 2021

APPLE AVENUE SUBDIVISION

4 of 4

Greenfield, CA Peoples Self Help Housing CalEEMod Results

BAPPENDIX

Date: 8/4/2022 4:50 PM

Apple Ave Subdivision_Proposed Emissions - Monterey County, Winter

EMFAC Off-Model Adjustment Factors for Gasoline Light Duty Vehicle to Account for the SAFE Vehicle Rule Not Applied

Apple Ave Subdivision_Proposed Emissions Monterey County, Winter

1.0 Project Characteristics

1.1 Land Usage

Urbanization

| Land Uses | Size | Metric | Lot Acreage | Floor Surface Area | Population |
|-----------------------|-------|---------------|-------------|--------------------|------------|
| Single Family Housing | 36.00 | Dwelling Unit | 4.60 | 64,800.00 | 103 |

Precipitation Freq (Davs)

55

1.2 Other Project Characteristics

Urban

| | | | | | , |
|----------------------------|--------------------|----------------------------|-------|----------------------------|-------|
| Climate Zone | 4 | | | Operational Year | 2025 |
| Utility Company | Pacific Gas and El | lectric Company | | | |
| CO2 Intensity (lb/MWhr) | 203.98 | CH4 Intensity (Ib/MWhr) | 0.033 | N2O Intensity (lb/MWhr) | 0.004 |

3.6

1.3 User Entered Comments & Non-Default Data

Project Characteristics -

Land Use - Lot acreage adjusted to match project description.

Water And Wastewater - The proposed project would connect to the municipal sanitary sewer system.

Wind Speed (m/s)

Construction Off-road Equipment Mitigation -

Mobile Land Use Mitigation - All residences will be residences will be low and very-low income 3-bedroom housing units.

Landscaping equipment is set to electric only to reflect phasing out of gas-powered landscaping tools potentially by 2024 (AB 1346).

100 percent of electrical energy demand from renewable sources.

| Table Name | Column Name | Default Value | New Value |
|------------|-------------------|---------------|-----------|
| tblLandUse | LotAcreage | 11.69 | 4.60 |
| tblWater | AerobicPercent | 87.46 | 97.79 |
| tblWater | SepticTankPercent | 10.33 | 0.00 |

Date: 8/4/2022 4:50 PM

Apple Ave Subdivision_Proposed Emissions - Monterey County, Winter

EMFAC Off-Model Adjustment Factors for Gasoline Light Duty Vehicle to Account for the SAFE Vehicle Rule Not Applied

2.0 Emissions Summary

2.1 Overall Construction (Maximum Daily Emission)

Unmitigated Construction

| | ROG | NOx | CO | SO2 | Fugitive PM10 | Exhaust PM10 | PM10 Total | Fugitive PM2.5 | Exhaust PM2.5 | PM2.5 Total | Bio- CO2 | NBio- CO2 | Total CO2 | CH4 | N2O | CO2e |
|---------|---------|---------|---------|--------|------------------|-----------------|------------|-------------------|------------------|-------------|----------|------------|------------|--------|--------|------------|
| Year | | | | | lb/d | day | | | | | | | lb/c | lay | | |
| 2022 | 3.2379 | 33.1402 | 21.0736 | 0.0400 | 19.8049 | 1.6136 | 21.4185 | 10.1417 | 1.4845 | 11.6262 | 0.0000 | 3,861.0676 | 3,861.0676 | 1.1977 | 0.0169 | 3,888.6989 |
| 2023 | 45.2543 | 14.6289 | 16.6899 | 0.0287 | 0.1643 | 0.7017 | 0.8356 | 0.0436 | 0.6603 | 0.6964 | 0.0000 | 2,739.9044 | 2,739.9044 | 0.6126 | 0.0162 | 2,760.0400 |
| Maximum | 45.2543 | 33.1402 | 21.0736 | 0.0400 | 19.8049 | 1.6136 | 21.4185 | 10.1417 | 1.4845 | 11.6262 | 0.0000 | 3,861.0676 | 3,861.0676 | 1.1977 | 0.0169 | 3,888.6989 |

2.2 Overall Operational

Unmitigated Operational

| | ROG | NOx | CO | SO2 | Fugitive PM10 | Exhaust PM10 | PM10 Total | Fugitive PM2.5 | Exhaust PM2.5 | PM2.5 Total | Bio- CO2 | NBio- CO2 | Total CO2 | CH4 | N2O | CO2e |
|----------|---------|--------|---------|-----------------|------------------|-----------------|------------|-------------------|------------------|-------------|----------|------------|------------|-----------------|-----------------|------------|
| Category | lb/day | | | | | | | | | | | lb/d | ay | | | |
| Area | 25.7962 | 0.7594 | 34.0865 | 0.0600 | | 4.4193 | 4.4193 | | 4.4193 | 4.4193 | 470.2053 | 485.6302 | 955.8355 | 0.5814 | 0.0396 | 982.1686 |
| Energy | 0.0283 | 0.2421 | 0.1030 | 1.5400e- 003 | | 0.0196 | 0.0196 | | 0.0196 | 0.0196 | | 308.9971 | 308.9971 | 5.9200e- 003 | 5.6600e- 003 | 310.8333 |
| Mobile | 0.9748 | 1.3959 | 9.9111 | 0.0202 | 2.0875 | 0.0173 | 2.1048 | 0.5568 | 0.0162 | 0.5730 | | 2,050.3162 | 2,050.3162 | 0.1388 | 0.1022 | 2,084.2395 |
| Total | 26.7993 | 2.3974 | 44.1005 | 0.0817 | 2.0875 | 4.4562 | 6.5437 | 0.5568 | 4.4551 | 5.0118 | 470.2053 | 2,844.9435 | 3,315.1488 | 0.7262 | 0.1474 | 3,377.2414 |

Biological Peer Review and Report









Planning for Success.

July 14, 2022

Paul Mugan Community Development Director City of Greenfield 599 El Camino Real Greenfield, CA 93927

Re: 296 Apple Avenue – Peer Review of Biological Resource Assessment

Dear Paul,

This letter documents a peer review of the biological resource assessment prepared to address potential biological and aquatic (wetland) resources occurring at or within the vicinity of the proposed project site at 296 Apple Avenue in the City of Greenfield, Monterey County, California:

 Biological Resource Assessment for 296 Apple Avenue, Althouse and Meade, Inc. July 2021

The purpose of this peer review is to determine if the assessment was conducted according to professional standards, comprehensively addresses biological resources with the potential to occur on or in the vicinity of the project site, and are adequate for inclusion in a legally-defensible environmental document.

Biological Resource Assessment Summary

- 1. The *Biological Resource Assessment* was prepared by Althouse and Meade in July 2021 and is based on a field visit on June 22, 2021.
- The Biological Resource Assessment contains a comprehensive description of the habitat conditions on the project site and in the surrounding area and includes a list of the habitat types and plant and animal species observed during field visits.

Paul Mugan Community Development Director City of Greenfield July 14, 2022, Page 2

- 3. The *Biological Resource Assessment* lists all sensitive biotic resources with potential to occur on the project site including the distribution and known occurrences of special-status species and sensitive habitats in the project area in the California Natural Diversity Database.
- 4. The field visit by an Althouse and Meade biologist found marginal habitat on the project site for several special-status plant and animal species.
- 5. The *Biological Resource Assessment* includes a comprehensive discussion of potential impacts (impact analysis) to special-status species and provides recommendations for project avoidance and minimization.
- 6. The field visit by an Althouse and Meade biologist did not find evidence for wetlands or jurisdictional Waters of the U.S.

Issue Areas

The Althouse and Meade biologist concluded that the following are the only specialstatus plant and wildlife species with a potential for occurrence at the site or in the immediate vicinity:

- protruding buckwheat (Eriogonum nudum var. inductum);
- Cooper's hawk (Accipiter cooperii);
- white-tailed kite (*Elanus leucurus*);
- Yuma myotis (Myotis yumanensis);
- bank swallow (Riparia riparia); and
- San Joaquin kit fox (Vulpes macrotis mutica)

Conclusions

We agree with the conclusion that there is low potential for the six special-status plant and animal species listed above to occur on the project site. The remaining species reported from the region and listed in Appendices A and B of the report are considered unlikely to occur due to the lack of suitable habitat. Raptors such as Cooper's hawk and white-tailed kite could potentially forage or nest on the site. Special-status bat species, such as Yuma myotis, could potentially roost in building crevices found on the site.

Paul Mugan Community Development Director City of Greenfield July 14, 2022, Page 3

There is a very low potential for San Joaquin kit fox to migrate through or forage at the project site.

The biological resource assessment takes a cautious approach and requires mitigation measures including pre-construction surveys for nesting birds and bats. A mitigation measure to avoid impacts to San Joaquin kit fox is also included. We agree that the proposed mitigation measures suitably reduce the potential for impacts to the species identified.

The biological resource assessment was prepared consistent with professional standards. All necessary components of this analysis were present including complete discussions of the regulatory setting, methodology, mapping, baseline environmental conditions, results of field surveys, and impact analysis. The assessment provides a comprehensive and accurate review and analysis of the biological resources found at the project site and provides avoidance and minimization measures to minimize impacts to sensitive species and habitat. It is our professional opinion that no additional analysis of biological resources is needed in order to prepare an adequate CEQA document.

I hope this peer review meets your needs at this time. If you have any questions, please contact me at <u>furtado@emcplanning.com</u>.

Sincerely,

Patrick Furtado, MS Senior Biologist

Patrick Furtado

Biological Resource Assessment

for

296 Apple Avenue

APN(s) 109-82-013 City of Greenfield, California



Prepared for

People's Self-Help Housing c/o Sheryl Flores

3533 Empleo Street San Luis Obispo, CA 93401 (805) 540-2465

by

ALTHOUSE AND MEADE, INC. BIOLOGICAL AND ENVIRONMENTAL SERVICES

1602 Spring Street Paso Robles, CA 93446 (805) 237-9626

July 2021

Reporting Biologist:

Daniel E. Meade Principal Scientist Althouse and Meade, Inc. 1602 Spring Street Paso Robles, CA 93446 (805) 237-9626 Dan@althouseandmeade.com Kristen Andersen Biologist III | Botanist Althouse and Meade, Inc. 1602 Spring Street Paso Robles, CA 93446 (805) 237-9626 Kristena@alt-me.com

I certify that this Biological Resource Assessment was prepared according to professional standards and that the statements furnished in the report and associated maps are true and correct to the best of my knowledge and belief.

| Daniel E. Meade | | |
|-----------------|----------|--|
| | 7/8/2021 | |
| Signature | Date | |
| | | |
| | | |
| | | |
| | | |
| | | |
| | 7/8/2021 | |
| Signature | Date | |

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Appendix A. Special Status Plants Reported from the Region Appendix B. Special Status Animals Reported from the Region

Cover Page: Photo of fallow cropland habitat and residential structures, view southeast. June 22, 2021.

SYNOPSIS

- This report describes the study of biological resources at a 4.9-acre site (Study Area) in the City of Greenfield, California. The Study Area includes Assessor's Parcel Number (APN) 109-82–013.
- The project is a residential development with associated infrastructure and parking, and would impact the entire site.
- Habitat types identified and mapped within the Study Area are fallow cropland and anthropogenic.
- Botanical surveys identified 33 species of vascular plants in the Study Area. One special status plant (protruding buckwheat) is known from within five miles of the Study Area. Protruding buckwheat was not found in the Study Area and no special status plants were observed in the Study Area.
- Wildlife surveys detected 10 animal species in the Study Area. There are four special status animals with low potential to occur in the Study Area (Cooper's hawk, white-tailed kite, Yuma myotis, and bank swallow). No special status animals were observed in the Study Area.
- Biological resources that could be impacted by the Project include: fallow cropland and anthropogenic habitats, nesting birds, special status birds, and bats. The project is within the historical range of San Joaquin kit fox, however this species has not been reported within 40 miles of the project within the last 30 years. Mitigation recommendations are provided to reduce potential impacts to sensitive biological resources.

1 INTRODUCTION

1.1 Purpose

This Biological Resource Assessment provides information regarding biological resources associated with 296 Apple Avenue, a 4.9-acre Study Area in the City of Greenfield, California. Results include a habitat assessment, botanical and wildlife inventory, a discussion of special status species that have potential to occur within the Study Area, an analysis of potential impacts to biological resources, and mitigation recommendations. Project plans are not finalized, therefore we assume the entire property will be developed.

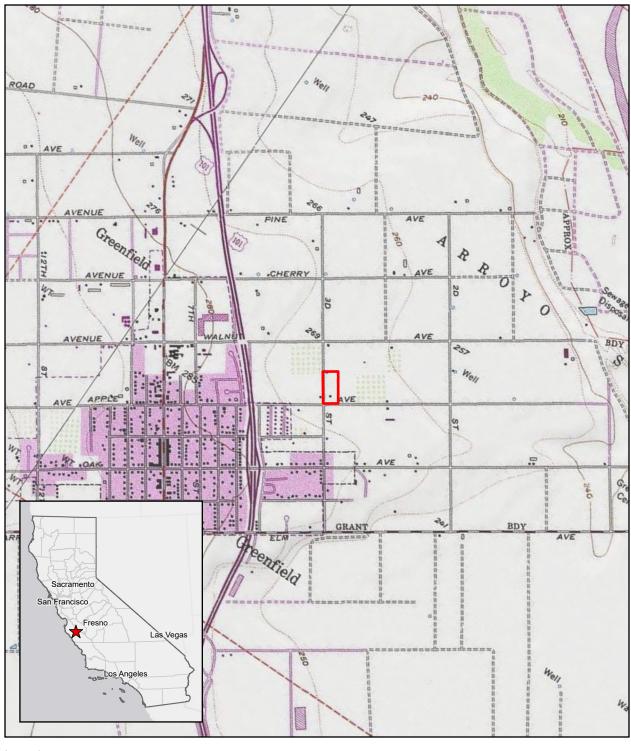
1.2 Project Location

The Study Area is in the City of Greenfield, east of State Highway 101 and 3rd Street, northwest of Apple Avenue. The site is in APN 109-082-013, equivalent to 4.9 acres. Location coordinates are 36.33001°N, 121.23698°W (WGS 84) in the Greenfield United States Geological Survey (USGS) 7.5-minute topographic quadrangle (Figure 1). The Study Area is planned for Medium Density Residential in the City's General Plan with a zoning designation of R-M, Multiple Family Residential - 7 to 15 dwelling units/acre (Greenfield 2017a, 2017b).

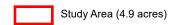
1.3 Local and Regional Context

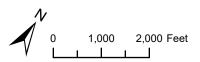
The City of Greenfield (City) is in south Salinas Valley, 35 miles south of the City of Salinas, and 13 miles north of King City in southern Monterey County. Highway 101, a major north-south route bisects the City. The Monterey/San Benito County line is 4.5 miles northeast. The region is largely agriculture fields and vineyards; however, over the years the City has maintained a rural community character (City 2005b). The area surrounding the site is mixed with single-family residences south, southwest, east, and northwest. Greenfield Park is located opposite 3rd Street and agricultural fields/rural residential development occurs to the west, northwest, and north. The Salinas River is two miles northeast. Elevations onsite and within the vicinity are flat at approximately 270 feet above mean sea level (Figure 2).

Figure 1. United States Geological Survey Topographic Map









PSHH GreenfieldMap Center: 121.23833°W 36.33036°N
Greenfield, Monterey County, California

USGS Quadrangle: Greenfield



Figure 2. Aerial Photograph

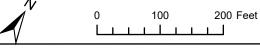




Study Area (4.9 acres)

PSHH GreenfieldMap Center: 121.23687°W 36.33003°N
Greenfield, Monterey County, California

Imagery Source: ESRI World Imagery





1.4 Regulatory Framework

Standards for environmental protection and restoration, in the form of laws and regulations, are created within three different organizational levels of government: Federal, State, and Local. Entities exist within each level to create and enforce regulations that help ensure protection of specific and pertinent regional issues threatening ecosystems and environments. The following regulations are applicable to the proposed Project.

1.4.1 Federal Law and Regulations

Endangered Species Act. The federal Endangered Species Act (FESA) provides the legal framework for the listing and protection of species (and their habitats) identified as being endangered or threatened with extinction. "Critical Habitat" is a term within the FESA designed to guide actions by federal agencies and is defined as "an area occupied by a species listed as threatened or endangered within which are found physical or geographical features essential to the conservation of the species, or an area not currently occupied by the species which is itself essential to the conservation of the species." Actions that jeopardize endangered or threatened species and/or critical habitat are considered a 'take' under the FESA. "Take" under federal definition means to harass, harm, pursue, hunt, shoot, wound, kill, trap, capture, or collect, or to attempt to engage in any such conduct.

Projects that would result in "take" of any federally listed threatened or endangered species, or critical habitats, are required to obtain permits from the USFWS through either Section 7 (interagency consultation with a federal nexus) or Section 10 (Habitat Conservation Plan) of FESA, depending on the involvement by the federal government in permitting and/or funding of the project. Through Section 10, it is required to prepare a Habitat Conservation Plan (HCP) to be approved by the United States Fish and Wildlife Service (USFWS), which results in the issuance of an Incidental Take Permit (ITP). Through Section 7, which can only occur when a separate federal nexus in a project exists (prompting interagency consultation), a consultation by the various federal agencies involved can take place to determine appropriate actions to mitigate negative effects on endangered and threatened species and their habitat.

Migratory Bird Treaty Act. All migratory, non-game bird species that are native to the U.S. or its territories are protected under the federal Migratory Bird Treaty Act (MBTA) of 1918 (50 C.F.R. Section 10.13), as amended under the Migratory Bird Treaty Reform Act of 2004. The MBTA makes it illegal to purposefully take (pursue, hunt, shoot, wound, kill, trap, capture, or collect) any migratory bird, or the parts, nests, or eggs of such a bird, except under the terms of a valid Federal permit. Migratory non-game native bird species are protected by international treaty under the federal MBTA.

1.4.2 State Law and Regulations

California Endangered Species Act. The California Endangered Species Act (CESA), similar to FESA, contains a process for listing of species and regulating potential impacts to listed species. State threatened and endangered species include both plants and wildlife, but do not include invertebrates. The designation "rare species" applies only to California native plants. State threatened and endangered plant species are regulated largely under the Native Plant Preservation Act in conjunction with the CESA. State threatened and endangered animal species are legally

protected against "take." The CESA authorizes the California Department of Fish and Wildlife (CDFW) to enter into a memorandum of agreement for take of listed species to issue an incidental take permit for a state-listed threatened and endangered species only if specific criteria are met. Section 2080 of the CESA prohibits the take of species listed as threatened or endangered pursuant to the Act. Section 2081 allows CDFW to authorize take prohibited under Section 2080 provided that: 1) the taking is incidental to an otherwise lawful activity; 2) the taking will be minimized and fully mitigated; 3) the applicant ensures adequate funding for minimization and mitigation; and 4) the authorization will not jeopardize the continued existence of the listed species.

California Environmental Quality Act (CEQA). CEQA defines a "project" as any action undertaken from public or private entity that requires discretionary governmental review (a non-ministerial permittable action). All "projects" are required to undergo some level of environmental review pursuant to CEQA, unless an exemption applies. CEQA's environmental review process includes an assessment of existing resources, broken up by categories (i.e., air quality, aesthetics, etc.), a catalog of potential impacts to those resources caused by the proposed project, and a quantifiable result determining the level of significance an impact would generate. The goal of environmental review under CEQA is to avoid or mitigate impacts that would lead to a "significant effect" on a given resource; section 15382 of the CEQA Guidelines defines a "significant effect" as

a substantial, or potentially substantial, adverse change in any of the physical conditions within the area affected by the project including land, air, water, minerals, flora, fauna, ambient noise, and objects of historic or aesthetic significance. An economic or social change by itself shall not be considered a significant effect on the environment, but may be considered in determining whether the physical change is significant.

Public agencies are required to implement CEQA and execute jurisdiction to determine when applicable activities are or are not subject to CEQA. A public agency with the most prominent nexus and jurisdiction to a project is called the lead agency. The lead agencies determine the scope of what is considered an impact and what constitutes a "significant effect". "Biological resources" is one of the varying categories considered during environmental review through CEQA. A lead agency can require a biological assessment to be prepared to report on existing biological resources and recommended mitigation measures that will reduce or lessen potential negative impacts to those biological resources. The questions listed in CEQA's Appendix G: Biological Resources section, which are used to guide assessment of impacts to biological resources are as follows:

- Does the project have a substantial adverse effect, either directly or through habitat modifications, on any species identified as a candidate, sensitive, or special status species in local or regional plans, policies, or regulations, or by the California Department of Fish and Game or U.S. Fish and Wildlife Service?
- Does the project have a substantial adverse effect on any riparian habitat or other sensitive natural community identified in local or regional plans, policies, regulations or by the California Department of Fish and Game or US Fish and Wildlife Service?
- Does the project have a substantial adverse effect on state or federally protected wetlands (including, but not limited to, marsh, vernal pool, coastal, etc.) through direct removal, filling, hydrological interruption, or other means?
- Does the project interfere substantially with the movement of any native resident or migratory fish or wildlife species or with established native resident or migratory wildlife corridors, or impede the use of native wildlife nursery sites?
- Does the project conflict with any local policies or ordinances protecting biological resources, such as a tree preservation policy or ordinance?

• Does the project conflict with the provisions of an adopted Habitat Conservation Plan, Natural Community Conservation Plan, or other approved local, regional, or state habitat conservation plan?

The lead agency has the final determination over whether a project is or is not permissible, based upon the environmental review, completed requirements and environmental documentation, and their judgement that the project will not have a significant effect on the environment, or that all significant effects have been mitigated for.

California Fish and Game Code (CFGC). The California Fish and Game Code (CFGC) is one of the 29 legal codes that form the general statutory law of California. A myriad of statutes regarding fish and game are specified in the CFGC; the following codes are specifically relevant to the proposed Project:

California Native Plant Protection Act. Sections 1900-1913 of the California Fish and Game Code contain the regulations of the Native Plant Protection Act of 1977. The intent of this act is to help conserve and protect rare and endangered plants in the state. The act allowed the CFGC to designate plants as rare or endangered.

1.4.3 Local Policies and Regulations

1.4.3.1 General Plan

The <u>Conservation</u>, <u>Recreation</u>, and <u>Open Space Element</u> (CROSE) of the General Plan focuses on protection and enhancement of community resources to ensure a high quality of living in Greenfield (City 2005a). These resources include agricultural, biological, historical/cultural, recreation, open space, and scenic resources. Goal 7.5 of the CROSE focuses on Biological Resources with 4 policies and 4 programs:

- Policy 7.5.1 Use land use planning to reduce the impact of development on important ecological and biological resources identified during application review and analysis.
- Policy 7.5.2 Encourage preservation of portions of important wildlife habitats that would be disturbed by major development.
- Policy 7.5.3 Develop open space uses in an ecologically sensitive manner.
- Policy 7.5.4 Development in sensitive habitat areas should be avoided or mitigated to the maximum extent possible.
 - <u>Program 7.5.A</u> Prior to development, areas with potential wildlife habitat shall be surveyed for special status plant and/or animal species. If any special status plant or animal species are found in areas proposed for development, the appropriate resource agencies shall be contacted and species-specific management strategies established to ensure the protection of the particular species.
 - <u>Program 7.5.B</u> Participate with regional, state, and federal agencies and organizations to establish and preserve open space that provides habitat for local wildlife.
 - <u>Program 7.5.C</u> At the discretion of the City, development proposals will be required to submit detailed biological resource assessments as part of the application or CEQA review process. Projects shall demonstrate compliance with the recommendations of those assessments.

 <u>Program 7.5.D</u> – The City shall explore the feasibility of a citywide habitat mitigation fee as an alternative to site-specific mitigation requirements.

1.4.3.2 Walnut Avenue Specific Plan (WASP)

The WASP is a 62.6-acre land use plan, intended for a multi-functional area for community events, activities, and shopping. The City prepared an Environmental Impact Report (EIR) for the WASP to help facilitate future development and streamline the CEQA process.

The EIR identified one biological resource mitigation measure This is the only Bio MM for the Walnut Avenue Specific Plan:

BIO-1 Prior to initiating construction activities for any individual project for which construction would begin during the period [of] February 1 to August 31, individual project developers will conduct pre-construction surveys for protected nesting birds. If present, appropriate protection measures will be implemented (DRAFT EIR, page 3-57).

1.4.3.3 City of Greenfield Tree Guidelines

Chapter 12.10.070 of the City's Municipal Code (City 2021) states that it is unlawful for any person other than the director or authorized agents or employees to do the following to any tree in any public street within the city or must receive a written permit prior to bracing, cutting, moving, planting, pruning, removing, replacing, spraying or trimming trees.

1.5 Special Status Species and Sensitive Habitat Regulations

For purposes of this Biological Resource Assessment, special status species are those plants and animals listed, proposed for listing, or candidates for listing as threatened or endangered by the USFWS under the FESA; those listed or proposed for listing as rare, threatened, or endangered by the CDFW under the CESA; animals designated as "Species of Special Concern," "Fully Protected," or "Watch List" by the CDFW; and plants with a California Rare Plant Rank (CRPR) of 1, 2, 3, or 4. In the following sections, further details are provided to highlight the different guidelines and qualifications that are used to help identify special status species in this report. In Sections 3.4 and 3.5, the various qualifications are listed in the special status species tables (Table 3 and Table 5) for each species with potential to occur in the project area.

1.5.1 California Natural Diversity Database (CNDDB)

"Special Plants" and "Special Animals" are broad terms used to refer to all the plant and animal taxa inventoried by the CNDDB, regardless of their legal or protection status (CDFW 2021b, CDFW 2021c). The Special Plants list includes vascular plants, high priority bryophytes (mosses, liverworts, and hornworts), and lichens. The Special Animals list is also referred to by the California Department of Fish and Wildlife (CDFW) as the list of "species at risk" or "special status species."

According to the CNDDB, Special Plants and Animals lists include: taxa that are officially listed or proposed for listing by California or the Federal Government as Endangered, Threatened, or Rare; taxa which meet the criteria for listing, as described in Section 15380 of CEQA Guidelines; taxa deemed biologically rare, restricted in range, declining in abundance, or otherwise vulnerable; population(s) in California that may be marginal to the taxon's entire range but are threatened with extirpation in California; and/or taxa closely associated with a habitat that is declining in California

at a significant rate. Separately, the Special Plants List includes taxa listed in the California Native Plant Society's Inventory of Rare and Endangered Plants of California, as well as taxa determined to be Sensitive Species by the Bureau of Land Management, U.S. Fish and Wildlife Service, or U.S. Forest Service. The Special Animals List distinctively includes taxa considered by the CDFW to be a Species of Special Concern (SSC) and taxa designated as a special status, sensitive, or declining species by other state or federal agencies.

1.5.2 Federal and State Endangered Species Listings

The Federal and California Endangered Species Acts are the regulatory documents that govern the listing and protection of species, and their habitats, identified as being endangered or threatened with extinction. Possible listing status under both Federal and California ESA includes Endangered and Threatened (FE, FT, CE, or CT). Species in the process of being listed are given the status of either Proposed Federally Endangered/Threatened, Candidate for California Endangered/Threatened (PE, PT, CCE, or CCT). The CESA has one additional status: Rare (CR).

1.5.3 Global and State Ranks

Global and State Ranks reflect an assessment of the condition of the species or habitats across its entire range. Basic ranks assign a numerical value from 1 to 5, respectively for species with highest risk to most secure. Other ranking variations include rank ranges, rank qualifiers, and infraspecific taxon ranks. All Heritage Programs, such as the CNDDB use the same ranking methodology, originally developed by The Nature Conservancy and now maintained and recently revised by NatureServe. Procedurally, state programs such as the CNDDB develop the State ranks. The Global ranks are determined collaboratively among the Heritage Programs for the states/provinces containing the species. Rank definitions, where G represents Global and S represents State, are as follows:

- G1/S1: Critically imperiled globally/in state because of extreme rarity (5 or fewer populations).
- G2/S2: Imperiled globally/in state because of rarity (6 to 20 populations).
- **G3/S3:** Vulnerable; rare and local throughout range or in a special habitat or narrowly endemic (on the order of 21 to 100 populations).
- G4/S4: Apparently secure globally/in state; uncommon but not rare (of no immediate conservation concern).
- **G5/S5:** Secure; common, widespread, and abundant.
- G#G#/S#S#: Rank range numerical range indicating uncertainty in the status of a species, (e.g., G2G3 more certain than G3, but less certain that G2).
- G/S#?: Inexact numeric rank
- Q: Questionable taxonomy Taxonomic distinctiveness of this entity is questionable.
- T#: Infraspecific taxa (subspecies or varieties) indicating an infraspecific taxon that has a lower numerical ranking (rarer) than the given global rank of species.

1.5.4 California Rare Plant Ranks

Plant species are considered rare when their distribution is confined to localized areas, their habitat is threatened, they are declining in abundance, or they are threatened in a portion of their range. The California Rare Plant Rank (CRPR) categories range from species with a low threat (4) to species that are presumed extinct (1A). All but a few species are endemic to California. All of them are judged to be vulnerable under present circumstances, or to have a high potential for becoming vulnerable. Threat ranks are assigned as decimal values to a CRPR to further define the level of threat to a given species. The rare plant ranks and threat levels are defined below.

- 1A: Plants presumed extirpated in California and either rare or extinct elsewhere.
- 1B: Plants rare, threatened, or endangered in California and elsewhere.
- 2A: Plants presumed extirpated in California, but common elsewhere
- 2B: Plants rare, threatened, or endangered in California, but more common elsewhere
- 4: Plants of limited distribution a watch list
- **0.1:** Seriously threatened in California (over 80% of occurrences threatened/high degree and immediacy of threat)
- **0.2:** Moderately threatened in California (20-80% occurrences threatened/moderate degree and immediacy of threat)
- **0.3:** Not very threatened in California (less than 20% of occurrences threatened/low degree and immediacy of threat or no current threats known)

1.5.5 California Department of Fish and Wildlife Animal Rank

The California Department of Fish and Wildlife (CDFW) assigns one of three ranks to Special Animals: Watch List (WL), Species of Special Concern (SSC), or Fully Protected (FP). Unranked species are referred to by the term Special Animal (SA).

Animals listed as Watch List (WL) are taxa that were previously designated as SSC, but no longer merit that status, or taxa that which do not yet meet SSC criteria, but for which there is concern and a need for additional information to clarify status.

Animals listed as California Species of Special Concern (SSC) may or may not be listed under California or federal Endangered Species Acts. They are considered rare or declining in abundance in California. The Special Concern designation is intended to provide the CDFW biologists, land planners, and managers with lists of species that require special consideration during the planning process to avert continued population declines and potential costly listing under federal and state endangered species laws. For many species of birds, the primary emphasis is on the breeding population in California. For some species that do not breed in California but winter here, emphasis is on wintering range. The SSC designation thus may include a comment regarding the specific protection provided such as nesting or wintering.

Animals listed as Fully Protected (FP) are those species considered by CDFW as rare or faced with possible extinction. Most, but not all, have subsequently been listed under the CESA or FESA. Fully Protected species may not be taken or possessed at any time and no provision of the

California Fish and Game code authorizes the issuance of permits or licenses to take any Fully Protected species.

1.5.6 Sensitive Habitats

Sensitive Natural Community is a state-wide designation given by CDFW to specific vegetation associations of ecological importance. Sensitive Natural Communities rarity and ranking involves the knowledge of range and distribution of a given type of vegetation, and the proportion of occurrences that are of good ecological integrity (CDFW 2019a). Evaluation is conducted at both the Global (G) and State (S) levels, resulting in a rank ranging from 1 for very rare and threatened to 5 for demonstrably secure. Natural Communities with ranks of S1-S3 are considered Sensitive Natural Communities in California and may need to be addressed in the environmental review processes of CEQA and its equivalents.

2 METHODS

2.1 Literature and Data Review

Althouse and Meade conducted a data search from the CNDDB and the California Native Plant Society (CNPS) On-line Inventory of Rare and Endangered Plants of California on June 22, 2021 (CDFW 2021a, CNPS 2021). Supplemental occurrence data included online herbarium records maintained by the Consortium of California Herbaria (CCH 2021). The search area included the Greenfield USGS 7.5-minute quadrangle and the 8 surrounding quadrangles (North Chalone Peak, Paraiso Springs, Pinalito Canyon, Reliz Canyon, San Lucas, Soledad, Thompson Canyon, and Topo Valley). Biologists used the compiled data to determine the potential for each sensitive plant and wildlife species to occur within the Study Area. The complete list of species and determinations is provided in Appendix A and Appendix B.

2.2 Sensitive Species Evaluation

Special status species lists produced by database and literature searches were cross-referenced and analyzed according to the described habitat types in the Study Area in order to identify all potential special status species that could occur in or near the Study Area. After review of the literature, and completing site visits, the following criteria were used to determine the potential for special-status species to occur within the Study Area:

- **Present:** The species was observed in the Study Area during field surveys.
- **High Potential:** Highly suitable habitat and CNDDB or CNPS occurrence records indicate the species is likely to occur in the Study Area or the immediate vicinity. Individuals may not have been observed during field surveys; however, the species likely occurs in or immediately adjacent to the Study Area and (for wildlife) could move into the Study Area in the future.
- Moderate Potential: Moderately suitable habitat is present in the Study Area and CNDDB occurrences or surveys have recorded the species in the vicinity of the Study Area. Individuals were not observed during field surveys, but the species could be present, at least seasonally or as a transient.
- Low Potential: Marginally suitable habitat is present in the Study Area, and there are no occurrence records or other historical (i.e., 50 years or older) records in the vicinity of the Study Area. Individuals were not observed during surveys and are not expected to be present.
- **No Potential:** Suitable habitat for the species is not present in the Study Area, and/or the species is not known to occur in the region.

Each special status species that could occur in or near the Study Area is individually discussed in Sections 3.4.1 and 3.5.1.

2.3 Soils

A soil report was created by importing the Study Area as an Area of Interest (AOI) into the Natural Resources Conservation Service (NRCS) Soil Survey Geographic Database (SSURGRO) via their

online portal. The resulting soil report was reviewed, and a map was created using the U.S. Department of Agriculture (USDA) NRCS Soil Survey GIS data (USDA 2020b). Soils data are summarized in Section 3.2.

2.4 Surveys

On June 22, 2021, Althouse and Meade, Inc. Biologist Kristen Andersen conducted a pedestrian survey to inventory plant and wildlife species, describe habitat types, and to collect photographic documentation of the property. Each habitat type was field inspected and described by species composition, as interpreted in Section 3.3. All plant and animal species observed in the field were identified and documented in Sections 3.4.2 and 3.5.2.

The survey method included meandering transects with an emphasis on identifying plants, animals, and habitat types within the Study Area and surrounding areas of the property. Transects were also utilized to describe general conditions and dominant species, compile species lists, and evaluate potential habitat for special status species.

TABLE 1. BIOLOGICAL SURVEY

| Survey Date Biologist | | Weather Observations | Activities |
|--------------------------|------------------|---------------------------------------|---|
| 6/22/2021 | Kristen Andersen | 71-76°F, clear skies, winds 5- 15 mph | Biological survey, habitat mapping, species inventory |

2.4.1 Botanical

Spring botanical surveys were conducted on June 22, 2021 by Kristen Andersen according to agency guidelines (USFWS 2000, CDFW 2018b, and CNPS 2001). All plant species observed on the property were identified and recorded by a qualified botanist. Botanical surveys were appropriately timed to identify all special status plant species known from the region (Table 3) that have potential to occur at the site. Focused survey efforts were conducted in habitats suitable for special status species. Identification of botanical resources included field observations and laboratory analysis of collected material. Botanical nomenclature used in this document follows the Jepson eFlora, with data provided by the participants of the Consortium of California Herbaria (CCH 2021). A list of plants observed in the Study Area and surrounding property were compiled in Table 4.

2.4.2 Wildlife

Identification of wildlife resources were made by direct observations or by visual signs of animal presence such as burrows/dens, vocalization, tracks, and/or scat. Wildlife observations were recorded during the June 22nd field survey and compiled in Table 6. Birds were identified by sight, using 10-power binoculars, or by vocalizations. Reptiles were identified by sight, often using binoculars. Mammals recorded in the Study Area were identified by sight, burrows, dens, scat, and tracks. Wildlife surveys were appropriately timed to identify all special status animal species known from the region that have potential to occur at the site (Table 5). Wildlife nomenclature for birds is in accordance with the American Ornithological Society Checklist (Chesser et al. 2019) and Revised Checklist of North American Mammals North of Mexico (Baker et al. 2003).

2.5 Maps

Mapping efforts utilized Samsung Galaxy Tab 4 tablets equipped with Garmin GLO GPS Receivers and a third-party mapping application. Biological resource habitats were mapped in the field onsite. Hand notation of habitats on high resolution aerials were digitized into polygon layers. Maps were created using aerial photo interpretation, field notation, and spatial data imported to Esri ArcGIS, a Geographic Information System (GIS) software program. Soil data was overlaid on a 2020 National Agriculture Imagery Program (NAIP) aerial of San Luis Obispo County (USDA 2020b).

3 RESULTS

3.1 Existing Conditions

The site is mostly undeveloped and is described as a flat agricultural fallow field that extends northwest from Apple Avenue toward Walnut Avenue. The property contains a main residence and a caretaker's residence in the southwestern portion, accessible from Apple Avenue (Photo 1 and Photo 2). The two residences and associated anthropogenic land uses consist of planted shade trees, the front and back yards, parking, and access. Fallow cropland habitat extends northwest of the residences and has not been actively farmed for several years. It has been planed flat and is compacted. Each habitat is further discussed in Section 3.3 below.



Photo 1. Main residence with abandoned house, view northwest. June 22, 2021.



Photo 2. Caretaker's residence (photo right) with fenced yard, view east. June 22, 2021.

3.2 Soils

Two soil map units are represented within the Study Area: Arroyo Seco gravelly sandy loam 20 to 2 percent slopes and Elder loam gravelly substratum 0 to 2 percent slopes (USDA 2021, Figure 3).

Arroyo Seco gravelly sandy loam 20 to 2 percent slopes (AsA) is the primary soil type represented on the Study Area, accounting for 93 percent. The typical soil profile is gravelly sandy loam (0 to 42 inches) over gravelly coarse sandy loam (42 to 60 inches). This soil class is considered well drained with a very low runoff class. This soil class formed from alluvial fans derived from igneous rock and is classified as prime farmland if irrigated (USDA 2020a).

Elder loam gravelly substratum 0 to 2 percent slopes (EcA) is located in the northwestern corner of the Study Area (approximately 7 percent). The typical soil profile is silty loam, 0 to 40 inches. Elder loam is well drained with a low runoff class. This soil class formed from alluvial fans derived from igneous and sedimentary rock and is classified as prime farmland if irrigated (USDA 2020a).

Figure 3. USDA Soil Survey



PSHH GreenfieldMap Center: 121.23687°W 36.33003°N
Greenfield, Monterey County, California

Source: USDA NRCS Soil Survey



3.3 Habitat Types

Table 2 lists two habitat types described and mapped within the Study Area (Figure 4). Most of the Study Area, approximately 4.16 acres, is fallow cropland. The remaining area consists of approximately 0.74-acre of anthropogenic land uses consisting of residential structures, planted vegetation, and driveways.

TABLE 2. HABITAT TYPES

| Habitat Type | Approximate Area (Acres) |
|-----------------|-----------------------------|
| Fallow Cropland | 4.16 |
| Anthropogenic | 0.74 |
| TOTAL | 4.90 |

3.3.1 Fallow Cropland

Approximately 4.16 acres of fallow cropland (85 percent) is present in the Study Area and is the dominant habitat type on the site. Historical aerials show farming was prevalent in this portion of the site dating back before 1989. Cropland habitat on the property has not been farmed in several years and is currently dominated by weedy forbs and bare ground (Photo 3 and Photo 4). Despite long periods of inactive farming, cropland habitat has not reverted to grassland and remains bare with weedy vegetation that tends to recruit in disturbed areas. Dominant species are Russian thistle (Salsola tragus), cheeseweed (Malva parviflora), foxtail barley (Hordeum murinum), wild mustard (Hirschfeldia incana), oriental rocket (Sisymbriium orientale), bindweed (Convolvulus arvensis), pigweed (Chenopodium murale), and prostrate knotweed (Polygonum aviculare). Soils within cropland habitat appeared to have not been tilled in several years, were planed flat and notably compacted (Photo 3). Very few burrows were observed indicating low presence of burrowing animals. No dens were observed.



Photo 3. Fallow cropland habitat with high percent bare ground, view northwest. June 22, 2021.



Photo 4. Fallow cropland habitat with weedy forbs and view of residential structures in background, view south. June 22, 2021.

3.3.2 Anthropogenic

The Study Area contains approximately 0.74-acre of anthropogenic land uses, defined by an abandoned residential structure with garden and driveway, and currently occupied trailer residence (Figure 4). Residential structures are surrounded by planted non-native trees, including Peruvian pepper trees (*Schinus molle*) and athel (*Tamarix aphylla*). One native palm tree, California fan palm (*Washingtonia filifera*), occurs on the north side of the house. An abandoned garden persists with planted vegetation such as fava beans (*Vicia faba*), tomatillos (*Physalis philadelphica*), and corn (*Zea mays*). Some planted escapees from the garden were observed within fallow cropland habitat. One house cat was observed hunting in the small garden, suggesting small mice or other rodents are present. Trash and debris piles were noted throughout the periphery of each home, which could also provide refugia for small mammals, snakes and lizards. Two European starlings (*Sturnus vulgaris*) were observed carrying nesting material to a Peruvian pepper tree in the northwest corner of anthropogenic habitat and several common bird species were observed vocalizing from the tree canopies.



Photo 5. Abandoned residential structure with small garden in southwest portion of the Study Area, view northwest. June 22, 2021.



Photo 6. Farming equipment and materials staged around abandoned house within anthropogenic habitat on the property, view southeast. June 22, 2021.

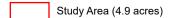
3.4 Potential Wetlands and Jurisdictional Waters

No evidence of wetlands or waters of the U.S. or State of California was observed in the Study Area during site visits conducted in June 2021. Data reviewed in the National Wetland Inventory (NWI) (USFWS 2021) and National Hydrography Dataset (NHD) (USGS 2021) showed no wetland or waters mapped in the Study Area, and supports in-field observations that wetlands or waters do not occur on the Study Area.

Figure 4. Biological Resources



Legend



- California Fan Palm (Washingtonia filifera); Native
- Peruvian Pepper Tree (Schinus molle); Introduced
- Athel (Tamarix aphylla); Introduced

Habitats

Anthropogenic (0.74 acres)

Fallow Cropland (4.16 acres)



PSHH GreenfieldMap Center: 121.23687°W 36.33003°N
Greenfield, Monterey County, California

Biological Survey Date: 06/22/2021



3.5 Botanical Resources

Literature and data base searches of special status plant occurrences within at least five miles of the Project determined 33 special status plant species are known to occur in the region (Appendix A, CDFW 2021b, CNPS 2021). Figure 5 depicts the current GIS data for special status plants mapped near the Study Area by the CNDDB. Figure 7 shows USFWS Critical Habitat designations.

3.5.1 Special Status Plant Species

Based on an analysis of known ecological requirements for the special status plant species reported from the region, and the habitat conditions that were observed in the Study Area, it was determined that one special status plant has low potential to occur within the Study Area: protruding buckwheat (*Eriogonum nudum* var. *inductum*). One species (Monterey spineflower), which is listed under the FESA and occurs within six miles of the Study Area, has no potential to occur on the site. Each species is discussed below and Table 3 summarizes species with potential to occur in the Study Area.

1. Protruding Buckwheat (*Eriogonum nudum* var. *inductum*) is a CRPR 4.2 species endemic to California. It is known to occur in shadscale scrub, foothill woodland, and chaparral habitats with clay soils between 100- and 1,100-meters elevation. It is a perennial herb that typically blooms between May and October. The closest known record is approximately 0.7 miles southwest of the Study Area (CCH #CDA22630) in 1975. The sandy loam and gravelly soils in the Study Area are not suited to support this species, however the nearest occurrence represents an anomaly for this species where protruding buckwheat was observed within a residential neighborhood similar to conditions found in the Study Area. The disturbed, cropland habitat in the Study Area is not likely to support protruding buckwheat and this species has low potential to occur. Protruding buckwheat was not detected in the Study Area during appropriately timed surveys in June 2021.

The remaining special status plant species was determined to have no potential to occur in the Study Area due to lack of suitable habitat present. However, this species is listed as threatened under the Federal Endangered Species Act (FESA), and although it is not expected to occur, Monterey spineflower warrants further discussion:

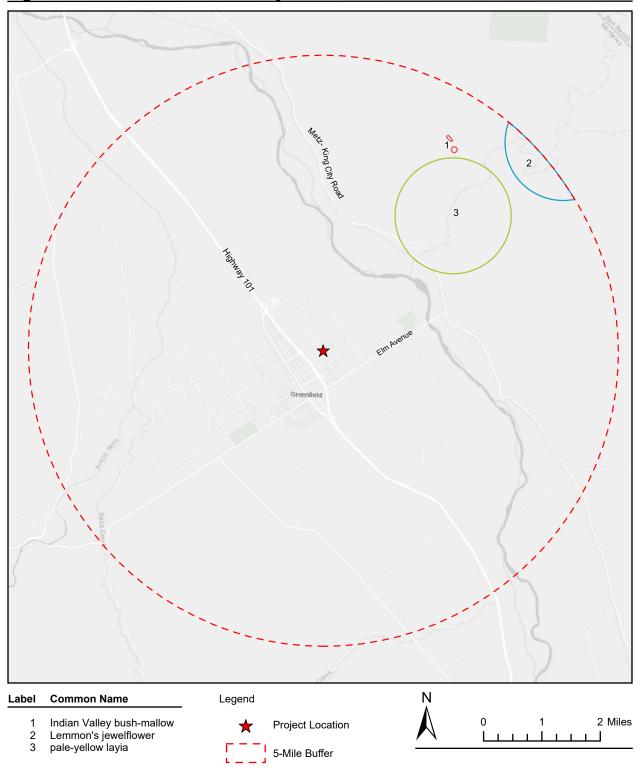
2. Monterey Spineflower (Chorizanthe pungens var. pungens) is listed as Threatened by the Federal Endangered Species Act (FESA) and is a CRPR 1B.2 variety. It is endemic to Santa Cruz, Monterey, and San Luis Obispo Counties. It is known to occur on sandy soils in coastal dunes, maritime chaparral, coastal scrub, cismontane woodland and grassland habitats between 3- and 450-meters elevation. It is an annual herb that typically blooms between April and June (sometimes July and August). The closest known record is approximately 5.4 miles northwest of the Study Area (CNDDB #28) in 2013. Despite the sandy loam soils in the Study Area, the historically disturbed quality of cropland habitat is not suitable for this species, and Monterey spineflower has no potential to occur on the site. Monterey spineflower was not detected in the Study Area during the appropriately timed June 2021 survey.

TABLE 3. SPECIAL STATUS PLANT LIST

| | Scientific Name | Common Name | Federal/State Status Global/State Rank CA Rare Plant Rank | Blooming Period | Habitat Preference | Potential to Occur |
|----|----------------------------------|------------------------|---|--------------------|---|--|
| 1. | Eriogonum nudum var. indictum | Protuding buckwheat | -/- G5T4/S4 4.2 | May-Oct | Clay soils, shadscale scrub, foothill woodland, chaparral | Low. Appropriate habitat with clay soils is not present, however the nearest occurrence is less than one mile from the site (CCH #CDA22630) in 1975. |

See section 1.5 for status and rank definitions.

Figure 5. California Natural Diversity Database Plant Records



PSHH GreenfieldMap Center: 121.23796°W 36.33093°N
Greenfield, Monterey County, California

CNDDB GIS Data Last Updated: June 2021



3.5.2 Botanical Survey Results

Botanical surveys conducted on June 22, 2021 identified 33 species, subspecies, and varieties of vascular plant taxa in the Study Area (Table 4). The list includes five species native to California and 28 introduced (naturalized or planted) species. Native plant species account for approximately 15 percent of the Study Area flora; introduced species account for approximately 85 percent.

TABLE 4. VASCULAR PLANT LIST

| Scientific Name | Common Name | Special Status | Origin | | | | |
|------------------------|--------------------------|-------------------|------------|--|--|--|--|
| Trees - 3 Species | | | | | | | |
| Schinus molle | Peruvian pepper tree | None | Introduced | | | | |
| Tamarix aphylla | Athel | None | Introduced | | | | |
| Washingtonia filifera | California fan palm | None | Native | | | | |
| Shrubs - 4 Species | | | | | | | |
| Baccharis pilularis | Coyote brush | None | Native | | | | |
| Hedera helix | English ivy | None | Introduced | | | | |
| Opuntia littoralis | Prickly pear | None | Native | | | | |
| Pelargonium x hortorum | Garden pelargonium | None | Introduced | | | | |
| Forbs - 21 Species | | | | | | | |
| Amsinckia sp. | Fiddleneck | None | Native | | | | |
| Artemisia douglasiana | Mugwort | None | Native | | | | |
| Bassia hyssopifolia | Five-hook bassia | None | Introduced | | | | |
| Centaurea solstitialis | Yellow star thistle | None | Introduced | | | | |
| Chenopodium album | Lamb's-quarters | None | Introduced | | | | |
| Chenopodium murale | Pigweed | None | Introduced | | | | |
| Convolvulus arvensis | Bindweed | None | Introduced | | | | |
| Erodium cicutarium | Redstem filaree | None | Introduced | | | | |
| Hirschfeldia incana | Wild mustard | None | Introduced | | | | |
| Lactuca serriola | Prickly lettuce | None | Introduced | | | | |
| Malva parviflora | Cheeseweed | None | Introduced | | | | |
| Oenothera speciosa | Mexican evening primrose | None | Introduced | | | | |
| Physalis philadelphica | Tomatillo | None | Introduced | | | | |

| Scientific Name | Common Name | Special Status | Origin |
|------------------------|----------------------|-------------------|------------|
| Polygonum aviculare | Prostrate knotweed | None | Introduced |
| Raphanus sativus | Wild radish | None | Introduced |
| Salsola tragus | Russian thistle | None | Introduced |
| Sisymbrium orientale | Oriental rocket | None | Introduced |
| Sonchus oleraceus | Common sow thistle | None | Introduced |
| Spergularia rubra | Red sand spurrey | None | Introduced |
| Taraxacum officinale | Dandelion | None | Introduced |
| Vicia faba Fava bean | | None | Introduced |
| Graminoids - 5 Species | | | |
| Bromus diandrus | Ripgut brome | None | Introduced |
| Cynodon dactylon | Bermuda grass | None | Introduced |
| Hordeum murinum | Foxtail barley | None | Introduced |
| Stipa tenuissima | Mexican feathergrass | None | Introduced |
| Zea mays Corn | | None | Introduced |

See Section 1.5 for status and rank definitions.

3.6 Wildlife Resources

Literature and data base searches of special status animal occurrences within at least five miles of the Project determined 43 special status animal species are known to occur in the region (Appendix B, CDFW 2021c). Figure 6 and Figure 7 depict the current GIS data for special status species mapped near the Study Area by the CNDDB and USFWS Critical Habitat.

3.6.1 Special Status Animal Species

Based on an analysis of known ecological requirements for the special-status wildlife species reported or known from the region (Appendix B), and the habitat conditions that were observed in the Study Area, it was determined that four special status animal species have low potential to occur within the Study Area (Cooper's hawk, white-tailed kite, Yuma myotis, and bank swallow). Each species is discussed below and summarized in Table 5.

1. Cooper's Hawk (Accipiter cooperii) is a CDFW Watch List species (for nesting occurrences only) that occurs regularly in California during the winter months and during spring and fall migration (CDFW 2018a). It is generally regarded as a regular but uncommon nesting species in San Luis Obispo County (Hall et al. 1992). Cooper's hawks frequent oak and riparian woodland habitats, and increasingly urban areas, where they prey primarily upon small birds (Curtis et al. 2006). The closest reported occurrence of nesting Cooper's hawk is located approximately 10 miles northeast of the Study Area (CNDDB #105), in riparian habitat near

Pinnacles National Park in 2006. Sightings have been reported of Cooper's hawks within less than one mile of the Study Area on eBird (Rinkert 2012), with several observations reported along the Salinas River approximately two miles east of the project (eBird 2021). One occurrence noted breeding behavior with the observance of a fledgling (Davis 2017), signifying that Cooper's hawks may nest more closely to the site than confirmed through the CNDDB. Suitable nesting habitat is not present in the Study Area, but this species could be seen foraging in the area and have a low potential to occur utilizing the site. Cooper's hawks were not observed during our June 2021 site survey.

- 2. White-tailed Kite (Elanus leucurus) is a CDFW Fully Protected species that can be found throughout California but known to forage and nest in certain areas of California in fluctuating numbers (CDFW 2018b; Lehman 2018). The species nests primarily in evergreen trees, especially coast live oaks (Quercus agrifolia), near meadows, marshes, farmlands or grasslands where it forages on small animals, especially voles (Dunk 1995). Communal nocturnal roosts sites, which may shift in location, are often used from early fall to early winter. The closest reported nesting occurrence of white-tailed kite is located approximately 3.5 miles north of the Study Area (CNDDB #155) near Pinnacles National Park in 2007, where an active nest was observed in a coast live oak tree within riparian woodland habitat. Observations of white-tailed kites have also been reported along the Salinas River near Metz Road, east of the project by approximately 2.0 miles (Bailey 2012). Suitable riparian or oak tree nesting habitat is not present in the Study Area but there is potential to find white-tailed kites "kiting" (hovering high above ground) or foraging within the Study Area. Due to the lack of nesting habitat and limited prey-base on site, potential for white-tailed kites to occur in the Study Area is reduced to low. The white-tailed kite was not observed on the property during June 2021 surveys.
- 3. Yuma Myotis (*Myotis yumanensis*) is a Special Animal tracked by the California Department of Fish and Game. The Yuma myotis is a small bat widely distributed throughout western North America. It is the species of bat most commonly associated with man-made structures. It is often associated with permanent water sources. Crevices are preferred roost areas including those found in cliffs, buildings and bridges, although it will also roost in tree cavities (Bogan et al. 2005). The species emerges after sunset and forages on insects. Yuma myotis has been recorded at seven localities within San Luis Obispo County (Pierson, 2002). Althouse and Meade, Inc. (A&M) biologists working with Paul Collins of the Santa Barbara Museum of Natural History identified this species acoustically in the Santa Margarita area in 2003. Yuma myotis could occur in the abandoned residential structure on the subject property, though roosting near a water source is preferred. The nearest CNDDB occurrence is approximately 11 miles north of the Study Area in Pinnacles National Park in 2002 (CNDDB #79). With the Salinas River to the east, it is possible that Yuma myotis could roost or forage within the Study Area, and this species has low potential to occur.
- **4. Bank Swallow** (*Riparia riparia*) is a state-listed threatened species with a Global Rank of G5 (Secure) and a State Rank of S2 (Imperiled). It typically nests in colonies, excavating tunnels into vertical sandbanks along rivers, streams, lakes, and ocean coasts. This species forages over any habitat, especially near water. The closest reported observation of bank swallow is historic, with an observation radius that overlaps with the Study Area, observed in 1972 (CNDDB #68). More recent observations made on eBird include a sighting within two miles east of the Study Area at a potential breeding site on a bank above Metz Road in 2015

(Rinkert 2015). The disturbed quality and cropland habitat in the Study Area provides low suitability for foraging and nesting for this species. Bank swallows were not observed during our June 2021 survey.

The remaining special status animal species was determined to have a discountable potential to occur in the Study Area due to low quality habitat and long term absence from the region. However, this species is listed as endangered under the Federal Endangered Species Act (FESA), and although they are not expected to occur, San Joaquin kit fox warrant further discussion:

5. San Joaquin Kit Fox (Vulpes macrotis mutica; SJKF) is federally listed as endangered and state listed as threatened. The SJKF is one of two subspecies of the kit fox, *Vulpes macrotis*, which is the smallest canid species in North America. It is endemic to the San Joaquin Valley and a few adjacent valleys in the central region of California (Cypher et al. 2013). The SJKF is primarily nocturnal and typically occurs in annual grassland or mixed shrub/grassland habitats throughout low, rolling hills and in valleys. They need loose sandy soils in order to dig their burrows and a prey population of black-tailed jackrabbits, rodents, desert cottontails, insects, some birds, reptiles and vegetation (CDFW 2014, CNDDB 2017). The most suitable habitat for SJKF has low precipitation, sparse vegetation coverage with high densities of kangaroo rats (*Dipodomys* spp.). For the SJKF to succeed in an area it needs large expanses of non-fragmented suitable habitat. This type of habitat is decreasing rapidly by conversion into agricultural land or degraded by urban development (Cypher et al. 2013). Female SJKF began preparing natal dens in September and October and then breeding occurs from December through February. Pups are born from January to March and family groups typically split up the following October (Meaney et al. 2006). The closest reported occurrence of the SJKF located approximately 2.0 miles from the project (CNDDB #1013), in 1975 along Metz Road, northeast of the Salinas River. This historical occurrence is one of several in the vicinity, all reported in the same year (1975). Two more recent occurrences within the ninequad CNDDB search are located more than 12 miles to the east (CNDDB #180) and 10 miles south (CNDDB #939) of the Study Area, reported in 1993 and 1988, respectively. These more recent occurrences were in areas geographically separated from the Study Area by valleys and canyons situated in the Diablo Range. Other supporting documents, such as the Historical Range figure (USFWS 2020), show that SJKF have not been documented in the area since before 1990, with recent occurrences (2006 to present) generally located to the east and south in the interior valleys, with the exception of a the most recent report in 2007 at the California National Guard post at Camp Roberts. Due to the lack of suitable grassland habitat and no recent occurrences near the Study Area, it is our professional opinion that kit fox have no potential to occur. The SJKF, or sign of SJKF, was not observed on the property during the 2021 site surveys and is not likely to be present.

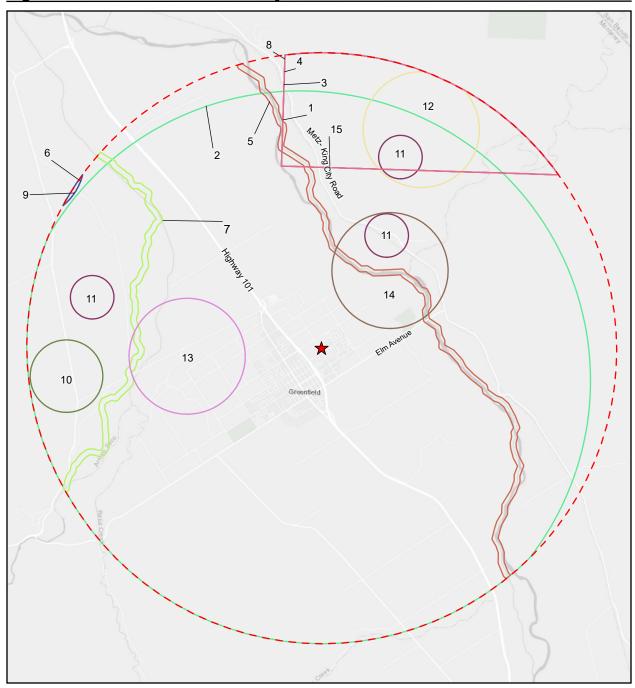
TABLE 5. SPECIAL STATUS ANIMAL LIST

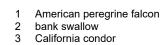
| | Scientific Name | Common Name | Federal/State Status Global/State Rank CDFW Status | Habitat Preference | Potential to Occur |
|----|---|---|--|---|--|
| 1. | Accipiter cooperii Cooper's Hawk -/- Oak woodland, riparian, open fields. G5/S4 Nests in dense trees, esp. coast live oak. | Accipiter cooperii Cooper's Hawk | Nests in dense trees, esp. coast live | No Potential (nesting). Appropriate woodland habitat is not present in the Study Area for nesting. | |
| | | | WL | L b ar S | Low (foraging). Cooper's hawks have been observed foraging in the vicinity and could be seen in flight over the Study Area, though foraging prey-base is limited on the site. |
| 2. | Elanus leucurus | Elanus leucurus White-Tailed Kite -/- Nests in dense tree canopy near open foraging areas FP | 1,0 | No Potential (nesting). Suitable | |
| | | | G5/S3S4 | foraging areas | nesting habitat of open-country trees or trees among forest or woodland |
| | | | FP | | edge is not present in the Study Area. |
| | | | | could be u nearby ne | Low (foraging). The Study Area could be utilized for forage, with nearby nesting occurrence within 3.5 mi north (CNDDB #155) in 2007. |
| 3. | Myotis yumanensis | Yuma Myotis | -/- | rock crevices, or under bridges. Feeds structure and s | Low. The abandoned residential |
| | | | G5/S4 | | structure and surrounding trees could provide roosting habitat, though |
| | | | SA | | foraging habitat (open water) is over 2 miles from the Study Area. |
| 4. | Riparia riparia | Bank Swallow | -/CT | Nests colonially in riparian and other | No Potential (nesting). Appropriate |
| | | G5/ | G5/S2 | lowland habitats west of the desert. Requires vertical banks or cliffs with sandy soils (to dig cavities) near streams, lakes, or the ocean. | riparian nesting habitat with vertical banks is not present in the Study Area. |
| | | | SA | | Low (foraging). Bank swallows have been documented in the area and could be transient over the site or utilize the site when foraging. |

| | Scientific Name | Common Name | Federal/State Status Global/State Rank CDFW Status | Habitat Preference | Potential to Occur |
|----|---------------------------|-------------|--|--|--|
| 5. | Vulpes macrotis mutica | | FE/CT | Annual grasslands or grassy open | No Potential. Appropriate open |
| | | | G4T2/S2 | stages with scattered shrubby vegetation. Needs loose textured | grassland habitat is not present and the mapped historical range for kit fox |
| | | | SA | sandy soil and prey base. | shows no observations in the area |
| | | | | | beyond 1990 (CDFW 2020). |

See section 1.5 for status and rank definitions.

Figure 6. California Natural Diversity Database Animal Records





Common Name

- golden eagle
- 5 Monterey hitch
- pallid bat

Label

Pinnacles optioservus Riffle beetle

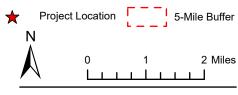
Label

- prairie falcon 8 Salinas pocket mouse
- San Joaquin coachwhip 10

Common Name

- 11 San Joaquin kit fox
- Townsend's big-eared bat 12
- 13 western bumble bee
- western spadefoot 14 15 white-tailed kite

Legend

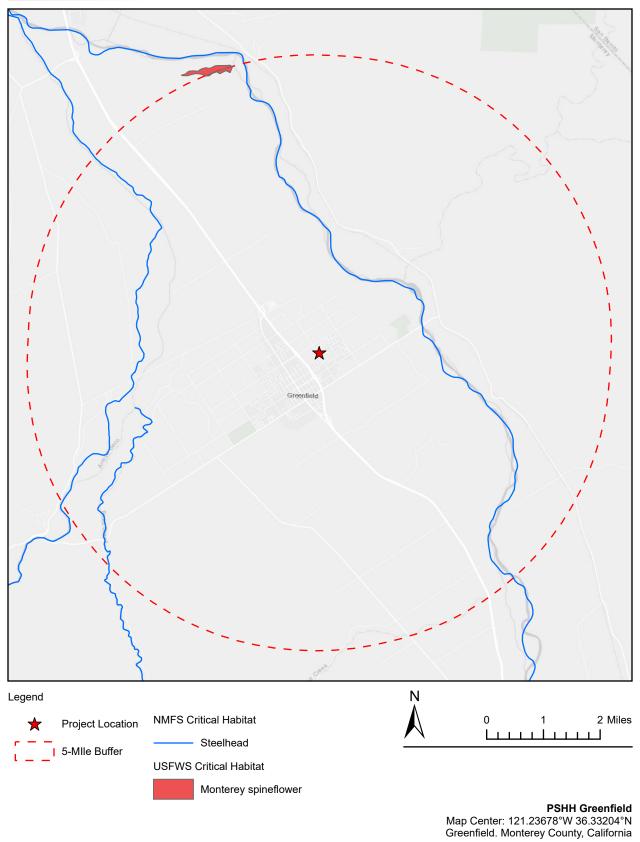


PSHH Greenfield Map Center: 121.23772°W 36.3303°N Greenfield, Monterey County, California

CNDDB GIS Data Last Updated: June 2021



Figure 7. United States Fish and Wildlife Service Critical Habitat



USFWS Critical Habitat Data Last Updated: August 14, 2019



3.6.2 Wildlife Survey Results

A total of 10 wildlife taxa were observed within the Study Area during the June 2021 surveys: nine birds, and one mammal. Table 6 provides a list of the wildlife observed in the Study Area. Several common bird species were observed utilizing the trees surrounding the residential structure, and two European starlings were observed carrying nesting material to a Peruvian pepper tree (*Schinus molle*) located by the northwest corner of the house. Very few burrows were observed across the site that appeared to be old gopher burrows, none of which were active. One house cat (*Felis catus*) was observed hunting in the abandoned garden area.

TABLE 6. WILDLIFE LIST

| Scientific Name | Common Name | Special Status | Habitat Type |
|-----------------------------|---------------------------|----------------|---------------------------------------|
| Birds - 9 Species | | | |
| Calypte anna | Anna's Hummingbird | None | Many habitats |
| Corvus brachyrhynchos | American Crow | None | Many habitats, esp. urban |
| Hirundo rustica | Barn Swallow | None | Riparian, grasslands, lakes |
| Melospiza melodia | Song Sparrow | None | Oak, riparian woodland |
| Mimus polyglottos | Northern Mockingbird | None | Riparian, chaparral, woodlands, urban |
| Petrochelidon pyrrhonota | Cliff Swallow | None | Urban; open areas near water |
| Streptopelia decaocto | Eurasian Collared Dove | None | Urban areas |
| Sturnus vulgaris | European Starling | None | Agricultural, livestock areas |
| Zenaida macroura | Mourning Dove | None | Open and semi-open habitats |
| Mammals – 1 Species | | | |
| Felis catus | Feral Cat | None | Varied |

See Section 1.6 for status and rank definitions.

3.6.3 Habitat Connectivity and Wildlife Movement

Wildlife movement corridors are defined as areas that connect suitable wildlife habitat areas in a region otherwise fragmented by rugged terrain, changes in vegetation, or human disturbance. Natural features such as canyon drainages, ridgelines, or areas with vegetation cover provide corridors for wildlife travel. Wildlife movement corridors are important because they provide access to mates, food, and water; allow the dispersal of individuals away from high population density areas; and facilitate the exchange of genetic traits between populations (Beier and Loe 1992). Wildlife movement corridors are considered sensitive by resource and conservation

agencies, including Monterey County. The Salinas River to the east of the proposed Project could provide connectivity to resources between the Diablo and Santa Lucia Ranges, however fragmentation and development likely detour movement around the City of Greenfield itself. Residential communities surround the Project site, thereby reducing potential for movement through the area. Although it is reasonable to assume that wildlife movement may occur locally within the Project area, the Project area does not provide a throughway for wildlife species to offsite areas of habitat and therefore does not function as a significant regional corridor.

4 ENVIRONMENTAL IMPACT ANALYSIS AND MITIGATION

There are two types of habitats present within the 4.9-acre Study Area: fallow cropland and anthropogenic. The Project could affect nesting birds, and special status bats (*Yuma myotis*). This section provides mitigation recommendations (**BIO**) designed to reduce impacts to biological resources onsite to less than significant, as summarized by Table 7.

TABLE 7. IMPACTS AND MITIGATION SUMMARY

| Biological Resource | Potential Effect from Proposed Project | Mitigation Measure |
|------------------------------|---|-------------------------------------|
| Fallow Cropland Habitat | Less than Significant | None See BIO-1 for nesting birds |
| Anthropogenic Habitat | Less than Significant | None See BIO-1 for nesting birds |
| Special Status Plants | No Effect | None |
| Nesting Birds | Less than Significant with Mitigation Incorporated | Preconstruction Surveys BIO-1 |
| Cooper's Hawk | No Effect (nesting) | None |
| White-tailed Kite | Negligible (foraging) | |
| Bank Swallow | | |
| Yuma myotis | Less than Significant with | Bat Surveys |
| | Mitigation Incorporated | BIO-2, BIO-3, BIO-4 |
| San Joaquin Kit Fox | None anticipated. | BIO-5 |
| | Less than Significant with Mitigation Incorporated | |
| Wildlife Corridors | No Effect | None |

4.1 Habitats

The proposed Project would impact up to 4.16 acres of fallow cropland habitat and 0.74 acre anthropogenic habitat during development of residential housing, landscaping, utilities, and parking (Figure 8). Final site plans will determine the extent of impacted agricultural and ruderal habitats and will include any temporary impacts that might occur during construction of the permanent infrastructure. Fallow cropland and anthropogenic habitats are not classified sensitive communities by CDFW or CNPS definition and impacts to these habitats are not considered significant, except where these habitat impacts affect other sensitive biological resources such as sensitive animals or nesting birds (see Section 4.4).

Figure 8. Biological Resources Impacts





4.2 Potential Wetlands and Jurisdictional Waters

No Waters of the U.S. or Waters of the State were observed. No mitigation is required for impacts to wetlands or waters.

4.3 Botanical Resources

No special status plants were detected during appropriately timed botanical surveys conducted in June 2021. No mitigation is required for botanical resources.

4.4 Wildlife Resources

4.4.1 Nesting Birds

Impacts to or take of nesting birds could occur if Project activities (i.e., removal of onsite structures and/or non-native trees) are conducted during nesting season (February 15 through August 31; CDFW). To reduce potential adverse effects of the proposed Project on nesting birds, the following mitigation measure is recommended.

BIO-1. Preconstruction Nesting Bird Survey. If ground or vegetation disturbing activities commence between February 15 and August 31, preconstruction nesting bird surveys shall be conducted within one week (7 days) of starting work. Surveys shall cover the entire work area plus a 100-foot buffer for non-raptor, common bird species. If surveys do not locate nesting birds, construction activities may commence. If an active bird nest (a nest with eggs or young) is located, a protective buffer shall be established by a qualified biologist. The buffer shall consist of a 50-foot radius no work area around the nest until the chicks have fledged and are no longer dependent on the nest. The qualified biologist may increase or decrease the buffer on a case-by-case basis in consultation with the City, if the species, location, topography, or work scope support the determination. A preconstruction survey report shall be submitted to the City immediately upon completion of the survey, and prior to start of work. The report shall detail appropriate fencing or flagging of buffer zones if applicable. A map of the project site and nest locations shall be included with the report.

4.4.2 Special Status Birds

Cooper's hawk, white-tailed kite, and bank swallow have no potential to nest on the property but could utilize the site for forage. Mitigation is only necessary to protect these species when nesting and impacts would be negligible to special status birds when foraging. No further mitigation is required for special status birds.

4.4.3 Special Status Bats

Special status bat species, Yuma myotis, and common bat species have potential to roost in existing, abandoned structures and in tree snags of mapped trees in the Study Area. To reduce impacts to potential roosting bat colonies, the following mitigation measures are recommended.

- BIO-2. Prior to demolition of structures or removal of large trees, a qualified biologist shall conduct a survey of existing structures and trees on the Property to determine if roosting bats are present. If possible, the survey shall be conducted during the non-breeding season (November through March). Surveys may include installation of bat detector technology to confirm presence and identify potential bat species. The biologist shall have access to all interior attics, as needed. If a colony of bats is found roosting in any structure, further surveys shall be conducted sufficient to determine the species present and the type of roost (day, night, maternity, etc.). If the bats are not part of an active maternity colony, passive exclusion measures may be implemented with approval from CDFW. November is the best time of the year to exclude bats from a roost because it is after the breeding season and before winter hibernation (not all species hibernate).
- **BIO-3.** If bats are roosting in a structure on the Property during the daytime but are not part of an active maternity colony, then exclusion measures must include one-way valves that allow bats to get out but are designed so that the bats may not re-enter the structure.

4.4.4 San Joaquin Kit Fox

SJKF are very unlikely to occur in the Study Area. To ensure that incidental take of SJKF does not occur, the following mitigation measure is provided (MM 3.4-2 extracted from the City of Greenfield, Mitigation Monitoring and Reporting Program; Baker 2016):

- **BIO-4.** During construction activities the project applicant shall use "best management practices" to ensure no incidental take of SJKF occurs during construction or from project-related activity onsite. The recommended measures (as outlined in the USFWS Standardized Recommendations for the Protection of the SJKF Prior to or During Ground Disturbance [June 1999]) include:
 - a. Restrict project-related vehicle traffic to established roads or other designated areas onsite. Vehicles should observe a 20-mile per hour speed limit in all project areas (except on paved pre-existing roads with an established speed limit). Off-road traffic outside of the designated project areas should be prohibited;
 - b. To the extent possible, night-time construction should be minimized;
 - c. All excavated, steep-walled holes or trenches more than two feet deep shall be covered at the close of each working day by plywood or similar materials or provided with one or more escape ramps constructed of earth fill or wooden planks. Before such holes or trenches are filled, each shall be thoroughly inspected for trapped animals that should be allowed to escape before proceeding;
 - d. All construction pipes, culverts, or similar structures with a diameter of 4-inches or greater that are stored open onsite for one or more nights shall be thoroughly

inspected for animals before the pipe is subsequently buried, capped, or otherwise used or moved in any way;

- e. All food-related trash items, such as wrappers, cans, bottles, and food scraps, shall be disposed of in closed containers and removed at least once a week from the project site;
- f. No firearms shall be allowed on the project site;
- g. No pets (i.e., dogs, cats, etc.) shall be permitted onsite;
- h. Use of rodenticides and herbicides in project areas shall be prohibited. If rodent control must be conducted, zinc phosphide is preferred because of a proven (and recognized by the USFWS) lower risk to kit fox.

Furthermore, the applicant shall retain a qualified biologist to present the importance of following best management practices to reduce impacts to possible fox (as well as other sensitive species) during project implementation. A fact sheet conveying this information shall be prepared by the biologist and distributed to any personnel who may enter the project site. Should a kit fox be found onsite, the biologist shall be notified immediately in order to outline additional avoidance measures that should be implemented as well as consult with regulatory agencies.

4.4.5 Habitat Connectivity and Wildlife Movement

This Project does not propose impacts that would impede or block wildlife from utilizing this site for movement; therefore, no mitigation measures are recommended.

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6 APPENDICES

- Appendix A. Special Status Plants Reported from the Region
- Appendix B. Special Status Animals Reported from the Region

APPENDIX A. SPECIAL STATUS PLANTS REPORTED FROM THE REGION

| | Scientific Name | Common Name | Federal/State Status Global/State Rank CA Rare Plant Rank | Blooming Period | Habitat Preference | Potential to Occur |
|----|--|----------------------------|---|--------------------|--|---|
| 1. | Acanthomintha obovata ssp. obovata | San Benito Thorn- Mint | -/- G4T3T4/S3S4 4.2 | Apr-Jul | Grassy slopes, oak woodland, chaparral, vertic clay, occasionally serpentine | No Potential. Appropriate habitat with suitable soils is not present in the Study Area. |
| 2. | Amsinckia douglasiana | Douglas' Fiddleneck | -/- G4/S4 4.2 | Mar-May | Valley and foothill grassland. Dry habitats with unstable shaly sedimentary slopes. 150-1600 m. | No Potential. Appropriate habitat with suitable soils is not present in the Study Area. |
| 3. | Astragalus macrodon | Salinas Milk-Vetch | -/- G4/S4 4.3 | Apr-Jul | Eroded pale shales or sandstone, serpentine alluvium | No Potential. Suitable soils are not present in the Study Area. |
| 4. | Astragalus nuttallii var. nuttallii | Ocean Bluff Milk- Vetch | -/- G4T4/S4 4.2 | Jan-Nov | Coastal bluffs, dunes. Sandy soils. <250 m. | No Potential. Appropriate coastal habitat is not present in the Study Area. |
| 5. | Caulanthus lemmonii | Lemmon's Jewelflower | -/- G3/S3 1B.2 | Feb-May | Grassland, chaparral, scrub | No Potential. Appropriate habitat it's not present in the Study Area. |
| 6. | Centromadia parryi ssp. congdonii | Congdon's Tarplant | -/- G3T1T2/S1S2 1B.1 | May-Nov | Grassland, disturbed sites. Terraces, swales, floodplains, Alkaline, heavy clay soil <300 m. | No Potential. Suitable soils and depressional features are not present in the Study Area. |
| 7. | Chorizanthe biloba var. immemora | Hernandez Spineflower | -/- G3T1T2/S1S2 1B.2 | May-Sep | Serpentine, gravel, vertic clay | No Potential. Suitable soils are not present in the Study Area. |

| | Scientific Name | Common Name | Federal/State Status Global/State Rank CA Rare Plant Rank | Blooming Period | Habitat Preference | Potential to Occur |
|-----|-------------------------------------|-------------------------|---|--------------------|---|---|
| 8. | Chorizanthe douglasii | Douglas' Spineflower | -/- G4/S4 4.3 | Apr-Jul | Cismontane woodland, lower montane coniferous forest, chaparral, coastal scrub, valley and foothill grassland; | No Potential. Appropriate habitat is not present and the heavily disturbed land use in the Study Area is not suitable |
| 9. | Chorizanthe pungens var. pungens | Monterey Spineflower | FT/- G2T2/S2 1B.2 | Apr-Aug | in sand or gravel. Sand | to support this species. No Potential. Suitable sandy substrate in wash habitat is not present in the Study Area. Nearest occurrenceis over 5 miles northwest (CNDDB #28). |
| 10. | Chorizanthe robusta var. robusta | Robust Spineflower | FE/- G2T1/S1 1B.1 | Apr-Sep | Sand or gravel, dunes, openings, coastal | No Potential. Appropriate sandy coastal habitat is not present in the Study Area. |
| 11. | Clarkia breweri | Brewer's Clarkia | -/- G4/S4 4.2 | Apr-Jun | Chaparral, talus, occasionally serpentine | No Potential. Appropriate habitat with suitable soils is not present in the Study Area. |
| 12. | Clarkia jolonensis | Jolon Clarkia | -/- G2/S2 1B.2 | Apr-Jun | Dry woodland | No Potential. Appropirate woodland habitat is not present in the Study Area. |
| 13. | Clarkia lewisii | Lewis' Clarkia | -/- G4/S4 4.3 | May-Jul | Coastal scrub, woodland, chaparral | No Potential. Appropriate habitat is not present in the Study Area. |
| 14. | Clinopodium mimuloides | Monkey-Flower Savory | -/- G3/S3 4.2 | Jun-Oct | Moist places, streambanks, chaparral, woodland | No Potential. Appropriate habitat with mesic conditions is not present in the Study Area. |

A-2

| | Scientific Name | Common Name | Federal/State Status Global/State Rank CA Rare Plant Rank | Blooming Period | Habitat Preference | Potential to Occur |
|-----|-------------------------------|---------------------|---|--------------------|---|--|
| 15. | Collinsia multicolor | San Francisco | -/- | Feb-May | Moist, +- shady scrub, forest | No Potential. Appropriate |
| | | Collinsia | G2/S2 | | | habitat with mesic conditions is not present in the Study |
| | | | 1B.2 | | | Area. |
| 16. | . Convolvulus | Small-Flowered | -/- | Mar-Jul | Clay substrates, occasionally | No Potential. Appropriate |
| | simulans | Morning-Glory | G4/S4 | | serpentine, annual grassland, coastal-sage scrub, chaparral | habitat and soils are not present in the Study Area to |
| | | | 4.2 | | | support this species. |
| 17. | Cryptantha rattanii | Rattan's Cryptantha | -/- | Apr-Jul | Rocky, gravelly slopes, | No Potential. Appropriate |
| | | | G4/S4 | | grassland, coastal scrub, chaparral, foothill woodland | sloping habitat is not present in the Study Area. |
| | | | 4.3 | | chaparai, roothiir woodiana | in the Study Med. |
| 18. | Delphinium | 1 . | -/- | Apr-Jun | Generally slopes in open woodland, eastern side of coast ranges | No Potential. Appropriate |
| | californicum ssp. interius | | G3T3/S3 | | | sloping woodland habitat is not present in the Study Area. |
| | | | 1B.2 | | | |
| 19. | Delphinium | Recurved Larkspur | -/- | Mar-Jun | Poorly drained, fine, alkaline | No Potential. Appropirate |
| | recurvatum | | G2?/S2? | | soils in grassland, Atriplex scrub | grassland habitat with alkaline soils is not present in |
| | | | 1B.2 | | seruo | the Study Area. |
| 20. | Delphinium | Umbrella Larkspur | -/- | Apr-Jun | Moist oak forest | No Potential. Appropriate |
| | umbraculorum | | G3/S3 | | | forest habitat is not present in the Study Area. |
| | | | 1B.3 | | | the Study Area. |
| 21. | Eriogonum | Butterworth's | -/CR | Jun-Jul | Sandstone | No Potential. Heavily |
| | butterworthianum | Buckwheat | G2/S2 | | | disturbed land use in the Study Area is not suitable to |
| | | | 1B.3 | | | study Area is not suitable to support this species. |

| | Scientific Name | Common Name | Federal/State Status Global/State Rank CA Rare Plant Rank | Blooming Period | Habitat Preference | Potential to Occur |
|-----|---|---------------------------------|---|--------------------|---|--|
| 22. | Eriogonum elegans | Elegant Wild Buckwheat | -/- G4G5/S4S5 4.3 | May-Nov | Uncommon. Cismontane woodland, valley and foothill grassland. Usually in sandy or gravelly substrates; often in washes, sometimes roadsides. | No Potential. Appropriate habitat is not present in the Study Area and nearest occurrence is over 9 miles southeast (CCH # SBBG179105) in 1931. |
| 23. | Eriogonum heermannii var. occidentale | Western Heermann's Buckwheat | -/- G5T2/S2 1B.2 | Jul-Oct | Gravel bars, steep, clay slopes, often serpentine | No Potential. Appropriate soils and sloping habitat is not present in the Study Area. |
| 24. | Eriogonum nortonii | Pinnacles Buckwheat | -/- G2/S2 1B.3 | Apr-Sep | Sand | No Potential. Heavily disturbed land use in the Study Area is not suitable to support this species. |
| 25. | Eriogonum nudum var. indictum | Protuding buckwheat | -/- G5T4/S4 4.2 | May-Oct | Clay soils, shadscale scrub, foothill woodland, chaparral | Low . Appropriate habitat with clay soils is not present, however the nearest occurrence is less than one mile from the site (CCH #CDA22630) in 1975. |
| 26. | Lagophylla diabolensis | Diablo Range Hare- Leaf | -/- G2/S2 1B.2 | Apr-Sep | Grassy openings in woodland, vertic clay | No Potential. Appropriate habitat with clay soils is not present in the Study Area. |
| 27. | Layia heterotricha | Pale-Yellow Layia | -/- G2/S2 1B.1 | Mar-Jun | Open clayey or sandy soil, sometimes +- alkaline | No Potential. Suitable soils are not present in the Study Area to support this species. |

| | Scientific Name | Common Name | Federal/State Status Global/State Rank CA Rare Plant Rank | Blooming Period | Habitat Preference | Potential to Occur |
|-----|----------------------------|---------------------|---|--------------------|---|--|
| 28. | Malacothamnus | Indian Valley Bush- | -/- | Apr-Oct | Open rocky slopes | No Potential. Appropriate |
| | aboriginum | Mallow | G3/S3 | | | rocky sloping habitat is not present in the Study Area. |
| | | | 1B.2 | | | present in the study rinem |
| 29. | Malacothamnus | Davidson's Bush- | -/- | Jun-Jan | Sandy washes in coastal | No Potential. Appropriate |
| | davidsonii | Mallow | G2/S2 | | scrub, riparian woodland, chaparral | habitat with sandy washes is not present in the Study |
| | | | 1B.2 | | Chapartai | Area. |
| 30. | 0. Plagiobothrys uncinatus | Hooked | -/- | Apr-May | Chaparral, canyon sides, rocky outcrops, +- fire follower | No Potential. Appropriate chaparral and canyon habitat is not present in the Study Area. |
| | | Popcornflower | G2/S2 | | | |
| | | | 1B.2 | | | |
| 31. | Senecio aphanactis | Chaparral Ragwort | -/- | Jan-May | Alkaline flats, dry open | No Potential. Appropriate alkaline soils are not present |
| | | | G3/S2 | | rocky areas | |
| | | | 2B.2 | | | in the Study Area. |
| 32. | Senecio astephanus | San Gabriel Ragwort | -/- | May-Jul | Steep rocky slopes in | No Potential. Appropriate |
| | | | G3/S3 | | chaparral/coastal-sage scrub and oak woodland | habitat is not present in the Study Area. |
| | | | 4.3 | | and oak woodiand | Study Area. |
| 33. | Sidalcea hickmanii | Hickman's | -/- | May-Jul | Chaparral | No Potential. Appropriate |
| | ssp. hickmanii | Checkerbloom | G3T2/S2 | | | chaparral habitat is not present in the Study Area. |
| | | | 1B.3 | | | |

State/Rank Abbreviations:

FE: Federally Endangered

FT: Federally Threatened

PE: Proposed Federally Endangered

PT: Proposed Federally Threatened

CE: California Endangered

CR: California Rare

CT: California Threatened

CCE: Candidate for California Endangered

CCT: Candidate for California Threatened

California Rare Plant Ranks:

CRPR 1A: Plants presumed extirpated in California and either rare or extinct elsewhere

CRPR 1B: Plants rare, threatened, or endangered in California and elsewhere

CRPR 2A: Plants presumed extirpated in California, but common elsewhere

CRPR 2B: Plants rare, threatened, or endangered in California, but more common elsewhere

CRPR 4: Plants of limited distribution - a watch list

0.1 - Seriously threatened in California (over 80% of occurrences threatened / high degree and immediacy of threat)

0.2 - Moderately threatened in California (20-80% occurrences threatened / moderate degree and immediacy of threat)

 $0.3 - Not \ very \ threatened \ in \ California \ (less \ than \ 20\% \ of \ occurrences \ threatened \ / \ low \ degree \ and \ immediacy \ of \ threat \ or \ no$

current threats known)

Global/State Ranks:

G1/S1 - Critically Imperiled

G2/S2 – Imperiled

G3/S3 - Vulnerable G4/S4 - Apparently Secure

G5/S5 - Secure

Q – Element is very rare but there are taxonomic questions associated with it.

Range rank – (e.g., S2S3 means rank is somewhere between S2 and S3)

? – (e.g., S2? Means rank is more certain than S2S3 but less certain that S2)

APPENDIX B. SPECIAL STATUS ANIMALS REPORTED FROM THE REGION

| | Scientific Name | Common Name | Federal/State Status Global/State Rank CDFW Status | Habitat Preference | Potential to Occur |
|----|--------------------|--|---|---|---|
| 1. | Accipiter cooperii | Cooper's Hawk | -/- | Oak woodland, riparian, open fields. | No Potential (nesting). Appropriate |
| | | | G5/S4 | Nests in dense trees, esp. coast live oak. | woodland habitat is not present in the Study Area for nesting. |
| | | | WL | | Low (foraging). Cooper's hawks have been observed foraging in the vicinity and could be seen in flight over the Study Area, though foraging prey-base is limited on the site. |
| 2. | Accipiter striatus | Sharp-Shinned | -/- | Riparian, coniferous, and deciduous | No Potential. Appropriate woodland |
| | | Hawk G5/S4 woodlands near water. WL | woodlands near water. | habitat is not present in the Study Area. | |
| | | | WL | | |
| 3. | Agelaius tricolor | Tricolored | -/CT | Requires open water, protected nesting | No Potential. Appropriate riparian |
| | | Blackbird | G2G3/S1S2 | | nesting habitat and water sources are not present in the the Study Area. |
| | | | SSC | 1 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, | r |
| 4. | | California Tiger | FT/CT | Need underground refuges, ground | No Potential. Seasonal water sources are |
| | californiense | Salamander | G2G3/S2S3 | squirrel burrows & vernal pools or other seasonal water for breeding. | not present and the Study Area is not within dispersal range between any |
| | | | WL | S | known breeding ponds. |
| 5. | Anniella pulchra | Northern California | -/- | Sandy or loose loamy soils under coastal | No Potential. Appropriate scrub or oak |
| | | Legless Lizard | Legless Lizard scrub or oak trees. Soil moisture essential | essential. | tree habitat with leaf litter and soil moisture is not present in the Study |
| | | | SSC | | Area. |
| 6. | Antrozous pallidus | Pallid Bat | -/- | Rock crevices, caves, tree hollows, | No Potential. The disturbed quality of |
| | | G5/S3 mines, old buildings, and bridges. | cropland habitat in the Study Area is not suitable foraging or roosting habitat for | | |
| | | | SSC | pallid bats. | |

| S | Scientific Name | Common Name | Federal/State Status Global/State Rank CDFW Status | Habitat Preference | Potential to Occur | |
|--------------|--------------------|-------------------|---|--|--|---|
| 7. A | Aquila chrysaetos | Golden Eagle | -/- | Nests in large, prominent trees in valley | No Potential. Nesting and foraging | |
| | | | G5/S3 | and foothill woodland. Requires adjacent food source. | habiat is not present in the Study Area. | |
| | | | FP | adjacent 100a Boaree. | | |
| 8. A | Ardea herodias | Great Blue Heron | -/- | Rookeries located in tall trees near | No Potential. Appropriate rookery | |
| | | | G5/S4 | foraging areas. | habitat is not present in the Study Area. | |
| | | | SA | | | |
| 9. A | sio otus | Long-Eared Owl | -/- | Riparian with tall willows and | No Potential. Appropriate riparian | |
| | | | G5/S3? | cottonwoods; CLOs paralleling streams; requires adjacent open land for hunting and presence of old crow, magpie, or raptor nests | | habitat is not present in the Study Area. |
| | | | SSC | | | |
| 10. A | Athene cunicularia | Burrowing Owl | -/- | Burrows in squirrel holes in open | No Potential. The disturbed quality of | |
| | | | G4/S3 | | cropland habitat in the Study Area is not suitable for this species, and compacted | |
| | | | SSC | | soils are not conducive to burrowing. | |
| | Bombus | Obscure Bumble | -/- | Open coastal grasslands and meadows. | No Potential. Appropriate grassland or | |
| C | aliginosus | Bee | G4?/S1S2 | | meadow habitat is not present in the Study Area. | |
| | | | SA | Phacelia. | , | |
| 12. B | Bombus crotchii | Crotch Bumble Bee | -/CCE | Open grassland and scrub habitats. Food | No Potential. Appropriate grassland and | |
| | | | G3G4/S1S2 | 1 0 | scrub habitats with specific host plants are not present in the Study Area. | |
| | | | SA | Eschscholzia, and Eriogonum. | 1 | |
| | Bombus | Western Bumble | -/CCE | Wide variety of natural, agricultural, | No Potential. Fallow cropland habiat on | |
| 0 | occidentalis | Bee | G2G3/S1 | urban, and rural habitats. Flower-rich | the site is nearly barren and suitable host | |
| | | | SA | | occurrence is 2.3 miles west of the Study Area (CNDDB #293) in 1967. | |

| | Scientific Name | Common Name | Federal/State Status Global/State Rank CDFW Status | Habitat Preference | Potential to Occur |
|-----|--------------------------|------------------------------|---|---|--|
| 14. | Branchinecta | Vernal Pool Fairy | FT/- | Clear water sandstone depression pools, | No Potential. Appropriate vernal pool |
| | lynchi | Shrimp | G3/S3 | grassed swale, earth slump, or basalt flow depression pools. | habitat is not present in the Study Area. |
| | | | SA | room are room. | |
| 15. | Corynorhinus | Townsend's Big- | -/- | Roosts in caves, abandoned buildings, | No Potential. Human disturbance is high |
| | townsendii | Eared Bat | G3G4/S2 | tunnels. Roosting sites limiting. Sensitive to human disturbance. | in the area and abandoned structures are typically only used along the Pacific |
| | | SSC | | coast, where human disturbance is absent. | |
| 16. | Dipodomys | Big-Eared | -/- | Forages under shrubs & in the open. | No Potential. Shrub habitat is not |
| | venustus elephantinus | 8 | G4T2/S2 | Burrows for cover and for nesting. | present and the Study Area is outside the known range for this species. |
| | - | | SSC | | - |
| 17. | Elanus leucurus | s leucurus White-Tailed Kite | -/- | Nests in dense tree canopy near open | No Potential (nesting). Suitable nesting |
| | | | G5/S3S4 | foraging areas | habitat of open-country trees or trees aong forest or woodland edge is not |
| | | | FP | | present in the Study Area. |
| | | | | | Low . The Study Area could be utilized for forage, with nearby nesting occurrence within 3.5 mi north (CNDDB #155) in 2007. |
| 18. | Emys marmorata | Western Pond | -/- | Permanent or semi-permanent streams, | No Potential. Appropirate aquatic |
| | | Turtle | G3G4/S3 | ponds, lakes. | resources are not present in the Study Area. |
| | | | SSC | | |
| 19. | Eumops perotis | Western Mastiff | -/- | Roosts in crevices in cliff faces, high | No Potential. Appropriate roosting and |
| | californicus | Bat | G5T4/S3S4 | buildings, trees, and tunnels. Inhabits many open, semi-arid to arid habitats, | foraging habitat is not present in the Study Area. |
| | | | SSC | including conifer and deciduous woodlands, coastal scrub, grasslands, and chaparral | • |

| | Scientific Name | Common Name | Federal/State Status Global/State Rank CDFW Status | Habitat Preference | Potential to Occur |
|-----|--------------------|--------------------|---|---|--|
| 20. | Falco mexicanus | Prairie Falcon | -/- | Inhabits dry, open terrain. Nests on | No Potential. Appropriate nesting and |
| | | | G5/S4 | cliffs near open areas for hunting. | foraging habitat is not present in the Study Area. |
| | | | WL | | |
| 21. | Falco peregrinus | American Peregrine | FD/CD | Nests on cliffs, banks, dunes, mounds, | No Potential. Appropriate nesting and |
| | anatum | Falcon | G4T4/S3S4 | and human-made structures, especially near water. | foraging habitat is not present in the Study Area. |
| | | | FP | | , |
| 22. | Gymnogyps | California Condor | FE/CE | Deep canyons containing clefts in the | No Potential. Canyon habitat is not |
| | californianus | | G1/S1 | rocky walls provide nesting sites. Forages up to 100 miles from roost/nest. | present and the Study Area is outside the known range for California condor. |
| | | | FP | Totages up to 100 miles from 100st fiest. | known range for Camfornia condor. |
| 23. | Idiostatus | Pinnacles | -/- | Known only from Pinnacles National | No Potential. The Study Area is outside |
| | kathleenae | Shieldback Katydid | G1G2/S1S2 | Monument. | the known range for this species. |
| | | | SA | | |
| 24. | Lasiurus | Western Red Bat | -/- | Roosts primarily in trees, from sea level | No Potential. Appropriate large-leaved |
| | blossevillii | | G5/S3 | | trees for roosting are not present in the Study Area |
| | | | SSC | | Study Theu |
| 25. | Lasiurus cinereus | Hoary Bat | -/- | Forages in open habitats or habitat | No Potential. The disturbed quality of |
| | | | G5/S4 | mosaics with trees. Roosts in dense foliage of medium to large trees. Feeds | the Study Area is not suited to support hoary bats. |
| | | | SA | on moths. Requires water. | |
| 26. | Lavinia exilicauda | Pajaro/Salinas | -/- | Monterey hitch can occupy a wide | No Potential. Stream habitat is not |
| | harengus | Hitch | G4T2T4/S2S4 | variety of habitats, although they are most abundant in lowland areas with | present in the Study Area. |
| | | | | large pools or in small reservoirs that | |
| 27. | Masticophis | San Joaquin | -/- | Open, dry, treeless areas, including | No Potential. Appropriate grassland or |
| | flagellum ruddocki | Coachwhip | G5T2T3/S2? | grasslands and saltbush scrub; takes refuge in burrows and under shaded | scrub habitat is not present in the Study Area. |
| | | | SSC | vegetation | |

| | Scientific Name | Common Name | Federal/State Status Global/State Rank CDFW Status | Habitat Preference | Potential to Occur |
|-----|---------------------------|--------------------------------------|---|--|---|
| 28. | Myotis ciliolabrum | Western Small- | -/- | Prefers open stands in forests and | No Potential. Appropriate forest or |
| | | Footed Myotis | G5/S3 | woodlands. Requires drinking water. Feeds on a wide variety of small flying | woodland habitat is not present in the Study Area. |
| | | | SA | insects. | |
| 29. | Myotis evotis | Long-Eared Myotis | -/- | Nursery colonies in buildings, crevices, | No Potential. Appropriate mixed |
| | | | G5/S3 | spaces under bark, and snags. Caves used primarily as night roosts. Most | coniferous forests are not present in vicninty to the Study Area; therefore the |
| | | | SA | commonly found in mixed coniferous forests, from humid coastal areas to montane forests. | abandoned structure has no roosting potential. |
| 30. | Myotis thysanodes | buildings, or crevices for maternity | | No Potential. The level of human | |
| | | | | disturbance is high and abandoned structures on site are not in protected | |
| | | | SA | locations among oak, pinon, and juniper forests | locations near appropriate forest habitat. |
| 31. | Myotis yumanensis | Yuma Myotis | -/- | rock crevices, or under bridges. Feeds structure and | Low. The abandoned residential |
| | | | G5/S4 | | structure and surrounding trees could provide roosting habitat, though foraing |
| | | | SA | • | habitat (open water) is over 2 miles from the Study Area. |
| 32. | Oncorhynchus | Steelhead - South- | FT/- | Federal listing refers to runs in coastal | No Potential. Riverine habitat is not |
| | mykiss irideus pop. 9 | Central California Coast Dps | G5T2Q/S2 | basins from the Pajaro River south to, but not including, the Santa Maria River. | present in the Study Area. |
| | | • | SA | G . | |
| 33. | Optioservus canus | Pinnacles | -/- | Found on rocks and in gravel of riffles in | No Potential. Stream habitat is not |
| | | Optioservus Riffle Beetle | G1/S1 | cool, swift, clear streams. | present in the Study Area. |
| | | | SA | | |
| 34. | Perognathus | Salinas Pocket | -/- | Annual grassland and desert shrub in | No Potential. The Study Area is outside |
| | inornatus psammophilus | Mouse | G4T2?/S1 | Salinas Valley, with friable soils | the known range for this species and suitable habitat is not present. |
| | | | SSC | | 1 |

| | Scientific Name | Common Name | Federal/State Status Global/State Rank CDFW Status | Habitat Preference | Potential to Occur |
|-----|---------------------------|------------------------|---|--|---|
| 35. | Phrynosoma blainvillii | Coast Horned Lizard | -/- G3G4/S3S4 | Frequents a wide variety of habitats, most common in lowlands along sandy | No Potential. Sandy wash habitat is not present in the Study Area. |
| | | | | washes with scattered low bushes. | |
| 26 | Dana hovlii | Foothill Yellow- | SSC -/CCT | Portly shoded shellow streems and | No Detential Aquetia recourage ere not |
| 30. | Rana boylii | Legged Frog | | Partly shaded, shallow streams and riffles with rocky substrate. Min. 15 | No Potential. Aquatic resources are not present in the Study Area. |
| | | | G3/S3 | weeks for larval development. | |
| | | | SSC | | |
| 37. | Rana draytonii | California Red- | FT/- | Lowlands and foothills in or near sources | No Potential. Riparian habitat with |
| | | Legged Frog | G2G3/S2S3 | of deep water with dense, shrubby or emergent riparian vegetation. Requires | aquatic resources suitable for breeding are not present in the Study Area. |
| | | | SSC | 11-20 weeks for larval development. | |
| 38. | Riparia riparia | Bank Swallow | -/CT | Nests colonially in riparian and other lowland habitats west of the desert. Requires vertical banks or cliffs with | No Potential (nesting). Appropriate riparian nesting habitat with vertical banks is not present in the Study Area. |
| | | | G5/S2 | | |
| | | | SA | sandy soils (to dig cavities) near streams, lakes, or the ocean. | Low (foraging). Bank swallows have been documented in the area and could be transient over the site or utilize the site when foraging. |
| 39. | Spea hammondii | Western Spadefoot | -/- | Grassland and woodland habitats with | No Potential. Appropriate breeding |
| | | | G3/S3 | vernal pools for breeding. Most of year spent underground. | habitat is not present within dispersal distance from the Study Area. |
| | | | SSC | spent underground. | distance from the study rife. |
| 40. | Taricha torosa | Coast Range Newt | -/- | Lives in terrestrial habitats & will | No Potential. Known breeding ponds |
| | | | G4/S4 | migrate over 1 km to breed in ponds, reservoirs & slow moving streams. | are not in the vicinity of the Study Area and dispersal is not likely to occur across |
| | | | SSC | reservoirs & slow moving streams. | the site. |
| 41. | Taxidea taxus | American Badger | -/- | Needs friable soils in open ground with | No Potential. Friable soils required for |
| | | | G5/S3 | abundant food source such as California ground squirrels. | denning are not present in the Study Area and the site is heavily disturbed |
| | | | SSC | | within residential community. |

| | Scientific Name | Common Name | Federal/State Status Global/State Rank CDFW Status | Habitat Preference | Potential to Occur |
|-----|-----------------------|--------------------|---|--|--|
| 42. | Vireo bellii pusillus | Least Bell's Vireo | FE/CE | Riparian habitat, near water or dry | No Potential. Appropriate riparian |
| | | | G5T2/S2 | streambed, <2000 ft. Nests in willows, mesquite, Baccharis. | habitat is not present in the Study Area. |
| | | | SA | 1, | |
| 43. | Vulpes macrotis | San Joaquin Kit | FE/CT | Annual grasslands or grassy open stages | No Potential. Appropriate open |
| | mutica | Fox | G4T2/S2 | with scattered shrubby vegetation. Needs loose textured sandy soil and prey | grassland habitat is not present and the mapped historic range for kit fox shows |
| | | | SA | base. | no observations in the area beyond 1990 (CDFW 2020). |

| FE: Federally Endangered |
|--|
| FT: Federally Threatened |
| PE: Proposed Federally Endangered |
| PT: Proposed Federally Threatened |
| CE: California Endangered |
| CT: California Threatened |
| CCE: Candidate for California Endangered |
| CCT: Candidate for California Threatened |

G2/S2 – Imperiled G3/S3 – Vulnerable G4/S4 – Apparently Secure

G5/S5 – Secure

Global/State Ranks:

G1/S1 - Critically Imperiled

Q – Element is very rare but there are taxonomic questions associated with it. Range rank – (e.g., S2S3 means rank is somewhere between S2 and S3) ? – (e.g., S2? Means rank is more certain than S2S3 but less certain that S2)

CDFW Rank:

WL: Watch List

SSC: Species of Special Concern

FP: Fully Protected SA: Special Animal

Geotechnical Investigation Report



GEOTECHNICAL INVESTIGATION REPORT PROPOSED RESIDENCES 296 APPLE AVE (APN 109-082-013-000) GREENFIELD, CALIFORNIA

July 28, 2021 PROJECT 21-9798

FOR

SHERYL FLORES
PEOPLES SELF HELP HOUSING
3533 EMPLEO STREET
SAN LUIS OBISPO, CA 93401

BY

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Pacific Coast Testing, Inc.

Pacific Coast Testing, Inc.

July 28, 2021 Project 21-9798

P.O. Box 6835 Greenfield, CA 93456 Tel: (805) 631-5108 Fax: (805) 631-5937

Sheryl Flores Peoples Self Help Housing 3533 Empleo Street San Luis Obispo, CA 93401

Subject: Geotechnical Investigation, Proposed Residences, 296 Apple Avenue (APN 109-

082-013-000), Greenfield, California

Dear Sheryl:

Pacific Coast Testing (PCT) is pleased to submit this Geotechnical Investigation Report for the proposed residences at 296 Apple Avenue in Greenfield, California. This report was prepared in accordance with the scope of services presented in our proposal. The report provides geotechnical recommendations for site preparation, foundations, slabs-on-grade, retaining walls, pavement sections etc.

As discussed in the report, the primary concerns from a geotechnical standpoint are the loose condition of the soils in the upper 3 to 4 feet and potential for differential movements. It is therefore important that the building pad areas be overexcavated and that the foundations bear in compacted soils.

Please contact the undersigned if you have any questions concerning the findings or conclusions provided in this report.

Sincerely,

PACIFIC COAST TESTING INC.

Ron J. Church GE #2184

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GEOTECHNICAL INVESTIGATION REPORT PROPOSED RESIDENCES 296 APPLE AVENUE (APN 109-082-013-000) GREENFIELD, CALIFORNIA

PROJECT 21-9798

1.0 INTRODUCTION

This report presents the results of our geotechnical investigation for the proposed residences to be located at 296 Apple Avenue (APN 109-082-013-000) in Greenfield, California. A site location map is presented in Figure 1.

The property is located north of Apple Avenue, east of 3rd Street, approximately 2000 feet east of the intersection of Walnut Avenue with Highway 101. Existing residential properties and vacant/agricultural land surround the site. Topographically, the terrain is relatively level with gradients of less than ten (10) percent. Site elevations are around 270 feet above mean sea level. The property covers an area of around 4.55 acres. At the time of our field investigation the boring locations were partially covered with native grasses and weeds. Based on available maps, the property had some agricultural use prior to the mid-1990's and has been vacant since that time. An existing residence is located on the southside of the property.

It is our understanding that the residences will be one and two-story, wood-framed structures with concrete slab-on-grade floors. Footing loads for the proposed residences are presently unavailable. For the purpose of this report, loads on the order of 15 kips (columns) and 1.0 kips per lineal foot (continuous) have been estimated.

The project description is based on a site reconnaissance performed by a Pacific Coast Testing, Inc., engineer and information provided by Peoples Self Help Housing. The topographic plan provided (by Monterey Bay Engineers) forms the basis for the "Site Plan", Figure 2.

In the event that there is change in the nature, design or location of improvements, or if the assumed loads are not consistent with actual design loads, the conclusions and recommendations contained in this report should be reviewed and modified, if required. Evaluations of the soils for hydrocarbons or other chemical properties are beyond the scope of the investigation.

2.0 PURPOSE AND SCOPE

The purpose of this study was to explore and evaluate the surface and subsurface soil conditions at the site and to develop geotechnical information and design criteria for the proposed project. The scope of this study included the following items.

- 1. A review of available soil and geologic information for this area of Greenfield.
- 2. A field study consisting of a site reconnaissance and an exploratory boring program to formulate a description of the subsurface conditions.
- A laboratory testing program performed on representative soil samples collected during our field study.
- 4. Engineering analysis of the data gathered during our field study, laboratory testing, and literature review. Development of recommendations for site preparation and grading, and geotechnical design criteria for foundations, slabon-grade construction, retaining walls, pavement design and underground facilities.
- 5. Preparation of this report summarizing our findings, conclusions, and recommendations regarding the geotechnical aspects of the project site.

3.0 SUBSURFACE SOIL CONDITIONS

Quaternary alluvium materials of the Salinas River has been mapped in the area of the Site (Dibblee, 2006). The alluvial soils are expected to consist of sands and gravels, which extend to unknown depths below the ground surface. Figure 3 shows a geologic map of the area. The near surface materials encountered in the exploratory borings to a depth of 3 to 5 feet consisted of brown gravelly silty sands and gravelly clayey sands. These materials were encountered in a slightly moist to moist state and in a loose to dense condition. The near surface materials were underlain by sandy gravels to a depth of 15 feet. These materials were encountered in a slightly

moist to moist state and in a dense to very dense condition. Based on previous borings in this area of Greenfield, sandy gravels and gravelly sands can be expected to a depth of 50 feet. The near surface gravelly silty sands have very low expansivity. No free ground water was encountered during our field exploration. Based on previous borings and our experience in this area of Greenfield, groundwater depths are greater than 40 feet below existing grades.

A more detailed description of the soils encountered is presented graphically on the "Exploratory Boring Logs," B-1 through B-6, Appendix A. An explanation of the symbols and descriptions used on these logs are presented on the "Soil Classification Chart.

The soil profile described above is generalized; therefore, the reader is advised to consult the boring logs (Appendix A) for soil conditions at specific locations. Care should be exercised in interpolating or extrapolating subsurface conditions between or beyond and borings. On the boring logs we have indicated the soil type, moisture content, grain size, dry density, and the applicable Unified Soil Classification System Symbol.

The locations of our exploratory borings, shown on Site Plan, Figure 2, were approximately determined from features at the site. Hence, accuracy can be implied only to the degree that this method warrants. Surface elevations at boring locations were not determined.

4.0 **SEISMIC CONSIDERATIONS**

4.1 <u>Seismic Coefficients</u>

Structures should be designed to resist the lateral forces generated by earthquake shaking in accordance with the building code and local design practice. This section presents seismic design parameters for use with the California Building Code (CBC) and ASCE 7-16. The site coordinates and the ASCE 7 Hazard Tool were used to obtain the seismic design criteria. The peak ground acceleration was estimated for a 2 percent probability of occurrence in 50 years using the USGS online deaggregation tool.

Seismic Data

| California Building Code Seismic Parameter | Values for Site Class D |
|--|----------------------------|
| Latitude, degrees | 36.330000 |
| Longitude, degrees | -121.237000 |
| S _s Seismic Factor | 1.500 |
| S ₁ Seismic Factor | 0.550 |
| Site Class | Sd, Stiff Soil |
| Fa, Short-Period Site Coefficient (@ 0.2-s Period) | 1.200 |
| F _v , Long-Period Site Coefficient (@ 1.0-s Period) | 1.750* |
| S _{MS} , Site Specific Response Parameter for Site Class at 0.2 sec | 1.800 |
| S _{M1} , Site Specific Response Parameter for Site Class at 1 sec | 0.963 |
| $S_{DS} = 2/3 S_{MS}$ | 1.200 |
| $S_{D1} = 2/3 S_{M1}$ | 0.642 |
| Peak Ground Acceleration (2% probability in 50 years) | 0.686 |
| Likely Magnitude (M) | 7.8 |

*Fv is based on Table 11.4.2 of ASCE 7-16 assuming the fundamental period (T) for the proposed structure is taken to be less than or equal to Ts (S_{D1}/S_{DS}) and Cs is determined by Eq. 12.8.2 (Exception 2 of 11.4.8). If the structure does not meet with this exception, updated values or a design response spectrum can be prepared, upon request.

4.2 Liquefaction Analysis

Liquefaction is described as the sudden loss of soil shear strength due to a rapid increase of pore water pressures caused by cyclic loading from a seismic event. In simple terms it means that the soil acts more like a fluid than a solid in a liquefiable event. In order for liquefaction to occur, the following are generally needed; granular soils (sand, silty sand and sandy silt), groundwater and low density (very loose to medium dense) conditions. A liquefaction study was not part of our scope for this project; however a preliminary evaluation can be provided based on the results of our soil borings and experience in this area of Greenfield. In general, dense to very dense sandy gravels and gravelly sands were found below a depth 5 feet. As discussed above, similar materials can be expected to a depth of 50 feet. Groundwater is also unlikely to be encountered to a depth of 40 feet. This information indicates that the potential for liquefaction would be in the low category.

4.3 Lateral Spreading

Due to the near level terrain, the potential for lateral spreading displacements in the building pad areas would be negligible.

4.4 Slope Stability

The building pad areas are located in near level terrain with gradients of less than ten (10) percent. There was no visual evidence of overall instability at the site, although, shallow erosion of the silty sands could occur if over-saturated conditions were to occur. However, the potential for slope movements to influence the proposed construction would be negligible.

4.5 Faulting

The San Andreas fault is located approximately 14 miles (22 km) northeast of the site, whereas the closest mapped fault, the Rinconada Fault is located approximately 5 miles (8 km) to the southwest. There are no active or potentially active faults in the direct vicinity of the property. The site is not within a State of California Fault Hazards Zone (Alquist-Priolo). It is our opinion that there is a negligible potential for fault rupture to impact the proposed construction based on review of the published maps. A fault map is provided in Figure 4.

5.0 CONCLUSIONS AND RECOMMENDATIONS

- 1. The site is suitable for the proposed residences provided the recommendations presented in this report are incorporated into the project plans and specifications.
- 2. All grading and foundation plans should be reviewed by Pacific Coast Testing Inc., hereinafter described as the Geotechnical Engineer, prior to contract bidding. This review should be performed to determine whether the recommendations contained within this report are incorporated into the project plans and specifications.

3. The Geotechnical Engineer should be notified at least two (2) working days before site clearing or grading operations commence and should be present to observe the stripping of deleterious material and provide consultation to the Grading Contractor in the field.

4. Field observation and testing during the grading operations should be provided by the Geotechnical Engineer so that a decision can be formed regarding the adequacy of the site preparation, the acceptability of fill materials, and the extent to which the earthwork construction and the degree of compaction comply with the project geotechnical specifications. Any work related to grading performed without the full knowledge of, and under direct observation of the Geotechnical Engineer, may render the recommendations of this report invalid.

5.1 Clearing and Stripping

- 1. All surface and subsurface deleterious materials should be removed from the proposed buildings and driveway areas and disposed of off-site. This includes, but is not limited to tree rootballs, any buried utility lines, loose fills, septic systems, debris, building materials, and any other surface and subsurface structures within proposed building areas. Voids left from site clearing, should be cleaned and backfilled as recommended for structural fill.
- 2. Once the site has been cleared, the exposed ground surface should be stripped to remove surface vegetation and organic soil. The surface may be disced, rather than stripped, if the organic content of the soil is not more than three percent by weight. If stripping is required, depths should be determined by a member of our staff in the field at the time of stripping. Strippings may be either disposed of off-site or stockpiled for future use in landscape areas if approved by the landscape architect.

5.2 Preparation of Building Pads

1. The intent of these recommendations is to overexcavate and re-compact the near surface soils and support the residences on conventional footings.

2. The native soils in the building pad areas should be excavated to a depth of four (4) feet below lowest existing grade or finish pad grade or two (2) feet below bottom of the deepest footing, whichever is deeper. The geotechnical engineer should observe and approve the bottom of the overexcavated areas prior to the placement of fill. The exposed surface should then be scarified to a depth of 8 inches, moisture conditioned to slightly above optimum moisture and compacted to at least ninety (90) percent of maximum dry density (ASTM D1557-02). The removed materials (see section 5.4) can then be replaced and similarly compacted. The lateral limits of excavation, scarification and fill placement should be at least 5 feet beyond the perimeter building and footing lines. Permanent fill and cut slopes should not exceed 2:1 (horizontal to vertical).

- 3. If loose or unstable soils are encountered at the bottom of the excavations, these areas should be excavated (18 inches minimum) and a layer of stabilization fabric (Mirafi HP370 or equivalent) and Class II/III Base placed prior to placing fill. The base should be compacted to 90% of ASTM D1557-02.
- 4. In order to help minimize potential settlement problems associated with structures supported on a non-uniform materials, the soils engineer should be consulted for specific site recommendations during site excavation and grading. In general, all proposed construction should be supported on a uniform thickness of compacted soil.
- 5. The above grading is based on the strength characteristics of the materials under conditions of normal moisture that would result from rain water and do not take into consideration the additional activating forces applied by seepage from springs or subsurface water. Areas of observed seepage should be provided with subsurface drains to release the hydrostatic pressures.
- 6. The near-surface soils may become partially or completely saturated during the rainy season. Grading operations during this time period may be difficult since the saturated materials may not be compactable, and they may not support

construction equipment. Consideration should be given to the seasonal limit of the grading operations on the site.

7. All final grades should be provided with a positive drainage gradient away from foundations. Final grades should provide for rapid removal of surface water runoff. Ponding of water should not be allowed on building pads or adjacent to foundations.

5.3 Preparation of Paved Areas

- 1. After clearing and grubbing, the existing soils should be removed to a depth of at least two (2) feet below the existing ground surface or one (1) foot below the proposed structural section, whichever is deeper. The bottom of the excavation should then be scarified, moisture-conditioned and compacted to at least 90 percent. Native fill materials can then be placed and similarly compacted.
- 2. The upper 12 inches of subgrade beneath all paved areas should be compacted to at least 95 percent relative compaction. Subgrade soils should not be allowed to dry out or have excessive construction traffic between the time of water conditioning and compaction, and the time of placement of the pavement structural section.

5.4 Structural Fill

- On-site gravelly silty sands and sandy gravels free of organic and deleterious material are suitable for use as structural fill. These fills should not contain rocks larger than 3 inches in greatest dimension and should have no more than 15 percent larger than 1.5 inches in greatest dimension.
- 2. Select import (decomposed granite or Class II/III Base) should be free of organic and other deleterious material and should be non-expansive with a plasticity index of 10 or less and a sand equivalent of at least 30. Before delivery to the site, a sample of the proposed import should be tested in our laboratory to determine its suitability for use as structural fill.

3. Structural fill using on-site inorganic soil or approved import should be placed in layers, each not exceeding eight inches in thickness before compaction. On-site inorganic or imported soil should be conditioned with water, or allowed to dry, to produce a soil water content at approximately optimum value and should be compacted to at least 90 percent relative compaction based on ASTM D1557-02.

5.5 **Foundations**

- Conventional continuous footings and spread footings may be used for support of the proposed residences. All of the foundation materials should be competent after preparation in accordance with the grading section of this report.
- 2. The perimeter footings should be at least 15 inches wide with a minimum embedment of 18 inches below pad grade or below adjacent finished grade, whichever is lower. Spread footings should be a minimum of 18 inches square and similarly embedded and tied to the perimeter footings with grade beams (min. 12" wide by 18" deep). The reinforcement for the perimeter footings and grade beams should be designed by the structural engineer; however, a minimum of four (4) No. 4 rebar should be provided, two (2) on the top and two (2) on the bottom with dowels (#3 bars at 18 inches on-center) to tie the footings and grade beams to the slab.
- An allowable dead plus live load bearing pressure of 2000 psf may be used.
 Total settlements on the order of 1-inch should be anticipated with differential settlements being 50 percent of this value over 20 feet
- 4. The above allowable pressures are for support of dead plus live loads and may be increased by one-third for short-term wind and seismic loads.
- 5. Lateral forces on structures may be resisted by passive pressure acting against the sides of shallow footings and/or friction between the soil and the bottom of the footing. For resistance to lateral loads, a friction factor of 0.35 may be utilized for sliding resistance at the base of the spread footings in undisturbed

native materials or engineered fill. A passive resistance of 350 pcf equivalent fluid weight may be used against the side of shallow footings. If friction and passive pressures are combined, the lesser value should be reduced by 33 percent.

5.6 Slab-On-Grade Construction

- Concrete slabs-on-grade and flatwork should not be placed directly on unprepared loose fill materials. Preparation of subgrade to receive concrete slabs-on-grade and flatwork should be processed as discussed in the preceding sections of this report.
- Where concrete slabs-on-grade are to be constructed, the slabs should be underlain by a minimum of 4 inches of clean free-draining material such as clean sand or permeable aggregate complying with Caltrans Standard Specifications 68, Class I, Type A or Type B, to service as a cushion and a capillary break. Clean sand should have less the 3% passing the No. 200 sieve. A 15-mil Stegotype membrane should be placed between the cushion and the slab to provide an effective vapor barrier, and to minimize moisture condensation under the floor covering. It is suggested that a 2-inch thick sand layer be placed on top of the membrane to assist in the curing of the concrete. The sand should be lightly moistened prior to placing concrete.
- 3. Concrete slabs-on-grade should be a minimum of 4 inches thick and should be reinforced with at least No. 3 reinforcing bars placed at 18 inches on-center both ways at or slightly above the center of the structural section. Reinforcing bars should have a minimum clear cover of 1.5 inches, and hot bars should be cooled prior to placing concrete. The aforementioned reinforcement may be used for anticipated uniform floor loads not exceeding 100 psf. If floor loads greater than 100 psf are anticipated, the slab should be evaluated by a structural engineer
- 4. All slabs should be poured at a maximum slump of less than 5 inches. Excessive water content is the major cause of concrete cracking. For design of concrete

floors, a modulus of subgrade reaction of k = 100 psi per inch would be applicable to on-site engineered fill soils.

5.7 Retaining Walls

 Retaining walls should be designed to resist lateral pressures from adjacent soils and surcharge loads applied behind the walls.

| | essure and Condition ompacted Fill) | Equivalent Fluid Pressure, pcf | | | |
|---|--|-----------------------------------|---------------------------|--|--|
| | | Unrestrained Wall | Rigidly Supported Wall | | |
| Active Case, | Active Case, Level-native soils | | | | |
| Drained | Level-granular backfill | 30 | - | | |
| At-Rest Case, | Level-native soils | | 55 | | |
| Drained | Level-sand backfill | | 45 | | |
| Passive Case, Level Drained 2:1 Sloping Down | | 350 125 | | | |
| For sloping backfill add 1 pcf for every 2 deg. (Active case) and 1.5 pcf for every 2 deg. (At-rest case) | | | | | |

- 2. Isolated retaining wall foundations should extend a minimum depth of 24 inches below lowest adjacent grade. An allowable toe pressure of 1,800 psf is recommended for footings supported on 24 inches of compacted soil. A coefficient of friction of 0.35 may be used between subgrade soil and concrete footings.
- 3. For retaining walls greater than 6 feet, as measured from the top of the foundation, a seismic horizontal surcharge of 10H² (pounds per linear foot of wall) may be assumed to act on retaining walls. The surcharge will act at a height of 0.33H above the wall base (where H is the height of the wall in feet). This surcharge force shall be added to an active design equivalent fluid pressure of 35 pounds per square foot of depth for the seismic condition.

4. In addition to the lateral soil pressure given above, retaining walls should be designed to support any design live load, such as from vehicle and construction surcharges, etc., to be supported by the wall backfill. If construction vehicles are required to operate within 10 feet of a wall, supplemental pressures will be induced and should be taken into account through design.

- 5. The above-recommended pressures are based on the assumption that sufficient subsurface drainage will be provided behind the walls to prevent the build-up of hydrostatic pressure. To achieve this, we recommend that a filter material be placed behind all proposed walls. The blanket of filter material should be a minimum of 12 inches thick and should extend from the bottom of the wall to within 12 inches of the ground surface. The top 12 inches should consist of water conditioned, compacted native soil. A 4-inch diameter drain pipe should be installed near the bottom of the filter blanket with perforations facing down. The drain pipe should be underlain by at least 4 inches of filter type material. Adequate gradients should be provided to discharge water that collects behind the retaining wall to an adequately controlled discharge system with suitably projected outlets. The filter material should conform to Class I, Type B permeable material as specified in Section 68 of the California Department of Transportation Standard Specifications, current edition. A typical 1" x #4 concrete coarse aggregate mix approximates this specification.
- 6. For hydrostatic loading conditions (i.e. no free drainage behind walls), an additional loading of 45 pcf equivalent fluid weight should be added to the above soil pressures. If it is necessary to design retaining structures for submerged conditions, allowed bearing and passive pressures should be reduced by 50 percent. In addition, soil friction beneath the base of the foundations should be neglected.
- 7. Precautions should be taken to ensure that heavy compaction equipment is not used immediately adjacent to walls, so as to prevent undue pressure against, and movement of, the walls. The use of water-stops/impermeable barriers

should be considered for any basement construction, and for building walls, which retain earth.

5.8 Pavement Design

 The following table provides recommended pavement sections based on an estimated R-Value of 40 for the near surface gravelly silty sand soils encountered at the site.

| RECOMMENDED MINIMUM ASPHALT CONCRETE PAVEMENT SECTIONS DESIGN THICKNESS | | | | | |
|---|--|--------|--|--|--|
| T.I. | A.Cin. | A.Bin. | | | |
| 4.5 | 2.5 | 6.0 | | | |
| 5.0 | 2.5 | 6.0 | | | |
| 5.5 | 3.0 | 7.0 | | | |
| 6.0 | 3.0 | 8.0 | | | |
| T.I. = A.C. = A.B. = | Asphaltic Concrete - must meet specifications for Caltrans Type A Asphalt Concrete | | | | |

- R-value samples should be obtained and tested at the completion of rough grading and the pavement sections confirmed or revised. All asphaltic concrete pavement sections and all sections should be crowned for good drainage.
- 3. All asphalt pavement construction and materials used should conform with Sections 26 and 39 of the latest edition of the Standard Specifications, State of California, Department of Transportation. Aggregate bases and sub-bases should also be compacted to a minimum relative compaction of 95 percent based on ASTM D1557-02.

5.9 <u>Underground Facilities Construction</u>

 The attention of contractors, particularly the underground contractors, should be drawn to the State of California Construction Safety Orders for "Excavations,

Trenches, Earthwork". Trenches or excavations greater than 5 feet in depth should be shored or sloped back in accordance with OSHA Regulations prior to entry.

- 2. For purposes of this section of the report, bedding is defined as material placed in a trench up to 1 foot above a utility pipe and backfill is all material placed in the trench above the bedding. Unless concrete bedding is required around utility pipes, free-draining sand should be used as bedding. Sand proposed for use as bedding should be tested in our laboratory to verify its suitability and to measure its compaction characteristics. Sand bedding should be compacted by mechanical means to achieve at least 90 percent relative compaction based on ASTM Test D1557-02.
- 3. On-site inorganic soil, or approved import, may be used as utility trench backfill. Proper compaction of trench backfill will be necessary under and adjacent to structural fill, building foundations, concrete slabs and vehicle pavements. In these areas, backfill should be conditioned with water (or allowed to dry), to produce a soil water content of about 2 to 3 percent above the optimum value and placed in horizontal layers each not exceeding 8 inches in thickness before compaction. Each layer should be compacted to at least 90 percent relative compaction based on ASTM Test D1557-02. The top lift of trench backfill under vehicle pavements should be compacted to the requirements given in report section 5.3 for vehicle pavement subgrades. Trench walls must be kept moist prior to and during backfill placement.

5.10 Surface and Subsurface Drainage

- Concentrated surface water runoff within or immediately adjacent to the site should be conveyed in pipes or in lined channels to discharge areas that are relatively level or that are adequately protected against erosion.
- 2. Water from roof downspouts should be conveyed in pipes that discharge in areas a safe distance away from structures. Surface drainage gradients should be

planned to prevent ponding and promote drainage of surface water away from building foundations, edges of pavements and sidewalks. For soil areas we recommend that a minimum of five (5) percent gradient be maintained.

- 3. Maintenance of slopes is important to their long-term performance. It is recommended that (where disturbed) slope surfaces be planted with appropriate drought-resistant vegetation as recommended by a landscape architect, and not over-irrigating, a primary source of surficial failures. In addition, an erosion control blanket (Greenfix CF072RR or equivalent) should be placed over the slopes to protect the vegetation while it becomes established. In addition, water should not be allowed to run over the sides of the slopes
- 4. Careful attention should be paid to erosion protection of soil surfaces adjacent to the edges of roads, curbs and sidewalks, and in other areas where "hard" edges of structures may cause concentrated flow of surface water runoff. Erosion resistant matting such as Miramat, or other similar products, may be considered for lining drainage channels.
- 5. Subdrains should be placed in established drainage courses and potential seepage areas. The location of subdrains should be determined during grading. The subdrain outlet should extend into a suitable protected area or could be connected to the proposed storm drain system. The outlet pipe should consist of an unperforated pipe the same diameter as the perforated pipe.

5.11 Percolation Testing

Three (3) percolation tests were performed at the property. The test boreholes were drilled to a depth of 5 feet and the rates determined be the falling-head method. Gravelly silty sands were encountered at the locations drilled. The results are summarized in the following table. An infiltration rate of 3 inches/hour would be generally applicable for the percolation rates obtained.

| Test No. | Depth (feet) | Soil Description | Percolation Rate |
|----------|-----------------|-----------------------------|---------------------|
| P-1 | 5 | Gravelly Silty Sand (SM-GP) | 5 min/inch |
| P-2 | 5 | Gravelly Silty Sand (SM-GP) | 14 min/inch |
| P-3 | 5 | Gravelly Silty Sand (SM-GP) | 4 min/inch |

5.12 Corrosion

1. To provide corrosion control guidelines, soil samples were obtained for resistivity testing. Testing was performed on a sample obtained from boring B-1. The results are presented on the following table.

| Soil Resistivity | | | | | |
|------------------|---------------------------|--|--|--|--|
| Sample Location | Soil Resistivity (ohm-cm) | | | | |
| B-1 @ 1.5 feet | 9800 | | | | |

2. One (1) soil sample was tested to measure ph and the concentration of sulfate and chlorides. The results are presented in the following table. The results indicate that sulfate salt content should not affect normally formulated concrete (Type II Cement). The resistivity and chloride measurements indicate that the potential for corrosion of ferrous pipes is in the mild corrosive range.

| Chemical Tests | | | | | | | |
|--------------------|-------|-----------|-----|-------------------------------|------------------------------|--|--|
| Sample Location | Depth | Soil Type | PH | Soluble Chlorides (ppm) | Soluble Sulfates (ppm) | | |
| B-2 | 2' | SM-GP | 7.7 | 40 | 20 | | |
| | | | | | | | |

5.13 Geotechnical Observation and Testing

1. Field exploration and site reconnaissance provides only a limited view of the

geotechnical conditions of the site. Substantially more information will be revealed during the excavation and grading phases of the construction. Stripping & clearing of vegetation, overexcavation, scarification, fill and backfill placement and compaction should be reviewed by the geotechnical professional during construction to evaluate if the materials encountered during construction are consistent with those assumed for this report.

2. Special inspection of grading should be provided in accordance with California Building Code Section 1705.6 and Table 1705.6. The special inspector should be under the direction of the engineer.

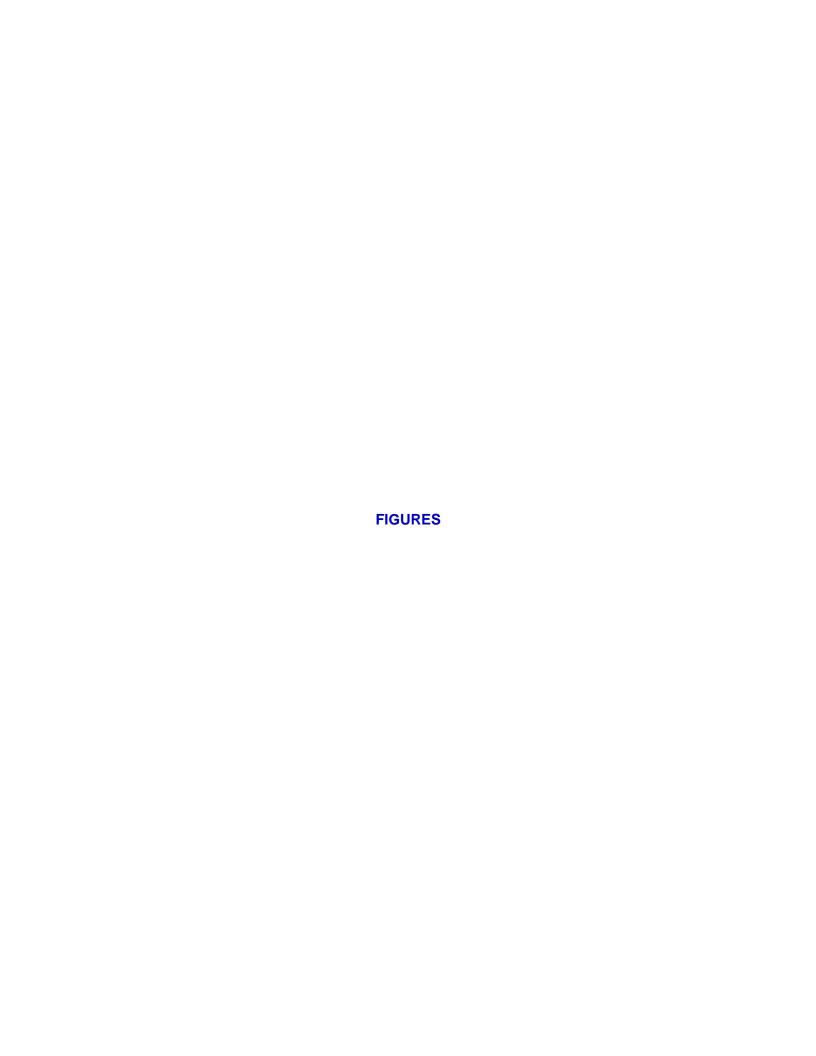
| VERIFICATION AND INSPECTION TASK | CONTINUOUS DURING TASK LISTED | PERIODIC DURING TASK LISTED |
|--|----------------------------------|--------------------------------|
| Verify materials below shallow foundations are adequate to achieve the design bearing capacity | | Х |
| 2. Verify excavations are extended to proper depth and have reached proper material | | Х |
| Perform classification and testing of compacted fill | | X |
| Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill | Х | |
| 5. Prior to placement of compacted fill, observe subgrade and verify that site has been prepared properly. | | Х |

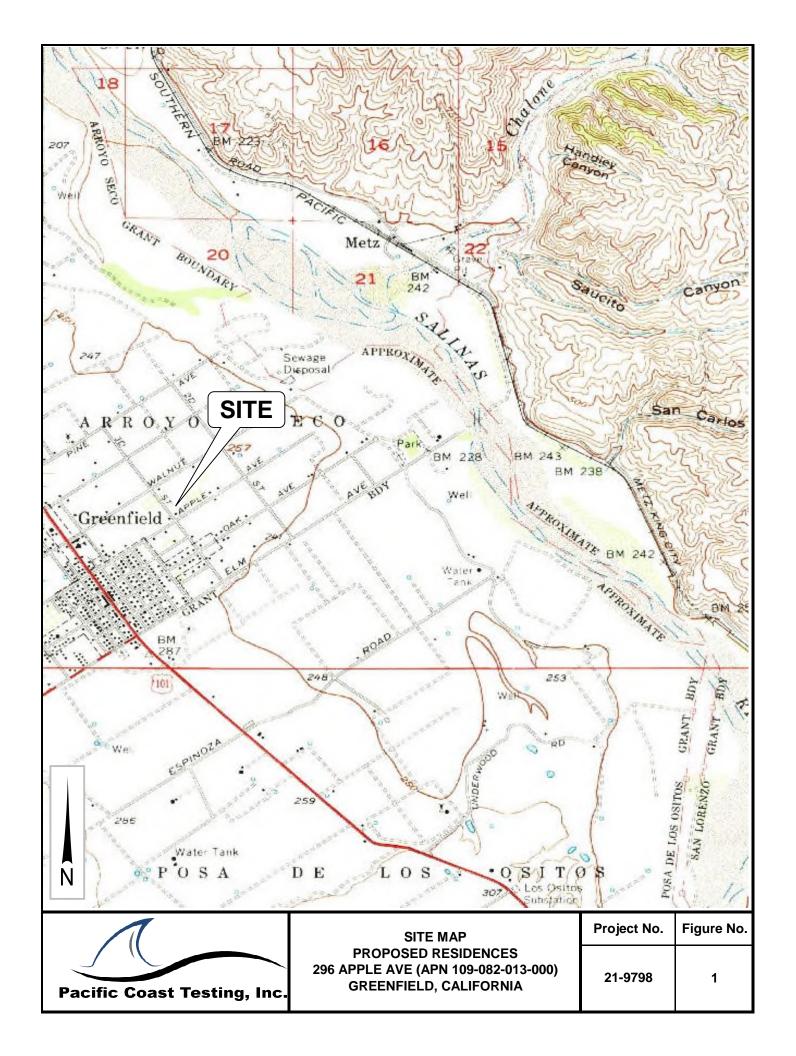
6.0 LIMITATIONS AND UNIFORMITY OF CONDITIONS

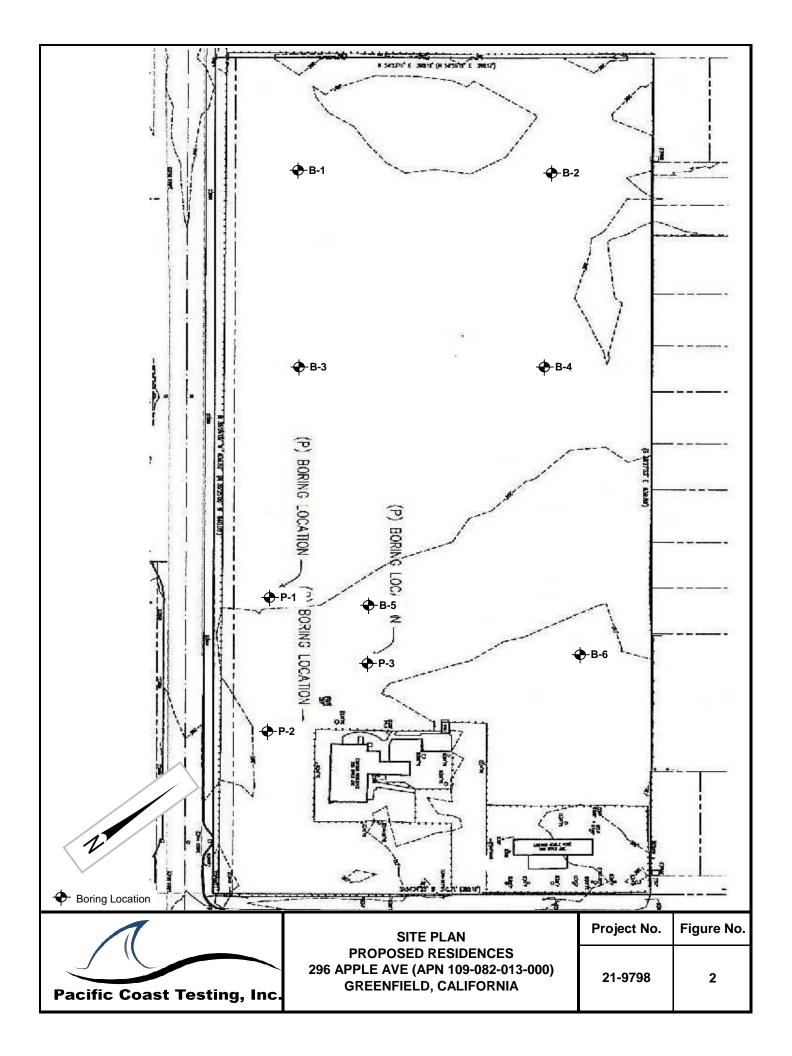
- 1. It should be noted that it is the responsibility of the owner or his/her representative to notify Pacific Coast Testing Inc. a minimum of 48 hours before any stripping, grading, or foundation excavations can commence at this site.
- The recommendations of this report are based upon the assumption that the soil
 conditions do not deviate from those disclosed during our study. Should any
 variations or undesirable conditions be encountered during grading of the site,

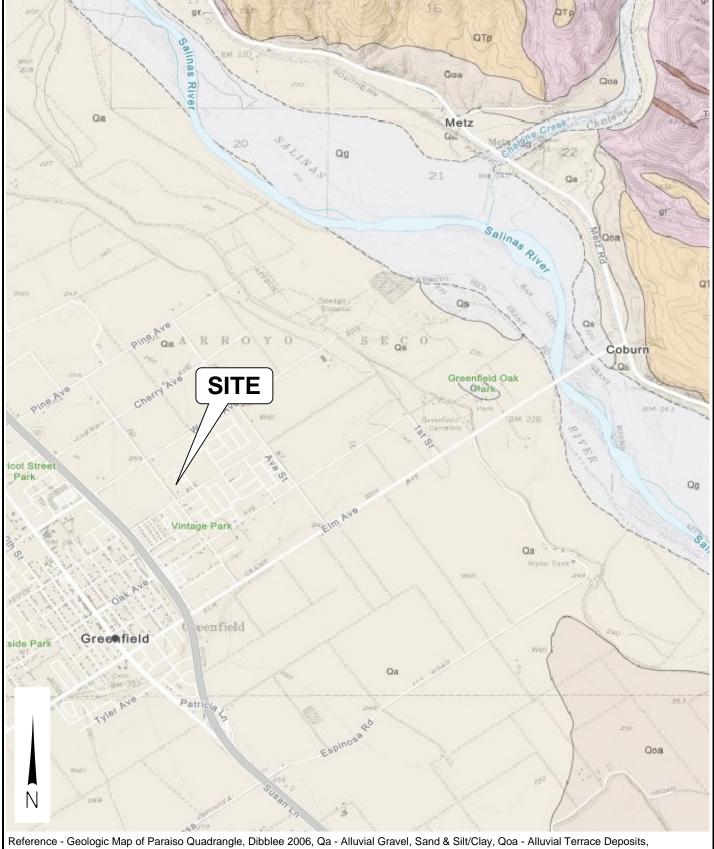
Pacific Coast Testing Inc. will provide supplemental recommendations as dictated by the field conditions.

- 3. This report is issued with the understanding that it is the responsibility of the owner or his/her representative to ensure that the information and recommendations contained herein are brought to the attention of the architect and engineer for the project and incorporated into the project plans and specifications. The owner or his/her representative is responsible for ensuring that the necessary steps are taken to see that the contractor and subcontractors carry out such recommendations in the field.
- 4. As of the present date, the findings of this report are valid for the property studied. With the passage of time, changes in the conditions of a property can occur whether they are due to natural processes or to the works of man on this or adjacent properties. Legislation or the broadening of knowledge may result in changes in applicable standards. Changes outside of our control may find this report to be invalid, wholly or partially. Therefore, this report should not be relied upon after a period of three (3) years without our review nor is it applicable for any properties other than those studied.
- 5. Validity of the recommendations contained in this report is also dependent upon the prescribed testing and observation program during the site preparation and construction phases. Our firm assumes no responsibility for construction compliance with these design concepts and recommendations unless we have been retained to perform continuous on-site testing and review during all phases of site preparation, grading, and foundation/slab construction. The Geotechnical Engineer should be notified at least two (2) working days before site clearing or grading operations commence to develop a program of quality control.







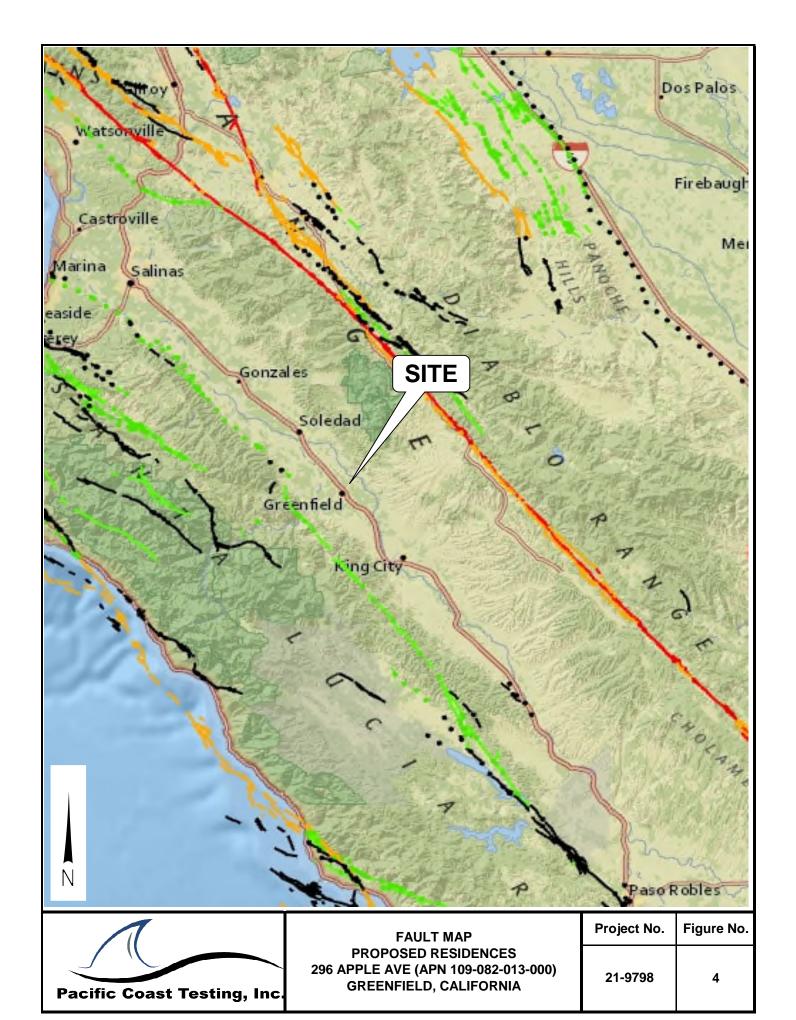


Reference - Geologic Map of Paraiso Quadrangle, Dibblee 2006, Qa - Alluvial Gravel, Sand & Silt/Clay, Qoa - Alluvial Terrace Deposits, Qg - Sand & Gravel



GEOLOGIC MAP PROPOSED RESIDENCES 296 APPLE AVE (APN 109-082-013-000) **GREENFIELD, CALIFORNIA**

| Project No. | Figure No. |
|-------------|------------|
| 21-9798 | 3 |



APPENDIX A

Field Investigation Key to Boring Logs Boring Logs

FIELD INVESTIGATION

Test Hole Drilling

The field investigation was conducted on June 18, 2021. Six (6) exploratory borings and three (3) percolation borings were drilled at the approximate locations indicated on the Site Plan, Figure 2. The locations of these borings were approximated in the field.

Undisturbed and bulk samples were obtained at various depths during test hole drilling. The undisturbed samples were obtained by driving a 2.4-inch inside diameter sampler into soils. Bulk samples were also obtained during drilling.

Logs of Boring

A continuous log of soils, as encountered in the borings was recorded at the time of the field investigation, by a Staff Engineer. The Exploration Boring Logs are attached.

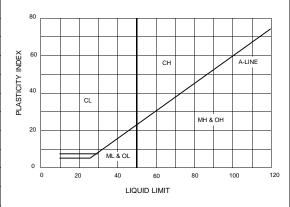
Locations and depth of sampling, in-situ soil dry densities and moisture contents are tabulated in the Boring Logs.

UNIFIED SOIL CLASSIFICATION SYSTEMS

| | Gitti IEB GGIE GE/tGGII IG/tiloit GTGTEIIIG | | | | | |
|---|---|--------------------------------------|-----|---------------------------------------|---|--|
| | MAJOR DIV | /ISION | SYM | BOLS | TYPICAL NAMES | |
| ODAVELO | | CLEAN GRAVELS WITH LITTLE | | 0,0 | WELL GRADED GRAVELS, GRAVEL-SAND MIXTURES | |
| | GRAVELS Over 50% > #4 sieve | ver 50% | GP | | POORLY GRADED GRAVELS, GRAVEL-SAND MIXTURES | |
| SOILS | > #4 Sieve | GRAVELS WITH | GM | | SILTY GRAVELS, POORLY GRADED GRAVEL-SAND-SILT MIXTURES | |
| AINED > #200 | | OVER 12% FINES | GC | | CLAYEY GRAVELS, POORLY GRADED GRAVEL-SAND-CLAY MIXTURES | |
| COARSE GRAINED SOILS Over 50% > #200 sieve | | CLEAN SANDS | | °i'i | WELL GRADED SANDS, GRAVELLY SANDS | |
| COAR | | or NO FINES | SP | ֓֞֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓ | POORLY GRADED SANDS, GRAVELLY SANDS | |
| | < #4 sieve | SANDS WITH OVER 12% FINES | SM | | SILTY SANDS, POORLY GRADED SAND-SILT MIXTURES | |
| | | | sc | ///, | CLAYEY SANDS, POORLY GRADED SAND-CLAY MIXTURES | |
| | | SILTS AND CLAYS Liquid limit < 50 | | | INORGANIC SILTS, SILTY OR CLAYEY FINE SANDS, OR CLAYEY SILTS WITH SLIGHT PLASTICITY | |
| OILS | | | | //// | INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY, SANDY, OR SILTY CLAYS, LEAN CLAYS | |
| NED S | | | | | ORGANIC CLAYS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY | |
| FINE GRAINED SOILS Over 50% < #200 sieve | | SILTS AND CLAYS Liquid limit > 50 | | | INORGANIC SILTS , MICACEOUS OR DIATOMACIOUS FINE SANDY OR SILTY SOILS, ELASTIC SILTS | |
| | | | | //// | INORGANIC CLAYS OF HIGH PLASTICITY, FAT CLAYS | |
| | | | | 14/1 | ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS | |
| F | HIGHLY ORGANIC CLAYS | | Pt | | PEAT AND OTHER HIGHLY ORGANIC SOILS | |

PLASTICITY CHART

USED FOR CLASSIFICATION OF FINE GRAINED SOILS



U.S. STANDARD SIEVE

6" 3" 3/4" 4 10 40 200

SOIL GRAIN SIZE

| BOULDERS | COBBLES | GRA | VEL | | SAND | | SILT | CLAY |
|--------------------------------|---------|--------|------|--------|--------|---------|-------|------|
| BOOLDENG | COBBLEC | COARSE | FINE | COARSE | MEDIUM | FINE | SILI | CLAT |
| 15 | 50 | 75 1 | 9 4. | 75 2. | 0 0.4 | 25 0.07 | 75 0. | 002 |
| SOIL GRAIN SIZE IN MILLIMETERS | | | | | | | | |

SAMPLE DRIVING RECORD

| BLOWS PER FOOT | DESCRIPTION |
|----------------|---|
| 25 | 25 BLOWS DROVE SAMPLER 12 INCHES, AFTER INITIAL 6 INCHES OF SEATING |
| 50/7" | 50 BLOWS DROVE SAMPLER 7 INCHES, AFTER INITIAL 6 INCHES OF SEATING |
| Ref/3" | 50 BLOWS DROVE SAMPLER 3 INCHES DURING OR AFTER INITIAL 6 INCHES OF SEATING |

NOTE: TO AVOID DAMAGE TO SAMPLING TOOLS, DRIVING IS LIMITED TO 50 BLOWS PER 6 INCHES DURING OR AFTER SEATING INTERVAL

KEY TO TEST DATA

| В | Bag Sample | CONS | Consolidation (ASTM D2435) |
|---------------|---|------|---|
| | Drive, No Sample Collected | DS | Cons. Drained Direct Shear (ASTM D3080) |
| | 2 1/2" O.D. Mod. California Sampler, Not Tested | PP | Pocket Penetrometer |
| | 2 1/2" O.D. Mod. California Sampler, Tested | GSD | Grain Size Distribution (ASTM D422) |
| | Standard Penetration Test | CP | Compaction Test (ASTM D1557) |
| 0 | Sample Attempted with No Recovery | EI | Expansion Index (ASTM D4829) |
| \sqsubseteq | Water Level at Time of Drilling | LL | Liquid Limit (in percent) |
| <u></u> | Water Level after Drilling | PI | Plasticity Index |

RELATIVE DENSITY

| SANDS, GRAVELS, AND NON PLASTIC SILTS | BLOWS/FOOT |
|--|------------|
| VERY LOOSE | 0 - 4 |
| LOOSE | 4 - 10 |
| MEDIUM DENSE | 10 - 30 |
| DENSE | 30 - 50 |
| VERY DENSE | OVER 50 |
| | |

RELATIVE DENSITY

| CLAYS AND PLASTIC SILTS | STRENGTH | BLOWS/FOOT |
|-------------------------|-----------|------------|
| VERY SOFT | 0 - 1/4 | 0 - 2 |
| SOFT | 1/4 - 1/2 | 2 - 4 |
| FIRM | 1/2 - 1 | 4 - 8 |
| STIFF | 1 - 2 | 8 - 16 |
| VERY STIFF | 2 - 4 | 16 - 32 |
| HARD | OVER 4 | OVER 32 |



| PROJECT NO.: | 21-9798 |
|---------------|-----------|
| DATE DRILLED: | 6/18/2021 |

SOIL CLASSIFICATION CHART AND BORING LOG LEGEND

PROPOSED RESIDENCES
GREENFIELD, CALIFORNIA

FIGURE NO. **A-1**

| LO | GGED BY: | JM | | DRILL RIG: | Simc | o 2 | 400 | | | В | ORIN | IG NO.: | B-1 |
|---------------------------|------------------------------------|---------------------------|---|-----------------------|-----------|----------------|-------------------------|----------------------|----------------------|--------------|---------------|------------------------------|-------------------------------|
| EL | EVATION: | 270' | BORING DIAMET | ΓER (INCH): | 5 | | | | | DAT | E DF | RILLED: | 18 June 2021 |
| | | | GROUN | IDWATER DE | EPTH (F | Γ): | | | | | | | |
| ELEVATION (FT) | DEPTH (FT) GRAPHIC LOG | | GEOTECHNICAL DESCRIPTION | | SOIL TYPE | SAMPLE | CONV. SPT BLOW COUNT | WATER CONTENT (%) | DRY DENSITY (PCF) | LIQUID LIMIT | PLASIT. INDEX | UNC. COMP. STRENGTH (PSF) | COMMENTS AND ADDITIONAL TESTS |
| 269 268 | 1 - | | lly Silty Sand: brown, moist, e grained, loose | fine to | SM- GP | В | | | | | | | EI = 0 |
| 267 266 | 3 - 4 - | dense, | increasing gravel | | | 4 | 32 | 7.8 | | | | | |
| 265 264 263 | 5 — 6 — 7 — | Sandy grained dense | Gravel: brown, moist, fine t d sand, some cobbles, dense | o coarse e to very | GP- SP | В | | | | | | | |
| 262 | 8 — | | | | | | | | | | | | |
| 261 | 9 — | | | | | II | 52 | 9.8 | | | | | |
| 259 | 10 - | | | | | | | | | | | | |
| 258 257 | 12 - 13 - | | | | | | | | | | | | |
| 256 | 14 — | | | | | Ш | 50/7" | 9.2 | | | | | |
| 255 | 15 — | Boring | terminated at 15 feet | | | | | | | | | | |
| 254 253 | 16 — - 17 — | | | | | | | | | | | | |
| 252 | 18 — | | | | | | | | | | | | |
| 251 | 19 — | | | | | | | | | | | | |
| 250 | 20 — | | | | | | | | | | | | |
| | | | EXPL | ORATO | RY B | OF | RING | LOC | GS | | | | |
| | | 1 | | | 296 A | | | | | | | NCE -082-0 | |
| Posific Coast Testing Inc | | | | | | FIGURE NO. A-2 | | | | | | | |

| LOGGED BY: JM DRILL RIG: Simco 2400 BORING NO.: B-2 | | | | | | | o 2 | 400 | | | В | ORIN | IG NO.: | B-2 |
|---|------------|-------|--------|--|-----------------------|-----------|-----|------------------------------|----------------------------------|------|-----|------|---------|--------------|
| EL | EVA | TION: | 270' | BORING DIAMET | ΓER (INCH): | 5 | | | | | DAT | E DF | RILLED: | 18 June 2021 |
| | | | | GROUN | IDWATER DE | EPTH (F | Γ): | | | | | | | |
| ELEVATION (FT) | DEPTH (FT) | | | | | | | UNC. COMP. STRENGTH (PSF) | COMMENTS AND ADDITIONAL TESTS | | | | | |
| 269 | 1 | | | y Silty Sand: brown, moist, grained, loose | fine to | SM- GP | | | | | | | | |
| 268 | 2 | | | | | | В | | 7.1 | | | | | |
| 267 | 3 | | dense | | | | | | | | | | | |
| 266 | 4 | | | Gravel: brown, moist, fine t I sand, some cobbles, very | | GP- SP | II | 55 | 6.3 | | | | | |
| 265 | 5 | _ | | | | | | | | | | | | |
| 264 | 6 | _ | | | | | В | | | | | | | |
| 263 | 7 | | | | | | | | | | | | | |
| 262 | 8 | | | | | | | | | | | | | |
| 261 | 9 | _ | | | | | Ш | 50/2" | 6.9 | | | | | |
| 260 | 10 | _::: | Boring | terminated at 10 feet | | | - | | | | | | | |
| 259 | 11 | _ | | | | | | | | | | | | |
| 258 | 12 | | | | | | | | | | | | | |
| 257 | 13 | - | | | | | | | | | | | | |
| 256 | 14 | | | | | | | | | | | | | |
| 255 | 15 | - | | | | | | | | | | | | |
| 254 | 16 | | | | | | | | | | | | | |
| 253 | 17 | _ | | | | | | | | | | | | |
| 252 | 18 | _ | | | | | | | | | | | | |
| 251 | 19 | - | | | | | | | | | | | | |
| 250 | 20 | | | | | | | | | | | | | |
| | | | | EXPL | ORATO | RY B | OF | RING | LO | GS | | | | |
| | | | 1 | | | | | | | | | | NCE | |
| | , | | | | | | | | AVE | (API | | | |)13-000) |
| Positio Coast Tosting Inc | | | | | FIGURE NO. A-3 | | | | | | | | | |

| LO | GGE | D BY: | JM | | DRILL RIG: | Simc | o 2 | 400 | | | В | ORIN | IG NO.: | B-3 |
|----------------|--|-------------|-----------|---|--------------|-----------|-----------|-------------------------|----------------------|----------------------|--------------|---------------|------------------------------|----------------------------------|
| EL | .EVA | TION: | 270' | BORING DIAME | ΓER (INCH): | 5 | | | | | DAT | E DF | RILLED: | 18 June 2021 |
| | | | | GROUN | IDWATER DI | EPTH (F | Γ): | | | | | | | |
| ELEVATION (FT) | DEPTH (FT) | GRAPHIC LOG | | GEOTECHNICAL DESCRIPTION | | SOIL TYPE | SAMPLE | CONV. SPT BLOW COUNT | WATER CONTENT (%) | DRY DENSITY (PCF) | LIQUID LIMIT | PLASIT. INDEX | UNC. COMP. STRENGTH (PSF) | COMMENTS AND ADDITIONAL TESTS |
| 269 | 1 | | | Sand: brown, slightly mo rained, some silt, loose | ist, fine to | SP- GP | | | | | | | | |
| 268 | 2 | | | | | | В | | 3.3 | | | | | |
| 267 | 3 | | dense | | | | | | | | | | | |
| 266 | 4 | | Sandy Gr | ravel: brown, moist, fine t | o coarse | GP- | <u>II</u> | 43 | 4.3 | | | | | |
| 265 | 5 | | | and, some cobbles, very | | SP- | | | | | | | | |
| 264 | 6 | | | | | | В | | | | | | | |
| 262 | 8 | | | | | | | | | | | | | |
| 261 | 9 | | | | | | | | | | | | | |
| 260 | 10 | _ | Boring te | rminated at 10 feet | | | " | 50 | 5.4 | | | | | |
| 259 | 11 | - | Doming to | Trimitatou at 10 100t | | | | | | | | | | |
| 258 | 12 | - | | | | | | | | | | | | |
| 257 | 13 | | | | | | | | | | | | | |
| 256 | 14 | _ | | | | | | | | | | | | |
| 255 | 15 | _ | | | | | | | | | | | | |
| 254 | 16 | _ | | | | | | | | | | | | |
| 253 | 17 | - | | | | | | | | | | | | |
| 252 251 | 18 19 | - | | | | | | | | | | | | |
| 250 | 20 | - | | | | | | | | | | | | |
| | | | l | EXPL | ORATO | RY B | OR | ING | LOC | 3S | | | | |
| | | | 1 | | | | P | ROF | POSI | ED R | ES | IDE | ENCE | S |
| | , | | | | - | | | | AVE | (API | | | | 013-000) |
| | PROJECT NO. DATE FIGURE N 21-9798 July-21 A-4 | | | | | | | | | | | | | |

| LO | GGED B | 3Y: , | JM | | DRILL RIG: | Simce | 2 | 400 | | | В | ORIN | IG NO.: | B-4 |
|----------------|---------------|--------------------------|------------------|--|--------------------|-----------|----------|----------------------|----------------------|----------------------|--------------|---------------|------------------------------|----------------------------------|
| EL | EVATIO | N: 2 | 270' | BORING DIAMET | ΓER (INCH): | 5 | | | | I | DAT | E DF | RILLED: | 18 June 2021 |
| | | | | GROUN | IDWATER DE | EPTH (F1 |): | | | | | | | |
| ELEVATION (FT) | | GRAPHIC LOG | | GEOTECHNICAL DESCRIPTION | | SOIL TYPE | SAMPLE | CONV. SPT BLOW COUNT | WATER CONTENT (%) | DRY DENSITY (PCF) | LIQUID LIMIT | PLASIT. INDEX | UNC. COMP. STRENGTH (PSF) | COMMENTS AND ADDITIONAL TESTS |
| 269 | | | | y Silty Sand: brown, moist, grained, loose | fine to | SM- GP | | | | | | | | |
| 268 | 2 – | | | | | | В | | 5.6 | | | | | |
| 267 | 3 – | | | | | | | | | | | | | |
| 266 | 4 | | | | | | | 29 | 5.9 | | | | | |
| 265 | 5 _ | 9 | Sandy grained | Gravel: brown, moist, fine t I sand, some cobbles, very | to coarse dense | GP | | | | | | | | |
| 264 | 6 – | | | | | | В | | | | | | | |
| 263 | 7 – | | | | | | | | | | | | | |
| 262 | 8 – | | | | | | | | | | | | | |
| 261 | 9 – | | | | | | Ш | 50/2" | 4.7 | | | | | |
| 260 | 10 — | E | Boring | terminated at 10 feet | | | | | | | | | | |
| 259 | 11 - | | | | | | | | | | | | | |
| 258 | 12 – | | | | | | | | | | | | | |
| 257 | 13 – | | | | | | | | | | | | | |
| 256 | 14 — | | | | | | | | | | | | | |
| 255 | 15 — | | | | | | | | | | | | | |
| 254 | 16 — | | | | | | | | | | | | | |
| 253 | 17 – | | | | | | | | | | | | | |
| 252 | 18 – | | | | | | | | | | | | | |
| 251 | 19 – | | | | | | | | | | | | | |
| 250 | 20 — | | | | | | | | | | | | | |
| | | | | EXPL | ORATO | RY B | OF | RING | LO | SS | | | | |
| | | | 1 | | | 000 - | | | | | | | ENCE | |
| | \mathcal{L} | | | | | | | | AVE | (API | | | |)13-000) FIGURE NO. |
| | Pacif | poific Coast Tosting Inc | | | | | A-5 | | | | | | | |

| LO | GGE | BY: | JM | | DRILL RIG: | Simce | o 2 | 400 | | | В | ORIN | NG NO.: | B-5 |
|---|-------------------|-------------|-----------------|--|--------------|-----------|--------|-------------------------|----------------------|----------------------|--------------|---------------|------------------------------|----------------------------------|
| EL | EVAT | ION: | 270' | BORING DIAMET | ΓER (INCH): | 5 | | | | | DAT | E DF | RILLED: | 18 June 2021 |
| | | | | GROUN | IDWATER DI | EPTH (F1 | Γ): | | | | | | | |
| ELEVATION (FT) | DEPTH (FT) | GRAPHIC LOG | | GEOTECHNICAL DESCRIPTION | | SOIL TYPE | SAMPLE | CONV. SPT BLOW COUNT | WATER CONTENT (%) | DRY DENSITY (PCF) | LIQUID LIMIT | PLASIT. INDEX | UNC. COMP. STRENGTH (PSF) | COMMENTS AND ADDITIONAL TESTS |
| 269 | 1 | | | lly Clayey Sand: brown, mo m grained, loose | ist, fine to | SC GP | D | | 5.0 | | | | | FL 44 |
| 268 | 2 | | | | | | В | | 5.0 | | | | | EI = 11 |
| 267 | 3 | | dense, | , increasing gravel | | | | | | | | | | |
| 266 | 4 | | | | | | Ш | 41 | 5.3 | | | | | |
| 265 | 5 | | Sandy | Gravel: brown, moist, fine t | o coarse | GP | | | | | | | | |
| 264 | 6 | | graine dense | d sand, some cobbles, dens | e to very | | | | | | | | | |
| 263 | 7 | | | | | | В | | | | | | | |
| 262 | 8 | | | | | | | | | | | | | |
| 261 | 9 | | | | | | Ш | 50/4" | 6.1 | | | | | |
| 260 | 10 | | gravell | ly sand (SP-GP) | | | | | | | | | | |
| 259 | 11 | | | | | | | | | | | | | |
| 258 | 12 | | | | | | | | | | | | | |
| 257 | 13 | | | | | | | | | | | | | |
| 256 | 14 | | | | | | В | | | | | | | |
| 255 | 15 | •••• | Boring | terminated at 15 feet | | | | | | | | | | |
| 254 | 16 | | | | | | | | | | | | | |
| 253 | 17 | | | | | | | | | | | | | |
| 252 | 18 | | | | | | | | | | | | | |
| 251 | 19 | | | | | | | | | | | | | |
| 250 | 20 | | | | | | | | | | | | | |
| | | • | <u> </u> | EXPL | ORATO | RY B | OF | RING | LO | GS | | | | |
| | | | 1 | | | | | | | | | | ENCE | |
| |) | | 1 | | | | | | AVE | (API | | | | 13-000) |
| PROJECT NO. DATE FIGURE NO. 21-9798 July-21 A-6 | | | | | | | | | | | | | | |

| LC | OGGED B | 3Y: . | M | DRILL RIG: | Simco | 2 | 400 | | | В | ORIN | IG NO.: | B-6 |
|----------------|-----------------------------|--------------|--|-------------|---------------------------------------|--------|-------------------------|----------------------|----------------------|--------------|-----------------------|------------------------------|----------------------------------|
| Е | LEVATIO | N: 2 | 70' BORING DIAME | TER (INCH): | 5 | | | | | DAT | E DF | RILLED: | 18 June 2021 |
| | | | GROU | NDWATER DI | EPTH (FT |): | | | | | | | |
| ELEVATION (FT) | | GRAPHIC LOG | GEOTECHNICAL DESCRIPTION | | SOIL TYPE | SAMPLE | CONV. SPT BLOW COUNT | WATER CONTENT (%) | DRY DENSITY (PCF) | LIQUID LIMIT | PLASIT. INDEX | UNC. COMP. STRENGTH (PSF) | COMMENTS AND ADDITIONAL TESTS |
| 269 | 1 - | n | Gravelly Silty Sand: brown, moist, nedium grained, loose | fine to | SM- GP | | | | | | | | |
| 268 | 2 - | | | | | В | | 4.8 | | | | | |
| 267 | 3 — | | | | | | | | | | | | |
| 266 | 4 — | | obbles, dense | | | | 50/2" | 5.9 | | | | | |
| 265 | 5 | III E | oring terminated at 5 feet | | | | | | | | | | |
| 264 | 6 – | | | | | | | | | | | | |
| 263 | 7 – | | | | | | | | | | | | |
| 262 | 8 – | | | | | | | | | | | | |
| 261 | 9 _ | | | | | | | | | | | | |
| 260 | 10 — | | | | | | | | | | | | |
| 259 | 11 — | | | | | | | | | | | | |
| 258 | 12 — | | | | | | | | | | | | |
| 257 | 13 — | | | | | | | | | | | | |
| 256 | 14 — | | | | | | | | | | | | |
| 255 | 15 — | | | | | | | | | | | | |
| 254 | 16 — | | | | | | | | | | | | |
| 253 | 17 — | | | | | | | | | | | | |
| 252 | 18 — | | | | | | | | | | | | |
| 251 | 19 - | | | | | | | | | | | | |
| 250 | 20 — | | | | | | | | | | | | |
| | | | EXPL | ORATO | RY BO | DR | ING | LOG | S | | | | |
| | | | 7 | | | | | | | | | NCE | |
| PROJECT NO | | | | | | AVE | (API | | | | 13-000) FIGURE NO. | | |
| | Pacific Coast Testing, Inc. | | | | PROJECT NO. DATE F 21-9798 July-21 | | | | | A-7 | | | |

| LC | OGGED BY: | JM | | DRILL RIG: | Simco | 2 | 400 | | | В | ORIN | NG NO.: | P-1 to 3 |
|---|------------------------|---------------------|---|------------------|-----------|--------|-------------------------|----------------------|----------------------|--------------|---------------|------------------------------|----------------------------------|
| Е | LEVATION: | 270' | BORING DIAME | TER (INCH): | 6 | | | | | DAT | E DF | RILLED: | 18 June 2021 |
| | | | GROUI | NDWATER D | EPTH (FT | Γ): | | | | | | | |
| ELEVATION (FT) | DEPTH (FT) GRAPHIC LOG | | GEOTECHNICAL DESCRIPTION | | SOIL TYPE | SAMPLE | CONV. SPT BLOW COUNT | WATER CONTENT (%) | DRY DENSITY (PCF) | LIQUID LIMIT | PLASIT. INDEX | UNC. COMP. STRENGTH (PSF) | COMMENTS AND ADDITIONAL TESTS |
| 269 | 1 - | Gravelly moist, fin | Silty Sand: brown, slightly e to medium grained, loos | / moist to se | SM- GP | | | | | | | | |
| 268 | 2 - | | | | | В | | 3.7 | | | | | |
| 267 | 3 — | | | | | | | | | | | | |
| 266 | 4 - | dense | | | | В | | 5.5 | | | | | |
| 265 | 5 | Borings to | erminated at 5 feet | | | | | | | | | | |
| 264 | 6 – | | | | | | | | | | | | |
| 263 | 7 - | | | | | | | | | | | | |
| 262 | 8 - | | | | | | | | | | | | |
| 261 | 9 — | | | | | | | | | | | | |
| 260 | 10 - | | | | | | | | | | | | |
| 259 | 11 — | | | | | | | | | | | | |
| 258 | 12 — | | | | | | | | | | | | |
| 257 | 13 — | | | | | | | | | | | | |
| 256 | 14 — | | | | | | | | | | | | |
| 255 | 15 — | | | | | | | | | | | | |
| 254 | 16 — | | | | | | | | | | | | |
| 253 | 17 — | | | | | | | | | | | | |
| 252 | 18 — | | | | | | | | | | | | |
| 251 | 19 | | | | | | | | | | | | |
| 250 | 20 — | | | | | | | | | | | | |
| | | | EXPL | ORATO | RY BO | DR | ING | LOG | S | | | | |
| | | 1 | | | 206 4 | | | | | | | NCE | |
| PROJECT NO. | | | | | (API | | 09- ATE | | FIGURE NO. | | | | |
| Pacific Coast Testing, Inc. 21-9798 July-21 | | | | | A-8 | | | | | | | | |

APPENDIX B

Moisture-Density Tests
Direct Shear Test
R-Value Test
Expansion Index Test

LABORATORY TESTING

Moisture-Density Tests

The field moisture content, as a percentage of the dry weight of the soil, was determined by weighing samples before and after oven drying. Dry densities, in pounds per cubic foot, were also determined for the undisturbed samples. Results of these determinations are shown in the Exploration Drill Hole Logs.

Direct Shear Test

Direct shear tests were performed on undisturbed samples, to determine strength characteristics of the soil. The test specimens were soaked prior to testing. Results of the shear strength tests are attached.

Resistance (R) Value Test

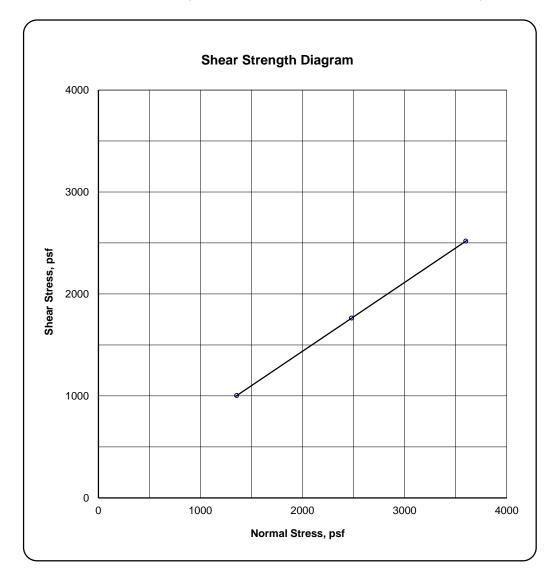
An R-Value test was estimated based on sieve analysis and plasticity on a bulk sample obtained from boring B-1. The results of the tests indicate that the gravelly silty sand soils have an R-Value of 40.

Expansion Index Test

An expansion index of 0 was obtained for the native gravelly silty sands encountered in boring B-1. The test procedure was performed in accordance with ASTM D4829 – Standard Test Method for Expansion Index of Soils.

DIRECT SHEAR TEST

ASTM D3080-11 (Modified for unconsolidated-undrained conditions)



| Sample Type: | Remolded Ring | Peak Shear Angle Cohesion (psf) | 34 90 |
|-------------------|---------------------|------------------------------------|----------|
| Soil Description: | Gravelly Silty Sand | Initial Moisture (%) | 7.8 |
| Sample Location: | B-1 @ 3 Feet | Initial Dry Density (pcf) | 101.7 |
| Project: PROPOS | SED RESIDENCES | Project No. | 21-9798 |

Acoustical Analysis



ACOUSTICAL ANALYSIS

296 APPLE AVENUE SUBDIVISION GREENFIELD, CALIFORNIA

WJVA Project No. 22-39

PREPARED FOR

EMC PLANNING 601 ABREGO STREET MONTEREY, CA 93940

PREPARED BY

WJV ACOUSTICS, INC. VISALIA, CALIFORNIA



DECEMBER 1, 2022

INTRODUCTION

The project site is comprised of Assessor's Parcel Number 109-082-013-000. The People's Self-Help Housing (PSHH) Housing Project is located on a 4.55-acre parcel at 296 Apple Avenue in Greenfield, California. The project site is currently vacant and undeveloped. To the west of the project site across 3rd Street is a park and agricultural land, with single-family residential properties surrounding the site to the south, east and north. The property is currently zoned Multi-Family Residential (R-M) with a residential density of 7 to 15 du/ac.

People's Self-Help Housing (PSHH) proposes to subdivide the property and build 36 detached single-family homes and a detention basin. The project address is 296 Apple Avenue, which is located on the NE corner of 3rd Street and Apple Avenue. The project site is zoned R-M (Multi-Family Residential). Proposed lots would be 3,160+ sf. The one- and two-story homes would range in size from 1,100 to 1,650 sf. Each home would have a 2-car garage as well as space to park two cars in the driveway. Proposed lots are 40 ft to 50 ft wide and 78 ft to 80 ft deep. Garages would be set back 20 ft to allow for cars to be parked in the driveways. Proposed side yards are 5 ft, and proposed rear yards are 10 ft.

The Planned Development component would allow reduced development standards for the Multiple-Family Residential (R-M) Zoning District, including reductions in minimum lot size and minimum lot width, while still resulting in a development that does not exceed the maximum allowed residential density of 15 dwelling units/acre.

This analysis, prepared by WJV Acoustics, Inc. (WJVA), is based upon a site visit and noise measurements conducted on June 28, 2022, project site plan provided by the applicant (Figure 1) and traffic obtained from the project traffic engineer (Associated Traffic Engineers). Any revisions to the analyzed site plan may require a reevaluation of the findings of this report.

Appendix A provides definitions of the acoustical terminology used in this report. Unless otherwise stated, all sound levels reported in this analysis are A-weighted sound pressure levels in decibels (dB). A-weighting de-emphasizes the very low and very high frequencies of sound in a manner similar to the human ear. Most community noise standards utilize A-weighted sound levels, as they correlate well with public reaction to noise. Appendix B provides typical A-weighted sound levels for common noise sources.

CRITERIA FOR ACCEPTABLE NOISE EXPOSURE

City of Greenfield-

The City of Greenfield Noise Element of the General Plan establishes noise level criteria in terms of the L_{dn} metric. The L_{dn} (Day-Night Average Level) is the time-weighted energy average noise level for a 24-hour day, with a 10 dB penalty added to noise levels occurring during the nighttime hours (10:00 p.m.-7:00 a.m.). The L_{dn} represents cumulative exposure to noise over an extended period of time and is therefore calculated based upon *annual average* conditions.

The Noise Element establishes a land use compatibility maximum noise level criterion of 60 dB L_{dn} for exterior transportation noise exposure in outdoor activity areas of new residential developments. The Noise Element also states "Where it is not possible to reduce noise in outdoor activity areas to 60 dB L_{dn} or less using a practical application of the best-available noise reduction measures, an exterior noise level of up to 65 dB L_{dn} may be allowed provided that available exterior noise level reduction measures have been implemented and interior noise levels are in compliance with this table." Outdoor activity areas generally include backyards of single-family residences and common use areas and individual patios or balconies of multi-family developments. The intent of the exterior noise level requirement is to provide an acceptable noise environment for outdoor activities and recreation. Table I provides the noise level standards for transportation noise sources applicable to the project.

The Noise Element also requires that interior noise exposure attributable to exterior noise sources not exceed 45 dB L_{dn}. The intent of the interior noise level standard is to provide an acceptable noise environment for indoor communication and sleep.

Table I
Noise Standards for New Uses Affected by Transportation Noise
City of Greenfield Noise Element

| New Land Use | Outdoor Activity Area - Ldn | Interior - Ldn/Peak Hour Leq1 | Notes |
|--|--------------------------------|----------------------------------|---------|
| All Residential | 60 | 45 | 2, 3, 4 |
| Transient Lodging | 65 | 45 | 5 |
| Hospitals & Nursing Homes | 60 | 45 | 6 |
| Theaters & Auditoriums | | 35 | |
| Churches, Meeting Halls, Schools, Libraries, etc. | 60 | 40 | |
| Office Buildings | 65 | 45 | 7 |
| Commercial Buildings | 65 | 50 | 7 |
| Playgrounds, Parks, etc. | 70 | | |
| Industry | 65 | 50 | 7 |

Notes:

- 1. For traffic noise within the City of Greenfield, Ldn and peak-hour Leq values are estimated to be approximately similar. Interior noise level standards are applied within noise-sensitive areas of the various land uses, with windows and doors in the closed positions.
- Outdoor activity areas for single-family residential uses are defined as back yards. For large parcels or residences
 with no clearly defined outdoor activity area, the standard shall be applicable within a 100-foot radius of the
 residence.
- 3. For multi-family residential uses, the exterior noise level standard shall be applied at the common outdoor recreation area, such as at pools, play areas or tennis courts. Where such areas are not provided, the standards shall be applied at individual patios and balconies of the development.
- 4. Where it is not possible to reduce noise in outdoor activity areas to 60 dB Ldn or less using a practical application of the best-available noise reduction measures, an exterior noise level of up to 65 dB Ldn may be allowed provided that available exterior noise level reduction measures have been implemented and interior noise levels are in compliance with this table.
- 5. Outdoor activity areas of transient lodging facilities include swimming pool and picnic areas.
- 6. Hospitals are often noise generating uses. The exterior noise level standards for hospitals are applicable only at clearly identified areas designated for outdoor relaxation by either hospital staff or patients.
- Only the exterior spaces of these uses designated for employee or customer relaxation have any degree of sensitivity to noise.

The City of Greenfield General Plan Noise Element also establishes noise level standard for non-transportation (stationary) noise sources. Table II provides the applicable noise level standards for stationary noise sources.

Table II

Noise Standards for New Uses Affected by Non-Transportation Noise

City of Greenfield Noise Element

| | Outdoor Activit | y Area - Leq | Interior - Leq | | |
|--|-----------------|--------------|----------------|-------|--|
| New Land Use | Daytime | Nighttime | Day and Night | Notes | |
| All Residential | 50 | 45 | 35 | 1, 2 | |
| Transient Lodging | 55 | | 40 | 3 | |
| Hospitals & Nursing Homes | 50 | 45 | 35 | 4 | |
| Theaters & Auditoriums | | | 35 | | |
| Churches, Meeting Halls, Schools, Libraries, etc. | 55 | | 40 | | |
| Office Buildings | 55 | | 45 | 5, 6 | |
| Commercial Buildings | 55 | | 45 | 5, 6 | |
| Playgrounds, Parks, etc. | 65 | | | 6 | |
| Industry | 65 | 65 | 50 | 5 | |

Notes:

- 8. Outdoor activity areas for single-family residential uses are defined as back yards. For large parcels or residences with no clearly defined outdoor activity area, the standard shall be applicable within a 100-foot radius of the residence
- 9. For multi-family residential uses, the exterior noise level standard shall be applied at the common outdoor recreation area, such as at pools, play areas or tennis courts. Where such areas are not provided, the standards shall be applied at individual patios and balconies of the development.
- 10. Outdoor activity areas of transient lodging facilities include swimming pool and picnic areas, and are not commonly used during nighttime hours.
- 11. Hospitals are often noise generating uses. The exterior noise level standards for hospitals are applicable only at clearly identified areas designated for outdoor relaxation by either hospital staff or patients.
- 12. Only the exterior spaces of these uses designated for employee or customer relaxation have any degree of sensitivity to noise.
- 13. The outdoor activity areas of office, commercial and park uses are not typically utilized during nighttime hours.

General: The Table 5 standards shall be reduced by 5 dB for sounds consisting primarily of speech or music, and for recurring impulsive sounds. If the existing ambient noise level exceeds the standards of Table 5, then the noise level standards shall be increased at 5 dB increments to encompass the ambient.

Additional noise level standards are provided in Table 17.60-1a of the City's Municipal Code. However, the noise standards described above provided in the General Plan Noise Element are slightly more restrictive and are therefore used as a basis for project compliance.

Construction Noise and Vibration-

Section 9.28.030-D of the City of Greenfield Municipal Code provides restrictions associated with residential construction activities. The Municipal Code states the following:

Construction Activities: Unless otherwise provided by permit, construction activities shall only be permitted between the hours of seven o'clock (7:00) A.M. and seven o'clock (7:00) P.M. Monday through Friday and between nine o'clock (9:00) A.M. and five o'clock (5:00) P.M. on Saturday and Sunday. Extended construction work hours must at all times be in strict compliance with the applicable permit.

There are no state or federal standards that specifically address construction vibration. Some guidance is provided by the Caltrans Transportation and Construction Vibration Guidance Manual. The Manual provides guidance for determining annoyance potential criteria and damage potential threshold criteria. These criteria are provided below in Table III and Table IV, and are presented in terms of peak particle velocity (PPV) in inches per second (in/sec).

| TABLE III GUIDELINE VIBRATION ANNOYANCE POTENTIAL CRITERIA | | | | |
|--|----------------------|---|--|--|
| | Maximum PPV (in/sec) | | | |
| Human Response | Transient Sources | Continuous/Frequent Intermittent Sources | | |
| Barely Perceptible | 0.04 | 0.01 | | |
| Distinctly Perceptible | 0.25 | 0.04 | | |
| Strongly Perceptible 0.9 | | 0.1 | | |
| Severe 2.0 0.4 | | | | |
| Source: Caltrans | | | | |

| TABLE IV GUIDELINE VIBRATION DAMAGE POTENTIAL THRESHOLD CRITERIA | | | | |
|--|-------------------|---|--|--|
| | Maximum | PPV (in/sec) | | |
| Structure and Condition | Transient Sources | Continuous/Frequent Intermittent Sources | | |
| Extremely fragile, historic buildings, ancient monuments | 0.12 | 0.08 | | |
| Fragile buildings | 0.2 | 0.1 | | |
| Historic and some old buildings | 0.5 | 0.25 | | |
| Older residential structures | 0.5 | 0.3 | | |
| New residential structures | 1.0 | 0.5 | | |
| Modern industrial/commercial buildings 2.0 0.5 | | | | |
| Source: Caltrans | | | | |

PROJECT SITE NOISE EXPOSURE

The project site is located at the northeast corner of 3rd Street and Apple Avenue, in Greenfield, California. The dominant sources of noise affecting the project site is vehicle traffic along 3rd Street. Additional sources of noise observed during the project site visit include noise associated with agricultural activities, noise from US Highway 101 and occasional aircraft overflights.

Traffic Noise Exposure

Noise exposure from traffic on 3rd Street was calculated for existing and cumulative (future) conditions using the FHWA Traffic Noise Model and traffic data obtained from the project traffic consultant, Associated Traffic Engineers. Additionally, WJVA analyzed noise levels associated with traffic on US 101, using traffic data obtained from Caltrans.

WJVA utilized the Federal Highway Administration (FHWA) Highway Traffic Noise Prediction Model (FHWA-RD-77-108). The FHWA Model is a standard analytical method used for roadway traffic noise calculations. The model is based upon reference energy emission levels for automobiles, medium trucks (2 axles) and heavy trucks (3 or more axles), with consideration given to vehicle volume, speed, roadway configuration, distance to the receiver, and the acoustical characteristics of the site. The FHWA Model was developed to predict hourly Leq values for free-flowing traffic conditions, and is generally considered to be accurate within ±1.5 dB. To predict Ldn values, it is necessary to determine the hourly distribution of traffic for a typical day and adjust the traffic volume input data to yield an equivalent hourly traffic volume.

Noise level measurements and concurrent traffic counts were conducted by WJVA staff within the project site on June 28, 2022. The purpose of the measurements was to evaluate the accuracy of the FHWA Model in describing traffic noise exposure within the project site. The measurement site was located adjacent to the project site at a distance of approximately 100 feet from the centerline of 3rd Street. The speed limit posted in the project vicinity was 35 mph (miles per hour). The project vicinity and noise monitoring site location are provided as Figure 2. A photograph showing the 3rd Street noise measurement site is provided as Figure 3. It should be noted, traffic volumes along the project site frontage with Apple Avenue are extremely low and a traffic noise calibration measurement was not possible. Additionally, due to the low traffic volumes along Apple Avenue, the project Traffic Study did not provide traffic volumes for Apple Avenue. Traffic along Apple Avenue is not considered to be a significant source of noise within the project site.

Noise monitoring equipment consisted of Larson-Davis Laboratories Model LDL-820 sound level analyzer equipped with a B&K Type 4176 1/2" microphone. The equipment complies with the specifications of the American National Standards Institute (ANSI) for Type I (Precision) sound level meters. The meter was calibrated in the field prior to use with a B&K Type 4230 acoustic calibrator to ensure the accuracy of the measurements. The microphone was located on a tripod at 5 feet above the ground.

Noise measurements were conducted in terms of the equivalent energy sound level (L_{eq}). Measured L_{eq} values were compared to L_{eq} values calculated (predicted) by the FHWA Model

using as inputs the traffic volumes, truck mix and vehicle speed observed during the noise measurements. The results of the comparison are shown in Table V.

TABLE V

COMPARISON OF MEASURED AND PREDICTED (FHWA MODEL) NOISE LEVELS 296 APPLE AVENUE SUBDIVISION, GREENFIELD

| | 3 rd Street |
|--|------------------------|
| Measurement Start Time | 11:40 a.m. |
| Observed # Autos/Hr. | 156 |
| Observed # Medium Trucks/Hr. | 24 |
| Observed # Heavy Trucks/Hr. | 0 |
| Observed Speed (MPH) | 35 |
| Distance, ft. (from center of roadway) | 100 |
| L _{eq} , dBA (Measured) | 53.5 |
| L _{eq} , dBA (Predicted) | 54.4 |
| Difference between Predicted and Measured Leq, dBA | 0.9 |

Note: FHWA "soft" site assumed for calculations.

Source: WJV Acoustics, Inc.

From Table V it may be determined that the traffic noise levels predicted by the FHWA Model were 0.9 dB higher than those measured for the conditions observed at the time of the noise measurements for 3rd Street. This is considered to be reasonable agreement with the model and therefore no adjustments to the model are necessary. The slight overprediction of the model when compared to measured noise levels is likely the result of traffic speeds below 35 mph, as there is an existing stop sign at Apple Avenue.

Annual Average Daily Traffic (AADT) data for 3rd Avenue in the project vicinity was obtained from the above-described project traffic study. Truck percentages and the day/night distribution of traffic were estimated by WJVA, based upon site previous studies prepared along similar roadways. A speed limit of 35 mph was assumed for the roadway for existing and future roadway configurations. Table VI summarizes annual average traffic data used to model noise exposure within the project site.

TABLE VI

3rd STREET TRAFFIC NOISE MODELING ASSUMPTIONS 296 APPLE AVENUE SUBDIVISION, GREENFIELD

| | 3 rd Street | | |
|------------------------------------|------------------------|------------|--|
| | Existing | Cumulative | |
| Annual Avenue Daily Traffic (AADT) | 3,270 | 3,560 | |
| Day/Night Split (%) | 90/10 | | |
| Assumed Vehicle Speed (mph) | 35 | | |
| % Medium Trucks (% AADT) | 2 | | |
| % Heavy Trucks (% AADT) | 1 | | |

Sources: Associated Traffic Engineers, Inc. WJV Acoustics, Inc.

Using data from Table VI, the FHWA Model, annual average traffic noise exposure was calculated for the closest proposed backyards from 3rd Street (approximately 60 feet from the roadway centerline). The calculated noise exposures for existing and cumulative traffic conditions for the closest proposed setbacks to 3rd Street were approximately 58 dB L_{dn} for both traffic scenarios.

WJVA also calculated project site traffic noise exposure related to traffic along US 101, which is located approximately 1,550 feet from the project site. Existing traffic volumes and truck percentages for US 101 in the project vicinity were obtained from Caltrans. Caltrans does not provide any future modeled traffic volumes. Therefore, WJVA referred to a traffic analysis provided for a previous study in Gonzales (City of Gonzales Sphere of Influence Circulation Study, Kimley-Horn, 2019), which analyzed future traffic volumes on US 101. WJVA applied the traffic volume growth rate for US 101 (derived from the Gonzales Circulation Study) to existing US 101 traffic volumes in the vicinity of the Apple Avenue Subdivision project, in Greenfield. Table VII summarizes annual average US 101 traffic data used to model noise exposure within the project site.

TABLE VII

US 101 TRAFFIC NOISE MODELING ASSUMPTIONS 296 APPLE AVENUE SUBDIVISION, GREENFIELD

| | US 101 | | |
|------------------------------------|----------|--------|--|
| | Existing | Future | |
| Annual Avenue Daily Traffic (AADT) | 29,000 | 37,990 | |
| Day/Night Split (%) | 83/17 | | |
| Assumed Vehicle Speed (mph) | 65 | | |
| % Medium Trucks (% AADT) | 6.3 | | |
| % Heavy Trucks (% AADT) | 9.9 | | |

Sources: Caltrans, Kimley-Horn WJV Acoustics, Inc.

Using data from Table VII, the FHWA Model, annual average traffic noise exposure was calculated for the closest proposed residential lots from US 101 (approximately 1,550 feet from the roadway centerline). The calculated noise exposures for existing and future traffic conditions for the closest proposed setbacks to US 101 were approximately 56 dB L_{dn}, and 58 dB L_{dn}, respectively.

In order to determine the overall project site traffic noise exposure, traffic noise exposure for both 3rd Street and US 101 were combined. The calculated project site noise exposure for combined (3rd Street and US 101) for existing and future/cumulative traffic conditions was calculated to be as follows:

Existing: 60 dB L_{dn}

• Cumulative/Future: 61 dB Ldn

The future noise exposure at the closest proposed lots to 3rd Street (and US 101) would have a combined cumulative/future noise exposure level of approximately 61 dB L_{dn}. Such levels exceed the City's 60 dB L_{dn} exterior noise level standard, and mitigation measures must be considered. The project proposes to include a 6-foot concrete panel screen wall along the 3rd Street project site frontage. The associated noise level reduction provided by the proposed concrete panel wall is discussed below.

Interior Noise Level Exposure:

The City's interior noise level standard is 45 dB L_{dn} . The worst-case exterior noise exposure for the residential buildings would be 61 dB L_{dn} . This means that the proposed residential construction must be capable of providing a minimum (worst-case scenario) outdoor-to-indoor noise level reduction (NLR) of approximately 16 dB (61-45=16).

A specific analysis of interior noise levels was not performed. However, it may be assumed that residential construction methods complying with current building code requirements will reduce exterior noise levels by a minimum of 10 dB with doors and windows open and 25 dB if windows and doors may remain closed.

NOISE MITIGATION

Exterior Noise Mitigation:

The exterior noise level within the closest proposed residential outdoor activity areas (backyards) to 3rd Street and US 101 would be exposed to an exterior noise exposure of approximately 61 dB L_{dn}, for future traffic conditions. The City of Greenfield Noise Element of the General Plan establishes a 60 dB L_{dn} criterion within outdoor activity areas. As described above, the proposed project design includes a 6-foot concrete panel screen wall along the 3rd Street project site roadway frontage. The noise level reduction provided by the proposed 6-foot concrete wall is discussed below.

A sound wall insertion loss program based on the FHWA Model was used to calculate the insertion loss (noise reduction) provided by the proposed 6-foot concrete panel screen wall. The model calculates the insertion loss of a wall of given height based on the effective height of the noise source, height of the receiver, distance from the receiver to the wall, and distance from the noise source to the wall. The standard assumptions used in the sound wall calculations are effective source heights of 8, 2 and 0 feet above the roadway for heavy trucks, medium trucks and automobiles, respectively. The standard height of a residential receiver is five feet above the ground elevation. It was assumed by WJVA that the building pad elevations at the closest proposed homes to 3rd Street would be approximately the same elevation as the roadway pavement (existing conditions).

Based upon the above-described assumptions and method of analysis, the noise level insertion loss value for the proposed concrete wall was calculated. The calculations indicated that the proposed 6-foot concrete panel screen wall would reduce traffic noise exposure within individual backyards by approximately 6 dB (3rd Street traffic noise exposure), resulting in a projected future exposure of approximately 55 dB L_{dn}. Such levels do not exceed the City's 60 dB L_{dn} exterior nose level standard.

Interior Noise Mitigation:

The proposed 6-foot sound wall along 3rd Street would provide exterior noise mitigation for first-floor construction only, but would not provide acoustic shielding at second-floor receiver locations. The closest proposed homes to 3rd Street (and US 101) would include one single-story home (Lot 6) and four two-story homes (Lot 7, Lot 14, Lot 15 and Lot 20). The two-story homes would provide additional acoustic shielding (noise attenuation) to the proposed homes located to the east. Furthermore, the homes located directly east of Lot 6 are all single-family construction. Therefore, proposed homes on Lot 7, Lot 14, Lot 15 and Lot 20 must include air conditioning or mechanical ventilation, so that windows and doors can remain closed for sound insulation purposes. Exterior noise levels at all other proposed homes will be sufficiently attenuated by the proposed 6-foot wall and/or the two-story construction homes located to their east.

CONSTRUCTION NOISE AND VIBRATION

Construction Noise-

Construction noise would occur at various locations within the project site through the build-out period. Construction activities could occur at distances of 50 feet or less from existing residential land uses. Table VIII provides typical construction-related noise levels at distances of 25 feet, 50 feet, and 100 feet.

| | TABLI TYPICAL CONSTRUC MAXIMUM NOISE | CTION EQUIPMENT E LEVELS, dBA | |
|---------------------|--------------------------------------|----------------------------------|---------|
| Type of Equipment | 25 Ft. | 50 Ft. | 100 Ft. |
| Concrete Saw | 96 | 90 | 84 |
| Crane | 87 | 81 | 75 |
| Excavator | 87 | 81 | 75 |
| Front End Loader | 85 | 79 | 73 |
| Jackhammer | 95 | 89 | 83 |
| Paver | 83 | 77 | 71 |
| Pneumatic Tools | 91 | 85 | 79 |
| Dozer | 87 | 81 | 76 |
| Rollers | 86 | 80 | 74 |
| Trucks | 92 | 86 | 80 |
| Pumps | 86 | 80 | 74 |
| Scrapers | 93 | 87 | 81 |
| Portable Generators | 87 | 81 | 74 |
| Backhoe | 92 | 86 | 80 |
| Grader | 92 | 86 | 80 |

Source: FHWA

Noise Control for Buildings and Manufacturing Plants, Bolt, Beranek & Newman, 1987

Noise impacts associated with construction activities typically depend on the noise levels generated by the type of equipment in use, the duration of usage of the equipment and the distance at which the equipment is used in respect to nearby sensitive receptors. Noise impacts typically occur when construction activities occur beyond the limited allowable hours of construction.

Construction noise is typically not considered to be a significant impact if construction is limited to the daytime hours and construction equipment is adequately maintained and muffled. Extraordinary noise-producing activities (e.g., pile driving) are not anticipated. In this case, all project construction activity must be confined to the hours of 7:00 a.m. to 7:00 p.m., Monday through Friday and between 9:00 a.m. to 5:00 p.m. on Saturday and Sunday. Construction noise impacts could result in annoyance or sleep disruption for nearby residents if nighttime operations were to occur or if equipment is not properly muffled or maintained.

Vibration-

Vibration from demolition and construction activities could be detected at the closest sensitive land uses, especially during movements by heavy equipment or loaded trucks and during some paving activities. Typical vibration levels at distances of 25 feet, 100 feet and 300 feet are summarized by Table IX. These levels would not be expected to exceed any significant threshold levels for annoyance or damage, as provided above in Table III and Table IV.

| TABLE IX | | | | | | | |
|--|-------|-------------------|--------|--|--|--|--|
| TYPICAL VIBRATION LEVELS DURING CONSTRUCTION | | | | | | | |
| | | PPV (in/sec) | | | | | |
| Equipment | @ 25′ | @ 100′ | @ 300′ | | | | |
| Bulldozer (Large) | 0.089 | 0.019 | 0.006 | | | | |
| Bulldozer (Small) | 0.003 | 0.0006 | 0.0002 | | | | |
| Loaded Truck | 0.076 | 0.017 | 0.005 | | | | |
| Jackhammer | 0.035 | 0.008 | 0.002 | | | | |
| Vibratory Roller | 0.210 | 0.046 | 0.013 | | | | |
| Caisson Drilling | 0.089 | 0.089 0.019 0.006 | | | | | |

After full project build out, it is not expected that ongoing operational activities will result in any vibration impacts at nearby sensitive uses. Activities involved in trash bin collection could result in minor on-site vibrations as the bin is placed back onto the ground (if such activities were to occur). Such vibrations would not be expected to be felt at the closest off-site sensitive uses.

CONCLUSIONS AND RECOMMENDATIONS

The project site is exposed to traffic noise from 3rd Street and U.S. Route 101. WJVA's analysis of the project indicates that the project will comply with all applicable City of Greenfield exterior and interior noise level standards provided the following mitigation measure is incorporated into project design:

 Proposed homes on Lot 7, Lot 14, Lot 15 and Lot 20 must include air conditioning or mechanical ventilation, so that windows and doors can remain closed for sound insulation purposes.

The conclusions and recommendations of this acoustical analysis are based upon the best information known to WJV Acoustics, Inc. (WJVA) at the time the analysis was prepared concerning the proposed project site, roadway configurations, traffic volumes and vehicle speeds. Any significant changes to these factors may require revisions to the findings of this report. Additionally, any significant future changes in motor vehicle technology, noise regulations or other factors beyond WJVA's control may result in long-term noise results different from those described by this analysis.

Respectfully submitted,

Walter J. Van Groningen

Multh Vans

President

WJV:wjv

FIGURE 1: PROJECT SITE PLAN

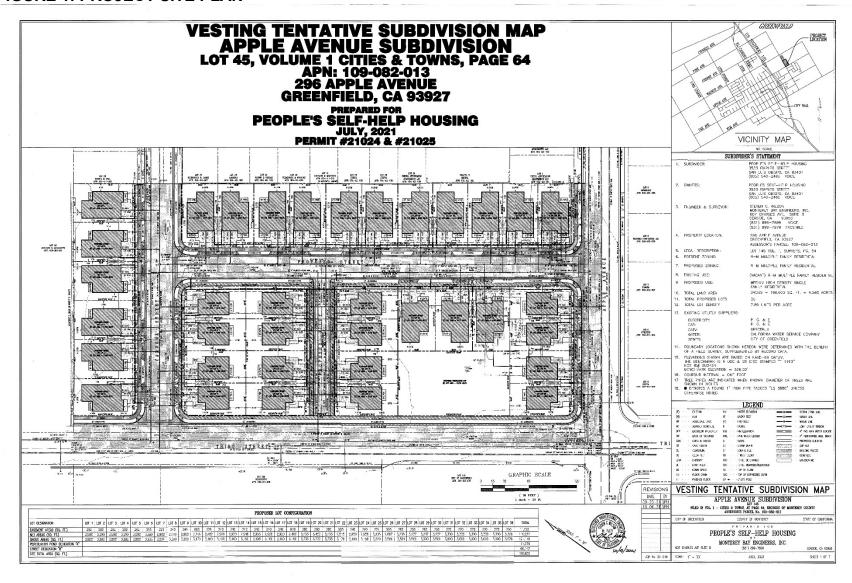


FIGURE 2: PROJECT SITE VICINITY AND NOISE MONITORING SITE LOCATIONS

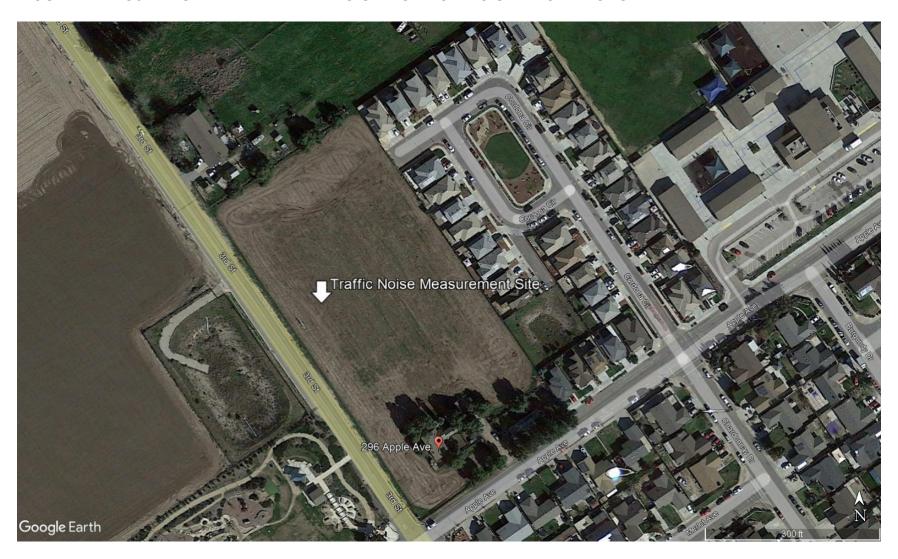


FIGURE 3: TRAFFIC NOISE MEASUREMENT LOCATION



APPENDIX A

ACOUSTICAL TERMINOLOGY

AMBIENT NOISE LEVEL: The composite of noise from all sources near and far. In this context, the ambient noise level constitutes the normal or existing level of environmental noise at a given location. CNEL: Community Noise Equivalent Level. The average equivalent sound level during a 24-hour day, obtained after addition of approximately five decibels to sound levels in the evening from 7:00 p.m. to 10:00 p.m. and ten decibels to sound levels in the night before 7:00 a.m. and after 10:00 p.m. DECIBEL, dB: A unit for describing the amplitude of sound, equal to 20 times the logarithm to the base 10 of the ratio of the pressure of the sound measured to the reference pressure, which is 20 micropascals (20 micronewtons per square meter). DNL/Ldn: Day/Night Average Sound Level. The average equivalent sound level during a 24-hour day, obtained after addition of ten decibels to sound levels in the night after 10:00 p.m. and before 7:00 a.m. Equivalent Sound Level. The sound level containing the same L_{eq}: total energy as a time varying signal over a given sample period. L_{eq} is typically computed over 1, 8 and 24-hour sample periods. NOTE: The CNEL and DNL represent daily levels of noise exposure averaged on an annual basis, while Leq represents the average noise exposure for a shorter time period, typically one hour.

exceeded 10 percent of the time.

The maximum noise level recorded during a noise event.

The sound level exceeded "n" percent of the time during a sample

interval (L_{90} , L_{50} , L_{10} , etc.). For example, L_{10} equals the level

L_{max}:

L_n:

ACOUSTICAL TERMINOLOGY

NOISE EXPOSURE CONTOURS:

Lines drawn about a noise source indicating constant levels of noise exposure. CNEL and DNL contours are frequently utilized to describe community exposure to noise.

NOISE LEVEL REDUCTION (NLR):

The noise reduction between indoor and outdoor environments or between two rooms that is the numerical difference, in decibels, of the average sound pressure levels in those areas or rooms. A measurement of Anoise level reduction@ combines the effect of the transmission loss performance of the structure plus the effect of acoustic absorption present in the receiving room.

SEL or SENEL:

Sound Exposure Level or Single Event Noise Exposure Level. The level of noise accumulated during a single noise event, such as an aircraft overflight, with reference to a duration of one second. More specifically, it is the time-integrated A-weighted squared sound pressure for a stated time interval or event, based on a reference pressure of 20 micropascals and a reference duration of one second.

SOUND LEVEL:

The sound pressure level in decibels as measured on a sound level meter using the A-weighting filter network. The A-weighting filter de-emphasizes the very low and very high frequency components of the sound in a manner similar to the response of the human ear and gives good correlation with subjective reactions to noise.

SOUND TRANSMISSION CLASS (STC):

The single-number rating of sound transmission loss for a construction element (window, door, etc.) over a frequency range where speech intelligibility largely occurs.

APPENDIX B EXAMPLES OF SOUND LEVELS

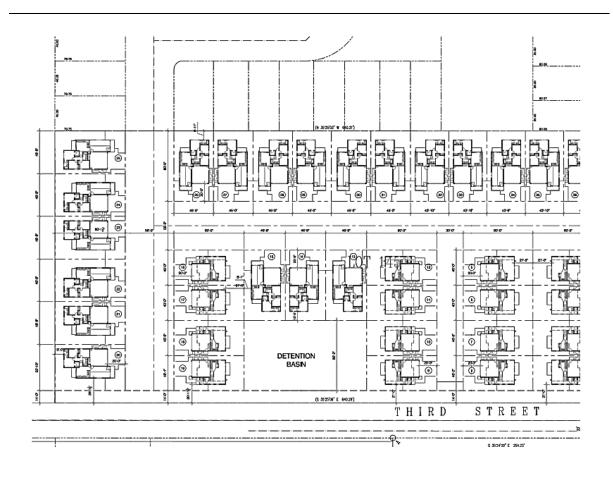
SUBJECTIVE NOISE SOURCE SOUND LEVEL **DESCRIPTION** 120 dB AMPLIFIED ROCK 'N ROLL > **DEAFENING** JET TAKEOFF @ 200 FT ▶ 100 dB **VERY LOUD** BUSY URBAN STREET > 80 dB **LOUD** FREEWAY TRAFFIC @ 50 FT > CONVERSATION @ 6 FT ▶ 60 dB **MODERATE** TYPICAL OFFICE INTERIOR > 40 dB SOFT RADIO MUSIC > **FAINT** RESIDENTIAL INTERIOR > WHISPER @ 6 FT ▶ 20 dB **VERY FAINT** HUMAN BREATHING > 0 dB

Traffic and Circulation Study



296 APPLE AVENUE RESIDENTIAL PROJECT CITY OF GREENFIELD, CALIFORNIA

TRAFFIC AND CIRCULATION STUDY



July 6, 2021 ATE #21040

People's Self Help Housing 3533 Empleo Street San Luis Obispo, CA 93401



ASSOCIATED TRANSPORTATION ENGINEERS

100 N. Hope Avenue, Suite 4, Santa Barbara, CA 93110-1686 * (805) 687-4418 * FAX (805) 682-8509



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Since 1978

Richard L. Pool, P.E. Scott A. Schell

July 6, 2021 21040R01

Sheryl Flores People's Self Help Housing

Delivered Via Email: sherylf@pshhc.org

TRAFFIC AND CIRCULATION STUDY FOR THE 296 APPLE AVENUE RESIDENTIAL PROJECT, CITY OF GREENFIELD

Associated Transportation Engineers (ATE) has prepared the following traffic and circulation study for the 296 Apple Avenue Residential Project, proposed in the City of Greenfield. It is understood that the study will be submitted to the City for environmental review.

We appreciate the opportunity to assist you with the project.

Associated Transportation Engineers

Scott A. Schell

Principal Transportation Planner

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INTRODUCTION

The following report addresses the traffic and circulation issues for the 296 Apple Avenue Project (the "Project") proposed in the City of Greenfield. The report evaluates existing and future traffic operations within the Project study area to determine the Project's consistency with the City's General Plan level of service policies. The transportation facilities analyzed in the study were determined based on input provided by City staff. An evaluation of the Project's potential CEQA impacts is also provided based on the new Vehicle Miles Travelled (VMT) requirements adopted under Senate Bill 743.

PROJECT DESCRIPTION

The Project is proposed on the northeast corner of the 3rd Street/Apple Avenue intersection on the east side of US 101 in the City of Greenfield. Figure 1 shows the location of the Project site within the City. The Project is proposing to construct 36 affordable single-family residential units. Figure 2 shows the Project site plan. As shown, access is proposed via one roadway connection to 3rd Street and one roadway connection to Apple Avenue.

EXISTING CONDITIONS

Street Network

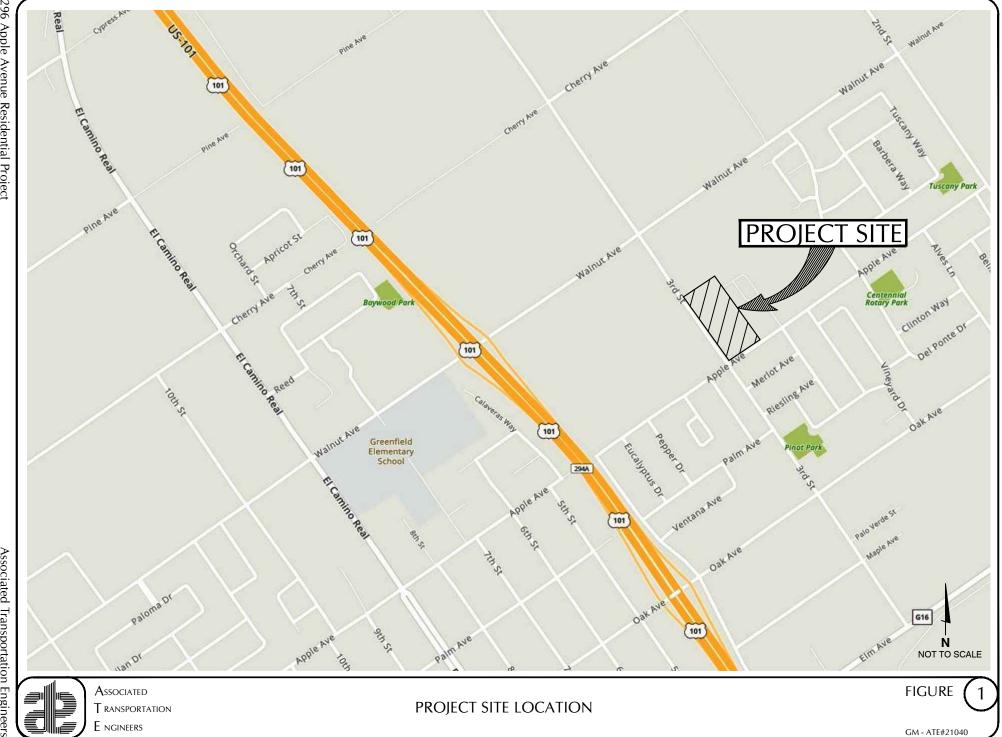
The Project site is served by a network of highways, arterial, collector, and local streets. Figure 3 illustrates the study-area street network, including the traffic controls and lane geometries at the key study-area intersections identified for analysis by the City. The following text provides a brief discussion of the existing street network.

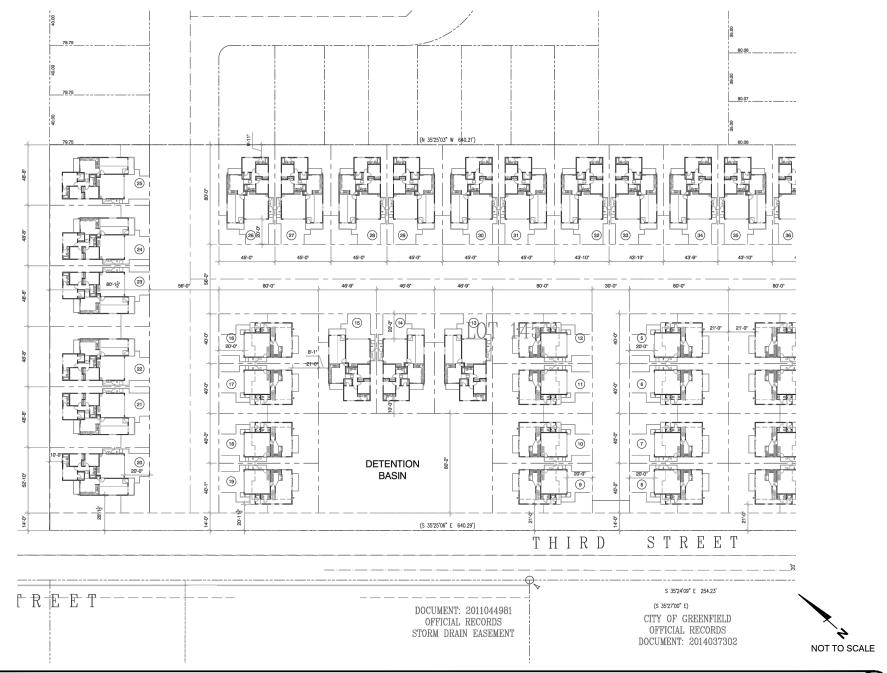
<u>US 101</u>, located west of the Project site, is a north-south freeway that connects Greenfield with Soledad to the north and King City to the south. US 101 is a 4-lane freeway within the Greenfield area.

<u>Walnut Avenue</u>, located north of the Project site, is a 2-lane east-west roadway that serves the central portion of the City. Walnut Avenue also provides direct access to/from US 101 via a full-access tight-diamond interchange. Walnut Avenue extends to 14th Street west of US 101 and to 2nd east of US 101.

 3^{rd} Street, located along the Project's western frontage, is a 2-lane north-south collector street that parallels the eastside of US 101. 3^{rd} Street extends from north of Walnut Avenue to Pine Avenue on the south. The Project would take access from 3^{rd} Street via a single connection located near the north side of the site. This east-west road would also connect to the existing residential street (Cardona Circle) that serves the neighborhood located immediately east of the Project site (see Figure 2 – Project Site Plan).

<u>Apple Avenue</u>, located along the Project's southern frontage, is a 2-lane east-west collector street that extends from 4th Street just east of US 101 to 2nd Street. The Project site would access Apple Avenue via a single connection (see Figure 2 – Project Site Plan).

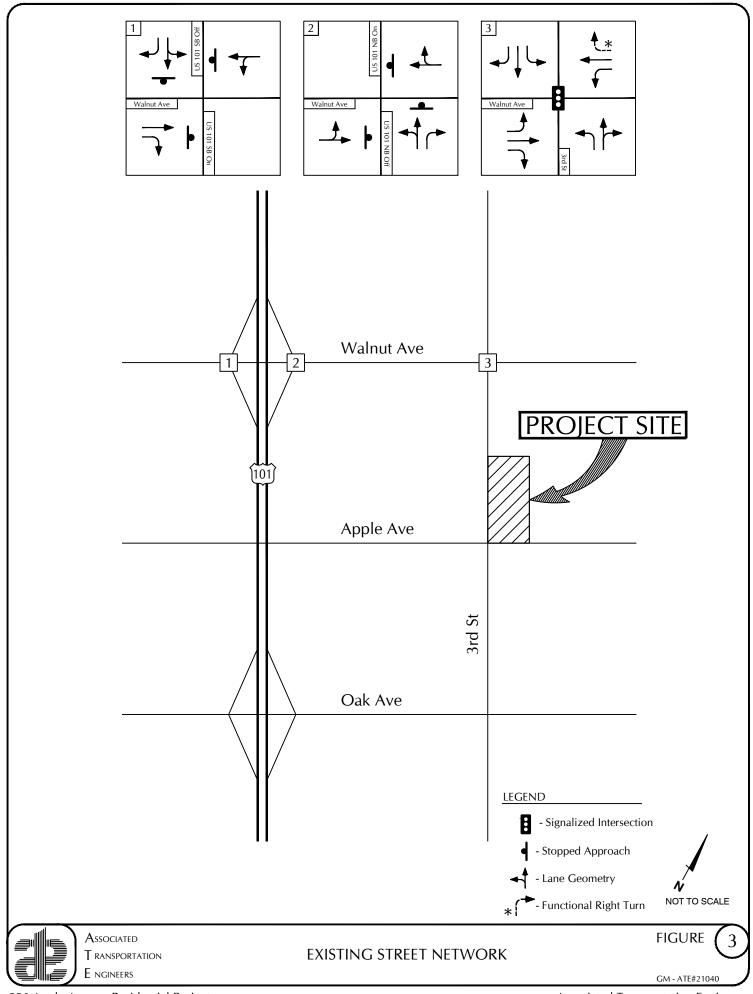






FIGURE

GM - ATE#21040



Intersection Operations

"Levels of Service" (LOS) A through F are used to rate intersection operations, with LOS A indicating very good operation and LOS F indicating poor operation (more complete definitions are contained in the Technical Appendix for reference). Because traffic flows are constrained at intersections in developed communities, detailed traffic flow analyses focus on the operating conditions of critical intersections during peak travel periods. Existing AM and PM peak hour traffic counts were collected at the key intersections identified for analysis in May 2021 (count data is contained in the Technical Appendix for reference). The AM peak hour is defined as the highest one hour of traffic flow counted between 7:00 AM and 9:00 AM, and the PM peak hour is defined as the highest one hour of traffic flow counted between 4:00 PM and 6:00 PM. Figure 4 presents the existing AM and PM peak hour traffic volumes for the study-area intersections.

Levels of service were calculated for the key intersections using the operations methodologies outlined in the Highway Capacity Manual (HCM)¹, which determines the levels of service based on the average seconds of delay per vehicle. Existing levels of service for the study-area intersections are listed in Table 1.

Table 1
Existing Levels of Service

| | | AM Peak Hour | | PM Peak Hour | |
|--------------------------------------|--------------|--------------|-------|--------------|-------|
| Intersection | Control | Delay | LOS | Delay | LOS |
| Walnut Avenue/US 101 SB Ramps | All-Way Stop | 11.2 Sec. | LOS B | 16.9 Sec. | LOS C |
| Walnut Avenue/US 101 NB Ramps | All-Way Stop | 12.2 Sec. | LOS B | 15.5 Sec. | LOS C |
| Walnut Avenue/3 rd Street | Signal | 13.4 Sec. | LOS B | 11.7 Sec. | LOS B |

LOS based on average delay per vehicle in seconds pursuant to HCM.

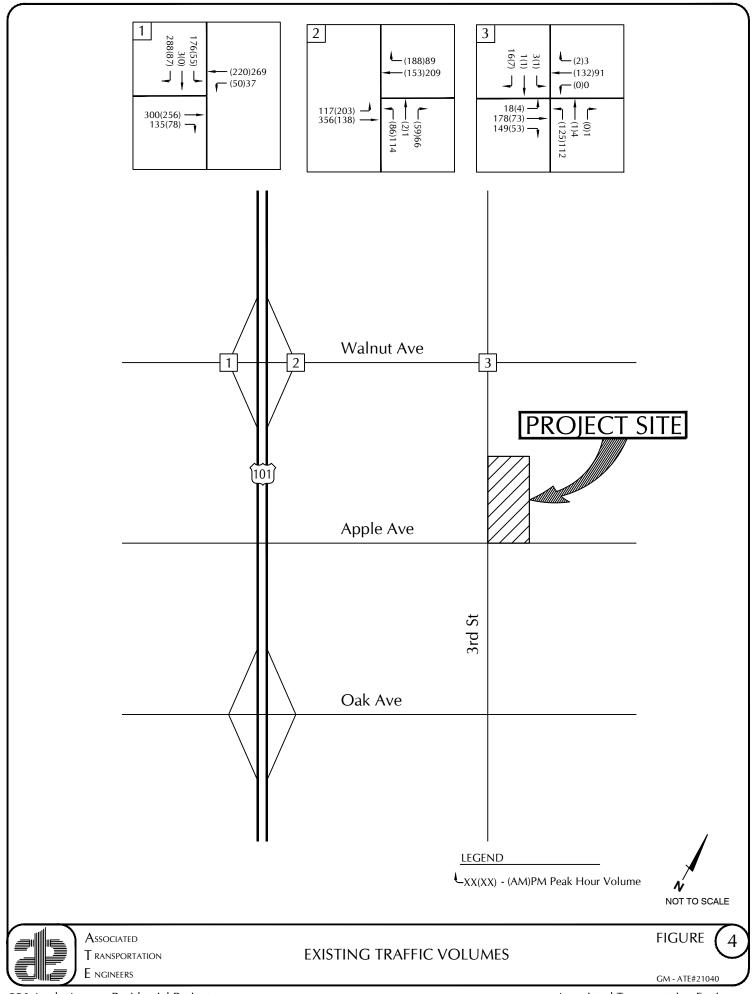
The data presented in Table 1 show that the study-area intersections currently operate at LOS B during the AM peak hour and LOS B-C during the PM peak hour – which meet the City's LOS D operating standard (see below - City of Greenfield LOS Policy).

CITY OF GREENFIELD LOS POLICY

Policy 3.2.3 of the City of Greenfield General Plan (GP) 2005-2025 Circulation Element states, "Strive to maintain Level of Service C as the minimum acceptable service standard for intersections and roadways during peak periods and accept an LOS D only when unavoidable and at identified locations."

Walnut Avenue from the US 101 interchange to 3rd Street, and 3rd Street north of Cherry Avenue, are City roadway segments classified as, "LOS D only when unavoidable and at identified locations". Note that the US 101/Walnut Avenue interchange improvements Project Study Report (PSR, Caltrans approved in February 2010) indicates that the Walnut

¹ Highway Capacity Manual, Transportation Research Board, 2016.



Avenue ramp intersection design improvements are based on peak hour LOS "D" threshold operations. The City's adopted General Plan Traffic Study (dated March 2005) designates LOS "C" and LOS "D" as the acceptable LOS threshold for practically all study roadways and intersections, respectively, under City GP Buildout conditions. Based on the above-listed policies and precedents, peak hour LOS "D" standard is generally regarded as the minimum acceptable threshold for all study intersections evaluated in this traffic study.

EXISTING + PROJECT ANALYSIS

Project Trip Generation

Trip generation estimates were calculated for the Project using rates presented in the Institute of Transportation Engineers (ITE) Trip Generation manual.² The ITE rates for Single Family Detached Housing (Land Use #210) were applied in the trip generation calculations. Table 2 shows the trip generation estimates developed for the Project (a detailed calculation worksheet is contained in the Technical Appendix for reference).

Table 2
Project Trip Generation

| | | Average Daily Trips | | | AM Peak Hour Trips | | PM Peak Hour Trips | |
|---------------------------|--------|---------------------|-------|------|-----------------------|------|-----------------------|--|
| Land Use | Size | Rate | Trips | Rate | Trips | Rate | Trips | |
| Single Family Residential | 36 DUs | 9.44 | 340 | 0.74 | 27 | 0.99 | 36 | |

As shown in Table 2, the Project is forecast to generate 340 average daily trips (ADT), with 27 trips occurring during the AM peak hour and 36 trips occurring during the PM peak hour.

Project Trip Distribution

Trip distribution percentages were developed for the Project based on existing traffic patterns in the study area, traffic studies prepared for other development projects in the area, and consideration of the land use patterns in the Greenfield area. Table 3 presents the trip distribution pattern developed for the Project. Figure 5 illustrates the trip distribution and assignment of Project traffic at the study-area intersections.

-

Trip Generation Manual, Institute of Transportation Engineers, 10th Edition, 2017.

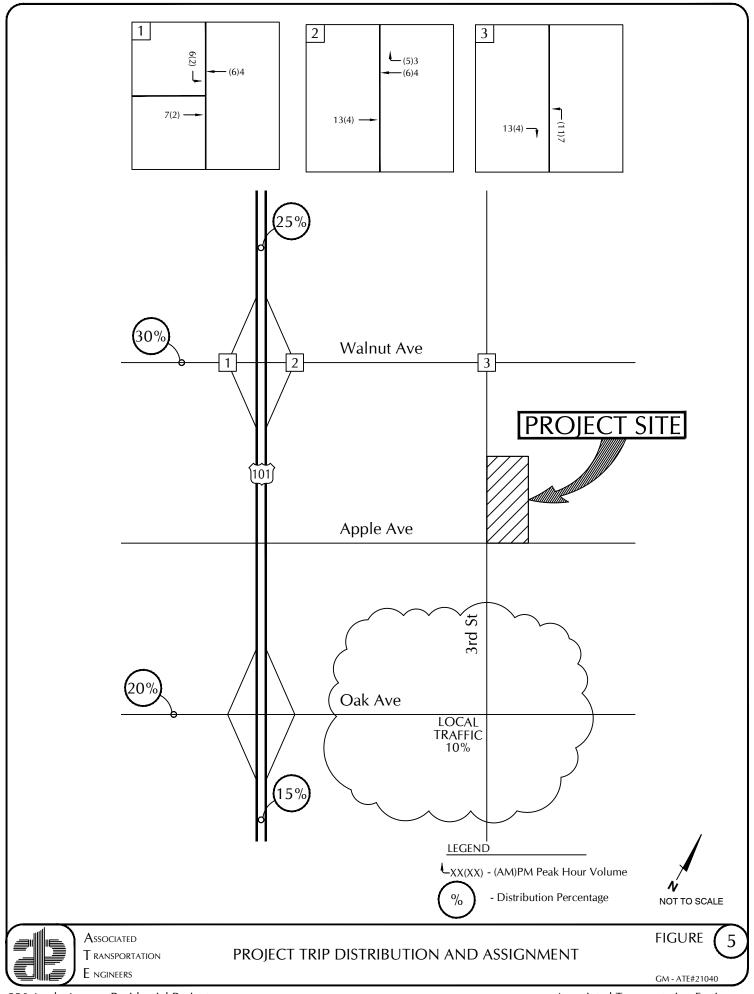


Table 3
Project Trip Distribution

| Origin/Destination | Direction | Distribution % |
|--------------------------|-----------|----------------|
| US 101 | North | 25% |
| US 101 | South | 15% |
| Walnut Ave w/o US 101 | West | 30% |
| Oak Ave w/o US 101 Route | West | 20% |
| Local Area s/o Site | South | 10% |
| Total | | 100% |

Existing + Project Intersection Operations

Levels of service were calculated for the study-area intersections assuming the Existing + Project traffic volumes shown on Figure 6. Tables 4 and 5 compare the Existing and Existing + Project levels of service and identify locations that are forecast to exceed the City's LOS D standard.

Table 4
Existing + Project Levels of Service - AM Peak Hour

| | Delay | Project Added | | |
|--------------------------------------|-----------------|-----------------------|-------|------------------------------|
| Intersection | Existing | Existing + Project | Trips | Exceed LOS D Standard? |
| Walnut Avenue/US 101 SB Ramps | 11.2 Sec./LOS B | 11.3 Sec./LOS B | 10 | No |
| Walnut Avenue/US 101 NB Ramps | 12.2 Sec./LOS B | 12.4 Sec./LOS B | 15 | No |
| Walnut Avenue/3 rd Street | 13.4 Sec./LOS B | 13.6 Sec./LOS B | 15 | No |

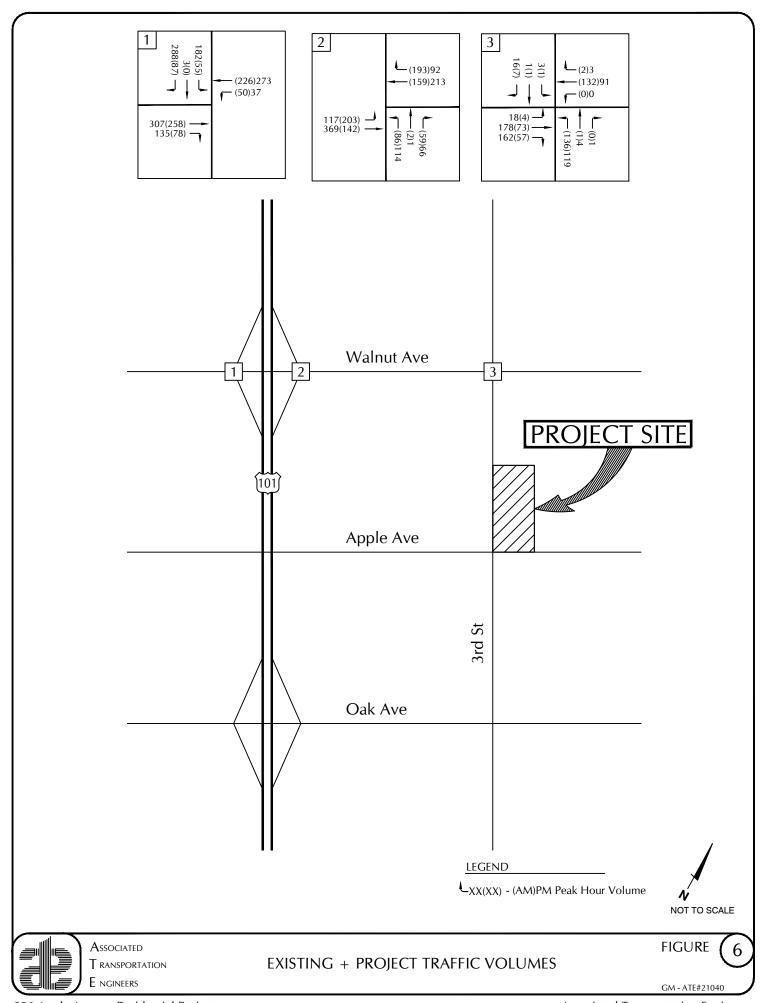
LOS based on average delay per vehicle in seconds pursuant to HCM.

Table 5
Existing + Project Levels of Service - PM Peak Hour

| | Delay / LOS | | Project Added | |
|-------------------------------|-----------------|-----------------------|---------------|------------------------------|
| Intersection | Existing | Existing + Project | Trips | Exceed LOS D Standard? |
| Walnut Avenue/US 101 SB Ramps | 16.9 Sec./LOS C | 17.4 Sec./LOS C | 17 | No |
| Walnut Avenue/US 101 NB Ramps | 15.5 Sec./LOS C | 16.1 Sec./LOS C | 20 | No |
| Walnut Avenue/3rd Street | 11.7 Sec./LOS B | 11.8 Sec./LOS B | 20 | No |

LOS based on average delay per vehicle in seconds pursuant to HCM.

As shown in Tables 4 and 5, the study-area intersections are forecast to continue to operate at LOS B during the AM peak hour and LOS B-C during the PM peak hour with Existing + Project traffic – which meet the City's LOS D operating standard. Thus, the Project would be consistent with the City's adopted level of service standards.



CUMULATIVE ANALYSIS

Traffic Forecasts

Cumulative conditions were forecast assuming traffic generated by the approved and pending development projects located in the Project study-area (see Technical Appendix for list of cumulative projects). Trip generation estimates were developed for cumulative projects using the rates presented in the ITE Trip Generation Manual (calculation worksheets contained in the Technical Appendix). The cumulative traffic was then assigned to the study-area street network based on the location of each project, existing traffic patterns observed in the study-area as well as a general knowledge of the population, employment, and commercial centers in area. Cumulative traffic forecasts are shown in Figure 7 and Cumulative + Project forecasts are shown in Figure 8.

Cumulative Intersection Operations

Tables 6 and 7 compare the Cumulative and Cumulative + Project levels of service for the study-area intersections and identify locations that are forecast to exceed the City's LOS D standard.

Table 6
Cumulative + Project Levels of Service - AM Peak Hour

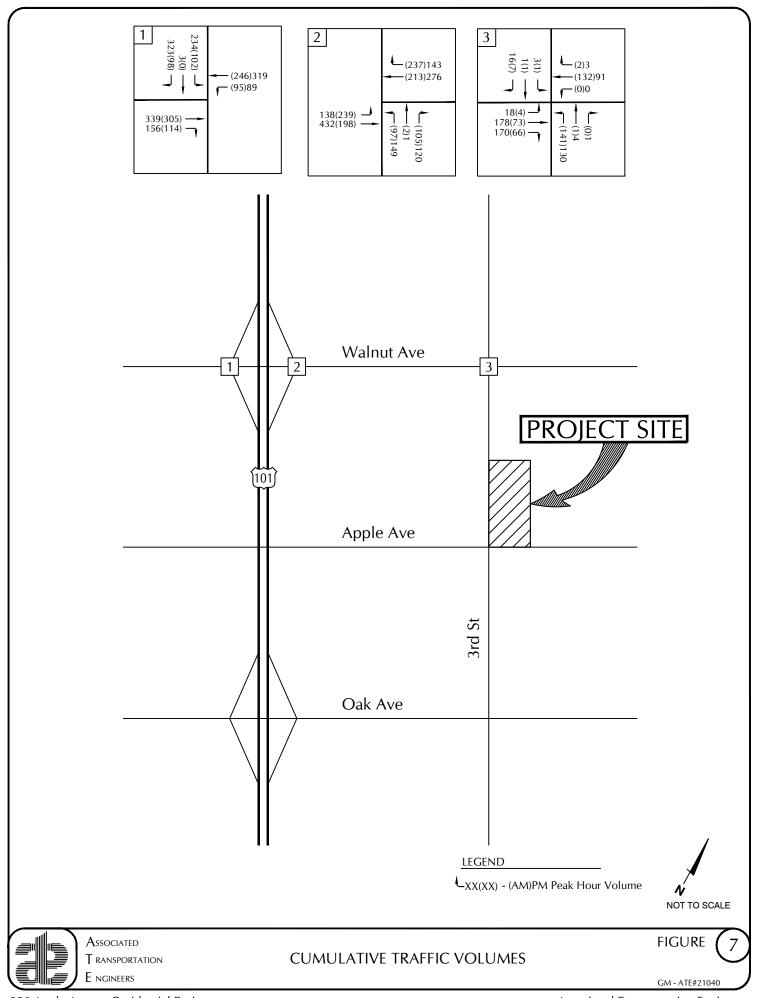
| | Delay / LOS | | Project Added | |
|--------------------------------------|-----------------|-------------------------|---------------|------------------------------|
| Intersection | Cumulative | Cumulative + Project | Trips | Exceed LOS D Standard? |
| Walnut Avenue/US 101 SB Ramps | 14.2 Sec./LOS B | 14.5 Sec./LOS B | 10 | No |
| Walnut Avenue/US 101 NB Ramps | 19.0 Sec./LOS C | 19.7 Sec./LOS C | 15 | No |
| Walnut Avenue/3 rd Street | 13.7 Sec./LOS B | 14.3 Sec./LOS B | 15 | No |

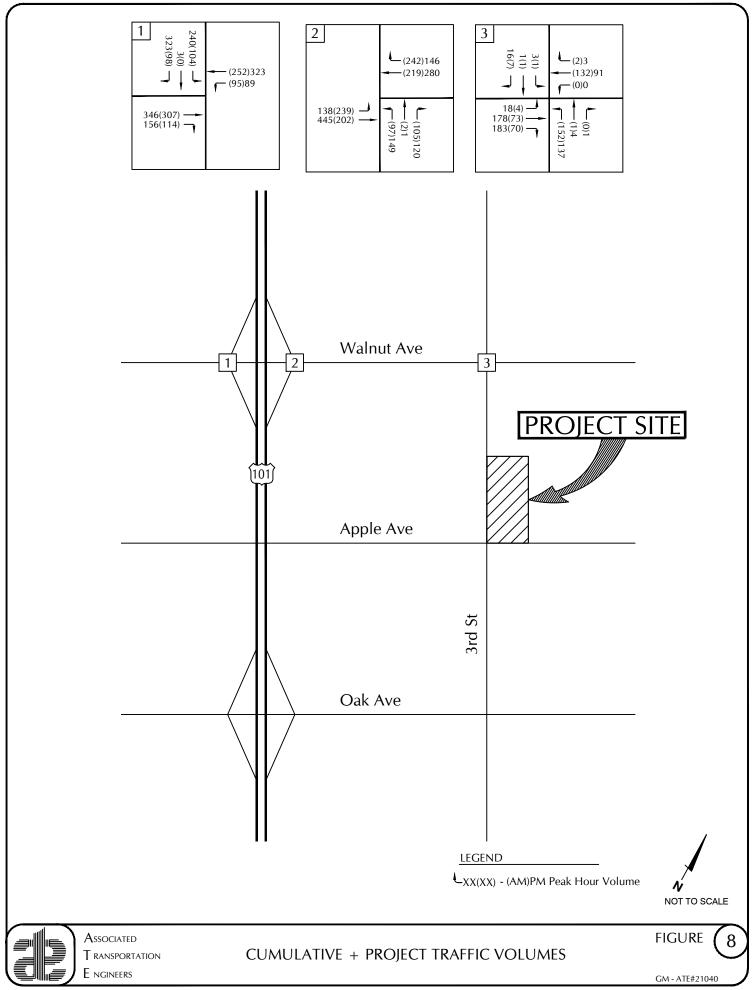
LOS based on average delay per vehicle in seconds pursuant to HCM.

Table 7
Cumulative + Project Levels of Service - PM Peak Hour

| | Delay / LOS | | Project Added | |
|--------------------------------------|-----------------|-------------------------|---------------|------------------------------|
| Intersection | Cumulative | Cumulative + Project | Trips | Exceed LOS D Standard? |
| Walnut Avenue/US 101 SB Ramps | 28.2 Sec./LOS D | 29.4 Sec./LOS D | 17 | No |
| Walnut Avenue/US 101 NB Ramps | 31.0 Sec./LOS D | 34.0 Sec./LOS D | 20 | No |
| Walnut Avenue/3 rd Street | 12.1 Sec./LOS B | 12.3 Sec./LOS B | 20 | No |

LOS based on average delay per vehicle in seconds pursuant to HCM.





As shown in Tables 6 and 7, the study-area intersections are forecast to operate at LOS D or better during the AM and PM peak hours with Cumulative and Cumulative + Project traffic, which meets the City's LOS D standard. Thus, the Project would be consistent with the City's adopted level of service standards under cumulative conditions.

SITE ACCESS

Access to the Project site is proposed via one roadway connection to 3rd Street and one roadway connection to Apple Avenue (see Figure 2 – Project Site Plan). The new roadway connection to 3rd Street would serve the Project site and connects to the existing roadway that serves the residential neighborhood immediately east of the Project site. The new roadway connection to Apple Avenue would serve the Project site as well as connect to the new east-west roadway near the northern end of the Project site, thereby providing secondary access points for the neighborhood. The new connections on 3rd Street and Apple Avenue are located on segments that are relatively flat and straight, thus providing adequate sight distances for traffic entering and exiting the Project site. Based on the Project's traffic generation (see Table 2), the Project would generate relatively low traffic volumes (less than 40 per hours during the AM & PM peak periods) and the driveways are forecast to operate at LOS A-B.

GENERAL PLAN BUILDOUT

City staff requested an analysis of General Plan Buildout traffic conditions, including full development of the Walnut Avenue Commercial Area Specific Plan, in order to determine the effects of the 296 Apple Avenue Residential Project at the US 101/Walnut Avenue interchange.

The General Plan Buildout traffic forecasts contained in the Walnut Avenue Commercial Area Specific Plan Transportation Impact Study were used for the analysis.³ The Walnut Avenue Specific Plan encompasses about 62.6 acres that is generally bounded by Cherry Avenue to the north, 3rd Street to the east, Apple Avenue to the south, and US 101 freeway to the west. The Specific Plan was developed and approved in order to change the existing zoning to allow for the Specific Plan's proposed lane uses. The City participated in the process by completing the Specific Plan and EIR documents, since the Regional Development Agency (RDA) bond funding was approved by City Council for constructing substantial offsite infrastructure improvements to Walnut Avenue and 3rd Street in order to support development of the Specific Plan.

As outlined in the planning documents, the Specific Plan area includes up to 445,000 SF of retail commercial floor space, a 130,270 SF neighborhood park, and 220 high-density residential dwelling units. The Specific Plan is anticipated to generate about 18,903 daily trips (after accounting for trip internalization/interaction between the commercial and residential portions). The Specific Plan commercial uses are forecast to attract a significant portion of trips from the US 101 mainline that are considered "diverted-linked" trips but will use the US 101/Walnut Avenue interchange. Regional freeway access to/from the SP site would be primarily obtained via the US 101/Walnut Avenue interchange and the US

-

Walnut Avenue Commercial Area Specific Plan Transportation Impact Study, Wood Rodgers, 2013.

101/Oak Avenue interchange. Local access for the Specific Plan would primarily be obtained from Walnut Avenue, 3rd Street, and Apple Avenue.

With the proposed SP project anticipated to begin and complete its first development phase by Year 2015, Year 2035 was regarded as a reasonable 20-year future planning horizon for Specific Plan traffic analysis. Table 8 lists the Year 2035 levels of service (full buildout of the City's General Plan plus full buildout of the Specific Plan) for the key intersections identified for analysis for the 296 Apple Avenue Residential Project. These level of service forecasts include the improvements that were planned under the Specific Plan (i.e., reconstruction of the US 101/Walnut Avenue interchange with signalized intersections and additional lanes at the Walnut Avenue/3rd Street intersection).

Table 8
Year 2035 Levels of Service

| | | AM Pea | k Hour | PM Pea | k Hour |
|--------------------------------------|---------|-----------|--------|-----------|--------|
| Intersection | Control | Delay | LOS | Delay | LOS |
| Walnut Avenue/US 101 SB Ramps | Signal | 16.8 Sec. | LOS B | 22.5 Sec. | LOS C |
| Walnut Avenue/US 101 NB Ramps | Signal | 12.3 Sec. | LOS B | 16.5 Sec. | LOS B |
| Walnut Avenue/3 rd Street | Signal | 43.0 Sec. | LOS D | 44.8 Sec. | LOS D |

Source: Walnut Avenue Commercial Area Specific Plan Transportation Impact Study. Levels of service assumed planned improvements.

As shown in Table 8, the US 101/Walnut Avenue interchange is forecast to operate at LOS B during the AM peak hour and LOS B-C during the PM peak hour with Year 2035 traffic. The Walnut Avenue/3rd street intersection is forecast to operate at LOS D during the AM and PM peak hours with Year 2035 traffic. These levels of service meet the City's LOS D operating standard.

Since the 2035 forecasts include full buildout of the City's General Plan, they include development of the 296 Apple Avenue Residential Project site. The preceding analyses found that the 296 Apple Avenue Residential Project would have a minor effect on vehicle delays and would not change the levels of service at the US 101/Walnut Avenue interchange (see Tables 4 and 5).

As noted in the traffic study prepared for the Walnut Avenue Commercial Area Specific Plan, Caltrans was supportive of the all-way stop controls at the tightly-spaced ramp terminal intersections which are now in place at the US 101/Walnut Avenue interchange – and currently operate at LOS B during the AM peak period and LOS B-C during the PM peak period. The Walnut Avenue Commercial Area Specific Plan also forecast that the interim improvements would adequately accommodate traffic demands for at least 10 years from the start of the land developments within the Specific Plan based on proposed phasing of the Specific Plan. At this time, very little development has occurred within the Specific Plan area.

Ultimately, the interchange will need to be upgraded with the preferred alternative configuration developed through the Caltrans planning processes. Caltrans previously prepared a Project Study Report (PSR) and Project Report (PR) for the interchange improvements. The Caltrans planning studies found that a 5-lane overcrossing with traffic signal controlling the on- and off-ramp intersections would be required to accommodate the future traffic generated by full buildout of the City's General Plan plus full buildout of the Specific Plan. The City's 20-year Traffic Improvement Fee Program (TIFP) includes the costs of the future interchange improvements, which the 296 Apple Avenue Residential Project would be required to contribute to in order to offset its incremental impact to the interchange.

VEHICLE MILES TRAVELED ANALYSIS

Recent legislation, Senate Bill 743, is moving away from the Level of Service (LOS) metric to a Vehicle Miles Travelled (VMT) metric to evaluate whether a project results in a significant traffic impact. Cities and counties were required to implement Senate Bill 743 by July 1, 2020. It is anticipated that LOS will still remain as a policy consistency issue for the City, though not as an impact metric under CEQA environmental review.

Per the State's Natural Resource Agency Updated Guidelines for the Implementation of the CEQA adopted in 2018, VMT has been designated as the most appropriate measure of transportation impacts. VMT refers to the amount and distance of automobile travel attributable to a project. Other relevant considerations may include the effects of the project on transit and non-motorized travel. For land use projects, vehicle miles traveled exceeding an applicable threshold of significance may indicate a significant impact. The City has not yet adopted VMT thresholds of significance.

<u>CEQA Guidelines.</u> The California Governor's Office of Planning and Research (OPR) published a Technical Advisory on Transportation that includes recommendations regarding assessment of VMT, thresholds of significance, and mitigation measures. The Technical Advisory provides screening tools to determine when a project may have a significant VMT impact, as follows:

"Many agencies use "screening thresholds" to quickly identify when a project should be expected to cause a less-than-significant impact without conducting a detailed study. (See e.g., CEQA Guidelines, §§ 15063(c)(3)(C), 15128, and Appendix G.) As explained below, this technical advisory suggests that lead agencies may screen out VMT impacts using project size, maps, transit availability, and provision of affordable housing.

Presumption of Less Than Significant Impact for Affordable Residential Development

Adding affordable housing to infill locations generally improves jobs-housing match, in turn shortening commutes and reducing VMT. Further, "... low-wage workers in particular would be more likely to choose a residential location close to their workplace, if one is available." In areas where existing jobs-housing match is closer to optimal, low income housing nevertheless generates less VMT than market-rate

housing. Therefore, a project consisting of a high percentage of affordable housing may be a basis for the lead agency to find a less-than-significant impact on VMT. Evidence supports a presumption of less than significant impact for a 100% affordable residential development (or the residential component of a mixed-use development) in infill locations. Lead agencies may develop their own presumption of less than significant impact for residential projects (or residential portions of mixed-use projects) containing a particular amount of affordable housing, based on local circumstances and evidence. Furthermore, a project which includes any affordable residential units may factor the effect of the affordability on VMT into the assessment of VMT generated by those units."

The OPR Technical Advisory states that affordable housing generates lower VMT than market rate housing. Affordable housing units are homes that are set aside for very low income and low income households. Providing affordable housing in infill areas can shorten commutes by providing housing closer to where people work, thereby reducing the amount of travel in the area. Thus, OPR presumes that affordable housing units have a less than significant impact on VMT, absent substantial evidence to the contrary, and do not require further VMT analysis. The City may apply screening to projects containing all (100 percent) affordable housing units. If a project contains affordable housing along with other land uses, the non-affordable housing uses need to meet at least one of the other screening criteria presented in this chapter to avoid further VMT analysis.

All of the Project's residential units would be affordable. Thus, the Project would be eligible for a finding of less than significant based on the adopted State thresholds.

RECOMMENDED IMPROVEMENTS

The traffic analysis found that the study-area intersections are forecast to operate in the LOS B-C range with Existing + Project and Cumulative + Project traffic. Thus, improvements to the study-area street network are not required since the forecasts meet the City's LOS D standard. The 296 Apple Avenue Residential Project would be required to contribute to the City's Traffic Improvement Fee Program (TIFP) to offset its incremental impact to the City's street network, including the improvements programmed for the US 101/Walnut Avenue interchange.

• • •

REFERENCES AND PERSONS CONTACTED

Associated Transportation Engineers

Richard L. Pool, PE, President Scott A. Schell, Principal Transportation Planner Dan Dawson, Supervising Transportation Planner Glenn Manaois, Traffic Engineer I

References

Highway Capacity Manual, Transportation Research Board, 2016.

Highway Design Manual, Chapter 400, California Department of Transportation, Updated July 2020.

Trip Generation, Institute of Transportation Engineers, 10th Edition, 2017.

Walnut Avenue Commercial Area Specific Plan Transportation Impact Study, Wood Rodgers, 2013.

Persons Contacted

Doug Pike, City of Greenfield (Contract Engineer) Sheryl Flores, People's Self Help Housing Shaveta Sharma, People's Self Help Housing

TECHNICAL APPENDIX

CONTENTS:

LEVEL OF SERVICE DEFINITIONS

INTERSECTION TURNING MOVEMENTS COUNTS

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INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEETS

Reference 1 - US 101 SB Ramps/Walnut Avenue Reference 2 - US 101 NB Ramps/Walnut Avenue

Reference 3 - 3rd Street/Walnut Avenue

LEVEL OF SERVICE DEFINITIONS

Signalized Intersection Level of Service Definitions

| LOS | Delay ^a | V/C Ratio | Definition |
|-----|--------------------|-------------|--|
| А | < 10.0 | < 0.60 | Progression is extremely favorable. Most vehicles arrive during the green phase. Many vehicles do not stop at all. |
| В | 10.1 - 20.0 | 0.61 - 0.70 | Good progression, short cycle lengths, or both. More vehicles stop than with LOS A, causing higher levels of delay. |
| С | 20.1 - 35.0 | 0.71 - 0.80 | Only fair progression, longer cycle lengths, or both, result in higher cycle lengths. Cycle lengths may fail to serve queued vehicles, and overflow occurs. Number of vehicles stopped is significant, though many still pass through intersection without stopping. |
| D | 35.1 - 55.0 | 0.81 - 0.90 | Congestion becomes more noticeable. Unfavorable progression, long cycle lengths and high v/c ratios result in longer delays. Many vehicles stop, and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable. |
| E | 55.1 - 80.0 | 0.91 - 1.00 | High delay values indicate poor progression, long cycle lengths and high v/c ratios. Individual cycle failures are frequent |
| F | > 80.0 | > 1.00 | Considered unacceptable for most drivers, this level occurs when arrival flow rates exceed the capacity of lane groups, resulting in many individual cycle failures. Poor progression and long cycle lengths may also contribute to high delay levels. |

^a Average control delay per vehicle in seconds.

Unsignalized Intersection Level of Service Definitions

The HCM¹ uses control delay to determine the level of service at unsignalized intersections. Control delay is the difference between the travel time actually experienced at the control device and the travel time that would occur in the absence of the traffic control device. Control delay includes deceleration from free flow speed, queue move-up time, stopped delay and acceleration back to free flow speed.

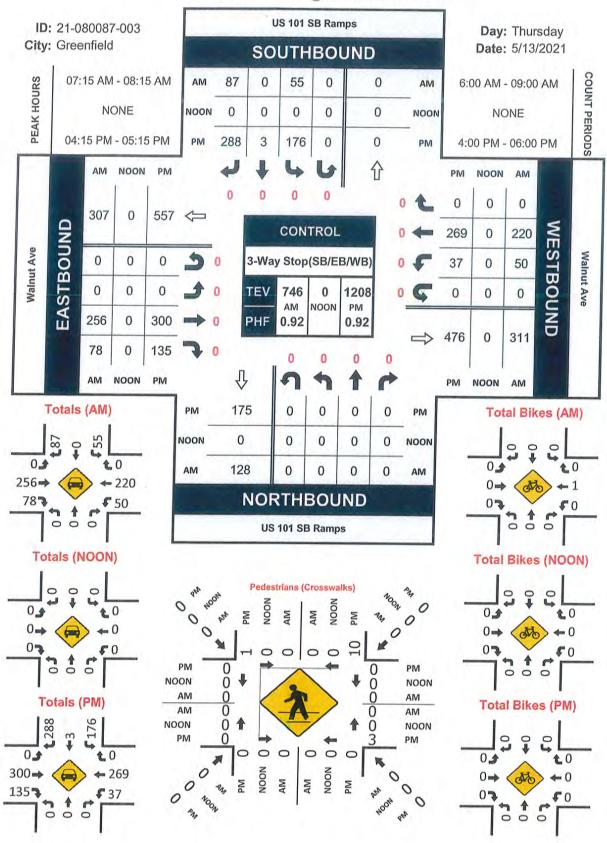
| LOS | Control Delay Seconds per Vehicle |
|-----|--------------------------------------|
| А | < 10.0 |
| В | 10.1 - 15.0 |
| С | 15.1 - 25.0 |
| D | 25.1 - 35.0 |
| E | 35.1 - 50.0 |
| F | > 50.0 |

¹ Highway Capacity Manual, National Research Board, 2000



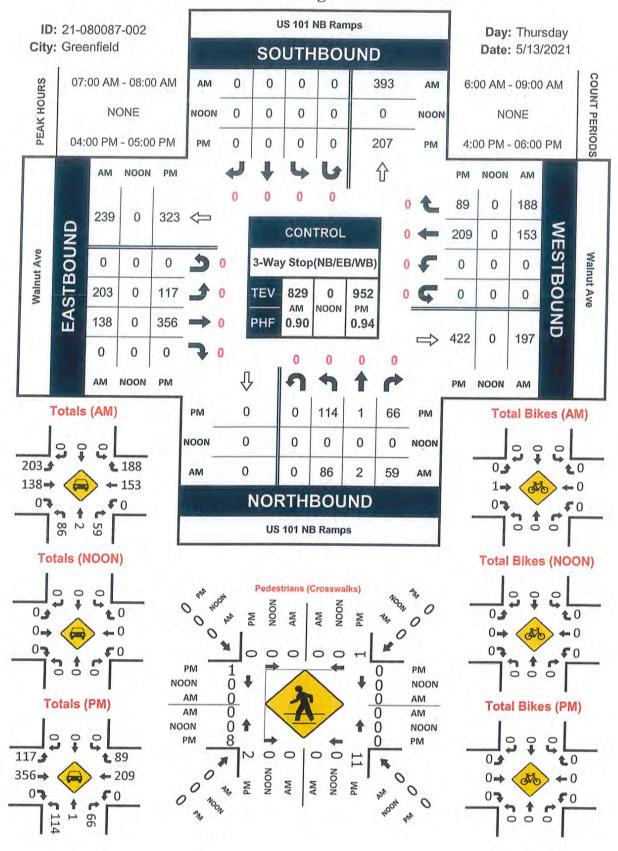
US 101 SB Ramps & Walnut Ave

Peak Hour Turning Movement Count



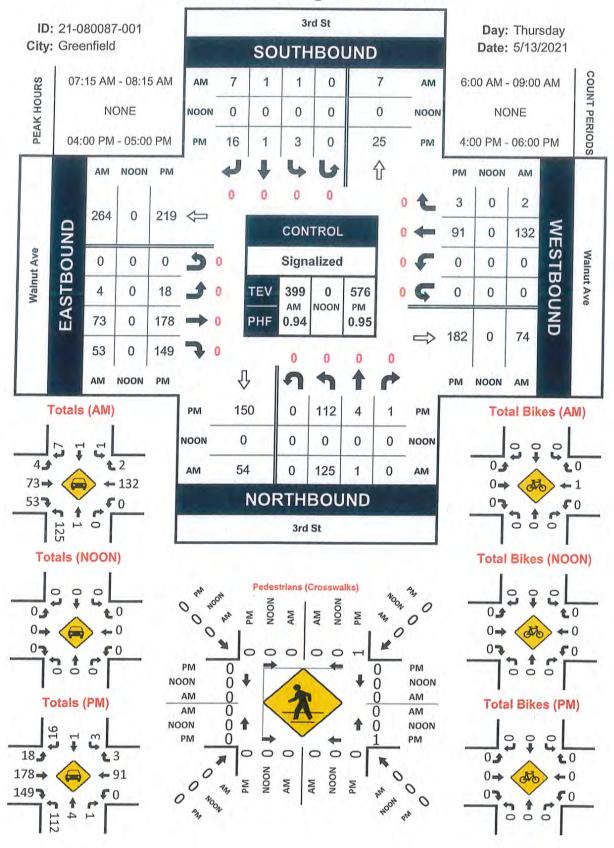
US 101 NB Ramps & Walnut Ave

Peak Hour Turning Movement Count



3rd St & Walnut Ave

Peak Hour Turning Movement Count



CUMULATIVE PROJECT INFORMATION

City of Germania

| | The way of the same | | | 5/1/2019 - | 5/1/2021 | | |
|---|--------------------------------------|---|---------------------------|-------------------------------|--|-------------------|--|
| | | වැණුලේ මණ්ඩල්ගේ පිරිබල්ලේගේ | A CVIDICATION | Aprilian Manti | ii Projecin | 769 Pancel # | |
| | 3/22/2021 | CUP - MD FARMS | Applicant | ALLEN@MDI RMS.ORG | The state of the s | | (1981) Section 1995 (1995) |
| | | Sign Permit for Cricket Wireless | | ruben66@ya oo.com | | 02410302200 | 23 S EL CAMINO REAL |
| | | Pre-Application for 37 SFH | | | | 10908201300 | 0 296 APPLE AVE |
| | 1 1 | Greenfield LLC | | robert@e7ca om | .c Commercial Cannabis | 10913100900 | 0 40597 CHERRY AVE |
| | | Center | Applicant | pali2200@yal oo.com | station | | Walnut Ave- The Vines |
| , | P A | Regulatory Permit | | | Commercial Cannabis | 109144007000 | 710 EL CAMINO REAL |
| | P | partments hase I | | | New multi- family project | 109171005000 | 1064 WALNUT AVE |
| | 8: | CCOUNT- (18 Tyler | Property Owner | eric@ldtrealty. com | Mullt-family residential | 024281037000 | 818 Tyler Avenue |
| | SF lot | eview/New FD on vacant t zoned R-L | pplicant | coatsconsultin g@gmail.com | Five lot SFR subdivions | 024072005000 0 | OAK TERRACE- LOT 2 |
| | Но | rmworker pusing | | Mike@avilacon st.com | Mullt-family residential | 109113003000 | 525 3rd st |
| - | 2/26/2020 Mii Su 7/12/2019 De | bdivision O | operty wner chitect | deborahrich@c omcast.net | | 109232001000 | ST |
| | Mix Pro Cor esio | ked Use oject mmercial/R dential | | david@djelliott inet | | 024151011000 | 4th & Palm Avenue |
| | 143 fam resio proj II ar | rtments- unit multi- | plicant | ilingo@icdema il.com | residential | | AVE |
| | - 1 | | | | | | A STATE OF THE PARTY OF THE PAR |

5/21/2021

Associated Transportation Engineers Approved Projects - Trip Generation Worksheet

| | SKYWAY BUSINESS PARK PROJECT - APPROVED PROJECT LIST (#21041) | SUSINES | S PARK F | ROJEC | T - APP | ROVED | PROJE | CT LIS | T (#21 | 141) | | | | | |
|----|---|---------|----------|-------|---------|------------------------------|----------|--------|--------|-------|----------|----------|--------|-----------------------------------|--------|
| | Project # / Land Use | ة | | | | AM Peak | eak | | | | | 7.80 | 1 | | |
| | | ñ | azic | | | | | | | | | LIN LEGK | eak | | |
| | | | | кате | lrips | Irips In % Trips Out % Trips | Trips | ort % | Trips | Rate | Trips | % u | Tring | Rate Trips In % Trips Out % Trips | Tribo |
| 2 | CIONI DEDINATION | | | | | | | | 1 | | | | 2 | 200 | 201 |
| 1 | GOIN PERINI (a) | Ϋ́ | Ϋ́ | ¥Z | Ϋ́ | ΔX | ΑN | ΔN | Ø Z | ź | < 7 | 4 | 4 | | |
| œ. | 298 Apple Avenue/h) | 5 | 1 | | | | 1 | 5 | ζ. | Ş | <u>ر</u> | Z Z | Z Z | Z Z | √ Z |
| | (a) price a cidal (b) | 3/ | SFDO | 0.74 | 27 | 25% | 7 | 75% | 20 | 000 | 27 | 650/ | ç | 240, | ; |
| Ω | Walnut Travel Center(c) | 0000 | Ľ | 177 | 100 | | | | 3 | 3 | 5 | 200 | 2 | 3/% | 4 |
| | ייייי מייייי (ס'יייייי) | 2,000 | ر ا | 75.99 | 228 | 21% | 116 | 12% | 112 | 88.35 | 265 | 51% | 135 | 100/ | 120 |
| , | Walnut Grove Apartments - Phase I(d) | 143 | MEDI | 0.16 | 22 | 7000 | 7. | 1022 | | | | 2 | 2 | 45.70 | 200 |
| o | Oak \/(io), Tomoso Lot 0/1 | 2 | | 2 | 3 | 43.70 | 0 | 17% | 51 | 0.56 | 80 | 63% | 20 | 37% | 30 |
| , | Can view Tellage LOLZ(D) | Ω | SFDU | 0.74 | 4 | 25% | · | 75% | ٣ | 000 | ц | /000 | c | 2010 | 3 |
| 9 | 525 3rd Street - Farm Workers Housing/a) | 7.70 | r. | 9 | . | | - | 200 | 7 | 0.00 | 0 | 02% | 7) | 3/% | ~ |
| | יום מוסיב ו מיוון איסואכים ויטמפוווא(ח) | 7 | MFDO | 0.46 | 25 | 23% | 7 | %// | 40 | 0.56 | 23 | 7029 | Ç | 270/ | 6 |
| 2 | Walnut Grove Apartments - Phase II & III/2\ | 1/13 | - 1011 | QV C | ۲ | 200 | | | 2 | 3 | 3 | 200 | 40 | 01.70 | 23 |
| | | 2 | מו | 0.40 | 00 | 23% | <u>၂</u> | %// | 2 | 0.56 | 80 | 63% | 50 | 37% | 30 |
| | | | | | | | | | | | | | 2 | | 5 |

(a) Application for new signs. No new day-to-day traffic.

(b) Trip generation rate derived from ITE Trip Generation Manual - Single-Family Detached Housing (ITE #210).
 (c) Trip generation rate derived from ITE Trip Generation Manual - Gasoline/Service Statioin with Convenience Market (ITE #945).

(d) Trip generation rate derived from ITE Trip Generation Manual - Multifamily Housing (Low Rise) (ITE #220).

INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEETS

Reference 1 - US 101 SB Ramps/Walnut Avenue Reference 2 - US 101 NB Ramps/Walnut Avenue Reference 3 - 3rd Street/Walnut Avenue

| Intersection | Addison | |
|---------------------------|---------|--|
| Intersection Delay, s/veh | 11.2 | |
| Intersection LOS | В | |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|--------------------------|------|----------|------|------|------|------|------|------|------|------|-------|------|
| Lane Configurations | | ↑ | 7" | | 4 | | | | | | र्स | 71 |
| Traffic Vol, veh/h | 0 | 256 | 78 | 50 | 220 | 0 | 0 | 0 | 0 | 55 | 0 | 87 |
| Future Vol, veh/h | 0 | 256 | 78 | 50 | 220 | 0 | 0 | 0 | 0 | 55 | 0 | 87 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 278 | 85 | 54 | 239 | 0 | 0 | 0 | 0 | 60 | 0 | 95 |
| Number of Lanes | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 1 |
| Approach | | EB | | WB | | | | | | SB | | |
| Opposing Approach | | WB | | EB | | | | | | | | |
| Opposing Lanes | | 1 | | 2 | | | | | | 0 | | |
| Conflicting Approach Lef | t | SB | | | | | | | | WB | | |
| Conflicting Lanes Left | | 2 | | 0 | | | | | | 1 | | |
| Conflicting Approach Rig | ht | | | SB | | | | | | EB | | |
| Conflicting Lanes Right | | 0 | | 2 | | | | | | 2 | | |
| HCM Control Delay | | 10.7 | | 12.7 | | | | | | 9.5 | | |
| HCM LOS | | В | | В | | | | | 1137 | Α | 11000 | |

| Lane | EBLn1 | EBLn2 | WBLn1 | SBLn1 | SBLn2 |
|------------------------|-------|-------|-------|-------|-------|
| Vol Left, % | 0% | 0% | 19% | 100% | 0% |
| Vol Thru, % | 100% | 0% | 81% | 0% | 0% |
| Vol Right, % | 0% | 100% | 0% | 0% | 100% |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 256 | 78 | 270 | 55 | 87 |
| LT Vol | 0 | 0 | 50 | 55 | 0 |
| Through Vol | 256 | 0 | 220 | 0 | 0 |
| RT Vol | 0 | 78 | 0 | 0 | 87 |
| Lane Flow Rate | 278 | 85 | 293 | 60 | 95 |
| Geometry Grp | 7 | 7 | 6 | 7 | 7 |
| Degree of Util (X) | 0.406 | 0.107 | 0.441 | 0.11 | 0.142 |
| Departure Headway (Hd) | 5.256 | 4.55 | 5.409 | 6.612 | 5.398 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 682 | 782 | 663 | 539 | 658 |
| Service Time | 3.021 | 2.315 | 3.476 | 4.395 | 3.181 |
| HCM Lane V/C Ratio | 0.408 | 0.109 | 0.442 | 0.111 | 0.144 |
| HCM Control Delay | 11.6 | 7.9 | 12.7 | 10.2 | 9.1 |
| HCM Lane LOS | В | Α | В | В | Α |
| HCM 95th-tile Q | 2 | 0.4 | 2.3 | 0.4 | 0.5 |

| Intersection | |
|---------------------------|------|
| Intersection Delay, s/veh | 11.3 |
| Intersection LOS | В |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|---------------------------|------|----------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ^ | 7" | | 4 | | | | | | 4 | 71 |
| Traffic Vol, veh/h | 0 | 258 | 78 | 50 | 226 | 0 | 0 | 0 | 0 | 57 | 0 | 87 |
| Future Vol, veh/h | 0 | 258 | 78 | 50 | 226 | 0 | 0 | 0 | 0 | 57 | 0 | 87 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 280 | 85 | 54 | 246 | 0 | 0 | 0 | 0 | 62 | 0 | 95 |
| Number of Lanes | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 1 |
| Approach | | EB | | WB | | | | | | SB | | |
| Opposing Approach | | WB | | EB | | | | | | | | |
| Opposing Lanes | | 1 | | 2 | | | | | | 0 | | 100 |
| Conflicting Approach Left | | SB | | | | | | | | WB | | |
| Conflicting Lanes Left | | 2 | | 0 | | | | | | 1 | 9 | |
| Conflicting Approach Righ | nt | | | SB | | | | | | EB | | |
| Conflicting Lanes Right | | 0 | | 2 | | | | | | 2 | | |
| HCM Control Delay | | 10.8 | | 12.9 | | | | | | 9.6 | | |
| HCM LOS | | В | | В | | | | | | Α | | |

| Lane | EBLn1 | EBLn2\ | WBLn1 | SBLn1 | SBLn2 |
|------------------------|-------|--------|-------|-------|-------|
| Vol Left, % | 0% | 0% | 18% | 100% | 0% |
| Vol Thru, % | 100% | 0% | 82% | 0% | 0% |
| Vol Right, % | 0% | 100% | 0% | 0% | 100% |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 258 | 78 | 276 | 57 | 87 |
| LT Vol | 0 | 0 | 50 | 57 | 0 |
| Through Vol | 258 | 0 | 226 | 0 | 0 |
| RT Vol | 0 | 78 | 0 | 0 | 87 |
| Lane Flow Rate | 280 | 85 | 300 | 62 | 95 |
| Geometry Grp | 7 | 7 | 6 | 7 | 7 |
| Degree of Util (X) | 0.411 | 0.108 | 0.452 | 0.114 | 0.142 |
| Departure Headway (Hd) | 5.273 | 4.568 | 5.421 | 6.633 | 5.419 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Сар | 679 | 778 | 660 | 537 | 655 |
| Service Time | 3.037 | 2.331 | 3.488 | 4.422 | 3.207 |
| HCM Lane V/C Ratio | 0.412 | 0.109 | 0.455 | 0.115 | 0.145 |
| HCM Control Delay | 11.7 | 7.9 | 12.9 | 10.3 | 9.1 |
| HCM Lane LOS | В | Α | В | В | A |
| HCM 95th-tile Q | 2 | 0.4 | 2.4 | 0.4 | 0.5 |

| ntersection | | |
|---------------------------|-----|--|
| Intersection Delay, s/veh | 4.2 | |
| Intersection LOS | В | |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|---------------------------|------|----------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↑ | 7" | | 4 | | | | | | ্ৰ | 7 |
| Traffic Vol, veh/h | 0 | 305 | 114 | 95 | 246 | 0 | 0 | 0 | 0 | 102 | 0 | 98 |
| Future Vol, veh/h | 0 | 305 | 114 | 95 | 246 | 0 | 0 | 0 | 0 | 102 | 0 | 98 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 332 | 124 | 103 | 267 | 0 | 0 | 0 | 0 | 111 | 0 | 107 |
| Number of Lanes | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 1 |
| Approach | | EB | | WB | | | | | | SB | | |
| Opposing Approach | | WB | | EB | | | | | | | | |
| Opposing Lanes | | 1 | | 2 | | | | | | 0 | | |
| Conflicting Approach Left | t | SB | | | | | | | | WB | | |
| Conflicting Lanes Left | | 2 | | 0 | | | | | | 1 | | |
| Conflicting Approach Rig | ht | | | SB | | | | | | EB | | |
| Conflicting Lanes Right | | 0 | | 2 | | | | | | 2 | | |
| HCM Control Delay | | 13 | | 17.5 | | | | | | 10.9 | | |
| HCM LOS | | В | | C | | | | | | В | | |

| Lane | EBLn1 | EBLn2\ | NBLn1 | SBLn1 | SBLn2 | |
|------------------------|-------|--------|-------|-------|-------|--|
| Vol Left, % | 0% | 0% | 28% | 100% | 0% | |
| Vol Thru, % | 100% | 0% | 72% | 0% | 0% | |
| Vol Right, % | 0% | 100% | 0% | 0% | 100% | |
| Sign Control | Stop | Stop | Stop | Stop | Stop | |
| Traffic Vol by Lane | 305 | 114 | 341 | 102 | 98 | |
| LT Vol | 0 | 0 | 95 | 102 | 0 | |
| Through Vol | 305 | 0 | 246 | 0 | 0 | |
| RT Vol | 0 | 114 | 0 | 0 | 98 | |
| Lane Flow Rate | 332 | 124 | 371 | 111 | 107 | |
| Geometry Grp | 7 | 7 | 6 | 7 | 7 | |
| Degree of Util (X) | 0.526 | 0.172 | 0.604 | 0.22 | 0.176 | |
| Departure Headway (Hd) | 5.711 | 5.003 | 5.87 | 7.159 | 5.939 | |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | |
| Сар | 633 | 717 | 615 | 502 | 604 | |
| Service Time | 3.439 | 2.73 | 3.897 | 4.897 | 3.676 | |
| HCM Lane V/C Ratio | 0.524 | 0.173 | 0.603 | 0.221 | 0.177 | |
| HCM Control Delay | 14.6 | 8.8 | 17.5 | 11.9 | 9.9 | |
| HCM Lane LOS | В | А | C | В | A | |
| HCM 95th-tile Q | 3.1 | 0.6 | 4 | 8.0 | 0.6 | |

| Intersection Delay, s/veh 14.5 | |
|--------------------------------|--|
| Interconting LOO | |
| Intersection LOS B | |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|---------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 1 | 7" | | 4 | | | | | | 4 | 7 |
| Traffic Vol, veh/h | 0 | 307 | 114 | 95 | 252 | 0 | 0 | 0 | 0 | 104 | 0 | 98 |
| Future Vol, veh/h | 0 | 307 | 114 | 95 | 252 | 0 | 0 | 0 | 0 | 104 | 0 | 98 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 334 | 124 | 103 | 274 | 0 | 0 | 0 | 0 | 113 | 0 | 107 |
| Number of Lanes | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 1 |
| Approach | | EB | | WB | | | | | | SB | | |
| Opposing Approach | | WB | | EB | | | | | | | | |
| Opposing Lanes | | 1 | | 2 | | | | | | 0 | | 100 |
| Conflicting Approach Left | | SB | | | | | | | | WB | | |
| Conflicting Lanes Left | 1 | 2 | | 0 | | | | | | 1 | | |
| Conflicting Approach Righ | nt | | | SB | | | | | | EB | | |
| Conflicting Lanes Right | | 0 | | 2 | | | | | | 2 | | |
| HCM Control Delay | | 13.2 | | 18 | | | | | | 11 | | |
| HCM LOS | | В | | C | | | | | | В | | |

| Lane | EBLn1 | EBLn2V | VBLn1 | SBLn1 | SBLn2 |
|------------------------|-------|--------|-------|-------|-------|
| Vol Left, % | 0% | 0% | 27% | 100% | 0% |
| Vol Thru, % | 100% | 0% | 73% | 0% | 0% |
| Vol Right, % | 0% | 100% | 0% | 0% | 100% |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 307 | 114 | 347 | 104 | 98 |
| LT Vol | 0 | 0 | 95 | 104 | 0 |
| Through Vol | 307 | 0 | 252 | 0 | 0 |
| RT Vol | 0 | 114 | 0 | 0 | 98 |
| Lane Flow Rate | 334 | 124 | 377 | 113 | 107 |
| Geometry Grp | 7 | 7 | 6 | 7 | 7 |
| Degree of Util (X) | 0.531 | 0.173 | 0.617 | 0.226 | 0.177 |
| Departure Headway (Hd) | 5.733 | 5.024 | 5.885 | 7.186 | 5.965 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 631 | 714 | 615 | 500 | 601 |
| Service Time | 3.464 | 2.754 | 3.914 | 4.924 | 3.703 |
| HCM Lane V/C Ratio | 0.529 | 0.174 | 0.613 | 0.226 | 0.178 |
| HCM Control Delay | 14.8 | 8.8 | 18 | 12 | 10 |
| HCM Lane LOS | В | A | C | В | A |
| HCM 95th-tile Q | 3.1 | 0.6 | 4.2 | 0.9 | 0.6 |

| Intersection | | |
|---------------------------|------|--|
| Intersection Delay, s/veh | 16.9 | |
| Intersection LOS | C | |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|---------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 1 | 7" | | 4 | | | | | | 4 | 7 |
| Traffic Vol, veh/h | 0 | 300 | 135 | 37 | 269 | 0 | 0 | 0 | 0 | 176 | 3 | 288 |
| Future Vol, veh/h | 0 | 300 | 135 | 37 | 269 | 0 | 0 | 0 | 0 | 176 | 3 | 288 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 326 | 147 | 40 | 292 | 0 | 0 | 0 | 0 | 191 | 3 | 313 |
| Number of Lanes | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 1 |
| Approach | | EB | | WB | | | | | | SB | 200 | 233 |
| Opposing Approach | | WB | | EB | | | | | | | | |
| Opposing Lanes | | 1 | | 2 | | | | | | 0 | | |
| Conflicting Approach Left | | SB | | | | | | | | WB | | |
| Conflicting Lanes Left | | 2 | | 0 | | | | | | 1 | | |
| Conflicting Approach Righ | nt | | | SB | | | | | | EB | | |
| Conflicting Lanes Right | | 0 | | 2 | | | | | | 2 | | |
| HCM Control Delay | | 16.3 | | 20.2 | | | | | | 15.3 | | |
| HCM LOS | | C | | C | | | | | | C | | |

| Lane | EBLn1 | EBLn2V | WBLn1 | SBLn1 | SBLn2 |
|------------------------|-------|--------|-------|-------|-------|
| Vol Left, % | 0% | 0% | 12% | 98% | 0% |
| Vol Thru, % | 100% | 0% | 88% | 2% | 0% |
| Vol Right, % | 0% | 100% | 0% | 0% | 100% |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 300 | 135 | 306 | 179 | 288 |
| LT Vol | 0 | 0 | 37 | 176 | 0 |
| Through Vol | 300 | 0 | 269 | 3 | 0 |
| RT Vol | 0 | 135 | 0 | 0 | 288 |
| Lane Flow Rate | 326 | 147 | 333 | 195 | 313 |
| Geometry Grp | 7 | 7 | 6 | 7 | 7 |
| Degree of Util (X) | 0.6 | 0.241 | 0.62 | 0.395 | 0.53 |
| Departure Headway (Hd) | 6.625 | 5.911 | 6.713 | 7.306 | 6.093 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 541 | 605 | 536 | 490 | 589 |
| Service Time | 4.394 | 3.68 | 4.779 | 5.074 | 3.86 |
| HCM Lane V/C Ratio | 0.603 | 0.243 | 0.621 | 0.398 | 0.531 |
| HCM Control Delay | 18.9 | 10.6 | 20.2 | 14.8 | 15.6 |
| HCM Lane LOS | C | В | C | В | C |
| HCM 95th-tile Q | 3.9 | 0.9 | 4.2 | 1.9 | 3.1 |

| Intersection | | |
|---------------------------|------|--|
| Intersection Delay, s/veh | 17.4 | |
| Intersection LOS | C | |
| | | |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|---------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 1 | 7 | | 4 | | | | | | 4 | 71 |
| Traffic Vol, veh/h | 0 | 307 | 135 | 37 | 273 | 0 | 0 | 0 | 0 | 182 | 3 | 288 |
| Future Vol, veh/h | 0 | 307 | 135 | 37 | 273 | 0 | 0 | 0 | 0 | 182 | 3 | 288 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 334 | 147 | 40 | 297 | 0 | 0 | 0 | 0 | 198 | 3 | 313 |
| Number of Lanes | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 1 |
| Approach | | EB | | WB | | | | | | SB | | |
| Opposing Approach | | WB | | EB | | | | | | | | |
| Opposing Lanes | | 1 | | 2 | | | | | | 0 | | |
| Conflicting Approach Left | | SB | | | | | | | | WB | | |
| Conflicting Lanes Left | | 2 | | 0 | | | | | | 1 | | |
| Conflicting Approach Rig | ht | | | SB | | | | | | EB | | |
| Conflicting Lanes Right | | 0 | | 2 | | | | | | 2 | | |
| HCM Control Delay | | 16.9 | | 20.8 | | | | | | 15.6 | | |
| HCM LOS | | C | | C | | | | | | C | | |

| Lane | EBLn1 | EBLn2\ | WBLn1 | SBLn1 | SBLn2 |
|------------------------|-------|--------|-------|-------|-------|
| Vol Left, % | 0% | 0% | 12% | 98% | 0% |
| Vol Thru, % | 100% | 0% | 88% | 2% | 0% |
| Vol Right, % | 0% | 100% | 0% | 0% | 100% |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 307 | 135 | 310 | 185 | 288 |
| LT Vol | 0 | 0 | 37 | 182 | 0 |
| Through Vol | 307 | 0 | 273 | 3 | 0 |
| RT Vol | 0 | 135 | 0 | 0 | 288 |
| Lane Flow Rate | 334 | 147 | 337 | 201 | 313 |
| Geometry Grp | 7 | 7 | 6 | 7 | 7 |
| Degree of Util (X) | 0.617 | 0.242 | 0.632 | 0.41 | 0.533 |
| Departure Headway (Hd) | 6.66 | 5.946 | 6.747 | 7.344 | 6.13 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 541 | 600 | 532 | 489 | 585 |
| Service Time | 4.431 | 3.717 | 4.814 | 5.114 | 3.899 |
| HCM Lane V/C Ratio | 0.617 | 0.245 | 0.633 | 0.411 | 0.535 |
| HCM Control Delay | 19.7 | 10.6 | 20.8 | 15.2 | 15.8 |
| HCM Lane LOS | C | В | C | C | C |
| HCM 95th-tile Q | 4.2 | 0.9 | 4.4 | 2 | 3.1 |

| Intersection | | |
|---------------------------|------|--|
| Intersection Delay, s/veh | 28.2 | |
| Intersection LOS | D | |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|---------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 1 | 7" | | 4 | | | | | | 4 | 7 |
| Traffic Vol, veh/h | 0 | 339 | 156 | 89 | 319 | 0 | 0 | 0 | 0 | 234 | 3 | 323 |
| Future Vol, veh/h | 0 | 339 | 156 | 89 | 319 | 0 | 0 | 0 | 0 | 234 | 3 | 323 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 368 | 170 | 97 | 347 | 0 | 0 | 0 | 0 | 254 | 3 | 351 |
| Number of Lanes | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 1 |
| Approach | | EB | | WB | | | | | | SB | | |
| Opposing Approach | | WB | | EB | | | | | | | | |
| Opposing Lanes | | 1 | | 2 | | | | | | 0 | | |
| Conflicting Approach Left | t | SB | | | | | | | | WB | | |
| Conflicting Lanes Left | | 2 | | 0 | | | | | | 1 | | |
| Conflicting Approach Rigi | ht | | | SB | | | | | | EB | | |
| Conflicting Lanes Right | | 0 | | 2 | | | | | | 2 | | |
| HCM Control Delay | | 23.7 | | 43.7 | | | | | | 21 | | |
| HCM LOS | | C | - | E | | | | | | C | | |

| Lane | EBLn1 | EBLn2\ | NBLn1 | SBLn1 | SBLn2 |
|------------------------|-------|--------|-------|-------|-------|
| Vol Left, % | 0% | 0% | 22% | 99% | 0% |
| Vol Thru, % | 100% | 0% | 78% | 1% | 0% |
| Vol Right, % | 0% | 100% | 0% | 0% | 100% |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 339 | 156 | 408 | 237 | 323 |
| LT Vol | 0 | 0 | 89 | 234 | 0 |
| Through Vol | 339 | 0 | 319 | 3 | 0 |
| RT Vol | 0 | 156 | 0 | 0 | 323 |
| Lane Flow Rate | 368 | 170 | 443 | 258 | 351 |
| Geometry Grp | 7 | 7 | 6 | 7 | 7 |
| Degree of Util (X) | 0.75 | 0.311 | 0.882 | 0.569 | 0.656 |
| Departure Headway (Hd) | 7.327 | 6.607 | 7.273 | 7.956 | 6.731 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 494 | 546 | 501 | 456 | 541 |
| Service Time | 5.047 | 4.328 | 5.273 | 5.656 | 4.431 |
| HCM Lane V/C Ratio | 0.745 | 0.311 | 0.884 | 0.566 | 0.649 |
| HCM Control Delay | 28.9 | 12.3 | 43.7 | 20.6 | 21.3 |
| HCM Lane LOS | D | В | E | C | C |
| HCM 95th-tile Q | 6.4 | 1.3 | 9.7 | 3.5 | 4.7 |

| Intersection | | | | | | | | | | | | |
|---------------------------|------|----------|--------|-------|-------|-------|------|------|------|------|------|------|
| Intersection Delay, s/veh | 29.4 | | | | | | | | | | | |
| Intersection LOS | D | | | | | | | | | | | |
| | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | ↑ | 7 | | ৰ | | | | | | 4 | 7 |
| Traffic Vol, veh/h | 0 | 346 | 156 | 89 | 323 | 0 | 0 | 0 | 0 | 240 | 3 | 323 |
| Future Vol, veh/h | 0 | 346 | 156 | 89 | 323 | 0 | 0 | 0 | 0 | 240 | 3 | 323 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 376 | 170 | 97 | 351 | 0 | 0 | 0 | 0 | 261 | 3 | 351 |
| Number of Lanes | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 1 |
| Approach | | EB | | WB | | | | | | SB | | |
| Opposing Approach | | WB | | EB | | | | | | | | |
| Opposing Lanes | | 1 | | 2 | | | | | | 0 | | 100 |
| Conflicting Approach Left | | SB | | | | | | | | WB | | |
| Conflicting Lanes Left | | 2 | | 0 | | | | | | 1 | | |
| Conflicting Approach Righ | nt | | | SB | | | | | | EB | | |
| Conflicting Lanes Right | | 0 | | 2 | | | | | | 2 | | |
| HCM Control Delay | | 24.9 | | 45.9 | | | | | | 21.5 | | |
| HCM LOS | | C | | E | | | | | | C | | 100 |
| | | | | | | | | | | | | |
| Lane | | EBLn1 | EBLn2V | VBLn1 | SBLn1 | SBLn2 | | | | | | |
| Vol Left, % | | 0% | 0% | 22% | 99% | 0% | | | | | | |
| Vol Thru, % | | 100% | 0% | 78% | 1% | 0% | | | | | | |
| Vol Right, % | | 0% | 100% | 0% | 0% | 100% | | | | | | |
| Sign Control | | Stop | Stop | Stop | Stop | Stop | | | | | | 1.27 |
| Traffic Vol by Lane | | 346 | 156 | 412 | 243 | 323 | | | | | | |
| LT Vol | | 0 | 0 | 89 | 240 | 0 | | | | | | |

| orgin contact | Ciop | Ciop | Otop | Otop | Ctop |
|------------------------|-------|-------|-------|-------|-------|
| Traffic Vol by Lane | 346 | 156 | 412 | 243 | 323 |
| LT Vol | 0 | 0 | 89 | 240 | 0 |
| Through Vol | 346 | 0 | 323 | 3 | 0 |
| RT Vol | 0 | 156 | 0 | 0 | 323 |
| Lane Flow Rate | 376 | 170 | 448 | 264 | 351 |
| Geometry Grp | 7 | 7 | 6 | 7 | 7 |
| Degree of Util (X) | 0.769 | 0.313 | 0.895 | 0.586 | 0.66 |
| Departure Headway (Hd) | 7.363 | 6.643 | 7.307 | 7.993 | 6.768 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 493 | 543 | 502 | 454 | 537 |
| Service Time | 5.083 | 4.363 | 5.307 | 5.693 | 4.468 |
| HCM Lane V/C Ratio | 0.763 | 0.313 | 0.892 | 0.581 | 0.654 |
| HCM Control Delay | 30.6 | 12.4 | 45.9 | 21.4 | 21.6 |
| HCM Lane LOS | D | В | E | C | C |
| HCM 95th-tile Q | 6.8 | 1.3 | 10 | 3.7 | 4.8 |
| | | | | | |

| Intersection | | | |
|---------------------------|------|--|--|
| Intersection Delay, s/veh | 12.2 | | |
| Intersection LOS | В | | |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|--------------------------|-------|------|------|------|------|------|------|------|-------|------|------|------|
| Lane Configurations | | 4 | | | 1 | | | 4 | 7 | | | |
| Traffic Vol, veh/h | 203 | 138 | 0 | 0 | 153 | 188 | 86 | 2 | 59 | 0 | 0 | 0 |
| Future Vol, veh/h | 203 | 138 | 0 | 0 | 153 | 188 | 86 | 2 | 59 | 0 | 0 | 0 |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 226 | 153 | 0 | 0 | 170 | 209 | 96 | 2 | 66 | 0 | 0 | 0 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 |
| Approach | EB | | | | WB | | NB | | Y Y 1 | | | |
| Opposing Approach | WB | | | | EB | | | | | | | |
| Opposing Lanes | 1 | | | | 1 | | 0 | | | | | |
| Conflicting Approach Let | ft | | | | NB | | EB | | | | | |
| Conflicting Lanes Left | 0 | | | | 2 | | 1 | | | | | |
| Conflicting Approach Rig | ht NB | | | | | | WB | | | | | |
| Conflicting Lanes Right | 2 | | | | 0 | | 1 | | | | | |
| HCM Control Delay | 13.4 | | | | 11.8 | | 10.3 | | | | | |
| HCM LOS | В | | | | В | | В | | | | | |

| Lane | NBLn1 | NBLn2 | EBLn1\ | WBLn1 | |
|------------------------|-------|-------|--------|-------|--|
| Vol Left, % | 98% | 0% | 60% | 0% | |
| Vol Thru, % | 2% | 0% | 40% | 45% | |
| Vol Right, % | 0% | 100% | 0% | 55% | |
| Sign Control | Stop | Stop | Stop | Stop | |
| Traffic Vol by Lane | 88 | 59 | 341 | 341 | |
| LT Vol | 86 | 0 | 203 | 0 | |
| Through Vol | 2 | 0 | 138 | 153 | |
| RT Vol | 0 | 59 | 0 | 188 | |
| Lane Flow Rate | 98 | 66 | 379 | 379 | |
| Geometry Grp | 7 | 7 | 2 | 2 | |
| Degree of Util (X) | 0.185 | 0.102 | 0.521 | 0.478 | |
| Departure Headway (Hd) | 6.821 | 5.614 | 4.95 | 4.54 | |
| Convergence, Y/N | Yes | Yes | Yes | Yes | |
| Cap | 529 | 642 | 720 | 784 | |
| Service Time | 4.521 | 3.314 | 3.033 | 2.62 | |
| HCM Lane V/C Ratio | 0.185 | 0.103 | 0.526 | 0.483 | |
| HCM Control Delay | 11.1 | 9 | 13.4 | 11.8 | |
| HCM Lane LOS | В | Α | В | В | |
| HCM 95th-tile Q | 0.7 | 0.3 | 3 | 2.6 | |

| Intersection | | |
|---------------------------|------|--|
| Intersection Delay, s/veh | 12.4 | |
| Intersection LOS | В | |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|--------------------------|--------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 4 | | | 1 | | | 4 | 71 | | | |
| Traffic Vol, veh/h | 203 | 142 | 0 | 0 | 159 | 193 | 86 | 2 | 59 | 0 | 0 | 0 |
| Future Vol, veh/h | 203 | 142 | 0 | 0 | 159 | 193 | 86 | 2 | 59 | 0 | 0 | 0 |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 226 | 158 | 0 | 0 | 177 | 214 | 96 | 2 | 66 | 0 | 0 | 0 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 |
| Approach | EB | | | | WB | | NB | | | | | |
| Opposing Approach | WB | | | | EB | | | | | | | |
| Opposing Lanes | 1 | | | | 1 | | 0 | | | | | |
| Conflicting Approach Let | ft | | | | NB | | EB | | | | | |
| Conflicting Lanes Left | 0 | | | | 2 | | 1 | | | | | |
| Conflicting Approach Rig | ght NB | | | | | | WB | | | | | |
| Conflicting Lanes Right | 2 | | | | 0 | | 1 | | | | | |
| HCM Control Delay | 13.6 | | | | 12.1 | | 10.3 | | | | | |
| HCM LOS | В | | | | В | | В | | | | | |

| Lane | NBLn1 | NBLn2 | EBLn1\ | WBLn1 |
|------------------------|-------|-------|--------|-------|
| Vol Left, % | 98% | 0% | 59% | 0% |
| Vol Thru, % | 2% | 0% | 41% | 45% |
| Vol Right, % | 0% | 100% | 0% | 55% |
| Sign Control | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 88 | 59 | 345 | 352 |
| LT Vol | 86 | 0 | 203 | 0 |
| Through Vol | 2 | 0 | 142 | 159 |
| RT Vol | 0 | 59 | 0 | 193 |
| Lane Flow Rate | 98 | 66 | 383 | 391 |
| Geometry Grp | 7 | 7 | 2 | 2 |
| Degree of Util (X) | 0.186 | 0.103 | 0.529 | 0.495 |
| Departure Headway (Hd) | 6.862 | 5.654 | 4.965 | 4.552 |
| Convergence, Y/N | Yes | Yes | Yes | Yes |
| Сар | 526 | 638 | 717 | 782 |
| Service Time | 4.562 | 3.354 | 3.051 | 2.633 |
| HCM Lane V/C Ratio | 0.186 | 0.103 | 0.534 | 0.5 |
| HCM Control Delay | 11.1 | 9 | 13.6 | 12.1 |
| HCM Lane LOS | В | Α | В | В |
| HCM 95th-tile Q | 0.7 | 0.3 | 3.1 | 2.8 |

| Intersection | | | | | | | | | | | | |
|---------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| Intersection Delay, s/veh | 19 | | | | | | | | | | | |
| Intersection LOS | С | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBI | SBT | SBR |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|--------------------------|--------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 4 | | | 1 | | | 4 | 7 | | | |
| Traffic Vol, veh/h | 239 | 198 | 0 | 0 | 213 | 237 | 97 | 2 | 105 | 0 | 0 | 0 |
| Future Vol, veh/h | 239 | 198 | 0 | 0 | 213 | 237 | 97 | 2 | 105 | 0 | 0 | 0 |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 266 | 220 | 0 | 0 | 237 | 263 | 108 | 2 | 117 | 0 | 0 | 0 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 |
| Approach | EB | | | | WB | | NB | | | | | |
| Opposing Approach | WB | | | | EB | | | | | | | |
| Opposing Lanes | 1 | | | | 1 | | 0 | | | | | |
| Conflicting Approach Le | ft | | | | NB | | EB | | | | | |
| Conflicting Lanes Left | 0 | | | | 2 | | 1 | | | | | - |
| Conflicting Approach Rig | ght NB | | | | | | WB | | | | | |
| Conflicting Lanes Right | 2 | | | | 0 | | 1 | | | | | |
| HCM Control Delay | 22.2 | | | | 19.2 | | 11.5 | | | | | |
| HCM LOS | C | | | | C | | В | | | | | |

| Lane | NBLn1 | NBLn2 | EBLn1 | WBLn1 |
|------------------------|-------|-------|-------|-------|
| Vol Left, % | 98% | 0% | 55% | 0% |
| Vol Thru, % | 2% | 0% | 45% | 47% |
| Vol Right, % | 0% | 100% | 0% | 53% |
| Sign Control | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 99 | 105 | 437 | 450 |
| LT Vol | 97 | 0 | 239 | 0 |
| Through Vol | 2 | 0 | 198 | 213 |
| RT Vol | 0 | 105 | 0 | 237 |
| Lane Flow Rate | 110 | 117 | 486 | 500 |
| Geometry Grp | 7 | 7 | 2 | 2 |
| Degree of Util (X) | 0.228 | 0.202 | 0.736 | 0.702 |
| Departure Headway (Hd) | 7.448 | 6.232 | 5.457 | 5.057 |
| Convergence, Y/N | Yes | Yes | Yes | Yes |
| Cap | 482 | 574 | 660 | 715 |
| Service Time | 5.204 | 3.988 | 3.5 | 3.103 |
| HCM Lane V/C Ratio | 0.228 | 0.204 | 0.736 | 0.699 |
| HCM Control Delay | 12.4 | 10.6 | 22.2 | 19.2 |
| HCM Lane LOS | В | В | C | C |
| HCM 95th-tile Q | 0.9 | 0.7 | 6.4 | 5.8 |

| Intersection | |
|-------------------------------|-----|
| Intersection Delay, s/veh 19. | 9.7 |
| Intersection LOS | C |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|--------------------------|--------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 4 | | | 13 | | | 4 | 7 | | | |
| Traffic Vol, veh/h | 239 | 202 | 0 | 0 | 219 | 242 | 97 | 2 | 105 | 0 | 0 | 0 |
| Future Vol, veh/h | 239 | 202 | 0 | 0 | 219 | 242 | 97 | 2 | 105 | 0 | 0 | 0 |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 | 0.90 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 266 | 224 | 0 | 0 | 243 | 269 | 108 | 2 | 117 | 0 | 0 | 0 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 |
| Approach | EB | | | | WB | | NB | | | | | 3 |
| Opposing Approach | WB | | | | EB | | | | | | | |
| Opposing Lanes | 1 | | | | 1 | | 0 | | | | | |
| Conflicting Approach Le | ft | | | | NB | | EB | | | | | |
| Conflicting Lanes Left | 0 | | | | 2 | | 1 | | | | | |
| Conflicting Approach Rig | ght NB | | | | | | WB | | | | | |
| Conflicting Lanes Right | 2 | | | | 0 | | 1 | | | | | 1.1 |
| HCM Control Delay | 22.9 | | | | 20.2 | | 11.5 | | | | | |
| HCM LOS | C | | | | C | | В | | | | | |

| Lane | NBLn1 | NBLn2 | EBLn1\ | WBLn1 |
|------------------------|-------|-------|--------|-------|
| Vol Left, % | 98% | 0% | 54% | 0% |
| Vol Thru, % | 2% | 0% | 46% | 48% |
| Vol Right, % | 0% | 100% | 0% | 52% |
| Sign Control | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 99 | 105 | 441 | 461 |
| LT Vol | 97 | 0 | 239 | 0 |
| Through Vol | 2 | 0 | 202 | 219 |
| RT Vol | 0 | 105 | 0 | 242 |
| Lane Flow Rate | 110 | 117 | 490 | 512 |
| Geometry Grp | 7 | 7 | 2 | 2 |
| Degree of Util (X) | 0.229 | 0.203 | 0.745 | 0.722 |
| Departure Headway (Hd) | 7.488 | 6.272 | 5.477 | 5.071 |
| Convergence, Y/N | Yes | Yes | Yes | Yes |
| Cap | 479 | 571 | 661 | 712 |
| Service Time | 5.247 | 4.03 | 3.523 | 3.116 |
| HCM Lane V/C Ratio | 0.23 | 0.205 | 0.741 | 0.719 |
| HCM Control Delay | 12.5 | 10.6 | 22.9 | 20.2 |
| HCM Lane LOS | В | В | C | C |
| HCM 95th-tile Q | 0.9 | 0.8 | 6.6 | 6.2 |

| Intersection | | |
|---------------------------|------|--|
| Intersection Delay, s/veh | 15.5 | |
| Intersection LOS | C | |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|--------------------------|--------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 4 | | | 1 | | | 4 | 7 | | | |
| Traffic Vol, veh/h | 117 | 356 | 0 | 0 | 209 | 89 | 114 | 1 | 66 | 0 | 0 | 0 |
| Future Vol, veh/h | 117 | 356 | 0 | 0 | 209 | 89 | 114 | 1 | 66 | 0 | 0 | 0 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 124 | 379 | 0 | 0 | 222 | 95 | 121 | 1 | 70 | 0 | 0 | 0 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 |
| Approach | EB | | | | WB | | NB | | | | | |
| Opposing Approach | WB | | | | EB | | | | | | | |
| Opposing Lanes | 1 | | | | 1 | | 0 | | | | | 146 |
| Conflicting Approach Let | ft | | | | NB | | EB | | | | | |
| Conflicting Lanes Left | 0 | | | | 2 | | 1 | | | | | |
| Conflicting Approach Rig | ght NB | | | | | | WB | | | | | |
| Conflicting Lanes Right | 2 | | | | 0 | | 1 | | | | | |
| HCM Control Delay | 19.3 | | | | 12.1 | | 11 | | | | | |
| HCM LOS | C | | | | В | | В | 1000 | | | | 777 |

| Lane | NBLn1 | NBLn2 | EBLn1\ | WBLn1 | |
|------------------------|-------|-------|--------|-------|--|
| Vol Left, % | 99% | 0% | 25% | 0% | |
| Vol Thru, % | 1% | 0% | 75% | 70% | |
| Vol Right, % | 0% | 100% | 0% | 30% | |
| Sign Control | Stop | Stop | Stop | Stop | |
| Traffic Vol by Lane | 115 | 66 | 473 | 298 | |
| LT Vol | 114 | 0 | 117 | 0 | |
| Through Vol | 1 | 0 | 356 | 209 | |
| RT Vol | 0 | 66 | 0 | 89 | |
| Lane Flow Rate | 122 | 70 | 503 | 317 | |
| Geometry Grp | 7 | 7 | 2 | 2 | |
| Degree of Util (X) | 0.239 | 0.114 | 0.707 | 0.445 | |
| Departure Headway (Hd) | 7.045 | 5.828 | 5.055 | 5.05 | |
| Convergence, Y/N | Yes | Yes | Yes | Yes | |
| Сар | 510 | 614 | 720 | 713 | |
| Service Time | 4.785 | 3.568 | 3.055 | 3.08 | |
| HCM Lane V/C Ratio | 0.239 | 0.114 | 0.699 | 0.445 | |
| HCM Control Delay | 12 | 9.3 | 19.3 | 12.1 | |
| HCM Lane LOS | В | Α | C | В | |
| HCM 95th-tile Q | 0.9 | 0.4 | 5.9 | 2.3 | |

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| Intersection | | | | |
|---------------------------|------|--|--|--|
| Intersection Delay, s/veh | 16.1 | | | |
| Intersection LOS | C | | | |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|--------------------------|-------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ৰ | | | 1 | | | ર્ન | 7 | | | |
| Traffic Vol, veh/h | 117 | 369 | 0 | 0 | 213 | 92 | 114 | 1 | 66 | 0 | 0 | 0 |
| Future Vol, veh/h | 117 | 369 | 0 | 0 | 213 | 92 | 114 | 1 | 66 | 0 | 0 | 0 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 124 | 393 | 0 | 0 | 227 | 98 | 121 | 1 | 70 | 0 | 0 | 0 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 |
| Approach | EB | | | | WB | | NB | | | | | |
| Opposing Approach | WB | | | | EB | | | | | | | |
| Opposing Lanes | 1 | | | | 1 | | 0 | | | | | |
| Conflicting Approach Let | ft | | | | NB | | EB | | | | | |
| Conflicting Lanes Left | 0 | | | | 2 | | 1 | | | | | |
| Conflicting Approach Rig | ht NB | | | | | | WB | | | | | |
| Conflicting Lanes Right | 2 | | | | 0 | | 1 | | | | | |
| HCM Control Delay | 20.4 | | | | 12.3 | | 11.1 | | | | | |
| HCM LOS | C | | | | В | | В | | | | | |
| | | | | | | | | | | | | |

| Lane | NBLn1 | NBLn2 | EBLn1 | WBLn1 |
|------------------------|-------|-------|-------|-------|
| Vol Left, % | 99% | 0% | 24% | 0% |
| Vol Thru, % | 1% | 0% | 76% | 70% |
| Vol Right, % | 0% | 100% | 0% | 30% |
| Sign Control | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 115 | 66 | 486 | 305 |
| LT Vol | 114 | 0 | 117 | 0 |
| Through Vol | 1 | 0 | 369 | 213 |
| RT Vol | 0 | 66 | 0 | 92 |
| Lane Flow Rate | 122 | 70 | 517 | 324 |
| Geometry Grp | 7 | 7 | 2 | 2 |
| Degree of Util (X) | 0.241 | 0.115 | 0.728 | 0.457 |
| Departure Headway (Hd) | 7.094 | 5.877 | 5.071 | 5.073 |
| Convergence, Y/N | Yes | Yes | Yes | Yes |
| Cap | 506 | 609 | 719 | 711 |
| Service Time | 4.837 | 3.62 | 3.071 | 3.105 |
| HCM Lane V/C Ratio | 0.241 | 0.115 | 0.719 | 0.456 |
| HCM Control Delay | 12.1 | 9.4 | 20.4 | 12.3 |
| HCM Lane LOS | В | A | C | В |
| HCM 95th-tile Q | 0.9 | 0.4 | 6.4 | 2.4 |

| Intersection | | |
|---------------------------|----|--|
| Intersection Delay, s/veh | 31 | |
| Intersection LOS | D | |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|--------------------------|--------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 4 | | | 1 | | | 4 | 7 | | | |
| Traffic Vol, veh/h | 138 | 432 | 0 | 0 | 276 | 143 | 149 | 1 | 120 | 0 | 0 | 0 |
| Future Vol, veh/h | 138 | 432 | 0 | 0 | 276 | 143 | 149 | 1 | 120 | 0 | 0 | 0 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 147 | 460 | 0 | 0 | 294 | 152 | 159 | 1 | 128 | 0 | 0 | 0 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 |
| Approach | EB | | | | WB | | NB | | | | WW- | |
| Opposing Approach | WB | | | | EB | | | | | | | |
| Opposing Lanes | 1 | | | | 1 | | 0 | | | | | |
| Conflicting Approach Le | ft | | | | NB | | EB | | | | | |
| Conflicting Lanes Left | 0 | | | | 2 | | 1 | | | | | |
| Conflicting Approach Rig | ght NB | | | | | | WB | | | | | |
| Conflicting Lanes Right | 2 | | | | 0 | | 1 | | | | | |
| HCM Control Delay | 47.2 | | | | 20.6 | | 13.1 | | | | | |
| HCM LOS | E | | | | C | | В | | | | | |

| Lane | NBLn1 | NBLn2 | EBLn1\ | WBLn1 | |
|------------------------|-------|-------|--------|-------|--|
| Vol Left, % | 99% | 0% | 24% | 0% | |
| Vol Thru, % | 1% | 0% | 76% | 66% | |
| Vol Right, % | 0% | 100% | 0% | 34% | |
| Sign Control | Stop | Stop | Stop | Stop | |
| Traffic Vol by Lane | 150 | 120 | 570 | 419 | |
| LT Vol | 149 | 0 | 138 | 0 | |
| Through Vol | 1 | 0 | 432 | 276 | |
| RT Vol | 0 | 120 | 0 | 143 | |
| Lane Flow Rate | 160 | 128 | 606 | 446 | |
| Geometry Grp | 7 | 7 | 2 | 2 | |
| Degree of Util (X) | 0.343 | 0.231 | 0.946 | 0.695 | |
| Departure Headway (Hd) | 7.735 | 6.509 | 5.618 | 5.617 | |
| Convergence, Y/N | Yes | Yes | Yes | Yes | |
| Cap | 463 | 549 | 643 | 637 | |
| Service Time | 5.515 | 4.288 | 3.682 | 3.689 | |
| HCM Lane V/C Ratio | 0.346 | 0.233 | 0.942 | 0.7 | |
| HCM Control Delay | 14.5 | 11.3 | 47.2 | 20.6 | |
| HCM Lane LOS | В | В | E | C | |
| HCM 95th-tile Q | 1.5 | 0.9 | 13 | 5.6 | |

| Intersection Delay, s/veh Intersection LOS Movement Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Peak Hour Factor Heavy Vehicles, % Mvmt Flow Number of Lanes | 34 D EBL 138 138 0.94 2 147 | EBT 445 445 0.94 2 | 0 0 0.94 | WBL 0 0 | WBT 280 | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|--|--|--------------------------------|----------------|---------------|-----------|------|------|------|------|------|------|------|
| Movement Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Peak Hour Factor Heavy Vehicles, % Mvmt Flow Number of Lanes | 138 138 0.94 2 147 | 445 445 0.94 2 | 0 | 0 | 4 | WBR | NBL | | | SBL | SBT | SBF |
| Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Peak Hour Factor Heavy Vehicles, % Mvmt Flow Number of Lanes | 138 138 0.94 2 147 | 445 445 0.94 2 | 0 | 0 | 4 | WBR | NBL | | | SBL | SBT | SBF |
| Traffic Vol, veh/h Future Vol, veh/h Peak Hour Factor Heavy Vehicles, % Mvmt Flow Number of Lanes | 138 0.94 2 147 | 445 445 0.94 2 | 0 | | | | | | | | | |
| Future Vol, veh/h Peak Hour Factor Heavy Vehicles, % Mvmt Flow Number of Lanes | 138 0.94 2 147 | 445 445 0.94 2 | 0 | | | | | 4 | 7" | | | |
| Peak Hour Factor Heavy Vehicles, % Mvmt Flow Number of Lanes | 0.94 2 147 | 0.94 | | 0 | 200 | 146 | 149 | 1 | 120 | 0 | 0 | (|
| Heavy Vehicles, % Mvmt Flow Number of Lanes | 2 147 | 2 | 0.94 | U | 280 | 146 | 149 | 1 | 120 | 0 | 0 | (|
| Mvmt Flow Number of Lanes | 147 | | 0.01 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Number of Lanes | | And the second second | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| | 0 | 473 | 0 | 0 | 298 | 155 | 159 | 1 | 128 | 0 | 0 | (|
| Approach | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | (|
| Approach | EB | | | | WB | | NB | | | | | |
| Opposing Approach | WB | | | | EB | | | | | | | |
| Opposing Lanes | 1 | | | | 1 | | 0 | | | | | |
| Conflicting Approach Left | | | | | NB | | EB | | | | | |
| Conflicting Lanes Left | 0 | | | | 2 | | 1 | | | | | |
| Conflicting Approach Righ | | | | | | | WB | | | | | |
| Conflicting Lanes Right | 2 | | | | 0 | | 1 | | | | | |
| | 52.7 | | | | 21.5 | | 13.2 | | | | | |
| HCM LOS | F | | | | С | | В | | | | | |
| | | IDI n1 | NBLn2 | EDI 541 | MDI 51 | | | | | | | |
| Lane Vol Left, % | | | | | | | | | | | | - |
| | | 99% | 0% | 24% | 0% | | | | | | | |
| Vol Thru, % | | 1% 0% | 0% | 76% 0% | 66% | 4.35 | | | | | | |
| Vol Right, % | | | 100% | | 34% | | | | | | | |
| Sign Control | | Stop 150 | Stop | Stop 583 | Stop | | | | | | | |
| Γraffic Vol by Lane ₋T Vol | | 149 | 120 | 138 | 426 0 | | | | | | | |
| Through Vol | | 149 | 0 | 445 | 280 | | | | | | | 20 |
| RT Vol | | 0 | 120 | 0 | 146 | | | | | | | |
| ane Flow Rate | | 160 | 128 | 620 | 453 | | | | | | | |
| Geometry Grp | | 7 | 7 | 2 | 2 | | | | | | | |
| Degree of Util (X) | | 0.345 | 0.233 | 0.972 | 0.711 | | | | | | | |
| Departure Headway (Hd) | | 7.788 | 6.562 | 5.64 | 5.649 | | | | | | | |
| Convergence, Y/N | المراشد | Yes | Yes | Yes | Yes | | | | | | | |
| Cap | | 460 | 544 | 639 | 634 | | | | | | | |
| Service Time | | 5.575 | 4.347 | 3.704 | 3.723 | | | | | | | |
| HCM Lane V/C Ratio | | 0.348 | 0.235 | 0.97 | 0.715 | | | | | | | |
| HCM Control Delay | | 14.7 | 11.4 | 52.7 | 21.5 | | | | | | | |
| HCM Lane LOS | | 14.7 B | 11.4 B | 52.7 F | 21.5 C | | | | | | | |
| HCM 95th-tile Q | | 1.5 | 0.9 | 14.1 | 5.9 | | | | | | | |

| | 1 | - | * | 1 | + | 1 | 1 | † | - | 1 | + | 4 |
|---------------------------|--------|------|------|------|------|------|------|------|------|------|---|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 1 | 1 | 7" | 1 | 1 | 7 | ħ | 1 | | 7 | ↑ | 7" |
| Traffic Volume (veh/h) | 4 | 73 | 53 | 0 | 132 | 2 | 125 | 1 | 0 | 1 | 1 | 7 |
| Future Volume (veh/h) | 4 | 73 | 53 | 0 | 132 | 2 | 125 | 1 | 0 | 1 | 1 | 7 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | า | No | | | No | | | No | | | No | |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 4 | 78 | 56 | 0 | 140 | 2 | 133 | 1 | 0 | 1 | 1 | 7 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 10 | 553 | 469 | 6 | 273 | 231 | 196 | 503 | 0 | 6 | 300 | 255 |
| Arrive On Green | 0.01 | 0.30 | 0.30 | 0.00 | 0.15 | 0.15 | 0.11 | 0.27 | 0.00 | 0.00 | 0.16 | 0.16 |
| Sat Flow, veh/h | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 1870 | 0 | 1781 | 1870 | 1585 |
| Grp Volume(v), veh/h | 4 | 78 | 56 | 0 | 140 | 2 | 133 | 1 | 0 | 1 | 1 | 7 |
| Grp Sat Flow(s), veh/h/ln | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 1870 | 0 | 1781 | 1870 | 1585 |
| Q Serve(g_s), s | 0.1 | 1.0 | 0.8 | 0.0 | 2.2 | 0.0 | 2.2 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 |
| Cycle Q Clear(g_c), s | 0.1 | 1.0 | 0.8 | 0.0 | 2.2 | 0.0 | 2.2 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 |
| Prop In Lane | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 0.00 | 1.00 | 100000000000000000000000000000000000000 | 1.00 |
| Lane Grp Cap(c), veh/h | 10 | 553 | 469 | 6 | 273 | 231 | 196 | 503 | 0 | 6 | 300 | 255 |
| V/C Ratio(X) | 0.41 | 0.14 | 0.12 | 0.00 | 0.51 | 0.01 | 0.68 | 0.00 | 0.00 | 0.17 | 0.00 | 0.03 |
| Avail Cap(c_a), veh/h | 286 | 1082 | 917 | 286 | 1082 | 917 | 338 | 1142 | 0 | 286 | 1088 | 922 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 15.4 | 8.1 | 8.0 | 0.0 | 12.3 | 11.4 | 13.3 | 8.3 | 0.0 | 15.5 | 11.0 | 11.0 |
| Incr Delay (d2), s/veh | 25.5 | 0.1 | 0.1 | 0.0 | 1.5 | 0.0 | 4.1 | 0.0 | 0.0 | 13.9 | 0.0 | 0.0 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%), veh/ | In 0.1 | 0.3 | 0.2 | 0.0 | 0.8 | 0.0 | 0.9 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Unsig. Movement Delay, | s/veh | | | | | | | | | | | |
| LnGrp Delay(d),s/veh | 40.9 | 8.2 | 8.1 | 0.0 | 13.8 | 11.4 | 17.4 | 8.3 | 0.0 | 29.4 | 11.0 | 11.1 |
| LnGrp LOS | D | Α | Α | Α | В | В | В | Α | Α | С | В | В |
| Approach Vol, veh/h | | 138 | | | 142 | | | 134 | | | 9 | |
| Approach Delay, s/veh | | 9.1 | | | 13.7 | | | 17.4 | | | 13.1 | |
| Approach LOS | | А | | | В | | | В | | | В | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), | s 4.5 | 12.9 | 0.0 | 13.7 | 7.9 | 9.5 | 4.7 | 9.0 | | | | |
| Change Period (Y+Rc), s | | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | | | | |
| Max Green Setting (Gmax | | 19.0 | 5.0 | 18.0 | 5.9 | 18.1 | 5.0 | 18.0 | | | | |
| Max Q Clear Time (g_c+l | | 2.0 | 0.0 | 3.0 | 4.2 | 2.1 | 2.1 | 4.2 | | | | - |
| Green Ext Time (p_c), s | 0.0 | 0.0 | 0.0 | 0.4 | 0.0 | 0.0 | 0.0 | 0.5 | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 6th Ctrl Delay | | | 13.4 | | | | | | | | | |
| HCM 6th LOS | | | В | | | | | | | | | |

| | 1 | - | 7 | 1 | ← | 4 | 1 | 1 | - | 1 | + | 1 |
|---|-----------|------------|------|------|----------|------|------|------|------|------|----------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | ^ | 7 | 7 | ^ | 7 | 7 | 13 | | 7 | ↑ | 7 |
| Traffic Volume (veh/h) | 4 | 73 | 57 | 0 | 132 | 2 | 136 | 1 | 0 | 1 | 1 | 7 |
| Future Volume (veh/h) | 4 | 73 | 57 | 0 | 132 | 2 | 136 | 1 | 0 | 1 | 1 | 7 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | | No | | | No | | | No | | | No | |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 4 | 78 | 61 | 0 | 140 | 2 | 145 | 1 | 0 | 1 | 1 | 7 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 10 | 552 | 467 | 6 | 273 | 231 | 204 | 510 | 0 | 6 | 299 | 253 |
| Arrive On Green | 0.01 | 0.29 | 0.29 | 0.00 | 0.15 | 0.15 | 0.11 | 0.27 | 0.00 | 0.00 | 0.16 | 0.16 |
| Sat Flow, veh/h | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 1870 | 0 | 1781 | 1870 | 1585 |
| Grp Volume(v), veh/h | 4 | 78 | 61 | 0 | 140 | 2 | 145 | 1 | 0 | 1 | 1 | 7 |
| Grp Sat Flow(s), veh/h/ln | | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 1870 | 0 | 1781 | 1870 | 1585 |
| Q Serve(g_s), s | 0.1 | 1.0 | 0.9 | 0.0 | 2.2 | 0.0 | 2.5 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 |
| Cycle Q Clear(g_c), s | 0.1 | 1.0 | 0.9 | 0.0 | 2.2 | 0.0 | 2.5 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 |
| Prop In Lane | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 0.00 | 1.00 | | 1.00 |
| Lane Grp Cap(c), veh/h | 10 | 552 | 467 | 6 | 273 | 231 | 204 | 510 | 0 | 6 | 299 | 253 |
| V/C Ratio(X) | 0.41 | 0.14 | 0.13 | 0.00 | 0.51 | 0.01 | 0.71 | 0.00 | 0.00 | 0.18 | 0.00 | 0.03 |
| Avail Cap(c_a), veh/h | 284 | 1075 | 911 | 284 | 1075 | 911 | 336 | 1135 | 0 | 284 | 1081 | 916 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 15.5 | 8.1 | 8.1 | 0.0 | 12.4 | 11.4 | 13.4 | 8.3 | 0.0 | 15.6 | 11.1 | 11.1 |
| Incr Delay (d2), s/veh | 25.5 | 0.1 | 0.1 | 0.0 | 1.5 | 0.0 | 4.5 | 0.0 | 0.0 | 14.1 | 0.0 | 0.0 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/li | | 0.3 | 0.2 | 0.0 | 0.8 | 0.0 | 1.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Unsig. Movement Delay, s | | 0.0 | 0.0 | 0.0 | 40.0 | 44.5 | 17.0 | 0.0 | 0.0 | 00.7 | 44.4 | |
| LnGrp Delay(d),s/veh LnGrp LOS | 41.0 D | 8.2 | 8.2 | 0.0 | 13.8 | 11.5 | 17.9 | 8.3 | 0.0 | 29.7 | 11.1 | 11.2 |
| | D | A 442 | Α | A | B | В | В | A | Α | С | В | В |
| Approach Vol, veh/h Approach Delay, s/veh | | 143 9.1 | | | 142 | | | 146 | | | 9 | |
| Approach LOS | - | | | | 13.8 | | _ | 17.8 | | | 13.2 | |
| Approach LOS | | А | | | В | | | В | | | В | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), s | | 13.0 | 0.0 | 13.7 | 8.1 | 9.5 | 4.7 | 9.1 | | | | |
| Change Period (Y+Rc), s | | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | | | | |
| Max Green Setting (Gmax | | 19.0 | 5.0 | 18.0 | 5.9 | 18.1 | 5.0 | 18.0 | | | | |
| Max Q Clear Time (g_c+l1 | | 2.0 | 0.0 | 3.0 | 4.5 | 2.1 | 2.1 | 4.2 | | | | |
| Green Ext Time (p_c), s | 0.0 | 0.0 | 0.0 | 0.4 | 0.0 | 0.0 | 0.0 | 0.5 | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 6th Ctrl Delay | | | 13.6 | | | | | | | | | |
| HCM 6th LOS | | | В | | | | | | | | | |

| | 1 | → | 7 | 1 | + | 1 | 1 | † | - | 1 | + | 4 |
|---------------------------|--|----------|------|------|------|------|------|----------------|------|------|----------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 1 | ↑ | 7 | 7 | 1 | 7" | 7 | T _P | | 7 | ↑ | 7 |
| Traffic Volume (veh/h) | 4 | 73 | 66 | 0 | 132 | 2 | 141 | 1 | 0 | 1 | 1 | 7 |
| Future Volume (veh/h) | 4 | 73 | 66 | 0 | 132 | 2 | 141 | 1 | 0 | 1 | 1 | 7 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | | No | | | No | | | No | | | No | |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 4 | 78 | 70 | 0 | 140 | 2 | 150 | 1 | 0 | 1 | 1 | 7 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 10 | 552 | 468 | 6 | 274 | 232 | 207 | 512 | 0 | 6 | 298 | 252 |
| Arrive On Green | 0.01 | 0.30 | 0.30 | 0.00 | 0.15 | 0.15 | 0.12 | 0.27 | 0.00 | 0.00 | 0.16 | 0.16 |
| Sat Flow, veh/h | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 1870 | 0 | 1781 | 1870 | 1585 |
| Grp Volume(v), veh/h | 4 | 78 | 70 | 0 | 140 | 2 | 150 | 1 | 0 | 1 | 1 | 7 |
| Grp Sat Flow(s),veh/h/ln | | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 1870 | 0 | 1781 | 1870 | 1585 |
| Q Serve(g_s), s | 0.1 | 1.0 | 1.0 | 0.0 | 2.2 | 0.0 | 2.6 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 |
| Cycle Q Clear(g_c), s | 0.1 | 1.0 | 1.0 | 0.0 | 2.2 | 0.0 | 2.6 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 |
| Prop In Lane | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 0.00 | 1.00 | | 1.00 |
| Lane Grp Cap(c), veh/h | 10 | 552 | 468 | 6 | 274 | 232 | 207 | 512 | 0 | 6 | 298 | 252 |
| V/C Ratio(X) | 0.41 | 0.14 | 0.15 | 0.00 | 0.51 | 0.01 | 0.72 | 0.00 | 0.00 | 0.18 | 0.00 | 0.03 |
| Avail Cap(c_a), veh/h | 283 | 1071 | 908 | 283 | 1071 | 908 | 340 | 1131 | 0 | 283 | 1071 | 908 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 15.6 | 8.1 | 8.2 | 0.0 | 12.4 | 11.5 | 13.4 | 8.3 | 0.0 | 15.6 | 11.1 | 11.2 |
| Incr Delay (d2), s/veh | 25.5 | 0.1 | 0.1 | 0.0 | 1.5 | 0.0 | 4.8 | 0.0 | 0.0 | 14.2 | 0.0 | 0.0 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ | | 0.3 | 0.3 | 0.0 | 0.8 | 0.0 | 1.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Unsig. Movement Delay, | of the late of the | | | | | | | | | | | |
| LnGrp Delay(d),s/veh | 41.1 | 8.3 | 8.3 | 0.0 | 13.8 | 11.5 | 18.2 | 8.3 | 0.0 | 29.8 | 11.1 | 11.2 |
| LnGrp LOS | D | Α | A | A | В | В | В | Α | Α | С | В | В |
| Approach Vol, veh/h | | 152 | | | 142 | | | 151 | | | 9 | |
| Approach Delay, s/veh | | 9.1 | | | 13.8 | | | 18.1 | | | 13.3 | |
| Approach LOS | | Α | | | В | | | В | | | В | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), | s 4.5 | 13.1 | 0.0 | 13.8 | 8.1 | 9.5 | 4.7 | 9.1 | | | | |
| Change Period (Y+Rc), s | | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | | | | |
| Max Green Setting (Gmax | x),5s0 | 19.0 | 5.0 | 18.0 | 6.0 | 18.0 | 5.0 | 18.0 | | | | |
| Max Q Clear Time (g_c+l | 1)2s0 | 2.0 | 0.0 | 3.0 | 4.6 | 2.1 | 2.1 | 4.2 | | | | |
| Green Ext Time (p_c), s | 0.0 | 0.0 | 0.0 | 0.5 | 0.1 | 0.0 | 0.0 | 0.5 | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 6th Ctrl Delay | | | 13.7 | | | | | - | 1 | | | |
| HCM 6th LOS | | | В | | | | | | | | | |

| | 1 | - | * | - | - | 4 | 1 | † | - | 1 | Į. | 1 |
|---------------------------|--------|----------|-------|------|----------|------|------|------|------|------|----------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | ^ | 7" | 7 | ^ | 7 | M | 4 | | 3 | ^ | 7" |
| Traffic Volume (veh/h) | 4 | 73 | 70 | 0 | 132 | 2 | 152 | 1 | 0 | 1 | 1 | 7 |
| Future Volume (veh/h) | 4 | 73 | 70 | 0 | 132 | 2 | 152 | 1 | 0 | 1 | 1 | 7 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | | No | | | No | | | No | | | No | |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 4 | 78 | 74 | 0 | 140 | 2 | 162 | 1 | 0 | 1 | 1 | 7 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 10 | 550 | 466 | 6 | 274 | 232 | 214 | 518 | 0 | 6 | 296 | 251 |
| Arrive On Green | 0.01 | 0.29 | 0.29 | 0.00 | 0.15 | 0.15 | 0.12 | 0.28 | 0.00 | 0.00 | 0.16 | 0.16 |
| Sat Flow, veh/h | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 1870 | 0 | 1781 | 1870 | 1585 |
| Grp Volume(v), veh/h | 4 | 78 | 74 | 0 | 140 | 2 | 162 | 1 | 0 | 1 | 1 | 7 |
| Grp Sat Flow(s), veh/h/ln | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 1870 | 0 | 1781 | 1870 | 1585 |
| Q Serve(g_s), s | 0.1 | 1.0 | 1.1 | 0.0 | 2.2 | 0.0 | 2.8 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 |
| Cycle Q Clear(g_c), s | 0.1 | 1.0 | 1.1 | 0.0 | 2.2 | 0.0 | 2.8 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 |
| Prop In Lane | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 0.00 | 1.00 | | 1.00 |
| Lane Grp Cap(c), veh/h | 10 | 550 | 466 | 6 | 274 | 232 | 214 | 518 | 0 | 6 | 296 | 251 |
| V/C Ratio(X) | 0.41 | 0.14 | 0.16 | 0.00 | 0.51 | 0.01 | 0.76 | 0.00 | 0.00 | 0.18 | 0.00 | 0.03 |
| Avail Cap(c_a), veh/h | 282 | 1066 | 903 | 282 | 1066 | 903 | 310 | 1125 | 0 | 282 | 1095 | 928 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 15.7 | 8.2 | 8.3 | 0.0 | 12.4 | 11.5 | 13.5 | 8.3 | 0.0 | 15.7 | 11.2 | 11.2 |
| Incr Delay (d2), s/veh | 25.5 | 0.1 | 0.2 | 0.0 | 1.5 | 0.0 | 6.2 | 0.0 | 0.0 | 14.3 | 0.0 | 0.0 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ | In 0.1 | 0.3 | 0.3 | 0.0 | 0.8 | 0.0 | 1.2 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Unsig. Movement Delay, | s/veh | | | | | | | | | | | |
| LnGrp Delay(d),s/veh | 41.1 | 8.3 | 8.4 | 0.0 | 13.9 | 11.5 | 19.7 | 8.3 | 0.0 | 30.0 | 11.2 | 11.3 |
| LnGrp LOS | D | Α | Α | Α | В | В | В | Α | Α | С | В | В |
| Approach Vol, veh/h | | 156 | - 100 | | 142 | | | 163 | | 100 | 9 | 4 |
| Approach Delay, s/veh | | 9.2 | | | 13.9 | | | 19.6 | | | 13.4 | |
| Approach LOS | | Α | | | В | | | В | | | В | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), | s 4.5 | 13.2 | 0.0 | 13.8 | 8.3 | 9.5 | 4.7 | 9.1 | | | | |
| Change Period (Y+Rc), s | | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | | | | |
| Max Green Setting (Gmax | | 19.0 | 5.0 | 18.0 | 5.5 | 18.5 | 5.0 | 18.0 | | | | |
| Max Q Clear Time (g_c+l | | 2.0 | 0.0 | 3.1 | 4.8 | 2.1 | 2.1 | 4.2 | | | | |
| Green Ext Time (p_c), s | 0.0 | 0.0 | 0.0 | 0.5 | 0.0 | 0.0 | 0.0 | 0.5 | | | | |
| Intersection Summary | | | 0.0 | | 0.0 | 9.19 | 0.0 | 0.0 | | | | |
| | | | 112 | | | | | | | | | |
| HCM 6th Ctrl Delay | | | 14.3 | | | | | | | | | |
| HCM 6th LOS | | | В | | | | | | | | | |

| | 1 | → | 7 | 1 | - | 1 | 4 | 1 | 1 | 1 | + | 4 |
|---------------------------|-------|----------|------|------|------|------|------|------|------|------|----------|-------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 1 | ↑ | 7" | 7 | ^ | 7 | 7 | 4 | | 7 | 1 | 7 |
| Traffic Volume (veh/h) | 18 | 178 | 149 | 0 | 91 | 3 | 112 | 4 | 1 | 3 | 1 | 16 |
| Future Volume (veh/h) | 18 | 178 | 149 | 0 | 91 | 3 | 112 | 4 | 1 | 3 | 1 | 16 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | | No | | | No | | | No | | | No | |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 19 | 187 | 157 | 0 | 96 | 3 | 118 | 4 | 1 | 3 | 1 | 17 |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 43 | 596 | 505 | 6 | 287 | 243 | 181 | 367 | 92 | 7 | 293 | 248 |
| Arrive On Green | 0.02 | 0.32 | 0.32 | 0.00 | 0.15 | 0.15 | 0.10 | 0.25 | 0.25 | 0.00 | 0.16 | 0.16 |
| Sat Flow, veh/h | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 1444 | 361 | 1781 | 1870 | 1585 |
| Grp Volume(v), veh/h | 19 | 187 | 157 | 0 | 96 | 3 | 118 | 0 | 5 | 3 | 1 | 17 |
| Grp Sat Flow(s), veh/h/ln | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 0 | 1805 | 1781 | 1870 | 1585 |
| Q Serve(g_s), s | 0.3 | 2.4 | 2.4 | 0.0 | 1.5 | 0.1 | 2.0 | 0.0 | 0.1 | 0.1 | 0.0 | 0.3 |
| Cycle Q Clear(g_c), s | 0.3 | 2.4 | 2.4 | 0.0 | 1.5 | 0.1 | 2.0 | 0.0 | 0.1 | 0.1 | 0.0 | 0.3 |
| Prop In Lane | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 0.20 | 1.00 | | 1.00 |
| Lane Grp Cap(c), veh/h | 43 | 596 | 505 | 6 | 287 | 243 | 181 | 0 | 459 | 7 | 293 | 248 |
| V/C Ratio(X) | 0.44 | 0.31 | 0.31 | 0.00 | 0.33 | 0.01 | 0.65 | 0.00 | 0.01 | 0.41 | 0.00 | 0.07 |
| Avail Cap(c_a), veh/h | 279 | 1055 | 894 | 279 | 1055 | 894 | 307 | 0 | 1075 | 279 | 1084 | 919 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 15.4 | 8.2 | 8.2 | 0.0 | 12.1 | 11.5 | 13.8 | 0.0 | 8.9 | 15.9 | 11.4 | 11.5 |
| ncr Delay (d2), s/veh | 6.9 | 0.3 | 0.3 | 0.0 | 0.7 | 0.0 | 3.9 | 0.0 | 0.0 | 32.8 | 0.0 | 0.1 |
| nitial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ | | 0.7 | 0.6 | 0.0 | 0.5 | 0.0 | 0.8 | 0.0 | 0.0 | 0.1 | 0.0 | 0.1 |
| Jnsig. Movement Delay, | s/veh | | | | | | | | | | | |
| _nGrp Delay(d),s/veh | 22.2 | 8.5 | 8.6 | 0.0 | 12.7 | 11.5 | 17.7 | 0.0 | 8.9 | 48.6 | 11.4 | 11.6 |
| _nGrp LOS | С | Α | Α | Α | В | В | В | Α | Α | D | В | В |
| Approach Vol, veh/h | | 363 | | | 99 | | | 123 | | | 21 | |
| Approach Delay, s/veh | | 9.3 | | | 12.7 | | | 17.4 | | | 16.9 | - |
| Approach LOS | | Α | | | В | | | В | | | В | |
| Γimer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), | s 4.6 | 12.6 | 0.0 | 14.7 | 7.7 | 9.5 | 5.3 | 9.4 | | | | |
| Change Period (Y+Rc), s | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | | | | |
| Max Green Setting (Gmax | | 19.0 | 5.0 | 18.0 | 5.5 | 18.5 | 5.0 | 18.0 | | | | |
| Max Q Clear Time (g_c+l | | 2.1 | 0.0 | 4.4 | 4.0 | 2.3 | 2.3 | 3.5 | | | | |
| Green Ext Time (p_c), s | 0.0 | 0.0 | 0.0 | 1.3 | 0.0 | 0.0 | 0.0 | 0.3 | | | | |
| ntersection Summary | | | 1.5 | | | | | | | | | |
| HCM 6th Ctrl Delay | | | 11.7 | | | | | - | | | | 13.31 |
| HCM 6th LOS | | | В | | | | | | | | | |

| | 1 | → | 7 | 1 | - | 1 | 1 | 1 | - | 1 | + | 4 |
|---------------------------|--------|----------|------|------|----------|------|------|--------|------|------|----------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | ^ | 7 | 7 | ^ | 7 | 7 | 13 | | Ť | ↑ | 7 |
| Traffic Volume (veh/h) | 18 | 178 | 162 | 0 | 91 | 3 | 119 | 4 | 1 | 3 | 1 | 16 |
| Future Volume (veh/h) | 18 | 178 | 162 | 0 | 91 | 3 | 119 | 4 | 1 | 3 | 1 | 16 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | 1 | No | | | No | | | No | | | No | |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 19 | 187 | 171 | 0 | 96 | 3 | 125 | 4 | 1 | 3 | 1 | 17 |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 43 | 595 | 504 | 6 | 287 | 243 | 187 | 371 | 93 | 7 | 292 | 247 |
| Arrive On Green | 0.02 | 0.32 | 0.32 | 0.00 | 0.15 | 0.15 | 0.10 | 0.26 | 0.26 | 0.00 | 0.16 | 0.16 |
| Sat Flow, veh/h | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 1444 | 361 | 1781 | 1870 | 1585 |
| Grp Volume(v), veh/h | 19 | 187 | 171 | 0 | 96 | 3 | 125 | 0 | 5 | 3 | 1 | 17 |
| Grp Sat Flow(s), veh/h/ln | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 0 | 1805 | 1781 | 1870 | 1585 |
| Q Serve(g_s), s | 0.3 | 2.4 | 2.6 | 0.0 | 1.5 | 0.1 | 2.2 | 0.0 | 0.1 | 0.1 | 0.0 | 0.3 |
| Cycle Q Clear(g_c), s | 0.3 | 2.4 | 2.6 | 0.0 | 1.5 | 0.1 | 2.2 | 0.0 | 0.1 | 0.1 | 0.0 | 0.3 |
| Prop In Lane | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | 000000 | 0.20 | 1.00 | | 1.00 |
| Lane Grp Cap(c), veh/h | 43 | 595 | 504 | 6 | 287 | 243 | 187 | 0 | 463 | 7 | 292 | 247 |
| V/C Ratio(X) | 0.44 | 0.31 | 0.34 | 0.00 | 0.33 | 0.01 | 0.67 | 0.00 | 0.01 | 0.41 | 0.00 | 0.07 |
| Avail Cap(c_a), veh/h | 278 | 1050 | 890 | 278 | 1050 | 890 | 311 | 0 | 1070 | 278 | 1074 | 910 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 15.4 | 8.3 | 8.4 | 0.0 | 12.1 | 11.5 | 13.8 | 0.0 | 8.9 | 15.9 | 11.4 | 11.5 |
| Incr Delay (d2), s/veh | 6.9 | 0.3 | 0.4 | 0.0 | 0.7 | 0.0 | 4.1 | 0.0 | 0.0 | 32.8 | 0.0 | 0.1 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ | In 0.2 | 0.7 | 0.7 | 0.0 | 0.5 | 0.0 | 0.9 | 0.0 | 0.0 | 0.1 | 0.0 | 0.1 |
| Unsig. Movement Delay, | | | | | | | | | | | | |
| LnGrp Delay(d),s/veh | 22.3 | 8.6 | 8.7 | 0.0 | 12.8 | 11.5 | 17.9 | 0.0 | 8.9 | 48.7 | 11.4 | 11.7 |
| LnGrp LOS | С | Α | Α | Α | В | В | В | Α | Α | D | В | В |
| Approach Vol, veh/h | | 377 | | 7-17 | 99 | | | 130 | | | 21 | |
| Approach Delay, s/veh | | 9.3 | | | 12.8 | | | 17.6 | | | 16.9 | |
| Approach LOS | | А | | | В | | | В | | | В | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), | s 4.6 | 12.7 | 0.0 | 14.7 | 7.9 | 9.5 | 5.3 | 9.4 | | | | |
| Change Period (Y+Rc), s | | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | | | | |
| Max Green Setting (Gmax | | 19.0 | 5.0 | 18.0 | 5.6 | 18.4 | 5.0 | 18.0 | | | | |
| Max Q Clear Time (g_c+l | | 2.1 | 0.0 | 4.6 | 4.2 | 2.3 | 2.3 | 3.5 | | | | |
| Green Ext Time (p_c), s | 0.0 | 0.0 | 0.0 | 1.3 | 0.0 | 0.0 | 0.0 | 0.3 | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 6th Ctrl Delay | - | | 11.8 | | | | | - | | | - | |
| HCM 6th LOS | | | В | | | | | | | | | - |
| | | | | | | | | | | | | |

| | 1 | → | 7 | - | + | 4 | 4 | † | - | 1 | + | 4 |
|---------------------------|--------|----------|------|------|------|------|------|------|------|------|----------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 1/2 | ^ | 7" | 7 | 1 | 7 | 7 | 1> | | 7 | 1 | 7 |
| Traffic Volume (veh/h) | 18 | 178 | 170 | 0 | 91 | 3 | 130 | 4 | 1 | 3 | 1 | 16 |
| Future Volume (veh/h) | 18 | 178 | 170 | 0 | 91 | 3 | 130 | 4 | 1 | 3 | 1 | 16 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | | No | | | No | | | No | | | No | |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 19 | 187 | 179 | 0 | 96 | 3 | 137 | 4 | 1 | 3 | 1 | 17 |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 43 | 592 | 502 | 6 | 286 | 242 | 195 | 376 | 94 | 7 | 290 | 246 |
| Arrive On Green | 0.02 | 0.32 | 0.32 | 0.00 | 0.15 | 0.15 | 0.11 | 0.26 | 0.26 | 0.00 | 0.16 | 0.16 |
| Sat Flow, veh/h | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 1444 | 361 | 1781 | 1870 | 1585 |
| Grp Volume(v), veh/h | 19 | 187 | 179 | 0 | 96 | 3 | 137 | 0 | 5 | 3 | 1 | 17 |
| Grp Sat Flow(s), veh/h/ln | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 0 | 1805 | 1781 | 1870 | 1585 |
| Q Serve(g_s), s | 0.3 | 2.4 | 2.8 | 0.0 | 1.5 | 0.1 | 2.4 | 0.0 | 0.1 | 0.1 | 0.0 | 0.3 |
| Cycle Q Clear(g_c), s | 0.3 | 2.4 | 2.8 | 0.0 | 1.5 | 0.1 | 2.4 | 0.0 | 0.1 | 0.1 | 0.0 | 0.3 |
| Prop In Lane | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 0.20 | 1.00 | | 1.00 |
| Lane Grp Cap(c), veh/h | 43 | 592 | 502 | 6 | 286 | 242 | 195 | 0 | 470 | 7 | 290 | 246 |
| V/C Ratio(X) | 0.44 | 0.32 | 0.36 | 0.00 | 0.34 | 0.01 | 0.70 | 0.00 | 0.01 | 0.41 | 0.00 | 0.07 |
| Avail Cap(c_a), veh/h | 276 | 1044 | 885 | 276 | 1044 | 885 | 332 | 0 | 1064 | 276 | 1044 | 885 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 15.5 | 8.4 | 8.5 | 0.0 | 12.2 | 11.6 | 13.8 | 0.0 | 8.8 | 16.0 | 11.5 | 11.6 |
| Incr Delay (d2), s/veh | 6.9 | 0.3 | 0.4 | 0.0 | 0.7 | 0.0 | 4.5 | 0.0 | 0.0 | 32.8 | 0.0 | 0.1 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ | In 0.2 | 0.7 | 0.7 | 0.0 | 0.5 | 0.0 | 1.0 | 0.0 | 0.0 | 0.1 | 0.0 | 0.1 |
| Unsig. Movement Delay, | s/veh | | | | | | | | | | | |
| LnGrp Delay(d),s/veh | 22.4 | 8.7 | 8.9 | 0.0 | 12.9 | 11.6 | 18.4 | 0.0 | 8.8 | 48.8 | 11.5 | 11.7 |
| LnGrp LOS | С | Α | Α | Α | В | В | В | Α | Α | D | В | В |
| Approach Vol, veh/h | | 385 | | | 99 | | | 142 | | | 21 | - 77 |
| Approach Delay, s/veh | | 9.5 | | | 12.8 | | | 18.0 | | | 17.0 | |
| Approach LOS | | Α | | | В | | | В | | | В | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), | s 4.6 | 12.9 | 0.0 | 14.7 | 8.0 | 9.5 | 5.3 | 9.4 | | | | |
| Change Period (Y+Rc), s | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | | | | |
| Max Green Setting (Gma | x),5s0 | 19.0 | 5.0 | 18.0 | 6.0 | 18.0 | 5.0 | 18.0 | | | | |
| Max Q Clear Time (g_c+l | 1)2s1 | 2.1 | 0.0 | 4.8 | 4.4 | 2.3 | 2.3 | 3.5 | | | | |
| Green Ext Time (p_c), s | 0.0 | 0.0 | 0.0 | 1.4 | 0.1 | 0.0 | 0.0 | 0.3 | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 6th Ctrl Delay | | 1 | 12.1 | | | | | | | | | |
| HCM 6th LOS | | | В | | | | | | | | | |

| | 1 | - | 7 | - | 4 | 1 | 4 | † | - | 1 | + | 1 |
|---------------------------|-------|----------|-------|-------|----------|------|------|------|------|------|----------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | 1 | 7 | 75 | ^ | 7 | 7 | 13 | | ሻ | ^ | 7 |
| Traffic Volume (veh/h) | 18 | 178 | 183 | 0 | 91 | 3 | 137 | 4 | 1 | 3 | 1 | 16 |
| Future Volume (veh/h) | 18 | 178 | 183 | 0 | 91 | 3 | 137 | 4 | 1 | 3 | 1 | 16 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | 1 | No | | | No | | | No | | | No | |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 19 | 187 | 193 | 0 | 96 | 3 | 144 | 4 | 1 | 3 | 1 | 17 |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 43 | 591 | 501 | 6 | 285 | 242 | 200 | 379 | 95 | 7 | 289 | 245 |
| Arrive On Green | 0.02 | 0.32 | 0.32 | 0.00 | 0.15 | 0.15 | 0.11 | 0.26 | 0.26 | 0.00 | 0.15 | 0.15 |
| Sat Flow, veh/h | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 1444 | 361 | 1781 | 1870 | 1585 |
| Grp Volume(v), veh/h | 19 | 187 | 193 | 0 | 96 | 3 | 144 | 0 | 5 | 3 | 1 | 17 |
| Grp Sat Flow(s), veh/h/ln | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 | 1781 | 0 | 1805 | 1781 | 1870 | 1585 |
| Q Serve(g_s), s | 0.3 | 2.5 | 3.1 | 0.0 | 1.5 | 0.1 | 2.5 | 0.0 | 0.1 | 0.1 | 0.0 | 0.3 |
| Cycle Q Clear(g_c), s | 0.3 | 2.5 | 3.1 | 0.0 | 1.5 | 0.1 | 2.5 | 0.0 | 0.1 | 0.1 | 0.0 | 0.3 |
| Prop In Lane | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 0.20 | 1.00 | | 1.00 |
| Lane Grp Cap(c), veh/h | 43 | 591 | 501 | 6 | 285 | 242 | 200 | 0 | 474 | 7 | 289 | 245 |
| V/C Ratio(X) | 0.44 | 0.32 | 0.39 | 0.00 | 0.34 | 0.01 | 0.72 | 0.00 | 0.01 | 0.41 | 0.00 | 0.07 |
| Avail Cap(c_a), veh/h | 275 | 1041 | 882 | 275 | 1041 | 882 | 330 | 0 | 1060 | 275 | 1041 | 882 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 15.6 | 8.4 | 8.6 | 0.0 | 12.2 | 11.6 | 13.9 | 0.0 | 8.8 | 16.1 | 11.6 | 11.7 |
| Incr Delay (d2), s/veh | 6.9 | 0.3 | 0.5 | 0.0 | 0.7 | 0.0 | 4.8 | 0.0 | 0.0 | 32.8 | 0.0 | 0.1 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ | | 0.7 | 0.8 | 0.0 | 0.5 | 0.0 | 1.1 | 0.0 | 0.0 | 0.1 | 0.0 | 0.1 |
| Unsig. Movement Delay, | | | | | | | | | | | | |
| LnGrp Delay(d),s/veh | 22.4 | 8.7 | 9.1 | 0.0 | 12.9 | 11.7 | 18.7 | 0.0 | 8.8 | 48.9 | 11.6 | 11.8 |
| LnGrp LOS | С | Α | Α | Α | В | В | В | Α | Α | D | В | В |
| Approach Vol, veh/h | 3 11 | 399 | 7 7 1 | Y-110 | 99 | | | 149 | 755 | | 21 | |
| Approach Delay, s/veh | | 9.6 | | | 12.9 | | | 18.4 | | | 17.1 | |
| Approach LOS | | Α | | | В | | | В | | | В | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), | s 4 6 | 13.0 | 0.0 | 14.7 | 8.1 | 9.5 | 5.3 | 9.4 | | - | | |
| Change Period (Y+Rc), s | | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | | | | |
| Max Green Setting (Gma | | 19.0 | 5.0 | 18.0 | 6.0 | 18.0 | 5.0 | 18.0 | | | | |
| Max Q Clear Time (g c+l | | 2.1 | 0.0 | 5.1 | 4.5 | 2.3 | 2.3 | 3.5 | | | | |
| Green Ext Time (p_c), s | 0.0 | 0.0 | 0.0 | 1.4 | 0.0 | 0.0 | 0.0 | 0.3 | | | | |
| | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 | 0.0 | 0.0 | | | | |
| Intersection Summary | | | 40.0 | | | | | | | | | |
| HCM 6th Ctrl Delay | | | 12.3 | | | | | | | | | |
| HCM 6th LOS | | | В | | | | | | | | | |