## Exhibit E



## REVISED Hydrology Report - SHL Cathiard LLC

Includes: WinTR55 Modeling
March 1, 2021

## BACKGROUND

The project site is located at 1889 West Zinfandel Lane, CA (APN 027-100-037), in the foothills on the west side of Napa Valley. Watersheds for the project area are defined as follows:

- Watershed 1: 60.6 acres comprised of Woods, Woods-Grass Combo, Rangeland, existing vineyard, and a pond (TABLE 3). This watershed contains new development Blocks $M$ and $O$. There is a cut-of swale on the west side of the access road that diverts the majority of run-on water to the northern watershed boundary. A flow-line was defined that analyzed an existing swale and existing culvert and discharges to the east and flows north. The existing culvert is a 10 " smooth walled pipe, non-metallic, and assumed to be asbestos. Both boundaries of the watershed converge at the outlet, which is an unnamed tributary to the Napa River.
o SubWatershed 1 was included to analyze anticipated maximum inlet flows to the existing culvert and to size new and replacement culvert (TABLE 4).

Soil types encountered in the watersheds include the following:

- (166) Montara clay loam, 5-30\% slopes, HSG=D
- (110) Boomer-Forward-Felta complex, 30-50\% slopes, HSG=D
- (161) Maxwell clay, 2-9\% slopes, HSG=D
- (171) Pleasanton loam, 2-5\% slopes, HSG=C
- (139) Forward silt loam, 12-57\% slopes, HSG=C
- (140) Forward silt loam, 30-75\% slopes, HSG=C
- (183) Water

Soil types that are rated C Hydrologic Soil Group (HSG) are fringe areas that will not be altered during development. For simplicity in modeling (and a conservative measure), all areas of the watersheds were assumed to be rated D HSG.

## METHODOLOGY

This approach of this hydrology analysis is summarized below; it was designed to address a few different objectives and areas of interest. Please refer to Pre- and Post-Development TR55 maps for watershed definitions and flowline locations.

1. The pre- and post-development cover types were mapped and analyzed as a Cn comparison to evaluate the overall change in cover within Watershed \#1 (TABLE 3). This Cn analysis is sufficient to show no change in peak discharge in areas where no drainage changes are proposed, such as the northern portion of the watershed and Block M.
2. Two different flow lines were defined within Watershed \#1 where drainage changes are proposed
a. The first was defined to analyze impact of culvert installations across Block O. In this case, segment lengths and slopes remain constant and properties for Segments 3a and 3b were changed to reflect culvert upgrades (see TABLE 5 and TABLE 6). Segments were defined as follows:

TABLE 1 (Pre and Post) Culvert Analysis Flowlines

|  | PRE |  |  |  | POST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| \# | Segment | Length | Slope | \# | Segment | Length | Slope |
| 1 | Sheet Flow | 100 | 21 | 1 | Sheet Flow | 100 | 21 |
| 2 | Shallow Concentrated | 393 | 19 | 2 | Shallow Concentrated | 393 | 19 |
| 3a | Concentrated (swale) | 248 | 18 | 3 a | Concentrated (12" pipe) | 248 | 18 |
| 3b | Concentrated (10" pipe) | 264 | 9 | 3b | Concentrated (18" pipe) | 264 | 9 |
| 4 | Concentrated (channel) | 2136 | 6 | 4 | Concentrated (channel) | 2136 | 6 |
|  | Total Length | 3141 |  |  | Total Length | 3141 |  |

b. The second flowline was defined across the landslide repair area to assess the impact of a proposed diversion ditch at the top of Block O. In this case, the post-development flowlines are longer to divert flow away from the landslide repair area, but water ultimately ends up in the same reach. Segments were defined as follows and a new ID\# was assigned to each new segment geometry:

TABLE 2 (Pre and Post) Ditch Analysis Flowlines

| PRE |  |  |  | POST |  | Length | Slope |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | \# | Segment |  |  |
| \# | Segment | Length | Slope | 5 | Sheet Flow | 100 | 22 |
| 5 | Sheet Flow | 100 | 22 | 8 | Shallow Concentrated | 41 | 25 |
| 6 | Shallow Concentrated | 759 | 15 | 9 | Concentrated (ditch + woods) | 483 | 19 |
| 7 | Concentrated (channel) | 2025 | 7 | 10 | Concentrated (channel) | 2481 | 7 |
|  | Total Length | 2884 |  |  | Total Length | 3105 |  |

3. Finally, Subwatershed \#1 was defined to analyze the run-off volumes that may be entering the new proposed culverts described in Bullet 2.a, above. The area of postconstruction Subwatershed \#1 is reduced due to the installation of the diversion ditch described in Bullet 2.b, above. In this case, the total area of the subwatershed is reduced by 0.27 acres, but there is no change in weighted Cn (TABLE 4).

WinTR55 Land Use designations for each watershed were defined as follows.
TABLE 3 (PRE + POST) Cover Type for Watershed \#1

| Watershed \#1 |  |  |  |
| :--- | ---: | ---: | ---: |
| Landuse <br> (acres) | POST <br> (acres) | Cn |  |
| Impervious (Pond) | 2.13 | 2.13 | 98 |
| Existing Vineyard (fair) | 16.03 | 16.03 | 84 |
| New Vineyard (good) | - | 13.1 | 80 |
| Rangeland (fair) | 7.19 | - | 84 |
| Wood/Grass Combo (fair) | 4.23 | 0.8 | 82 |
| Woods (good) | 26.98 | 22.57 | 77 |
| Farmstead | 4.03 | 5.96 | 86 |
| Total acres | $\mathbf{6 0 . 5 9}$ | $\mathbf{6 0 . 5 9}$ |  |
| Weighted Cn |  | $\mathbf{8 1}$ | $\mathbf{8 1}$ |

TABLE 4 (PRE + POST) Cover Type for SubWatershed \#1

| SubWatershed \#1 |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | :---: | :---: | :---: | :---: |
| Landuse | PRE <br> (acres) | POST <br> (acres) | Cn |  |  |  |  |
| Existing Vineyard (fair) | 0.52 | 0.52 | 84 |  |  |  |  |
| New Vineyard (good) | - | 1.89 | 80 |  |  |  |  |
| Rangeland (fair) | 1.36 | - | 84 |  |  |  |  |
| Woods-Grass Combo (fair) | 0.02 | - | 82 |  |  |  |  |
| Woods (good) | 1.27 | 0.31 | 77 |  |  |  |  |
| Farmstead | 0.14 | 0.32 | 86 |  |  |  |  |
| Total acres |  |  |  |  | $\mathbf{3 . 3 1}$ | $\mathbf{3 . 0 4}$ |  |
| Weighted Cn |  |  |  |  |  |  |  |

Due to limitations in the number of concentrated flow segments that can be input to WinTR55, Segments 3.a and 3.b were consolidated for Pre-development (TABLE 5) and Post-Development (TABLE 6) conditions. Furthermore, it was discovered while revising this hydrology report that a 12 " and 18 " corrugated culvert is adequate to handle predicted storm flows while still reducing the post-development time of concentration (the original submission specified a 18 " and 24 " culvert, respectively).

TABLE 5 PRE-Development assumptions for Segments 3a + 3b (Culvert Analysis)

| Watershed 1: Segments 3.a and 3.b Existing Conditions Flow Velocity |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Channel | $\begin{gathered} d \\ (\mathrm{in}) \end{gathered}$ | Length <br> (ft) | slope | n | $\begin{gathered} \mathrm{A} \\ \left(\mathrm{ft}^{2}\right) \\ \hline \end{gathered}$ | $\begin{aligned} & \text { WP } \\ & (\mathrm{ft}) \end{aligned}$ | Velocity $(\mathrm{ft} / \mathrm{s})$ |
| (3.a) Swale | - | 248 | 18\% | 0.040 | 2.00 | 3.83 | 10 |
| (3.b) Smooth Asbestos Pipe | 10 | 264 | 9\% | 0.011 | 0.44 | 1.75 | 16.1 |
| Total Length |  | 512 |  |  | weighted ave |  | 13.1 |

TABLE 6 POST-Development assumptions for Segments 3a $+3 b$ (Culvert Analysis)

| Watershed 1: Segments 3.a and 3.b Post-Development Flow Velocity |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Channel | $\begin{gathered} d \\ \text { (in) } \end{gathered}$ | Length (ft) | slope | n | $\begin{gathered} \mathrm{A} \\ \left(\mathrm{ft}^{2}\right) \end{gathered}$ | WP <br> (ft) | Velocity (ft/s) |
| (3.a) Corrugated PE | 12 | 248 | 18\% | 0.025 | 0.63 | 2.09 | 11.3 |
| (3.b) Corrugated PE | 18 | 264 | 9\% | 0.025 | 1.42 | 3.14 | 10.5 |
| Total Length 512 |  |  |  |  | weighted ave |  | 10.9 |

TABLE 7 POST-Development assumptions for Segment 9 (Ditch Analysis)

"POST" conditions in the new vineyard areas will establish at least 80\% cover crop in all vineyard blocks, which qualifies as "good" hydrologic condition per the NRCS Engineering Fieldbook. In addition, the 10 " asbestos culvert will be upgraded to an $18^{\prime \prime}$ corrugated PE pipe and the inlet swale will be upgraded to a 12 " corrugated PE pipe. The outlet of the lower culvert will be improved to a pipe level spreader. Although the level spreader will further reduce flow velocities and disperse concentrated flow, it was not included in the hydrology analysis.

## RESULTS

Run-off potential in Watershed 1 from the Culvert Analysis flowline will decrease due to the drainage upgrades (TABLE 7):

TABLE 8 (Pre + Post) Peak Flow for Culvert Analysis Flowline

|  | Peak Flow (cfs) |  |  |  |
| :--- | :---: | :---: | :---: | :---: |
|  | $2-\mathrm{yr}$ | $10-\mathrm{yr}$ | $50-\mathrm{yr}$ | $100-\mathrm{yr}$ |
| Watershed - 1 (Existing) | 21.25 | 36.95 | 53.48 | 60.50 |
| Watershed - 1 (Future) | 21.19 | 33.32 | 48.27 | 54.65 |

TABLE 9 (Pre + Post) Peak Flow for Ditch Analysis Flowline

|  | Peak Flow (cfs) |  |  |  |
| :--- | :---: | :---: | :---: | :---: |
|  | $2-\mathrm{yr}$ | $10-\mathrm{yr}$ | $50-\mathrm{yr}$ | $100-\mathrm{yr}$ |
| Watershed - 1 (Existing) | 19.16 | 33.38 | 48.36 | 54.71 |
| Watershed - 1 (Future) | 19.13 | 33.32 | 48.27 | 54.65 |

Maximum peak flow to the culvert was calculated using SubWatershed 1 at the inlet to Segment 3.b (TABLE 8), which would be a conservatively high estimate for the inlet to culvert at Segment 3.a (see attached watershed map for segment locations). It is a small sub-watershed ( 3.3 acres) and there was no difference in existing versus post-development results, because the weighted Cn is the same and Tc was less than 0.1 and defaulted to their minimum value (0.1) for both cases.

TABLE 10

|  | Peak Flow (cfs) |  |  |
| :--- | :---: | :---: | :---: |
|  | $10-\mathrm{yr}$ | $25-\mathrm{yr}$ | $100-\mathrm{yr}$ |
| SubWatershed-1 (culvert) | 2.16 | 2.71 | 3.53 |

Figure 1 illustrates that the proposed culvert sizes (12 in and 18 in) are adequate to handle modeled peak flows.
SARAH PIStone
CPESC \#9225

## ATTACHMENTS

Figure 1: Pipe Discharge Capacity
Land Cover Map - Existing
Land Cover Map - Future
WinTR55 Results - Watershed 1: Existing (Culvert Analysis Flowline)
WinTR55 Results - Watershed 1: Future (Culvert Analysis Flowline)
WinTR55 Results - Watershed 1: Existing (Ditch Analysis Flowline)
WinTR55 Results - Watershed 1: Future (Ditch Analysis Flowline)
WinTR55 Results - SubWatershed 1: Existing
WinTR55 Results - SubWatershed 1: Future

Figure 3-2
Discharge Rates for ADS Single Wall Heavy Duty and Highway Pipe


Note: Based on a design Manning's " $n$ " described in Table 3-1 for respective diameter.
Pipe may not be available in all diameters shown.
Solid lines indicate pipe diameters. Dashed lines indicate approximate average velocity.
© ADS, Inc., July 2014

Figure 1 From ADS. Inc. Drainage Handbook, July 2014



## WinTR-55 Current Data Description

--- Identification Data ---

| User: | SPistone CULVERT ANALYSIS | Date: 11/30/20 |
| :---: | :---: | :---: |
| Project: | CATHIARD | Units: English |
| SubTitle: | Watershed 1 - PRE | Areal Units: Acres |
| State: | California |  |
| County: | Napa |  |
| Filename: | C:\Users\Sarah\OneDrive - LincolnA | Clients \Chateau Smith |

--- Sub-Area Data ---

| Name | Description | Reach | Area(ac) | RCN | Tc |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Watershed1 | Watershed | Outlet | 60.59 | 81 | . 177 |
| Total area | . 59 (ac) |  |  |  |  |

--- Storm Data --
Rainfall Depth by Rainfall Return Period

| $\begin{aligned} & 2-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 5-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 10-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 25-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 50-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 100-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & -\mathrm{Yr} \\ & (\mathrm{in}) \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.98 | 4.98 | 5.78 | 6.85 | 7.64 | 8.43 | . 0 |
| Storm Data Source: <br> Rainfall Distribution Type: <br> Dimensionless Unit Hydrograph: |  |  | User-provided custom storm data Type CA-1 |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  | rd> |  |  |  |

SPistone

CATHIARD
Watershed 1 - PRE Napa County, California

Watershed Peak Table

| Sub-Area | Peak Flow by Rainfall Return Period |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| or Reach | 2-Yr | 10-Yr | 50-Yr | 100-Yr |
| Identifier | (cfs) | (cfs) | (cfs) | (cfs) |
| SUBAREAS |  |  |  |  |
| Watershed1 | 21.25 | 36.95 | 53.48 | 60.50 |
| REACHES |  |  |  |  |
| OUTLET | 21.25 | 36.95 | 53.48 | 60.50 |






--- Sub-Area Data ---

| Name | Description | Reach | Area(ac) | RCN | Tc |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Watershed1 | Watershed | Outlet | 60.59 | 81 | . 179 |
| Total area: | 0.59 (ac) |  |  |  |  |

--- Storm Data --
Rainfall Depth by Rainfall Return Period

| $\begin{aligned} & 2-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 5-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 10-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 25-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 50-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 100-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & -\mathrm{Yr} \\ & (\mathrm{in}) \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.98 | 4.98 | 5.78 | 6.85 | 7.64 | 8.43 | . 0 |
| Storm Data Source: <br> Rainfall Distribution Type: <br> Dimensionless Unit Hydrograph: |  |  | User-provided custom storm data Type CA-1 <standard> |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |








```
--- Sub-Area Data ---
```

| Name | Description | Reach | Area(ac) | RCN | Tc |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Watershed1 | Watershed | Outlet | 60.59 | 81 | . 33 |
| Total area: | . 59 (ac) |  |  |  |  |

--- Storm Data --

## Rainfall Depth by Rainfall Return Period

| $\begin{aligned} & 2-Y r \\ & (i n) \end{aligned}$ | $\begin{aligned} & 5-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 10-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 25-Y r \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 50-Y r \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 100-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & -\mathrm{Yr} \\ & (\mathrm{in}) \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.98 | 4.98 | 5.78 | 6.85 | 7.64 | 8.43 | 0 |
| Storm Da | ce: |  | User-provided custom storm data |  |  |  |
| Rainfall | ibutio |  | Type CA-1 |  |  |  |
| Dimensio | Unit H | ph: | <standard> |  |  |  |

SPistone

CATHIARD
Watershed 1 - PRE Napa County, California

Watershed Peak Table

| Sub-Area | $\begin{array}{r} 2-\mathrm{Yr} \\ (\mathrm{cfs}) \end{array}$ | Flow $10-\mathrm{Yr}$ (cfs) | Rainfall Return Period |  |
| :---: | :---: | :---: | :---: | :---: |
| or Reach |  |  | 50-Yr | 100-Yr |
| Identifier |  |  | (cfs) | (cfs) |
| SUBAREAS |  |  |  |  |
| Watershed1 | 19.16 | 33.38 | 48.36 | 54.71 |
| REACHES |  |  |  |  |
| OUTLET | 19.16 | 33.38 | 48.36 | 54.71 |

Watershed 1 - PRE
WinTR-55, Version 1.00.10
Page 1
11/30/20
22:16:10



[^0]
--- Sub-Area Data ---

| Name | Description | Reach | Area(ac) | RCN | Tc |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Watershed1 | Watershed | Outlet | 60.59 | 81 | . 333 |
| Total area: | 0.59 (ac) |  |  |  |  |

--- Storm Data --

## Rainfall Depth by Rainfall Return Period

| $\begin{aligned} & 2-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 5-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 10-\mathrm{Yr} \\ & (\mathrm{in}) \end{aligned}$ | $\begin{aligned} & 25-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & 50-\mathrm{Yr} \\ & (\mathrm{in}) \end{aligned}$ | $\begin{aligned} & 100-\mathrm{Yr} \\ & \text { (in) } \end{aligned}$ | $\begin{aligned} & -Y r \\ & (i n) \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.98 | 4.98 | 5.78 | 6.85 | 7.64 | 8.43 | . 0 |
| Storm Data Source: <br> Rainfall Distribution Type: <br> Dimensionless Unit Hydrograph: |  |  | User-provided custom storm data Type CA-1 <standard> |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |




## Napa County, California

## Sub-Area Summary Table

| Sub-Area Identifier | Drainage Area (ac) | ```Time of Concentration (hr)``` | Curve Number | Receiving Reach | Sub-Area Description |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Watershed1 | 60.59 | 0.333 | 81 | Outlet | Watershed |
| Total Area: | 60.59 ( |  |  |  |  |

SPistone
CATHIARD
Watershed 1 - POST
Napa County, California
Sub-Area Time of Concentration Details

| Sub-Area Identifier/ | Flow Length (ft) | Slope (ft/ft) | Mannings's | End <br> Area (sq ft) | ```Wetted Perimeter (ft)``` | Velocity <br> (ft/sec) | Travel Time (hr) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Watershed1 |  |  |  |  |  |  |  |
| SHEET | 100 | 0.2200 | 0.800 |  |  |  | 0.214 |
| SHALLOW | 41 | 0.2500 | 0.050 |  |  |  | 0.001 |
| Table 7 --> CHANNEL | 483 |  |  |  |  | 7.500 | 0.018 |
| CHANNEL | 2481 | 0.0700 | 0.040 | 3.50 | 6.00 | 6.892 | 0.100 |
|  |  |  |  | Time of Concentration |  |  | . 333 |






[^0]:    

