# DOWNTOWN TAFT SPECIFIC PLAN LOCAL TRANSPORTATION ANALYSIS 

Draft: May 27, 2022

Prepared for:

City of Taft
Planning and Development Services Department
209 E. Kern Street
Taft, CA 93268

Prepared by:


Engineering Company

Job Number 19403-T

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# DOWNTOWN TAFT SPECIFIC PLAN <br> LOCAL TRANSPORTATION ANALYSIS 

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## INTRODUCTION

The following Local Transportation Analysis has been prepared for the Downtown Taft Specific Plan project in accordance with the Kern County Standards for Traffic Engineering (February 23, 2010) and the City of Taft General Plan Circulation Element (June 2010, Amended April 2017). The study evaluates the potential level of service (LOS) deficiencies and transportation improvements that may need to be considered in association with the traffic generated by the proposed Downtown Taft Specific Plan project. As also required, an Active Transportation and Public Transit Assessment is evaluated in this study. The site is generally bounded by the mid-block alley between Kern Street (State Route 33) and Lucard Street to the north, Front Street to the south, $10^{\text {th }}$ Street to the west, and State Route 33 (SR33/Westside Highway) to the east. Exhibit 1 shows the project vicinity map.

## PROJECT DESCRIPTION

The Downtown Taft Specific Plan covers approximately 212 acres and would allow for the development of up to 3,120 residential dwelling units (DU), and would expand the downtown district to develop up to 891,059 square-feet (SF) of commercial retail uses and up to $1,132,718$ square-feet of commercial office uses. In addition, up to 224,039 square-feet of public institutional uses and up to 229,281 square-feet of industrial uses would be developed within the Specific Plan.

Below is a breakdown of the proposed land use types within the Downtown Taft Specific Plan:

## Residential Uses

- Single-Family Detached Dwelling Units:
- Single-Family Attached Dwelling Units (Townhomes):
- Multi-Family Dwelling Units:


## Retail Uses

- Retail Services Building Area:
- Restaurants Building Area:
- Arts \& Entertainment Building Area:
- Accommodation Building Area


## Office Uses

- Office Services Building Area
- Medical Services Building Area

Public Administration Uses

- Public Administration Building Area
- Education Building Area

Industrial Uses

- Transportation/Warehousing Building Area
- Wholesale Building Area
$3, \mathbf{3 2 0} \mathrm{DU}$
665 DU
1,186 DU
1,269 DU
891,059 SF
695,038 SF
59,762 SF
71,715 SF
64,543 SF
1,132,718 SF
471,455 SF
661,262 SF
$\underline{\mathbf{2 2 4}, 039 \mathrm{SF}}$
116,500 SF
107,539 SF
229,281 SF
170,457 SF
58,824 SF


Primary access to the Downtown Taft Specific Plan would be provided from Kern Street (SR-33), East Kern Street, Westside Highway (SR-33), $10^{\text {th }}$ Street and $6^{\text {th }}$ Street. Local access would be provided from all north-south streets intersecting Kern Street (SR-33) between ${ }^{\text {st }}$ Street and $10^{\text {th }}$ Street, Front Street, and Supply Row.

Exhibit 2 shows the Downtown Taft Specific Plan preferred land use plan.

## STUDY AREA AND ANALYSIS METHODOLOGY

## Project Study Area

As agreed upon with the City of Taft Planning and Development Services staff, the following intersections and roadway segments are included in the project study area, which are also illustrated in Exhibit 3:

## Study Intersections

1. Kern Street (SR-33) $/ 10^{\text {th }}$ Street
2. Kern Street (SR-33) / $6^{\text {th }}$ Street
3. Kern Street (SR-33) / $4^{\text {th }}$ Street
4. Kern Street (SR-33) / E. Kern Street / 1 ${ }^{\text {st }}$ Street (5-legged intersection)
5. Center Street $/ 10^{\text {th }}$ Street
6. Center Street / Westside Highway (SR-33)

## Study Roadway Segments

1. Kern Street (SR-33) between $10^{\text {th }}$ Street and $6^{\text {th }}$ Street
2. Kern Street (SR-33) between $6^{\text {th }}$ Street and $1^{\text {st }}$ Street
3. Westside Highway (SR-33) between $1^{\text {st }}$ Street and Center Street
4. Westside Highway (SR-33) between Center Street and Main Street
5. Center Street between $10^{\text {th }}$ Street and $6^{\text {th }}$ Street
6. Center Street between $6^{\text {th }}$ Street and Westside Highway
7. Main Street between $10^{\text {th }}$ Street and $6^{\text {th }}$ Street
8. Main Street between $6^{\text {th }}$ Street and Westside Highway
9. $10^{\text {th }}$ Street between Kern Street (SR-33) and Center Street
10. $6^{\text {th }}$ Street between Kern Street (SR-33) and Center Street

## Analysis Methodology

As agreed upon with the City of Taft Planning and Development Services staff, the following scenarios were evaluated in this Local Transportation Analysis:

- Existing Conditions: This scenario reflects the conditions on the ground at the time the traffic count data was obtained (March 2022).
- Horizon Year 2042 With Project Conditions: This scenario reflects the Horizon Year 2042 traffic volumes with the buildout of the Downtown Taft Specific Plan project, based on the model volumes generated by the Kern Council of Governments (Kern COG) regional travel demand model with the proposed Specific Plan land uses.




## Intersection Analysis Methodology

Intersection operations were determined at the study area intersections for the weekday AM and PM peak hours. The AM peak hour intersection analysis evaluates LOS during the hour with the highest vehicular traffic between 7:00 AM and 9:00 AM. The PM peak hour intersection analysis evaluates LOS during the hour with the highest vehicular traffic between 4:00 PM and 6:00 PM.

The Level of Service (LOS) for signalized intersections was analyzed using the methodologies described in Chapter 19 of the $6^{\text {th }}$ Edition Highway Capacity Manual (HCM 6). The LOS for signalized intersections is defined in terms of control delay, which is made up of several factors that relate to right-of-way control, geometrics and traffic volumes. The signalized intersection analysis also considers intersection spacing and coordination.

The LOS for two-way and all-way stop controlled intersections was calculated using the methodologies described in Chapters 20 and 21 of the $6^{\text {th }}$ Edition HCM. The LOS for a two-way stop controlled intersection is determined by the computed control delay for each minor street movement and major street left-turns, and not for the intersection as a whole. The LOS reported reflects the highest delay and associated LOS for an individual movement, typically occurring on the stop controlled approach.

The computerized analysis of signalized and unsignalized intersection operations was performed utilizing the Synchro 11 traffic analysis software. The Synchro 11 software supports the HCM-6 methodologies for signalized and stop controlled intersections and was utilized to produce the analysis results.

The HCM-6 methodology for two-way stop controlled intersections within the Synchro 11 software program does not accept analysis of a five-legged intersection. Therefore, the five-legged SR-33/East Kern Street/ $1^{\text {st }}$ Street intersection was analyzed using the HCM-6 methodology within the PTV Vistro Version 2021 software program.

Signal timing data and parameters such as cycle lengths, splits, clearance intervals, etc. were obtained from the current signal timing sheets provided by Caltrans District 6 and input into the Synchro software. Synchro reports delays, which correspond to a particular LOS, to describe the overall operation of an intersection.

The criteria for the LOS grade designations are provided in Table 1. LOS provides a quick overview of how well an intersection is performing. Within the City of Taft, LOS D or better is considered acceptable for all signalized and unsignalized intersections during the peak hours.

## Roadway Segment Analysis Methodology

Roadway segments were analyzed based on the volume-to-capacity (v/c) ratios and the City's daily LOS capacity thresholds per Table 4.4-4 (Roadway Segment Daily Volume Thresholds) of the City of Taft General Plan Draft Environmental Impact Report (July 2009). The City of Taft considers LOS D or better to be acceptable for daily roadway segment operations. Table 2 presents the roadway segment capacity and LOS thresholds utilized by the City of Taft.

TABLE 1
LOS CRITERIA FOR INTERSECTIONS

| HCM: CONTROL DELAY (SEC/VEH) <br> ICU: VOLUME/CAPACITY (V/C) |  |  | DESCRIPTION |
| :---: | :---: | :---: | :--- |
| LOS | SIGNALIZED <br> INTERSECTIONS | UNSIGNALIZED <br> INTERSECTIONS |  |
| A | $\leq 10$ seconds | $\leq 10$ seconds | Operations with very low delay and most <br> vehicles do not stop. |
| B | $>10$ and $\leq 20$ seconds | $>10$ and $\leq 15$ seconds | Operations with good progression but with <br> some restricted movements. |
| C | $>20$ and $\leq 35$ seconds | $>15$ and $\leq 25$ seconds | Operations where a significant number of <br> vehicles are stopping with some backup and <br> light congestion. |
| D | $>35$ and $\leq 55$ seconds | $>25$ and $\leq 35$ seconds | Operations where congestion is noticeable, <br> longer delays occur, and many vehicles <br> stop. The proportion of vehicles not <br> stopping declines. |
| E | $>55$ and $\leq 80$ seconds | $>35$ and $\leq 50$ seconds | Operations where there is significant <br> delay, extensive queuing, and poor <br> progression. |
| F | $>80$ seconds | $>50$ seconds | Operations that are unacceptable to most <br> drivers, when the arrival rates exceed the <br> capacity of the intersection. |

Source: $6^{\text {th }}$ Edition Highway Capacity Manual. City of Taft General Plan DEIR (July 2009).

TABLE 2
LOS CRITERIA FOR ROADWAY SEGMENTS

| Facility Type | Number of Lanes | Daily Volume Threshold |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LOS A | LOS B | LOS C | LOS D | LOS E |
| Residential | 2 | 600 | 1,200 | 2,000 | 3,000 | 4,500 |
| Residential collector with frontage | 2 | 1,600 | 3,200 | 4,800 | 6,400 | 8,000 |
| Residential collector without frontage | 2 | 6,000 | 7,000 | 8,000 | 9,000 | 10,000 |
|  | 2 | 9,000 | 10,000 | 12,000 | 13,500 | 15,000 |
| Arterial, low access control | 2 | 10,800 | 12,600 | 14,400 | 16,200 | 18,000 |
|  | 4 | 18,000 | 21,000 | 24,000 | 27,000 | 30,000 |
|  | 6 | 27,000 | 31,500 | 36,000 | 40,500 | 45,000 |
| Arterial, moderate access control | 2 | 12,000 | 14,000 | 16,000 | 18,000 | 20,000 |
|  | 4 | 21,600 | 25,200 | 28,800 | 32,400 | 36,000 |
|  | 6 | 32,400 | 37,800 | 43,200 | 48,600 | 54,000 |
| Arterial, high access control | 4 | 24,000 | 28,000 | 32,000 | 36,000 | 40,000 |
|  | 6 | 36,000 | 42,000 | 48,000 | 54,000 | 60,000 |
| Freeway | 4 | 60,000 | 70,000 | 80,000 | 90,000 | 100,000 |
|  | 6 | 90,000 | 105,000 | 120,000 | 135,000 | 150,000 |
| Expressway | 2 | 8,100 | 13,300 | 18,900 | 24,300 | 27,000 |
|  | 4 | 16,200 | 26,500 | 37,800 | 48,600 | 54,000 |
| Rural, 2-lane highway | 2 | 2,400 | 4,800 | 7,900 | 13,500 | 22,900 |
| Rural, 2-lane road, paved shoulders | 2 | 2,200 | 4,300 | 7,100 | 12,200 | 20,000 |
| Rural, 2-lane road, no shoulders | 2 | 1,800 | 3,600 | 5,900 | 10,100 | 17,000 |

Source: City of Taft General Plan DEIR (July 2009).

## EXISTING TRANSPORTATION CONDITIONS

## Existing Roadway Network

The following is a brief description of roadways within the study area as identified in the City of Taft General Plan (Amended April 2017):

Kern Street/State Route 33 (SR-33) is classified as an Arterial Highway and extends from the northwest to the southeast boundaries of the City of Taft. The roadway is currently constructed with two travel lanes in each direction from approximately 500 feet west of Cascade Place to the intersection with $1^{\text {st }}$ Street and East Kern Street. Southeast of $1^{\text {st }}$ Street, SR-33 is reduced to one travel lane in each direction. A striped center two-way left-turn lane is currently provided along Kern Street/SR-33 from 500 feet of Cascade Place to $10^{\text {th }}$ Street. No left-turn lanes are provided Kern Street/SR-33 southeast of $9^{\text {th }}$ Street within the Specific Plan area, and opposing travel lanes are separated by a striped double yellow line. On-street parking along Kern Street/SR-33 is generally permitted between $10^{\text {th }}$ Street and $1^{\text {st }}$ Street, but is prohibited along SR-33 southeast of $1^{\text {st }}$ Street. Bicycle facilities are currently not provided along Kern Street/SR-33. The posted speed limit on Kern Street/SR-33 is 35 mph between $10^{\text {th }}$ Street and $1^{\text {st }}$ Street, and is 40 mph southeast of $1^{\text {st }}$ Street.
$\mathbf{1 0}^{\text {th }}$ Street is classified as an Arterial and extends from Ash Street to A Street/Oak Street within the City of Taft. The roadway is currently constructed with two travel lanes in each direction and a striped center two-way left-turn lane from Ash Street to Main Street. $10^{\text {th }}$ Street narrows from four travel lanes to three travel lanes with a center two-way left-turn lane between Main Street and Front Street. South of Front Street, $10^{\text {th }}$ Street is striped with one travel lane in each direction without a center two-way left-turn lane. On-street parking is generally prohibited along $10^{\text {th }}$ Street from Ash Street to Front Street. South of Front Street, on-street parking is generally permitted along the east side of the street. Bicycle facilities are currently not provided along $10^{\text {th }}$ Street. The posted speed limit on $10^{\text {th }}$ Street is 35 mph .
$\mathbf{6}^{\text {th }}$ Street is classified as a Collector and extends from Ash Street to Oak Street within the City of Taft. The roadway is currently constructed with two travel lanes in each direction from Ash Street to Main Street. $6^{\text {th }}$ Street narrows from four travel lanes to two travel lanes south of Main Street. The opposing travel lanes are separated by a striped double yellow lines. No left-turn lanes are provided along $6^{\text {th }}$ Street. On-street parking is generally permitted along $6{ }^{\text {th }}$ Street. South of Front Street, on-street parking is generally permitted along the east side of the street. Bicycle facilities are currently not provided along $6^{\text {th }}$ Street. The posted speed limit on $6{ }^{\text {th }}$ Street is 35 mph .

Center Street is classified as a Local Street and extends from $10^{\text {th }}$ Street to SR-33 within the City of Taft. The roadway is currently constructed with one travel lane in each direction, and the opposing travel lanes are separated by a dashed yellow line. No left-turn lanes are provided along Center Street. Onstreet parking is generally permitted on both sides of Center Street, and angled parking spaces are provided along one or both sides of the street between $10^{\text {th }}$ Street and $2^{\text {nd }}$ Street. Bicycle facilities are currently not provided along Center Street. The posted speed limit on Center Street is 25 mph .

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Main Street is classified as a Local Street and extends from $10^{\text {th }}$ Street to SR-33 within the City of Taft. The roadway is currently constructed with one travel lane in each direction, and the opposing travel lanes are separated by a dashed yellow line. No left-turn lanes are provided along Main Street. On-street parking is generally permitted on one or both sides of Main Street between $10^{\text {th }}$ Street and $2^{\text {nd }}$ Street. Onstreet parking is generally prohibited along Main Street between $2^{\text {nd }}$ Street and SR-33, except for a short section between 300 feet and 600 feet west of SR-33 where on-street parking is permitted along the north side of the roadway. Bicycle facilities are currently not provided along Main Street between $10^{\text {th }}$ Street and $2^{\text {nd }}$ Street. A Class II bicycle lane is currently provided in each direction of travel along Main Street between $2^{\text {nd }}$ Street and SR-33. The posted speed limit on Main Street is 25 mph between $10^{\text {th }}$ Street and $2^{\text {nd }}$ Street, and is 35 mph between $2^{\text {nd }}$ Street and SR-33.

Exhibit 4 illustrates the existing (2022) transportation conditions within the study area as described above.

## Existing Transportation Volumes

Existing vehicular, pedestrian and bicycle volumes at the following six (6) study intersections were obtained from counts conducted by Metro Traffic Data on Tuesday, March 15, 2022 during the AM (7-9) and PM (4-6) peak periods while local schools were in regular session:

1. Kern Street (SR-33) $/ 10^{\text {th }}$ Street
2. Kern Street (SR-33) $/ 6^{\text {th }}$ Street
3. Kern Street (SR-33) / $4^{\text {th }}$ Street
4. Kern Street (SR-33) / E. Kern Street / 1 ${ }^{\text {st }}$ Street (5-legged intersection)
5. Center Street $/ 10^{\text {th }}$ Street
6. Center Street / Westside Highway (SR-33)

Additionally, machine counts were collected on Tuesday, March 15, 2022 on the following 10 roadway segments listed below in order to document the daily vehicular traffic volumes for a 24 -hour period:

1. Kern Street (SR-33) between $10^{\text {th }}$ Street and $6^{\text {th }}$ Street
2. Kern Street (SR-33) between $6^{\text {th }}$ Street and $1^{\text {st }}$ Street
3. Westside Highway (SR-33) between $1^{\text {st }}$ Street and Center Street
4. Westside Highway (SR-33) between Center Street and Main Street
5. Center Street between $10^{\text {th }}$ Street and $6^{\text {th }}$ Street
6. Center Street between $6^{\text {th }}$ Street and Westside Highway
7. Main Street between $10^{\text {th }}$ Street and $6^{\text {th }}$ Street
8. Main Street between $6^{\text {th }}$ Street and Westside Highway
9. $10^{\text {th }}$ Street between Kern Street (SR-33) and Center Street
10. $6^{\text {th }}$ Street between Kern Street (SR-33) and Center Street

Exhibit 5 shows the existing intersection turning movement volumes and average daily traffic (ADT) volumes within the study area. Appendix A contains the vehicular, pedestrian and bicycle count data.



NOT TO SCALE

## Existing Intersection Operations

The existing intersection operations results are based on existing turning movement volumes collected, existing intersection geometry and existing signal timing.

Table 3 shows the existing conditions intersection operations during the peak hours. Appendix B contains the existing conditions intersection operations worksheets.

TABLE 3
EXISTING INTERSECTION OPERATIONS

| INTERSECTION |  |  |  | EXISTING (2022) |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | DELAY ${ }^{1}$ | LOS |
| 1 | Kern Street (SR-33) / 10th Street | AM peak <br> PM peak | (S) OVERALL <br> (S) OVERALL | $\begin{aligned} & 27.6 \\ & 26.7 \\ & \hline \end{aligned}$ | C |
| ${ }^{2}$ | Kern Street (SR-33) / 6th Street | AM peak <br> PM peak | (AWSC) OVERALL (AWSC) OVERALL | $\begin{aligned} & 15.6 \\ & 12.0 \\ & \hline \end{aligned}$ | C |
| 3 | Kern Street (SR-33) / 4th Street | AM peak <br> PM peak | (AWSC) OVERALL (AWSC) OVERALL | $\begin{aligned} & 12.0 \\ & 10.9 \end{aligned}$ | B |
| 4 | Kern Street (SR-33) / E. Kern Street / 1st S | ret ${ }^{2}$ <br> AM peak <br> PM peak | $\begin{gathered} \text { (TWSC) } \\ \text { NBL } \\ \text { (TWSC) } \\ \text { NBT } \\ \hline \end{gathered}$ | $\begin{aligned} & 13.9 \\ & 16.0 \\ & \hline \end{aligned}$ | B |
| 5 | Center Street / Westside Highway (SR-33) | AM peak <br> PM peak | (TWSC) EBL (TWSC) WBL | $\begin{aligned} & 9.1 \\ & 9.8 \end{aligned}$ | A |
| 6 | Center Street / 10th Street | AM peak <br> PM peak | (OWSC) WBL (OWSC) WBL | $\begin{aligned} & 12.2 \\ & 11.6 \\ & \hline \end{aligned}$ | B |

Footnotes: S = Signalized; AWSC = All-Way-Stop Controlled; TWSC = Two-Way-Stop Controlled; OWSC = One-Way-Stop Controlled
${ }^{1}$ Delay is measured in seconds per vehicle.
${ }^{2}$ PTV Vistro software program was used as interface to HCM-6 methodology to analyze the 5-legged Kern Street (SR-33) / E. Kern Street / 1st Street because the Synchro 11 software program does not accept analysis of 5-legged intersections.

As shown in Table 3, all study intersections currently operate at LOS C or better during the AM and PM peak hours.

## Existing Conditions Roadway Segment Analysis

The existing roadway level of service results are based on existing daily traffic volumes collected and functional classification roadway capacity. Table 4 summarizes the roadway segment capacity analysis results under Existing Conditions. As shown in Table 4, all study roadway segments currently operate at LOS B or better.

## PROJECT TRIP GENERATION

The trip generation for the Downtown Taft Specific Plan project was calculated based on the published Institute of Transportation Engineers (ITE) $11^{\text {th }}$ Edition Trip Generation Manual (September 2021) trip rates for the proposed land uses.

The specific ITE trip generation rates for the proposed residential land use types were applied, but due to the greater uncertainty of what specific types of retail or office uses would ultimately occupy the Specific Plan area, the "Shopping Center Over 150K" (ITE Land Use Code 820) trip rate was applied to all uses under the "Retail" category. In addition, the "Office Park" (ITE Land Use Code 750) trip rate was applied to all uses under the "Office" category.

Although 661,262 square-feet of the office uses is proposed as "medical services", the only medical office trip rate in the $11^{\text {th }}$ Edition ITE Trip Generation Manual is the "Medical-Dental Office Building Stand-Alone" (ITE Land Use Code 720), which is intended to only be used for a stand-alone medical office building. Applying the ITE Land Use Code 720 trip rate to the 661,262 square-feet of medical services use would result in an artificially high trip generation compared to the other proposed uses, and therefore the general "Office Park" ITE Land Use Code 750) trip rate was applied to the 661,262 squarefeet of proposed medical services.

The average ITE trip rates were applied to the proposed residential land use types, but for all nonresidential uses, fitted curve equations were applied, where available, to calculate the trip generation of the retail, office, public administration and industrial uses.

Table 5 shows the ITE trip generation rates that were applied to the proposed Downtown Taft Specific Plan land uses, and Table 6 presents the trip generation of the buildout of the Downtown Taft Specific Plan. Appendix C contains the $11^{\text {th }}$ Edition ITE Trip Generation Manual (September 2021) trip rate sheets.

As shown in Table 6, the Downtown Taft Specific Plan project is forecast to generate a net total of 58,925 trips per day, with a net total of 3,862 trips occurring during the AM peak hour ( $2,350 \mathrm{IN}, 1,512$ OUT), and a net total of 5,145 trips occurring during the PM peak hour (2,109 IN, 3,036 OUT).

The net total trip generation of the Downtown Taft Specific Plan project accounts for an internal capture trip reduction that was calculated using the NCHRP Report 684 Internal Capture Estimation Tool, which is also shown in Table 6. The internal capture calculation worksheets are also provided in Appendix C.

TABLE 4
EXISTING CONDITIONS ROADWAY SEGMENT ANALYSIS

| ROADWAY SEGMENT |  | Classification ${ }^{1}$ | \# of <br> Lanes | Daily Capacity ${ }^{1}$ | EXISTING (2022) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Volume |  |  | LOS |
| 1 | Kern Street (SR-33) between 10th Street and 6th Street |  | Arterial Highway | 4 | 30,000 | 9,278 | A |
| 2 | Kern Street (SR-33) between 6th Street and 1st Street | Arterial Highway | 4 | 30,000 | 7,520 | A |
| 3 | Westside Highway (SR-33) between 1st Street and Center Street | Arterial Highway | 2 | 18,000 | 2,929 | A |
| 4 | Westside Highway (SR-33) between Center Street and Main Street | Arterial Highway | 2 | 18,000 | 3,187 | A |
| 5 | Center Street between 10th Street and 6th Street | Collector | 2 | 8,000 | 2,360 | B |
| 6 | Center Street between 6th Street and Westside Highway (SR-33) | Collector | 2 | 8,000 | 331 | A |
| 7 | Main Street between 10th Street and 6th Street | Collector | 2 | 8,000 | 1,223 | A |
| 8 | Main Street between 6th Street and Westside Highway (SR-33) | Collector | 2 | 8,000 | 1,443 | A |
| 9 | 10th Street between Kern Street (SR-33) and Center Street | Arterial | 4 | 30,000 | 9,390 | A |
| 10 | 6th Street between Kern Street (SR-33) and Center Street | Collector | 4 | 15,000 | 2,637 | A |

Footnotes:
${ }^{1}$ Roadway classification and daily capacity taken from Table 4.4-4 (Roadway Segment Daily Volume Thresholds) of the City of Taft General Plan Draft Environmental Impact Report (July 2009)

TABLE 5
ITE TRIP GENERATION RATES

| Land Use | Unit | Daily <br> Rate (per unit) | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Rate | Inbound $(\% \mathbf{A M})$ | Outbound $(\% \mathbf{A M})$ | Rate | Inbound (\% PM) | Outbound (\% PM) |
| Single-Family Detached (LU Code 210) | DU | 9.43 | 0.70 | 26\% | 74\% | 0.94 | 63\% | 37\% |
| Single-Family Attached (LU Code 215) | DU | 7.20 | 0.48 | 31\% | 69\% | 0.57 | 57\% | 43\% |
| Multi-Family Low-Rise (LU Code 220) | DU | 6.74 | 0.40 | 24\% | 76\% | 0.51 | 63\% | 37\% |
| Shopping Center $>150 \mathrm{k}$ (LU Code 820) | KSF | $\begin{gathered} \mathrm{T}=\underset{5863.73^{\mathrm{a}}}{26.11(\mathrm{X})+} \end{gathered}$ | $\begin{aligned} \mathrm{T}= & 0.59(\mathrm{X})+ \\ & 133.55^{\mathrm{a}} \end{aligned}$ | 62\% | 38\% | $\begin{aligned} \operatorname{Ln}(\mathrm{T}) & =0.72 \operatorname{Ln}(\mathrm{X}) \\ & +3.02^{\mathrm{a}} \end{aligned}$ | 48\% | 52\% |
| Office Park (LU Code 750) | KSF | $\begin{aligned} \operatorname{Ln}(\mathrm{T}) & =0.89 \operatorname{Ln}(\mathrm{X}) \\ & +3.10^{\mathrm{a}} \end{aligned}$ | $\begin{gathered} \mathrm{T}=0.94(\mathrm{X})+ \\ 194.06^{\mathrm{a}} \end{gathered}$ | 89\% | 11\% | $\begin{gathered} \mathrm{T}=1.26(\mathrm{X})+ \\ 20.98^{\mathrm{a}} \end{gathered}$ | 14\% | 86\% |
| Public Administration (LU Code 730) | KSF | 22.59 | 3.34 | 75\% | 25\% | $\begin{aligned} \operatorname{Ln}(\mathrm{T}) & =0.97 \operatorname{Ln}(\mathrm{X}) \\ & +0.62^{\mathrm{a}} \end{aligned}$ | 25\% | 75\% |
| Warehousing (LU Code 150) | KSF | $\begin{gathered} \mathrm{T}=1.58(\mathrm{X})+ \\ 38.29^{\mathrm{a}} \end{gathered}$ | $\begin{gathered} \mathrm{T}=0.12(\mathrm{X})+ \\ 23.62^{\mathrm{a}} \end{gathered}$ | 77\% | 23\% | $\begin{aligned} & \mathrm{T}= 0.12(\mathrm{X})+ \\ & 26.48^{\mathrm{a}} \end{aligned}$ | 28\% | 72\% |

Footnotes:
Source: ITE Trip Generation Manual, 11th Edition (2021)
DU = Dwelling Unit; KSF = Thousand Square-Feet
${ }^{\text {a }}$ Fitted curve equation provided to calculate the trip generation.

TABLE 6
PROJECT TRIP GENERATION

| Land Use | Size | Unit | Daily <br> Trips | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Total | Inbound | Outbound | Total | Inbound | Outbound |
| Single-Family Detached (LU Code 210) | 665 | DU | 6,271 | 466 | 121 | 345 | 625 | 394 | 231 |
| Single-Family Attached (LU Code 215) | 1,186 | DU | 8,539 | 569 | 176 | 393 | 676 | 385 | 291 |
| Multi-Family Low-Rise (LU Code 220) | 1,269 | DU | 8,553 | 508 | 122 | 386 | 647 | 408 | 239 |
| Retail (LU Code 820) | 891.059 | KSF | 29,129 | 659 | 409 | 250 | 2,726 | 1,308 | 1,418 |
| Office Park (LU Code 750) | 1132.718 | KSF | 11,601 | 1,259 | 1,121 | 138 | 1,448 | 203 | 1,245 |
| Public Administration (LU Code 730) | 224.039 | KSF | 5,061 | 748 | 561 | 187 | 355 | 89 | 266 |
| Warehousing (LU Code 150) | 229.281 | KSF | 401 | 51 | 39 | 12 | 54 | 15 | 39 |
| Subtotal Project Trips |  |  | 69,555 | 4,260 | 2,549 | 1,711 | 6,531 | 2,802 | 3,729 |
| Internal Capture Trip Reduction ${ }^{\text {a }}$ |  |  | -10,630 | -398 | -199 | -199 | -1,386 | -693 | -693 |
| NET TOTAL PROJECT TRIPS |  |  | 58,925 | 3,862 | 2,350 | 1,512 | 5,145 | 2,109 | 3,036 |

Footnotes:
Source: ITE Trip Generation Manual, 11th Edition (2021)
DU = Dwelling Unit; KSF = Thousand Square-Feet
${ }^{\text {a }}$ Internal capture was calculated using the NCHRP Report 684 Internal Capture Estimation Tool (see Appendix C).

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## HORIZON YEAR 2042 WITH PROJECT TRANSPORTATION CONDITIONS

Horizon Year 2042 With Project conditions reflect the buildout of the Downtown Taft Specific Plan and City of Taft General Plan. The Kern Council of Governments (Kern COG) Year 2042 regional travel demand model was used to develop the Horizon Year 2042 With Project traffic volumes in the study area. RICK provided Kern COG the proposed land uses to input into the five Travel Analysis Zones (TAZs) that make up the Downtown Taft Specific Plan area in the Kern COG model, which are listed below:

- TAZ 1805
- TAZ 1806
- TAZ 1816
- TAZ 1817
- TAZ 1819

Kern COG conducted a model run with the proposed land use changes using the Year 2042 regional travel demand model and provided RICK with a model plot that illustrate the Horizon Year 2042 With Specific plan daily volumes for the Greater Taft Subarea. A copy of the Kern COG Horizon Year 2042 With Specific Plan Model ADT plot is contained in Appendix D.

## Horizon Year 2042 With Project Conditions Roadway Network

The Horizon Year 2042 With Project roadway network is based on the current Kern COG 2018 Regional Transportation Plan (RTP) and the buildout of the City of Taft General Plan Circulation Element. Neither the Kern COG 2018 RTP nor the City of Taft General Plan Circulation Element include any future roadway improvements within the Downtown Taft Specific Plan area. Therefore, the Horizon Year 2042 roadway network is essentially the same as existing conditions.

## Horizon Year 2042 With Project Conditions Traffic Volumes

The Horizon Year 2042 With Project traffic volumes were derived based on the forecast growth in traffic in the Kern COG regional travel demand model. Growth factors were developed based on the forecast traffic growth from Baseline Year 2020 to Horizon Year 2042 with the Downtown Taft Specific Plan. These growth factors were applied to the existing conditions (Year 2022) daily traffic volumes to reflect 20 years of traffic growth from 2022 to 2042.

The Horizon Year 2042 With Project daily traffic volumes derived from the model growth in traffic were post-processed to develop the intersection turning movement volumes during the AM and PM peak hours. Adjustments were made where appropriate to reflect changes in traffic patterns and growth on all legs of the study intersections. The Horizon Year 2042 With Project intersection post-processing worksheets are also provided in Appendix D. Exhibit 6 illustrates the Horizon Year 2042 With Project traffic volumes at the study intersections and roadway segments with the buildout of the Downtown Taft Specific Plan.

Project-specific daily and AM/PM peak hour trips associated with the proposed Downtown Taft Specific Plan land uses were calculated based on the changes in the daily traffic volumes between the Horizon Year 2042 baseline (without project) volumes from the Kern COG regional travel demand model and the Horizon Year 2042 With Specific Plan volumes from the model run conducted by Kern COG. The Horizon Year 2042 baseline volumes were also post-processed to derive the Horizon Year 2042 baseline AM/PM peak hour turning movement volumes, and the project-specific AM/PM peak hour trips were calculated by subtracting the Horizon Year 2042 baseline volumes from the Horizon Year 2042 With Project volumes. Exhibit 7 illustrates the project-specific daily and AM/PM peak hour trips with the buildout of the Downtown Taft Specific Plan.



## Horizon Year 2042 With Project Conditions Intersection Operations

Table 7 displays the operational analysis results for the study intersections under Horizon Year 2042 conditions with the buildout of the Downtown Taft Specific Plan. Appendix E contains the Horizon Year 2042 With Project intersection analysis worksheets.

TABLE 7
HORIZON YEAR 2042 WITH PROJECT INTERSECTION OPERATIONS


[^0]As shown in Table 7, the following intersections are forecast to operate at a deficient LOS E or F during the AM and PM peak hours under Horizon Year 2042 conditions with the buildout of the Downtown Taft Specific Plan:

- Kern Street (SR-33) / 6th Street (AM: LOS F; PM: LOS E)
- Kern Street (SR-33) / 4th Street (AM/PM: LOS F)
- Kern Street (SR-33) / E. Kern Street / 1st Street (AM: LOS E; PM: LOS F)


## Horizon Year 2042 With Project Conditions Roadway Segment Analysis

Table 8 summarizes the roadway segment capacity analysis results under Horizon Year 2042 conditions with the buildout of the Downtown Taft Specific Plan. As shown in Table 8, all study roadway segments are forecast to operate at LOS D or better under Horizon Year 2042 conditions with the buildout of the Downtown Taft Specific Plan.

## ACTIVE TRANSPORTATION AND PUBLIC TRANSIT ASSESSMENT

## Existing Pedestrian Network

Sidewalks are generally provided along both sides of most roadways within the Specific Plan area. Below are descriptions of the existing pedestrian facilities along key roadways within the Specific Plan:

## Kern Street/West Side Highway (SR-33)

Sidewalk widths range from 12 feet to 15 feet along both sides of Kern Street (SR-33) between $10^{\text {th }}$ Street and $8^{\text {th }}$ Street. Sidewalks along Kern Street (SR-33) between $8^{\text {th }}$ Street and $1^{\text {st }}$ Street are primarily non-contiguous on both sides of the roadway and are approximately five (5) feet in width. Several wide sections of sidewalk 12-15 feet in width are also provided along Kern Street (SR-33) between $8^{\text {th }}$ Street and $1^{\text {st }}$ Street. No pedestrian facilities are provided along West Side Highway (SR-33) southwest of the Kern Street (SR-33)/East Kern Street/ $/{ }^{\text {st }}$ Street intersection.

Pedestrian crossings are provided at most intersections along the Kern Street (SR-33) corridor between $10^{\text {th }}$ Street and $1^{\text {st }}$ Street. Signalized pedestrian crossings with high-visibility ladder crosswalks are provided across all four legs of the Kern Street (SR-33)/ $10^{\text {th }}$ Street intersection. There are also several uncontrolled pedestrian crossings across Kern Street (SR-33) where high-visibility ladder crosswalks are provided and solar-powered LED enhanced pedestrian crossing signs with push-button activation, which are provided at the following intersections: Kern Street (SR-33) $/ 8^{\text {th }}$ Street, Kern Street (SR-33) $/ 7^{\text {th }}$ Street, Kern Street (SR-33) $/ 5^{\text {th }}$ Street, Kern Street (SR-33) $/ 3^{\text {rd }}$ Street, Kern Street (SR-33) $/ 2^{\text {nd }}$ Street, and Kern Street (SR-33)/East Kern Street/1 ${ }^{\text {st }}$ Street. High-visibility ladder crosswalks are also provided across all four legs of the all-way-stop controlled intersections of Kern Street (SR-33)/6 ${ }^{\text {th }}$ Street and Kern Street (SR-33) $/ 4^{\text {th }}$ Street.

## Center Street

Contiguous sidewalks treated with pavers are provided along both sides of Center Street between $10^{\text {th }}$ Street and $2^{\text {nd }}$ Street adjacent to primarily commercial uses, and range between 6 feet and 12 feet in width. East of $2^{\text {nd }}$ Street, land uses transition from commercial to residential, and the standard 5 -foot wide contiguous sidewalks are provided on both sides of Center Street between $2^{\text {nd }}$ Street and $1^{\text {st }}$ Street. No sidewalks are provided along Center Street between $1^{\text {st }}$ Street and SR-33, where adjacent properties are primarily undeveloped.

## TABLE 8

HORIZON YEAR 2042 WITH PROJECT ROADWAY SEGMENT ANALYSIS

| ROADWAY SEGMENT |  | Classification ${ }^{1}$ | $\begin{gathered} \text { \# of } \\ \text { Lanes } \end{gathered}$ | Capacity ${ }^{1}$ | HORIZON YR (2042) WITH PROJECT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Volume |  |  | LOS |
| 1 | Kern Street (SR-33) between 10th Street and 6th Street |  | Arterial Highway | 4 | 30,000 | 15,612 | A |
| 2 | Kern Street (SR-33) between 6th Street and 1st Street | Arterial Highway | 4 | 30,000 | 12,654 | A |
| 3 | Westside Highway (SR-33) between 1st Street and Center Street | Arterial Highway | 2 | 18,000 | 4,929 | A |
| 4 | Westside Highway (SR-33) between Center Street and Main Street | Arterial Highway | 2 | 18,000 | 5,363 | A |
| 5 | Center Street between 10th Street and 6th Street | Collector | 2 | 8,000 | 5,086 | D |
| 6 | Center Street between 6th Street and Westside Highway (SR-33) | Collector | 2 | 8,000 | 1,247 | A |
| 7 | Main Street between 10th Street and 6th Street | Collector | 2 | 8,000 | 2,636 | B |
| 8 | Main Street between 6th Street and Westside Highway (SR-33) | Collector | 2 | 8,000 | 3,110 | B |
| 9 | 10th Street between Kern Street (SR-33) and Center Street | Arterial | 4 | 30,000 | 15,604 | A |
| 10 | 6th Street between Kern Street (SR-33) and Center Street | Collector | 4 | 15,000 | 5,621 | A |

Footnotes:
${ }^{\prime}$ Roadway classification and daily capacity taken from Table 4.4-4 (Roadway Segment Daily Volume Thresholds) of the City of Taft General Plan Draft Environmental Impact Report (July 2009)

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Both controlled and uncontrolled pedestrian crossings are provided at several intersections and at midblock locations along Center Street between $10^{\text {th }}$ Street and $2^{\text {nd }}$ Street. High-visibility ladder crosswalks and/or crosswalks treated with pavers are provided at uncontrolled mid-block crossings between $7^{\text {th }}$ Street and $6^{\text {th }}$ Street, between $6^{\text {th }}$ Street and $5^{\text {th }}$ Street, between $5^{\text {th }}$ Street and $4^{\text {th }}$ Street, between $4^{\text {th }}$ Street and $3^{\text {rd }}$ Street, and between $3^{\text {rd }}$ Street and $2^{\text {nd }}$ Street. High-visibility ladder crosswalks are also provided across Center Street and other intersection legs at the intersections of Center Street $/ 7^{\text {th }}$ Street, Center Street $/ 6^{\text {th }}$ Street, Center Street $/ 4^{\text {th }}$ Street, Center Street $/ 3^{\text {rd }}$ Street, and Center Street $/ 2^{\text {nd }}$ Street. Crosswalks treated with pavers are provided across all four legs of the Center Street $/ 5^{\text {th }}$ Street intersection.

## Main Street

Most of the segment of Main Street between $10^{\text {th }}$ Street and $7^{\text {th }}$ Street is currently lacking sidewalks along one or both sides of the roadway. Along Main Street between $7^{\text {th }}$ Street and $3^{\text {rd }}$ Street, a mix of contiguous and non-contiguous sidewalks are provided on both sides of the street, with a few missing gaps along undeveloped parcels. Sidewalk widths range from 5 feet to 10 feet along Main Street between $7^{\text {th }}$ Street and $3^{\text {rd }}$ Street. Along Main Street between $3^{\text {rd }}$ Street and $2^{\text {nd }}$ Street, sidewalks are only provided along the north side of the street, and along Main Street between $2^{\text {nd }}$ Street and SR-33, where adjacent properties are primarily undeveloped, sidewalks are not provided except for short sections along developed parcels.

An uncontrolled mid-block pedestrian crossing with a crosswalk treated with pavers is provided across Main Street between $6^{\text {th }}$ Street and $4^{\text {th }}$ Street on the east leg of the former Main Street $/ 5^{\text {th }}$ Street intersection. The former $5^{\text {th }}$ Street between Center Street and Main Street was converted to a linear park in which vehicular traffic is prohibited.

## $10^{\text {th }}$ Street

Contiguous sidewalks ranging between 5 feet and 10 feet in width are currently provided on both sides of $10^{\text {th }}$ Street between Kern Street (SR-33) and Main Street. Contiguous sidewalks are only provided along the west side of $10^{\text {th }}$ Street between Main Street and Supply Row, and along $10^{\text {th }}$ Street south of Supply Row, contiguous sidewalks are provided on both sides of the street. An uncontrolled mid-block pedestrian crossing with a high-visibility ladder crosswalk is provided across $10^{\text {th }}$ Street at the location where the Taft Rails to Trails Multi-Use Path crosses $10^{\text {th }}$ Street.

## $6^{\text {th }}$ Street

Contiguous sidewalks approximately 10 feet in width are currently provided on both sides of $6^{\text {th }}$ Street between Kern Street (SR-33) and Main Street. Between Main Street and Supply Row, contiguous sidewalks approximately 5 feet in width are currently provided on both sides of $6^{\text {th }}$ Street. Contiguous sidewalk is currently only provided along the west side of $6^{\text {th }}$ Street between Supply Row and Front Street.

High-visibility ladder crosswalks are provided across $6^{\text {th }}$ Street at the intersections of $6^{\text {th }}$ Street/Kern Street (SR-33), $6^{\text {th }}$ Street/North Street, $6^{\text {th }}$ Street/Center Street, and at an uncontrolled mid-block pedestrian crossing at the location where the Taft Rails to Trails Multi-Use Path crosses $6^{\text {th }}$ Street between Main Street and Supply Row.

## Existing Bicycle Network

Most of the roadways within the Specific Plan area are currently lacking bicycle facilities except for Main Street between $2^{\text {nd }}$ Street and SR-33, where a narrow shoulder with "bike lane" signage is provided in both directions of travel, but the shoulder lane is discontinuous in the eastbound direction. The lane widths of the existing bicycle lanes and associated signage along Main Street between $2^{\text {nd }}$ Street and SR33 are substandard, and therefore these existing bicycle lanes are not considered Class II bicycle lanes.

A Class I bike path (Taft Rails to Trails) is currently provided within the Specific Plan area between Main Street and Supply Row. The Taft Rails to Trails bike path is oriented in a general east-west direction and is approximately two (2) miles in length. The bike path is approximately 12 feet wide and is divided by a dashed line along the western and eastern sections, but is undivided through the middle section of the bike path. There are no current plans to extend the existing Taft Rails to Trails Class I bike path, although the City of Taft General Plan Circulation Element identifies future trail connections to the community of Fellows to the northwest and the community of Maricopa to the southeast.

## Existing Transit Network

Taft Area Transit (TAT) and Kern Transit currently provide the following transit bus routes within the Specific Plan area and through the City of Taft:

- TAT Taft-Maricopa Route: The Taft-Maricopa Route extends between the Cities of Taft and Maricopa via Kern Street/SR-33. Within the City of Taft, the Taft-Maricopa Route extends north from Kern Street (SR-33) onto Wildcat Way (aligned with $7^{\text {th }}$ Street) and provides transit access to Taft High School, Taft College, and the Albertsons shopping center. The route continues west along Ash Street and heads south on $10^{\text {th }}$ Street back to Kern Street (SR-33), and proceeds southeast along Kern Street (SR-33) through the Specific Plan area toward Maricopa.

Service is currently provided Monday through Friday between 7:12am and 6:05pm, and runs three (3) times daily. During the morning peak hour, the Taft-Maricopa Route begins at the Maricopa Post Office at 7:12am, and ends at the bus stop along eastbound Kern Street (SR-33) at $2^{\text {nd }}$ Street at 7:42am. The Taft-Maricopa Route runs again in the afternoon, beginning at Kern Street (SR-33) at $2^{\text {nd }}$ Street at $1: 34 \mathrm{pm}$. The afternoon route circulates through the City of Taft and heads to Maricopa, then returns to circulate through Taft a second time and ends at the bus stop along eastbound Kern Street (SR-33) at $2^{\text {nd }}$ Street at 2:25pm. A third route runs during the evening peak hour, beginning at Kern Street (SR-33) at $2^{\text {nd }}$ Street at $5: 14 \mathrm{pm}$. The evening route circulates through the City of Taft then heads to Maricopa, and then returns to circulate through Taft a second time and ends at the bus stop along eastbound Kern Street (SR-33) at $2^{\text {nd }}$ Street at 6:05pm.

- Kern Transit Route 120 (Taft-Bakersfield): Kern Transit Route 120 extends between Taft and Bakersfield via $6^{\text {th }}$ Street, Harrison Street, Highway 119, Highway 43, and Highway 58. Within the City of Taft and the Specific Plan area, Route 120 heads south on $6^{\text {th }}$ Street, heads west on Kern Street (SR-33), heads south on $8^{\text {th }}$ Street, heads east on Main Street, heads south on $4^{\text {th }}$ Street, heads west on Supply Row to the Taft Transit Center ( 550 Supply Row), continues west on Supply Row, and heads north on $6^{\text {th }}$ Street back to Highway 119 and Bakersfield. Within the City of Taft, stops are provided at Taft College, the Heritage Park Senior Complex, ( 8 至 Street at North Street), and the Taft Transit Center.

Service is currently provided Monday through Friday between 6:10am and 9:02pm, and runs four (4) times daily. Service is also provided on Saturday between 7:50am and 6:57pm, and runs three (3) times daily from Taft to Bakersfield, and two (2) times daily from Bakersfield to Taft. During the weekday morning hours, westbound Route 120 departs the Downtown Bakersfield Transit Center at 6:10am, and arrives at the Taft Transit Center at 7:16am. Eastbound Route 120 during the morning hours departs Taft College at 7:40am and arrives at the Downtown Bakersfield Transit Center at 9:07am. Headways during the morning peak period are approximately every two (2) hours. Route 120 also operates afternoon service between Taft and Bakersfield from 12:47pm to $3: 57 \mathrm{pm}$. During the evening hours, westbound Route 120 departs the Downtown Bakersfield Transit Center at $5: 47 \mathrm{pm}$ and arrives at the Taft Transit Center at 7:09pm. Eastbound Route 120 during the evening hours departs Taft College at $7: 35 \mathrm{pm}$ and arrives at the Downtown Bakersfield Transit Center at 9:02pm.

The Taft Area Transit (TAT) Taft-Maricopa Route and Kern Transit Route 120 maps and schedules as described above are provided in Appendix F.

A total of four (4) transit bus stops for the Taft Area Transit (TAT) Taft-Maricopa Route are currently provided along Kern Street (SR-33) within the Specific Plan area, which are listed below:

- Eastbound Kern Street (SR-33) at $5^{\text {th }}$ Street: No shelter or amenities provided.
- Eastbound Kern Street (SR-33) at $2^{\text {nd }}$ Street: No shelter or amenities provided.
- Westbound Kern Street (SR-33) at $2^{\text {nd }}$ Street: No shelter or amenities provided.
- Westbound Kern Street (SR-33) at $4^{\text {th }}$ Street: Bench, shelter and trash receptacle provided.

The Taft Transit Center, located at 550 Supply Row within the Specific Plan area, serves Kern Transit Route 120 between Taft and Bakersfield. One transit bus stop is also provided for Kern Transit Route 120 within the Specific Plan area along southbound $8^{\text {th }}$ Street at North Street next to the Heritage Park Senior Complex, where a bench and shelter is provided.

## Future Pedestrian Network

The Kern Region Active Transportation Plan (Alta Planning + Design, March 2018) provides recommendations for future pedestrian facility improvements in the City of Taft in order of priority and are ranked as Tier 1 (highest priority), Tier 2 (medium priority) and Tier 3 (lowest priority). Below is a list of the recommended Tier 1 and Tier 2 pedestrian facility improvements from the Kern Region Active Transportation Plan that are within and adjacent to the Downtown Taft Specific Plan area, in order of priority:

## Tier 1 Projects (Highest Priority: 1-10 Years)

- $10^{\text {th }}$ Street from Ash Street to Main Street: Pedestrian corridor improvements (high-visibility crossings and traffic calming measures)
- $6^{\text {th }}$ Street from Ash Street to Main Street: Pedestrian corridor improvements (high-visibility crossings and traffic calming measures)


## Tier 2 Projects (Medium Priority: 10-15 Years)

- Kern Street (SR-33) from Cascade Place to Taft Highway: Pedestrian corridor improvements (some corridor improvements have already been completed)

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All of the above-listed planned future pedestrian corridor improvements are anticipated to be completed prior to buildout of the Downtown Taft Specific Plan.

## Recommended Future Pedestrian Facility Improvements With Specific Plan

In addition to the above-listed planned future pedestrian corridor improvements, the following pedestrian facility improvements are recommended to enhance pedestrian safety and access within the Downtown Taft Specific Plan:

- North Street from $8^{\text {th }}$ Street to $1^{\text {st }}$ Street: Improve existing sidewalks and provide high-visibility crosswalks at all intersections.
- Center Street from $2^{\text {nd }}$ Street to Westside Highway (SR-33): Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- Main Street from $10^{\text {th }}$ Street to $2^{\text {nd }}$ Street: Close the existing sidewalk gaps and provide wide sidewalks ( $10+$ feet in width) along both sides of the street. Provide high-visibility crosswalks at all intersections.
- Main Street from $2^{\text {nd }}$ Street to Westside Highway (SR-33): Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- Supply Row from $10^{\text {th }}$ Street to $2^{\text {nd }}$ Street: Provide wide sidewalks ( $10+$ feet in width) along both sides of the street, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- $6^{\text {th }}$ Street from Main Street to Front Street: Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- $4^{\text {th }}$ Street from Kern Street (SR-33) to Front Street: Improve existing sidewalks and provide highvisibility crosswalks at all intersections.
- $2^{\text {nd }}$ Street/Olive Avenue from Kern Street (SR-33) to Front Street: Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- Front Street from $10^{\text {th }}$ Street to Oak Street: Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.


## Future Bicycle Network

The Kern Region Active Transportation Plan (Alta Planning + Design, March 2018) provides recommendations for future bicycle facility improvements in the City of Taft in order of priority and are ranked as Tier 1 (highest priority), Tier 2 (medium priority) and Tier 3 (lowest priority). Below is a list of the recommended Tier 1 and Tier 2 bicycle facility improvements from the Kern Region Active Transportation Plan that are within and adjacent to the Downtown Taft Specific Plan area, in order of priority:

## Tier 1 Projects (Highest Priority: 1-10 Years)

- $6^{\text {th }}$ Street from Ash Street to Oak Street: Class II Buffered Bike Lanes
- $10^{\text {th }}$ Street from Ash Street to Center Street: Class IV Cycle Track
- Kern Street (SR-33) from Cascade Place to $1^{\text {st }}$ Street: Class II Buffered Bike Lanes
- $10^{\text {th }}$ Street from Center Street to F Street: Class II Bike Lanes
- $2^{\text {nd }}$ Street from Calvin Street to Supply Row: Class III Bike Boulevard


## Tier 2 Projects (Medium Priority: 10-15 Years)

- $\quad 1^{\text {st }}$ Street from Calvin Street to Kern Street: Class II Buffered Bike Lanes
- Westside Highway (SR-33) from $1^{\text {st }}$ Street to SR-119: Class II Bike Lanes

All of the above-listed planned future bicycle facility improvements are anticipated to be completed prior to buildout of the Downtown Taft Specific Plan.

## Recommended Future Bicycle Facility Improvements With Specific Plan

In addition to the above-listed planned future bicycle facility projects, the following bicycle facility improvements are recommended to enhance bicycle safety and improve access to the future land uses within the Downtown Taft Specific Plan:

- $2^{\text {nd }}$ Street from Kern Street (SR-33) to Supply Row: Provide Class II bike lanes (upgrade from planned Class III Bike Boulevard in Kern Region Active Transportation Plan)
- $\quad 1^{\text {st }}$ Street from Kern Street (SR-33) to Center Street: Provide Class II bike lanes
- Center Street from $2^{\text {nd }}$ Street to West Side Highway (SR-33): Provide Class II bike lanes
- Main Street from $2^{\text {nd }}$ Street to West Side Highway (SR-33): Provide Class II bike lanes
- North Street from $8^{\text {th }}$ Street to $1^{\text {st }}$ Street: Provide Class III Bike Boulevard
- $8^{\text {th }}$ Street from Kern Street (SR-33) to Main Street: Provide Class III Bike Boulevard


## Future Transit Network

Specific planned future transit network or facility improvements within or adjacent to the Downtown Taft Specific Plan area are not identified, but below are recommended future transit network and facility improvements that are consistent with transit policies in the City of Taft General Plan Circulation Element (June 2010, Amended April 2017).

## Recommended Future Transit Network and Facility Improvements With Specific Plan

- Coordinate with Taft Area Transit (TAT) to provide benches, shelters and trash receptacles at the existing bus stops along Kern Street (SR-33).
- Coordinate with Taft Area Transit (TAT) to expand the Taft-Maricopa Route to include $10^{\text {th }}$ Street between Kern Street (SR-33) and Main Street, and Main Street between $10^{\text {th }}$ Street and West Side Highway (SR-33), and to install sheltered bus stops along the expanded route.
- Coordinate with Taft Area Transit (TAT) to provide all-day service and to expand weekday hours of operation to between $6: 00 \mathrm{am}$ and $7: 00 \mathrm{pm}$ and to provide limited weekend service for the TaftMaricopa Route.
- Coordinate with Kern Transit to expand Route 120 to include Kern Street (SR-33) between $6^{\text {th }}$ Street and $2^{\text {nd }}$ Street, $2^{\text {nd }}$ Street between Kern Street (SR-33) and Main Street, and Main Street between $4^{\text {th }}$ Street and $2^{\text {nd }}$ Street.
- Coordinate with Kern Transit to provide additional sheltered bus stops along both the existing Route 120 and the recommended expanded Route 120 within the Downtown Taft Specific Plan area.
- Coordinate with Kern Transit to expand the Route 120 weekday and Saturday hours of operation with the first eastbound bus leaving Taft at 6:00am, and with the last westbound bus arriving in Taft at 9:00pm.


## CONCLUSIONS AND RECOMMENDATIONS

This Local Transportation Analysis evaluated the potential level of service (LOS) deficiencies and transportation improvements that may need to be considered in association with the traffic generated by the proposed Downtown Taft Specific Plan project. Below is a summary of the analysis findings and recommended transportation improvements:

## Intersection Operations Analysis Findings

The results of the Existing Conditions intersection analysis showed that all study intersections are currently operating at LOS C or better during the AM and PM peak hours.

Under Horizon Year 2042 conditions with the buildout of the Downtown Taft Specific Plan, the intersection analysis results showed that the following intersections are forecast to operate at a deficient LOS E or F during the AM and PM peak hours:

- Kern Street (SR-33) / 6th Street (AM: LOS F; PM: LOS E)
- Kern Street (SR-33) / 4th Street (AM/PM: LOS F)
- Kern Street (SR-33) / E. Kern Street / 1st Street (AM: LOS E; PM: LOS F)


## Roadway Segment Analysis Findings

The results of the Existing Conditions roadway segment operations analysis showed that all study roadway segments currently operate at LOS B or better.

Under Horizon Year 2042 conditions with the buildout of the Downtown Taft Specific Plan, the roadway segment analysis results showed that all study roadway segments are forecast to operate at LOS D or better.

## Recommended Transportation Improvements

The following improvements are recommended to improve operations at the three deficient study intersections to acceptable levels of service:

## Kern Street (SR-33) / 6th Street

- Install traffic signal
- Restripe eastbound and westbound approaches to provide one left-turn lane, one through lane, and one shared through/right-turn lane
- Restripe northbound and southbound approaches to provide one left-turn lane, one through lane, and one right-turn lane
- Coordinate with Caltrans to determine the signal interconnect requirements between the existing signal at $10^{\text {th }}$ Street and the proposed signals at $6^{\text {th }}$ Street and $4^{\text {th }}$ Street


## Kern Street (SR-33) / 4th Street

- Install traffic signal
- Restripe eastbound and westbound approaches to provide one left-turn lane, one through lane, and one shared through/right-turn lane
- Restripe northbound and southbound approaches to provide one left-turn lane and one shared through/right-turn lane
- Coordinate with Caltrans to determine the signal interconnect requirements between the existing signal at $10^{\text {th }}$ Street and the proposed signals at $6^{\text {th }}$ Street and $4^{\text {th }}$ Street


## Kern Street (SR-33) / E. Kern Street / 1st Street

- Install stop signs on the eastbound (Kern Street/SR-33) and northwestbound (SR-33) approaches to convert intersection to all-way stop control.

The recommended transportation improvements as described above are also illustrated graphically in Exhibit 8.

The Peak Hour Warrant (Warrant 3) from the California Manual on Uniform Traffic Devices (CAMUTCD, 2014 Edition, Revision 6, March 30, 2021) was conducted at the Kern Street (SR-33) / 6th Street and Kern Street (SR-33) / 4th Street intersections to determine if installation of a traffic signal would be justified under Horizon Year 2042 conditions with the buildout of the Downtown Taft Specific Plan as recommended.

The findings showed that the CA MUTCD Peak Hour Warrant (Warrant 3) is Satisfied at both the Kern Street (SR-33) / 6th Street and Kern Street (SR-33) / 4th Street intersections during the AM and PM peak hours. The CA MUTCD Peak Hour Warrant (Warrant 3) worksheets are provided in Appendix G.

The recommended installation of traffic signals at the Kern Street (SR-33) / 6th Street and Kern Street (SR-33) / 4th Street intersections would be subject to Caltrans approval as they would be operated and maintained by Caltrans. It is anticipated that the two proposed signals at the Kern Street (SR-33) / 6th Street and Kern Street (SR-33) / 4th Street intersections would need to be coordinated due to the distance between the two intersections (approximately 925 feet). Caltrans may also require the two intersections to be interconnected to the existing signal at the Kern Street (SR-33) / 10th Street intersection, which is located approximately 1,750 feet ( $1 / 3$ mile) west of $6^{\text {th }}$ Street.

The guidance for application of multi-way stop control in Section 2B. 07 of the CA MUTCD (2014 Edition, Revision 6, March 30, 2021) was reviewed to determine if installation of all-way stop control at the Kern Street (SR-33) / E. Kern Street / 1st Street intersection would be justified based on the future Horizon Year 2042 traffic volumes with the buildout of the Downtown Taft Specific Plan. The findings show that the Horizon Year 2042 approach volumes at the Kern Street (SR-33) / E. Kern Street / 1st Street intersection during the peak hours exceed the CA MUTCD multi-stop control criteria based on minimum volumes and delay. The CA MUTCD multi-way stop evaluation worksheet is also provided in Appendix G.

Table 9 shows the operational analysis results for the three deficient study intersections with the recommended improvements as previously described. Appendix H contains the Horizon Year 2042 With Project intersection analysis worksheets with the recommended improvements.

As shown in Table 9, the recommended improvements are anticipated to improve operations to LOS D or better during both the AM and PM peak hours.


Engineering Company

TABLE 9
HORIZON YEAR 2042 WITH PROJECT INTERSECTION OPERATIONS WITH RECOMMENDED IMPROVEMENTS

| INTERSECTION | HORIZON YR (2042) WITH PROJECT |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | WITHOUT <br> IMPROVEMENTS |  |  | $\begin{gathered} \text { WITH } \\ \text { IMPROVEMENTS } \end{gathered}$ |  |  |
|  | CONTROL | DELAY ${ }^{1}$ | LOS | CONTROL | DELAY ${ }^{1}$ | LOS |
| $\begin{array}{ll} 2 \begin{array}{l} \text { Kern Street (SR-33)/ } \\ 6^{\text {th }} \text { Street } \end{array} \\ & \text { AM peak } \\ & \text { PM peak } \end{array}$ | (AWSC) OVERALL <br> (AWSC) OVERALL | 117.7 42.9 | F E | (S) <br> OVERALL <br> (S) <br> OVERALL | $\begin{gathered} 34.2 \\ 30.2 \end{gathered}$ | C <br> C |
| Kern Street (SR-33)/  <br> $44^{\text {th }}$ Street  AM peak <br>   <br>   <br>  PM peak | (AWSC) OVERALL <br> (AWSC) OVERALL | $\begin{array}{r} 118.8 \\ 55.4 \\ \hline \end{array}$ | F | (S) <br> OVERALL <br> (S) <br> OVERALL | $\begin{array}{r} 35.8 \\ 31.1 \\ \hline \end{array}$ | D C |
| 4 <br> Kern Street (SR-33) / E. Kern Street/ $1^{\text {st }}$ Street <br> AM peak <br> PM peak | $\begin{gathered} \text { (TWSC) } \\ \text { NBT } \\ (\text { TWSC }) \\ \text { NBT } \end{gathered}$ | $\begin{array}{r} 47.6 \\ 67.2 \\ \hline \end{array}$ | E | (AWSC) <br> OVERALL <br> (AWSC) <br> OVERALL | $\begin{array}{r} 12.3 \\ 13.9 \\ \hline \end{array}$ | B B |

Footnotes: S = Signalized; AWSC = All-Way-Stop Controlled; TWSC = Two-Way-Stop Controlled
${ }^{1}$ Delay is measured in seconds per vehicle.

## Active Transportation and Transit Assessment Recommendations

Based on the findings of the assessment of existing and planned future pedestrian, bicycle and transit facilities, the following pedestrian, bicycle and transit network and facility improvements are recommended to be implemented within the Downtown Taft Specific Plan area:

Recommended Pedestrian Facility Improvements

- North Street from $8^{\text {th }}$ Street to $1^{\text {st }}$ Street: Improve existing sidewalks and provide high-visibility crosswalks at all intersections.
- Center Street from $2^{\text {nd }}$ Street to Westside Highway (SR-33): Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- Main Street from $10^{\text {th }}$ Street to $2^{\text {nd }}$ Street: Close the existing sidewalk gaps and provide wide sidewalks ( $10+$ feet in width) along both sides of the street. Provide high-visibility crosswalks at all intersections.
- Main Street from $2^{\text {nd }}$ Street to Westside Highway (SR-33): Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- Supply Row from $10^{\text {th }}$ Street to $2^{\text {nd }}$ Street: Provide wide sidewalks ( $10+$ feet in width) along both sides of the street, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- $6^{\text {th }}$ Street from Main Street to Front Street: Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- $4^{\text {th }}$ Street from Kern Street (SR-33) to Front Street: Improve existing sidewalks and provide highvisibility crosswalks at all intersections.
- $2^{\text {nd }}$ Street/Olive Avenue from Kern Street (SR-33) to Front Street: Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- Front Street from $10^{\text {th }}$ Street to Oak Street: Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.


## Recommended Bicycle Facility Improvements

- $2^{\text {nd }}$ Street from Kern Street (SR-33) to Supply Row: Provide Class II bike lanes (upgrade from planned Class III Bike Boulevard in Kern Region Active Transportation Plan)
- $1^{\text {st }}$ Street from Kern Street (SR-33) to Center Street: Provide Class II bike lanes
- Center Street from $2^{\text {nd }}$ Street to West Side Highway (SR-33): Provide Class II bike lanes
- Main Street from $2^{\text {nd }}$ Street to West Side Highway (SR-33): Provide Class II bike lanes
- North Street from $8^{\text {th }}$ Street to $1^{\text {st }}$ Street: Provide Class III Bike Boulevard
- $8^{\text {th }}$ Street from Kern Street (SR-33) to Main Street: Provide Class III Bike Boulevard


## Recommended Future Transit Network and Facility Improvements

- Coordinate with Taft Area Transit (TAT) to provide benches, shelters and trash receptacles at the existing bus stops along Kern Street (SR-33).
- Coordinate with Taft Area Transit (TAT) to expand the Taft-Maricopa Route to include $10^{\text {th }}$ Street between Kern Street (SR-33) and Main Street, and Main Street between $10^{\text {th }}$ Street and West Side Highway (SR-33), and to install sheltered bus stops along the expanded route.
- Coordinate with Taft Area Transit (TAT) to provide all-day service and to expand weekday hours of operation to between 6:00am and 7:00pm and to provide limited weekend service for the TaftMaricopa Route.
- Coordinate with Kern Transit to expand Route 120 to include Kern Street (SR-33) between $6^{\text {th }}$ Street and $2^{\text {nd }}$ Street, $2^{\text {nd }}$ Street between Kern Street (SR-33) and Main Street, and Main Street between $4^{\text {th }}$ Street and $2^{\text {nd }}$ Street.
- Coordinate with Kern Transit to provide additional sheltered bus stops along both the existing Route 120 and the recommended expanded Route 120 within the Downtown Taft Specific Plan area.
- Coordinate with Kern Transit to expand the Route 120 weekday and Saturday hours of operation with the first eastbound bus leaving Taft at 6:00am, and with the last westbound bus arriving in Taft at 9:00pm.

Engineering Company

## REFERENCES

1. Kern County, Standards for Traffic Engineering, February 2010.
2. City of Taft, General Plan Circulation Element, June 2010, Amended April 2017.
3. City of Taft, General Plan Draft Environmental Impact Report, July 2009.
4. Kern Council of Governments, $\underline{2018 \text { Regional Transportation Plan, August } 2018 . ~}$
5. State of California Office of Planning and Research (OPR), Technical Advisory on Evaluating Transportation Impacts in CEQA, December 2018.
6. Institute of Transportation Engineers (ITE), Trip Generation Manual, $11^{\text {th }}$ Edition, September 2021.
7. Transportation Research Board, Highway Capacity Manual $6^{\text {th }}$ Edition, Washington, D.C., 2016.
8. Trafficware LLC, Synchro, Version 11, Sugar Land, Texas, 2021.
9. Alta Planning + Design, Kern Region Active Transportation Plan, March 2018.
10. California Department of Transportation, California Manual on Uniform Traffic Control Devices (MUTCD), 2014 Edition, Revision 6, March 2021.

## APPENDIX A

VEHICLE, PEDESTRIAN AND BICYCLE COUNTS

##  Mato Tatatic Datalac.

Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

# Turning Movement Report 

Prepared For:

Rick Engineering Company

| LOCATION | Kern St @ 10th St | LATITUDE |
| ---: | :---: | :---: |
| COUNTY | Kern | LONGITUDE |
| COLLECTION DATE | Tuesday, March 15,2022 | WEATHER |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 7:00 AM - 7:15 AM | 0 | 13 | 33 | 15 | 2 | 0 | 18 | 17 | 22 | 3 | 0 | 18 | 42 | 17 | 6 | 0 | 8 | 26 | 7 | 3 |
| 7:15 AM - 7:30 AM | 0 | 10 | 46 | 51 | 3 | 0 | 13 | 30 | 21 | 2 | 0 | 33 | 62 | 12 | 5 | 0 | 13 | 42 | 13 | 0 |
| 7:30 AM - 7:45 AM | 0 | 22 | 57 | 39 | 7 | 0 | 21 | 62 | 33 | 3 | 0 | 42 | 66 | 17 | 5 | 0 | 31 | 60 | 20 | 3 |
| 7:45 AM - 8:00 AM | 0 | 16 | 45 | 30 | 1 | 0 | 13 | 50 | 24 | 0 | 0 | 28 | 53 | 14 | 2 | 0 | 22 | 57 | 15 | 1 |
| 8:00 AM - 8:15 AM | 0 | 19 | 33 | 9 | 2 | 0 | 10 | 32 | 13 | 1 | 0 | 20 | 49 | 11 | 7 | 0 | 9 | 40 | 10 | 6 |
| 8:15 AM - 8:30 AM | 0 | 15 | 35 | 14 | 4 | 0 | 9 | 31 | 23 | 4 | 0 | 16 | 36 | 6 | 3 | 0 | 7 | 24 | 11 | 5 |
| 8:30 AM - 8:45 AM | 0 | 11 | 26 | 12 | 2 | 0 | 3 | 22 | 22 | 3 | 0 | 21 | 33 | 13 | 5 | 0 | 8 | 30 | 4 | 2 |
| 8:45 AM - 9:00 AM | 0 | 13 | 22 | 13 | 1 | 0 | 9 | 21 | 20 | 0 | 0 | 20 | 42 | 8 | 3 | 0 | 8 | 36 | 10 | 1 |
| TOTAL | 0 | 119 | 297 | 183 | 22 | 0 | 96 | 265 | 178 | 16 | 0 | 198 | 383 | 98 | 36 | 0 | 106 | 315 | 90 | 21 |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 4:00 PM - 4:15 PM | 0 | 22 | 41 | 25 | 1 | 0 | 16 | 49 | 30 | 1 | 0 | 33 | 47 | 24 | 3 | 0 | 20 | 55 | 5 | 3 |
| 4:15 PM - 4:30 PM | 0 | 25 | 47 | 20 | 2 | 0 | 14 | 50 | 38 | 0 | 0 | 31 | 39 | 19 | 5 | 0 | 20 | 46 | 11 | 1 |
| 4:30 PM - 4:45 PM | 0 | 20 | 39 | 18 | 0 | 0 | 19 | 48 | 31 | 1 | 0 | 36 | 50 | 31 | 3 | 0 | 20 | 55 | 10 | 0 |
| 4:45 PM - 5:00 PM | 0 | 33 | 47 | 26 | 1 | 0 | 18 | 40 | 33 | 1 | 0 | 34 | 56 | 23 | 3 | 0 | 16 | 38 | 9 | 3 |
| 5:00 PM - 5:15 PM | 0 | 22 | 65 | 22 | 1 | 0 | 17 | 62 | 43 | 3 | 0 | 25 | 50 | 21 | 3 | 0 | 24 | 53 | 9 | 0 |
| 5:15 PM - 5:30 PM | 0 | 25 | 45 | 15 | 2 | 0 | 20 | 52 | 31 | 1 | 0 | 29 | 48 | 25 | 2 | 0 | 17 | 47 | 13 | 0 |
| 5:30 PM - 5:45 PM | 0 | 21 | 55 | 18 | 0 | 0 | 24 | 48 | 40 | 0 | 0 | 26 | 48 | 24 | 3 | 0 | 16 | 56 | 9 | 2 |
| 5:45 PM - 6:00 PM | 0 | 25 | 36 | 27 | 1 | 1 | 15 | 47 | 41 | 0 | 0 | 29 | 50 | 19 | 0 | 0 | 22 | 43 | 15 | 1 |
| TOTAL | 0 | 193 | 375 | 171 | 8 | 1 | 143 | 396 | 287 | 7 | 0 | 243 | 388 | 186 | 22 | 0 | 155 | 393 | 81 | 10 |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PEAK HOUR | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 7:15 AM - 8:15 AM | 0 | 67 | 181 | 129 | 13 | 0 | 57 | 174 | 91 | 6 | 0 | 123 | 230 | 54 | 19 | 0 | 75 | 199 | 58 | 10 |
| 4:45 PM - 5:45 PM | 0 | 101 | 212 | 81 | 4 | 0 | 79 | 202 | 147 | 5 | 0 | 114 | 202 | 93 | 11 | 0 | 73 | 194 | 40 | 5 |



Kern St (SR 33)


10th St

##  Meto Traticic Data Inc.

Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20
Hanford, CA 93230
800-975-6938 Phone/Fax www.metrotrafficdata.com

## Turning Movement Report

Prepared For:
Rick Engineering Company
5620 Friars Rd
San Diego, CA 92110

| LOCATION | Kern St @ 10th St |
| ---: | :---: |
| COUNTY | Kern |
| COLLECTION DATE | Tuesday, March 15, 2022 |


| LATITUDE | 35.1449 |
| :---: | :---: |
| LONGITUDE | -119.4652 |
| WEATHER | Clear |


| Time | Northbound Bikes |  |  | N.Leg Peds | Southbound Bikes |  |  | S.Leg Peds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg <br> Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 7:00 AM - 7:15 AM | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 1 |
| 7:15 AM - 7:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |
| 7:30 AM - 7:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 |
| 7:45 AM - 8:00 AM | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 |
| 8:00 AM - 8:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:15 AM - 8:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 1 | 1 | 0 | 0 | 0 | 0 |
| 8:30 AM - 8:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| 8:45 AM - 9:00 AM | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 |
| TOTAL | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 3 | 1 | 0 | 1 | 8 | 0 | 0 | 0 | 6 |


| Time | Northbound Bikes |  |  | N.Leg <br> Peds | Southbound Bikes |  |  | S.Leg <br> Peds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 4:00 PM - 4:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 1 |
| 4:15 PM - 4:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| 4:30 PM - 4:45 PM | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 9 | 0 | 0 | 0 | 1 |
| 4:45 PM - 5:00 PM | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 4 |
| 5:00 PM - 5:15 PM | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 2 | 0 | 1 | 0 | 4 |
| 5:15 PM - 5:30 PM | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 |
| 5:30 PM - 5:45 PM | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 |
| 5:45 PM - 6:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |
| TOTAL | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 15 | 0 | 1 | 0 | 18 |


|  | Northbound Bikes |  |  | N.Leg Peds | Southbound Bikes |  |  | S.Leg Peds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg <br> Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PEAK HOUR | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 7:15 AM - 8:15 AM | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 3 |
| 4:45 PM - 5:45 PM | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 4 | 0 | 1 | 0 | 13 |


|  | Bikes | Peds |
| :---: | :---: | :---: |
| AM Peak Total | 0 | 10 |
| PM Peak Total | $\mathbf{1}$ | $\mathbf{2 8}$ |

Kern St (SR 33)


Page 2 of 3

##  Mato Tatatic Datalac.

Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

# Turning Movement Report 

Prepared For:

Rick Engineering Company
LOCATION $\quad$ Kern St @ 6th St

COLLECTION DATE $\qquad$ Tuesday, March 15, 2022 $\qquad$

| LATITUDE | 35.1431 |
| :---: | :---: |
| LONGITUDE | -119.4596 |
| WEATHER | Clear |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 7:00 AM - 7:15 AM | 0 | 1 | 18 | 1 | 0 | 0 | 3 | 8 | 6 | 0 | 0 | 9 | 45 | 0 | 2 | 0 | 1 | 29 | 5 | 2 |
| 7:15 AM - 7:30 AM | 0 | 5 | 36 | 4 | 0 | 0 | 11 | 18 | 17 | 0 | 0 | 18 | 52 | 1 | 3 | 0 | 4 | 57 | 12 | 1 |
| 7:30 AM - 7:45 AM | 0 | 6 | 59 | 9 | 0 | 0 | 9 | 55 | 27 | 0 | 0 | 23 | 60 | 6 | 4 | 0 | 3 | 87 | 15 | 5 |
| 7:45 AM - 8:00 AM | 0 | 2 | 27 | 4 | 2 | 0 | 12 | 34 | 18 | 2 | 0 | 23 | 68 | 1 | 2 | 0 | 7 | 69 | 9 | 1 |
| 8:00 AM - 8:15 AM | 0 | 4 | 13 | 4 | 1 | 0 | 11 | 21 | 12 | 1 | 0 | 15 | 47 | 4 | 6 | 0 | 2 | 44 | 3 | 5 |
| 8:15 AM - 8:30 AM | 0 | 4 | 9 | 3 | 0 | 0 | 2 | 14 | 4 | 0 | 0 | 7 | 50 | 4 | 3 | 0 | 2 | 40 | 3 | 7 |
| 8:30 AM - 8:45 AM | 0 | 3 | 9 | 3 | 0 | 0 | 7 | 9 | 9 | 0 | 0 | 7 | 35 | 1 | 3 | 0 | 2 | 31 | 7 | 0 |
| 8:45 AM - 9:00 AM | 0 | 5 | 8 | 2 | 0 | 0 | 11 | 11 | 10 | 0 | 0 | 11 | 40 | 3 | 2 | 0 | 4 | 41 | 5 | 1 |
| TOTAL | 0 | 30 | 179 | 30 | 3 | 0 | 66 | 170 | 103 | 3 | 0 | 113 | 397 | 20 | 25 | 0 | 25 | 398 | 59 | 22 |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 4:00 PM - 4:15 PM | 0 | 8 | 19 | 5 | 0 | 0 | 13 | 35 | 14 | 1 | 0 | 16 | 72 | 5 | 4 | 0 | 11 | 66 | 14 | 4 |
| 4:15 PM - 4:30 PM | 0 | 5 | 8 | 7 | 1 | 0 | 19 | 25 | 16 | 0 | 0 | 11 | 58 | 1 | 1 | 0 | 4 | 55 | 11 | 1 |
| 4:30 PM - 4:45 PM | 0 | 6 | 20 | 11 | 1 | 0 | 13 | 20 | 19 | 0 | 0 | 22 | 57 | 2 | 1 | 0 | 1 | 54 | 10 | 1 |
| 4:45 PM - 5:00 PM | 0 | 8 | 15 | 4 | 1 | 0 | 7 | 21 | 17 | 0 | 0 | 21 | 69 | 6 | 3 | 0 | 4 | 56 | 5 | 0 |
| 5:00 PM - 5:15 PM | 0 | 9 | 17 | 5 | 0 | 0 | 8 | 16 | 23 | 0 | 0 | 26 | 58 | 6 | 1 | 0 | 5 | 53 | 13 | 0 |
| 5:15 PM - 5:30 PM | 0 | 5 | 22 | 5 | 2 | 0 | 12 | 26 | 22 | 0 | 0 | 17 | 54 | 2 | 3 | 0 | 9 | 57 | 9 | 1 |
| 5:30 PM - 5:45 PM | 0 | 5 | 18 | 10 | 1 | 0 | 11 | 44 | 17 | 1 | 0 | 21 | 72 | 3 | 2 | 0 | 5 | 61 | 12 | 2 |
| 5:45 PM - 6:00 PM | 0 | 3 | 22 | 8 | 0 | 0 | 6 | 31 | 17 | 0 | 0 | 14 | 61 | 6 | 1 | 0 | 8 | 53 | 8 | 1 |
| TOTAL | 0 | 49 | 141 | 55 | 6 | 0 | 89 | 218 | 145 | 2 | 0 | 148 | 501 | 31 | 16 | 0 | 47 | 455 | 82 | 10 |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PEAK HOUR | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 7:15 AM - 8:15 AM | 0 | 17 | 135 | 21 | 3 | 0 | 43 | 128 | 74 | 3 | 0 | 79 | 227 | 12 | 15 | 0 | 16 | 257 | 39 | 12 |
| 5:00 PM - 6:00 PM | 0 | 22 | 79 | 28 | 3 | 0 | 37 | 117 | 79 | 1 | 0 | 78 | 245 | 17 | 7 | 0 | 27 | 224 | 42 | 4 |



Kern St (SR 33)


6th St

##  Meto Traticic Data Inc.

Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20
Hanford, CA 93230
800-975-6938 Phone/Fax www.metrotrafficdata.com

## Turning Movement Report

Prepared For:
Rick Engineering Company
5620 Friars Rd
San Diego, CA 92110

| LOCATION | Kern St @ 6th St |
| :---: | :---: |
| COUNTY | Kern |
| COLLECTION DATE | Tuesday, March 15, 2022 |


| LATITUDE | 35.1431 |
| ---: | :---: |
| LONGITUDE | -119.4596 |
| WEATHER | Clear |


| Time | Northbound Bikes |  |  | N.Leg Peds | Southbound Bikes |  |  | S.Leg Peds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 7:00 AM - 7:15 AM | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 0 |
| 7:15 AM - 7:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 0 | 0 | 0 | 8 |
| 7:30 AM - 7:45 AM | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 |
| 7:45 AM - 8:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 |
| 8:00 AM - 8:15 AM | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| 8:15 AM - 8:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:30 AM - 8:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
| 8:45 AM - 9:00 AM | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 0 | 0 | 5 | 0 | 1 | 0 | 2 | 0 | 0 | 0 | 29 | 0 | 0 | 0 | 9 |


| Time | Northbound Bikes |  |  | N.Leg <br> Peds | Southbound Bikes |  |  | S.Leg <br> Peds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 4:00 PM - 4:15 PM | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 2 |
| 4:15 PM - 4:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
| 4:30 PM - 4:45 PM | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4:45 PM - 5:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 |
| 5:00 PM - 5:15 PM | 0 | 0 | 0 | 2 | 0 | 0 | 1 | 2 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 1 |
| 5:15 PM - 5:30 PM | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 |
| 5:30 PM - 5:45 PM | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |
| 5:45 PM - 6:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 |
| TOTAL | 1 | 1 | 0 | 5 | 0 | 0 | 1 | 4 | 0 | 0 | 0 | 15 | 0 | 0 | 0 | 6 |


|  | Northbound Bikes |  |  | N.Leg Peds | Southbound Bikes |  |  | S.Leg Peds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PEAK HOUR | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 7:15 AM - 8:15 AM | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 19 | 0 | 0 | 0 | 9 |
| 5:00 PM - 6:00 PM | 1 | 1 | 0 | 3 | 0 | 0 | 1 | 3 | 0 | 0 | 0 | 9 | 0 | 0 | 0 | 4 |


|  | Bikes | Peds |
| :---: | :---: | :---: |
| AM Peak Total | 0 | 31 |
| PM Peak Total | 3 | 19 |

Kern St (SR 33)


Page 2 of 3

##  Mato Tatatic Datalac.

Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

# Turning Movement Report 

Prepared For:

Rick Engineering Company
LOCATION $\quad$ Kern St @ 4th St

COLLECTION DATE $\qquad$ Tuesday, March 15, 2022 $\qquad$

| LATITUDE | 35.1422 |
| :---: | :---: |
| LONGITUDE | -119.4567 |
| WEATHER | Clear |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 7:00 AM - 7:15 AM | 0 | 1 | 10 | 2 | 1 | 0 | 3 | 6 | 7 | 0 | 0 | 5 | 36 | 2 | 3 | 0 | 3 | 30 | 3 | 2 |
| 7:15 AM - 7:30 AM | 0 | 6 | 22 | 3 | 1 | 0 | 5 | 20 | 8 | 0 | 0 | 14 | 41 | 4 | 4 | 0 | 3 | 54 | 2 | 1 |
| 7:30 AM - 7:45 AM | 0 | 17 | 32 | 7 | 1 | 0 | 11 | 38 | 11 | 3 | 0 | 11 | 57 | 3 | 2 | 0 | 7 | 60 | 10 | 4 |
| 7:45 AM - 8:00 AM | 0 | 9 | 29 | 8 | 1 | 0 | 10 | 33 | 14 | 0 | 0 | 15 | 50 | 8 | 3 | 0 | 3 | 49 | 13 | 1 |
| 8:00 AM - 8:15 AM | 0 | 8 | 13 | 3 | 0 | 0 | 8 | 13 | 4 | 0 | 0 | 9 | 41 | 5 | 7 | 0 | 3 | 42 | 2 | 6 |
| 8:15 AM - 8:30 AM | 0 | 4 | 9 | 2 | 1 | 0 | 8 | 8 | 7 | 3 | 1 | 5 | 46 | 5 | 2 | 0 | 2 | 35 | 2 | 7 |
| 8:30 AM - 8:45 AM | 0 | 1 | 9 | 4 | 2 | 0 | 5 | 4 | 3 | 1 | 0 | 2 | 32 | 7 | 4 | 0 | 3 | 40 | 1 | 0 |
| 8:45 AM - 9:00 AM | 0 | 3 | 6 | 2 | 0 | 0 | 2 | 14 | 7 | 1 | 0 | 13 | 38 | 2 | 1 | 0 | 5 | 34 | 6 | 2 |
| TOTAL | 0 | 49 | 130 | 31 | 7 | 0 | 52 | 136 | 61 | 8 | 1 | 74 | 341 | 36 | 26 | 0 | 29 | 344 | 39 | 23 |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 4:00 PM - 4:15 PM | 0 | 16 | 17 | 8 | 0 | 0 | 5 | 22 | 7 | 0 | 0 | 12 | 69 | 10 | 5 | 0 | 4 | 61 | 12 | 4 |
| 4:15 PM - 4:30 PM | 0 | 6 | 25 | 5 | 1 | 0 | 7 | 14 | 9 | 0 | 0 | 5 | 61 | 9 | 1 | 0 | 8 | 54 | 10 | 1 |
| 4:30 PM - 4:45 PM | 0 | 9 | 18 | 5 | 0 | 0 | 13 | 14 | 5 | 0 | 0 | 10 | 56 | 5 | 1 | 0 | 8 | 54 | 7 | 1 |
| 4:45 PM - 5:00 PM | 0 | 6 | 13 | 2 | 0 | 0 | 10 | 14 | 12 | 0 | 0 | 11 | 61 | 3 | 2 | 0 | 9 | 47 | 10 | 0 |
| 5:00 PM - 5:15 PM | 0 | 8 | 24 | 5 | 0 | 0 | 3 | 17 | 13 | 0 | 0 | 10 | 51 | 5 | 1 | 0 | 8 | 41 | 7 | 1 |
| 5:15 PM - 5:30 PM | 1 | 12 | 12 | 7 | 0 | 0 | 6 | 13 | 11 | 0 | 0 | 9 | 53 | 5 | 3 | 0 | 7 | 54 | 8 | 0 |
| 5:30 PM - 5:45 PM | 1 | 13 | 24 | 7 | 0 | 0 | 6 | 18 | 8 | 1 | 0 | 9 | 62 | 7 | 3 | 0 | 10 | 56 | 8 | 3 |
| 5:45 PM - 6:00 PM | 0 | 7 | 21 | 7 | 0 | 0 | 4 | 20 | 10 | 0 | 0 | 9 | 63 | 2 | 1 | 0 | 0 | 46 | 9 | 1 |
| TOTAL | 2 | 77 | 154 | 46 | 1 | 0 | 54 | 132 | 75 | 1 | 0 | 75 | 476 | 46 | 17 | 0 | 54 | 413 | 71 | 11 |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PEAK HOUR | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 7:15 AM - 8:15 AM | 0 | 40 | 96 | 21 | 3 | 0 | 34 | 104 | 37 | 3 | 0 | 49 | 189 | 20 | 16 | 0 | 16 | 205 | 27 | 12 |
| 4:00 PM - 5:00 PM | 0 | 37 | 73 | 20 | 1 | 0 | 35 | 64 | 33 | 0 | 0 | 38 | 247 | 27 | 9 | 0 | 29 | 216 | 39 | 6 |



Kern St (SR 33)


##  Metro Traficic Data Inc.

Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20
Hanford, CA 93230
800-975-6938 Phone/Fax www.metrotrafficdata.com

## Turning Movement Report

Prepared For:
Rick Engineering Company
5620 Friars Rd
San Diego, CA 92110

| LOCATION | Kern St @ 4th St |
| ---: | :---: |
| COUNTY | Kern |
| COLLECTION DATE | Tuesday, March 15, 2022 |


| LATITUDE | 35.1422 |
| :---: | :---: |
| LONGITUDE | -119.4567 |
| WEATHER | Clear |


| Time | Northbound Bikes |  |  | N.Leg Peds | Southbound Bikes |  |  | S.LegPeds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 7:00 AM - 7:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| 7:15 AM - 7:30 AM | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 1 |
| 7:30 AM - 7:45 AM | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 0 |
| 7:45 AM - 8:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:00 AM - 8:15 AM | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 |
| 8:15 AM - 8:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
| 8:30 AM - 8:45 AM | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:45 AM - 9:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 13 | 0 | 0 | 0 | 3 |


| Time | Northbound Bikes |  |  | N.Leg Peds | Southbound Bikes |  |  | S.Leg <br> Peds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 4:00 PM - 4:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
| 4:15 PM - 4:30 PM | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 |
| 4:30 PM - 4:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4:45 PM - 5:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:00 PM - 5:15 PM | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 2 |
| 5:15 PM - 5:30 PM | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
| 5:30 PM - 5:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:45 PM - 6:00 PM | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 1 | 1 | 7 | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 5 | 0 | 1 | 0 | 3 |


|  | Northbound Bikes |  |  | N.Leg Peds | Southbound Bikes |  |  | S.Leg Peds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg <br> Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PEAK HOUR | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 7:15 AM - 8:15 AM | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 9 | 0 | 0 | 0 | 2 |
| 4:00 PM - 5:00 PM | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 |


|  |  |  |
| :---: | :---: | :---: |
| Bikes | Peds |  |
| AM Peak Total | 0 | 16 |
| PM Peak Total | 1 | 6 |

Kern St (SR 33)



Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20
Hanford, CA 93230
800-975-6938 Phone/Fax
www.metrotrafficdata.com

## Turning Movement Report

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

| LOCATION | Kern St @ 1st |
| ---: | :---: |
| ${ } \ldots }$ | Kern |


| LATITUDE | 35.1407 |
| :---: | :---: |
| LONGITUDE | -119.4519 |
| WEATHER | Clear |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound (SR33) |  |  |  |  | Westbound (Kern St) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | Left | Thru | R-Kern | R-33 | Trucks | L-Kern | L-33 | Thru | Right | Trucks | L-1st | L-Kern | Thru | Right | Trucks | Left | Thru | R-1st | R-Kern | Trucks | L-33 | L-1st | Thru | Right | Trucks |
| 7:00 AM - 7:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 3 | 0 | 1 | 12 | 17 | 0 | 2 | 0 | 15 | 0 | 0 | 1 | 0 | 0 | 15 | 1 | 1 |
| 7:15 AM - 7:30 AM | 1 | 0 | 2 | 2 | 0 | 0 | 2 | 0 | 7 | 0 | 4 | 15 | 18 | 5 | 3 | 0 | 19 | 0 | 0 | 2 | 0 | 2 | 24 | 2 | 1 |
| 7:30 AM - 7:45 AM | 3 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 10 | 24 | 25 | 4 | 2 | 1 | 18 | 1 | 0 | 2 | 0 | 0 | 33 | 2 | 2 |
| 7:45 AM - 8:00 AM | 1 | 0 | 1 | 0 | 0 | 4 | 2 | 1 | 4 | 0 | 1 | 26 | 24 | 7 | 1 | 0 | 9 | 0 | 0 | 0 | 0 | 3 | 30 | 0 | 0 |
| 8:00 AM - 8:15 AM | 3 | 0 | 1 | 0 | 0 | 0 | 2 | 1 | 5 | 1 | 5 | 19 | 18 | 7 | 3 | 0 | 22 | 0 | 2 | 1 | 0 | 2 | 15 | 2 | 1 |
| 8:15 AM - 8:30 AM | 4 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 1 | 16 | 34 | 2 | 3 | 0 | 19 | 0 | 0 | 6 | 0 | 0 | 10 | 1 | 0 |
| 8:30 AM - 8:45 AM | 3 | 0 | 1 | 1 | 0 | 1 | 2 | 2 | 3 | 0 | 2 | 10 | 22 | 3 | 6 | 1 | 21 | 2 | 0 | 0 | 0 | 0 | 17 | 0 | 0 |
| 8:45 AM - 9:00 AM | 2 | 0 | 1 | 1 | 0 | 0 | 4 | 0 | 3 | 0 | 3 | 21 | 13 | 4 | 1 | 1 | 12 | 1 | 0 | 2 | 1 | 0 | 11 | 1 | 1 |
| TOTAL | 17 | 1 | 8 | 4 | 0 | 5 | 16 | 4 | 36 | 1 | 27 | 143 | 171 | 32 | 21 | 3 | 135 | 4 | 2 | 14 | 1 | 7 | 155 | 9 | 6 |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound (SR33) |  |  |  |  | Westbound (Kern St) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | Left | Thru | R-Kern | R-33 | Trucks | L-Kern | L-33 | Thru | Right | Trucks | L-1st | L-Kern | Thru | Right | Trucks | Left | Thru | R-1st | R-Kern | Trucks | L-33 | L-1st | Thru | Right | Trucks |
| 4:00 PM - 4:15 PM | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 1 | 3 | 0 | 1 | 36 | 22 | 5 | 1 | 0 | 40 | 4 | 1 | 4 | 0 | 0 | 18 | 0 | 0 |
| 4:15 PM - 4:30 PM | 4 | 2 | 1 | 0 | 0 | 1 | 1 | 0 | 4 | 0 | 2 | 34 | 27 | 2 | 3 | 2 | 27 | 2 | 0 | 1 | 0 | 1 | 25 | 0 | 0 |
| 4:30 PM - 4:45 PM | 2 | 0 | 0 | 0 | 0 | 0 | 3 | 1 | 5 | 0 | 6 | 30 | 16 | 1 | 2 | 0 | 29 | 2 | 0 | 1 | 0 | 0 | 17 | 2 | 0 |
| 4:45 PM - 5:00 PM | 1 | 0 | 2 | 0 | 0 | 1 | 1 | 1 | 5 | 0 | 2 | 39 | 19 | 2 | 1 | 0 | 25 | 3 | 0 | 0 | 0 | 1 | 20 | 2 | 1 |
| 5:00 PM - 5:15 PM | 1 | 1 | 3 | 0 | 0 | 1 | 2 | 1 | 5 | 1 | 5 | 30 | 20 | 4 | 1 | 0 | 22 | 2 | 0 | 0 | 0 | 2 | 26 | 1 | 0 |
| 5:15 PM - 5:30 PM | 3 | 1 | 1 | 0 | 0 | 3 | 1 | 0 | 6 | 1 | 5 | 28 | 15 | 2 | 3 | 0 | 20 | 1 | 0 | 0 | 0 | 3 | 21 | 0 | 0 |
| 5:30 PM - 5:45 PM | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 2 | 29 | 29 | 4 | 3 | 0 | 23 | 1 | 0 | 2 | 0 | 1 | 28 | 1 | 1 |
| 5:45 PM - 6:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 7 | 0 | 5 | 34 | 24 | 4 | 1 | 0 | 24 | 2 | 0 | 1 | 0 | 1 | 17 | 1 | 0 |
| TOTAL | 15 | 5 | 8 | 0 | 0 | 6 | 9 | 5 | 40 | 2 | 28 | 260 | 172 | 24 | 15 | 2 | 210 | 17 | 1 | 9 | 0 | 9 | 172 |  |  |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound (SR33) |  |  |  |  | Westbound (Kern St) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PEAK HOUR | Left | Thru | R-Kern | R-33 | Trucks | L-Kern | L-33 | Thru | Right | Trucks | L-1st | L-Kern | Thru | Right | Trucks | Left | Thru | R-1st | R-Kern | Trucks | L-33 | L-1st | Thru | Right | Trucks |
| 7:15 AM - 8:15 AM | 8 | 0 | 5 | 2 | 0 | 4 | 6 | 2 | 23 | 1 | 20 | 84 | 85 | 23 | 9 | 1 | 68 | 1 | 2 | 5 | 0 | 7 | 102 | 6 | 4 |
| 4:00 PM - 5:00 PM | 9 | 2 | 4 | 0 | 0 | 2 | 5 | 3 | 17 | 0 | 11 | 139 | 84 | 10 | 7 | 2 | 121 | 11 | 1 | 6 | 0 | 2 | 80 | 4 | 1 |

Kern St (SR 33)


1 st St

## Nem Metro Traftic Data Inc.

Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20
Hanford, CA 93230
800-975-6938 Phone/Fax
www.metrotrafficdata.com

## Turning Movement Report

Prepared For:
Rick Enginering 5620 Friars Rd an Diego, CA 92110

| LOCATION | Kern St @ 1st |
| ---: | :---: |
| COUNTY | Kern |
| COLLECTION DATE | Tuesday, March 15, 2022 |


| LATITUDE | 35.1407 |
| :---: | :---: |
| LONGITUDE | -119.4519 |
| WEATHER | Clear |


|  | Northbound Bicycles |  |  |  | Southbound Bicycles |  |  |  | Eastbound Bicycles |  |  |  | Westbound SR33 Bicycles |  |  |  | Westbound Kern Bicycles |  |  |  | Pedestrians in each Crosswalk |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | Left | Thru | Right-1 | Right-2 | Left-1 | Left-2 | Thru | Right | Left-1 | Left-2 | Thru | Right | Left | Thru | Right-1 | Right-2 | Left-1 | Left-2 | Thru | Right | S-Leg | N-Leg | W-Leg | E33-Leg | EK-Leg |
| 7:00 AM - 7:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:15 AM - 7:30 AM | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:30 AM - 7:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 0 | 0 |
| 7:45 AM - 8:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:00 AM - 8:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 3 | 0 | 0 |
| 8:15 AM - 8:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:30 AM - 8:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:45 AM - 9:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 2 | 5 | 0 | 0 |


|  | Northbound Bicycles |  |  |  | Southbound Bicycles |  |  |  | Eastbound Bicycles |  |  |  | Westbound SR33 Bicycles |  |  |  | Westbound Kern Bicycles |  |  |  | Pedestrians in each Crosswalk |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | Left | Thru | Right-1 | Right-2 | Left-1 | Left-2 | Thru | Right | Left-1 | Left-2 | Thru | Right | Left | Thru | Right-1 | Right-2 | Left-1 | Left-2 | Thru | Right | S-Leg | N-Leg | W-Leg | E33-Leg | EK-Leg |
| 4:00 PM - 4:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |
| 4:15 PM - 4:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4:30 PM - 4:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 |
| 4:45 PM - 5:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:00 PM - 5:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:15 PM - 5:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:30 PM - 5:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 2 |
| 5:45 PM - 6:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 0 | 0 |
| TOTAL | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 1 | 0 | 4 | 2 | 2 | 2 |


|  | Northbound Bicycles |  |  |  | Southbound Bicycles |  |  |  | Eastbound Bicycles |  |  |  | Westbound SR33 Bicycles |  |  |  | Westbound Kern Bicycles |  |  |  | Pedestrians in each Crosswalk |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PEAK HOUR | Left | Thru | Right-1 | Right-2 | Left-1 | Left-2 | Thru | Right | Left-1 | Left-2 | Thru | Right | Left | Thru | Right-1 | Right-2 | Left-1 | Left-2 | Thru | Right | S-Leg | N-Leg | W-Leg | E33-Leg | EK-Leg |
| 7:15 AM - 8:15 AM | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 5 | 0 | 0 |
| 4:00 PM - 5:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 |


|  |  |  |
| :---: | :---: | :---: |
|  | Bikes | Peds |
| AM Peak Total | 2 | 7 |
| PM Peak Total | 3 | 1 |



1 st St

##  MetroTatafic Datalnc.

Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

# Turning Movement Report 

Prepared For:

Rick Engineering Company

| LOCATION | Center St @ West Side Hwy | LATITUDE |
| ---: | :---: | :---: |
| COUNTY | Kern | LONGITUDE |
| COLLECTION DATE | Tuesday, March 15,2022 | WEATHER |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 7:00 AM - 7:15 AM | 0 | 2 | 14 | 0 | 1 | 0 | 0 | 18 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:15 AM - 7:30 AM | 0 | 2 | 19 | 0 | 2 | 0 | 0 | 25 | 0 | 2 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:30 AM - 7:45 AM | 0 | 2 | 18 | 0 | 3 | 0 | 0 | 24 | 0 | 1 | 0 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:45 AM - 8:00 AM | 0 | 2 | 8 | 0 | 1 | 0 | 0 | 22 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:00 AM - 8:15 AM | 0 | 1 | 23 | 0 | 1 | 1 | 0 | 20 | 2 | 4 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:15 AM - 8:30 AM | 0 | 1 | 21 | 0 | 6 | 0 | 0 | 31 | 0 | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:30 AM - 8:45 AM | 0 | 1 | 24 | 0 | 0 | 0 | 0 | 22 | 0 | 5 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:45 AM - 9:00 AM | 0 | 1 | 14 | 0 | 2 | 0 | 0 | 19 | 1 | 1 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 12 | 141 | 0 | 16 | 1 | 0 | 181 | 3 | 16 | 0 | 4 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 4:00 PM - 4:15 PM | 0 | 3 | 36 | 0 | 3 | 0 | 2 | 20 | 1 | 3 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 1 | 1 | 0 |
| 4:15 PM - 4:30 PM | 0 | 1 | 26 | 0 | 1 | 0 | 0 | 23 | 0 | 1 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4:30 PM - 4:45 PM | 0 | 0 | 31 | 0 | 1 | 0 | 0 | 16 | 0 | 1 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4:45 PM - 5:00 PM | 0 | 1 | 26 | 0 | 0 | 0 | 0 | 19 | 0 | 1 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:00 PM - 5:15 PM | 0 | 3 | 21 | 0 | 0 | 0 | 0 | 16 | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:15 PM - 5:30 PM | 0 | 3 | 15 | 0 | 0 | 0 | 0 | 16 | 0 | 3 | 0 | 1 | 0 | 4 | 1 | 0 | 0 | 0 | 0 | 0 |
| 5:30 PM - 5:45 PM | 0 | 2 | 21 | 0 | 2 | 0 | 0 | 26 | 0 | 1 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:45 PM - 6:00 PM | 0 | 5 | 25 | 0 | 1 | 0 | 0 | 21 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 18 | 201 | 0 | 8 | 0 | 2 | 157 | 1 | 11 | 0 | 2 | 0 | 24 | 1 | 0 | 0 | 1 | 1 | 0 |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PEAK HOUR | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 8:00 AM - 9:00 AM | 0 | 4 | 82 | 0 | 9 | 1 | 0 | 92 | 3 | 12 | 0 | 2 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4:00 PM - 5:00 PM | 0 | 5 | 119 | 0 | 5 | 0 | 2 | 78 | 1 | 6 | 0 | 0 | 0 | 13 | 0 | 0 | 0 | 1 | 1 | 0 |



Center St


West Side Hwy (SR 33)

##  Metro Traficic Data Inc.

Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20
Hanford, CA 93230
800-975-6938 Phone/Fax www.metrotrafficdata.com

## Turning Movement Report

Prepared For:
Rick Engineering Company
5620 Friars Rd
San Diego, CA 92110

| LOCATION | Center St @ West Side Hwy |
| ---: | :---: |
| COUNTY | Kern |
| COLLECTION DATE | Tuesday, March 15, 2022 |


| LATITUDE | 35.1387 |
| ---: | :---: |
| LONGITUDE | -119.4489 |
| WEATHER | Clear |


| Time | Northbound Bikes |  |  | N.Leg Peds | Southbound Bikes |  |  | S.Leg Peds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 7:00 AM - 7:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:15 AM - 7:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:30 AM - 7:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:45 AM - 8:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:00 AM - 8:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:15 AM - 8:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:30 AM - 8:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:45 AM - 9:00 AM | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |


| Time | Northbound Bikes |  |  | N.Leg Peds | Southbound Bikes |  |  | S.Leg <br> Peds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 4:00 PM - 4:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4:15 PM - 4:30 PM | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4:30 PM - 4:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4:45 PM - 5:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:00 PM - 5:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
| 5:15 PM - 5:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:30 PM - 5:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:45 PM - 6:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |


|  | Northbound Bikes |  |  | N.Leg Peds | Southbound Bikes |  |  | S.Leg <br> Peds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg <br> Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PEAK HOUR | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 8:00 AM - 9:00 AM | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4:00 PM - 5:00 PM | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |


|  |  |  |
| :---: | :---: | :---: |
| AM Peak Total | 1 | 0 |
| Peds |  |  |
| PM Peak Total | 1 | 0 |

Center St


Driveway

##  Mato Tatatic Datalac.

Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

# Turning Movement Report 

Prepared For:

Rick Engineering Company
LOCATION $\quad$ 10th St @ Center St

| LATITUDE | 35.1429 |
| :---: | :---: |
| LONGITUDE | -119.4651 |
| WEATHER | Clear | -


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 7:00 AM - 7:15 AM | 0 | 0 | 70 | 6 | 2 | 0 | 6 | 39 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 4 | 0 |
| 7:15 AM - 7:30 AM | 0 | 0 | 108 | 7 | 2 | 0 | 1 | 48 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 2 | 1 |
| 7:30 AM - 7:45 AM | 0 | 0 | 116 | 12 | 10 | 0 | 10 | 103 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 6 | 0 |
| 7:45 AM - 8:00 AM | 0 | 0 | 86 | 10 | 0 | 0 | 10 | 77 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 5 | 0 |
| 8:00 AM - 8:15 AM | 0 | 0 | 65 | 13 | 2 | 0 | 9 | 50 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 3 | 0 |
| 8:15 AM - 8:30 AM | 0 | 0 | 58 | 7 | 7 | 0 | 2 | 45 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 5 | 0 |
| 8:30 AM - 8:45 AM | 0 | 0 | 51 | 4 | 0 | 0 | 5 | 32 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 4 | 1 |
| 8:45 AM - 9:00 AM | 0 | 0 | 43 | 6 | 0 | 0 | 6 | 29 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 10 | 1 |
| TOTAL | 0 | 0 | 597 | 65 | 23 | 0 | 49 | 423 | 0 | 16 | 0 | 0 | 0 | 0 | 0 | 0 | 28 | 0 | 39 | 3 |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 4:00 PM - 4:15 PM | 0 | 0 | 70 | 12 | 1 | 0 | 18 | 71 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 9 | 0 | 15 | 0 |
| 4:15 PM - 4:30 PM | 0 | 0 | 74 | 14 | 2 | 0 | 11 | 79 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 14 | 0 |
| 4:30 PM - 4:45 PM | 0 | 0 | 79 | 10 | 1 | 0 | 10 | 91 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 13 | 0 |
| 4:45 PM - 5:00 PM | 0 | 0 | 92 | 11 | 0 | 0 | 11 | 76 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 10 | 0 |
| 5:00 PM - 5:15 PM | 0 | 0 | 89 | 13 | 2 | 0 | 19 | 89 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 0 | 15 | 0 |
| 5:15 PM - 5:30 PM | 0 | 0 | 83 | 17 | 1 | 0 | 16 | 83 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 11 | 0 |
| 5:30 PM - 5:45 PM | 0 | 0 | 68 | 9 | 0 | 0 | 18 | 68 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 6 | 0 |
| 5:45 PM - 6:00 PM | 0 | 0 | 73 | 17 | 1 | 0 | 19 | 69 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 12 | 1 |
| TOTAL | 0 | 0 | 628 | 103 | 8 | 0 | 122 | 626 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 55 | 0 | 96 | 1 |


|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Eastbound |  |  |  |  | Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PEAK HOUR | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks | U-Turn | Left | Thru | Right | Trucks |
| 7:15 AM - 8:15 AM | 0 | 0 | 375 | 42 | 14 | 0 | 30 | 278 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 16 | 1 |
| 4:30 PM - 5:30 PM | 0 | 0 | 343 | 51 | 4 | 0 | 56 | 339 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 27 | 0 | 49 | 0 |



10 th St

##  Metro Traficic Data Inc.

Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20
Hanford, CA 93230
800-975-6938 Phone/Fax www.metrotrafficdata.com

## Turning Movement Report

Prepared For:
Rick Engineering Company
5620 Friars Rd
San Diego, CA 92110

| LOCATION | Center St @ West Side Hwy |
| ---: | :---: |
| COUNTY | Kern |
| COLLECTION DATE | Tuesday, March 15, 2022 |


| LATITUDE | 35.1387 |
| ---: | :---: |
| LONGITUDE | -119.4489 |
| WEATHER | Clear |


| Time | Northbound Bikes |  |  | N.Leg Peds | Southbound Bikes |  |  | S.Leg Peds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 7:00 AM - 7:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:15 AM - 7:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:30 AM - 7:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:45 AM - 8:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:00 AM - 8:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:15 AM - 8:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:30 AM - 8:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:45 AM - 9:00 AM | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |


| Time | Northbound Bikes |  |  | N.Leg Peds | Southbound Bikes |  |  | S.Leg <br> Peds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 4:00 PM - 4:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4:15 PM - 4:30 PM | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4:30 PM - 4:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4:45 PM - 5:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:00 PM - 5:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
| 5:15 PM - 5:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:30 PM - 5:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:45 PM - 6:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |


|  | Northbound Bikes |  |  | N.Leg Peds | Southbound Bikes |  |  | S.Leg <br> Peds | Eastbound Bikes |  |  | E.Leg Peds | Westbound Bikes |  |  | W.Leg <br> Peds |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PEAK HOUR | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  | Left | Thru | Right |  |
| 8:00 AM - 9:00 AM | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4:00 PM - 5:00 PM | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |


|  |  |  |
| :---: | :---: | :---: |
| AM Peak Total | 1 | 0 |
| Peds |  |  |
| PM Peak Total | 1 | 0 |

Center St


Driveway

Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20
Hanford, CA 93230
800-975-6938 Phone/Fax
www.metrotrafficdata.com

## 24 Hour Count Report

Prepared For:

| STREET | Kern St (SR33) | LATITUDE | 35.14327294 |
| ---: | :---: | :---: | :---: |
| SEGMENT | btwn $10 \mathrm{th} \mathrm{St} / 6 \mathrm{th} \mathrm{St}$ | LONGITUDE |  |
| COLLECTION DATE | Tuesday, March 15,2022 | WEATHER | Clear |

NUMBER OF LANES $\qquad$

|  | Eastbound |  |  |  |  | Westbound |  |  |  |  | Hourly <br> Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hour | 1st | 2nd | 3rd | 4th | Total | 1st | 2nd | 3rd | 4th | Total |  |
| 12:00 AM | 6 | 3 | 3 | 4 | 16 | 5 | 1 | 6 | 3 | 15 | 31 |
| 1:00 AM | 2 | 2 | 2 | 4 | 10 | 2 | 4 | 3 | 2 | 11 | 21 |
| 2:00 AM | 2 | 0 | 4 | 1 | 7 | 1 | 1 | 3 | 0 | 5 | 12 |
| 3:00 AM | 2 | 4 | 4 | 2 | 12 | 2 | 4 | 3 | 3 | 12 | 24 |
| 4:00 AM | 11 | 15 | 12 | 12 | 50 | 9 | 14 | 12 | 16 | 51 | 101 |
| 5:00 AM | 22 | 24 | 37 | 27 | 110 | 26 | 34 | 51 | 43 | 154 | 264 |
| 6:00 AM | 37 | 41 | 36 | 47 | 161 | 49 | 38 | 43 | 46 | 176 | 337 |
| 7:00 AM | 54 | 71 | 89 | 92 | 306 | 36 | 79 | 120 | 89 | 324 | 630 |
| 8:00 AM | 66 | 61 | 43 | 54 | 224 | 60 | 48 | 43 | 56 | 207 | 431 |
| 9:00 AM | 55 | 50 | 56 | 56 | 217 | 53 | 61 | 58 | 59 | 231 | 448 |
| 10:00 AM | 61 | 58 | 55 | 72 | 246 | 72 | 64 | 67 | 95 | 298 | 544 |
| 11:00 AM | 96 | 83 | 78 | 68 | 325 | 74 | 74 | 82 | 75 | 305 | 630 |
| 12:00 PM | 84 | 83 | 71 | 69 | 307 | 80 | 92 | 86 | 76 | 334 | 641 |
| 1:00 PM | 83 | 74 | 65 | 64 | 286 | 72 | 69 | 69 | 83 | 293 | 579 |
| 2:00 PM | 84 | 102 | 87 | 103 | 376 | 92 | 85 | 105 | 93 | 375 | 751 |
| 3:00 PM | 105 | 88 | 102 | 99 | 394 | 104 | 92 | 102 | 81 | 379 | 773 |
| 4:00 PM | 93 | 70 | 81 | 96 | 340 | 88 | 76 | 79 | 81 | 324 | 664 |
| 5:00 PM | 90 | 73 | 96 | 81 | 340 | 85 | 84 | 83 | 73 | 325 | 665 |
| 6:00 PM | 74 | 61 | 67 | 62 | 264 | 83 | 72 | 54 | 61 | 270 | 534 |
| 7:00 PM | 61 | 51 | 57 | 38 | 207 | 58 | 51 | 56 | 55 | 220 | 427 |
| 8:00 PM | 54 | 34 | 50 | 31 | 169 | 44 | 37 | 43 | 42 | 166 | 335 |
| 9:00 PM | 47 | 48 | 35 | 32 | 162 | 38 | 31 | 29 | 27 | 125 | 287 |
| 10:00 PM | 27 | 13 | 9 | 10 | 59 | 15 | 15 | 10 | 9 | 49 | 108 |
| 11:00 PM | 8 | 4 | 3 | 5 | 20 | 9 | 3 | 3 | 6 | 21 | 41 |
| Total | 49.7\% |  |  |  | 4608 | 50.3\% |  |  |  | 4670 |  |
|  | 9278 |  |  |  |  |  |  |  |  |  |  |
| AM\% | 37.4\% | AM Peak 666 |  |  | 7:15 am to 8:15 am |  |  | AM P.H.F. 0.80 |  |  |  |
| PM\% | 62.6\% | PM Peak 789 |  |  | 2:45 pm to 3:45 pm |  |  | PM P.H.F. |  | 0.94 |  |



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800-975-6938 Phone/Fax
www.metrotrafficdata.com

## 24 Hour Count Report

Prepared For:

| STREET | Kern St (SR33) | LATITUDE | 35.14207405 |
| :---: | :---: | :---: | :---: |
| SEGMENT | btwn 6th St / 1st St | LONGITUDE | -119.4563608 |
| COLLECTION DATE | Tuesday, March 15, 2022 | WEATHER | Clear |

NUMBER OF LANES $\qquad$

|  | Eastbound |  |  |  |  | Westbound |  |  |  |  | Hourly <br> Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hour | 1st | 2nd | 3rd | 4th | Total | 1st | 2nd | 3rd | 4th | Total |  |
| 12:00 AM | 2 | 4 | 4 | 2 | 12 | 5 | 2 | 6 | 3 | 16 | 28 |
| 1:00 AM | 0 | 1 | 4 | 2 | 7 | 2 | 6 | 2 | 0 | 10 | 17 |
| 2:00 AM | 2 | 2 | 4 | 1 | 9 | 1 | 1 | 3 | 0 | 5 | 14 |
| 3:00 AM | 1 | 3 | 5 | 2 | 11 | 2 | 3 | 3 | 4 | 12 | 23 |
| 4:00 AM | 9 | 12 | 12 | 18 | 51 | 7 | 12 | 7 | 10 | 36 | 87 |
| 5:00 AM | 16 | 22 | 37 | 37 | 112 | 24 | 25 | 35 | 41 | 125 | 237 |
| 6:00 AM | 40 | 50 | 37 | 41 | 168 | 35 | 36 | 31 | 42 | 144 | 312 |
| 7:00 AM | 41 | 49 | 75 | 68 | 233 | 36 | 59 | 77 | 65 | 237 | 470 |
| 8:00 AM | 52 | 56 | 41 | 42 | 191 | 47 | 39 | 44 | 45 | 175 | 366 |
| 9:00 AM | 47 | 44 | 49 | 47 | 187 | 41 | 47 | 56 | 48 | 192 | 379 |
| 10:00 AM | 56 | 50 | 48 | 62 | 216 | 46 | 53 | 57 | 80 | 236 | 452 |
| 11:00 AM | 72 | 64 | 62 | 63 | 261 | 62 | 51 | 55 | 56 | 224 | 485 |
| 12:00 PM | 58 | 78 | 54 | 64 | 254 | 59 | 63 | 71 | 56 | 249 | 503 |
| 1:00 PM | 67 | 68 | 44 | 52 | 231 | 72 | 51 | 50 | 70 | 243 | 474 |
| 2:00 PM | 75 | 81 | 60 | 61 | 277 | 70 | 66 | 81 | 87 | 304 | 581 |
| 3:00 PM | 80 | 67 | 96 | 89 | 332 | 82 | 52 | 79 | 78 | 291 | 623 |
| 4:00 PM | 82 | 73 | 74 | 73 | 302 | 77 | 72 | 69 | 66 | 284 | 586 |
| 5:00 PM | 59 | 66 | 75 | 74 | 274 | 56 | 69 | 74 | 55 | 254 | 528 |
| 6:00 PM | 62 | 54 | 56 | 53 | 225 | 53 | 67 | 44 | 49 | 213 | 438 |
| 7:00 PM | 41 | 50 | 51 | 37 | 179 | 44 | 52 | 43 | 32 | 171 | 350 |
| 8:00 PM | 42 | 29 | 29 | 29 | 129 | 31 | 28 | 40 | 26 | 125 | 254 |
| 9:00 PM | 35 | 35 | 22 | 18 | 110 | 28 | 25 | 18 | 14 | 85 | 195 |
| 10:00 PM | 16 | 12 | 10 | 8 | 46 | 12 | 11 | 5 | 6 | 34 | 80 |
| 11:00 PM | 8 | 5 | 3 | 4 | 20 | 6 | 4 | 4 | 4 | 18 | 38 |
| Total | 51.0\% |  |  |  | 3837 | 49.0\% |  |  |  | 3683 |  |
|  | 7520 |  |  |  |  |  |  |  |  |  |  |
| AM\% | 38.2\% | AM Peak 508 |  |  | 10:45 am to 11:45 am |  |  | AM P.H.F. |  | 0.89 |  |
| PM\% | 61.8\% | PM Peak 646 |  |  | 3:30 pm to 4:30 pm |  |  | PM P.H.F. |  | 0.92 |  |



## Metro Traffic Data Inc.

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Hanford, CA 93230
800-975-6938 Phone/Fax
www.metrotrafficdata.com

## 24 Hour Count Report

Prepared For

## Rick Engineering Company

5620 Friars Rd
San Diego, CA 92110

| STREET | Westside Hwy (SR 33) | LATITUDE | 35.139115 |
| :---: | :---: | :---: | :---: |
| SEGMENT | btwn 1st St / Center St | LONGITUDE | -119.4491566 |
| COLLECTION DATE | Tuesday, March 15, 2022 | WEATHER | Clear |

NUMBER OF LANES $\qquad$

|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Hourly <br> Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hour | 1st | 2nd | 3rd | 4th | Total | 1st | 2nd | 3rd | 4th | Total |  |
| 12:00 AM | 2 | 3 | 2 | 2 | 9 | 1 | 1 | 1 | 1 | 4 | 13 |
| 1:00 AM | 2 | 2 | 1 | 0 | 5 | 2 | 1 | 0 | 2 | 5 | 10 |
| 2:00 AM | 2 | 0 | 3 | 0 | 5 | 0 | 0 | 3 | 2 | 5 | 10 |
| 3:00 AM | 1 | 2 | 1 | 1 | 5 | 1 | 2 | 4 | 2 | 9 | 14 |
| 4:00 AM | 3 | 5 | 1 | 3 | 12 | 5 | 8 | 8 | 8 | 29 | 41 |
| 5:00 AM | 6 | 6 | 21 | 12 | 45 | 5 | 16 | 24 | 25 | 70 | 115 |
| 6:00 AM | 15 | 19 | 12 | 18 | 64 | 33 | 41 | 34 | 24 | 132 | 196 |
| 7:00 AM | 14 | 20 | 19 | 8 | 61 | 18 | 25 | 24 | 22 | 89 | 150 |
| 8:00 AM | 24 | 22 | 25 | 14 | 85 | 23 | 31 | 22 | 20 | 96 | 181 |
| 9:00 AM | 19 | 20 | 23 | 21 | 83 | 24 | 18 | 23 | 19 | 84 | 167 |
| 10:00 AM | 18 | 24 | 24 | 32 | 98 | 25 | 21 | 22 | 23 | 91 | 189 |
| 11:00 AM | 20 | 28 | 34 | 26 | 108 | 33 | 26 | 21 | 29 | 109 | 217 |
| 12:00 PM | 30 | 22 | 24 | 18 | 94 | 21 | 37 | 24 | 26 | 108 | 202 |
| 1:00 PM | 27 | 22 | 20 | 30 | 99 | 27 | 29 | 23 | 25 | 104 | 203 |
| 2:00 PM | 22 | 30 | 31 | 37 | 120 | 23 | 24 | 23 | 22 | 92 | 212 |
| 3:00 PM | 33 | 24 | 45 | 47 | 149 | 21 | 23 | 25 | 24 | 93 | 242 |
| 4:00 PM | 37 | 26 | 31 | 26 | 120 | 23 | 23 | 16 | 19 | 81 | 201 |
| 5:00 PM | 22 | 16 | 21 | 25 | 84 | 16 | 16 | 26 | 21 | 79 | 163 |
| 6:00 PM | 21 | 22 | 11 | 12 | 66 | 17 | 11 | 19 | 13 | 60 | 126 |
| 7:00 PM | 11 | 14 | 10 | 9 | 44 | 16 | 17 | 9 | 11 | 53 | 97 |
| 8:00 PM | 9 | 6 | 9 | 6 | 30 | 15 | 8 | 9 | 11 | 43 | 73 |
| 9:00 PM | 7 | 11 | 5 | 2 | 25 | 12 | 10 | 12 | 7 | 41 | 66 |
| 10:00 PM | 4 | 2 | 2 | 2 | 10 | 8 | 4 | 2 | 3 | 17 | 27 |
| 11:00 PM | 2 | 3 | 0 | 3 | 8 | 0 | 4 | 1 | 1 | 6 | 14 |
| Total | 48.8\% |  |  |  | 1429 | 51.2\% |  |  |  | 1500 |  |
|  | 2929 |  |  |  |  |  |  |  |  |  |  |
| AM\% | 44.5\% | AM Peak 217 |  |  | 11:00 am to 12:00 pm |  |  | AM P.H.F. 0.99 |  |  |  |
| PM\% | 55.5\% | PM Peak 250 |  |  | 3:30 pm to 4:30 pm |  |  | PM P.H.F. |  | 0.88 |  |



Metro Traffic Data Inc.
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## 24 Hour Count Report

Prepared For:

NUMBER OF LANES $\qquad$

|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Hourly <br> Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hour | 1st | 2nd | 3rd | 4th | Total | 1st | 2nd | 3rd | 4th | Total |  |
| 12:00 AM | 3 | 3 | 2 | 3 | 11 | 1 | 1 | 1 | 1 | 4 | 15 |
| 1:00 AM | 2 | 2 | 1 | 0 | 5 | 2 | 1 | 0 | 2 | 5 | 10 |
| 2:00 AM | 2 | 0 | 3 | 0 | 5 | 0 | 0 | 3 | 2 | 5 | 10 |
| 3:00 AM | 1 | 2 | 1 | 2 | 6 | 1 | 2 | 4 | 2 | 9 | 15 |
| 4:00 AM | 3 | 6 | 1 | 3 | 13 | 5 | 8 | 8 | 9 | 30 | 43 |
| 5:00 AM | 6 | 6 | 23 | 12 | 47 | 5 | 17 | 26 | 30 | 78 | 125 |
| 6:00 AM | 17 | 21 | 13 | 21 | 72 | 36 | 46 | 37 | 29 | 148 | 220 |
| 7:00 AM | 16 | 21 | 20 | 10 | 67 | 19 | 26 | 27 | 23 | 95 | 162 |
| 8:00 AM | 24 | 22 | 25 | 15 | 86 | 21 | 31 | 23 | 22 | 97 | 183 |
| 9:00 AM | 19 | 20 | 23 | 25 | 87 | 23 | 19 | 25 | 20 | 87 | 174 |
| 10:00 AM | 18 | 28 | 26 | 33 | 105 | 27 | 20 | 26 | 28 | 101 | 206 |
| 11:00 AM | 27 | 32 | 37 | 31 | 127 | 31 | 26 | 28 | 32 | 117 | 244 |
| 12:00 PM | 31 | 24 | 25 | 21 | 101 | 24 | 38 | 26 | 26 | 114 | 215 |
| 1:00 PM | 31 | 26 | 23 | 30 | 110 | 28 | 30 | 27 | 26 | 111 | 221 |
| 2:00 PM | 27 | 27 | 35 | 40 | 129 | 25 | 27 | 25 | 28 | 105 | 234 |
| 3:00 PM | 37 | 27 | 49 | 51 | 164 | 26 | 28 | 30 | 27 | 111 | 275 |
| 4:00 PM | 39 | 27 | 31 | 27 | 124 | 23 | 25 | 21 | 22 | 91 | 215 |
| 5:00 PM | 24 | 18 | 23 | 30 | 95 | 18 | 20 | 29 | 23 | 90 | 185 |
| 6:00 PM | 23 | 25 | 10 | 12 | 70 | 20 | 15 | 20 | 15 | 70 | 140 |
| 7:00 PM | 10 | 14 | 11 | 9 | 44 | 19 | 17 | 13 | 11 | 60 | 104 |
| 8:00 PM | 9 | 6 | 9 | 6 | 30 | 16 | 9 | 10 | 12 | 47 | 77 |
| 9:00 PM | 7 | 11 | 5 | 2 | 25 | 13 | 12 | 13 | 8 | 46 | 71 |
| 10:00 PM | 4 | 2 | 2 | 2 | 10 | 8 | 4 | 3 | 3 | 18 | 28 |
| 11:00 PM | 2 | 3 | 0 | 3 | 8 | 1 | 4 | 1 | 1 | 7 | 15 |
| Total | 48.4\% |  |  |  | 1541 | 51.6\% |  |  |  | 1646 |  |
|  | 3187 |  |  |  |  |  |  |  |  |  |  |
| AM\% | 44.1\% | AM Peak 244 |  |  | 11:00 am to 12:00 pm |  |  | AM P.H.F. 0.94 |  |  |  |
| PM\% | 55.9\% | PM Peak 275 |  |  | 3:00 pm to 4:00 pm |  |  | PM P.H.F. |  | 0.87 |  |



Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

## 24 Hour Count Report

Prepared For:

| STREET | Center St | LATITUDE |  |
| ---: | :---: | :---: | :---: |
| SEGMENT | btwn 10 th $\mathrm{St} / 6 \mathrm{th} \mathrm{St}$ | LONGITUDE | -119.4648361 |
|  | Tuesday, March 15,2022 | WEATHER | Clear |

NUMBER OF LANES $\qquad$

|  | Eastbound |  |  |  |  | Westbound |  |  |  |  | Hourly Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hour | 1st | 2nd | 3rd | 4th | Total | 1st | 2nd | 3rd | 4th | Total |  |
| 12:00 AM | 0 | 0 | 1 | 2 | 3 | 1 | 1 | 1 | 0 | 3 | 6 |
| 1:00 AM | 0 | 0 | 3 | 0 | 3 | 0 | 0 | 1 | 0 | 1 | 4 |
| 2:00 AM | 0 | 0 | 0 | 2 | 2 | 0 | 0 | 1 | 0 | 1 | 3 |
| 3:00 AM | 0 | 1 | 1 | 2 | 4 | 0 | 0 | 1 | 2 | 3 | 7 |
| 4:00 AM | 4 | 6 | 8 | 5 | 23 | 0 | 1 | 2 | 1 | 4 | 27 |
| 5:00 AM | 13 | 15 | 6 | 8 | 42 | 4 | 4 | 6 | 6 | 20 | 62 |
| 6:00 AM | 8 | 6 | 6 | 7 | 27 | 7 | 5 | 4 | 11 | 27 | 54 |
| 7:00 AM | 12 | 8 | 22 | 20 | 62 | 5 | 9 | 9 | 8 | 31 | 93 |
| 8:00 AM | 22 | 9 | 9 | 12 | 52 | 5 | 9 | 7 | 15 | 36 | 88 |
| 9:00 AM | 16 | 22 | 12 | 17 | 67 | 5 | 14 | 14 | 11 | 44 | 111 |
| 10:00 AM | 23 | 18 | 26 | 14 | 81 | 8 | 13 | 13 | 20 | 54 | 135 |
| 11:00 AM | 24 | 29 | 20 | 23 | 96 | 17 | 27 | 28 | 20 | 92 | 188 |
| 12:00 PM | 23 | 23 | 19 | 16 | 81 | 15 | 18 | 18 | 19 | 70 | 151 |
| 1:00 PM | 25 | 20 | 20 | 30 | 95 | 22 | 18 | 6 | 15 | 61 | 156 |
| 2:00 PM | 20 | 27 | 27 | 33 | 107 | 20 | 20 | 24 | 16 | 80 | 187 |
| 3:00 PM | 28 | 21 | 29 | 27 | 105 | 20 | 15 | 25 | 22 | 82 | 187 |
| 4:00 PM | 30 | 25 | 20 | 22 | 97 | 24 | 20 | 19 | 18 | 81 | 178 |
| 5:00 PM | 32 | 33 | 27 | 36 | 128 | 26 | 13 | 13 | 18 | 70 | 198 |
| 6:00 PM | 27 | 17 | 20 | 24 | 88 | 28 | 25 | 17 | 18 | 88 | 176 |
| 7:00 PM | 26 | 19 | 12 | 23 | 80 | 20 | 17 | 16 | 13 | 66 | 146 |
| 8:00 PM | 26 | 18 | 10 | 15 | 69 | 16 | 14 | 14 | 10 | 54 | 123 |
| 9:00 PM | 11 | 6 | 8 | 6 | 31 | 4 | 12 | 5 | 10 | 31 | 62 |
| 10:00 PM | 4 | 1 | 0 | 1 | 6 | 2 | 3 | 0 | 0 | 5 | 11 |
| 11:00 PM | 2 | 1 | 2 | 0 | 5 | 0 | 1 | 0 | 1 | 2 | 7 |
| Total | 57.4\% |  |  |  | 1354 | 42.6\% |  |  |  | 1006 |  |
|  | 2360 |  |  |  |  |  |  |  |  |  |  |
| AM\% | 33.0\% | AM Peak 188 |  |  | 11:00 am to 12:00 pm |  |  | AM P.H.F. 0.84 |  |  |  |
| PM\% | 67.0\% | PM Peak 198 |  |  | 5:00 pm to 6:00 pm |  |  | PM P.H.F. |  | 0.85 |  |



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## 24 Hour Count Report

Prepared For:

NUMBER OF LANES $\qquad$

|  | Eastbound |  |  |  |  | Westbound |  |  |  |  | Hourly |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hour | 1st | 2nd | 3rd | 4th | Total | 1st | 2nd | 3rd | 4th | Total | Totals |
| 12:00 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 2 | 2 |
| 1:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 2:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 3:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 1 |
| 4:00 AM | 0 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 1 | 2 |
| 5:00 AM | 0 | 1 | 2 | 5 | 8 | 0 | 0 | 1 | 0 | 1 | 9 |
| 6:00 AM | 3 | 6 | 5 | 6 | 20 | 0 | 1 | 3 | 3 | 7 | 27 |
| 7:00 AM | 1 | 2 | 4 | 1 | 8 | 2 | 2 | 2 | 2 | 8 | 16 |
| 8:00 AM | 1 | 1 | 2 | 3 | 7 | 3 | 1 | 1 | 2 | 7 | 14 |
| 9:00 AM | 1 | 2 | 3 | 2 | 8 | 2 | 1 | 1 | 5 | 9 | 17 |
| 10:00 AM | 3 | 0 | 4 | 4 | 11 | 1 | 3 | 1 | 2 | 7 | 18 |
| 11:00 AM | 2 | 2 | 6 | 4 | 14 | 7 | 4 | 4 | 6 | 21 | 35 |
| 12:00 PM | 4 | 2 | 2 | 0 | 8 | 2 | 3 | 1 | 3 | 9 | 17 |
| 1:00 PM | 1 | 3 | 4 | 1 | 9 | 4 | 6 | 3 | 0 | 13 | 22 |
| 2:00 PM | 3 | 3 | 2 | 5 | 13 | 6 | 2 | 5 | 4 | 17 | 30 |
| 3:00 PM | 4 | 5 | 5 | 5 | 19 | 4 | 3 | 4 | 5 | 16 | 35 |
| 4:00 PM | 3 | 2 | 5 | 3 | 13 | 5 | 1 | 0 | 1 | 7 | 20 |
| 5:00 PM | 3 | 5 | 3 | 2 | 13 | 3 | 3 | 2 | 5 | 13 | 26 |
| 6:00 PM | 3 | 4 | 2 | 2 | 11 | 2 | 3 | 0 | 0 | 5 | 16 |
| 7:00 PM | 4 | 0 | 4 | 2 | 10 | 0 | 0 | 1 | 2 | 3 | 13 |
| 8:00 PM | 1 | 1 | 1 | 1 | 4 | 0 | 0 | 0 | 0 | 0 | 4 |
| 9:00 PM | 1 | 2 | 1 | 1 | 5 | 0 | 0 | 0 | 0 | 0 | 5 |
| 10:00 PM | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 |
| 11:00 PM | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 |
| Total | 55.6\% |  |  |  | 184 | 44.4\% |  |  |  | 147 |  |
|  | 331 |  |  |  |  |  |  |  |  |  |  |
| AM\% | 42.6\% | AM Peak 35 |  |  | 11:00 am to 12:00 pm |  |  | AM P.H.F. |  | 0.88 |  |
| PM\% | 57.4\% | PM Peak 35 |  |  | 3:15 pm to 4:15 pm |  |  | PM P.H.F. |  | 0.88 |  |



Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

## 24 Hour Count Report

Prepared For:

| STREET | Main St | LATITUDE |  |
| ---: | :---: | :---: | :---: |
| SEGMENT | btwn 10 th St $/ 6$ th St | LONGITUDE | -119.4612194 |
|  | Tuesday, March 15,2022 | WEATHER | Clear |

NUMBER OF LANES $\qquad$

|  | Eastbound |  |  |  |  | Westbound |  |  |  |  | Hourly Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hour | 1st | 2nd | 3rd | 4th | Total | 1st | 2nd | 3rd | 4th | Total |  |
| 12:00 AM | 1 | 0 | 0 | 0 | 1 | 1 | 1 | 0 | 0 | 2 | 3 |
| 1:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 1 |
| 2:00 AM | 0 | 0 | 1 | 1 | 2 | 1 | 0 | 0 | 0 | 1 | 3 |
| 3:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 3 | 0 | 4 | 4 |
| 4:00 AM | 1 | 1 | 0 | 3 | 5 | 0 | 0 | 1 | 1 | 2 | 7 |
| 5:00 AM | 2 | 1 | 3 | 5 | 11 | 0 | 3 | 1 | 2 | 6 | 17 |
| 6:00 AM | 4 | 8 | 5 | 12 | 29 | 2 | 2 | 4 | 6 | 14 | 43 |
| 7:00 AM | 12 | 17 | 26 | 18 | 73 | 9 | 3 | 8 | 7 | 27 | 100 |
| 8:00 AM | 6 | 7 | 9 | 8 | 30 | 15 | 11 | 6 | 5 | 37 | 67 |
| 9:00 AM | 6 | 7 | 7 | 11 | 31 | 4 | 9 | 11 | 8 | 32 | 63 |
| 10:00 AM | 14 | 2 | 7 | 10 | 33 | 6 | 6 | 14 | 8 | 34 | 67 |
| 11:00 AM | 9 | 7 | 12 | 18 | 46 | 10 | 20 | 8 | 18 | 56 | 102 |
| 12:00 PM | 12 | 8 | 12 | 7 | 39 | 13 | 9 | 5 | 9 | 36 | 75 |
| 1:00 PM | 6 | 6 | 6 | 8 | 26 | 12 | 9 | 10 | 9 | 40 | 66 |
| 2:00 PM | 17 | 13 | 16 | 10 | 56 | 6 | 12 | 17 | 10 | 45 | 101 |
| 3:00 PM | 15 | 16 | 12 | 10 | 53 | 12 | 12 | 11 | 15 | 50 | 103 |
| 4:00 PM | 11 | 16 | 7 | 11 | 45 | 17 | 8 | 17 | 19 | 61 | 106 |
| 5:00 PM | 6 | 13 | 7 | 8 | 34 | 19 | 16 | 7 | 12 | 54 | 88 |
| 6:00 PM | 10 | 5 | 8 | 10 | 33 | 5 | 8 | 3 | 7 | 23 | 56 |
| 7:00 PM | 10 | 3 | 5 | 7 | 25 | 14 | 11 | 7 | 11 | 43 | 68 |
| 8:00 PM | 7 | 3 | 5 | 3 | 18 | 6 | 9 | 6 | 4 | 25 | 43 |
| 9:00 PM | 9 | 2 | 1 | 2 | 14 | 6 | 4 | 3 | 3 | 16 | 30 |
| 10:00 PM | 0 | 0 | 1 | 0 | 1 | 2 | 0 | 0 | 2 | 4 | 5 |
| 11:00 PM | 0 | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 2 | 4 | 5 |
| Total | 49.6\% |  |  |  | 606 | 50.4\% |  |  |  | 617 |  |
|  | 1223 |  |  |  |  |  |  |  |  |  |  |
| AM\% | 39.0\% | AM Peak 102 |  |  | 11:00 am to 12:00 pm |  |  | AM P.H.F. |  | 0.71 |  |
| PM\% | 61.0\% | PM Peak 108 |  |  | 4:30 pm to 5:30 pm |  |  | PM P.H.F. |  | 0.90 |  |



Metro Traffic Data Inc．
310 N．Irwin Street－Suite 20 Hanford，CA 93230

800－975－6938 Phone／Fax www．metrotrafficdata．com

## 24 Hour Count Report

Prepared For：

| STREET | Main St | LATITUDE | 35．14043638 |
| ---: | :---: | :---: | :---: |
|  | btwn 6 th St $/ \mathrm{SR} 33$ | LONGITUDE |  |
| SEGMENT | Tuesday，March 15,2022 | WEATHER | Clear |
| COLLECTION DATE |  |  |  |

NUMBER OF LANES $\qquad$

|  | Eastbound |  |  |  |  | Westbound |  |  |  |  | Hourly Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hour | 1st | 2nd | 3rd | 4th | Total | 1st | 2nd | 3rd | 4th | Total |  |
| 12：00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 1 |
| 1：00 AM | 1 | 0 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 2 |
| 2：00 AM | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 1 | 2 |
| 3：00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 1 | 3 | 3 |
| 4：00 AM | 1 | 1 | 1 | 3 | 6 | 0 | 1 | 1 | 2 | 4 | 10 |
| 5：00 AM | 3 | 2 | 6 | 5 | 16 | 0 | 2 | 3 | 3 | 8 | 24 |
| 6：00 AM | 6 | 7 | 3 | 12 | 28 | 2 | 5 | 5 | 8 | 20 | 48 |
| 7：00 AM | 13 | 17 | 26 | 19 | 75 | 10 | 5 | 10 | 8 | 33 | 108 |
| 8：00 AM | 9 | 11 | 6 | 7 | 33 | 13 | 8 | 7 | 6 | 34 | 67 |
| 9：00 AM | 5 | 6 | 8 | 10 | 29 | 7 | 7 | 12 | 10 | 36 | 65 |
| 10：00 AM | 13 | 7 | 7 | 9 | 36 | 10 | 8 | 12 | 15 | 45 | 81 |
| 11：00 AM | 11 | 10 | 17 | 19 | 57 | 15 | 22 | 11 | 26 | 74 | 131 |
| 12：00 PM | 12 | 8 | 10 | 10 | 40 | 17 | 13 | 7 | 5 | 42 | 82 |
| 1：00 PM | 8 | 6 | 7 | 15 | 36 | 11 | 13 | 11 | 15 | 50 | 86 |
| 2：00 PM | 10 | 18 | 19 | 11 | 58 | 9 | 17 | 24 | 11 | 61 | 119 |
| 3：00 PM | 15 | 14 | 19 | 14 | 62 | 15 | 19 | 16 | 25 | 75 | 137 |
| 4：00 PM | 15 | 18 | 9 | 14 | 56 | 23 | 11 | 24 | 16 | 74 | 130 |
| 5：00 PM | 6 | 14 | 10 | 11 | 41 | 26 | 19 | 11 | 21 | 77 | 118 |
| 6：00 PM | 16 | 6 | 7 | 7 | 36 | 13 | 6 | 7 | 8 | 34 | 70 |
| 7：00 PM | 7 | 2 | 7 | 6 | 22 | 19 | 8 | 12 | 15 | 54 | 76 |
| 8：00 PM | 8 | 2 | 5 | 3 | 18 | 6 | 10 | 4 | 6 | 26 | 44 |
| 9：00 PM | 7 | 4 | 0 | 2 | 13 | 4 | 4 | 4 | 2 | 14 | 27 |
| 10：00 PM | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 1 | 2 | 5 | 5 |
| 11：00 PM | 0 | 0 | 1 | 0 | 1 | 1 | 2 | 0 | 3 | 6 | 7 |
| Total | 46．2\％ |  |  |  | 666 | 53．8\％ |  |  |  | 777 |  |
|  | 1443 |  |  |  |  |  |  |  |  |  |  |
| AM\％ | 37．6\％ | AM Peak 131 |  |  | 11：00 am to 12：00 pm |  |  | AM P．H．F． 0.73 |  |  |  |
| PM\％ | 62．4\％ | PM Peak 145 |  |  | 3：15 pm to 4：15 pm |  |  | PM P．H．F． |  | 0.93 |  |



## Metro Traffic Data Inc.

310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax
www.metrotrafficdata.com

## 24 Hour Count Report

Prepared For:

## Rick Engineering Company

5620 Friars Rd
San Diego, CA 92110

| STREET | 10th St | LATITUDE | 35.14316411 |
| ---: | :--- | ---: | :--- |
| SEGMENT | btwn SR $33 /$ Center St | LONGITUDE |  |
| COLLECTION DATE | Tuesday, March 15,2022 | WEATHER | Clear |

NUMBER OF LANES $\qquad$

|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Hourly <br> Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hour | 1st | 2nd | 3rd | 4th | Total | 1st | 2nd | 3rd | 4th | Total |  |
| 12:00 AM | 6 | 5 | 2 | 4 | 17 | 11 | 7 | 5 | 1 | 24 | 41 |
| 1:00 AM | 3 | 0 | 3 | 0 | 6 | 3 | 5 | 3 | 4 | 15 | 21 |
| 2:00 AM | 1 | 3 | 2 | 0 | 6 | 1 | 0 | 1 | 1 | 3 | 9 |
| 3:00 AM | 2 | 2 | 4 | 4 | 12 | 0 | 5 | 5 | 2 | 12 | 24 |
| 4:00 AM | 9 | 10 | 19 | 26 | 64 | 0 | 4 | 7 | 12 | 23 | 87 |
| 5:00 AM | 46 | 50 | 67 | 50 | 213 | 13 | 12 | 23 | 29 | 77 | 290 |
| 6:00 AM | 76 | 55 | 56 | 64 | 251 | 20 | 30 | 26 | 33 | 109 | 360 |
| 7:00 AM | 74 | 110 | 122 | 91 | 397 | 45 | 49 | 113 | 87 | 294 | 691 |
| 8:00 AM | 68 | 63 | 55 | 53 | 239 | 59 | 47 | 37 | 35 | 178 | 417 |
| 9:00 AM | 46 | 53 | 46 | 64 | 209 | 36 | 57 | 38 | 43 | 174 | 383 |
| 10:00 AM | 51 | 47 | 62 | 84 | 244 | 54 | 45 | 43 | 69 | 211 | 455 |
| 11:00 AM | 71 | 70 | 78 | 77 | 296 | 86 | 65 | 61 | 64 | 276 | 572 |
| 12:00 PM | 77 | 86 | 53 | 75 | 291 | 67 | 77 | 69 | 62 | 275 | 566 |
| 1:00 PM | 76 | 73 | 63 | 79 | 291 | 78 | 57 | 76 | 60 | 271 | 562 |
| 2:00 PM | 91 | 94 | 88 | 98 | 371 | 82 | 117 | 104 | 92 | 395 | 766 |
| 3:00 PM | 80 | 84 | 71 | 99 | 334 | 103 | 100 | 99 | 91 | 393 | 727 |
| 4:00 PM | 85 | 88 | 92 | 102 | 367 | 89 | 90 | 101 | 87 | 367 | 734 |
| 5:00 PM | 104 | 94 | 74 | 85 | 357 | 108 | 99 | 86 | 88 | 381 | 738 |
| 6:00 PM | 77 | 68 | 60 | 69 | 274 | 85 | 84 | 62 | 75 | 306 | 580 |
| 7:00 PM | 70 | 57 | 48 | 61 | 236 | 78 | 63 | 51 | 68 | 260 | 496 |
| 8:00 PM | 54 | 55 | 42 | 32 | 183 | 61 | 66 | 48 | 47 | 222 | 405 |
| 9:00 PM | 24 | 53 | 38 | 23 | 138 | 44 | 30 | 28 | 37 | 139 | 277 |
| 10:00 PM | 19 | 16 | 8 | 9 | 52 | 29 | 21 | 12 | 9 | 71 | 123 |
| 11:00 PM | 10 | 9 | 7 | 8 | 34 | 15 | 8 | 3 | 6 | 32 | 66 |
| Total | 52.0\% |  |  |  | 4882 | 48.0\% |  |  |  | 4508 |  |
|  | 9390 |  |  |  |  |  |  |  |  |  |  |
| AM\% | 35.7\% | AM Peak 699 |  |  | 7:15 am to 8:15 am |  |  | AM P.H.F. 0.74 |  |  |  |
| PM\% | 64.3\% | PM Peak 776 |  |  | 2:15 pm to 3:15 pm |  |  | PM P.H.F. |  | 0.92 |  |



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800－975－6938 Phone／Fax
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## 24 Hour Count Report

Prepared For：

## Rick Engineering Company

5620 Friars Rd
San Diego，CA 92110

| STREET | 6th St | LATITUDE |  |
| ---: | :---: | :---: | :---: |
| SEGMENT | btwn SR $33 /$ Center St | LONGITUDE | -119.4607701 |
|  | Tuesday，March 15,2022 | WEATHER | Clear |

NUMBER OF LANES $\qquad$

|  | Northbound |  |  |  |  | Southbound |  |  |  |  | Hourly Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hour | 1st | 2nd | 3rd | 4th | Total | 1st | 2nd | 3rd | 4th | Total |  |
| 12：00 AM | 0 | 1 | 1 | 1 | 3 | 1 | 0 | 0 | 0 | 1 | 4 |
| 1：00 AM | 0 | 2 | 1 | 0 | 3 | 2 | 1 | 0 | 1 | 4 | 7 |
| 2：00 AM | 0 | 1 | 0 | 3 | 4 | 2 | 1 | 1 | 0 | 4 | 8 |
| 3：00 AM | 0 | 1 | 1 | 1 | 3 | 2 | 0 | 2 | 1 | 5 | 8 |
| 4：00 AM | 4 | 0 | 2 | 4 | 10 | 0 | 1 | 1 | 3 | 5 | 15 |
| 5：00 AM | 4 | 6 | 8 | 16 | 34 | 4 | 6 | 8 | 5 | 23 | 57 |
| 6：00 AM | 15 | 4 | 6 | 22 | 47 | 8 | 5 | 1 | 5 | 19 | 66 |
| 7：00 AM | 21 | 44 | 67 | 31 | 163 | 11 | 23 | 63 | 37 | 134 | 297 |
| 8：00 AM | 15 | 15 | 12 | 10 | 52 | 28 | 17 | 8 | 7 | 60 | 112 |
| 9：00 AM | 14 | 12 | 11 | 15 | 52 | 12 | 7 | 12 | 14 | 45 | 97 |
| 10：00 AM | 15 | 12 | 5 | 15 | 47 | 13 | 12 | 13 | 12 | 50 | 97 |
| 11：00 AM | 18 | 20 | 20 | 15 | 73 | 20 | 16 | 30 | 24 | 90 | 163 |
| 12：00 PM | 16 | 17 | 19 | 17 | 69 | 24 | 13 | 21 | 22 | 80 | 149 |
| 1：00 PM | 15 | 19 | 14 | 24 | 72 | 25 | 15 | 21 | 23 | 84 | 156 |
| 2：00 PM | 31 | 27 | 19 | 25 | 102 | 34 | 39 | 28 | 22 | 123 | 225 |
| 3：00 PM | 31 | 25 | 25 | 24 | 105 | 35 | 41 | 31 | 33 | 140 | 245 |
| 4：00 PM | 15 | 13 | 23 | 17 | 68 | 33 | 25 | 25 | 23 | 106 | 174 |
| 5：00 PM | 22 | 21 | 25 | 27 | 95 | 34 | 31 | 46 | 39 | 150 | 245 |
| 6：00 PM | 11 | 25 | 16 | 18 | 70 | 27 | 23 | 29 | 26 | 105 | 175 |
| 7：00 PM | 19 | 11 | 22 | 19 | 71 | 20 | 18 | 23 | 18 | 79 | 150 |
| 8：00 PM | 6 | 13 | 5 | 4 | 28 | 20 | 9 | 14 | 9 | 52 | 80 |
| 9：00 PM | 6 | 5 | 4 | 2 | 17 | 15 | 18 | 8 | 10 | 51 | 68 |
| 10：00 PM | 1 | 3 | 3 | 2 | 9 | 5 | 3 | 3 | 5 | 16 | 25 |
| 11：00 PM | 2 | 1 | 0 | 2 | 5 | 5 | 3 | 1 | 0 | 9 | 14 |
| Total | 45．6\％ |  |  |  | 1202 | 54．4\％ |  |  |  | 1435 |  |
|  | 2637 |  |  |  |  |  |  |  |  |  |  |
| AM\％ | 35．3\％ | AM Peak 308 |  |  | 7：15 am to 8：15 am |  |  | AM P．H．F． 0.59 |  |  |  |
| PM\％ | 64．7\％ | PM Peak 245 |  |  | 3：00 pm to 4：00 pm |  |  | PM P．H．F． |  | 0.93 |  |



APPENDIX B
EXISTING CONDITIONS
INTERSECTION OPERATIONS ANALYSIS WORKSHEETS

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.


## Intersection

Intersection Delay, s/veh15.6
Intersection LOS

| Movement EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | * $\uparrow$ |  |  | * $\uparrow$ |  |  | $\uparrow \uparrow$ |  |  | * $\uparrow$ |  |
| Traffic Vol, veh/h 79 | 227 | 12 | 16 | 257 | 39 | 17 | 135 | 21 | 43 | 128 | 74 |
| Future Vol, veh/h 79 | 227 | 12 | 16 | 257 | 39 | 17 | 135 | 21 | 43 | 128 | 74 |
| Peak Hour Factor 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 |
| Heavy Vehicles, \% 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow 108 | 311 | 16 | 22 | 352 | 53 | 23 | 185 | 29 | 59 | 175 | 101 |
| Number of Lanes 0 | 2 | 0 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 2 | 0 |
| Approach EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes 2 |  |  | 2 |  |  | 2 |  |  | 2 |  |  |
| Conflicting Approach Left SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left 2 |  |  | 2 |  |  | 2 |  |  | 2 |  |  |
| Conflicting Approach RighNB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right 2 |  |  | 2 |  |  | 2 |  |  | 2 |  |  |
| HCM Control Delay 17.4 |  |  | 15.8 |  |  | 13.5 |  |  | 14.7 |  |  |
| HCM LOS C |  |  | C |  |  | B |  |  | B |  |  |


| Lane | NBLn1 NBLn2 EBLn1 EBLn2WBLn1WBLn2 SBLn1 SBLn2 |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $20 \%$ | $0 \%$ | $41 \%$ | $0 \%$ | $11 \%$ | $0 \%$ | $40 \%$ | $0 \%$ |
| Vol Thru, \% | $80 \%$ | $76 \%$ | $59 \%$ | $90 \%$ | $89 \%$ | $77 \%$ | $60 \%$ | $46 \%$ |
| Vol Right, \% | $0 \%$ | $24 \%$ | $0 \%$ | $10 \%$ | $0 \%$ | $23 \%$ | $0 \%$ | $54 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 85 | 89 | 193 | 126 | 145 | 168 | 107 | 138 |
| LT Vol | 17 | 0 | 79 | 0 | 16 | 0 | 43 | 0 |
| Through Vol | 68 | 68 | 114 | 114 | 129 | 129 | 64 | 64 |
| RT Vol | 0 | 21 | 0 | 12 | 0 | 39 | 0 | 74 |
| Lane Flow Rate | 116 | 121 | 264 | 172 | 198 | 229 | 147 | 189 |
| Geometry Grp | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.26 | 0.263 | 0.557 | 0.35 | 0.412 | 0.463 | 0.325 | 0.388 |
| Departure Headway (Hd) | 8.092 | 7.817 | 7.606 | 7.327 | 7.491 | 7.267 | 7.975 | 7.382 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 443 | 458 | 475 | 490 | 479 | 496 | 450 | 486 |
| Service Time | 5.856 | 5.581 | 5.366 | 5.086 | 5.25 | 5.025 | 5.733 | 5.139 |
| HCM Lane V/C Ratio | 0.262 | 0.264 | 0.556 | 0.351 | 0.413 | 0.462 | 0.327 | 0.389 |
| HCM Control Delay | 13.7 | 13.4 | 19.6 | 14 | 15.4 | 16.2 | 14.6 | 14.8 |
| HCM Lane LOS | B | B | C | B | C | C | B | B |
| HCM 95th-tile Q | 1 | 1 | 3.3 | 1.6 | 2 | 2.4 | 1.4 | 1.8 |



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | $\hat{4}_{\text {¢ }}$ |  |  | * $\hat{\square}$ |  |  | $\uparrow$ |  |  | $\dagger$ |  |
| Traffic Vol, veh/h | 49 | 189 | 20 | 16 | 205 | 27 | 40 | 96 | 21 | 34 | 104 | 37 |
| Future Vol, veh/h | 49 | 189 | 20 | 16 | 205 | 27 | 40 | 96 | 21 | 34 | 104 | 37 |
| Peak Hour Factor | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 62 | 239 | 25 | 20 | 259 | 34 | 51 | 122 | 27 | 43 | 132 | 47 |
| Number of Lanes | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 2 |  |  | 2 |  |  | 1 |  |  | 1 |  |  |
| Conflicting Approach Le | ft SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 1 |  |  | 1 |  |  | 2 |  |  | 2 |  |  |
| Conflicting Approach Ric | ghNB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 1 |  |  | 2 |  |  | 2 |  |  |
| HCM Control Delay | 11.9 |  |  | 11.5 |  |  | 12.3 |  |  | 12.6 |  |  |
| HCM LOS | B |  |  | B |  |  | B |  |  | B |  |  |


| Lane | NBLn1 EBLn1 EBLn2WBLn1WBLn2 SBLn1 |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $25 \%$ | $34 \%$ | $0 \%$ | $14 \%$ | $0 \%$ | $19 \%$ |
| Vol Thru, \% | $61 \%$ | $66 \%$ | $83 \%$ | $86 \%$ | $79 \%$ | $59 \%$ |
| Vol Right, \% | $13 \%$ | $0 \%$ | $17 \%$ | $0 \%$ | $21 \%$ | $21 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 157 | 144 | 115 | 119 | 130 | 175 |
| LT Vol | 40 | 49 | 0 | 16 | 0 | 34 |
| Through Vol | 96 | 95 | 95 | 103 | 103 | 104 |
| RT Vol | 21 | 0 | 20 | 0 | 27 | 37 |
| Lane Flow Rate | 199 | 182 | 145 | 150 | 164 | 222 |
| Geometry Grp | 2 | 7 | 7 | 7 | 7 | 2 |
| Degree of Util (X) | 0.336 | 0.328 | 0.25 | 0.268 | 0.283 | 0.368 |
| Departure Headway (Hd) | 6.091 | 6.505 | 6.206 | 6.424 | 6.207 | 5.985 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 586 | 550 | 575 | 557 | 575 | 598 |
| Service Time | 4.171 | 4.276 | 3.978 | 4.197 | 3.979 | 4.063 |
| HCM Lane V/C Ratio | 0.34 | 0.331 | 0.252 | 0.269 | 0.285 | 0.371 |
| HCM Control Delay | 12.3 | 12.5 | 11.1 | 11.6 | 11.4 | 12.6 |
| HCM Lane LOS | B | B | B | B | B | B |
| HCM 95th-tile Q | 1.5 | 1.4 | 1 | 1.1 | 1.2 | 1.7 |

Generated with PTV VISTRO
Version 2021 (SP 0-6)
Scenario 1: 1 EX 2022 AM

## Intersection Level Of Service Report

Intersection 4: Kern Street (SR-33) / 1st Street

| Control Type: | Two-way stop | Delay $(\mathrm{sec} / \mathrm{veh}):$ | 13.9 |
| :---: | :---: | :---: | :---: |
| Analysis Method: | HCM 6 th Edition | Level Of Service: | B |
| Analysis Period: | 15 minutes | Volume to Capacity $(\mathrm{v} / \mathrm{c}):$ | 0.021 |

Intersection Setup

| Name | 1st Street |  |  |  | 1st Street |  |  |  | Kern Street (SR-33) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Northbound |  |  |  | Southbound |  |  |  | Eastbound |  |  |  |
| Lane Configuration | $\stackrel{1}{4}$ |  |  |  | $\Rightarrow$ |  |  |  | $4$ |  |  |  |
| Turning Movement | Left2 | Left | Thru | Right | Left2 | Left | Thru | Right | Left2 | Left | Thru | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 30.00 |  |  |  | 30.00 |  |  |  | 30.00 |  |  |  |
| Grade [\%] | 0.00 |  |  |  | 0.00 |  |  |  | 0.00 |  |  |  |
| Crosswalk | No |  |  |  | No |  |  |  | No |  |  |  |

Volumes

| Name | 1st Street |  |  |  | 1st Street |  |  |  | Kern Street (SR-33) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 8 | 0 | 5 | 2 | 4 | 6 | 2 | 23 | 20 | 84 | 85 | 23 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 2.00 | 2.00 | 0.00 | 2.00 | 0.00 | 2.00 | 2.00 | 2.00 | 2.00 | 0.00 | 2.00 | 2.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 8 | 0 | 5 | 2 | 4 | 6 | 2 | 23 | 20 | 84 | 85 | 23 |
| Peak Hour Factor | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 2 | 0 | 1 | 1 | 1 | 2 | 1 | 7 | 6 | 24 | 24 | 7 |
| Total Analysis Volume [veh/h] | 9 | 0 | 6 | 2 | 5 | 7 | 2 | 26 | 23 | 97 | 98 | 26 |
| Pedestrian Volume [ped/h] | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  |

Version 2021 (SP 0-6)
Intersection Settings

| Priority Scheme | Stop | Stop | Free |
| :---: | :---: | :---: | :---: |
| Flared Lane | No | No |  |
| Storage Area [veh] | 0 | 0 | 0 |
| Two-Stage Gap Acceptance | No | No |  |
| Number of Storage Spaces in Median | 0 | 0 | 0 |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.02 | 0.00 | 0.01 | 0.00 | 0.01 | 0.01 | 0.00 | 0.03 | 0.02 | 0.06 | 0.00 | 0.00 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 13.87 | 14.79 | 12.82 | 8.92 | 12.17 | 12.22 | 13.01 | 8.79 | 7.57 | 7.55 | 0.00 | 0.00 |
| Movement LOS | B | B | B | A | B | B | B | A | A | A | A | A |
| 95th-Percentile Queue Length [veh/n] | 0.11 | 0.11 | 0.11 | 0.11 | 0.17 | 0.17 | 0.17 | 0.17 | 0.25 | 0.25 | 0.13 | 0.00 |
| 95th-Percentile Queue Length [ff/ln] | 2.80 | 2.80 | 2.80 | 2.80 | 4.18 | 4.18 | 4.18 | 4.18 | 6.37 | 6.37 | 3.19 | 0.00 |
| d_A, Approach Delay [s/veh] | 12.92 |  |  |  | 10.02 |  |  |  | 3.71 |  |  |  |
| Approach LOS | B |  |  |  | B |  |  |  | A |  |  |  |
| d_l, Intersection Delay [s/veh] | 5.44 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection LOS | B |  |  |  |  |  |  |  |  |  |  |  |

Version 2021 (SP 0-6)
Scenario 1: 1 EX 2022 AM
Intersection Setup

| Name | Kern Street (SR-33) |  |  |  | E Kern Street |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Westbound |  |  |  | Southwestbound |  |  |  |
| Lane Configuration | $\stackrel{\rightharpoonup}{F}$ |  |  |  | YK |  |  |  |
| Turning Movement | Left | Thru | Right | Right2 | Left | Thru | Right | Right2 |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 30.00 |  |  |  | 30.00 |  |  |  |
| Grade [\%] | 0.00 |  |  |  | 0.00 |  |  |  |
| Crosswalk | No |  |  |  | No |  |  |  |

## Volumes

| Name | Kern Street (SR-33) |  |  |  | E Kern Street |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 1 | 68 | 1 | 2 | 2 | 5 | 102 | 6 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 2.00 | 2.00 | 2.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 1 | 68 | 1 | 2 | 2 | 5 | 102 | 6 |
| Peak Hour Factor | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 0 | 20 | 0 | 1 | 1 | 1 | 29 | 2 |
| Total Analysis Volume [veh/h] | 1 | 78 | 1 | 2 | 2 | 6 | 117 | 7 |
| Pedestrian Volume [ped/h] |  |  | 0 |  |  |  | 0 |  |

Version 2021 (SP 0-6)
Scenario 1: 1 EX 2022 AM
Intersection Settings

| Priority Scheme | Free | Stop |
| :---: | :---: | :---: |
| Flared Lane |  |  |
| Storage Area [veh] | 0 | 0 |
| Two-Stage Gap Acceptance |  | No |
| Number of Storage Spaces in Median | 0 | 0 |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.01 | 0.13 | 0.01 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 7.47 | 0.00 | 0.00 | 0.00 | 12.76 | 13.41 | 9.36 | 9.36 |
| Movement LOS | A | A | A | A | B | B | A | A |
| 95th-Percentile Queue Length [veh/ln] | 0.00 | 0.00 | 0.00 | 0.00 | 0.27 | 0.27 | 0.25 | 0.24 |
| 95th-Percentile Queue Length [ft/ln] | 0.05 | 0.05 | 0.05 | 0.05 | 6.68 | 6.68 | 6.31 | 5.94 |
| d_A, Approach Delay [s/veh] | 0.09 |  |  |  | 9.60 |  |  |  |
| Approach LOS | A |  |  |  | A |  |  |  |
| d_I, Intersection Delay [s/veh] | 5.44 |  |  |  |  |  |  |  |
| Intersection LOS | B |  |  |  |  |  |  |  |






|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.


## Intersection

Intersection Delay, s/veh 12
Intersection LOS B

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  |  |  |  | * $\hat{\square}$ |  |  | 4 ${ }^{\text {¢ }}$ |  |  | $\mathrm{A}_{4}$ |  |
| Traffic Vol, veh/h | 78 | 245 | 17 | 27 | 224 | 42 | 22 | 79 | 28 | 37 | 117 | 79 |
| Future Vol, veh/h | 78 | 245 | 17 | 27 | 224 | 42 | 22 | 79 | 28 | 37 | 117 | 79 |
| Peak Hour Factor | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 88 | 275 | 19 | 30 | 252 | 47 | 25 | 89 | 31 | 42 | 131 | 89 |
| Number of Lanes | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 2 | 0 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 2 |  |  | 2 |  |  | 2 |  |  | 2 |  |  |
| Conflicting Approach L | ft SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 2 |  |  | 2 |  |  | 2 |  |  | 2 |  |  |
| Conflicting Approach R | ghNB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Righ | 2 |  |  | 2 |  |  | 2 |  |  | 2 |  |  |
| HCM Control Delay | 12.8 |  |  | 11.8 |  |  | 10.8 |  |  | 11.6 |  |  |
| HCM LOS | B |  |  | B |  |  | B |  |  | B |  |  |


| Lane | NBLn1 NBLn2 | EBLn1 | EBLn2WBLn1WBLn2 SBLn1 SBLn2 |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $36 \%$ | $0 \%$ | $39 \%$ | $0 \%$ | $19 \%$ | $0 \%$ | $39 \%$ | $0 \%$ |
| Vol Thru, \% | $64 \%$ | $59 \%$ | $61 \%$ | $88 \%$ | $81 \%$ | $73 \%$ | $61 \%$ | $43 \%$ |
| Vol Right, \% | $0 \%$ | $41 \%$ | $0 \%$ | $12 \%$ | $0 \%$ | $27 \%$ | $0 \%$ | $57 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 62 | 68 | 201 | 140 | 139 | 154 | 96 | 138 |
| LT Vol | 22 | 0 | 78 | 0 | 27 | 0 | 37 | 0 |
| Through Vol | 40 | 40 | 123 | 123 | 112 | 112 | 59 | 59 |
| RT Vol | 0 | 28 | 0 | 17 | 0 | 42 | 0 | 79 |
| Lane Flow Rate | 69 | 76 | 225 | 157 | 156 | 173 | 107 | 154 |
| Geometry Grp | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.138 | 0.142 | 0.409 | 0.272 | 0.283 | 0.29 | 0.208 | 0.274 |
| Departure Headway (Hd) | 7.2 | 6.72 | 6.533 | 6.249 | 6.523 | 6.23 | 6.99 | 6.382 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 495 | 530 | 550 | 572 | 548 | 574 | 511 | 559 |
| Service Time | 4.983 | 4.504 | 4.301 | 4.017 | 4.295 | 4.002 | 4.764 | 4.157 |
| HCM Lane V/C Ratio | 0.139 | 0.143 | 0.409 | 0.274 | 0.285 | 0.301 | 0.209 | 0.275 |
| HCM Control Delay | 11.1 | 10.6 | 13.8 | 11.4 | 11.9 | 11.7 | 11.6 | 11.6 |
| HCM Lane LOS | B | B | B | B | B | B | B | B |
| HCM 95th-tile Q | 0.5 | 0.5 | 2 | 1.1 | 1.2 | 1.2 | 0.8 | 1.1 |




| Lane | NBLn1 EBLn1 EBLn2WBLn1WBLn2 SBLn1 |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $28 \%$ | $24 \%$ | $0 \%$ | $21 \%$ | $0 \%$ | $27 \%$ |
| Vol Thru, \% | $56 \%$ | $76 \%$ | $82 \%$ | $79 \%$ | $73 \%$ | $48 \%$ |
| Vol Right, \% | $15 \%$ | $0 \%$ | $18 \%$ | $0 \%$ | $27 \%$ | $25 \%$ |
| Sign Control | Sttop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 130 | 162 | 151 | 137 | 147 | 132 |
| LT Vol | 37 | 38 | 0 | 29 | 0 | 35 |
| Through Vol | 73 | 124 | 124 | 108 | 108 | 64 |
| RT Vol | 20 | 0 | 27 | 0 | 39 | 33 |
| Lane Flow Rate | 148 | 184 | 171 | 156 | 167 | 150 |
| Geometry Grp | 2 | 7 | 7 | 7 | 7 | 2 |
| Degree of Util (X) | 0.242 | 0.306 | 0.273 | 0.261 | 0.266 | 0.243 |
| Departure Headway (Hd) | 5.894 | 5.998 | 5.752 | 6.03 | 5.734 | 5.83 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 609 | 598 | 624 | 595 | 626 | 615 |
| Service Time | 3.943 | 3.74 | 3.493 | 3.773 | 3.477 | 3.879 |
| HCM Lane V/C Ratio | 0.243 | 0.308 | 0.274 | 0.262 | 0.267 | 0.244 |
| HCM Control Delay | 10.8 | 11.4 | 10.7 | 10.9 | 10.6 | 10.8 |
| HCM Lane LOS | B | B | B | B | B | B |
| HCM 95th-tile Q | 0.9 | 1.3 | 1.1 | 1 | 1.1 | 0.9 |

## Intersection Level Of Service Report

Intersection 4: Kern Street (SR-33) / 1st Street

| Control Type: | Two-way stop | Delay $(\mathrm{sec} / \mathrm{veh}):$ | 16.0 |
| :---: | :---: | :---: | :---: |
| Analysis Method: | HCM 6 th Edition | Level Of Service: | C |
| Analysis Period: | 15 minutes | Volume to Capacity $(\mathrm{v} / \mathrm{c}):$ | 0.006 |

Intersection Setup

| Name | 1st Street |  |  |  | 1st Street |  |  |  | Kern Street (SR-33) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Northbound |  |  |  | Southbound |  |  |  | Eastbound |  |  |  |
| Lane Configuration | $\stackrel{1}{4}$ |  |  |  | $\Rightarrow$ |  |  |  | $4$ |  |  |  |
| Turning Movement | Left2 | Left | Thru | Right | Left2 | Left | Thru | Right | Left2 | Left | Thru | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 30.00 |  |  |  | 30.00 |  |  |  | 30.00 |  |  |  |
| Grade [\%] | 0.00 |  |  |  | 0.00 |  |  |  | 0.00 |  |  |  |
| Crosswalk | No |  |  |  | No |  |  |  | No |  |  |  |

## Volumes

| Name | 1st Street |  |  |  | 1 st Street |  |  |  |  | Kern Street (SR-33) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 9 | 2 | 4 | 0 | 2 | 5 | 3 | 17 | 11 | 139 | 84 | 10 |  |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |  |
| Heavy Vehicles Percentage [\%] | 2.00 | 2.00 | 0.00 | 2.00 | 0.00 | 2.00 | 2.00 | 2.00 | 2.00 | 0.00 | 2.00 | 2.00 |  |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |  |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Total Hourly Volume [veh/h] | 9 | 2 | 4 | 0 | 2 | 5 | 3 | 17 | 11 | 139 | 84 | 10 |  |
| Peak Hour Factor | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 |  |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |  |
| Total 15-Minute Volume [veh/h] | 2 | 1 | 1 | 0 | 1 | 1 | 1 | 5 | 3 | 37 | 22 | 3 |  |
| Total Analysis Volume [veh/h] | 10 | 2 | 4 | 0 | 2 | 5 | 3 | 18 | 12 | 148 | 89 | 11 |  |
| Pedestrian Volume [ped/h] |  |  | 0 |  |  |  | 0 |  |  |  | 0 |  |  |

Version 2021 (SP 0-6)
Scenario 2: 2 EX 2022 PM
Intersection Settings

| Priority Scheme | Stop | Stop | Free |
| :---: | :---: | :---: | :---: |
| Flared Lane | No | No |  |
| Storage Area [veh] | 0 | 0 | 0 |
| Two-Stage Gap Acceptance | No | No |  |
| Number of Storage Spaces in Median | 0 | 0 | 0 |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.03 | 0.01 | 0.01 | 0.00 | 0.00 | 0.01 | 0.01 | 0.02 | 0.01 | 0.10 | 0.00 | 0.00 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 15.26 | 16.00 | 14.52 | 9.01 | 14.00 | 14.09 | 14.57 | 8.94 | 7.77 | 7.75 | 0.00 | 0.00 |
| Movement LOS | C | C | B | A | B | B | B | A | A | A | A | A |
| 95th-Percentile Queue Length [veh/ln] | 0.14 | 0.14 | 0.14 | 0.14 | 0.14 | 0.14 | 0.14 | 0.14 | 0.37 | 0.37 | 0.18 | 0.00 |
| 95th-Percentile Queue Length [ft/ln] | 3.38 | 3.38 | 3.38 | 3.38 | 3.39 | 3.39 | 3.39 | 3.39 | 9.15 | 9.15 | 4.58 | 0.00 |
| d_A, Approach Delay [s/veh] | 15.17 |  |  |  | 10.82 |  |  |  | 4.77 |  |  |  |
| Approach LOS | C |  |  |  | B |  |  |  | A |  |  |  |
| d_I, Intersection Delay [s/veh] | 5.05 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection LOS | C |  |  |  |  |  |  |  |  |  |  |  |

Version 2021 (SP 0-6)
Scenario 2: 2 EX 2022 PM
Intersection Setup

| Name | Kern Street (SR-33) |  |  |  | E Kern Street |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Westbound |  |  |  | Southwestbound |  |  |  |
| Lane Configuration | $\stackrel{\rightharpoonup}{F}$ |  |  |  | YK |  |  |  |
| Turning Movement | Left | Thru | Right | Right2 | Left | Thru | Right | Right2 |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 30.00 |  |  |  | 30.00 |  |  |  |
| Grade [\%] | 0.00 |  |  |  | 0.00 |  |  |  |
| Crosswalk | No |  |  |  | No |  |  |  |

## Volumes

| Name | Kern Street (SR-33) |  |  |  | E Kern Street |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 2 | 121 | 11 | 1 | 1 | 6 | 80 | 4 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 2.00 | 2.00 | 2.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 2 | 121 | 11 | 1 | 1 | 6 | 80 | 4 |
| Peak Hour Factor | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 1 | 32 | 3 | 0 | 0 | 2 | 21 | 1 |
| Total Analysis Volume [veh/h] | 2 | 129 | 12 | 1 | 1 | 6 | 85 | 4 |
| Pedestrian Volume [ped/h] |  |  | 0 |  |  |  | 0 |  |

Version 2021 (SP 0-6)
Scenario 2: 2 EX 2022 PM
Intersection Settings

| Priority Scheme | Free | Stop |
| :---: | :---: | :---: |
| Flared Lane |  |  |
| Storage Area [veh] | 0 | 0 |
| Two-Stage Gap Acceptance |  | No |
| Number of Storage Spaces in Median | 0 | 0 |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.02 | 0.10 | 0.00 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 7.42 | 0.00 | 0.00 | 0.00 | 14.51 | 14.71 | 9.51 | 9.55 |
| Movement LOS | A | A | A | A | B | B | A | A |
| 95th-Percentile Queue Length [veh/ln] | 0.00 | 0.00 | 0.00 | 0.00 | 0.21 | 0.21 | 0.19 | 0.18 |
| 95th-Percentile Queue Length [ft/ln] | 0.10 | 0.10 | 0.10 | 0.10 | 5.29 | 5.29 | 4.87 | 4.46 |
| d_A, Approach Delay [s/veh] | 0.10 |  |  |  | 9.89 |  |  |  |
| Approach LOS | A |  |  |  | A |  |  |  |
| d_l, Intersection Delay [s/veh] | 5.05 |  |  |  |  |  |  |  |
| Intersection LOS | C |  |  |  |  |  |  |  |




| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | * |  |  |  | ${ }^{7}$ | 44 |
| Traffic Vol, veh/h | 27 | 49 | 343 | 51 | 56 | 339 |
| Future Vol, veh/h | 27 | 49 | 343 | 51 | 56 | 339 |
| Conflicting Peds, \#/hr | 0 | 1 | 0 | 5 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 50 | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 29 | 53 | 373 | 55 | 61 | 368 |


| Major/Minor M | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 712 | 220 | 0 | 0 | 433 | 0 |
| Stage 1 | 406 | - | - | - | - | - |
| Stage 2 | 306 | - | - | - | - | - |
| Critical Hdwy | 6.84 | 6.94 | - | - | 4.14 | - |
| Critical Hdwy Stg 1 | 5.84 |  | - | - | - | - |
| Critical Hdwy Stg 2 | 5.84 | - | - | - | - | - |
| Follow-up Hdwy | 3.52 | 3.32 | - | - | 2.22 | - |
| Pot Cap-1 Maneuver | 367 | 784 | - | - | 1123 | - |
| Stage 1 | 641 | - | - | - | - | - |
| Stage 2 | 720 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 345 | 780 | - | - | 1118 | - |
| Mov Cap-2 Maneuver | 458 | - | - | - | - | - |
| Stage 1 | 638 | - | - | - | - | - |
| Stage 2 | 680 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 11.6 |  | 0 |  | 1.2 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 624 | 1118 | - |
| HCM Lane V/C Ratio |  | - | - | 0.132 | 0.054 | - |
| HCM Control Delay (s) |  | - | - | 11.6 | 8.4 | - |
| HCM Lane LOS |  | - | - | B | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0.5 | 0.2 | - |

# APPENDIX C 

ITE TRIP GENERATION RATE SHEETS / NCHRP REPORT 684 INTERNAL CAPTURE WORKSHEETS

# Land Use: 210 Single-Family Detached Housing 

## Description

A single-family detached housing site includes any single-family detached home on an individual lot. A typical site surveyed is a suburban subdivision.

## Specialized Land Use

Data have been submitted for several single-family detached housing developments with homes that are commonly referred to as patio homes. A patio home is a detached housing unit that is located on a small lot with little (or no) front or back yard. In some subdivisions, communal maintenance of outside grounds is provided for the patio homes. The three patio home sites total 299 dwelling units with overall weighted average trip generation rates of 5.35 vehicle trips per dwelling unit for weekday, 0.26 for the AM adjacent street peak hour, and 0.47 for the PM adjacent street peak hour. These patio home rates based on a small sample of sites are lower than those for single-family detached housing (Land Use 210), lower than those for single-family attached housing (Land Use 251), and higher than those for senior adult housing -- single-family (Land Use 251). Further analysis of this housing type will be conducted in a future edition of Trip Generation Manual.

## Additional Data

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (https://www.ite.org/technical-resources/topics/trip-and-parking-generation/).

For 30 of the study sites, data on the number of residents and number of household vehicles are available. The overall averages for the 30 sites are 3.6 residents per dwelling unit and 1.5 vehicles per dwelling unit.

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Arizona, California, Connecticut, Delaware, Illinois, Indiana, Kentucky, Maryland, Massachusetts, Minnesota, Montana, New Jersey, North Carolina, Ohio, Ontario (CAN), Oregon, Pennsylvania, South Carolina, South Dakota, Tennessee, Vermont, Virginia, and West Virginia.

## Source Numbers

$100,105,114,126,157,167,177,197,207,211,217,267,275,293,300,319,320,356,357,367$, $384,387,407,435,522,550,552,579,598,601,603,614,637,711,716,720,728,735,868,869$, $903,925,936,1005,1007,1008,1010,1033,1066,1077,1078,1079$

# Single-Family Detached Housing (210) 

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday

## Setting/Location: General Urban/Suburban

Number of Studies: 174
Avg. Num. of Dwelling Units: 246
Directional Distribution: $50 \%$ entering, $50 \%$ exiting
Vehicle Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 9.43 | $4.45-22.61$ | 2.13 |

Data Plot and Equation


## Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

## Setting/Location: General Urban/Suburban

Number of Studies: 192
Avg. Num. of Dwelling Units: 226
Directional Distribution: 26\% entering, $74 \%$ exiting
Vehicle Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 0.70 | $0.27-2.27$ | 0.24 |

Data Plot and Equation


# Single-Family Detached Housing (210) 

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

## Setting/Location: General Urban/Suburban

Number of Studies: 208
Avg. Num. of Dwelling Units: 248
Directional Distribution: $63 \%$ entering, $37 \%$ exiting
Vehicle Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 0.94 | $0.35-2.98$ | 0.31 |

Data Plot and Equation


## Land Use: 215 Single-Family Attached Housing

## Description

Single-family attached housing includes any single-family housing unit that shares a wall with an adjoining dwelling unit, whether the walls are for living space, a vehicle garage, or storage space.

## Additional Data

The database for this land use includes duplexes (defined as a single structure with two distinct dwelling units, typically joined side-by-side and each with at least one outside entrance) and townhouses/rowhouses (defined as a single structure with three or more distinct dwelling units, joined side-by-side in a row and each with an outside entrance).

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (https://www.ite.org/technical-resources/topics/trip-and-parking-generation/).

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in British Columbia (CAN), California, Georgia, Illinois, Maryland, Massachusetts, Minnesota, New Jersey, Ontario (CAN), Oregon, Pennsylvania, South Dakota, Utah, Virginia, and Wisconsin.

## Source Numbers

$168,204,211,237,305,306,319,321,357,390,418,525,571,583,638,735,868,869,870,896$, 912, 959, 1009, 1046, 1056, 1058, 1077

# Single-Family Attached Housing <br> (215) 

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday

## Setting/Location: General Urban/Suburban

Number of Studies: 22
Avg. Num. of Dwelling Units: 120
Directional Distribution: $50 \%$ entering, $50 \%$ exiting
Vehicle Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 7.20 | $4.70-10.97$ | 1.61 |

Data Plot and Equation


# Single-Family Attached Housing <br> (215) 

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

## Setting/Location: General Urban/Suburban

Number of Studies: 46
Avg. Num. of Dwelling Units: 135
Directional Distribution: $31 \%$ entering, $69 \%$ exiting
Vehicle Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 0.48 | $0.12-0.74$ | 0.14 |

Data Plot and Equation


# Single-Family Attached Housing <br> (215) 

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

## Setting/Location: General Urban/Suburban

Number of Studies: 51
Avg. Num. of Dwelling Units: 136
Directional Distribution: $57 \%$ entering, $43 \%$ exiting
Vehicle Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 0.57 | $0.17-1.25$ | 0.18 |

Data Plot and Equation


# Land Use: 220 Multifamily Housing (Low-Rise) 

## Description

Low-rise multifamily housing includes apartments, townhouses, and condominiums located within the same building with at least three other dwelling units and that have two or three floors (levels). Various configurations fit this description, including walkup apartment, mansion apartment, and stacked townhouse.

- A walkup apartment typically is two or three floors in height with dwelling units that are accessed by a single or multiple entrances with stairways and hallways.
- A mansion apartment is a single structure that contains several apartments within what appears to be a single-family dwelling unit.
- A fourplex is a single two-story structure with two matching dwelling units on the ground and second floors. Access to the individual units is typically internal to the structure and provided through a central entry and stairway.
- A stacked townhouse is designed to match the external appearance of a townhouse. But, unlike a townhouse dwelling unit that only shares walls with an adjoining unit, the stacked townhouse units share both floors and walls. Access to the individual units is typically internal to the structure and provided through a central entry and stairway.

Multifamily housing (mid-rise) (Land Use 221), multifamily housing (high-rise) (Land Use 222), affordable housing (Land Use 223), and off-campus student apartment (low-rise) (Land Use 225) are related land uses.

## Land Use Subcategory

Data are presented for two subcategories for this land use: (1) not close to rail transit and (2) close to rail transit. A site is considered close to rail transit if the walking distance between the residential site entrance and the closest rail transit station entrance is $1 / 2$ mile or less.

## Additional Data

For the three sites for which both the number of residents and the number of occupied dwelling units were available, there were an average of 2.72 residents per occupied dwelling unit.

For the two sites for which the numbers of both total dwelling units and occupied dwelling units were available, an average of 96.2 percent of the total dwelling units were occupied.

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip
generation resource page on the ITE website (https://www.ite.org/technical-resources/topics/trip-and-parking-generation/).

For the three sites for which data were provided for both occupied dwelling units and residents, there was an average of 2.72 residents per occupied dwelling unit.

It is expected that the number of bedrooms and number of residents are likely correlated to the trips generated by a residential site. To assist in future analysis, trip generation studies of all multifamily housing should attempt to obtain information on occupancy rate and on the mix of residential unit sizes (i.e., number of units by number of bedrooms at the site complex).

The sites were surveyed in the 1980s, the 1990s, the 2000s, the 2010s, and the 2020s in British Columbia (CAN), California, Delaware, Florida, Georgia, Illinois, Indiana, Maine, Maryland, Massachusetts, Minnesota, New Jersey, Ontario (CAN), Oregon, Pennsylvania, South Carolina, South Dakota, Tennessee, Texas, Utah, and Washington.

## Source Numbers

188, 204, 237, 300, 305, 306, 320, 321, 357, 390, 412, 525, 530, 579, 583, 638, 864, 866, 896, 901, $903,904,936,939,944,946,947,948,963,964,966,967,1012,1013,1014,1036,1047,1056$, 1071, 1076

# Multifamily Housing (Low-Rise) Not Close to Rail Transit (220) 

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday

## Setting/Location: General Urban/Suburban

Number of Studies: 22
Avg. Num. of Dwelling Units: 229
Directional Distribution: $50 \%$ entering, $50 \%$ exiting
Vehicle Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 6.74 | $2.46-12.50$ | 1.79 |

Data Plot and Equation


# Multifamily Housing (Low-Rise) Not Close to Rail Transit (220) 

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

## Setting/Location: General Urban/Suburban

Number of Studies: 49
Avg. Num. of Dwelling Units: 249
Directional Distribution: $24 \%$ entering, $76 \%$ exiting
Vehicle Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 0.40 | $0.13-0.73$ | 0.12 |

Data Plot and Equation


# Multifamily Housing (Low-Rise) Not Close to Rail Transit (220) 

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

## Setting/Location: General Urban/Suburban

Number of Studies: 59
Avg. Num. of Dwelling Units: 241
Directional Distribution: 63\% entering, 37\% exiting
Vehicle Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 0.51 | $0.08-1.04$ | 0.15 |

Data Plot and Equation


# Land Use: 820 <br> Shopping Center (>150k) 

## Description

A shopping center is an integrated group of commercial establishments that is planned, developed, owned, and managed as a unit. Each study site in this land use has at least 150,000 square feet of gross leasable area (GLA). It often has more than one anchor store. Various names can be assigned to a shopping center within this size range, depending on its specific size and tenants, such as community center, regional center, superregional center, fashion center, and power center.

A shopping center of this size typically contains more than retail merchandising facilities. Office space, a movie theater, restaurants, a post office, banks, a health club, and recreational facilities are common tenants.

A shopping center of this size can be enclosed or open-air. The vehicle trips generated at a shopping center are based upon the total GLA of the center. In the case of a smaller center without an enclosed mall or peripheral buildings, the GLA is the same as the gross floor area of the building.

The 150,000 square feet GLA threshold value between community/regional shopping center and shopping plaza (Land Use 821) is based on an examination of trip generation data. For a shopping plaza that is smaller than the threshold value, the presence or absence of a supermarket within the plaza has a measurable effect on site trip generation. For a shopping center that is larger than the threshold value, the trips generated by its other major tenants mask any effects of the presence or absence of an on-site supermarket.

Shopping plaza (40-150k) (Land Use 821), strip retail plaza (<40k) (Land Use 822), and factory outlet center (Land Use 823) are related uses.

## Additional Data

Many shopping centers-in addition to the integrated unit of shops in one building or enclosed around a mall-include outparcels (peripheral buildings or pads located on the perimeter of the center adjacent to the streets and major access points). These buildings are typically drive-in banks, retail stores, restaurants, or small offices. Although the data herein do not indicate which of the centers studied include peripheral buildings, it can be assumed that some of the data show their effect.

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (https://www.ite.org/technical-resources/topics/trip-and-parking-generation/).

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Alberta (CAN), California, Colorado, Connecticut, Delaware, Florida, Georgia, Illinois, Indiana, Iowa, Kentucky,

Maryland, Massachusetts, Michigan, Minnesota, New Jersey, New York, North Carolina, Ohio, Oklahoma, Pennsylvania, Tennessee, Texas, Vermont, Virginia, Washington, West Virginia, and Wisconsin.

## Source Numbers

$77,110,154,156,159,190,199,202,204,213,251,269,294,295,299,304,305,307,308,309$, $311,314,315,316,317,319,365,385,404,414,423,442,446,562,629,702,715,728,868,871$, $880,899,912,926,946,962,973,974,978,1034,1040,1067$

## Shopping Center (>150k) (820)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA
On a: Weekday

## Setting/Location: General Urban/Suburban

Number of Studies: 108
Avg. 1000 Sq. Ft. GLA: 538
Directional Distribution: 50\% entering, $50 \%$ exiting
Vehicle Trip Generation per 1000 Sq. Ft. GLA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 37.01 | $17.27-81.53$ | 12.79 |

Data Plot and Equation


## Shopping Center (>150k) (820)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

## Setting/Location: General Urban/Suburban

Number of Studies: 44
Avg. 1000 Sq. Ft. GLA: 546
Directional Distribution: 62\% entering, 38\% exiting
Vehicle Trip Generation per 1000 Sq. Ft. GLA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 0.84 | $0.30-3.11$ | 0.42 |

Data Plot and Equation


## Shopping Center (>150k) (820)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

## Setting/Location: General Urban/Suburban

Number of Studies: 126
Avg. 1000 Sq. Ft. GLA: 581
Directional Distribution: $48 \%$ entering, $52 \%$ exiting
Vehicle Trip Generation per 1000 Sq. Ft. GLA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 3.40 | $1.57-7.58$ | 1.26 |

Data Plot and Equation


## Land Use: 750 Office Park

## Description

An office park is typically a suburban subdivision or planned unit development that contains general office buildings and support services, such as banks, restaurants, and service stations, arranged in a park- or campus-like atmosphere. General office building (Land Use 710), corporate headquarters building (Land Use 714), single tenant office building (Land Use 715), research and development center (Land Use 760), and business park (Land Use 770) are related uses.

## Additional Data

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Alberta (CAN), Connecticut, Georgia, Indiana, Massachusetts, New Jersey, New York, and Pennsylvania.

## Source Numbers

160, 161, 184, 185, 253, 300, 301, 356, 550, 618, 912, 972, 973

# Office Park <br> (750) 

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday

## Setting/Location: General Urban/Suburban

Number of Studies: 10
Avg. 1000 Sq. Ft. GFA: 479
Directional Distribution: 50\% entering, $50 \%$ exiting
Vehicle Trip Generation per 1000 Sq. Ft. GFA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 11.07 | $7.56-14.50$ | 2.14 |

Data Plot and Equation


# Office Park <br> (750) 

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

## Setting/Location: General Urban/Suburban

Number of Studies: 23
Avg. 1000 Sq. Ft. GFA: 498
Directional Distribution: 89\% entering, 11\% exiting

## Vehicle Trip Generation per 1000 Sq. Ft. GFA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 1.33 | $0.60-4.74$ | 0.51 |

Data Plot and Equation


# Office Park <br> (750) 

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

## Setting/Location: General Urban/Suburban

Number of Studies: 20
Avg. 1000 Sq. Ft. GFA: 563
Directional Distribution: 14\% entering, $86 \%$ exiting

## Vehicle Trip Generation per 1000 Sq. Ft. GFA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 1.30 | $0.64-3.03$ | 0.32 |

## Data Plot and Equation



# Land Use: 730 Government Office Building 

## Description

A government office building is an individual building containing either the entire function or simply one agency of a city, county, state, federal, or other governmental unit.

## Additional Data

Each study site in the current database serves a municipal or county agency.
The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (https://www.ite.org/technical-resources/topics/trip-and-parking-generation/).

The sites were surveyed in the 2000s and the 2010s in Oregon and Texas.

## Source Numbers

579, 889

## Government Office Building <br> (730)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday

## Setting/Location: General Urban/Suburban

Number of Studies: 7
Avg. 1000 Sq. Ft. GFA: 11
Directional Distribution: 50\% entering, 50\% exiting
Vehicle Trip Generation per 1000 Sq. Ft. GFA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 22.59 | $0.71-59.66$ | 17.03 |

Data Plot and Equation


## Government Office Building <br> (730)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

## Setting/Location: General Urban/Suburban

Number of Studies: 7
Avg. 1000 Sq. Ft. GFA: 11
Directional Distribution: $75 \%$ entering, $25 \%$ exiting
Vehicle Trip Generation per 1000 Sq. Ft. GFA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 3.34 | $0.45-7.38$ | 2.18 |

Data Plot and Equation


## Government Office Building <br> (730)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

## Setting/Location: General Urban/Suburban

Number of Studies: 8
Avg. 1000 Sq. Ft. GFA: 22
Directional Distribution: $25 \%$ entering, $75 \%$ exiting
Vehicle Trip Generation per 1000 Sq. Ft. GFA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 1.71 | $1.09-6.19$ | 1.24 |

## Data Plot and Equation



## Land Use: 150 Warehousing

## Description

A warehouse is primarily devoted to the storage of materials, but it may also include office and maintenance areas. High-cube transload and short-term storage warehouse (Land Use 154), highcube fulfillment center warehouse (Land Use 155), high-cube parcel hub warehouse (Land Use 156), and high-cube cold storage warehouse (Land Use 157) are related uses.

## Additional Data

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (https://www.ite.org/technical-resources/topics/trip-and-parking-generation/).

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in California, Connecticut, Minnesota, New Jersey, New York, Ohio, Oregon, Pennsylvania, and Texas.

## Source Numbers

$184,331,406,411,443,579,583,596,598,611,619,642,752,869,875,876,914,940,1050$

## Warehousing <br> (150)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday

## Setting/Location: General Urban/Suburban

Number of Studies: 31
Avg. 1000 Sq. Ft. GFA: 292
Directional Distribution: 50\% entering, $50 \%$ exiting
Vehicle Trip Generation per 1000 Sq. Ft. GFA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 1.71 | $0.15-16.93$ | 1.48 |

Data Plot and Equation


## Warehousing <br> (150)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

## Setting/Location: General Urban/Suburban

Number of Studies: 36
Avg. 1000 Sq. Ft. GFA: 448
Directional Distribution: 77\% entering, 23\% exiting
Vehicle Trip Generation per 1000 Sq. Ft. GFA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 0.17 | $0.02-1.93$ | 0.19 |

Data Plot and Equation


## Warehousing <br> (150)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

## Setting/Location: General Urban/Suburban

Number of Studies: 49
Avg. 1000 Sq. Ft. GFA: 400
Directional Distribution: $28 \%$ entering, $72 \%$ exiting

## Vehicle Trip Generation per 1000 Sq. Ft. GFA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 0.18 | $0.01-1.80$ | 0.18 |

## Data Plot and Equation



| NCHRP 8-51 Internal Trip Capture Estimation Tool |  |  |  |  |
| ---: | :---: | ---: | ---: | ---: |
| Project Name: | Downtown Taft Specific Plan |  | Organization: |  |
| Project Location: | Taft, CA |  | Performed By: |  |
| Scenario Description: |  | Date: |  |  |
| Analysis Year: | Horizon Year 2042 |  | Checked By: |  |
| Analysis Period: | AM Street Peak Hour | Date: |  |  |

Table 1-A: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)

| Table 1-A: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Land Use | Development Data (For Information Only) |  |  | Estimated Vehicle-Trips |  |  |
|  | ITE LUCs ${ }^{1}$ | Quantity | Units | Total | Entering | Exiting |
| Office |  |  |  | 2007 | 1682 | 325 |
| Retail |  |  |  | 659 | 409 | 250 |
| Restaurant |  |  |  | 0 |  |  |
| Cinema/Entertainment |  |  |  | 0 |  |  |
| Residential |  |  |  | 1543 | 419 | 1124 |
| Hotel |  |  |  | 0 |  |  |
| All Other Land Uses ${ }^{2}$ |  |  |  | 51 | 39 | 12 |
| Total |  |  |  | 4260 | 2549 | 1711 |


| Table 2-A: Mode Split and Vehicle Occupancy Estimates |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Land Use | Entering Trips |  |  | Exiting Trips |  |  |
|  | Veh. Occ. | \% Transit | \% Non-Motorized | Veh. Occ. | \% Transit | \% Non-Motorized |
| Office | 1.00 |  |  | 1.00 |  |  |
| Retail | 1.00 |  |  | 1.00 |  |  |
| Restaurant | 1.00 |  |  | 1.00 |  |  |
| Cinema/Entertainment |  |  |  |  |  |  |
| Residential |  |  |  |  |  |  |
| Hotel |  |  |  |  |  |  |
| All Other Land Uses ${ }^{2}$ |  |  |  |  |  |  |

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)

| Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance) |  |  |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin (From) |  | Destination (To) |  |  |  |  |  |
|  | Office | Retail | Restaurant | Cinema/Entertainment | Residential |  |  |
| Office |  |  |  |  |  |  |  |
| Retail |  |  |  |  |  |  |  |
| Restaurant |  |  |  |  |  |  |  |
| Cinema/Entertainment |  |  |  |  |  |  |  |
| Residential |  |  |  |  |  |  |  |
| Hotel |  |  |  |  |  |  |  |


| Table 4-A: Internal Person-Trip Origin-Destination Matrix* |  |  |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin (From) |  | Destination (To) |  |  |  |  |  |
|  | Office | Retail | Restaurant | Cinema/Entertainment | Residential |  |  |
| Office |  | 91 | 0 | 0 | 0 | 0 |  |
| Retail | 67 |  | 0 | 0 | 0 | 0 |  |
| Restaurant | 0 | 0 |  | 0 | 0 |  |  |
| Cinema/Entertainment | 0 | 0 | 0 |  | 0 |  |  |
| Residential | 22 | 11 | 0 | 0 | 0 |  |  |
| Hotel | 0 | 0 | 0 | 0 | 0 |  |  |


| Table 5-A: Computations Summary |  |  |  | Table 6-A: Internal Trip Capture Percentages by Land Use |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Total | Entering | Exiting | Land Use | Entering Trips | Exiting Trips |
| All Person-Trips | 4,260 | 2,549 | 1,711 | Office | 5\% | 28\% |
| Internal Capture Percentage | 9\% | 8\% | 12\% | Retail | 25\% | 30\% |
|  |  |  |  | Restaurant | N/A | N/A |
| External Vehicle-Trips ${ }^{3}$ | 3,862 | 2,350 | 1,512 | Cinema/Entertainment | N/A | N/A |
| External Transit-Trips ${ }^{4}$ | 0 | 0 | 0 | Residential | 2\% | 3\% |
| External Non-Motorized Trips ${ }^{4}$ | 0 | 0 | 0 | Hotel | N/A | N/A |

${ }^{1}$ Land Use Codes (LUCs) from Trip Generation Informational Report, published by the Institute of Transportation Engineers.
${ }^{2}$ Total estimate for all other land uses at mixed-use development site-not subject to internal trip capture computations in this estimator
${ }^{3}$ Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A

## ${ }^{4}$ Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

| Project Name: | Downtown Taft Specific Plan |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Analysis Period: | AM Street Peak Hour |  |  |  |  |  |
| Table 7-A: Conversion of Vehicle-Trip Ends to Person-Trip Ends |  |  |  |  |  |  |
| Land Use | Table 7-A (D): Entering Trips |  |  | Table 7-A (O): Exiting Trips |  |  |
|  | Veh. Occ. | Vehicle-Trips | Person-Trips* | Veh. Occ. | Vehicle-Trips | Person-Trips* |
| Office | 1.00 | 1682 | 1682 | 1.00 | 325 | 325 |
| Retail | 1.00 | 409 | 409 | 1.00 | 250 | 250 |
| Restaurant | 1.00 | 0 | 0 | 1.00 | 0 | 0 |
| Cinema/Entertainment | 1.00 | 0 | 0 | 1.00 | 0 | 0 |
| Residential | 1.00 | 419 | 419 | 1.00 | 1124 | 1124 |
| Hotel | 1.00 | 0 | 0 | 1.00 | 0 | 0 |


| Table 8-A (0): Internal Person-Trip Origin-Destination Matrix (Computed at Origin) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin (From) | Destination (To) |  |  |  |  |  |
|  | Office | Retail | Restaurant | Cinema/Entertainment | Residential | Hotel |
| Office |  | 91 | 205 | 0 | 3 | 0 |
| Retail | 73 |  | 33 | 0 | 35 | 0 |
| Restaurant | 0 | 0 |  | 0 | 0 | 0 |
| Cinema/Entertainment | 0 | 0 | 0 |  | 0 | 0 |
| Residential | 22 | 11 | 225 | 0 |  | 0 |
| Hotel | 0 | 0 | 0 | 0 | 0 |  |


| Table 8-A (D): Internal Person-Trip Origin-Destination Matrix (Computed at Destination) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin (From) | Destination (To) |  |  |  |  |  |
|  | Office | Retail | Restaurant | Cinema/Entertainment | Residential | Hotel |
| Office |  | 131 | 0 | 0 | 0 | 0 |
| Retail | 67 |  | 0 | 0 | 8 | 0 |
| Restaurant | 235 | 33 |  | 0 | 21 | 0 |
| Cinema/Entertainment | 0 | 0 | 0 |  | 0 | 0 |
| Residential | 50 | 70 | 0 | 0 |  | 0 |
| Hotel | 50 | 16 | 0 | 0 | 0 |  |


| Table 9-A (D): Internal and External Trips Summary (Entering Trips) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Destination Land Use | Person-Trip Estimates |  |  | External Trips by Mode* |  |  |
|  | Internal | External | Total | Vehicles ${ }^{1}$ | Transit ${ }^{2}$ | Non-Motorized ${ }^{2}$ |
| Office | 89 | 1593 | 1682 | 1593 | 0 | 0 |
| Retail | 102 | 307 | 409 | 307 | 0 | 0 |
| Restaurant | 0 | 0 | 0 | 0 | 0 | 0 |
| Cinema/Entertainment | 0 | 0 | 0 | 0 | 0 | 0 |
| Residential | 8 | 411 | 419 | 411 | 0 | 0 |
| Hotel | 0 | 0 | 0 | 0 | 0 | 0 |
| All Other Land Uses ${ }^{3}$ | 0 | 39 | 39 | 39 | 0 | 0 |


| Table 9-A (0): Internal and External Trips Summary (Exiting Trips) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin Land Use | Person-Trip Estimates |  |  | External Trips by Mode* |  |  |
|  | Internal | External | Total | Vehicles ${ }^{1}$ | Transit ${ }^{2}$ | Non-Motorized ${ }^{2}$ |
| Office | 91 | 234 | 325 | 234 | 0 | 0 |
| Retail | 75 | 175 | 250 | 175 | 0 | 0 |
| Restaurant | 0 | 0 | 0 | 0 | 0 | 0 |
| Cinema/Entertainment | 0 | 0 | 0 | 0 | 0 | 0 |
| Residential | 33 | 1091 | 1124 | 1091 | 0 | 0 |
| Hotel | 0 | 0 | 0 | 0 | 0 | 0 |
| All Other Land Uses ${ }^{3}$ | 0 | 12 | 12 | 12 | 0 | 0 |

${ }^{1}$ Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A

## ${ }^{2}$ Person-Trips

${ }^{3}$ Total estimate for all other land uses at mixed-use development site-not subject to internal trip capture computations in this estimator
*Indicates computation that has been rounded to the nearest whole number.

| NCHRP 8-51 Internal Trip Capture Estimation Tool |  |  |  |
| ---: | :---: | :---: | ---: |
| Project Name: | Downtown Taft Specific Plan |  | Organization: |
| Project Location: | Taft, CA |  |  |
| Scenario Description: |  | Performed By: |  |
| Analysis Year: | Horizon Year 2042 | Date: |  |
| Analysis Period: | PM Street Peak Hour | Checked By: |  |
|  |  | Date: |  |


| Table 1-P: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Land Use | Development Data (For Information Only) |  |  | Estimated Vehicle-Trips |  |  |
|  | ITE LUCs ${ }^{1}$ | Quantity | Units | Total | Entering | Exiting |
| Office |  |  |  | 1803 | 292 | 1511 |
| Retail |  |  |  | 2726 | 1308 | 1418 |
| Restaurant |  |  |  | 0 |  |  |
| Cinema/Entertainment |  |  |  | 0 |  |  |
| Residential |  |  |  | 1948 | 1187 | 761 |
| Hotel |  |  |  | 0 |  |  |
| All Other Land Uses ${ }^{2}$ |  |  |  | 54 | 15 | 39 |
| Total |  |  |  | 6531 | 2802 | 3729 |


| Table 2-P: Mode Split and Vehicle Occupancy Estimates |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Land Use | Entering Trips |  |  | Exiting Trips |  |  |
|  | Veh. Occ. | \% Transit | \% Non-Motorized | Veh. Occ. | \% Transit | \% Non-Motorized |
| Office | 1.00 |  |  | 1.00 |  |  |
| Retail | 1.00 |  |  | 1.00 |  |  |
| Restaurant | 1.00 |  |  | 1.00 |  |  |
| Cinema/Entertainment |  |  |  |  |  |  |
| Residential |  |  |  |  |  |  |
| Hotel |  |  |  |  |  |  |
| All Other Land Uses ${ }^{2}$ |  |  |  |  |  |  |


| Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance) |  |  |  |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin (From) |  | Destination (To) |  |  |  |  |  |  |
|  | Office | Retail | Restaurant | Cinema/Entertainment | Residential |  |  |  |
| Office |  |  |  |  |  |  |  |  |
| Retail |  |  |  |  |  |  |  |  |
| Restaurant |  |  |  |  |  |  |  |  |
| Cinema/Entertainment |  |  |  |  |  |  |  |  |
| Residential |  |  |  |  |  |  |  |  |
| Hotel |  |  |  |  |  |  |  |  |


| Table 4-P: Internal Person-Trip Origin-Destination Matrix* |  |  |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin (From) | Destination (To) |  |  |  |  |  |  |
|  | Office | Retail | Restaurant | Cinema/Entertainment | Residential | Hotel |  |
| Office |  | 105 | 0 | 0 | 30 | 0 |  |
| Retail | 28 |  | 0 | 0 | 369 | 0 |  |
| Restaurant | 0 | 0 |  | 0 | 0 | 0 |  |
| Cinema/Entertainment | 0 | 0 | 0 |  | 0 |  |  |
| Residential | 30 | 131 | 0 | 0 | 0 |  |  |
| Hotel | 0 | 0 | 0 | 0 | 0 | 0 |  |


| Table 5-P: Computations Summary |  |  |  | Table 6-P: Internal Trip Capture Percentages by Land Use |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Total | Entering | Exiting | Land Use | Entering Trips | Exiting Trips |
| All Person-Trips | 6,531 | 2,802 | 3,729 | Office | 20\% | 9\% |
| Internal Capture Percentage | 21\% | 25\% | 19\% | Retail | 18\% | 28\% |
|  |  |  |  | Restaurant | N/A | N/A |
| External Vehicle-Trips ${ }^{3}$ | 5,145 | 2,109 | 3,036 | Cinema/Entertainment | N/A | N/A |
| External Transit-Trips ${ }^{4}$ | 0 | 0 | 0 | Residential | 34\% | 21\% |
| External Non-Motorized Trips ${ }^{4}$ | 0 | 0 | 0 | Hotel | N/A | N/A |

${ }^{1}$ Land Use Codes (LUCs) from Trip Generation Informational Report , published by the Institute of Transportation Engineers.
${ }^{2}$ Total estimate for all other land uses at mixed-use development site-not subject to internal trip capture computations in this estimator
${ }^{3}$ Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P

## ${ }^{4}$ Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

| Project Name: | Downtown Taft Specific Plan |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Analysis Period: | PM Street Peak Hour |  |  |  |  |  |
| Table 7-P: Conversion of Vehicle-Trip Ends to Person-Trip Ends |  |  |  |  |  |  |
| Land Use | Table 7-P (D): Entering Trips |  |  | Table 7-P (0): Exiting Trips |  |  |
|  | Veh. Occ. | Vehicle-Trips | Person-Trips* | Veh. Occ. | Vehicle-Trips | Person-Trips* |
| Office | 1.00 | 292 | 292 | 1.00 | 1511 | 1511 |
| Retail | 1.00 | 1308 | 1308 | 1.00 | 1418 | 1418 |
| Restaurant | 1.00 | 0 | 0 | 1.00 | 0 | 0 |
| Cinema/Entertainment | 1.00 | 0 | 0 | 1.00 | 0 | 0 |
| Residential | 1.00 | 1187 | 1187 | 1.00 | 761 | 761 |
| Hotel | 1.00 | 0 | 0 | 1.00 | 0 | 0 |


| Table 8-P (O): Internal Person-Trip Origin-Destination Matrix (Computed at Origin) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin (From) | Destination (To) |  |  |  |  |  |
|  | Office | Retail | Restaurant | Cinema/Entertainment | Residential | Hotel |
| Office |  | 302 | 60 | 0 | 30 | 0 |
| Retail | 28 |  | 411 | 57 | 369 | 71 |
| Restaurant | 0 | 0 |  | 0 | 0 | 0 |
| Cinema/Entertainment | 0 | 0 | 0 |  | 0 | 0 |
| Residential | 30 | 320 | 160 | 0 |  | 23 |
| Hotel | 0 | 0 | 0 | 0 | 0 |  |


| Table 8-P (D): Internal Person-Trip Origin-Destination Matrix (Computed at Destination) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin (From) | Destination (To) |  |  |  |  |  |
|  | Office | Retail | Restaurant | Cinema/Entertainment | Residential | Hotel |
| Office |  | 105 | 0 | 0 | 47 | 0 |
| Retail | 91 |  | 0 | 0 | 546 | 0 |
| Restaurant | 88 | 654 |  | 0 | 190 | 0 |
| Cinema/Entertainment | 18 | 52 | 0 |  | 47 | 0 |
| Residential | 166 | 131 | 0 | 0 |  | 0 |
| Hotel | 0 | 26 | 0 | 0 | 0 |  |


| Table 9-P (D): Internal and External Trips Summary (Entering Trips) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Destination Land Use | Person-Trip Estimates |  |  | External Trips by Mode* |  |  |
|  | Internal | External | Total | Vehicles ${ }^{1}$ | Transit ${ }^{2}$ | Non-Motorized ${ }^{2}$ |
| Office | 58 | 234 | 292 | 234 | 0 | 0 |
| Retail | 236 | 1072 | 1308 | 1072 | 0 | 0 |
| Restaurant | 0 | 0 | 0 | 0 | 0 | 0 |
| Cinema/Entertainment | 0 | 0 | 0 | 0 | 0 | 0 |
| Residential | 399 | 788 | 1187 | 788 | 0 | 0 |
| Hotel | 0 | 0 | 0 | 0 | 0 | 0 |
| All Other Land Uses ${ }^{3}$ | 0 | 15 | 15 | 15 | 0 | 0 |


| Table 9-P (0): Internal and External Trips Summary (Exiting Trips) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin Land Use | Person-Trip Estimates |  |  | External Trips by Mode* |  |  |
| Origin Land Use | Internal | External | Total | Vehicles ${ }^{1}$ | Transit ${ }^{2}$ | Non-Motorized ${ }^{2}$ |
| Office | 135 | 1376 | 1511 | 1376 | 0 | 0 |
| Retail | 397 | 1021 | 1418 | 1021 | 0 | 0 |
| Restaurant | 0 | 0 | 0 | 0 | 0 | 0 |
| Cinema/Entertainment | 0 | 0 | 0 | 0 | 0 | 0 |
| Residential | 161 | 600 | 761 | 600 | 0 | 0 |
| Hotel | 0 | 0 | 0 | 0 | 0 | 0 |
| All Other Land Uses ${ }^{3}$ | 0 | 39 | 39 | 39 | 0 | 0 |

${ }^{1}$ Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P

[^1]
## APPENDIX D

KERN COG HORIZON YEAR 2042 WITH SPECIFIC PLAN REGIONAL MODEL ADT PLOT AND VOLUME POST-PROCESSING WORKSHEETS
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# APPENDIX E 

HORIZON YEAR 2042 WITH PROJECT INTERSECTION OPERATIONS ANALYSIS WORKSHEETS

|  | $\rangle$ | $\rightarrow$ |  | 7 | $\leftarrow$ |  | 4 | 4 | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ |  |  | \% | 瑯 |  | \% | 性 |  | ${ }^{*}$ | ¢4 | F |
| Traffic Volume (veh/h) | 207 | 387 | 91 | 126 | 335 | 98 | 111 | 301 | 214 | 95 | 289 | 151 |
| Future Volume (veh/h) | 207 | 387 | 91 | 126 | 335 | 98 | 111 | 301 | 214 | 95 | 289 | 151 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 |  | 1.00 | 1.00 |  | 0.98 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 269 | 503 | 118 | 164 | 435 | 127 | 144 | 391 | 278 | 123 | 375 | 196 |
| Peak Hour Factor | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 |
| Percent Heavy Veh, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 300 | 833 | 194 | 194 | 628 | 182 | 173 | 547 | 384 | 151 | 931 | 414 |
| Arrive On Green | 0.17 | 0.29 | 0.29 | 0.11 | 0.23 | 0.23 | 0.10 | 0.27 | 0.27 | 0.08 | 0.26 | 0.26 |
| Sat Flow, veh/h | 1781 | 2859 | 667 | 1781 | 2708 | 783 | 1781 | 1991 | 1399 | 1781 | 3554 | 1580 |
| Grp Volume(v), veh/h | 269 | 312 | 309 | 164 | 284 | 278 | 144 | 348 | 321 | 123 | 375 | 196 |
| Grp Sat Flow(s),veh/h/ln | 1781 | 1777 | 1749 | 1781 | 1777 | 1714 | 1781 | 1777 | 1613 | 1781 | 1777 | 1580 |
| Q Serve(g_s), s | 15.1 | 15.4 | 15.6 | 9.3 | 15.0 | 15.2 | 8.1 | 18.1 | 18.4 | 7.0 | 8.9 | 10.7 |
| Cycle Q Clear(g_c), s | 15.1 | 15.4 | 15.6 | 9.3 | 15.0 | 15.2 | 8.1 | 18.1 | 18.4 | 7.0 | 8.9 | 10.7 |
| Prop In Lane | 1.00 |  | 0.38 | 1.00 |  | 0.46 | 1.00 |  | 0.87 | 1.00 |  | 1.00 |
| Lane Grp Cap(c), veh/h | 300 | 517 | 509 | 194 | 412 | 398 | 173 | 488 | 443 | 151 | 931 | 414 |
| V/C Ratio(X) | 0.90 | 0.60 | 0.61 | 0.84 | 0.69 | 0.70 | 0.83 | 0.71 | 0.72 | 0.81 | 0.40 | 0.47 |
| Avail Cap(c_a), veh/h | 334 | 711 | 700 | 210 | 588 | 567 | 188 | 573 | 520 | 162 | 1093 | 486 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(l) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 41.7 | 31.2 | 31.2 | 44.8 | 35.9 | 36.1 | 45.4 | 33.5 | 33.6 | 46.1 | 31.2 | 31.8 |
| Incr Delay (d2), s/veh | 22.6 | 5.1 | 5.3 | 22.6 | 5.6 | 6.0 | 22.4 | 8.6 | 9.9 | 22.9 | 1.3 | 3.8 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile BackOfQ(50\%),veh/ln | 8.4 | 7.2 | 7.2 | 5.2 | 7.0 | 6.9 | 4.6 | 8.8 | 8.2 | 4.0 | 3.9 | 4.4 |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay (d),s/veh | 64.3 | 36.3 | 36.6 | 67.3 | 41.5 | 42.1 | 67.8 | 42.1 | 43.5 | 69.0 | 32.5 | 35.7 |
| LnGrp LOS | E | D | D | E | D | D | E | D | D | E | C | D |
| Approach Vol, veh/h |  | 890 |  |  | 726 |  |  | 813 |  |  | 694 |  |
| Approach Delay, s/veh |  | 44.9 |  |  | 47.6 |  |  | 47.2 |  |  | 39.8 |  |
| Approach LOS |  | D |  |  | D |  |  | D |  |  | D |  |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |  |  |  |  |
| Phs Duration ( $\mathrm{G}+\mathrm{Y}+\mathrm{Rc}$ ), s | 16.9 | 36.6 | 15.7 | 33.2 | 22.9 | 30.6 | 14.4 | 34.5 |  |  |  |  |
| Change Period ( $Y+R \mathrm{R}$ ), s | * 5.7 | 6.8 | * 5.7 | 6.4 | * 5.7 | 6.8 | * 5.7 | 6.4 |  |  |  |  |
| Max Green Setting (Gmax), s | *12 | 41.0 | *11 | 31.5 | *19 | 33.9 | *9.3 | 33.0 |  |  |  |  |
| Max Q Clear Time (g_c+1), s | 11.3 | 17.6 | 10.1 | 12.7 | 17.1 | 17.2 | 9.0 | 20.4 |  |  |  |  |
| Green Ext Time (p_c), s | 0.0 | 10.3 | 0.0 | 7.8 | 0.1 | 6.0 | 0.0 | 7.3 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Ctrl Delay |  |  | 45.0 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | D |  |  |  |  |  |  |  |  |  |
| Notes |  |  |  |  |  |  |  |  |  |  |  |  |

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.


## Intersection


Intersection LOS
F

| Movement EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | * $\uparrow$ |  |  | * ${ }^{\text {¢ }}$ |  |  | * $\uparrow$ |  |  | * $\uparrow$ |  |
| Traffic Vol, veh/h 133 | 382 | 40 | 47 | 412 | 76 | 57 | 296 | 61 | 84 | 260 | 147 |
| Future Vol, veh/h 133 | 382 | 40 | 47 | 412 | 76 | 57 | 296 | 61 | 84 | 260 | 147 |
| Peak Hour Factor 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 |
| Heavy Vehicles, \% 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow 182 | 523 | 55 | 64 | 564 | 104 | 78 | 405 | 84 | 115 | 356 | 201 |
| Number of Lanes 0 | 2 | 0 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 2 | 0 |
| Approach EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes 2 |  |  | 2 |  |  | 2 |  |  | 2 |  |  |
| Conflicting Approach Left SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left 2 |  |  | 2 |  |  | 2 |  |  | 2 |  |  |
| Conflicting Approach RighNB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right 2 |  |  | 2 |  |  | 2 |  |  | 2 |  |  |
| HCM Control Delay 163.9 |  |  | 122.7 |  |  | 66.7 |  |  | 103.1 |  |  |
| HCM LOS F |  |  | F |  |  | F |  |  | F |  |  |


| Lane | NBLn1 NBLn2 EBLn1 EBLn2WBLn1WBLn2 SBLn1 SBLn2 |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $28 \%$ | $0 \%$ | $41 \%$ | $0 \%$ | $19 \%$ | $0 \%$ | $39 \%$ | $0 \%$ |
| Vol Thru, \% | $72 \%$ | $71 \%$ | $59 \%$ | $83 \%$ | $81 \%$ | $73 \%$ | $61 \%$ | $47 \%$ |
| Vol Right, $\%$ | $0 \%$ | $29 \%$ | $0 \%$ | $17 \%$ | $0 \%$ | $27 \%$ | $0 \%$ | $53 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 205 | 209 | 324 | 231 | 253 | 282 | 214 | 277 |
| LT Vol | 57 | 0 | 133 | 0 | 47 | 0 | 84 | 0 |
| Through Vol | 148 | 148 | 191 | 191 | 206 | 206 | 130 | 130 |
| RT Vol | 0 | 61 | 0 | 40 | 0 | 76 | 0 | 147 |
| Lane Flow Rate | 281 | 286 | 444 | 316 | 347 | 386 | 293 | 379 |
| Geometry Grp | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.896 | 0.886 | 1.389 | 0.962 | 1.067 | 1.159 | 0.921 | 1.132 |
| Departure Headway (Hd) | 12.987 | 12.62512 .035 | 11.69 | 12.078 | 11.78112 .475 | 11.875 |  |  |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 281 | 289 | 306 | 313 | 305 | 313 | 293 | 307 |
| Service Time | 10.687 | 10.325 | 9.735 | 9.39 | 9.778 | 9.48110 .175 | 9.575 |  |
| HCM Lane V/C Ratio | 1 | 0.99 | 1.451 | 1.01 | 1.138 | 1.233 | 1 | 1.235 |
| HCM Control Delay | 68.3 | 65.1 | 225.5 | 77.5 | 107.5 | 136.4 | 71.5 | 127.5 |
| HCM Lane LOS | F | F | F | F | F | F | F | F |
| HCM 95th-tile Q | 8 | 7.9 | 21.7 | 9.8 | 12.2 | 15 | 8.7 | 14.1 |

Intersection
Intersection Delay, s/velim. F
Intersection LOS F


| Lane | NBLn1 EBLn1 | EBLn2WBLn1WBLn2 SBLn1 |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $23 \%$ | $37 \%$ | $0 \%$ | $21 \%$ | $0 \%$ | $19 \%$ |
| Vol Thru, \% | $63 \%$ | $63 \%$ | $75 \%$ | $79 \%$ | $76 \%$ | $61 \%$ |
| Vol Right, \% | $14 \%$ | $0 \%$ | $25 \%$ | $0 \%$ | $24 \%$ | $20 \%$ |
| Sign Control | Sttop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 392 | 251 | 213 | 220 | 228 | 428 |
| LT Vol | 91 | 92 | 0 | 47 | 0 | 81 |
| Through Vol | 248 | 159 | 159 | 173 | 173 | 263 |
| RT Vol | 53 | 0 | 54 | 0 | 55 | 84 |
| Lane Flow Rate | 496 | 318 | 270 | 278 | 288 | 542 |
| Geometry Grp | 2 | 7 | 7 | 7 | 7 | 2 |
| Degree of Util (X) | 1.286 | 0.859 | 0.702 | 0.749 | 0.754 | 1.397 |
| Departure Headway (Hd) | 10.54211 .42911 .04511 .40311 .10810 .211 |  |  |  |  |  |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 334 | 321 | 329 | 320 | 328 | 363 |
| Service Time | 8.542 | 9.129 | 8.745 | 9.103 | 8.808 | 8.211 |
| HCM Lane V/C Ratio | 1.429 | 0.991 | 0.821 | 0.869 | 0.878 | 1.493 |
| HCM Control Delay | 179.1 | 56.1 | 36.1 | 41.5 | 41.1 | 222.5 |
| HCM Lane LOS | F | F | E | E | E | F |
| HCM 95th-tile Q | 20.3 | 7.7 | 5 | 5.7 | 5.8 | 24.9 |

Generated with PTV VISTRO
Version 2021 (SP 0-6)
Scenario 3: 3 HY 2042 AM

## Intersection Level Of Service Report

Intersection 4: Kern Street (SR-33) / 1st Street

| Control Type: | Two-way stop | Delay $(\mathrm{sec} / \mathrm{veh}):$ | 47.6 |
| :---: | :---: | :---: | :---: |
| Analysis Method: | HCM 6th Edition | Level Of Service: | E |
| Analysis Period: | 15 minutes | Volume to Capacity $(\mathrm{v} / \mathrm{c}):$ | 0.084 |

Intersection Setup

| Name | 1st Street |  |  |  | 1st Street |  |  |  | Kern Street (SR-33) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Northbound |  |  |  | Southbound |  |  |  | Eastbound |  |  |  |
| Lane Configuration | $\stackrel{1}{4}$ |  |  |  | $\Rightarrow$ |  |  |  | $4$ |  |  |  |
| Turning Movement | Left2 | Left | Thru | Right | Left2 | Left | Thru | Right | Left2 | Left | Thru | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 30.00 |  |  |  | 30.00 |  |  |  | 30.00 |  |  |  |
| Grade [\%] | 0.00 |  |  |  | 0.00 |  |  |  | 0.00 |  |  |  |
| Crosswalk | No |  |  |  | No |  |  |  | No |  |  |  |

Volumes

| Name | 1st Street |  |  |  | 1st Street |  |  |  | Kern Street (SR-33) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 40 | 10 | 20 | 10 | 10 | 10 | 28 | 37 | 24 | 136 | 143 | 60 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 2.00 | 2.00 | 0.00 | 2.00 | 0.00 | 2.00 | 2.00 | 2.00 | 2.00 | 0.00 | 2.00 | 2.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 40 | 10 | 20 | 10 | 10 | 10 | 28 | 37 | 24 | 136 | 143 | 60 |
| Peak Hour Factor | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 11 | 3 | 6 | 3 | 3 | 3 | 8 | 11 | 7 | 39 | 41 | 17 |
| Total Analysis Volume [veh/h] | 46 | 11 | 23 | 11 | 11 | 11 | 32 | 43 | 28 | 156 | 164 | 69 |
| Pedestrian Volume [ped/h] | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  |

Version 2021 (SP 0-6)
Scenario 3: 3 HY 2042 AM
Intersection Settings

| Priority Scheme | Stop | Stop | Free |
| :---: | :---: | :---: | :---: |
| Flared Lane | No | No |  |
| Storage Area [veh] | 0 | 0 | 0 |
| Two-Stage Gap Acceptance | No | No |  |
| Number of Storage Spaces in Median | 0 | 0 | 0 |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.29 | 0.08 | 0.08 | 0.01 | 0.04 | 0.04 | 0.11 | 0.04 | 0.02 | 0.11 | 0.00 | 0.00 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 43.27 | 47.65 | 32.11 | 24.14 | 19.44 | 19.46 | 20.36 | 11.27 | 7.83 | 7.81 | 0.00 | 0.00 |
| Movement LOS | E | E | D | C | C | C | C | B | A | A | A | A |
| 95th-Percentile Queue Length [veh/ln] | 2.24 | 2.24 | 2.24 | 2.24 | 0.88 | 0.88 | 0.88 | 0.88 | 0.43 | 0.43 | 0.21 | 0.00 |
| 95th-Percentile Queue Length [ft/ln] | 56.00 | 56.00 | 56.00 | 56.00 | 22.03 | 22.03 | 22.03 | 22.03 | 10.75 | 10.75 | 5.37 | 0.00 |
| d_A, Approach Delay [s/veh] | 38.67 |  |  |  | 16.12 |  |  |  | 3.45 |  |  |  |
| Approach LOS | E |  |  |  | C |  |  |  | A |  |  |  |
| d_l, Intersection Delay [s/veh] | 10.34 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection LOS | E |  |  |  |  |  |  |  |  |  |  |  |

Version 2021 (SP 0-6)
Scenario 3: 3 HY 2042 AM
Intersection Setup

| Name | Kern Street (SR-33) |  |  |  | E Kern Street |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Westbound |  |  |  | Southwestbound |  |  |  |
| Lane Configuration | $\stackrel{\rightharpoonup}{F}$ |  |  |  | YK |  |  |  |
| Turning Movement | Left | Thru | Right | Right2 | Left | Thru | Right | Right2 |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 30.00 |  |  |  | 30.00 |  |  |  |
| Grade [\%] | 0.00 |  |  |  | 0.00 |  |  |  |
| Crosswalk | No |  |  |  | No |  |  |  |

## Volumes

| Name | Kern Street (SR-33) |  |  |  | E Kern Street |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 12 | 114 | 10 | 10 | 10 | 43 | 172 | 10 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 2.00 | 2.00 | 2.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 12 | 114 | 10 | 10 | 10 | 43 | 172 | 10 |
| Peak Hour Factor | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 3 | 33 | 3 | 3 | 3 | 12 | 49 | 3 |
| Total Analysis Volume [veh/h] | 14 | 131 | 11 | 11 | 11 | 49 | 198 | 11 |
| Pedestrian Volume [ped/h] |  |  | 0 |  |  |  | 0 |  |

Version 2021 (SP 0-6)
Scenario 3: 3 HY 2042 AM
Intersection Settings

| Priority Scheme | Free | Stop |
| :---: | :---: | :---: |
| Flared Lane |  |  |
| Storage Area [veh] | 0 | 0 |
| Two-Stage Gap Acceptance |  | No |
| Number of Storage Spaces in Median | 0 | 0 |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.01 | 0.00 | 0.00 | 0.00 | 0.04 | 0.20 | 0.28 | 0.02 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 7.73 | 0.00 | 0.00 | 0.00 | 23.82 | 24.86 | 12.77 | 11.19 |
| Movement LOS | A | A | A | A | C | C | B | B |
| 95th-Percentile Queue Length [veh/ln] | 0.03 | 0.03 | 0.03 | 0.03 | 1.55 | 1.55 | 1.12 | 0.70 |
| 95th-Percentile Queue Length [ft/n] | 0.80 | 0.80 | 0.80 | 0.80 | 38.63 | 38.63 | 28.11 | 17.60 |
| d_A, Approach Delay [s/veh] | 0.65 |  |  |  | 15.36 |  |  |  |
| Approach LOS | A |  |  |  | C |  |  |  |
| d_l, Intersection Delay [s/veh] | 10.34 |  |  |  |  |  |  |  |
| Intersection LOS | E |  |  |  |  |  |  |  |




| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 1.4 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | * ${ }^{\text {r }}$ |  |  |  | ${ }^{7}$ | 44 |
| Traffic Vol, veh/h | 32 | 34 | 623 | 80 | 62 | 462 |
| Future Vol, veh/h | 32 | 34 | 623 | 80 | 62 | 462 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 50 | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 76 | 76 | 76 | 76 | 76 | 76 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 42 | 45 | 820 | 105 | 82 | 608 |


| Major/Minor M | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 1341 | 463 | 0 | 0 | 925 | 0 |
| Stage 1 | 873 | - | - | - | - | - |
| Stage 2 | 468 | - | - | - | - | - |
| Critical Hdwy | 6.84 | 6.94 | - | - | 4.14 | - |
| Critical Hdwy Stg 1 | 5.84 |  | - | - | - | - |
| Critical Hdwy Stg 2 | 5.84 | - | - | - | - | - |
| Follow-up Hdwy | 3.52 | 3.32 | - | - | 2.22 | - |
| Pot Cap-1 Maneuver | 144 | 546 | - | - | 734 | - |
| Stage 1 | 369 | - | - | - | - | - |
| Stage 2 | 597 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 128 | 546 | - | - | 734 | - |
| Mov Cap-2 Maneuver | 254 | - | - | - | - | - |
| Stage 1 | 369 | - | - | - | - | - |
| Stage 2 | 530 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 18.6 |  | 0 |  | 1.2 |  |
| HCM LOS | C |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 351 | 734 | - |
| HCM Lane V/C Ratio |  | - | - | 0.247 | 0.111 | - |
| HCM Control Delay (s) |  | - | - | 18.6 | 10.5 | - |
| HCM Lane LOS |  | - | - | C | B | - |
| HCM 95th \%tile Q(veh) |  | - | - | 1 | 0.4 | - |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.


## Intersection

Intersection Delay, s/veh42.9
Intersection LOS


| Lane | NBLn1 NBLn2 EBLn1 EBLn2WBLn1WBLn2 SBLn1 SBLn2 |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $38 \%$ | $0 \%$ | $39 \%$ | $0 \%$ | $26 \%$ | $0 \%$ | $36 \%$ | $0 \%$ |
| Vol Thru, \% | $62 \%$ | $59 \%$ | $61 \%$ | $81 \%$ | $74 \%$ | $70 \%$ | $64 \%$ | $46 \%$ |
| Vol Right, \% | $0 \%$ | $41 \%$ | $0 \%$ | $19 \%$ | $0 \%$ | $30 \%$ | $0 \%$ | $54 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 152 | 160 | 337 | 255 | 254 | 270 | 199 | 281 |
| LT Vol | 58 | 0 | 131 | 0 | 65 | 0 | 71 | 0 |
| Through Vol | 94 | 94 | 206 | 206 | 189 | 189 | 128 | 128 |
| RT Vol | 0 | 66 | 0 | 49 | 0 | 81 | 0 | 153 |
| Lane Flow Rate | 171 | 180 | 379 | 287 | 285 | 303 | 224 | 316 |
| Geometry Grp | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.486 | 0.492 | 1.023 | 0.747 | 0.765 | 0.784 | 0.605 | 0.812 |
| Departure Headway (Hd) | 10.52910 .024 | 9.728 | 9.384 | 9.823 | 9.46810 .005 | 9.419 |  |  |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 345 | 361 | 375 | 390 | 370 | 385 | 362 | 388 |
| Service Time | 8.229 | 7.724 | 7.416 | 7.072 | 7.523 | 7.168 | 7.705 | 7.119 |
| HCM Lane V/C Ratio | 0.496 | 0.499 | 1.011 | 0.736 | 0.77 | 0.787 | 0.619 | 0.814 |
| HCM Control Delay | 22.8 | 22.1 | 84.7 | 35.1 | 38.2 | 39.1 | 26.9 | 42.1 |
| HCM Lane LOS | C | C | F | E | E | E | D | E |
| HCM 95th-tile Q | 2.5 | 2.6 | 12.5 | 5.9 | 6.2 | 6.6 | 3.8 | 7.2 |

## Intersection

Intersection Delay, s/veh55.4
Intersection LOS
F

| Movement EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | * $\uparrow$ |  |  | * $\uparrow$ |  |  | \$ |  |  | * |  |
| Traffic Vol, veh/h 74 | 416 | 65 | 69 | 363 | 76 | 84 | 200 | 51 | 89 | 162 | 74 |
| Future Vol, veh/h 74 | 416 | 65 | 69 | 363 | 76 | 84 | 200 | 51 | 89 | 162 | 74 |
| Peak Hour Factor 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 |
| Heavy Vehicles, \% 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow 84 | 473 | 74 | 78 | 413 | 86 | 95 | 227 | 58 | 101 | 184 | 84 |
| Number of Lanes 0 | 2 | 0 | 0 | 2 | 0 | 0 | 1 | 0 | 0 | 1 | 0 |
| Approach EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes 2 |  |  | 2 |  |  | 1 |  |  | 1 |  |  |
| Conflicting Approach Left SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left 1 |  |  | 1 |  |  | 2 |  |  | 2 |  |  |
| Conflicting Approach RighNB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right 1 |  |  | 1 |  |  | 2 |  |  | 2 |  |  |
| HCM Control Delay 47.6 |  |  | 39 |  |  | 80 |  |  | 69.2 |  |  |
| HCM LOS E |  |  | E |  |  | F |  |  | F |  |  |


| Lane | NBLn1 EBLn1 EBLn2WBLn1WBLn2 SBLn1 |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $25 \%$ | $26 \%$ | $0 \%$ | $28 \%$ | $0 \%$ | $27 \%$ |
| Vol Thru, $\%$ | $60 \%$ | $74 \%$ | $76 \%$ | $72 \%$ | $70 \%$ | $50 \%$ |
| Vol Right, \% | $15 \%$ | $0 \%$ | $24 \%$ | $0 \%$ | $30 \%$ | $23 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 335 | 282 | 273 | 251 | 258 | 325 |
| LT Vol | 84 | 74 | 0 | 69 | 0 | 89 |
| Through Vol | 200 | 208 | 208 | 182 | 182 | 162 |
| RT Vol | 51 | 0 | 65 | 0 | 76 | 74 |
| Lane Flow Rate | 381 | 320 | 310 | 285 | 293 | 369 |
| Geometry Grp | 2 | 7 | 7 | 7 | 7 | 2 |
| Degree of Util (X) | 1.008 | 0.867 | 0.813 | 0.775 | 0.768 | 0.963 |
| Departure Headway (Hd) | 9.534 | 9.964 | 9.65 | 10.03 | 9.667 | 9.571 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 384 | 365 | 378 | 364 | 377 | 380 |
| Service Time | 7.497 | 7.664 | 7.35 | 7.73 | 7.367 | 7.571 |
| HCM Lane V/C Ratio | 0.992 | 0.877 | 0.82 | 0.783 | 0.777 | 0.971 |
| HCM Control Delay | 80 | 51.9 | 43.1 | 40 | 38 | 69.2 |
| HCM Lane LOS | F | F | E | E | E | F |
| HCM 95th-tile Q | 12.2 | 8.2 | 7.2 | 6.3 | 6.3 | 10.8 |

Synchro 11 Report

## Intersection Level Of Service Report <br> Intersection 4: Kern Street (SR-33) / 1st Street

| Control Type: | Two-way stop | Delay $(\mathrm{sec} / \mathrm{veh}):$ | 67.2 |
| :---: | :---: | :---: | :---: |
| Analysis Method: | HCM 6 th Edition | Level Of Service: | F |
| Analysis Period: | 15 minutes | Volume to Capacity $(\mathrm{v} / \mathrm{c}):$ | 0.100 |

Intersection Setup

| Name | 1st Street |  |  |  | 1st Street |  |  |  | Kern Street (SR-33) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Northbound |  |  |  | Southbound |  |  |  | Eastbound |  |  |  |
| Lane Configuration | $\stackrel{1}{4}$ |  |  |  | $\stackrel{t}{6}$ |  |  |  | $\stackrel{F}{4}$ |  |  |  |
| Turning Movement | Left2 | Left | Thru | Right | Left2 | Left | Thru | Right | Left2 | Left | Thru | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 30.00 |  |  |  | 30.00 |  |  |  | 30.00 |  |  |  |
| Grade [\%] | 0.00 |  |  |  | 0.00 |  |  |  | 0.00 |  |  |  |
| Crosswalk | No |  |  |  | No |  |  |  | No |  |  |  |

## Volumes

| Name | 1st Street |  |  |  | 1 st Street |  |  |  |  | Kern Street (SR-33) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 45 | 10 | 20 | 10 | 10 | 10 | 20 | 27 | 16 | 233 | 140 | 37 |  |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |  |
| Heavy Vehicles Percentage [\%] | 2.00 | 2.00 | 0.00 | 2.00 | 0.00 | 2.00 | 2.00 | 2.00 | 2.00 | 0.00 | 2.00 | 2.00 |  |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |  |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Total Hourly Volume [veh/h] | 45 | 10 | 20 | 10 | 10 | 10 | 20 | 27 | 16 | 233 | 140 | 37 |  |
| Peak Hour Factor | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 |  |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |  |
| Total 15-Minute Volume [veh/h] | 12 | 3 | 5 | 3 | 3 | 3 | 5 | 7 | 4 | 62 | 37 | 10 |  |
| Total Analysis Volume [veh/h] | 48 | 11 | 21 | 11 | 11 | 11 | 21 | 29 | 17 | 248 | 149 | 39 |  |
| Pedestrian Volume [ped/h] |  |  | 0 |  |  |  | 0 |  |  |  | 0 |  |  |

Version 2021 (SP 0-6)
Scenario 4: 4 HY 2042 PM
Intersection Settings

| Priority Scheme | Stop | Stop | Free |
| :---: | :---: | :---: | :---: |
| Flared Lane | No | No |  |
| Storage Area [veh] | 0 | 0 | 0 |
| Two-Stage Gap Acceptance | No | No |  |
| Number of Storage Spaces in Median | 0 | 0 | 0 |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.38 | 0.10 | 0.10 | 0.01 | 0.06 | 0.06 | 0.11 | 0.03 | 0.01 | 0.18 | 0.00 | 0.00 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 63.19 | 67.16 | 51.69 | 38.34 | 28.29 | 28.37 | 27.47 | 13.26 | 8.30 | 8.27 | 0.00 | 0.00 |
| Movement LOS | F | F | F | E | D | D | D | B | A | A | A | A |
| 95th-Percentile Queue Length [veh/ln] | 3.13 | 3.13 | 3.13 | 3.13 | 0.99 | 0.99 | 0.99 | 0.99 | 0.72 | 0.72 | 0.36 | 0.00 |
| 95th-Percentile Queue Length [ft/ln] | 78.17 | 78.17 | 78.17 | 78.17 | 24.67 | 24.67 | 24.67 | 24.67 | 18.00 | 18.00 | 9.00 | 0.00 |
| d_A, Approach Delay [s/veh] | 58.01 |  |  |  | 22.01 |  |  |  | 4.84 |  |  |  |
| Approach LOS | F |  |  |  | C |  |  |  | A |  |  |  |
| d_I, Intersection Delay [s/veh] | 11.79 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection LOS | F |  |  |  |  |  |  |  |  |  |  |  |

Version 2021 (SP 0-6)
Scenario 4: 4 HY 2042 PM
Intersection Setup

| Name | Kern Street (SR-33) |  |  |  | E Kern Street |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Westbound |  |  |  | Southwestbound |  |  |  |
| Lane Configuration | $\stackrel{\rightharpoonup}{F}$ |  |  |  | YK |  |  |  |
| Turning Movement | Left | Thru | Right | Right2 | Left | Thru | Right | Right2 |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 30.00 |  |  |  | 30.00 |  |  |  |
| Grade [\%] | 0.00 |  |  |  | 0.00 |  |  |  |
| Crosswalk | No |  |  |  | No |  |  |  |

## Volumes

| Name | Kern Street (SR-33) |  |  |  | E Kern Street |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 10 | 204 | 15 | 10 | 10 | 30 | 135 | 10 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 2.00 | 2.00 | 2.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 10 | 204 | 15 | 10 | 10 | 30 | 135 | 10 |
| Peak Hour Factor | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 3 | 54 | 4 | 3 | 3 | 8 | 36 | 3 |
| Total Analysis Volume [veh/h] | 11 | 217 | 16 | 11 | 11 | 32 | 144 | 11 |
| Pedestrian Volume [ped/h] |  |  | 0 |  |  |  | 0 |  |

Version 2021 (SP 0-6)
Scenario 4: 4 HY 2042 PM
Intersection Settings

| Priority Scheme | Free | Stop |
| :---: | :---: | :---: |
| Flared Lane |  |  |
| Storage Area [veh] | 0 | 0 |
| Two-Stage Gap Acceptance |  | No |
| Number of Storage Spaces in Median | 0 | 0 |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.01 | 0.00 | 0.00 | 0.00 | 0.06 | 0.18 | 0.23 | 0.02 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 7.62 | 0.00 | 0.00 | 0.00 | 32.24 | 31.34 | 13.71 | 11.63 |
| Movement LOS | A | A | A | A | D | D | B | B |
| 95th-Percentile Queue Length [veh/ln] | 0.02 | 0.02 | 0.02 | 0.02 | 1.43 | 1.43 | 0.99 | 0.55 |
| 95th-Percentile Queue Length [ft/n] | 0.60 | 0.60 | 0.60 | 0.60 | 35.67 | 35.67 | 24.77 | 13.86 |
| d_A, Approach Delay [s/veh] | 0.33 |  |  |  | 17.48 |  |  |  |
| Approach LOS | A |  |  |  | C |  |  |  |
| d_l, Intersection Delay [s/veh] | 11.79 |  |  |  |  |  |  |  |
| Intersection LOS | F |  |  |  |  |  |  |  |




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay，s／veh | 2.8 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{F}$ |  | 个 |  | 1 | 个个 |
| Traffic Vol，veh／h | 58 | 106 | 570 | 100 | 113 | 563 |
| Future Vol，veh／h | 58 | 106 | 570 | 100 | 113 | 563 |
| Conflicting Peds，\＃／hr | 0 | 1 | 0 | 5 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 50 | - |
| Veh in Median Storage，\＃ | 0 | - | 0 | - | - | 0 |
| Grade，\％ | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 63 | 115 | 620 | 109 | 123 | 612 |


| Major／Minor M | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 1232 | 371 | 0 | 0 | 734 | 0 |
| Stage 1 | 680 | － | － | － | － | － |
| Stage 2 | 552 | － | － | － | － | － |
| Critical Hdwy | 6.84 | 6.94 | － | － | 4.14 | － |
| Critical Hdwy Stg 1 | 5.84 | － | － | － | － | － |
| Critical Hdwy Stg 2 | 5.84 | － | － | － | － | － |
| Follow－up Hdwy | 3.52 | 3.32 | － | － | 2.22 | － |
| Pot Cap－1 Maneuver | 169 | 626 | － | － | 867 | － |
| Stage 1 | 465 | － | － | － | － | － |
| Stage 2 | 541 | － | － | － | － | － |
| Platoon blocked，\％ |  |  | － | － |  | － |
| Mov Cap－1 Maneuver | 144 | 622 | － | － | 863 | － |
| Mov Cap－2 Maneuver | 277 | － | － | － | － | － |
| Stage 1 | 463 | － | － | － | － | － |
| Stage 2 | 464 | － | － | － | － | － |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay，s | 19.1 |  | 0 |  | 1.6 |  |
| HCM LOS | C |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane／Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity（veh／h） |  | － | － | 432 | 863 | － |
| HCM Lane V／C Ratio |  | － | － | 0.413 | 0.142 | － |
| HCM Control Delay（s） |  | － | － | 19.1 | 9.9 | － |
| HCM Lane LOS |  | － | － | C | A | － |
| HCM 95th \％tile Q（veh） |  | － | － | 2 | 0.5 | － |

## APPENDIX F

TAFT AREA TRANSIT AND KERN TRANSIT ROUTES AND SCHEDULES


Taft-Maricopa Weekday Route / Ruta de Taft-Maricopa de Entre Semana

|  | $\begin{aligned} & \text { Kern St } \\ & \& 2 n d S t \end{aligned}$ | Chamber of Commerce | Taft High School | Cougar Ct \& | Fort | Albertsons Plaza | Kern St <br> \& 5th St | $\begin{aligned} & \text { Little } \\ & \text { Caesars } \end{aligned}$ | Stanislaus St |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  | Maricopa | Tejon St |  |
|  |  |  |  | Wildcat Way |  |  |  |  | Post Office | City Park | Stanislaus St |
| 1 |  |  |  |  |  |  |  |  | 7:12 AM | 7:13 AM | 7:14 AM |
| 2 | 1:34 PM | 1:35 PM | 1:36 PM | 1:39 PM | 1:41 PM | 1:43 PM | 1:44 PM | 1:45 PM | 1:55 PM | 1:56 PM | 1:57 PM |
| 3 | 5:14 PM | 5:15 PM | 5:16 PM | 5:19 PM | 5:21 PM | 5:23 PM | 5:24 PM | 5:25 PM | 5:35 PM | 5:36 PM | 5:37 PM |


| Taft-Maricopa Weekday Route (CONTINUED) / Ruta de Taft-Maricopa de Entre Semana (CONTINUADA) |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Madera St \& | Maricopa | Kern St \& | Chamber of | Taft High | Cougar Ct \& |  | Albertsons | Kern St | Little |
|  | Elkhorn St | City Hall | 2nd St | Commerce | School | Wildcat Way | Fort | Plaza | \& 5th St | Caesars |
| 1 | 7:15 AM | 7:18 AM | 7:29 AM | 7:30 AM | 7:32 AM | 7:37 AM | 7:39 AM | 7:40 AM | 7:41 AM | 7:42 AM |
| 2 | 1:58 PM | 2.01 PM | 2:12 PM | 2:13 PM | 2:15 PM | 2:20 PM | 2:22 PM | 2:23 PM | 2:24 PM | 2:25 PM |
| 3 | 5:38 PM | 5:41 PM | 5:52 PM | 5:53 PM | 5:55 PM | 6:00 PM | 6:02 PM | 6:03 PM | 6:04 PM | 6:05 PM |



TAFT-MARICOPA AREA TRANSIT
SYSTEM MAP \& SCHEDULE HORARIO Y MAPA DE SISTEMA

661.763 .3822 www.cityoftaft.org


EFFECTIVE JANUARY 1, 2019 / EFECTIVO 1 DE ENERO DE 2019

## Welcome Aboard! / ¡Bienvenido a Bordo!

Taft Area Transit (TAT) travels throughout the cities of Taft and Maricopa. For additional service information or transportation planning assistance call (661) 763-3822

Taft Area Transit (TAT) viaja a través de las ciudades de Taft y Maricopa. Para información adicional del servicio o asistencia de planificación de transporte llame al (661) 763-3822

## Passenger Guidelines

See the system map for service and area routes. For additional service information or transportation planning assistance call (661) 763-3822.

## Do

- Pay your fare (exact change only).
- Be respectful of your driver and fellow passengers.
- Reserve front seats for the elderly or disabled passengers.
- Keep hands and objects inside vehicle
- Remain seated until vehicle comes to a complete stop.
- Report any vandalism to driver


## Don't

No smoking.

- No offensive language or threatening behavior
- No littering.
- No music or excessive noise
- No food or drinks onboard.
- No marking or defacing vehicle.
- No pets/animals except assistance animals with an official tag.
Note: TAT drivers may refuse service to persons acting inappropriately or causing harm to other customers or driver. Passengers unwilling to abide by these guidelines may be barred from using the City of Taft's transportation services.


## Bikes on Bus

Getting around on Taft Area Transit just got easier. Take your bike along when you ride the bus. TAT buses feature easy-to-load bike racks. Motorized bicycles may not be loaded on the TAT bus bike racks.

## Guías de Pasajeros

Vea el mapa del sistema para las rutas y áreas de servicio. Para información adicional del servicio o asistencia de planificación de transporte llame a (661) 763-3822.

## Hacer:

- Pague su tarifa (cambio exacto solamente). - Sea respetuoso de su conductor y los demás pasajeros.
- Reserva los asientos delanteros para los
pasajeros de edad avanzada o discapacitados
- Mantenga las manos y los objetos en el
interior del vehículo
- Permanezca sentado hasta que el vehículo se haya detenido por completo.
- Reporte cualquier vandalismo al conductor.


## No Hacer:

- No fumar.
- Ningún lenguaje ofensivo o comportamiento amenazante
- No tirar basura
- No música o ruido excesivo.
- No comida o bebidas a bordo.
- Ningún marcando o desfigurando del vehículo.
- No mascotas / animales, excepto los animales de asistencia con una etiqueta oficial
Nota: Los conductores TAT pueden negar el servicio a las personas que actúan de manera inapropiada o causar daño a otros clientes o el conductor. Los pasajeros que no están dispuestos a cumplir con estas guías pueden ser excluidos de usar los servicios de transporte de la ciudad de Taft.


## Bicicletas en el Autobús

Moverse en Taft Area Transit ahora es más fácil Tome su bicicleta junto al montar el autobús. TAT autobuses disponen bastidores de bicicletas fáciles de carga. Bicicletas motorizadas no se pueden cargar en los bastidores de bicicleta de los autobuses TAT.

## Dial-A-Ride Information

TAT offers a curb-to-curb, reservation based Dial-A-Ride service that is open to the public Dial-A-Ride is limited to the City of Taft and does not travel to the City of Maricopa. Reservations must be made a minimum of two hours before you wish to ride by calling (661) 763-3822. Standing eservations may also be made ADA-certified reservations may also be made. ADA-certified atrons and seniors ( $60+$ years) may fill out an application to receive discount Dial-A-Ride pricing Persons with disabilities interested in using Dial-A-Ride must submit an application signed by a physician. A photocopy of a state-issued dentification card is also required. To request a Dial-A-Ride application call (661) 763-3822.
Please be ready when the vehicle arrives! Drivers will not wait longer than 2 minutes past the scheduled pick-up time. To schedule a ride call 2 hours prior to pickup time desired (661) 763-3822.

## Información Dial-A-Ride

TAT ofrece al público servicio de curva a curva, basado en reserva por medio de Dial-A-Ride Dial-A-Ride está limitado a la Ciudad de Taft y no viaja a la Ciudad de Maricopa. Reservaciones deben de ser hechas por lo menos dos horas antes de viajar llamando al (661) 763-3822. También se pueden hace reservaciones permanentes. Patrones certificados ADA y mayores de edad ( $60+$ años) pueden llenar una aplicación para recibir descuento. Personas con discapacidad interesados en utilizar Dial-A-Ride, deberán presentar una aplicación firmada por un médico. También se requiere una fotocopia de una tarjeta de identificación emitida por el estado. Para solicitar una aplicación llame al (661) 763-3822.

Por favor, ¡estar listo cuando llegue el vehículo! os conductores no esperaran más de 2 minutos después de la hora de recogida programada. Para programar un viaje, por favor llame 2 horas antes de la hora deseada (661) 763-3822.

## Holidays

TAT does not operate on New Year's Day, Labor Day, Independence Day, Lincoln's Birthday Washington's Birthday, Memorial Day, Veteran's Day, Thanksgiving Day, Thanksgiving Day Friday, Christmas Eve, Christmas Day, and New Year's Eve.

Dial-A-Ride Hours of Operation

## Monday - Friday

Saturday 10:15 a.m. to 2:30 p.m

Dial-A-Ride Fares
One-Way Fare
(General Public)
ADA / Senior / Youth
Children under 5
per fare paying adul
12-Trip Pass
(General Public)

ADA / Senior / Youth
\$17.50
ADA Attendant (with paid
registered ADA patron)
Note: Have exact fare. Drivers do not make change.

## Horas de Operación Dial-A-Ride

| Lunes - Viernes | 7:15 a.m. a 5:30 p.m |
| :--- | ---: |
| Sábado | 10:15 a.m. a 2:30 p.m |

Tarifas de Dial-A-Ride

Viaje Sencillo (Publico Genera)
ADA / Mayores de Edad / Jovenes \$1.75
Niños menores de 5 Gratis
años acompañados
por pasajero que paga
Pase de12 Viajes (Publico General)
ADA / Mayores de Edad / Jovenes

Asistente ADA (con pasajero
registrado ADA que paga)
Nota: Tenga tarifa exacta. Los conductores no hacen el cambio.

## Días de Fiesta

TAT no opera en el día de Año Nuevo, Día del Trabajo Día de la Independencia, el Cumpleaños de Lincoln, el Cumpleaños de Washington, Día Conmemoratívo, Dia de los Veteranos, Día de Acción de Gracias, el Viernes Después de Día de Acción de Gracias, Víspera de Navidad, el día de Navidad y Fin de Año

| DAYS: | DIRECTIONS: |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Weekday Saturday | Easthound West |  |  |  |
| STOP | 175237 | 175084 | 175248 | 175238 |
| Taft |  |  |  |  |
| Taft College -6th St. at Cougar Ct. (WB) | 7:40am | 10:00am | 2:30pm | 7:35pm |
| 8th St. at North St. (Heritage Park Senior Complex) | 7:44am | 10:04am | 2:34pm | 7:39pm |
| Taft Transit Center | 7:48am | 10:08am | $2: 38 \mathrm{pm}$ | 7:43pm |
| Harrison St. at Cedar St. | 7:54am | 10:14am | 2:44pm | 7:49pm |
| Valley Acres (East) | 8:01am | 10:21am | 2:51pm | 7:56pm |
| Dustin Acres East |  |  |  |  |
| Bakersfield |  |  |  |  |
| Stockdale Hwy. at Buena Vista Rd. | 8:31am | 10:51am | 3:21pm | 8:26pm |
| Cal State University Bakersfield | 8:37am | 10:57am | 3:27pm | 8:32pm |
| Stockdale Hwy. at New Stine Rd. (7/11) |  |  |  |  |
| Brundage Ln. at A St. (East) |  |  |  |  |
| Kern County Superior Court - Truxtun Ave. |  |  |  |  |
| Bakersfield Amtrak | 8:59am | 11:19am | $3: 49 \mathrm{pm}$ | 8:54pm |
| Greyhound/18th St. | 9:04am | 11:24am | $3: 54 \mathrm{pm}$ | 8:59pm |
| GET Downtown Transit Center | 9:07am | 11:27am | 3.57 pm | 9:02pm |

## Schedules

Monday through Saturday



## Schedules

| DAYS: |  | DIRECTIONS: |  |
| :---: | :--- | :---: | :--- |
| Weekday | Saturday | Eastbound | Westbound |


| STOP | 175139 | 175130 |
| :---: | :---: | :---: |
| Bakersfield |  |  |
| Bakersfield Amtrak | 10:27am | 3:42pm |
| Greyhound/18th St. | 10:32am | 3:47pm |
| GET Downtown Transit Center | 10:35am | 3.50pm |
| Chester Ave. at Truxtun Ave. - City Hall |  |  |
| Brundage Ln. at A St. |  |  |
| Stockdale Hwy. at New Stine Rd. |  |  |
| Cal State University Bakersfield | 10:57am | 4:12pm |
| Stockdale Hwy. at Buena Vista Rd. | 11:02am | 4:17pm |
| Taft |  |  |
| Dustin Acres West |  |  |
| Valley Acres (West) | 11:30am | 4:45pm |
| Harrison St. at Cedar St. (WB) | 11:40am | 4:55pm |
| Taft College -6th St. at Cougar Ct. (WB) | 11:42am | 4:57pm |
| 8th St. at North St. (Heritage Park Senior Complex) | 11:46am | 5:01pm |
| Taft Transit Center | 11:49am | 5:04pm |

## Route Map



## Route Map



## APPENDIX G

## CA MUTCD PEAK HOUR WARRANT (WARRANT 3) WORKSHEETS

## SIGNAL WARRANT ANALYSIS

| Minor Street: | Kern Street | Number of Lanes on minor street: | 1 |
| :--- | :--- | :--- | ---: |
| Major Street | 4th Street | Number of Lanes on major street: | 2 |
| Scenario: | Horizon Year 2042 | Total Number of Vehicles Entering: | 1731 |
|  | AM Peak | Number of Approaches: | 4 |
|  |  | Intersection total delay (seconds): | 118.8 |
|  |  | Minor St approach total delay (veh-hr): | 14.1 |

## Part A

|  | Criteria Met? |
| :---: | :---: |
|  | YES |
|  | YES |

## Part B

| Volume on Major St ( $\Sigma$ of both Approaches) | Volumes on Minor St (higher approach) |
| :---: | :---: |
| 911 | 428 |

Figure 4C-3. Warrant 3, Peak Hour

*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Does the Intersection Meet Peak Hour Signal Warrants?:

| PART A |  |
| :--- | :---: |
| PART B | YES |

## SIGNAL WARRANT ANALYSIS

| Minor Street: | Kern Street | Number of Lanes on minor street: | 1 |
| :--- | :--- | :--- | ---: |
| Major Street | 4th Street | Number of Lanes on major street: | 2 |
| Scenario: | Horizon Year 2042 | Total Number of Vehicles Entering: | 1723 |
|  | PM Peak | Number of Approaches: | 4 |
|  |  | Intersection total delay (seconds): | 55.4 |
|  |  | Minor St approach total delay (veh-hr): | 5.2 |

## Part A

|  | Criteria Met? |
| :---: | :---: |
|  | YES |
|  | YES |
|  | YES |

## Part B

| Volume on Major St ( $\Sigma$ of both Approaches) | Volumes on Minor St (higher approach) |
| :---: | :---: |
| 1063 | 335 |

Figure 4C-3. Warrant 3, Peak Hour

*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Does the Intersection Meet Peak Hour Signal Warrants?:

| PART A | YES |
| :--- | :--- |
| PART B | YES |

## SIGNAL WARRANT ANALYSIS

| Minor Street: | Kern Street | Number of Lanes on minor street: | 2 |
| :--- | :--- | :--- | ---: |
| Major Street | 6th Street | Number of Lanes on major street: | 2 |
| Scenario: | Horizon Year 2042 | Total Number of Vehicles Entering: | 1995 |
|  | AM Peak | Number of Approaches: | 4 |
|  |  | Intersection total delay (seconds): | 117.7 |
|  |  | Minor St approach total delay (veh-hr): | 16.1 |

## Part A

|  | Criteria Met? |
| :---: | :---: |
|  | YES |
|  | YES |
|  | YES |

## Part B

| Volume on Major St ( $\Sigma$ of both Approaches) | Volumes on Minor St (higher approach) |
| :---: | :---: |
| 1090 | 491 |

Figure 4C-3. Warrant 3, Peak Hour

*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Does the Intersection Meet Peak Hour Signal Warrants?:

| PART A |  |
| :--- | :---: |
| PART B | YES |

## SIGNAL WARRANT ANALYSIS

| Minor Street: | Kern Street | Number of Lanes on minor street: | 2 |
| :--- | :--- | :--- | ---: |
| Major Street | 6th Street | Number of Lanes on major street: | 2 |
| Scenario: | Horizon Year 2042 | Total Number of Vehicles Entering: | 1907 |
|  | PM Peak | Number of Approaches: | 4 |
|  |  | Intersection total delay (seconds): | 42.9 |
|  |  | Minor St approach total delay (veh-hr): | 5.7 |

## Part A

|  | Criteria Met? |
| :---: | :---: |
|  | YES |
|  | YES |
|  | YES |

## Part B

| Volume on Major St ( $\Sigma$ of both Approaches) | Volumes on Minor St (higher approach) |
| :---: | :---: |
| 1115 | 480 |

Figure 4C-3. Warrant 3, Peak Hour

*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

| Does the Intersection Meet Peak Hour Signal Warrants?: |  |
| :--- | :---: |
| PART A | YES |
| PART B | YES |

# APPENDIX H 

## HORIZON YEAR 2042 WITH PROJECT INTERSECTION OPERATIONS WITH RECOMMENDED IMPROVEMENTS ANALYSIS WORKSHEETS

|  | 4 | $\rightarrow$ |  | 7 |  | 4 | 4 | $\dagger$ | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 中 ${ }^{\text {a }}$ |  | ${ }^{7}$ | 中 ${ }^{\text {a }}$ |  | ${ }^{1}$ | 4 | 「 | ${ }^{1}$ | 4 | 「 |
| Traffic Volume（veh／h） | 133 | 382 | 40 | 47 | 412 | 76 | 57 | 296 | 61 | 84 | 260 | 147 |
| Future Volume（veh／h） | 133 | 382 | 40 | 47 | 412 | 76 | 57 | 296 | 61 | 84 | 260 | 147 |
| Initial Q（Qb），veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped－Bike Adj（A＿pbT） | 1.00 |  | 0.99 | 1.00 |  | 0.99 | 1.00 |  | 0.97 | 1.00 |  | 0.98 |
| Parking Bus，Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow，veh／h／ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate，veh／h | 182 | 523 | 55 | 64 | 564 | 104 | 78 | 405 | 84 | 115 | 356 | 201 |
| Peak Hour Factor | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 |
| Percent Heavy Veh，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap，veh／h | 212 | 924 | 97 | 129 | 714 | 131 | 139 | 530 | 434 | 154 | 545 | 453 |
| Arrive On Green | 0.12 | 0.28 | 0.28 | 0.07 | 0.24 | 0.24 | 0.08 | 0.28 | 0.28 | 0.09 | 0.29 | 0.29 |
| Sat Flow，veh／h | 1781 | 3244 | 340 | 1781 | 2994 | 550 | 1781 | 1870 | 1534 | 1781 | 1870 | 1554 |
| Grp Volume（v），veh／h | 182 | 286 | 292 | 64 | 334 | 334 | 78 | 405 | 84 | 115 | 356 | 201 |
| Grp Sat Flow（s），veh／h／ln | 1781 | 1777 | 1807 | 1781 | 1777 | 1767 | 1781 | 1870 | 1534 | 1781 | 1870 | 1554 |
| Q Serve（g＿s），s | 8.7 | 11.9 | 11.9 | 3.0 | 15.3 | 15.4 | 3.7 | 17.1 | 3.6 | 5.5 | 14.4 | 9.1 |
| Cycle Q Clear（g＿c），s | 8.7 | 11.9 | 11.9 | 3.0 | 15.3 | 15.4 | 3.7 | 17.1 | 3.6 | 5.5 | 14.4 | 9.1 |
| Prop In Lane | 1.00 |  | 0.19 | 1.00 |  | 0.31 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Lane Grp Cap（c），veh／h | 212 | 506 | 515 | 129 | 424 | 422 | 139 | 530 | 434 | 154 | 545 | 453 |
| V／C Ratio（X） | 0.86 | 0.56 | 0.57 | 0.50 | 0.79 | 0.79 | 0.56 | 0.76 | 0.19 | 0.75 | 0.65 | 0.44 |
| Avail Cap（c＿a），veh／h | 212 | 577 | 587 | 189 | 554 | 551 | 165 | 672 | 551 | 165 | 672 | 558 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter（I） | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay（d），s／veh | 37.4 | 26.4 | 26.4 | 38.6 | 30.9 | 30.9 | 38.5 | 28.4 | 23.5 | 38.6 | 26.8 | 25.0 |
| Incr Delay（d2），s／veh | 27.9 | 1.0 | 1.0 | 2.9 | 5.6 | 5.9 | 3.5 | 4.0 | 0.2 | 15.8 | 1.6 | 0.7 |
| Initial Q Delay（d3），s／veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \％ile BackOfQ（50\％），veh／ln | 5.3 | 4.9 | 5.1 | 1.4 | 6.9 | 6.9 | 1.7 | 7.9 | 1.3 | 3.0 | 6.4 | 3.3 |
| Unsig．Movement Delay，s／veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay（d），s／veh | 65.4 | 27.4 | 27.4 | 41.5 | 36.5 | 36.8 | 41.9 | 32.4 | 23.7 | 54.5 | 28.4 | 25.6 |
| LnGrp LOS | E | C | C | D | D | D | D | C | C | D | C | C |
| Approach Vol，veh／h |  | 760 |  |  | 732 |  |  | 567 |  |  | 672 |  |
| Approach Delay，s／veh |  | 36.5 |  |  | 37.1 |  |  | 32.4 |  |  | 32.1 |  |
| Approach LOS |  | D |  |  | D |  |  | C |  |  | C |  |
| Timer－Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |  |  |  |  |
| Phs Duration（ $G+Y+R c$ ），$s$ | 12.2 | 30.9 | 12.0 | 31.5 | 11.5 | 31.6 | 16.0 | 27.5 |  |  |  |  |
| Change Period（Y＋Rc），s | ＊ 4.7 | 6.4 | ＊ 5.7 | 6.8 | ＊ 4.7 | 6.4 | ＊ 5.7 | 6.8 |  |  |  |  |
| Max Green Setting（Gmax），s | ＊ 8 | 31.1 | ＊ 9.2 | 28.1 | ＊ 8 | 31.1 | ＊ 10 | 27.0 |  |  |  |  |
| Max Q Clear Time（g＿c＋l1），s | 7.5 | 19.1 | 5.0 | 13.9 | 5.7 | 16.4 | 10.7 | 17.4 |  |  |  |  |
| Green Ext Time（p＿c），s | 0.0 | 2.1 | 0.0 | 2.9 | 0.0 | 2.4 | 0.0 | 2.8 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Ctrl Delay |  |  | 34.7 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | C |  |  |  |  |  |  |  |  |  |
| Notes |  |  |  |  |  |  |  |  |  |  |  |  |

＊HCM 6th computational engine requires equal clearance times for the phases crossing the barrier．

|  | 4 | $\rightarrow$ | $\downarrow$ | 7 | - | 4 | 4 | $\dagger$ | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 性 |  | \% | 中t |  | \% | $\uparrow$ |  | ${ }^{7}$ | $\hat{\beta}$ |  |
| Traffic Volume (veh/h) | 92 | 318 | 54 | 47 | 345 | 55 | 91 | 248 | 53 | 81 | 263 | 84 |
| Future Volume (veh/h) | 92 | 318 | 54 | 47 | 345 | 55 | 91 | 248 | 53 | 81 | 263 | 84 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 |  | 0.99 | 1.00 |  | 0.97 | 1.00 |  | 0.99 | 1.00 |  | 0.99 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 116 | 403 | 68 | 59 | 437 | 70 | 115 | 314 | 67 | 103 | 333 | 106 |
| Peak Hour Factor | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 |
| Percent Heavy Veh, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 169 | 639 | 107 | 132 | 580 | 92 | 168 | 457 | 98 | 164 | 412 | 131 |
| Arrive On Green | 0.09 | 0.21 | 0.21 | 0.07 | 0.19 | 0.19 | 0.09 | 0.31 | 0.31 | 0.09 | 0.30 | 0.30 |
| Sat Flow, veh/h | 1781 | 3042 | 509 | 1781 | 3056 | 486 | 1781 | 1491 | 318 | 1781 | 1356 | 432 |
| Grp Volume(v), veh/h | 116 | 234 | 237 | 59 | 253 | 254 | 115 | 0 | 381 | 103 | 0 | 439 |
| Grp Sat Flow(s),veh/h/ln | 1781 | 1777 | 1774 | 1781 | 1777 | 1765 | 1781 | 0 | 1810 | 1781 | 0 | 1788 |
| Q Serve(g_s), s | 4.9 | 9.3 | 9.4 | 2.5 | 10.4 | 10.6 | 4.8 | 0.0 | 14.3 | 4.3 | 0.0 | 17.6 |
| Cycle Q Clear (g_c), s | 4.9 | 9.3 | 9.4 | 2.5 | 10.4 | 10.6 | 4.8 | 0.0 | 14.3 | 4.3 | 0.0 | 17.6 |
| Prop In Lane | 1.00 |  | 0.29 | 1.00 |  | 0.28 | 1.00 |  | 0.18 | 1.00 |  | 0.24 |
| Lane Grp Cap(c), veh/h | 169 | 373 | 373 | 132 | 337 | 335 | 168 | 0 | 555 | 164 | 0 | 543 |
| V/C Ratio(X) | 0.69 | 0.63 | 0.64 | 0.45 | 0.75 | 0.76 | 0.68 | 0.00 | 0.69 | 0.63 | 0.00 | 0.81 |
| Avail Cap(c_a), veh/h | 237 | 495 | 494 | 204 | 463 | 460 | 237 | 0 | 831 | 214 | 0 | 798 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(l) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 1.00 |
| Uniform Delay (d), s/veh | 34.0 | 27.9 | 27.9 | 34.4 | 29.7 | 29.7 | 34.0 | 0.0 | 23.6 | 33.9 | 0.0 | 24.9 |
| Incr Delay (d2), s/veh | 4.9 | 1.7 | 1.8 | 2.3 | 4.4 | 4.9 | 4.8 | 0.0 | 1.5 | 3.9 | 0.0 | 3.9 |
| Initial Q Delay (d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile BackOfQ(50\%),veh/ln | 2.3 | 3.9 | 4.0 | 1.1 | 4.6 | 4.7 | 2.3 | 0.0 | 6.2 | 2.0 | 0.0 | 7.8 |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay(d),s/veh | 38.9 | 29.6 | 29.7 | 36.7 | 34.1 | 34.6 | 38.8 | 0.0 | 25.1 | 37.9 | 0.0 | 28.8 |
| LnGrp LOS | D | C | C | D | C | C | D | A | C | D | A | C |
| Approach Vol, veh/h |  | 587 |  |  | 566 |  |  | 496 |  |  | 542 |  |
| Approach Delay, s/veh |  | 31.5 |  |  | 34.6 |  |  | 28.3 |  |  | 30.5 |  |
| Approach LOS |  | C |  |  | C |  |  | C |  |  | C |  |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |  |  |  |  |
| Phs Duration ( $\mathrm{G}+\mathrm{Y}+\mathrm{Rc}$ ), s | 12.8 | 30.2 | 11.5 | 23.1 | 13.0 | 30.0 | 13.0 | 21.5 |  |  |  |  |
| Change Period ( $\mathrm{Y}+\mathrm{Rc}$ ), s | *5.7 | 6.4 | * 5.7 | 6.8 | * 5.7 | 6.4 | * 5.7 | 6.8 |  |  |  |  |
| Max Green Setting (Gmax), s | *9.3 | 35.6 | *8.9 | 21.6 | *10 | 34.6 | *10 | 20.2 |  |  |  |  |
| Max Q Clear Time (g_c+11), s | 6.3 | 16.3 | 4.5 | 11.4 | 6.8 | 19.6 | 6.9 | 12.6 |  |  |  |  |
| Green Ext Time (p_c), s | 0.1 | 2.4 | 0.0 | 1.9 | 0.1 | 2.6 | 0.1 | 1.8 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Ctrl Delay |  |  | 31.3 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | C |  |  |  |  |  |  |  |  |  |
| Notes |  |  |  |  |  |  |  |  |  |  |  |  |

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.


## Intersection Level Of Service Report

Intersection 4: Kern Street (SR-33) / 1st Street

| Control Type: | All-way stop | Delay $(\mathrm{sec} / \mathrm{veh}):$ | 12.3 |
| :---: | :---: | :---: | :---: |
| Analysis Method: | HCM 6 th Edition | Level Of Service: | B |
| Analysis Period: | 15 minutes | Volume to Capacity $(\mathrm{v} / \mathrm{c}):$ | 0.399 |

Intersection Setup

| Name | 1st Street |  |  |  | 1st Street |  |  |  | Kern Street (SR-33) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Northbound |  |  |  | Southbound |  |  |  | Eastbound |  |  |  |
| Lane Configuration | $\stackrel{1}{4}$ |  |  |  | $\Rightarrow$ |  |  |  | $4$ |  |  |  |
| Turning Movement | Left2 | Left | Thru | Right | Left2 | Left | Thru | Right | Left2 | Left | Thru | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 30.00 |  |  |  | 30.00 |  |  |  | 30.00 |  |  |  |
| Grade [\%] | 0.00 |  |  |  | 0.00 |  |  |  | 0.00 |  |  |  |
| Crosswalk | No |  |  |  | No |  |  |  | No |  |  |  |

## Volumes

| Name | 1st Street |  |  |  | 1st Street |  |  |  | Kern Street (SR-33) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 40 | 10 | 20 | 10 | 10 | 10 | 28 | 37 | 24 | 136 | 143 | 60 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 2.00 | 2.00 | 0.00 | 2.00 | 0.00 | 2.00 | 2.00 | 2.00 | 2.00 | 0.00 | 2.00 | 2.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 40 | 10 | 20 | 10 | 10 | 10 | 28 | 37 | 24 | 136 | 143 | 60 |
| Peak Hour Factor | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 11 | 3 | 6 | 3 | 3 | 3 | 8 | 11 | 7 | 39 | 41 | 17 |
| Total Analysis Volume [veh/h] | 46 | 11 | 23 | 11 | 11 | 11 | 32 | 43 | 28 | 156 | 164 | 69 |
| Pedestrian Volume [ped/h] | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  |

Version 2021 (SP 0-6)
Intersection Settings
Lanes

| Capacity per Entry Lane [veh/h] | 496 | 550 | 523 | 579 |
| :---: | :---: | :---: | :---: | :---: |
| Degree of Utilization, x | 0.18 | 0.18 | 0.40 | 0.36 |

Movement, Approach, \& Intersection Results

| 95th-Percentile Queue Length [veh] | 0.67 | 0.64 | 1.90 | 1.63 |
| :---: | :---: | :---: | :---: | :---: |
| 95th-Percentile Queue Length [ft] | 16.63 | 15.88 | 47.43 | 40.84 |
| Approach Delay [s/veh] | 11.88 | 10.94 | 13.23 |  |
| Approach LOS | B | B | B |  |
| Intersection Delay [s/veh] | 12.29 | B |  |  |
| Intersection LOS |  |  |  |  |

Version 2021 (SP 0-6)
Scenario 5: 5 HY 2042 AM - MITI
Intersection Setup

| Name | Kern Street (SR-33) |  |  |  | E Kern Street |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Westbound |  |  |  | Southwestbound |  |  |  |
| Lane Configuration | $\stackrel{\rightharpoonup}{F}$ |  |  |  | YK |  |  |  |
| Turning Movement | Left | Thru | Right | Right2 | Left | Thru | Right | Right2 |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 30.00 |  |  |  | 30.00 |  |  |  |
| Grade [\%] | 0.00 |  |  |  | 0.00 |  |  |  |
| Crosswalk | No |  |  |  | No |  |  |  |

## Volumes

| Name | Kern Street (SR-33) |  |  |  | E Kern Street |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 12 | 114 | 10 | 10 | 10 | 43 | 172 | 10 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 2.00 | 2.00 | 2.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 12 | 114 | 10 | 10 | 10 | 43 | 172 | 10 |
| Peak Hour Factor | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 | 0.8700 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 3 | 33 | 3 | 3 | 3 | 12 | 49 | 3 |
| Total Analysis Volume [veh/h] | 14 | 131 | 11 | 11 | 11 | 49 | 198 | 11 |
| Pedestrian Volume [ped/h] |  |  | 0 |  |  |  | 0 |  |

Version 2021 (SP 0-6)
Intersection Settings
Lanes

| Capacity per Entry Lane [veh/h] | 524 | 554 | 586 |
| :---: | :---: | :---: | :---: |
| Degree of Utilization, $x$ | 0.32 | 0.24 | 0.23 |

Movement, Approach, \& Intersection Results

| 95th-Percentile Queue Length [veh] | 1.36 | 0.94 | 0.88 |
| :---: | :---: | :---: | :---: |
| 95 th-Percentile Queue Length [ft] | 34.11 | 23.60 | 21.99 |
| Approach Delay [s/veh] | 13.07 |  | 10.96 |
| Approach LOS | B | B |  |
| Intersection Delay [s/veh] |  | 12.29 |  |
| Intersection LOS | B |  |  |


|  | 4 | $\rightarrow$ | \％ | 6 |  | 4 | $4$ | $\dagger$ | \％ |  | $\dagger$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{1}$ | 㻢 |  | ${ }^{1}$ | 㻢 |  | ${ }^{7}$ | 4 | 「 | ${ }^{1 /}$ | 4 | 「 |
| Traffic Volume（veh／h） | 131 | 412 | 49 | 65 | 377 | 81 | 58 | 188 | 66 | 71 | 256 | 153 |
| Future Volume（veh／h） | 131 | 412 | 49 | 65 | 377 | 81 | 58 | 188 | 66 | 71 | 256 | 153 |
| Initial Q（Qb），veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped－Bike Adj（A＿pbT） | 1.00 |  | 0.99 | 1.00 |  | 0.99 | 1.00 |  | 0.98 | 1.00 |  | 0.99 |
| Parking Bus，Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow，veh／h／ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate，veh／h | 147 | 463 | 55 | 73 | 424 | 91 | 65 | 211 | 74 | 80 | 288 | 172 |
| Peak Hour Factor | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 |
| Percent Heavy Veh，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap，veh／h | 198 | 763 | 90 | 158 | 628 | 134 | 149 | 430 | 356 | 164 | 446 | 374 |
| Arrive On Green | 0.11 | 0.24 | 0.24 | 0.09 | 0.22 | 0.22 | 0.08 | 0.23 | 0.23 | 0.09 | 0.24 | 0.24 |
| Sat Flow，veh／h | 1781 | 3198 | 378 | 1781 | 2911 | 620 | 1781 | 1870 | 1546 | 1781 | 1870 | 1567 |
| Grp Volume（v），veh／h | 147 | 256 | 262 | 73 | 257 | 258 | 65 | 211 | 74 | 80 | 288 | 172 |
| Grp Sat Flow（s），veh／h／ln | 1781 | 1777 | 1799 | 1781 | 1777 | 1754 | 1781 | 1870 | 1546 | 1781 | 1870 | 1567 |
| Q Serve（g＿s），s | 5.4 | 8.6 | 8.7 | 2.6 | 8.9 | 9.1 | 2.3 | 6.6 | 2.6 | 2.9 | 9.3 | 6.3 |
| Cycle Q Clear（g＿c），s | 5.4 | 8.6 | 8.7 | 2.6 | 8.9 | 9.1 | 2.3 | 6.6 | 2.6 | 2.9 | 9.3 | 6.3 |
| Prop In Lane | 1.00 |  | 0.21 | 1.00 |  | 0.35 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Lane Grp Cap（c），veh／h | 198 | 424 | 429 | 158 | 383 | 378 | 149 | 430 | 356 | 164 | 446 | 374 |
| V／C Ratio（X） | 0.74 | 0.60 | 0.61 | 0.46 | 0.67 | 0.68 | 0.44 | 0.49 | 0.21 | 0.49 | 0.65 | 0.46 |
| Avail Cap（c＿a），veh／h | 273 | 729 | 738 | 257 | 713 | 704 | 212 | 864 | 715 | 212 | 864 | 724 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter（I） | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay（d），s／veh | 29.0 | 22.8 | 22.8 | 29.1 | 24.2 | 24.3 | 29.3 | 22.5 | 20.9 | 29.0 | 23.1 | 21.9 |
| Incr Delay（d2），s／veh | 6.8 | 1.4 | 1.4 | 2.1 | 2.0 | 2.2 | 2.0 | 0.9 | 0.3 | 2.2 | 1.6 | 0.9 |
| Initial Q Delay（d3），s／veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \％ile BackOfQ（50\％），veh／ln | 2.5 | 3.5 | 3.6 | 1.2 | 3.7 | 3.7 | 1.0 | 2.8 | 0.9 | 1.3 | 4.0 | 2.2 |
| Unsig．Movement Delay，s／veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay（d），s／veh | 35.8 | 24.2 | 24.2 | 31.3 | 26.2 | 26.4 | 31.3 | 23.3 | 21.2 | 31.3 | 24.6 | 22.8 |
| LnGrp LOS | D | C | C | C | C | C | C | C | C | C | C | C |
| Approach Vol，veh／h |  | 665 |  |  | 588 |  |  | 350 |  |  | 540 |  |
| Approach Delay，s／veh |  | 26.8 |  |  | 26.9 |  |  | 24.4 |  |  | 25.0 |  |
| Approach LOS |  | C |  |  | C |  |  | C |  |  | C |  |
| Timer－Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |  |  |  |  |
| Phs Duration（ $G+Y+R c$ ），$s$ | 10.9 | 21.9 | 11.7 | 22.8 | 10.3 | 22.5 | 13.2 | 21.3 |  |  |  |  |
| Change Period（Y＋Rc），s | ＊ 4.7 | 6.4 | ＊ 5.7 | 6.8 | ＊ 4.7 | 6.4 | ＊ 5.7 | 6.8 |  |  |  |  |
| Max Green Setting（Gmax），s | ＊ 8 | 31.1 | ＊ 9.7 | 27.6 | ＊ 8 | 31.1 | ＊ 10 | 27.0 |  |  |  |  |
| Max Q Clear Time（g＿c＋l1），s | 4.9 | 8.6 | 4.6 | 10.7 | 4.3 | 11.3 | 7.4 | 11.1 |  |  |  |  |
| Green Ext Time（p＿c），s | 0.0 | 1.4 | 0.1 | 2.8 | 0.0 | 2.1 | 0.1 | 2.7 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Ctrl Delay |  |  | 26.0 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | C |  |  |  |  |  |  |  |  |  |
| Notes |  |  |  |  |  |  |  |  |  |  |  |  |

＊HCM 6th computational engine requires equal clearance times for the phases crossing the barrier．

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.


## Intersection Level Of Service Report

Intersection 4: Kern Street (SR-33) / 1st Street

| Control Type: | All-way stop | Delay $(\mathrm{sec} / \mathrm{veh}):$ | 13.9 |
| :---: | :---: | :---: | :---: |
| Analysis Method: | HCM 6th Edition | Level Of Service: | B |
| Analysis Period: | 15 minutes | Volume to Capacity $(\mathrm{v} / \mathrm{c}):$ | 0.515 |

Intersection Setup

| Name | 1st Street |  |  |  | 1st Street |  |  |  | Kern Street (SR-33) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Northbound |  |  |  | Southbound |  |  |  | Eastbound |  |  |  |
| Lane Configuration | $\stackrel{1}{4}$ |  |  |  | $\Rightarrow$ |  |  |  | $4$ |  |  |  |
| Turning Movement | Left2 | Left | Thru | Right | Left2 | Left | Thru | Right | Left2 | Left | Thru | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 30.00 |  |  |  | 30.00 |  |  |  | 30.00 |  |  |  |
| Grade [\%] | 0.00 |  |  |  | 0.00 |  |  |  | 0.00 |  |  |  |
| Crosswalk | No |  |  |  | No |  |  |  | No |  |  |  |

## Volumes

| Name | 1st Street |  |  |  | 1st Street |  |  |  | Kern Street (SR-33) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 45 | 10 | 20 | 10 | 10 | 10 | 20 | 27 | 16 | 233 | 140 | 37 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 2.00 | 2.00 | 0.00 | 2.00 | 0.00 | 2.00 | 2.00 | 2.00 | 2.00 | 0.00 | 2.00 | 2.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 45 | 10 | 20 | 10 | 10 | 10 | 20 | 27 | 16 | 233 | 140 | 37 |
| Peak Hour Factor | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 12 | 3 | 5 | 3 | 3 | 3 | 5 | 7 | 4 | 62 | 37 | 10 |
| Total Analysis Volume [veh/h] | 48 | 11 | 21 | 11 | 11 | 11 | 21 | 29 | 17 | 248 | 149 | 39 |
| Pedestrian Volume [ped/h] | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  |

Version 2021 (SP 0-6)
Intersection Settings
Lanes

| Capacity per Entry Lane [veh/h] | 482 | 532 | 515 | 566 |
| :---: | :---: | :---: | :---: | :---: |
| Degree of Utilization, $x$ | 0.19 | 0.14 | 0.51 | 0.33 |

Movement, Approach, \& Intersection Results

| 95th-Percentile Queue Length [veh] | 0.69 | 0.47 | 2.91 | 1.45 |
| :---: | :---: | :---: | :---: | :---: |
| 95th-Percentile Queue Length [ft] | 17.23 | 11.64 | 72.78 | 36.23 |
| Approach Delay [s/veh] | 12.21 | 10.82 | 14.94 |  |
| Approach LOS | B | B | B |  |
| Intersection Delay [s/veh] |  |  |  |  |
| Intersection LOS | B |  |  |  |

Version 2021 (SP 0-6)
Scenario 6: 6 HY 2042 PM - MITI
Intersection Setup

| Name | Kern Street (SR-33) |  |  |  | E Kern Street |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Westbound |  |  |  | Southwestbound |  |  |  |
| Lane Configuration | $\stackrel{\rightharpoonup}{F}$ |  |  |  | YK |  |  |  |
| Turning Movement | Left | Thru | Right | Right2 | Left | Thru | Right | Right2 |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 30.00 |  |  |  | 30.00 |  |  |  |
| Grade [\%] | 0.00 |  |  |  | 0.00 |  |  |  |
| Crosswalk | No |  |  |  | No |  |  |  |

## Volumes

| Name | Kern Street (SR-33) |  |  |  | E Kern Street |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 10 | 204 | 15 | 10 | 10 | 30 | 135 | 10 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 2.00 | 2.00 | 2.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 10 | 204 | 15 | 10 | 10 | 30 | 135 | 10 |
| Peak Hour Factor | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 | 0.9400 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 3 | 54 | 4 | 3 | 3 | 8 | 36 | 3 |
| Total Analysis Volume [veh/h] | 11 | 217 | 16 | 11 | 11 | 32 | 144 | 11 |
| Pedestrian Volume [ped/h] |  |  | 0 |  |  |  | 0 |  |

Version 2021 (SP 0-6)
Scenario 6: 6 HY 2042 PM - MITI
Intersection Settings
Lanes

| Capacity per Entry Lane [veh/h] | 530 | 524 | 553 |
| :---: | :---: | :---: | :---: |
| Degree of Utilization, x | 0.48 | 0.19 | 0.18 |

Movement, Approach, \& Intersection Results

| 95th-Percentile Queue Length [veh] | 2.59 | 0.69 | 0.65 |
| :---: | :---: | :---: | :---: |
| 95th-Percentile Queue Length [ft] | 64.79 | 17.25 | 16.17 |
| Approach Delay [s/veh] | 15.95 | 10.90 |  |
| Approach LOS | C | B |  |
| Intersection Delay [s/veh] |  | 13.92 |  |
| Intersection LOS | B |  |  |


[^0]:    Footnotes: S = Signalized; AWSC = All-Way-Stop Controlled; TWSC = Two-Way-Stop Controlled; OWSC = One-Way-Stop Controlled
    ${ }^{1}$ Delay is measured in seconds per vehicle.
    ${ }^{2}$ PTV Vistro software program was used as interface to HCM-6 methodology to analyze the 5-legged Kern Street (SR-33) / E. Kern Street / 1st Street because the Synchro 11 software program does not accept analysis of 5-legged intersections.

[^1]:    ${ }^{2}$ Person-Trips
    ${ }^{3}$ Total estimate for all other land uses at mixed-use development site-not subject to internal trip capture computations in this estimator
    *Indicates computation that has been rounded to the nearest whole number.

