DOWNTOWN TAFT SPECIFIC PLAN LOCAL TRANSPORTATION ANALYSIS

Draft: May 27, 2022

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INTRODUCTION

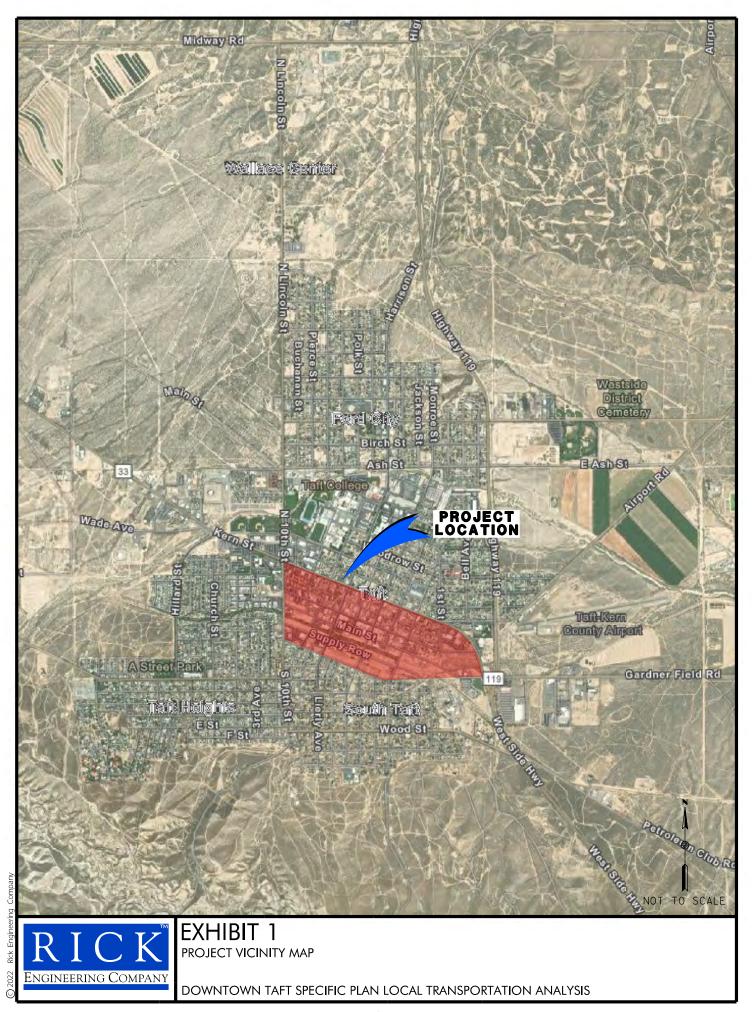
The following Local Transportation Analysis has been prepared for the Downtown Taft Specific Plan project in accordance with the Kern County *Standards for Traffic Engineering* (February 23, 2010) and the City of Taft *General Plan Circulation Element* (June 2010, Amended April 2017). The study evaluates the potential level of service (LOS) deficiencies and transportation improvements that may need to be considered in association with the traffic generated by the proposed Downtown Taft Specific Plan project. As also required, an Active Transportation and Public Transit Assessment is evaluated in this study. The site is generally bounded by the mid-block alley between Kern Street (State Route 33) and Lucard Street to the north, Front Street to the south, 10th Street to the west, and State Route 33 (SR-33/Westside Highway) to the east. **Exhibit 1** shows the project vicinity map.

PROJECT DESCRIPTION

The Downtown Taft Specific Plan covers approximately 212 acres and would allow for the development of up to 3,120 residential dwelling units (DU), and would expand the downtown district to develop up to 891,059 square-feet (SF) of commercial retail uses and up to 1,132,718 square-feet of commercial office uses. In addition, up to 224,039 square-feet of public institutional uses and up to 229,281 square-feet of industrial uses would be developed within the Specific Plan.

Below is a breakdown of the proposed land use types within the Downtown Taft Specific Plan:

Resi	dential Uses	3,120 DU
•	Single-Family Detached Dwelling Units:	665 DU
•	Single-Family Attached Dwelling Units (Townhomes):	1,186 DU
•	Multi-Family Dwelling Units:	1,269 DU
Reta	il Uses	891,059 SF
•	Retail Services Building Area:	695,038 SF
•	Restaurants Building Area:	59,762 SF
•	Arts & Entertainment Building Area:	71,715 SF
•	Accommodation Building Area	64,543 SF
Offic	ce Uses	1,132,718 SF
Office •	<u>ce Uses</u> Office Services Building Area	1,132,718 SF 471,455 SF
Office •		
•	Office Services Building Area	471,455 SF
•	Office Services Building Area Medical Services Building Area	471,455 SF 661,262 SF
•	Office Services Building Area Medical Services Building Area ic Administration Uses	471,455 SF 661,262 SF 224,039 SF
Publ	Office Services Building Area Medical Services Building Area ic Administration Uses Public Administration Building Area	471,455 SF 661,262 SF 224,039 SF 116,500 SF
Publ	Office Services Building Area Medical Services Building Area ic Administration Uses Public Administration Building Area Education Building Area	471,455 SF 661,262 SF 224,039 SF 116,500 SF 107,539 SF





Primary access to the Downtown Taft Specific Plan would be provided from Kern Street (SR-33), East Kern Street, Westside Highway (SR-33), 10th Street and 6th Street. Local access would be provided from all north-south streets intersecting Kern Street (SR-33) between 1st Street and 10th Street, Front Street, and Supply Row.

Exhibit 2 shows the Downtown Taft Specific Plan preferred land use plan.

STUDY AREA AND ANALYSIS METHODOLOGY

Project Study Area

As agreed upon with the City of Taft Planning and Development Services staff, the following intersections and roadway segments are included in the project study area, which are also illustrated in **Exhibit 3**:

Study Intersections

- 1. Kern Street (SR-33) / 10th Street
- 2. Kern Street (SR-33) / 6th Street
- 3. Kern Street (SR-33) / 4th Street
- 4. Kern Street (SR-33) / E. Kern Street / 1st Street (5-legged intersection)
- 5. Center Street / 10th Street
- 6. Center Street / Westside Highway (SR-33)

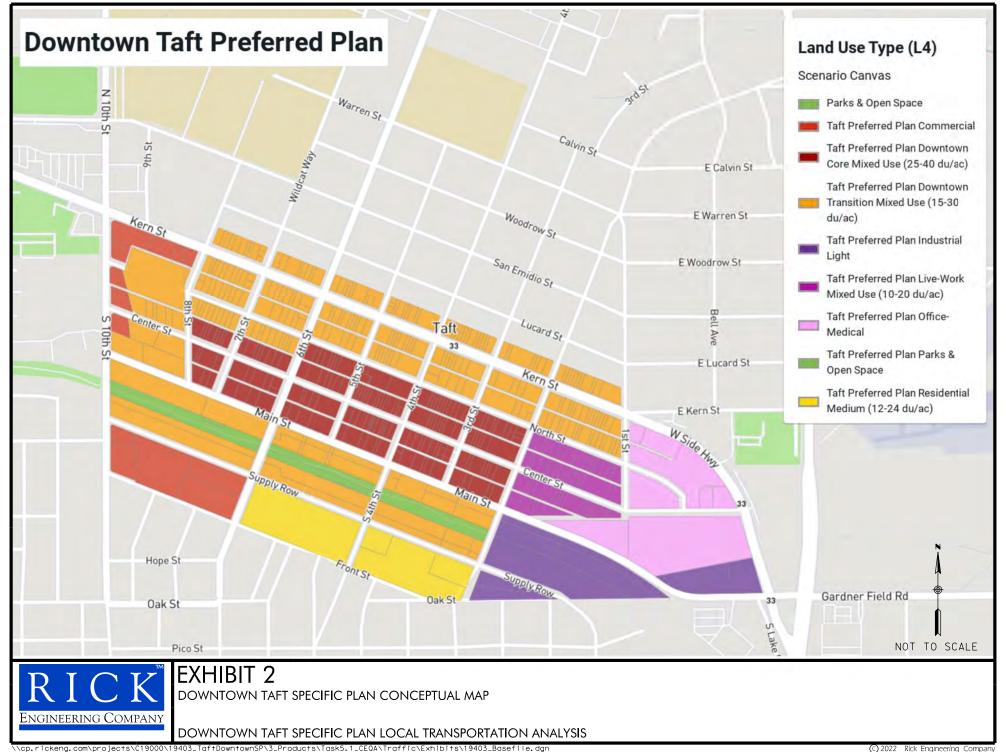
Study Roadway Segments

- 1. Kern Street (SR-33) between 10th Street and 6th Street
- 2. Kern Street (SR-33) between 6th Street and 1st Street
- 3. Westside Highway (SR-33) between 1st Street and Center Street
- 4. Westside Highway (SR-33) between Center Street and Main Street
- 5. Center Street between 10th Street and 6th Street
- 6. Center Street between 6th Street and Westside Highway
- 7. Main Street between 10th Street and 6th Street
- 8. Main Street between 6th Street and Westside Highway
- 9. 10th Street between Kern Street (SR-33) and Center Street
- 10. 6th Street between Kern Street (SR-33) and Center Street

Analysis Methodology

As agreed upon with the City of Taft Planning and Development Services staff, the following scenarios were evaluated in this Local Transportation Analysis:

- Existing Conditions: This scenario reflects the conditions on the ground at the time the traffic count data was obtained (March 2022).
- Horizon Year 2042 With Project Conditions: This scenario reflects the Horizon Year 2042 traffic volumes with the buildout of the Downtown Taft Specific Plan project, based on the model volumes generated by the Kern Council of Governments (Kern COG) regional travel demand model with the proposed Specific Plan land uses.







Intersection Analysis Methodology

Intersection operations were determined at the study area intersections for the weekday AM and PM peak hours. The AM peak hour intersection analysis evaluates LOS during the hour with the highest vehicular traffic between 7:00 AM and 9:00 AM. The PM peak hour intersection analysis evaluates LOS during the hour with the highest vehicular traffic between 4:00 PM and 6:00 PM.

The Level of Service (LOS) for signalized intersections was analyzed using the methodologies described in Chapter 19 of the 6th Edition Highway Capacity Manual (HCM 6). The LOS for signalized intersections is defined in terms of control delay, which is made up of several factors that relate to right-of-way control, geometrics and traffic volumes. The signalized intersection analysis also considers intersection spacing and coordination.

The LOS for two-way and all-way stop controlled intersections was calculated using the methodologies described in Chapters 20 and 21 of the 6th Edition HCM. The LOS for a two-way stop controlled intersection is determined by the computed control delay for each minor street movement and major street left-turns, and not for the intersection as a whole. The LOS reported reflects the highest delay and associated LOS for an individual movement, typically occurring on the stop controlled approach.

The computerized analysis of signalized and unsignalized intersection operations was performed utilizing the *Synchro 11* traffic analysis software. The *Synchro 11* software supports the HCM-6 methodologies for signalized and stop controlled intersections and was utilized to produce the analysis results.

The HCM-6 methodology for two-way stop controlled intersections within the *Synchro 11* software program does not accept analysis of a five-legged intersection. Therefore, the five-legged SR-33/East Kern Street/1st Street intersection was analyzed using the HCM-6 methodology within the *PTV Vistro Version 2021* software program.

Signal timing data and parameters such as cycle lengths, splits, clearance intervals, etc. were obtained from the current signal timing sheets provided by Caltrans District 6 and input into the Synchro software. Synchro reports delays, which correspond to a particular LOS, to describe the overall operation of an intersection.

The criteria for the LOS grade designations are provided in **Table 1**. LOS provides a quick overview of how well an intersection is performing. Within the City of Taft, LOS D or better is considered acceptable for all signalized and unsignalized intersections during the peak hours.

Roadway Segment Analysis Methodology

Roadway segments were analyzed based on the volume-to-capacity (v/c) ratios and the City's daily LOS capacity thresholds per Table 4.4-4 (Roadway Segment Daily Volume Thresholds) of the City of Taft General Plan Draft Environmental Impact Report (July 2009). The City of Taft considers LOS D or better to be acceptable for daily roadway segment operations. **Table 2** presents the roadway segment capacity and LOS thresholds utilized by the City of Taft.



TABLE 1 LOS CRITERIA FOR INTERSECTIONS

	HCM: CONTROL DEL ICU: VOLUME/CAP.		
LOS	SIGNALIZED INTERSECTIONS	UNSIGNALIZED INTERSECTIONS	DESCRIPTION
A	≤10 seconds	≤10 seconds	Operations with very low delay and most vehicles do not stop.
В	>10 and ≤20 seconds	>10 and ≤15 seconds	Operations with good progression but with some restricted movements.
С	>20 and <35 seconds	>15 and <u><</u> 25 seconds	Operations where a significant number of vehicles are stopping with some backup and light congestion.
D	>35 and ≤55 seconds	>25 and <u><</u> 35 seconds	Operations where congestion is noticeable, longer delays occur, and many vehicles stop. The proportion of vehicles not stopping declines.
Е	>55 and ≤80 seconds	>35 and ≤50 seconds	Operations where there is significant delay, extensive queuing, and poor progression.
F	>80 seconds	>50 seconds	Operations that are unacceptable to most drivers, when the arrival rates exceed the capacity of the intersection.

Source: 6th Edition Highway Capacity Manual. City of Taft General Plan DEIR (July 2009).



TABLE 2 LOS CRITERIA FOR ROADWAY SEGMENTS

Essilito Tomo	Number						
Facility Type	of Lanes	LOS A	LOS B	LOS C	LOS D	LOS E	
Residential	2	600	1,200	2,000	3,000	4,500	
Residential collector with frontage	2	1,600	3,200	4,800	6,400	8,000	
Residential collector without frontage	2	6,000	7,000	8,000	9,000	10,000	
	2	9,000	10,000	12,000	13,500	15,000	
Arterial, low access control	2	10,800	12,600	14,400	16,200	18,000	
	4	18,000	21,000	24,000	27,000	30,000	
	6	27,000	31,500	36,000	40,500	45,000	
Arterial, moderate access control	2	12,000	14,000	16,000	18,000	20,000	
	4	21,600	25,200	28,800	32,400	36,000	
	6	32,400	37,800	43,200	48,600	54,000	
Arterial, high access control	4	24,000	28,000	32,000	36,000	40,000	
	6	36,000	42,000	48,000	54,000	60,000	
Freeway	4	60,000	70,000	80,000	90,000	100,000	
	6	90,000	105,000	120,000	135,000	150,000	
Expressway	2	8,100	13,300	18,900	24,300	27,000	
	4	16,200	26,500	37,800	48,600	54,000	
Rural, 2-lane highway	2	2,400	4,800	7,900	13,500	22,900	
Rural, 2-lane road, paved shoulders	2	2,200	4,300	7,100	12,200	20,000	
Rural, 2-lane road, no shoulders	2	1,800	3,600	5,900	10,100	17,000	

Source: City of Taft General Plan DEIR (July 2009).



EXISTING TRANSPORTATION CONDITIONS

Existing Roadway Network

The following is a brief description of roadways within the study area as identified in the *City of Taft General Plan* (Amended April 2017):

Kern Street/State Route 33 (SR-33) is classified as an Arterial Highway and extends from the northwest to the southeast boundaries of the City of Taft. The roadway is currently constructed with two travel lanes in each direction from approximately 500 feet west of Cascade Place to the intersection with 1st Street and East Kern Street. Southeast of 1st Street, SR-33 is reduced to one travel lane in each direction. A striped center two-way left-turn lane is currently provided along Kern Street/SR-33 from 500 feet of Cascade Place to 10th Street. No left-turn lanes are provided Kern Street/SR-33 southeast of 9th Street within the Specific Plan area, and opposing travel lanes are separated by a striped double yellow line. On-street parking along Kern Street/SR-33 is generally permitted between 10th Street and 1st Street, but is prohibited along SR-33 southeast of 1st Street. Bicycle facilities are currently not provided along Kern Street/SR-33. The posted speed limit on Kern Street/SR-33 is 35 mph between 10th Street and 1st Street, and is 40 mph southeast of 1st Street.

10th Street is classified as an Arterial and extends from Ash Street to A Street/Oak Street within the City of Taft. The roadway is currently constructed with two travel lanes in each direction and a striped center two-way left-turn lane from Ash Street to Main Street. 10th Street narrows from four travel lanes to three travel lanes with a center two-way left-turn lane between Main Street and Front Street. South of Front Street, 10th Street is striped with one travel lane in each direction without a center two-way left-turn lane. On-street parking is generally prohibited along 10th Street from Ash Street to Front Street. South of Front Street, on-street parking is generally permitted along the east side of the street. Bicycle facilities are currently not provided along 10th Street. The posted speed limit on 10th Street is 35 mph.

6th Street is classified as a Collector and extends from Ash Street to Oak Street within the City of Taft. The roadway is currently constructed with two travel lanes in each direction from Ash Street to Main Street. 6th Street narrows from four travel lanes to two travel lanes south of Main Street. The opposing travel lanes are separated by a striped double yellow lines. No left-turn lanes are provided along 6th Street. On-street parking is generally permitted along 6th Street. South of Front Street, on-street parking is generally permitted along the east side of the street. Bicycle facilities are currently not provided along 6th Street. The posted speed limit on 6th Street is 35 mph.

Center Street is classified as a Local Street and extends from 10th Street to SR-33 within the City of Taft. The roadway is currently constructed with one travel lane in each direction, and the opposing travel lanes are separated by a dashed yellow line. No left-turn lanes are provided along Center Street. Onstreet parking is generally permitted on both sides of Center Street, and angled parking spaces are provided along one or both sides of the street between 10th Street and 2nd Street. Bicycle facilities are currently not provided along Center Street. The posted speed limit on Center Street is 25 mph.



Main Street is classified as a Local Street and extends from 10th Street to SR-33 within the City of Taft. The roadway is currently constructed with one travel lane in each direction, and the opposing travel lanes are separated by a dashed yellow line. No left-turn lanes are provided along Main Street. On-street parking is generally permitted on one or both sides of Main Street between 10th Street and 2nd Street. On-street parking is generally prohibited along Main Street between 2nd Street and SR-33, except for a short section between 300 feet and 600 feet west of SR-33 where on-street parking is permitted along the north side of the roadway. Bicycle facilities are currently not provided along Main Street between 10th Street and 2nd Street. A Class II bicycle lane is currently provided in each direction of travel along Main Street between 2nd Street and SR-33. The posted speed limit on Main Street is 25 mph between 10th Street and 2nd Street, and is 35 mph between 2nd Street and SR-33.

Exhibit 4 illustrates the existing (2022) transportation conditions within the study area as described above.

Existing Transportation Volumes

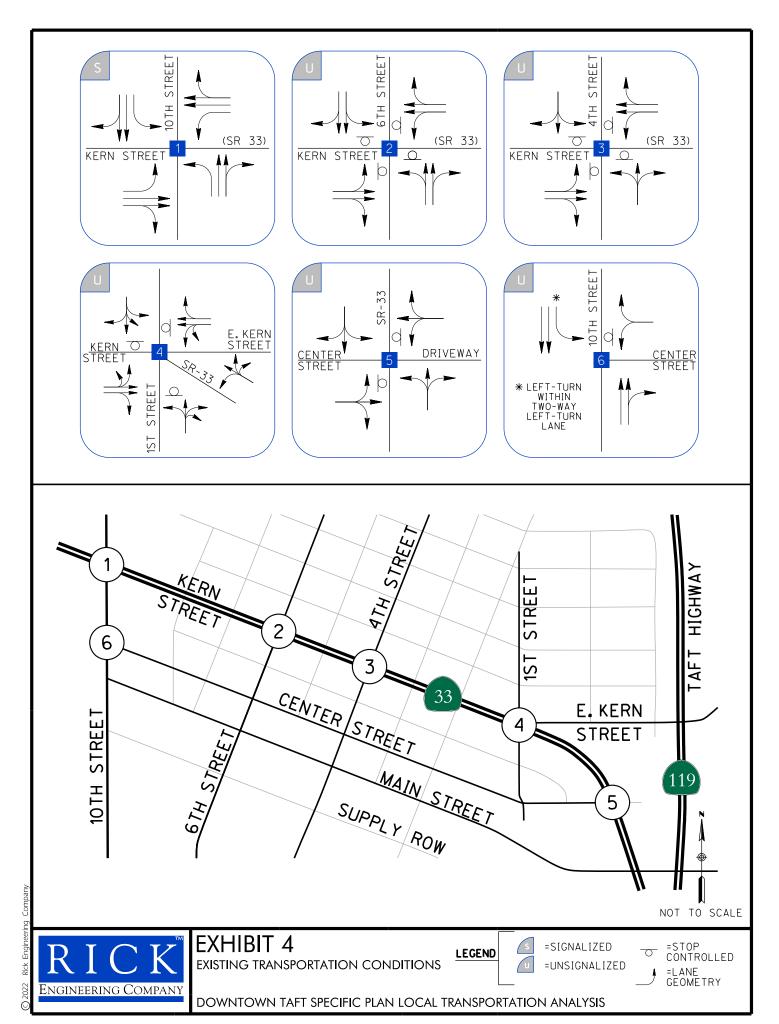
Existing vehicular, pedestrian and bicycle volumes at the following six (6) study intersections were obtained from counts conducted by Metro Traffic Data on Tuesday, March 15, 2022 during the AM (7-9) and PM (4-6) peak periods while local schools were in regular session:

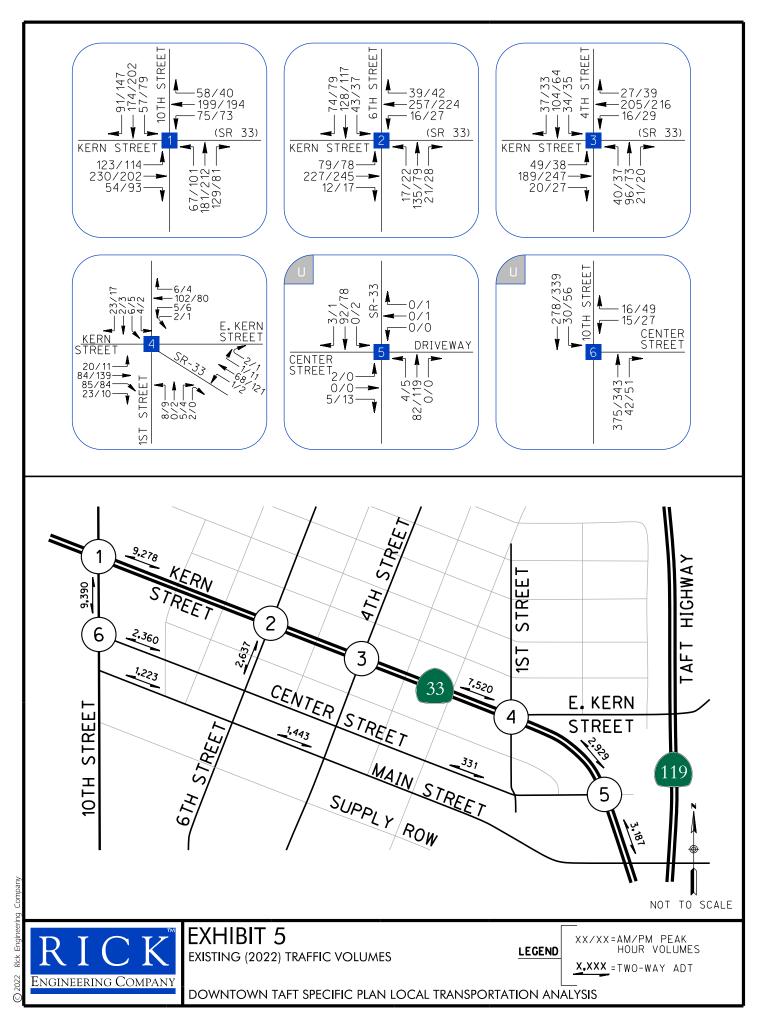
- 1. Kern Street (SR-33) / 10th Street
- 2. Kern Street (SR-33) / 6th Street
- 3. Kern Street (SR-33) / 4th Street
- 4. Kern Street (SR-33) / E. Kern Street / 1st Street (5-legged intersection)
- 5. Center Street / 10th Street
- 6. Center Street / Westside Highway (SR-33)

Additionally, machine counts were collected on Tuesday, March 15, 2022 on the following 10 roadway segments listed below in order to document the daily vehicular traffic volumes for a 24-hour period:

- 1. Kern Street (SR-33) between 10th Street and 6th Street
- 2. Kern Street (SR-33) between 6th Street and 1st Street
- 3. Westside Highway (SR-33) between 1st Street and Center Street
- 4. Westside Highway (SR-33) between Center Street and Main Street
- 5. Center Street between 10th Street and 6th Street
- 6. Center Street between 6th Street and Westside Highway
- 7. Main Street between 10th Street and 6th Street
- 8. Main Street between 6th Street and Westside Highway
- 9. 10th Street between Kern Street (SR-33) and Center Street
- 10. 6th Street between Kern Street (SR-33) and Center Street

Exhibit 5 shows the existing intersection turning movement volumes and average daily traffic (ADT) volumes within the study area. **Appendix A** contains the vehicular, pedestrian and bicycle count data.







Existing Intersection Operations

The existing intersection operations results are based on existing turning movement volumes collected, existing intersection geometry and existing signal timing.

Table 3 shows the existing conditions intersection operations during the peak hours. **Appendix B** contains the existing conditions intersection operations worksheets.

TABLE 3
EXISTING INTERSECTION OPERATIONS

				EXISTIN	NG (2022)
	INTERSECTION			DELAY ¹	LOS
1	Kern Street (SR-33) / 10th Street				
		AM peak	(S)		
			OVERALL	27.6	C
		PM peak	(S)		
			OVERALL	26.7	С
2	Kern Street (SR-33) / 6th Street				
		AM peak	(AWSC)		
			OVERALL	15.6	C
		PM peak	(AWSC)		
			OVERALL	12.0	В
3	Kern Street (SR-33) / 4th Street				
		AM peak	(AWSC)		
			OVERALL	12.0	В
		PM peak	(AWSC)		
			OVERALL	10.9	В
4	Kern Street (SR-33) / E. Kern Street / 1st St.				
		AM peak	(TWSC)		
			NBL	13.9	В
		PM peak	(TWSC)		
			NBT	16.0	С
5	Center Street / Westside Highway (SR-33)				
		AM peak	(TWSC)		
			EBL	9.1	A
		PM peak	(TWSC)		
			WBL	9.8	A
6	Center Street / 10th Street				
		AM peak	(OWSC)		
			WBL	12.2	В
		PM peak	(OWSC)		
			WBL	11.6	В

 $Footnotes: S = Signalized; AWSC = All-Way-Stop\ Controlled; TWSC = Two-Way-Stop\ Controlled; OWSC = One-Way-Stop\ Controlled$

As shown in Table 3, all study intersections currently operate at LOS C or better during the AM and PM peak hours.

¹Delay is measured in seconds per vehicle.

²PTV Vistro software program was used as interface to HCM-6 methodology to analyze the 5-legged Kern Street (SR-33) / E. Kern Street / 1st Street because the Synchro 11 software program does not accept analysis of 5-legged intersections.



Existing Conditions Roadway Segment Analysis

The existing roadway level of service results are based on existing daily traffic volumes collected and functional classification roadway capacity. **Table 4** summarizes the roadway segment capacity analysis results under Existing Conditions. As shown in Table 4, all study roadway segments currently operate at LOS B or better.

PROJECT TRIP GENERATION

The trip generation for the Downtown Taft Specific Plan project was calculated based on the published Institute of Transportation Engineers (ITE) 11th Edition Trip Generation Manual (September 2021) trip rates for the proposed land uses.

The specific ITE trip generation rates for the proposed residential land use types were applied, but due to the greater uncertainty of what specific types of retail or office uses would ultimately occupy the Specific Plan area, the "Shopping Center Over 150K" (ITE Land Use Code 820) trip rate was applied to all uses under the "Retail" category. In addition, the "Office Park" (ITE Land Use Code 750) trip rate was applied to all uses under the "Office" category.

Although 661,262 square-feet of the office uses is proposed as "medical services", the only medical office trip rate in the 11th Edition ITE Trip Generation Manual is the "Medical-Dental Office Building Stand-Alone" (ITE Land Use Code 720), which is intended to only be used for a stand-alone medical office building. Applying the ITE Land Use Code 720 trip rate to the 661,262 square-feet of medical services use would result in an artificially high trip generation compared to the other proposed uses, and therefore the general "Office Park" ITE Land Use Code 750) trip rate was applied to the 661,262 square-feet of proposed medical services.

The average ITE trip rates were applied to the proposed residential land use types, but for all non-residential uses, fitted curve equations were applied, where available, to calculate the trip generation of the retail, office, public administration and industrial uses.

Table 5 shows the ITE trip generation rates that were applied to the proposed Downtown Taft Specific Plan land uses, and **Table 6** presents the trip generation of the buildout of the Downtown Taft Specific Plan. **Appendix C** contains the 11th Edition ITE Trip Generation Manual (September 2021) trip rate sheets.

As shown in Table 6, the Downtown Taft Specific Plan project is forecast to generate a net total of 58,925 trips per day, with a net total of 3,862 trips occurring during the AM peak hour (2,350 IN, 1,512 OUT), and a net total of 5,145 trips occurring during the PM peak hour (2,109 IN, 3,036 OUT).

The net total trip generation of the Downtown Taft Specific Plan project accounts for an internal capture trip reduction that was calculated using the NCHRP Report 684 Internal Capture Estimation Tool, which is also shown in Table 6. The internal capture calculation worksheets are also provided in Appendix C.



TABLE 4
EXISTING CONDITIONS ROADWAY SEGMENT ANALYSIS

ROADWAY SEGMENT		Classification!	# of	Daily	EXISTING (2022)	
	ROADWAY SEGMENI	Classification ¹	Lanes	Capacity ¹	Volume	LOS
1	Kern Street (SR-33) between 10th Street and 6th Street	Arterial Highway	4	30,000	9,278	A
2	Kern Street (SR-33) between 6th Street and 1st Street	Arterial Highway	4	30,000	7,520	A
3	Westside Highway (SR-33) between 1st Street and Center Street	Arterial Highway	2	18,000	2,929	A
4 Westside Highway (SR-33) between Center Street and Main Street		Arterial Highway	2	18,000	3,187	A
5	Center Street between 10th Street and 6th Street	Collector	2	8,000	2,360	В
6	Center Street between 6th Street and Westside Highway (SR-33)	Collector	2	8,000	331	A
7	Main Street between 10th Street and 6th Street	Collector	2	8,000	1,223	A
8	Main Street between 6th Street and Westside Highway (SR-33)	Collector	2	8,000	1,443	A
9	10th Street between Kern Street (SR-33) and Center Street	Arterial	4	30,000	9,390	A
10	6th Street between Kern Street (SR-33) and Center Street	Collector	4	15,000	2,637	A

Footnotes

¹Roadway classification and daily capacity taken from Table 4.4-4 (Roadway Segment Daily Volume Thresholds) of the City of Taft General Plan Draft Environmental Impact Report (July 2009)



TABLE 5 ITE TRIP GENERATION RATES

		Daily	AM Peak Hour			PM Peak Hour			
Land Use	Unit	Rate (per unit)	Rate	Inbound (% AM)	Outbound (% AM)	Rate	Inbound (% PM)	Outbound (% PM)	
Single-Family Detached (LU Code 210)	DU	9.43	0.70	26%	74%	0.94	63%	37%	
Single-Family Attached (LU Code 215)	DU	7.20	0.48	31%	69%	0.57	57%	43%	
Multi-Family Low-Rise (LU Code 220)	DU	6.74	0.40	24%	76%	0.51	63%	37%	
Shopping Center >150k (LU Code 820)	KSF	$T = 26.11(X) + 5863.73^{a}$	$T = 0.59(X) + 133.55^{a}$	62%	38%	$Ln(T) = 0.72Ln(X) + 3.02^{a}$	48%	52%	
Office Park (LU Code 750)	KSF	$Ln(T) = 0.89Ln(X) + 3.10^{a}$	$T = 0.94(X) + 194.06^{a}$	89%	11%	$T = 1.26(X) + 20.98^{a}$	14%	86%	
Public Administration (LU Code 730)	KSF	22.59	3.34	75%	25%	$Ln(T) = 0.97Ln(X) + 0.62^{a}$	25%	75%	
Warehousing (LU Code 150)	KSF	$T = 1.58(X) + 38.29^{a}$	$T = 0.12(X) + 23.62^{a}$	77%	23%	$T = 0.12(X) + 26.48^{a}$	28%	72%	

Footnotes:

Source: ITE Trip Generation Manual, 11th Edition (2021) DU = Dwelling Unit; KSF = Thousand Square-Feet

^aFitted curve equation provided to calculate the trip generation.



TABLE 6 PROJECT TRIP GENERATION

Ladilla	Land Use Size Unit		Daily	Daily AM Peak Hour			PM Peak Hour		
Land Use	Size	Unit	Trips	Total	Inbound	Outbound	Total	Inbound	Outbound
Single-Family Detached (LU Code 210)	665	DU	6,271	466	121	345	625	394	231
Single-Family Attached (LU Code 215)	1,186	DU	8,539	569	176	393	676	385	291
Multi-Family Low-Rise (LU Code 220)	1,269	DU	8,553	508	122	386	647	408	239
Retail (LU Code 820)	891.059	KSF	29,129	659	409	250	2,726	1,308	1,418
Office Park (LU Code 750)	1132.718	KSF	11,601	1,259	1,121	138	1,448	203	1,245
Public Administration (LU Code 730)	224.039	KSF	5,061	748	561	187	355	89	266
Warehousing (LU Code 150)	229.281	KSF	401	51	39	12	54	15	39
Subtotal Project Trips			69,555	4,260	2,549	1,711	6,531	2,802	3,729
Internal Co	-10,630	-398	-199	-199	-1,386	-693	-693		
NET TOTAL	58,925	3,862	2,350	1,512	5,145	2,109	3,036		

Footnotes:

Source: ITE Trip Generation Manual, 11th Edition (2021) DU = Dwelling Unit; KSF = Thousand Square-Feet

^aInternal capture was calculated using the NCHRP Report 684 Internal Capture Estimation Tool (see Appendix C).



HORIZON YEAR 2042 WITH PROJECT TRANSPORTATION CONDITIONS

Horizon Year 2042 With Project conditions reflect the buildout of the Downtown Taft Specific Plan and City of Taft General Plan. The Kern Council of Governments (Kern COG) Year 2042 regional travel demand model was used to develop the Horizon Year 2042 With Project traffic volumes in the study area. RICK provided Kern COG the proposed land uses to input into the five Travel Analysis Zones (TAZs) that make up the Downtown Taft Specific Plan area in the Kern COG model, which are listed below:

- TAZ 1805
- TAZ 1806
- TAZ 1816
- TAZ 1817
- TAZ 1819

Kern COG conducted a model run with the proposed land use changes using the Year 2042 regional travel demand model and provided RICK with a model plot that illustrate the Horizon Year 2042 With Specific plan daily volumes for the Greater Taft Subarea. A copy of the Kern COG Horizon Year 2042 With Specific Plan Model ADT plot is contained in **Appendix D**.

Horizon Year 2042 With Project Conditions Roadway Network

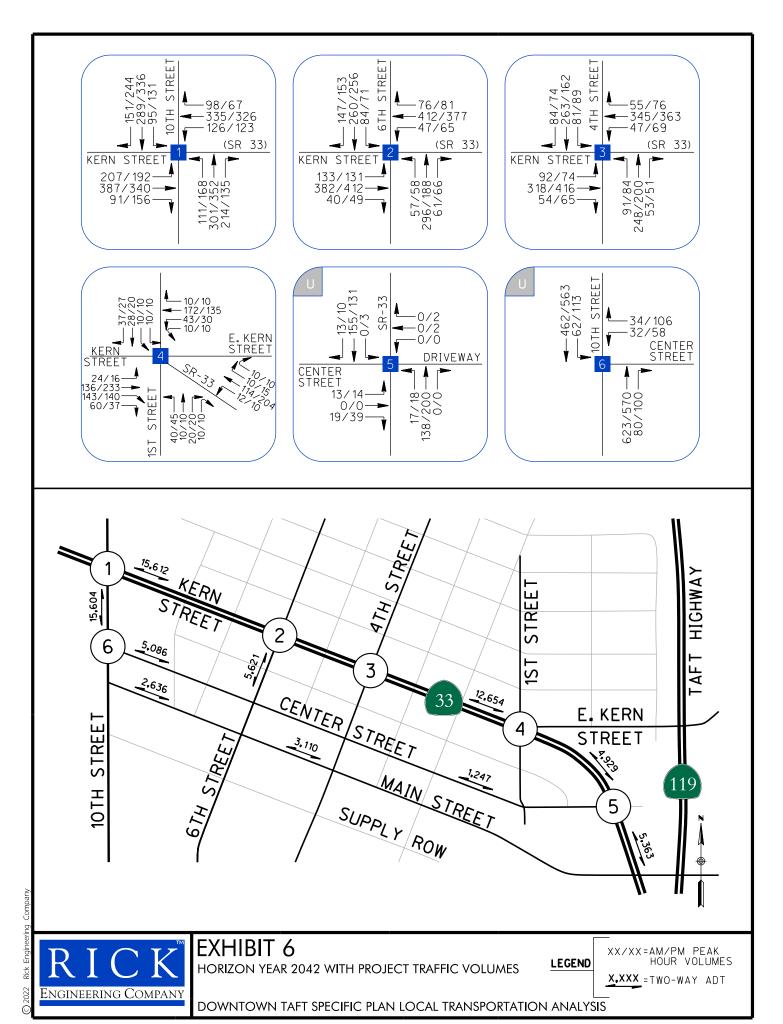
The Horizon Year 2042 With Project roadway network is based on the current Kern COG 2018 Regional Transportation Plan (RTP) and the buildout of the City of Taft General Plan Circulation Element. Neither the Kern COG 2018 RTP nor the City of Taft General Plan Circulation Element include any future roadway improvements within the Downtown Taft Specific Plan area. Therefore, the Horizon Year 2042 roadway network is essentially the same as existing conditions.

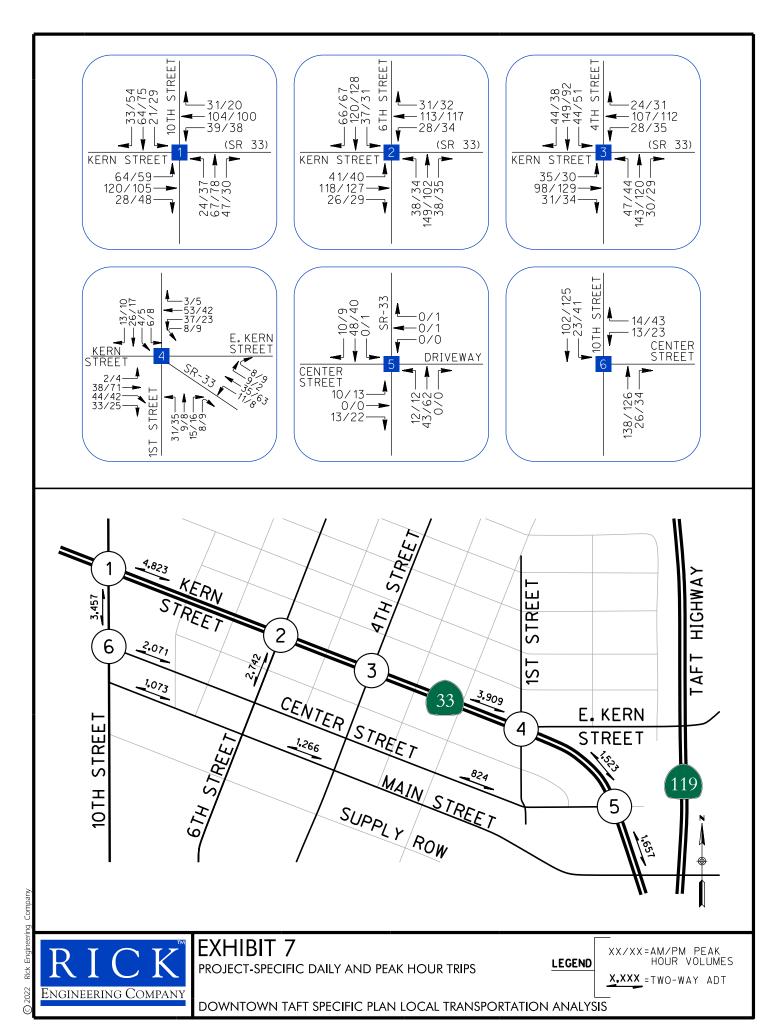
Horizon Year 2042 With Project Conditions Traffic Volumes

The Horizon Year 2042 With Project traffic volumes were derived based on the forecast growth in traffic in the Kern COG regional travel demand model. Growth factors were developed based on the forecast traffic growth from Baseline Year 2020 to Horizon Year 2042 with the Downtown Taft Specific Plan. These growth factors were applied to the existing conditions (Year 2022) daily traffic volumes to reflect 20 years of traffic growth from 2022 to 2042.

The Horizon Year 2042 With Project daily traffic volumes derived from the model growth in traffic were post-processed to develop the intersection turning movement volumes during the AM and PM peak hours. Adjustments were made where appropriate to reflect changes in traffic patterns and growth on all legs of the study intersections. The Horizon Year 2042 With Project intersection post-processing worksheets are also provided in Appendix D. **Exhibit 6** illustrates the Horizon Year 2042 With Project traffic volumes at the study intersections and roadway segments with the buildout of the Downtown Taft Specific Plan.

Project-specific daily and AM/PM peak hour trips associated with the proposed Downtown Taft Specific Plan land uses were calculated based on the changes in the daily traffic volumes between the Horizon Year 2042 baseline (without project) volumes from the Kern COG regional travel demand model and the Horizon Year 2042 With Specific Plan volumes from the model run conducted by Kern COG. The Horizon Year 2042 baseline volumes were also post-processed to derive the Horizon Year 2042 baseline AM/PM peak hour turning movement volumes, and the project-specific AM/PM peak hour trips were calculated by subtracting the Horizon Year 2042 baseline volumes from the Horizon Year 2042 With Project volumes. **Exhibit 7** illustrates the project-specific daily and AM/PM peak hour trips with the buildout of the Downtown Taft Specific Plan.







Horizon Year 2042 With Project Conditions Intersection Operations

Table 7 displays the operational analysis results for the study intersections under Horizon Year 2042 conditions with the buildout of the Downtown Taft Specific Plan. **Appendix E** contains the Horizon Year 2042 With Project intersection analysis worksheets.

TABLE 7 HORIZON YEAR 2042 WITH PROJECT INTERSECTION OPERATIONS

INTERSECTION				HORIZON Y	
	1,121,5251161,			DELAY ¹	LOS
1	Kern Street (SR-33) / 10th Street				
		AM peak	(S)		
			OVERALL	45.0	D
		PM peak	(S)		
			OVERALL	40.0	D
2	Kern Street (SR-33) / 6th Street				
		AM peak	(AWSC)		
			OVERALL	117.7	F
		PM peak	(AWSC)		
			OVERALL	42.9	E
3	Kern Street (SR-33) / 4th Street				
		AM peak	(AWSC)		
			OVERALL	118.8	F
		PM peak	(AWSC)		
			OVERALL	55.4	F
4	Kern Street (SR-33) / E. Kern Street / 1st Street				
		AM peak	(TWSC)		
			NBT	47.6	E
		PM peak	(TWSC)		
			NBT	67.2	F
5	Center Street / Westside Highway (SR-33)				
		AM peak	(TWSC)		_
			EBL	10.3	В
		PM peak	(TWSC)		ъ
<u> </u>			WBL	11.0	В
6	Center Street / 10th Street		(OHIGG)		
		AM peak	(OWSC)	10.6	a
			WBL	18.6	C
		PM peak	(OWSC)	10.1	C
			WBL	19.1	С

 $Footnotes: S = Signalized; AWSC = All-Way-Stop\ Controlled; TWSC = Two-Way-Stop\ Controlled; OWSC = One-Way-Stop\ Controlled$

¹Delay is measured in seconds per vehicle.

²PTV Vistro software program was used as interface to HCM-6 methodology to analyze the 5-legged Kern Street (SR-33) / E. Kern Street / 1st Street because the Synchro 11 software program does not accept analysis of 5-legged intersections.



As shown in Table 7, the following intersections are forecast to operate at a deficient LOS E or F during the AM and PM peak hours under Horizon Year 2042 conditions with the buildout of the Downtown Taft Specific Plan:

- Kern Street (SR-33) / 6th Street (AM: LOS F; PM: LOS E)
- Kern Street (SR-33) / 4th Street (AM/PM: LOS F)
- Kern Street (SR-33) / E. Kern Street / 1st Street (AM: LOS E; PM: LOS F)

Horizon Year 2042 With Project Conditions Roadway Segment Analysis

Table 8 summarizes the roadway segment capacity analysis results under Horizon Year 2042 conditions with the buildout of the Downtown Taft Specific Plan. As shown in Table 8, all study roadway segments are forecast to operate at LOS D or better under Horizon Year 2042 conditions with the buildout of the Downtown Taft Specific Plan.

ACTIVE TRANSPORTATION AND PUBLIC TRANSIT ASSESSMENT

Existing Pedestrian Network

Sidewalks are generally provided along both sides of most roadways within the Specific Plan area. Below are descriptions of the existing pedestrian facilities along key roadways within the Specific Plan:

Kern Street/West Side Highway (SR-33)

Sidewalk widths range from 12 feet to 15 feet along both sides of Kern Street (SR-33) between 10th Street and 8th Street. Sidewalks along Kern Street (SR-33) between 8th Street and 1st Street are primarily non-contiguous on both sides of the roadway and are approximately five (5) feet in width. Several wide sections of sidewalk 12-15 feet in width are also provided along Kern Street (SR-33) between 8th Street and 1st Street. No pedestrian facilities are provided along West Side Highway (SR-33) southwest of the Kern Street (SR-33)/East Kern Street/1st Street intersection.

Pedestrian crossings are provided at most intersections along the Kern Street (SR-33) corridor between 10th Street and 1st Street. Signalized pedestrian crossings with high-visibility ladder crosswalks are provided across all four legs of the Kern Street (SR-33)/10th Street intersection. There are also several uncontrolled pedestrian crossings across Kern Street (SR-33) where high-visibility ladder crosswalks are provided and solar-powered LED enhanced pedestrian crossing signs with push-button activation, which are provided at the following intersections: Kern Street (SR-33)/8th Street, Kern Street (SR-33)/7th Street, Kern Street (SR-33)/5th Street, Kern Street (SR-33)/2nd Street, and Kern Street (SR-33)/East Kern Street/1st Street. High-visibility ladder crosswalks are also provided across all four legs of the all-way-stop controlled intersections of Kern Street (SR-33)/6th Street and Kern Street (SR-33)/4th Street.

Center Street

Contiguous sidewalks treated with pavers are provided along both sides of Center Street between 10th Street and 2nd Street adjacent to primarily commercial uses, and range between 6 feet and 12 feet in width. East of 2nd Street, land uses transition from commercial to residential, and the standard 5-foot wide contiguous sidewalks are provided on both sides of Center Street between 2nd Street and 1st Street. No sidewalks are provided along Center Street between 1st Street and SR-33, where adjacent properties are primarily undeveloped.



TABLE 8
HORIZON YEAR 2042 WITH PROJECT ROADWAY SEGMENT ANALYSIS

ROADWAY SEGMENT		Classification ¹	# of Lanes	Capacity ¹	HORIZON YR (2042) WITH PROJECT	
			Lanes		Volume	LOS
1	Kern Street (SR-33) between 10th Street and 6th Street	Arterial Highway	4	30,000	15,612	A
2	Kern Street (SR-33) between 6th Street and 1st Street	Arterial Highway	4	30,000	12,654	A
3	Westside Highway (SR-33) between 1st Street and Center Street	Arterial Highway	2	18,000	4,929	A
4 Westside Highway (SR-33) between Center Street and Main Street		Arterial Highway	2	18,000	5,363	A
5 Center Street between 10th Street and 6th Street		Collector	2	8,000	5,086	D
6 Center Street between 6th Street and Westside Highway (SR-33)		Collector	2	8,000	1,247	A
7	Main Street between 10th Street and 6th Street	Collector	2	8,000	2,636	В
8	Main Street between 6th Street and Westside Highway (SR-33)	Collector	2	8,000	3,110	В
9	10th Street between Kern Street (SR-33) and Center Street	Arterial	4	30,000	15,604	A
10	6th Street between Kern Street (SR-33) and Center Street	Collector	4	15,000	5,621	A

Footnotes:

¹Roadway classification and daily capacity taken from Table 4.4-4 (Roadway Segment Daily Volume Thresholds) of the City of Taft General Plan Draft Environmental Impact Report (July 2009)



Both controlled and uncontrolled pedestrian crossings are provided at several intersections and at midblock locations along Center Street between 10th Street and 2nd Street. High-visibility ladder crosswalks and/or crosswalks treated with pavers are provided at uncontrolled mid-block crossings between 7th Street and 6th Street, between 6th Street and 5th Street, between 5th Street and 4th Street, between 4th Street and 3rd Street, and between 3rd Street and 2nd Street. High-visibility ladder crosswalks are also provided across Center Street and other intersection legs at the intersections of Center Street/7th Street, Center Street/6th Street, Center Street/4th Street, Center Street/3rd Street, and Center Street/2nd Street. Crosswalks treated with pavers are provided across all four legs of the Center Street/5th Street intersection.

Main Street

Most of the segment of Main Street between 10th Street and 7th Street is currently lacking sidewalks along one or both sides of the roadway. Along Main Street between 7th Street and 3rd Street, a mix of contiguous and non-contiguous sidewalks are provided on both sides of the street, with a few missing gaps along undeveloped parcels. Sidewalk widths range from 5 feet to 10 feet along Main Street between 7th Street and 3rd Street. Along Main Street between 3rd Street and 2nd Street, sidewalks are only provided along the north side of the street, and along Main Street between 2nd Street and SR-33, where adjacent properties are primarily undeveloped, sidewalks are not provided except for short sections along developed parcels.

An uncontrolled mid-block pedestrian crossing with a crosswalk treated with pavers is provided across Main Street between 6th Street and 4th Street on the east leg of the former Main Street/5th Street intersection. The former 5th Street between Center Street and Main Street was converted to a linear park in which vehicular traffic is prohibited.

10th Street

Contiguous sidewalks ranging between 5 feet and 10 feet in width are currently provided on both sides of 10th Street between Kern Street (SR-33) and Main Street. Contiguous sidewalks are only provided along the west side of 10th Street between Main Street and Supply Row, and along 10th Street south of Supply Row, contiguous sidewalks are provided on both sides of the street. An uncontrolled mid-block pedestrian crossing with a high-visibility ladder crosswalk is provided across 10th Street at the location where the Taft Rails to Trails Multi-Use Path crosses 10th Street.

6th Street

Contiguous sidewalks approximately 10 feet in width are currently provided on both sides of 6th Street between Kern Street (SR-33) and Main Street. Between Main Street and Supply Row, contiguous sidewalks approximately 5 feet in width are currently provided on both sides of 6th Street. Contiguous sidewalk is currently only provided along the west side of 6th Street between Supply Row and Front Street.

High-visibility ladder crosswalks are provided across 6th Street at the intersections of 6th Street/Kern Street (SR-33), 6th Street/North Street, 6th Street/Center Street, and at an uncontrolled mid-block pedestrian crossing at the location where the Taft Rails to Trails Multi-Use Path crosses 6th Street between Main Street and Supply Row.



Existing Bicycle Network

Most of the roadways within the Specific Plan area are currently lacking bicycle facilities except for Main Street between 2nd Street and SR-33, where a narrow shoulder with "bike lane" signage is provided in both directions of travel, but the shoulder lane is discontinuous in the eastbound direction. The lane widths of the existing bicycle lanes and associated signage along Main Street between 2nd Street and SR-33 are substandard, and therefore these existing bicycle lanes are not considered Class II bicycle lanes.

A Class I bike path (Taft Rails to Trails) is currently provided within the Specific Plan area between Main Street and Supply Row. The Taft Rails to Trails bike path is oriented in a general east-west direction and is approximately two (2) miles in length. The bike path is approximately 12 feet wide and is divided by a dashed line along the western and eastern sections, but is undivided through the middle section of the bike path. There are no current plans to extend the existing Taft Rails to Trails Class I bike path, although the City of Taft General Plan Circulation Element identifies future trail connections to the community of Fellows to the northwest and the community of Maricopa to the southeast.

Existing Transit Network

Taft Area Transit (TAT) and Kern Transit currently provide the following transit bus routes within the Specific Plan area and through the City of Taft:

- TAT Taft-Maricopa Route: The Taft-Maricopa Route extends between the Cities of Taft and Maricopa via Kern Street/SR-33. Within the City of Taft, the Taft-Maricopa Route extends north from Kern Street (SR-33) onto Wildcat Way (aligned with 7th Street) and provides transit access to Taft High School, Taft College, and the Albertsons shopping center. The route continues west along Ash Street and heads south on 10th Street back to Kern Street (SR-33), and proceeds southeast along Kern Street (SR-33) through the Specific Plan area toward Maricopa.
 - Service is currently provided Monday through Friday between 7:12am and 6:05pm, and runs three (3) times daily. During the morning peak hour, the Taft-Maricopa Route begins at the Maricopa Post Office at 7:12am, and ends at the bus stop along eastbound Kern Street (SR-33) at 2nd Street at 7:42am. The Taft-Maricopa Route runs again in the afternoon, beginning at Kern Street (SR-33) at 2nd Street at 1:34pm. The afternoon route circulates through the City of Taft and heads to Maricopa, then returns to circulate through Taft a second time and ends at the bus stop along eastbound Kern Street (SR-33) at 2nd Street at 2:25pm. A third route runs during the evening peak hour, beginning at Kern Street (SR-33) at 2nd Street at 5:14pm. The evening route circulates through the City of Taft then heads to Maricopa, and then returns to circulate through Taft a second time and ends at the bus stop along eastbound Kern Street (SR-33) at 2nd Street at 6:05pm.
- **Kern Transit Route 120 (Taft-Bakersfield):** Kern Transit Route 120 extends between Taft and Bakersfield via 6th Street, Harrison Street, Highway 119, Highway 43, and Highway 58. Within the City of Taft and the Specific Plan area, Route 120 heads south on 6th Street, heads west on Kern Street (SR-33), heads south on 8th Street, heads east on Main Street, heads south on 4th Street, heads west on Supply Row to the Taft Transit Center (550 Supply Row), continues west on Supply Row, and heads north on 6th Street back to Highway 119 and Bakersfield. Within the City of Taft, stops are provided at Taft College, the Heritage Park Senior Complex, (8th Street at North Street), and the Taft Transit Center.



Service is currently provided Monday through Friday between 6:10am and 9:02pm, and runs four (4) times daily. Service is also provided on Saturday between 7:50am and 6:57pm, and runs three (3) times daily from Taft to Bakersfield, and two (2) times daily from Bakersfield to Taft. During the weekday morning hours, westbound Route 120 departs the Downtown Bakersfield Transit Center at 6:10am, and arrives at the Taft Transit Center at 7:16am. Eastbound Route 120 during the morning hours departs Taft College at 7:40am and arrives at the Downtown Bakersfield Transit Center at 9:07am. Headways during the morning peak period are approximately every two (2) hours. Route 120 also operates afternoon service between Taft and Bakersfield from 12:47pm to 3:57pm. During the evening hours, westbound Route 120 departs the Downtown Bakersfield Transit Center at 5:47pm and arrives at the Taft Transit Center at 7:09pm. Eastbound Route 120 during the evening hours departs Taft College at 7:35pm and arrives at the Downtown Bakersfield Transit Center at 9:02pm.

The Taft Area Transit (TAT) Taft-Maricopa Route and Kern Transit Route 120 maps and schedules as described above are provided in **Appendix F**.

A total of four (4) transit bus stops for the Taft Area Transit (TAT) Taft-Maricopa Route are currently provided along Kern Street (SR-33) within the Specific Plan area, which are listed below:

- Eastbound Kern Street (SR-33) at 5th Street: No shelter or amenities provided.
- Eastbound Kern Street (SR-33) at 2nd Street: No shelter or amenities provided.
- Westbound Kern Street (SR-33) at 2nd Street: No shelter or amenities provided.
- Westbound Kern Street (SR-33) at 4th Street: Bench, shelter and trash receptacle provided.

The Taft Transit Center, located at 550 Supply Row within the Specific Plan area, serves Kern Transit Route 120 between Taft and Bakersfield. One transit bus stop is also provided for Kern Transit Route 120 within the Specific Plan area along southbound 8th Street at North Street next to the Heritage Park Senior Complex, where a bench and shelter is provided.

Future Pedestrian Network

The Kern Region Active Transportation Plan (Alta Planning + Design, March 2018) provides recommendations for future pedestrian facility improvements in the City of Taft in order of priority and are ranked as Tier 1 (highest priority), Tier 2 (medium priority) and Tier 3 (lowest priority). Below is a list of the recommended Tier 1 and Tier 2 pedestrian facility improvements from the Kern Region Active Transportation Plan that are within and adjacent to the Downtown Taft Specific Plan area, in order of priority:

Tier 1 Projects (Highest Priority: 1-10 Years)

- 10th Street from Ash Street to Main Street: Pedestrian corridor improvements (high-visibility crossings and traffic calming measures)
- 6th Street from Ash Street to Main Street: Pedestrian corridor improvements (high-visibility crossings and traffic calming measures)

Tier 2 Projects (Medium Priority: 10-15 Years)

• Kern Street (SR-33) from Cascade Place to Taft Highway: Pedestrian corridor improvements (some corridor improvements have already been completed)



All of the above-listed planned future pedestrian corridor improvements are anticipated to be completed prior to buildout of the Downtown Taft Specific Plan.

Recommended Future Pedestrian Facility Improvements With Specific Plan

In addition to the above-listed planned future pedestrian corridor improvements, the following pedestrian facility improvements are recommended to enhance pedestrian safety and access within the Downtown Taft Specific Plan:

- North Street from 8th Street to 1st Street: Improve existing sidewalks and provide high-visibility crosswalks at all intersections.
- Center Street from 2nd Street to Westside Highway (SR-33): Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- Main Street from 10th Street to 2nd Street: Close the existing sidewalk gaps and provide wide sidewalks (10+ feet in width) along both sides of the street. Provide high-visibility crosswalks at all intersections.
- Main Street from 2nd Street to Westside Highway (SR-33): Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- Supply Row from 10th Street to 2nd Street: Provide wide sidewalks (10+ feet in width) along both sides of the street, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- 6th Street from Main Street to Front Street: Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- 4th Street from Kern Street (SR-33) to Front Street: Improve existing sidewalks and provide high-visibility crosswalks at all intersections.
- 2nd Street/Olive Avenue from Kern Street (SR-33) to Front Street: Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- Front Street from 10th Street to Oak Street: Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.

Future Bicycle Network

The Kern Region Active Transportation Plan (Alta Planning + Design, March 2018) provides recommendations for future bicycle facility improvements in the City of Taft in order of priority and are ranked as Tier 1 (highest priority), Tier 2 (medium priority) and Tier 3 (lowest priority). Below is a list of the recommended Tier 1 and Tier 2 bicycle facility improvements from the Kern Region Active Transportation Plan that are within and adjacent to the Downtown Taft Specific Plan area, in order of priority:

Tier 1 Projects (Highest Priority: 1-10 Years)

- 6th Street from Ash Street to Oak Street: Class II Buffered Bike Lanes
- 10th Street from Ash Street to Center Street: Class IV Cycle Track
- Kern Street (SR-33) from Cascade Place to 1st Street: Class II Buffered Bike Lanes
- 10th Street from Center Street to F Street: Class II Bike Lanes
- 2nd Street from Calvin Street to Supply Row: Class III Bike Boulevard



Tier 2 Projects (Medium Priority: 10-15 Years)

- 1st Street from Calvin Street to Kern Street: Class II Buffered Bike Lanes
- Westside Highway (SR-33) from 1st Street to SR-119: Class II Bike Lanes

All of the above-listed planned future bicycle facility improvements are anticipated to be completed prior to buildout of the Downtown Taft Specific Plan.

Recommended Future Bicycle Facility Improvements With Specific Plan

In addition to the above-listed planned future bicycle facility projects, the following bicycle facility improvements are recommended to enhance bicycle safety and improve access to the future land uses within the Downtown Taft Specific Plan:

- 2nd Street from Kern Street (SR-33) to Supply Row: Provide Class II bike lanes (upgrade from planned Class III Bike Boulevard in *Kern Region Active Transportation Plan*)
- 1st Street from Kern Street (SR-33) to Center Street: Provide Class II bike lanes
- Center Street from 2nd Street to West Side Highway (SR-33): Provide Class II bike lanes
- Main Street from 2nd Street to West Side Highway (SR-33): Provide Class II bike lanes
- North Street from 8th Street to 1st Street: Provide Class III Bike Boulevard
- 8th Street from Kern Street (SR-33) to Main Street: Provide Class III Bike Boulevard

Future Transit Network

Specific planned future transit network or facility improvements within or adjacent to the Downtown Taft Specific Plan area are not identified, but below are recommended future transit network and facility improvements that are consistent with transit policies in the City of Taft *General Plan Circulation Element* (June 2010, Amended April 2017).

Recommended Future Transit Network and Facility Improvements With Specific Plan

- Coordinate with Taft Area Transit (TAT) to provide benches, shelters and trash receptacles at the existing bus stops along Kern Street (SR-33).
- Coordinate with Taft Area Transit (TAT) to expand the Taft-Maricopa Route to include 10th Street between Kern Street (SR-33) and Main Street, and Main Street between 10th Street and West Side Highway (SR-33), and to install sheltered bus stops along the expanded route.
- Coordinate with Taft Area Transit (TAT) to provide all-day service and to expand weekday hours of operation to between 6:00am and 7:00pm and to provide limited weekend service for the Taft-Maricopa Route.
- Coordinate with Kern Transit to expand Route 120 to include Kern Street (SR-33) between 6th Street and 2nd Street, 2nd Street between Kern Street (SR-33) and Main Street, and Main Street between 4th Street and 2nd Street.
- Coordinate with Kern Transit to provide additional sheltered bus stops along both the existing Route 120 and the recommended expanded Route 120 within the Downtown Taft Specific Plan area.
- Coordinate with Kern Transit to expand the Route 120 weekday and Saturday hours of operation with the first eastbound bus leaving Taft at 6:00am, and with the last westbound bus arriving in Taft at 9:00pm.



CONCLUSIONS AND RECOMMENDATIONS

This Local Transportation Analysis evaluated the potential level of service (LOS) deficiencies and transportation improvements that may need to be considered in association with the traffic generated by the proposed Downtown Taft Specific Plan project. Below is a summary of the analysis findings and recommended transportation improvements:

Intersection Operations Analysis Findings

The results of the Existing Conditions intersection analysis showed that all study intersections are currently operating at LOS C or better during the AM and PM peak hours.

Under Horizon Year 2042 conditions with the buildout of the Downtown Taft Specific Plan, the intersection analysis results showed that the following intersections are forecast to operate at a deficient LOS E or F during the AM and PM peak hours:

- Kern Street (SR-33) / 6th Street (AM: LOS F; PM: LOS E)
- Kern Street (SR-33) / 4th Street (AM/PM: LOS F)
- Kern Street (SR-33) / E. Kern Street / 1st Street (AM: LOS E; PM: LOS F)

Roadway Segment Analysis Findings

The results of the Existing Conditions roadway segment operations analysis showed that all study roadway segments currently operate at LOS B or better.

Under Horizon Year 2042 conditions with the buildout of the Downtown Taft Specific Plan, the roadway segment analysis results showed that all study roadway segments are forecast to operate at LOS D or better.

Recommended Transportation Improvements

The following improvements are recommended to improve operations at the three deficient study intersections to acceptable levels of service:

Kern Street (SR-33) / 6th Street

- Install traffic signal
- Restripe eastbound and westbound approaches to provide one left-turn lane, one through lane, and one shared through/right-turn lane
- Restripe northbound and southbound approaches to provide one left-turn lane, one through lane, and one right-turn lane
- Coordinate with Caltrans to determine the signal interconnect requirements between the existing signal at 10th Street and the proposed signals at 6th Street and 4th Street



Kern Street (SR-33) / 4th Street

- Install traffic signal
- Restripe eastbound and westbound approaches to provide one left-turn lane, one through lane, and one shared through/right-turn lane
- Restripe northbound and southbound approaches to provide one left-turn lane and one shared through/right-turn lane
- Coordinate with Caltrans to determine the signal interconnect requirements between the existing signal at 10th Street and the proposed signals at 6th Street and 4th Street

Kern Street (SR-33) / E. Kern Street / 1st Street

• Install stop signs on the eastbound (Kern Street/SR-33) and northwestbound (SR-33) approaches to convert intersection to all-way stop control.

The recommended transportation improvements as described above are also illustrated graphically in **Exhibit 8**.

The Peak Hour Warrant (Warrant 3) from the *California Manual on Uniform Traffic Devices* (CA-MUTCD, 2014 Edition, Revision 6, March 30, 2021) was conducted at the Kern Street (SR-33) / 6th Street and Kern Street (SR-33) / 4th Street intersections to determine if installation of a traffic signal would be justified under Horizon Year 2042 conditions with the buildout of the Downtown Taft Specific Plan as recommended.

The findings showed that the CA MUTCD Peak Hour Warrant (Warrant 3) is <u>Satisfied</u> at both the Kern Street (SR-33) / 6th Street and Kern Street (SR-33) / 4th Street intersections during the AM and PM peak hours. The CA MUTCD Peak Hour Warrant (Warrant 3) worksheets are provided in **Appendix G**.

The recommended installation of traffic signals at the Kern Street (SR-33) / 6th Street and Kern Street (SR-33) / 4th Street intersections would be subject to Caltrans approval as they would be operated and maintained by Caltrans. It is anticipated that the two proposed signals at the Kern Street (SR-33) / 6th Street and Kern Street (SR-33) / 4th Street intersections would need to be coordinated due to the distance between the two intersections (approximately 925 feet). Caltrans may also require the two intersections to be interconnected to the existing signal at the Kern Street (SR-33) / 10th Street intersection, which is located approximately 1,750 feet (1/3 mile) west of 6th Street.

The guidance for application of multi-way stop control in Section 2B.07 of the CA MUTCD (2014 Edition, Revision 6, March 30, 2021) was reviewed to determine if installation of all-way stop control at the Kern Street (SR-33) / E. Kern Street / 1st Street intersection would be justified based on the future Horizon Year 2042 traffic volumes with the buildout of the Downtown Taft Specific Plan. The findings show that the Horizon Year 2042 approach volumes at the Kern Street (SR-33) / E. Kern Street / 1st Street intersection during the peak hours exceed the CA MUTCD multi-stop control criteria based on minimum volumes and delay. The CA MUTCD multi-way stop evaluation worksheet is also provided in Appendix G.

Table 9 shows the operational analysis results for the three deficient study intersections with the recommended improvements as previously described. **Appendix H** contains the Horizon Year 2042 With Project intersection analysis worksheets with the recommended improvements.

As shown in Table 9, the recommended improvements are anticipated to improve operations to LOS D or better during both the AM and PM peak hours.

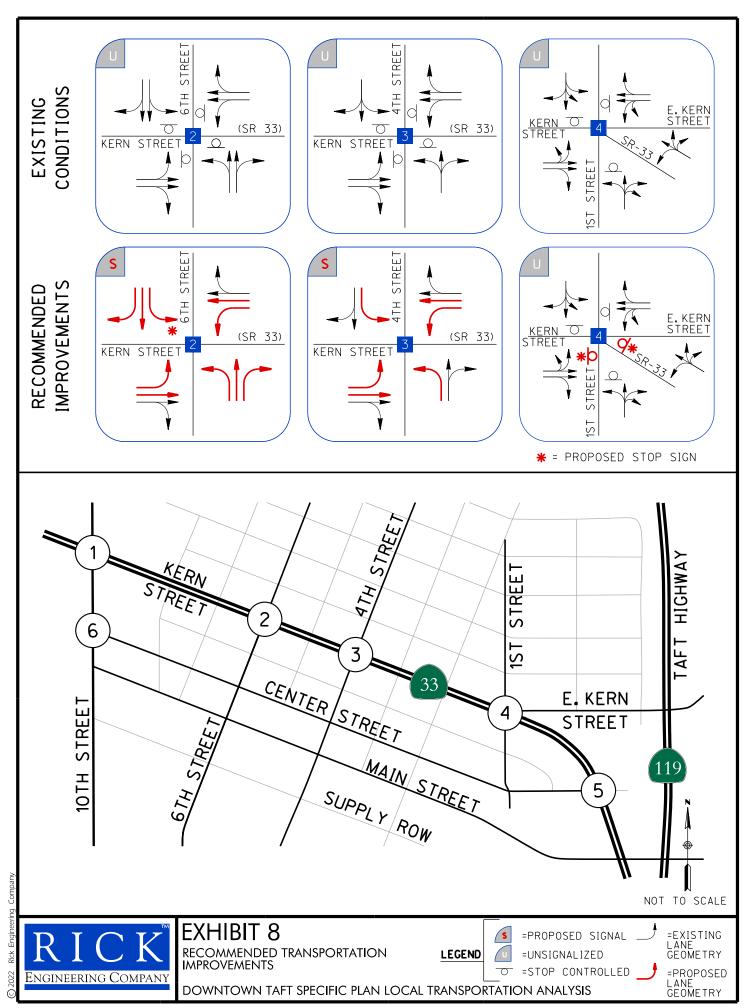




TABLE 9 HORIZON YEAR 2042 WITH PROJECT INTERSECTION OPERATIONS WITH RECOMMENDED IMPROVEMENTS

	HORIZON YR (2042) WITH PROJECT						
INTERSECTION	WITHOUT IMPROVEMENTS			WITH IMPROVEMENTS			
	CONTROL	DELAY ¹	LOS	CONTROL	DELAY ¹	LOS	
2 Kern Street (SR-33) / 6 th Street							
AM peak	(AWSC)			(S)			
	OVERALL	117.7	F	OVERALL	34.2	C	
PM peak	(AWSC)			(S)			
	OVERALL	42.9	E	OVERALL	30.2	C	
3 Kern Street (SR-33) / 4th Street							
AM peak	(AWSC)			(S)			
_	OVERALL	118.8	F	OVERALL	35.8	D	
PM peak	(AWSC)			(S)			
	OVERALL	55.4	F	OVERALL	31.1	C	
4 Kern Street (SR-33) / E. Kern Street/ 1st Street							
AM peak	(TWSC)			(AWSC)			
	NBT	47.6	E	OVERALL	12.3	В	
PM peak	(TWSC)			(AWSC)			
	NBT	67.2	F	OVERALL	13.9	В	

Footnotes: S = Signalized; AWSC = All-Way-Stop Controlled; TWSC = Two-Way-Stop Controlled

¹Delay is measured in seconds per vehicle.

Active Transportation and Transit Assessment Recommendations

Based on the findings of the assessment of existing and planned future pedestrian, bicycle and transit facilities, the following pedestrian, bicycle and transit network and facility improvements are recommended to be implemented within the Downtown Taft Specific Plan area:

Recommended Pedestrian Facility Improvements

- North Street from 8th Street to 1st Street: Improve existing sidewalks and provide high-visibility crosswalks at all intersections.
- Center Street from 2nd Street to Westside Highway (SR-33): Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- Main Street from 10th Street to 2nd Street: Close the existing sidewalk gaps and provide wide sidewalks (10+ feet in width) along both sides of the street. Provide high-visibility crosswalks at all intersections.
- Main Street from 2nd Street to Westside Highway (SR-33): Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- Supply Row from 10th Street to 2nd Street: Provide wide sidewalks (10+ feet in width) along both sides of the street, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.



- 6th Street from Main Street to Front Street: Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- 4th Street from Kern Street (SR-33) to Front Street: Improve existing sidewalks and provide high-visibility crosswalks at all intersections.
- 2nd Street/Olive Avenue from Kern Street (SR-33) to Front Street: Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.
- Front Street from 10th Street to Oak Street: Improve existing sidewalks, close the existing sidewalk gaps, and provide high-visibility crosswalks at all intersections.

Recommended Bicycle Facility Improvements

- 2nd Street from Kern Street (SR-33) to Supply Row: Provide Class II bike lanes (upgrade from planned Class III Bike Boulevard in *Kern Region Active Transportation Plan*)
- 1st Street from Kern Street (SR-33) to Center Street: Provide Class II bike lanes
- Center Street from 2nd Street to West Side Highway (SR-33): Provide Class II bike lanes
- Main Street from 2nd Street to West Side Highway (SR-33): Provide Class II bike lanes
- North Street from 8th Street to 1st Street: Provide Class III Bike Boulevard
- 8th Street from Kern Street (SR-33) to Main Street: Provide Class III Bike Boulevard

Recommended Future Transit Network and Facility Improvements

- Coordinate with Taft Area Transit (TAT) to provide benches, shelters and trash receptacles at the existing bus stops along Kern Street (SR-33).
- Coordinate with Taft Area Transit (TAT) to expand the Taft-Maricopa Route to include 10th
 Street between Kern Street (SR-33) and Main Street, and Main Street between 10th Street and
 West Side Highway (SR-33), and to install sheltered bus stops along the expanded route.
- Coordinate with Taft Area Transit (TAT) to provide all-day service and to expand weekday hours
 of operation to between 6:00am and 7:00pm and to provide limited weekend service for the TaftMaricopa Route.
- Coordinate with Kern Transit to expand Route 120 to include Kern Street (SR-33) between 6th Street and 2nd Street, 2nd Street between Kern Street (SR-33) and Main Street, and Main Street between 4th Street and 2nd Street.
- Coordinate with Kern Transit to provide additional sheltered bus stops along both the existing Route 120 and the recommended expanded Route 120 within the Downtown Taft Specific Plan area.
- Coordinate with Kern Transit to expand the Route 120 weekday and Saturday hours of operation with the first eastbound bus leaving Taft at 6:00am, and with the last westbound bus arriving in Taft at 9:00pm.



REFERENCES

- 1. Kern County, Standards for Traffic Engineering, February 2010.
- 2. City of Taft, General Plan Circulation Element, June 2010, Amended April 2017.
- 3. City of Taft, General Plan Draft Environmental Impact Report, July 2009.
- 4. Kern Council of Governments, 2018 Regional Transportation Plan, August 2018.
- 5. State of California Office of Planning and Research (OPR), <u>Technical Advisory on Evaluating Transportation Impacts in CEQA</u>, December 2018.
- 6. Institute of Transportation Engineers (ITE), <u>Trip Generation Manual</u>, 11th Edition, September 2021.
- 7. Transportation Research Board, Highway Capacity Manual 6th Edition, Washington, D.C., 2016.
- 8. Trafficware LLC, Synchro, Version 11, Sugar Land, Texas, 2021.
- 9. Alta Planning + Design, Kern Region Active Transportation Plan, March 2018.
- 10. California Department of Transportation, <u>California Manual on Uniform Traffic Control Devices</u> (MUTCD), 2014 Edition, Revision 6, March 2021.

APPENDIX A

VEHICLE, PEDESTRIAN AND BICYCLE COUNTS



310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

Turning Movement Report

Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

 LOCATION
 Kern St @ 10th St
 LATITUDE
 35.1449

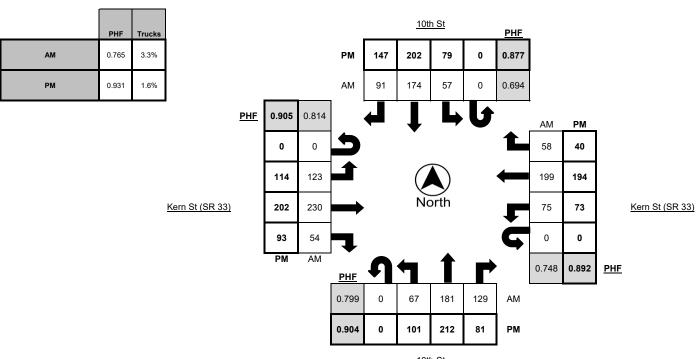
 COUNTY
 Kern
 LONGITUDE
 -119.4652

 COLLECTION DATE
 Tuesday, March 15, 2022
 WEATHER
 Clear

		١	Northbour	ıd			S	outhbour	nd			I	Eastboun	d			1	Westboun	d	
Time	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
7:00 AM - 7:15 AM	0	13	33	15	2	0	18	17	22	3	0	18	42	17	6	0	8	26	7	3
7:15 AM - 7:30 AM	0	10	46	51	3	0	13	30	21	2	0	33	62	12	5	0	13	42	13	0
7:30 AM - 7:45 AM	0	22	57	39	7	0	21	62	33	3	0	42	66	17	5	0	31	60	20	3
7:45 AM - 8:00 AM	0	16	45	30	1	0	13	50	24	0	0	28	53	14	2	0	22	57	15	1
8:00 AM - 8:15 AM	0	19	33	9	2	0	10	32	13	1	0	20	49	11	7	0	9	40	10	6
8:15 AM - 8:30 AM	0	15	35	14	4	0	9	31	23	4	0	16	36	6	3	0	7	24	11	5
8:30 AM - 8:45 AM	0	11	26	12	2	0	3	22	22	3	0	21	33	13	5	0	8	30	4	2
8:45 AM - 9:00 AM	0	13	22	13	1	0	9	21	20	0	0	20	42	8	3	0	8	36	10	1
TOTAL	0	119	297	183	22	0	96	265	178	16	0	198	383	98	36	0	106	315	90	21

		N	lorthboun	d			S	outhbour	d			1	Eastboun	d			١	Vestboun	d	
Time	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
4:00 PM - 4:15 PM	0	22	41	25	1	0	16	49	30	1	0	33	47	24	3	0	20	55	5	3
4:15 PM - 4:30 PM	0	25	47	20	2	0	14	50	38	0	0	31	39	19	5	0	20	46	11	1
4:30 PM - 4:45 PM	0	20	39	18	0	0	19	48	31	1	0	36	50	31	3	0	20	55	10	0
4:45 PM - 5:00 PM	0	33	47	26	1	0	18	40	33	1	0	34	56	23	3	0	16	38	9	3
5:00 PM - 5:15 PM	0	22	65	22	1	0	17	62	43	3	0	25	50	21	3	0	24	53	9	0
5:15 PM - 5:30 PM	0	25	45	15	2	0	20	52	31	1	0	29	48	25	2	0	17	47	13	0
5:30 PM - 5:45 PM	0	21	55	18	0	0	24	48	40	0	0	26	48	24	3	0	16	56	9	2
5:45 PM - 6:00 PM	0	25	36	27	1	1	15	47	41	0	0	29	50	19	0	0	22	43	15	1
TOTAL	0	193	375	171	8	1	143	396	287	7	0	243	388	186	22	0	155	393	81	10

		N	orthboun	ıd			S	outhbour	ıd				Eastbound	d			'	Vestboun	d	
PEAK HOUR	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
7:15 AM - 8:15 AM	0	67	181	129	13	0	57	174	91	6	0	123	230	54	19	0	75	199	58	10
4:45 PM - 5:45 PM	0	101	212	81	4	0	79	202	147	5	0	114	202	93	11	0	73	194	40	5



10th St



310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

Turning Movement Report

Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

 LOCATION
 Kern St @ 10th St
 LATITUDE
 35.1449

 COUNTY
 Kern
 LONGITUDE
 -119.4652

 COLLECTION DATE
 Tuesday, March 15, 2022
 WEATHER
 Clear

	Nort	hbound E	Bikes	N.Leg	Sout	thbound E	Bikes	S.Leg	Eas	tbound B	ikes	E.Leg	Wes	tbound B	ikes	W.Leg
Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
7:00 AM - 7:15 AM	0	0	0	1	0	0	0	0	0	0	0	3	0	0	0	1
7:15 AM - 7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
7:30 AM - 7:45 AM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	1
7:45 AM - 8:00 AM	0	0	0	3	0	0	0	0	0	0	0	2	0	0	0	0
8:00 AM - 8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM - 8:30 AM	0	0	0	0	0	0	0	1	1	0	1	1	0	0	0	0
8:30 AM - 8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
8:45 AM - 9:00 AM	0	0	0	2	0	0	0	1	0	0	0	1	0	0	0	1
TOTAL	0	0	0	6	0	0	0	3	1	0	1	8	0	0	0	6

	Nort	hbound E	Bikes	N.Leg	Sout	thbound E	Bikes	S.Leg	Eas	tbound B	ikes	E.Leg	Wes	tbound B	ikes	W.Leg
Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
4:00 PM - 4:15 PM	0	0	0	0	0	0	0	1	0	0	0	2	0	0	0	1
4:15 PM - 4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
4:30 PM - 4:45 PM	0	0	0	2	0	0	0	0	0	0	0	9	0	0	0	1
4:45 PM - 5:00 PM	0	0	0	3	0	0	0	0	0	0	0	1	0	0	0	4
5:00 PM - 5:15 PM	0	0	0	1	0	0	0	3	0	0	0	2	0	1	0	4
5:15 PM - 5:30 PM	0	0	0	1	0	0	0	2	0	0	0	0	0	0	0	4
5:30 PM - 5:45 PM	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	1
5:45 PM - 6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
TOTAL	0	0	0	8	0	0	0	6	0	0	0	15	0	1	0	18

	Nort	thbound E	Bikes	N.Leg	Sout	hbound E	Bikes	S.Leg	Eas	tbound Bi	ikes	E.Leg	Wes	tbound B	ikes	W.Leg
PEAK HOUR	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
7:15 AM - 8:15 AM	0	0	0	3	0	0	0	1	0	0	0	3	0	0	0	3
4:45 PM - 5:45 PM	0	0	0	6	0	0	0	5	0	0	0	4	0	1	0	13

	Bikes	Peds						10th St		Peds <>			
AM Peak Total	0	10				PM	0	0	0	6			
PM Peak Total	1	28				AM	0	0	0	3			
			Peds <>	13	3		4	1	L		AM	PM	_
				0	0	1				L	0	0	
	Ker	n St (SR	<u>(33)</u>	0	0	\rightarrow	•) .	←	0	1	Kern St (SR 33)
				0	0	7		North	ľ	F	0	0	
			!	PM	AM	Peds <>	4	1		,	3	4	Peds <>
						1	0	0	0	АМ			4 —
						5	0	0	0	РМ			

10th St

Page 2 of 3



310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

Turning Movement Report

Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

LOCATION	Kern St @ 6th St	LATITUDE	35.1431
COUNTY	Kern	LONGITUDE	-119.4596
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear

		١	Northboun	ıd			S	outhbour	ıd				Eastboun	d			,	Westboun	d	
Time	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
7:00 AM - 7:15 AM	0	1	18	1	0	0	3	8	6	0	0	9	45	0	2	0	1	29	5	2
7:15 AM - 7:30 AM	0	5	36	4	0	0	11	18	17	0	0	18	52	1	3	0	4	57	12	1
7:30 AM - 7:45 AM	0	6	59	9	0	0	9	55	27	0	0	23	60	6	4	0	3	87	15	5
7:45 AM - 8:00 AM	0	2	27	4	2	0	12	34	18	2	0	23	68	1	2	0	7	69	9	1
8:00 AM - 8:15 AM	0	4	13	4	1	0	11	21	12	1	0	15	47	4	6	0	2	44	3	5
8:15 AM - 8:30 AM	0	4	9	3	0	0	2	14	4	0	0	7	50	4	3	0	2	40	3	7
8:30 AM - 8:45 AM	0	3	9	3	0	0	7	9	9	0	0	7	35	1	3	0	2	31	7	0
8:45 AM - 9:00 AM	0	5	8	2	0	0	11	11	10	0	0	11	40	3	2	0	4	41	5	1
TOTAL	0	30	179	30	3	0	66	170	103	3	0	113	397	20	25	0	25	398	59	22

		N	lorthboun	d			S	outhbour	d			1	Eastboun	d			١	Vestboun	d	
Time	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
4:00 PM - 4:15 PM	0	8	19	5	0	0	13	35	14	1	0	16	72	5	4	0	11	66	14	4
4:15 PM - 4:30 PM	0	5	8	7	1	0	19	25	16	0	0	11	58	1	1	0	4	55	11	1
4:30 PM - 4:45 PM	0	6	20	11	1	0	13	20	19	0	0	22	57	2	1	0	1	54	10	1
4:45 PM - 5:00 PM	0	8	15	4	1	0	7	21	17	0	0	21	69	6	3	0	4	56	5	0
5:00 PM - 5:15 PM	0	9	17	5	0	0	8	16	23	0	0	26	58	6	1	0	5	53	13	0
5:15 PM - 5:30 PM	0	5	22	5	2	0	12	26	22	0	0	17	54	2	3	0	9	57	9	1
5:30 PM - 5:45 PM	0	5	18	10	1	0	11	44	17	1	0	21	72	3	2	0	5	61	12	2
5:45 PM - 6:00 PM	0	3	22	8	0	0	6	31	17	0	0	14	61	6	1	0	8	53	8	1
TOTAL	0	49	141	55	6	0	89	218	145	2	0	148	501	31	16	0	47	455	82	10

		N	lorthboun	d			S	outhbour	ıd			E	Eastboun	t			١	Vestboun	d	
PEAK HOUR	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
7:15 AM - 8:15 AM	0	17	135	21	3	0	43	128	74	3	0	79	227	12	15	0	16	257	39	12
5:00 PM - 6:00 PM	0	22	79	28	3	0	37	117	79	1	0	78	245	17	7	0	27	224	42	4

	PHF	Trucks							<u>6th</u>	n St		<u>PHF</u>				
АМ	0.730	3.1%					PM	79	117	37	0	0.809				
PM	0.892	1.5%					AM	74	128	43	0	0.673				
			•	PHF	0.885	0.864		4	1	L	b	•	AM	PM		
					0	0	2		•			1	39	42		
					78	79							257	224		
			Kern St (SR 33)		245	227	\rightarrow		No	orth		T	16	27		Kern St (SR 33)
					17	12	7					Ġ	0	0		
					PM	AM	PHF	A	4	1	P	•	0.743	0.939	<u>PHF</u>	
							0.584	0	17	135	21	AM				
							0.977	0	22	79	28	РМ				
									<u>6th</u>	n St	1					



310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

Turning Movement Report

Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

LOCATION	Kern St @ 6th St	LATITUDE	35.1431	
COUNTY	Kern	LONGITUDE	-119.4596	
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear	

	Nort	hbound E	Bikes	N.Leg	Sout	thbound E	Bikes	S.Leg	Eas	tbound B	ikes	E.Leg	Wes	tbound B	ikes	W.Leg
Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
7:00 AM - 7:15 AM	0	0	0	1	0	0	0	0	0	0	0	7	0	0	0	0
7:15 AM - 7:30 AM	0	0	0	0	0	0	0	0	0	0	0	14	0	0	0	8
7:30 AM - 7:45 AM	0	0	0	1	0	0	0	0	0	0	0	2	0	0	0	0
7:45 AM - 8:00 AM	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0
8:00 AM - 8:15 AM	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	1
8:15 AM - 8:30 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0
8:30 AM - 8:45 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0
8:45 AM - 9:00 AM	0	0	0	1	0	1	0	1	0	0	0	2	0	0	0	0
TOTAL	0	0	0	5	0	1	0	2	0	0	0	29	0	0	0	9

	Nort	hbound E	Bikes	N.Leg	Sout	thbound E	Bikes	S.Leg	Eas	tbound B	ikes	E.Leg	Wes	tbound B	ikes	W.Leg
Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
4:00 PM - 4:15 PM	0	0	0	1	0	0	0	0	0	0	0	3	0	0	0	2
4:15 PM - 4:30 PM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0
4:30 PM - 4:45 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM - 5:00 PM	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0
5:00 PM - 5:15 PM	0	0	0	2	0	0	1	2	0	0	0	6	0	0	0	1
5:15 PM - 5:30 PM	0	1	0	1	0	0	0	1	0	0	0	2	0	0	0	0
5:30 PM - 5:45 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
5:45 PM - 6:00 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1
TOTAL	1	1	0	5	0	0	1	4	0	0	0	15	0	0	0	6

	Nort	thbound E	Bikes	N.Leg	Sout	hbound E	Bikes	S.Leg	Eas	tbound Bi	ikes	E.Leg	Wes	tbound B	ikes	W.Leg
PEAK HOUR	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
7:15 AM - 8:15 AM	0	0	0	3	0	0	0	0	0	0	0	19	0	0	0	9
5:00 PM - 6:00 PM	1	1	0	3	0	0	1	3	0	0	0	9	0	0	0	4

	Bikes	Peds						6th St		Peds <>	_			
AM Peak Total	0	31				PM	1	0	0	3				
PM Peak Total	3	19				AM	0	0	0	3				
			Peds <>	4	9		4	1	L	,	AM	PM		
				0	0			•		L	0	0		
	Ker	n St (SR	<u>(33)</u>	0	0	\rightarrow	•) .	←	0	0		Kern St (SR 33)
				0	0	1		North	ľ	L	0	0		
				PM	AM	Peds <>	4	1	P		19	9	Peds <>	
						0	0	0	0	AM			-	
						3	1	1	0	PM				

6th St

Page 2 of 3



310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

Turning Movement Report

Prepared For:

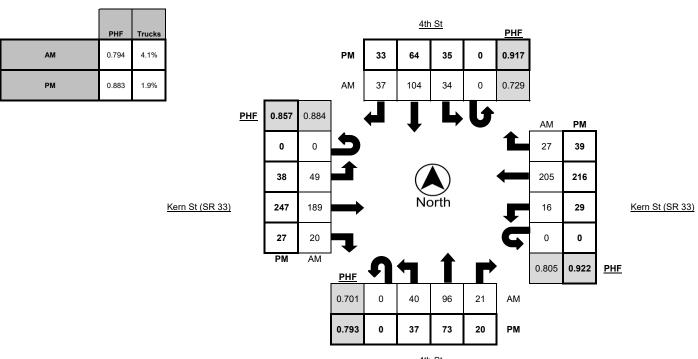
Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

LOCATION	Kern St @ 4th St	LATITUDE	35.1422	
COUNTY	Kern	LONGITUDE	-119.4567	
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear	

		N	lorthboun	d			S	outhbour	ıd				Eastboun	t			١	Vestboun	d	
Time	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
7:00 AM - 7:15 AM	0	1	10	2	1	0	3	6	7	0	0	5	36	2	3	0	3	30	3	2
7:15 AM - 7:30 AM	0	6	22	3	1	0	5	20	8	0	0	14	41	4	4	0	3	54	2	1
7:30 AM - 7:45 AM	0	17	32	7	1	0	11	38	11	3	0	11	57	3	2	0	7	60	10	4
7:45 AM - 8:00 AM	0	9	29	8	1	0	10	33	14	0	0	15	50	8	3	0	3	49	13	1
8:00 AM - 8:15 AM	0	8	13	3	0	0	8	13	4	0	0	9	41	5	7	0	3	42	2	6
8:15 AM - 8:30 AM	0	4	9	2	1	0	8	8	7	3	1	5	46	5	2	0	2	35	2	7
8:30 AM - 8:45 AM	0	1	9	4	2	0	5	4	3	1	0	2	32	7	4	0	3	40	1	0
8:45 AM - 9:00 AM	0	3	6	2	0	0	2	14	7	1	0	13	38	2	1	0	5	34	6	2
TOTAL	0	49	130	31	7	0	52	136	61	8	1	74	341	36	26	0	29	344	39	23

		N	lorthboun	d			S	outhbour	ıd			1	Eastbound	t			١	Nestboun	d	
Time	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
4:00 PM - 4:15 PM	0	16	17	8	0	0	5	22	7	0	0	12	69	10	5	0	4	61	12	4
4:15 PM - 4:30 PM	0	6	25	5	1	0	7	14	9	0	0	5	61	9	1	0	8	54	10	1
4:30 PM - 4:45 PM	0	9	18	5	0	0	13	14	5	0	0	10	56	5	1	0	8	54	7	1
4:45 PM - 5:00 PM	0	6	13	2	0	0	10	14	12	0	0	11	61	3	2	0	9	47	10	0
5:00 PM - 5:15 PM	0	8	24	5	0	0	3	17	13	0	0	10	51	5	1	0	8	41	7	1
5:15 PM - 5:30 PM	1	12	12	7	0	0	6	13	11	0	0	9	53	5	3	0	7	54	8	0
5:30 PM - 5:45 PM	1	13	24	7	0	0	6	18	8	1	0	9	62	7	3	0	10	56	8	3
5:45 PM - 6:00 PM	0	7	21	7	0	0	4	20	10	0	0	9	63	2	1	0	0	46	9	1
TOTAL	2	77	154	46	1	0	54	132	75	1	0	75	476	46	17	0	54	413	71	11

		١	orthboun	ıd			S	Southbour	nd				Eastboun	t			١	Vestboun	d	
PEAK HOUR	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
7:15 AM - 8:15 AM	0	40	96	21	3	0	34	104	37	3	0	49	189	20	16	0	16	205	27	12
4:00 PM - 5:00 PM	0	37	73	20	1	0	35	64	33	0	0	38	247	27	9	0	29	216	39	6



4th St



310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

Turning Movement Report

Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

LOCATION	Kern St @ 4th St	LATITUDE_	35.1422
COUNTY_	Kern	LONGITUDE_	-119.4567
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER_	Clear

	Nort	thbound E	Bikes	N.Leg	Sout	thbound E	Bikes	S.Leg	Eas	tbound B	ikes	E.Leg	Wes	tbound B	ikes	W.Leg
Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
7:00 AM - 7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
7:15 AM - 7:30 AM	0	0	0	1	0	0	0	2	0	0	0	2	0	0	0	1
7:30 AM - 7:45 AM	0	0	0	1	0	0	0	0	0	0	0	6	0	0	0	0
7:45 AM - 8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM - 8:15 AM	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	1
8:15 AM - 8:30 AM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0
8:30 AM - 8:45 AM	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM - 9:00 AM	0	0	0	0	0	0	0	1	0	0	0	3	0	0	0	0
TOTAL	0	0	0	5	0	0	0	4	0	0	0	13	0	0	0	3

	Nort	thbound E	Bikes	N.Leg	Sout	thbound E	Bikes	S.Leg	Eas	tbound B	ikes	E.Leg	Wes	tbound B	ikes	W.Leg
Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
4:00 PM - 4:15 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0
4:15 PM - 4:30 PM	0	0	0	2	0	0	0	0	0	0	0	0	0	1	0	1
4:30 PM - 4:45 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0
4:45 PM - 5:00 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0
5:00 PM - 5:15 PM	0	0	1	1	0	0	0	3	0	0	0	2	0	0	0	2
5:15 PM - 5:30 PM	0	1	0	1	0	0	0	0	0	0	0	1	0	0	0	0
5:30 PM - 5:45 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0
5:45 PM - 6:00 PM	0	0	0	3	0	0	0	2	0	0	0	1	0	0	0	0
TOTAL	0	1	1	7	0	0	0	8	0	0	0	5	0	1	0	3

	Nort	thbound E	Bikes	N.Leg	Sout	hbound E	Bikes	S.Leg	Eas	tbound Bi	ikes	E.Leg	Wes	tbound B	ikes	W.Leg
PEAK HOUR	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
7:15 AM - 8:15 AM	0	0	0	3	0	0	0	2	0	0	0	9	0	0	0	2
4:00 PM - 5:00 PM	0	0	0	2	0	0	0	2	0	0	0	1	0	1	0	1

	Bikes	Peds						4th St		Peds <>				
AM Peak Total	0	16				PM	0	0	0	2				
PM Peak Total	1	6				AM	0	0	0	3				
			Peds <>	1	2		4	1	L	,	AM	PM		
				0	0			•		L	0	0		
	Ker	n St (SF	R 33)	0	0	\rightarrow	•) .	←	0	1		Kern St (SR 33)
				0	0			North	ľ	L	0	0		
			'	PM	AM	Peds <>	4	1		•	9	1	Peds <>	
						2	0	0	0	AM			-	
						2	0	0	0	РМ				

4th St

Page 2 of 3



Metro Traffic Data Inc. 310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

Turning Movement Report

Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

LOCATION	Kern St @ 1st	LATITUDE_	35.1407
COUNTY	Kern	LONGITUDE	-119.4519
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER_	Clear

			- 1	lorthboun	t			S	outhboun	ıd				astbound	t			Wes	tbound (S	R33)			West	bound (Ke	ern St)	
Time		Left	Thru	R-Kern	R-33	Trucks	L-Kern	L-33	Thru	Right	Trucks	L-1st	L-Kern	Thru	Right	Trucks	Left	Thru	R-1st	R-Kern	Trucks	L-33	L-1st	Thru	Right	Trucks
7:00 AM - 7:	:15 AM	0	0	0	0	0	0	4	0	3	0	1	12	17	0	2	0	15	0	0	1	0	0	15	1	1
7:15 AM - 7:	:30 AM	1	0	2	2	0	0	2	0	7	0	4	15	18	5	3	0	19	0	0	2	0	2	24	2	1
7:30 AM - 7:	:45 AM	3	0	1	0	0	0	0	0	7	0	10	24	25	4	2	1	18	1	0	2	0	0	33	2	2
7:45 AM - 8:	:00 AM	1	0	1	0	0	4	2	1	4	0	1	26	24	7	1	0	9	0	0	0	0	3	30	0	0
8:00 AM - 8:	:15 AM	3	0	1	0	0	0	2	1	5	1	5	19	18	7	3	0	22	0	2	1	0	2	15	2	1
8:15 AM - 8:	:30 AM	4	1	1	0	0	0	0	0	4	0	1	16	34	2	3	0	19	0	0	6	0	0	10	1	0
8:30 AM - 8:	:45 AM	3	0	1	1	0	1	2	2	3	0	2	10	22	3	6	1	21	2	0	0	0	0	17	0	0
8:45 AM - 9:	:00 AM	2	0	1	1	0	0	4	0	3	0	3	21	13	4	1	1	12	1	0	2	1	0	11	1	1
TOTAL	L	17	1	8	4	0	5	16	4	36	1	27	143	171	32	21	3	135	4	2	14	1	7	155	9	6

		1	Northboun	d			S	outhbour	ıd				astbound	d			Wes	tbound (S	R33)			West	bound (Ke	ern St)	
Time	Left	Thru	R-Kern	R-33	Trucks	L-Kern	L-33	Thru	Right	Trucks	L-1st	L-Kern	Thru	Right	Trucks	Left	Thru	R-1st	R-Kern	Trucks	L-33	L-1st	Thru	Right	Trucks
4:00 PM - 4:15 PM	2	0	1	0	0	0	0	1	3	0	1	36	22	5	1	0	40	4	1	4	0	0	18	0	0
4:15 PM - 4:30 PM	4	2	1	0	0	1	1	0	4	0	2	34	27	2	3	2	27	2	0	1	0	1	25	0	0
4:30 PM - 4:45 PM	2	0	0	0	0	0	3	1	5	0	6	30	16	1	2	0	29	2	0	1	0	0	17	2	0
4:45 PM - 5:00 PM	1	0	2	0	0	1	1	1	5	0	2	39	19	2	1	0	25	3	0	0	0	1	20	2	1
5:00 PM - 5:15 PM	1	1	3	0	0	1	2	1	5	1	5	30	20	4	1	0	22	2	0	0	0	2	26	1	0
5:15 PM - 5:30 PM	3	1	1	0	0	3	1	0	6	1	5	28	15	2	3	0	20	1	0	0	0	3	21	0	0
5:30 PM - 5:45 PM	2	1	0	0	0	0	0	0	5	0	2	29	29	4	3	0	23	1	0	2	0	1	28	1	1
5:45 PM - 6:00 PM	0	0	0	0	0	0	1	1	7	0	5	34	24	4	1	0	24	2	0	1	0	1	17	1	0
TOTAL	15	5	8	0	0	6	9	5	40	2	28	260	172	24	15	2	210	17	1	9	0	9	172	7	2

			Northboun	d			S	outhboun	d				astbound	t			West	tbound (S	R33)			West	bound (Ke	ern St)	
PEAK HOUR	Left	Thru	R-Kern	R-33	Trucks	L-Kern	L-33	Thru	Right	Trucks	L-1st	L-Kern	Thru	Right	Trucks	Left	Thru	R-1st	R-Kern	Trucks	L-33	L-1st	Thru	Right	Trucks
7:15 AM - 8:15 AM	8	0	5	2	0	4	6	2	23	1	20	84	85	23	9	1	68	1	2	5	0	7	102	6	4
4:00 PM - 5:00 PM	9	2	4	0	0	2	5	3	17	0	11	139	84	10	7	2	121	11	1	6	0	2	80	4	1

			1	1	1			1	1				1	1	1							
	PHF	Trucks										<u>1st</u>	t St		PHF							
АМ	0.870	4.2%								PM	17	3	5	2	0.75							
PM	0.939	2.8%								АМ	23	2	6	4	0.795			AM	PM			
							PHF	0.938	0.841		Į	1	L	L		1	4	6	4			
								11	20			•	7			+		102	80			
								139	84		•					1		5	6	<u>Kern St</u>		
				Kern St	(SR 33)	<u>)</u>		84	85	\rightarrow	•					•	4	2	1			
								10	23	7							(0.821	0.875	<u>PHF</u>		
								PM	AM				N	orth				AM	PM	9		
																•	<	2	1			
																1		1	11			
																←		68	121	Westside	Hwy (SR 3	<u>33)</u>
																1		1	2			
											PHF	4	1	P	~			0.75	0.75	<u>PHF</u>		
											0.75	8	0	5	2	AM				•		
											0.536	9	2	4	0	PM						
													1st St	-	-	1						



Metro Traffic Data Inc. 310 N. Irwin Street - Suite 20 Hanford, CA 93230

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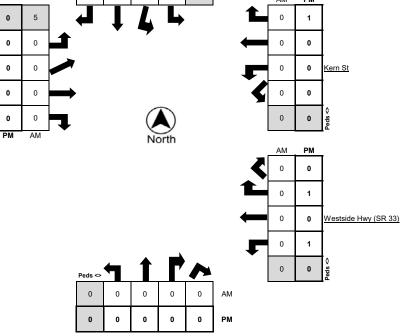
Turning Movement Report

Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

LC	CATION				K	Cern St @ 1	lst						LA	ATITUDE					35.1407						
	COUNTY					Kern							LON	IGITUDE					-119.4519)					
COLLECTIO	ON DATE				Tuesda	ay, March 1	5, 2022				-		w	EATHER					Clear						
			nd Bicycle			Southbour					d Bicycles				SR33 Bicy			stbound I				edestrian			
Time	Left	Thru		Right-2		Left-2	Thru	Right	Left-1	Left-2	Thru	Right	Left		Right-1		Left-1	Left-2	Thru	Right	S-Leg	N-Leg		E33-Leg	
7:00 AM - 7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM - 7:30 AM 7:30 AM - 7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	2	0	0
7:45 AM - 8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM - 8:15 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	3	0	0
8:15 AM - 8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM - 8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM - 9:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0
TOTAL	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	2	5	0	0
			•	•	•	•	•			•	•			•	•			•		•					
	1	Northbour	nd Bicycle			Southbour	nd Bicycle	s		Eastboun	d Bicycles	3	We	stbound \$	SR33 Bicy		We	stbound I	Kern Bicy	cles	P	Pedestrian	s in each	Crosswal	
Time	Left	Thru		Right-2	Left-1	Left-2	Thru	Right	Left-1	Left-2	Thru	Right	Left	Thru	Right-1	Right-2	Left-1	Left-2	Thru	Right	S-Leg	N-Leg	W-Leg	E33-Leg	
4:00 PM - 4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0	0	0	0	0
4:15 PM - 4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0
4:30 PM - 4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0
4:45 PM - 5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM - 5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM - 5:30 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM - 5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2
5:45 PM - 6:00 PM TOTAL	0 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0 1	0	0	0	0	0	0	4	2 2	0 2	0
IUIAL	U	U	U	0	U	U	U	0	U	1	0	U	1	U	1	U	0	0	U	1	U	4	2	2	2
		المعطمانية والمعاددة	nd Bicycle			Southbour	ad Diamala	•		Faathaun	d Bicycles		Ma	athauad (SR33 Bicy	alaa	18/0	stbound I	/aum Diam	alaa		Pedestrian	a in aaah	Creesural	le .
PEAK HOUR	Left	Thru		Right-2		Left-2	Thru	Right	Left-1	Left-2	Thru	Right	Left	Thru	Right-1		Left-1	Left-2	Thru	Right	S-Leg	N-Leg		E33-Leg	
				Ť	Loit-i		111114			Loit-2		_			Ť	_					Ť				Ť
7:15 AM - 8:15 AM	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	5	0	0
4:00 PM - 5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1	0	1	0	0	0
												<u>1st</u>	St												
	Bikes	Peds													Peds <>										
AM Peak Total	2	7								РМ	0	0	0	0	1										
			l																						
PM Peak Total	3	1	Ī							AM	0	1	0	1	2										
			Į .												_		_	AM	PM						

Kern St (SR 33)





310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

Turning Movement Report

Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

LOCATION	Center St @ West Side Hwy	LATITUDE	35.1387
COUNTY	Kern	LONGITUDE	-119.4489
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear

		١	Northbour	ıd			S	Southbour	nd				Eastboun	d			,	Westboun	d	
Time	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
7:00 AM - 7:15 AM	0	2	14	0	1	0	0	18	0	1	0	0	0	1	0	0	0	0	0	0
7:15 AM - 7:30 AM	0	2	19	0	2	0	0	25	0	2	0	1	0	1	0	0	0	0	0	0
7:30 AM - 7:45 AM	0	2	18	0	3	0	0	24	0	1	0	1	0	3	0	0	0	0	0	0
7:45 AM - 8:00 AM	0	2	8	0	1	0	0	22	0	0	0	0	0	1	0	0	0	0	0	0
8:00 AM - 8:15 AM	0	1	23	0	1	1	0	20	2	4	0	0	0	1	0	0	0	0	0	0
8:15 AM - 8:30 AM	0	1	21	0	6	0	0	31	0	2	0	1	0	0	0	0	0	0	0	0
8:30 AM - 8:45 AM	0	1	24	0	0	0	0	22	0	5	0	1	0	1	0	0	0	0	0	0
8:45 AM - 9:00 AM	0	1	14	0	2	0	0	19	1	1	0	0	0	3	0	0	0	0	0	0
TOTAL	0	12	141	0	16	1	0	181	3	16	0	4	0	11	0	0	0	0	0	0

		N	lorthboun	d			S	outhbour	d			1	Eastbound	d			١	Vestboun	d	
Time	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
4:00 PM - 4:15 PM	0	3	36	0	3	0	2	20	1	3	0	0	0	3	0	0	0	1	1	0
4:15 PM - 4:30 PM	0	1	26	0	1	0	0	23	0	1	0	0	0	2	0	0	0	0	0	0
4:30 PM - 4:45 PM	0	0	31	0	1	0	0	16	0	1	0	0	0	5	0	0	0	0	0	0
4:45 PM - 5:00 PM	0	1	26	0	0	0	0	19	0	1	0	0	0	3	0	0	0	0	0	0
5:00 PM - 5:15 PM	0	3	21	0	0	0	0	16	0	1	0	1	0	2	0	0	0	0	0	0
5:15 PM - 5:30 PM	0	3	15	0	0	0	0	16	0	3	0	1	0	4	1	0	0	0	0	0
5:30 PM - 5:45 PM	0	2	21	0	2	0	0	26	0	1	0	0	0	3	0	0	0	0	0	0
5:45 PM - 6:00 PM	0	5	25	0	1	0	0	21	0	0	0	0	0	2	0	0	0	0	0	0
TOTAL	0	18	201	0	8	0	2	157	1	11	0	2	0	24	1	0	0	1	1	0

		١	orthboun	ıd			S	outhbour	ıd				Eastbound	d			'	Vestboun	d	
PEAK HOUR	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
8:00 AM - 9:00 AM	0	4	82	0	9	1	0	92	3	12	0	2	0	5	0	0	0	0	0	0
4:00 PM - 5:00 PM	0	5	119	0	5	0	2	78	1	6	0	0	0	13	0	0	0	1	1	0

	PHF	Trucks						<u>We</u>	st Side I	Hwy (SR	33)	<u>PHF</u>				
АМ	0.875	11.1%					РМ	1	78	2	0	0.88				
PM	0.821	5.0%					AM	3	92	0	1	0.774				
			•	PHF	0.65	0.583		4	1	L	b		AM	PM	_	
					0	0	2		•			1	0	1		
					0	2						—	0	1		
			Center St		0	0	\longrightarrow		No	orth		₽	0	0		<u>Driveway</u>
					13	5	7					5	0	0		
					PM	AM	PHF	A	4	1	P	•	#####	0.25	<u>PHF</u>	
							0.86	0	4	82	0	AM				
							0.795	0	5	119	0	PM				

West Side Hwy (SR 33)



310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

Turning Movement Report

Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

LOCATION	Center St @ West Side Hwy	LATITUDE	35.1387	
COUNTY	Kern	LONGITUDE	-119.4489	
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear	

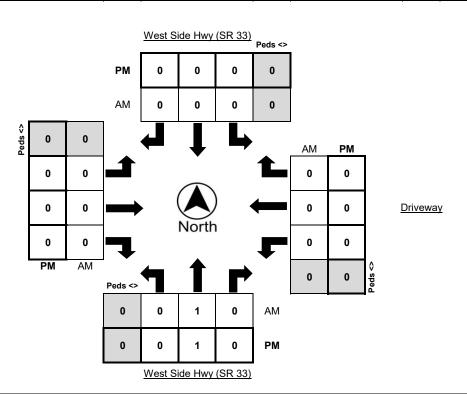
	Nort	hbound E	Bikes	N.Leg	Sout	thbound E	Bikes	S.Leg	Eas	tbound B	ikes	E.Leg	Wes	tbound B	ikes	W.Leg
Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
7:00 AM - 7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM - 7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM - 7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM - 8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM - 8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM - 8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM - 8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM - 9:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0

	Nort	hbound E	Bikes	N.Leg	Sout	thbound E	Bikes	S.Leg	Eas	tbound B	ikes	E.Leg	Wes	tbound B	ikes	W.Leg
Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
4:00 PM - 4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM - 4:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM - 4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM - 5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM - 5:15 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0
5:15 PM - 5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM - 5:45 PM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0
5:45 PM - 6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL	0	1	0	0	0	0	0	2	0	0	0	1	0	0	0	0

	Northbound Bikes		Bikes	N.Leg	Sout	hbound E	Bikes	S.Leg	Eas	tbound B	ikes	E.Leg	Wes	tbound B	ikes	W.Leg
PEAK HOUR	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
8:00 AM - 9:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:00 PM - 5:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0

	Bikes	Peds
AM Peak Total	1	0
PM Peak Total	1	0

Center St



Page 2 of 3



310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

Turning Movement Report

Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

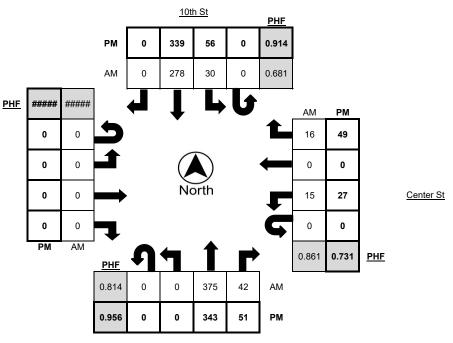
LOCATION	10th St @ Center St	LATITUDE	35.1429
COUNTY	Kern	LONGITUDE	-119.4651
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear

	Northbound						S	Southbour	nd				Eastboun	d			1	Westboun	d	
Time	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
7:00 AM - 7:15 AM	0	0	70	6	2	0	6	39	0	4	0	0	0	0	0	0	1	0	4	0
7:15 AM - 7:30 AM	0	0	108	7	2	0	1	48	0	3	0	0	0	0	0	0	7	0	2	1
7:30 AM - 7:45 AM	0	0	116	12	10	0	10	103	0	2	0	0	0	0	0	0	3	0	6	0
7:45 AM - 8:00 AM	0	0	86	10	0	0	10	77	0	0	0	0	0	0	0	0	3	0	5	0
8:00 AM - 8:15 AM	0	0	65	13	2	0	9	50	0	2	0	0	0	0	0	0	2	0	3	0
8:15 AM - 8:30 AM	0	0	58	7	7	0	2	45	0	3	0	0	0	0	0	0	4	0	5	0
8:30 AM - 8:45 AM	0	0	51	4	0	0	5	32	0	2	0	0	0	0	0	0	3	0	4	1
8:45 AM - 9:00 AM	0	0	43	6	0	0	6	29	0	0	0	0	0	0	0	0	5	0	10	1
TOTAL	0	0	597	65	23	0	49	423	0	16	0	0	0	0	0	0	28	0	39	3

		N	lorthboun	d			Southbound						Eastboun	d			,	Vestboun	d	
Time	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
4:00 PM - 4:15 PM	0	0	70	12	1	0	18	71	0	0	0	0	0	0	0	0	9	0	15	0
4:15 PM - 4:30 PM	0	0	74	14	2	0	11	79	0	1	0	0	0	0	0	0	6	0	14	0
4:30 PM - 4:45 PM	0	0	79	10	1	0	10	91	0	2	0	0	0	0	0	0	6	0	13	0
4:45 PM - 5:00 PM	0	0	92	11	0	0	11	76	0	0	0	0	0	0	0	0	8	0	10	0
5:00 PM - 5:15 PM	0	0	89	13	2	0	19	89	0	2	0	0	0	0	0	0	11	0	15	0
5:15 PM - 5:30 PM	0	0	83	17	1	0	16	83	0	1	0	0	0	0	0	0	2	0	11	0
5:30 PM - 5:45 PM	0	0	68	9	0	0	18	68	0	0	0	0	0	0	0	0	7	0	6	0
5:45 PM - 6:00 PM	0	0	73	17	1	0	19	69	0	0	0	0	0	0	0	0	6	0	12	1
TOTAL	0	0	628	103	8	0	122	626	0	6	0	0	0	0	0	0	55	0	96	1

		N	orthboun	ıd			Southbound						Eastbound	d			'	Vestboun	d	
PEAK HOUR	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks	U-Turn	Left	Thru	Right	Trucks
7:15 AM - 8:15 AM	0	0	375	42	14	0	30	278	0	7	0	0	0	0	0	0	15	0	16	1
4:30 PM - 5:30 PM	0	0	343	51	4	0	56	339	0	5	0	0	0	0	0	0	27	0	49	0

	PHF	Trucks
АМ	0.756	2.9%
PM	0.916	1.0%



10th St



310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

Turning Movement Report

Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

LOCATION	Center St @ West Side Hwy	LATITUDE	35.1387	
COUNTY	Kern	LONGITUDE	-119.4489	
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear	

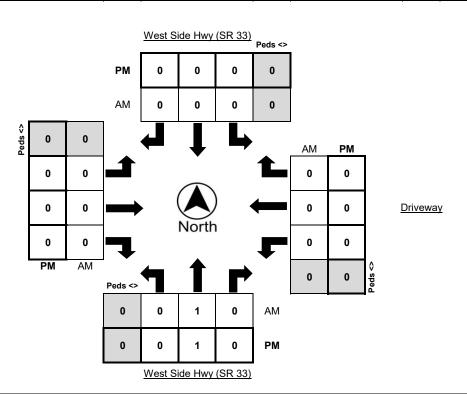
	Nort	hbound E	Bikes	N.Leg	Sout	thbound E	Bikes	S.Leg	Eas	tbound B	ikes	E.Leg	Wes	tbound B	ikes	W.Leg
Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
7:00 AM - 7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM - 7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM - 7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM - 8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM - 8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM - 8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM - 8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM - 9:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0

	Nort	hbound E	Bikes	N.Leg	Sout	thbound E	Bikes	S.Leg	Eas	tbound B	ikes	E.Leg	Wes	tbound B	ikes	W.Leg
Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
4:00 PM - 4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM - 4:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM - 4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM - 5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM - 5:15 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0
5:15 PM - 5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM - 5:45 PM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0
5:45 PM - 6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL	0	1	0	0	0	0	0	2	0	0	0	1	0	0	0	0

	Nort	thbound E	Bikes	N.Leg	Southbound Bikes		S.Leg	Eastbound Bikes		E.Leg Westbound Bikes		ikes	W.Leg			
PEAK HOUR	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds
8:00 AM - 9:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:00 PM - 5:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0

	Bikes	Peds
AM Peak Total	1	0
PM Peak Total	1	0

Center St



Page 2 of 3



Metro Traffic Data Inc.

310 N. Irwin Street - Suite 20 Hanford, CA 93230

800-975-6938 Phone/Fax www.metrotrafficdata.com

24 Hour Count Report

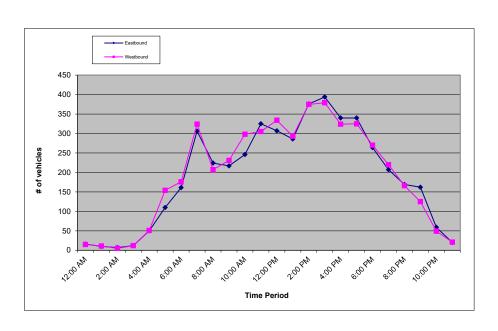
Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

STREET	Kern St (SR33)	LATITUDE	35.14327294	
SEGMENT	btwn 10th St / 6th St	LONGITUDE	-119.4601627	
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear	

		Е	astbour	nd				Hourly			
Hour	1st	2nd	3rd	4th	Total	1st	2nd	3rd	4th	Total	Totals
12:00 AM	6	3	3	4	16	5	1	6	3	15	31
1:00 AM	2	2	2	4	10	2	4	3	2	11	21
2:00 AM	2	0	4	1	7	1	1	3	0	5	12
3:00 AM	2	4	4	2	12	2	4	3	3	12	24
4:00 AM	11	15	12	12	50	9	14	12	16	51	101
5:00 AM	22	24	37	27	110	26	34	51	43	154	264
6:00 AM	37	41	36	47	161	49	38	43	46	176	337
7:00 AM	54	71	89	92	306	36	79	120	89	324	630
8:00 AM	66	61	43	54	224	60	48	43	56	207	431
9:00 AM	55	50	56	56	217	53	61	58	59	231	448
10:00 AM	61	58	55	72	246	72	64	67	95	298	544
11:00 AM	96	83	78	68	325	74	74	82	75	305	630
12:00 PM	84	83	71	69	307	80	92	86	76	334	641
1:00 PM	83	74	65	64	286	72	69	69	83	293	579
2:00 PM	84	102	87	103	376	92	85	105	93	375	751
3:00 PM	105	88	102	99	394	104	92	102	81	379	773
4:00 PM	93	70	81	96	340	88	76	79	81	324	664
5:00 PM	90	73	96	81	340	85	84	83	73	325	665
6:00 PM	74	61	67	62	264	83	72	54	61	270	534
7:00 PM	61	51	57	38	207	58	51	56	55	220	427
8:00 PM	54	34	50	31	169	44	37	43	42	166	335
9:00 PM	47	48	35	32	162	38	31	29	27	125	287
10:00 PM	27	13	9	10	59	15	15	10	9	49	108
11:00 PM	8	4	3	5	20	9	3	3	6	21	41
Total		49.	7%	•	4608		50.	3%		4670	
iotai					92	78					

AM% 37.4% AM Peak 666 7:15 am to 8:15 am AM P.H.F. 0.80 PM% 62.6% PM Peak 789 2:45 pm to 3:45 pm PM P.H.F. 0.94





Metro Traffic Data Inc.

310 N. Irwin Street - Suite 20 Hanford, CA 93230

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24 Hour Count Report

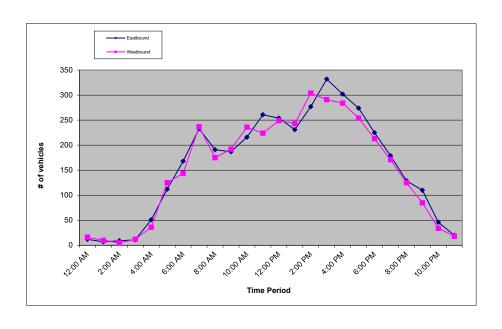
Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

STREET	Kern St (SR33)	LATITUDE	35.14207405
SEGMENT	btwn 6th St / 1st St	LONGITUDE	-119.4563608
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear

		Е	astbour	nd			W	estboui	nd		Hourly	
Hour	1st	2nd	3rd	4th	Total	1st	2nd	3rd	4th	Total	Totals	
12:00 AM	2	4	4	2	12	5	2	6	3	16	28	
1:00 AM	0	1	4	2	7	2	6	2	0	10	17	
2:00 AM	2	2	4	1	9	1	1	3	0	5	14	
3:00 AM	1	3	5	2	11	2	3	3	4	12	23	
4:00 AM	9	12	12	18	51	7	12	7	10	36	87	
5:00 AM	16	22	37	37	112	24	25	35	41	125	237	
6:00 AM	40	50	37	41	168	35	36	31	42	144	312	
7:00 AM	41	49	75	68	233	36	59	77	65	237	470	
8:00 AM	52	56	41	42	191	47	39	44	45	175	366	
9:00 AM	47	44	49	47	187	41	47	56	48	192	379	
10:00 AM	56	50	48	62	216	46	53	57	80	236	452	
11:00 AM	72	64	62	63	261	62	51	55	56	224	485	
12:00 PM	58	78	54	64	254	59	63	71	56	249	503	
1:00 PM	67	68	44	52	231	72	51	50	70	243	474	
2:00 PM	75	81	60	61	277	70	66	81	87	304	581	
3:00 PM	80	67	96	89	332	82	52	79	78	291	623	
4:00 PM	82	73	74	73	302	77	72	69	66	284	586	
5:00 PM	59	66	75	74	274	56	69	74	55	254	528	
6:00 PM	62	54	56	53	225	53	67	44	49	213	438	
7:00 PM	41	50	51	37	179	44	52	43	32	171	350	
8:00 PM	42	29	29	29	129	31	28	40	26	125	254	
9:00 PM	35	35	22	18	110	28	25	18	14	85	195	
10:00 PM	16	12	10	8	46	12	11	5	6	34	80	
11:00 PM	8	5	3	4	20	6	4	4	4	18	38	
Total		51.	0%		3837		49.	0%		3683		
Iolai					75	7520						

AM% 38.2% AM Peak 508 10:45 am to 11:45 am AM P.H.F. 0.89 PM% 61.8% PM Peak 646 3:30 pm to 4:30 pm PM P.H.F. 0.92





Metro Traffic Data Inc.

310 N. Irwin Street - Suite 20 Hanford, CA 93230

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24 Hour Count Report

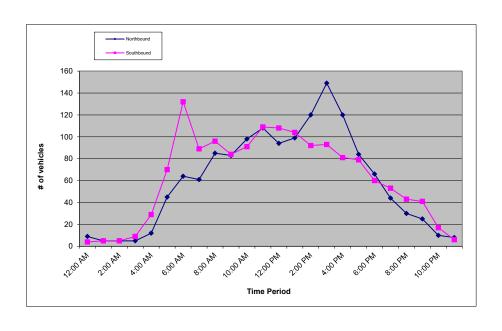
Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

STREET	Westside Hwy (SR 33)	LATITUDE	35.139115	
SEGMENT	btwn 1st St / Center St	LONGITUDE	-119.4491566	
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear	

		No	orthbou	nd			Sc	uthbou	nd		Hourly
Hour	1st	2nd	3rd	4th	Total	1st	2nd	3rd	4th	Total	Totals
12:00 AM	2	3	2	2	9	1	1	1	1	4	13
1:00 AM	2	2	1	0	5	2	1	0	2	5	10
2:00 AM	2	0	3	0	5	0	0	3	2	5	10
3:00 AM	1	2	1	1	5	1	2	4	2	9	14
4:00 AM	3	5	1	3	12	5	8	8	8	29	41
5:00 AM	6	6	21	12	45	5	16	24	25	70	115
6:00 AM	15	19	12	18	64	33	41	34	24	132	196
7:00 AM	14	20	19	8	61	18	25	24	22	89	150
8:00 AM	24	22	25	14	85	23	31	22	20	96	181
9:00 AM	19	20	23	21	83	24	18	23	19	84	167
10:00 AM	18	24	24	32	98	25	21	22	23	91	189
11:00 AM	20	28	34	26	108	33	26	21	29	109	217
12:00 PM	30	22	24	18	94	21	37	24	26	108	202
1:00 PM	27	22	20	30	99	27	29	23	25	104	203
2:00 PM	22	30	31	37	120	23	24	23	22	92	212
3:00 PM	33	24	45	47	149	21	23	25	24	93	242
4:00 PM	37	26	31	26	120	23	23	16	19	81	201
5:00 PM	22	16	21	25	84	16	16	26	21	79	163
6:00 PM	21	22	11	12	66	17	11	19	13	60	126
7:00 PM	11	14	10	9	44	16	17	9	11	53	97
8:00 PM	9	6	9	6	30	15	8	9	11	43	73
9:00 PM	7	11	5	2	25	12	10	12	7	41	66
10:00 PM	4	2	2	2	10	8	4	2	3	17	27
11:00 PM	2	3	0	3	8	0	4	1	1	6	14
Total	48.8% 142					9 51.2% 1500					
iotai		2929									

AM% 44.5% AM Peak 217 11:00 am to 12:00 pm AM P.H.F. 0.99
PM% 55.5% PM Peak 250 3:30 pm to 4:30 pm PM P.H.F. 0.88





Metro Traffic Data Inc.

310 N. Irwin Street - Suite 20 Hanford, CA 93230

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24 Hour Count Report

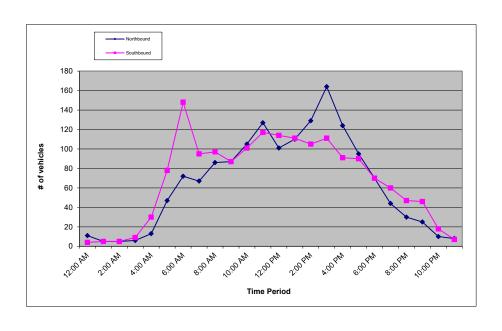
Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

STREET	Westside Hwy (SR 33)	LATITUDE	35.13842693	
SEGMENT	btwn Center St / Main St	LONGITUDE	-119.4487852	
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear	

		No	orthbou	nd			Sc	uthbou	nd		Hourly
Hour	1st	2nd	3rd	4th	Total	1st	2nd	3rd	4th	Total	Totals
12:00 AM	3	3	2	3	11	1	1	1	1	4	15
1:00 AM	2	2	1	0	5	2	1	0	2	5	10
2:00 AM	2	0	3	0	5	0	0	3	2	5	10
3:00 AM	1	2	1	2	6	1	2	4	2	9	15
4:00 AM	3	6	1	3	13	5	8	8	9	30	43
5:00 AM	6	6	23	12	47	5	17	26	30	78	125
6:00 AM	17	21	13	21	72	36	46	37	29	148	220
7:00 AM	16	21	20	10	67	19	26	27	23	95	162
8:00 AM	24	22	25	15	86	21	31	23	22	97	183
9:00 AM	19	20	23	25	87	23	19	25	20	87	174
10:00 AM	18	28	26	33	105	27	20	26	28	101	206
11:00 AM	27	32	37	31	127	31	26	28	32	117	244
12:00 PM	31	24	25	21	101	24	38	26	26	114	215
1:00 PM	31	26	23	30	110	28	30	27	26	111	221
2:00 PM	27	27	35	40	129	25	27	25	28	105	234
3:00 PM	37	27	49	51	164	26	28	30	27	111	275
4:00 PM	39	27	31	27	124	23	25	21	22	91	215
5:00 PM	24	18	23	30	95	18	20	29	23	90	185
6:00 PM	23	25	10	12	70	20	15	20	15	70	140
7:00 PM	10	14	11	9	44	19	17	13	11	60	104
8:00 PM	9	6	9	6	30	16	9	10	12	47	77
9:00 PM	7	11	5	2	25	13	12	13	8	46	71
10:00 PM	4	2	2	2	10	8	4	3	3	18	28
11:00 PM	2	3	0	3	8	1	4	1	1	7	15
Total		48.	4%		1541		51.	6%		1646	
Total					31	87					

AM% 44.1% AM Peak 244 11:00 am to 12:00 pm AM P.H.F. 0.94
PM% 55.9% PM Peak 275 3:00 pm to 4:00 pm PM P.H.F. 0.87





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24 Hour Count Report

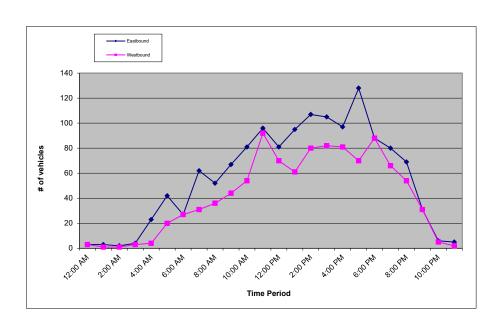
Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

STREET	Center St	LATITUDE	35.14275569
SEGMENT	btwn 10th St / 6th St	LONGITUDE	-119.4648361
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear

	Eastbound					Westbound				Hourly	
Hour	1st	2nd	3rd	4th	Total	1st	2nd	3rd	4th	Total	Totals
12:00 AM	0	0	1	2	3	1	1	1	0	3	6
1:00 AM	0	0	3	0	3	0	0	1	0	1	4
2:00 AM	0	0	0	2	2	0	0	1	0	1	3
3:00 AM	0	1	1	2	4	0	0	1	2	3	7
4:00 AM	4	6	8	5	23	0	1	2	1	4	27
5:00 AM	13	15	6	8	42	4	4	6	6	20	62
6:00 AM	8	6	6	7	27	7	5	4	11	27	54
7:00 AM	12	8	22	20	62	5	9	9	8	31	93
8:00 AM	22	9	9	12	52	5	9	7	15	36	88
9:00 AM	16	22	12	17	67	5	14	14	11	44	111
10:00 AM	23	18	26	14	81	8	13	13	20	54	135
11:00 AM	24	29	20	23	96	17	27	28	20	92	188
12:00 PM	23	23	19	16	81	15	18	18	19	70	151
1:00 PM	25	20	20	30	95	22	18	6	15	61	156
2:00 PM	20	27	27	33	107	20	20	24	16	80	187
3:00 PM	28	21	29	27	105	20	15	25	22	82	187
4:00 PM	30	25	20	22	97	24	20	19	18	81	178
5:00 PM	32	33	27	36	128	26	13	13	18	70	198
6:00 PM	27	17	20	24	88	28	25	17	18	88	176
7:00 PM	26	19	12	23	80	20	17	16	13	66	146
8:00 PM	26	18	10	15	69	16	14	14	10	54	123
9:00 PM	11	6	8	6	31	4	12	5	10	31	62
10:00 PM	4	1	0	1	6	2	3	0	0	5	11
11:00 PM	2	1	2	0	5	0	1	0	1	2	7
Total	57.4% 13				1354	4 42.6% 1006				1006	
· Stai					23	60					

AM% 33.0% AM Peak 188 11:00 am to 12:00 pm AM P.H.F. 0.84 PM% 67.0% PM Peak 198 5:00 pm to 6:00 pm PM P.H.F. 0.85





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24 Hour Count Report

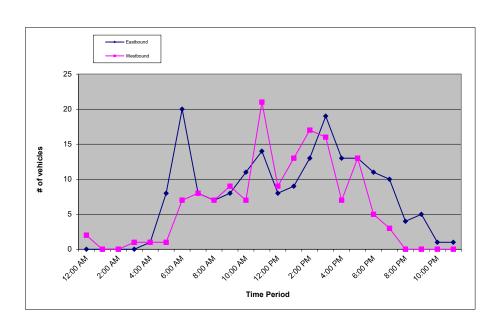
Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

STREET	Center St	LATITUDE	35.1386581
SEGMENT	btwn 6th St / SR 33	LONGITUDE	-119.4493748
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear

		Е	astbour	ıd			W	estbour	nd		Hourly
Hour	1st	2nd	3rd	4th	Total	1st	2nd	3rd	4th	Total	Totals
12:00 AM	0	0	0	0	0	1	0	0	1	2	2
1:00 AM	0	0	0	0	0	0	0	0	0	0	0
2:00 AM	0	0	0	0	0	0	0	0	0	0	0
3:00 AM	0	0	0	0	0	0	0	0	1	1	1
4:00 AM	0	0	0	1	1	0	1	0	0	1	2
5:00 AM	0	1	2	5	8	0	0	1	0	1	9
6:00 AM	3	6	5	6	20	0	1	3	3	7	27
7:00 AM	1	2	4	1	8	2	2	2	2	8	16
8:00 AM	1	1	2	3	7	3	1	1	2	7	14
9:00 AM	1	2	3	2	8	2	1	1	5	9	17
10:00 AM	3	0	4	4	11	1	3	1	2	7	18
11:00 AM	2	2	6	4	14	7	4	4	6	21	35
12:00 PM	4	2	2	0	8	2	3	1	3	9	17
1:00 PM	1	3	4	1	9	4	6	3	0	13	22
2:00 PM	3	3	2	5	13	6	2	5	4	17	30
3:00 PM	4	5	5	5	19	4	3	4	5	16	35
4:00 PM	3	2	5	3	13	5	1	0	1	7	20
5:00 PM	3	5	3	2	13	3	3	2	5	13	26
6:00 PM	3	4	2	2	11	2	3	0	0	5	16
7:00 PM	4	0	4	2	10	0	0	1	2	3	13
8:00 PM	1	1	1	1	4	0	0	0	0	0	4
9:00 PM	1	2	1	1	5	0	0	0	0	0	5
10:00 PM	0	0	1	0	1	0	0	0	0	0	1
11:00 PM	1	0	0	0	1	0	0	0	0	0	1
Total	55.6% 184				184	44.4% 147					
i Stai					33	31					

AM% 42.6% AM Peak 35 11:00 am to 12:00 pm AM P.H.F. 0.88 PM% 57.4% PM Peak 35 3:15 pm to 4:15 pm PM P.H.F. 0.88





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24 Hour Count Report

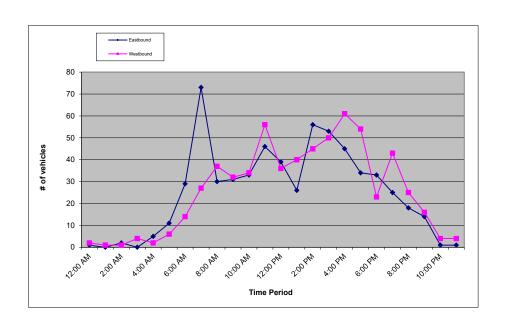
Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

STREET	Main St	LATITUDE	35.14063427
SEGMENT	btwn 10th St / 6th St	LONGITUDE	-119.4612194
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear

Ī		Eastbound					Westbound				Hourly
Hour	1st	2nd	3rd	4th	Total	1st	2nd	3rd	4th	Total	Totals
12:00 AM	1	0	0	0	1	1	1	0	0	2	3
1:00 AM	0	0	0	0	0	0	0	0	1	1	1
2:00 AM	0	0	1	1	2	1	0	0	0	1	3
3:00 AM	0	0	0	0	0	0	1	3	0	4	4
4:00 AM	1	1	0	3	5	0	0	1	1	2	7
5:00 AM	2	1	3	5	11	0	3	1	2	6	17
6:00 AM	4	8	5	12	29	2	2	4	6	14	43
7:00 AM	12	17	26	18	73	9	3	8	7	27	100
8:00 AM	6	7	9	8	30	15	11	6	5	37	67
9:00 AM	6	7	7	11	31	4	9	11	8	32	63
10:00 AM	14	2	7	10	33	6	6	14	8	34	67
11:00 AM	9	7	12	18	46	10	20	8	18	56	102
12:00 PM	12	8	12	7	39	13	9	5	9	36	75
1:00 PM	6	6	6	8	26	12	9	10	9	40	66
2:00 PM	17	13	16	10	56	6	12	17	10	45	101
3:00 PM	15	16	12	10	53	12	12	11	15	50	103
4:00 PM	11	16	7	11	45	17	8	17	19	61	106
5:00 PM	6	13	7	8	34	19	16	7	12	54	88
6:00 PM	10	5	8	10	33	5	8	3	7	23	56
7:00 PM	10	3	5	7	25	14	11	7	11	43	68
8:00 PM	7	3	5	3	18	6	9	6	4	25	43
9:00 PM	9	2	1	2	14	6	4	3	3	16	30
10:00 PM	0	0	1	0	1	2	0	0	2	4	5
11:00 PM	0	0	1	0	1	0	2	0	2	4	5
Total	49.6%				606	5 50.4% 617				617	
i Stai					12	23					

AM% 39.0% AM Peak 102 11:00 am to 12:00 pm AM P.H.F. 0.71 PM% 61.0% PM Peak 108 4:30 pm to 5:30 pm PM P.H.F. 0.90





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24 Hour Count Report

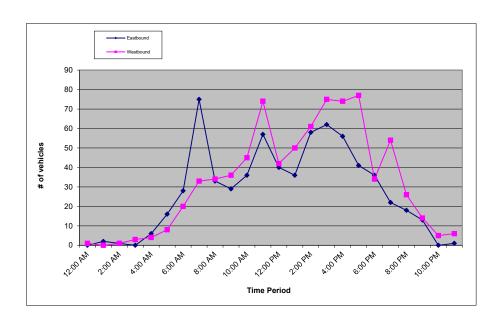
Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

STREET	Main St	LATITUDE	35.14043638	
SEGMENT	btwn 6th St / SR 33	LONGITUDE	-119.4605772	
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear	

		Е	astbour	nd		Westbound				Hourly	
Hour	1st	2nd	3rd	4th	Total	1st	2nd	3rd	4th	Total	Totals
12:00 AM	0	0	0	0	0	0	1	0	0	1	1
1:00 AM	1	0	1	0	2	0	0	0	0	0	2
2:00 AM	0	0	1	0	1	0	0	0	1	1	2
3:00 AM	0	0	0	0	0	0	0	2	1	3	3
4:00 AM	1	1	1	3	6	0	1	1	2	4	10
5:00 AM	3	2	6	5	16	0	2	3	3	8	24
6:00 AM	6	7	3	12	28	2	5	5	8	20	48
7:00 AM	13	17	26	19	75	10	5	10	8	33	108
8:00 AM	9	11	6	7	33	13	8	7	6	34	67
9:00 AM	5	6	8	10	29	7	7	12	10	36	65
10:00 AM	13	7	7	9	36	10	8	12	15	45	81
11:00 AM	11	10	17	19	57	15	22	11	26	74	131
12:00 PM	12	8	10	10	40	17	13	7	5	42	82
1:00 PM	8	6	7	15	36	11	13	11	15	50	86
2:00 PM	10	18	19	11	58	9	17	24	11	61	119
3:00 PM	15	14	19	14	62	15	19	16	25	75	137
4:00 PM	15	18	9	14	56	23	11	24	16	74	130
5:00 PM	6	14	10	11	41	26	19	11	21	77	118
6:00 PM	16	6	7	7	36	13	6	7	8	34	70
7:00 PM	7	2	7	6	22	19	8	12	15	54	76
8:00 PM	8	2	5	3	18	6	10	4	6	26	44
9:00 PM	7	4	0	2	13	4	4	4	2	14	27
10:00 PM	0	0	0	0	0	1	1	1	2	5	5
11:00 PM	0	0	1	0	1	1	2	0	3	6	7
Total		46.	2%		666		53.	8%		777	
iotai					14	43				•	

AM% 37.6% AM Peak 131 11:00 am to 12:00 pm AM P.H.F. 0.73 PM% 62.4% PM Peak 145 3:15 pm to 4:15 pm PM P.H.F. 0.93





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24 Hour Count Report

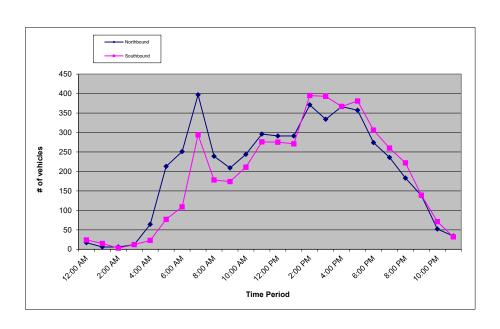
Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

STREET	10th St	LATITUDE	35.14316411
SEGMENT	btwn SR 33 / Center St	LONGITUDE	-119.4651725
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER	Clear

		No	orthbou	nd			Sc	uthbou	nd		Hourly
Hour	1st	2nd	3rd	4th	Total	1st	2nd	3rd	4th	Total	Totals
12:00 AM	6	5	2	4	17	11	7	5	1	24	41
1:00 AM	3	0	3	0	6	3	5	3	4	15	21
2:00 AM	1	3	2	0	6	1	0	1	1	3	9
3:00 AM	2	2	4	4	12	0	5	5	2	12	24
4:00 AM	9	10	19	26	64	0	4	7	12	23	87
5:00 AM	46	50	67	50	213	13	12	23	29	77	290
6:00 AM	76	55	56	64	251	20	30	26	33	109	360
7:00 AM	74	110	122	91	397	45	49	113	87	294	691
8:00 AM	68	63	55	53	239	59	47	37	35	178	417
9:00 AM	46	53	46	64	209	36	57	38	43	174	383
10:00 AM	51	47	62	84	244	54	45	43	69	211	455
11:00 AM	71	70	78	77	296	86	65	61	64	276	572
12:00 PM	77	86	53	75	291	67	77	69	62	275	566
1:00 PM	76	73	63	79	291	78	57	76	60	271	562
2:00 PM	91	94	88	98	371	82	117	104	92	395	766
3:00 PM	80	84	71	99	334	103	100	99	91	393	727
4:00 PM	85	88	92	102	367	89	90	101	87	367	734
5:00 PM	104	94	74	85	357	108	99	86	88	381	738
6:00 PM	77	68	60	69	274	85	84	62	75	306	580
7:00 PM	70	57	48	61	236	78	63	51	68	260	496
8:00 PM	54	55	42	32	183	61	66	48	47	222	405
9:00 PM	24	53	38	23	138	44	30	28	37	139	277
10:00 PM	19	16	8	9	52	29	21	12	9	71	123
11:00 PM	10	9	7	8	34	15	8	3	6	32	66
Total	52.0% 4882										
Iotai						90	-				

AM% 35.7% AM Peak 699 7:15 am to 8:15 am AM P.H.F. 0.74 PM% 64.3% PM Peak 776 2:15 pm to 3:15 pm PM P.H.F. 0.92





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24 Hour Count Report

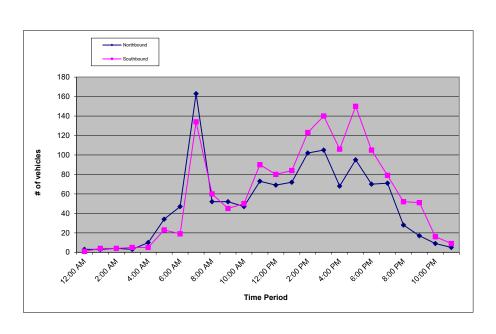
Prepared For:

Rick Engineering Company 5620 Friars Rd San Diego, CA 92110

STREET	6th St	LATITUDE	35.14079557
SEGMENT	btwn SR 33 / Center St	LONGITUDE	-119.4607701
COLLECTION DATE	Tuesday, March 15, 2022	WEATHER_	Clear
			

		Northbound					Southbound				Hourly
Hour	1st	2nd	3rd	4th	Total	1st	2nd	3rd	4th	Total	Totals
12:00 AM	0	1	1	1	3	1	0	0	0	1	4
1:00 AM	0	2	1	0	3	2	1	0	1	4	7
2:00 AM	0	1	0	3	4	2	1	1	0	4	8
3:00 AM	0	1	1	1	3	2	0	2	1	5	8
4:00 AM	4	0	2	4	10	0	1	1	3	5	15
5:00 AM	4	6	8	16	34	4	6	8	5	23	57
6:00 AM	15	4	6	22	47	8	5	1	5	19	66
7:00 AM	21	44	67	31	163	11	23	63	37	134	297
8:00 AM	15	15	12	10	52	28	17	8	7	60	112
9:00 AM	14	12	11	15	52	12	7	12	14	45	97
10:00 AM	15	12	5	15	47	13	12	13	12	50	97
11:00 AM	18	20	20	15	73	20	16	30	24	90	163
12:00 PM	16	17	19	17	69	24	13	21	22	80	149
1:00 PM	15	19	14	24	72	25	15	21	23	84	156
2:00 PM	31	27	19	25	102	34	39	28	22	123	225
3:00 PM	31	25	25	24	105	35	41	31	33	140	245
4:00 PM	15	13	23	17	68	33	25	25	23	106	174
5:00 PM	22	21	25	27	95	34	31	46	39	150	245
6:00 PM	11	25	16	18	70	27	23	29	26	105	175
7:00 PM	19	11	22	19	71	20	18	23	18	79	150
8:00 PM	6	13	5	4	28	20	9	14	9	52	80
9:00 PM	6	5	4	2	17	15	18	8	10	51	68
10:00 PM	1	3	3	2	9	5	3	3	5	16	25
11:00 PM	2	1	0	2	5	5	3	1	0	9	14
Total	45.6% 1202				1202	54.4% 1435					
Iotai				-	26	37					

AM% 35.3% AM Peak 308 7:15 am to 8:15 am AM P.H.F. 0.59
PM% 64.7% PM Peak 245 3:00 pm to 4:00 pm PM P.H.F. 0.93



APPENDIX B

EXISTING CONDITIONS INTERSECTION OPERATIONS ANALYSIS WORKSHEETS

	۶	→	•	•	←	•	4	†	/	/	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ť	∱ ∱		ሻ	ተ ኈ		7	ተ ኈ		ሻ	^	7
Traffic Volume (veh/h)	123	230	54	75	199	58	67	181	129	57	174	91
Future Volume (veh/h)	123	230	54	75	199	58	67	181	129	57	174	91
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		0.98	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	160	299	70	97	258	75	87	235	168	74	226	118
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	201	659	152	171	580	165	165	484	332	154	833	370
Arrive On Green	0.11	0.23	0.23	0.10	0.21	0.21	0.09	0.24	0.24	0.09	0.23	0.23
Sat Flow, veh/h	1781	2866	661	1781	2720	772	1781	2013	1380	1781	3554	1579
Grp Volume(v), veh/h	160	184	185	97	166	167	87	206	197	74	226	118
Grp Sat Flow(s), veh/h/ln	1781	1777	1750	1781	1777	1715	1781	1777	1616	1781	1777	1579
Q Serve(g_s), s	6.2	6.3	6.5	3.7	5.8	6.0	3.3	7.1	7.5	2.8	3.7	4.4
Cycle Q Clear(g_c), s	6.2	6.3	6.5	3.7	5.8	6.0	3.3	7.1	7.5	2.8	3.7	4.4
Prop In Lane	1.00		0.38	1.00		0.45	1.00		0.85	1.00		1.00
Lane Grp Cap(c), veh/h	201	409	403	171	379	366	165	427	388	154	833	370
V/C Ratio(X)	0.80	0.45	0.46	0.57	0.44	0.46	0.53	0.48	0.51	0.48	0.27	0.32
Avail Cap(c_a), veh/h	477	1047	1032	284	854	825	259	827	752	234	1603	712
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	30.7	23.4	23.5	30.6	24.2	24.3	30.7	23.1	23.3	30.9	22.2	22.5
Incr Delay (d2), s/veh	2.7	3.5	3.8	1.1	2.2	2.4	1.0	3.9	4.7	0.9	0.8	2.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.7	2.9	2.9	1.6	2.5	2.5	1.4	3.2	3.2	1.2	1.5	1.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	33.4	27.0	27.3	31.7	26.4	26.7	31.7	27.0	28.0	31.7	23.0	24.7
LnGrp LOS	С	С	С	С	С	С	С	С	С	С	С	С
Approach Vol, veh/h		529			430			490			418	
Approach Delay, s/veh		29.0			27.7			28.2			25.0	
Approach LOS		C C			C C			C C			C C	
											U	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.5	23.1	12.3	23.0	13.7	21.9	11.8	23.5				
Change Period (Y+Rc), s	* 5.7	6.8	* 5.7	6.4	* 5.7	6.8	* 5.7	6.4				
Max Green Setting (Gmax), s	* 11	41.8	* 10	32.0	* 19	34.1	* 9.3	33.0				
Max Q Clear Time (g_c+I1), s	5.7	8.5	5.3	6.4	8.2	8.0	4.8	9.5				
Green Ext Time (p_c), s	0.0	6.8	0.0	5.4	0.1	4.2	0.0	6.6				
Intersection Summary												
HCM 6th Ctrl Delay			27.6									
HCM 6th LOS			С									
Notes												

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Existing AM Synchro 11 Report REC Page 1

Intersection					
Intersection Delay, s/v	/eh15.6				
Intersection LOS	С				
morsoonen 200	Ŭ				

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		414			414			414			4î.		
Traffic Vol, veh/h	79	227	12	16	257	39	17	135	21	43	128	74	
Future Vol, veh/h	79	227	12	16	257	39	17	135	21	43	128	74	
Peak Hour Factor	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	108	311	16	22	352	53	23	185	29	59	175	101	
Number of Lanes	0	2	0	0	2	0	0	2	0	0	2	0	
Approach	EB			WB			NB			SB			
Opposing Approach	WB			EB			SB			NB			
Opposing Lanes	2			2			2			2			
Conflicting Approach Le	eft SB			NB			EB			WB			
Conflicting Lanes Left	2			2			2			2			
Conflicting Approach Ri	ghtNB			SB			WB			EB			
Conflicting Lanes Right	2			2			2			2			
HCM Control Delay	17.4			15.8			13.5			14.7			
HCM LOS	С			С			В			В			

Lane	NBLn1	NBLn ₂	EBLn1	EBLn ₂ \	VBLn ₁ \	WBLn2	SBLn1	SBLn2
Vol Left, %	20%	0%	41%	0%	11%	0%	40%	0%
Vol Thru, %	80%	76%	59%	90%	89%	77%	60%	46%
Vol Right, %	0%	24%	0%	10%	0%	23%	0%	54%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	85	89	193	126	145	168	107	138
LT Vol	17	0	79	0	16	0	43	0
Through Vol	68	68	114	114	129	129	64	64
RT Vol	0	21	0	12	0	39	0	74
Lane Flow Rate	116	121	264	172	198	229	147	189
Geometry Grp	7	7	7	7	7	7	7	7
Degree of Util (X)	0.26	0.263	0.557	0.35	0.412	0.463	0.325	0.388
Departure Headway (Hd)	8.092	7.817	7.606	7.327	7.491	7.267	7.975	7.382
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Cap	443	458	475	490	479	496	450	486
Service Time	5.856	5.581	5.366	5.086	5.25	5.025	5.733	5.139
HCM Lane V/C Ratio	0.262	0.264	0.556	0.351	0.413	0.462	0.327	0.389
HCM Control Delay	13.7	13.4	19.6	14	15.4	16.2	14.6	14.8
HCM Lane LOS	В	В	С	В	С	С	В	В
HCM 95th-tile Q	1	1	3.3	1.6	2	2.4	1.4	1.8

Existing AM Synchro 11 Report REC Page 2

Intersection					
Intersection Delay, s/ve Intersection LOS	h 12				
Intersection LOS	В				

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		414			र्सी			4			4		
Traffic Vol, veh/h	49	189	20	16	205	27	40	96	21	34	104	37	
Future Vol, veh/h	49	189	20	16	205	27	40	96	21	34	104	37	
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	62	239	25	20	259	34	51	122	27	43	132	47	
Number of Lanes	0	2	0	0	2	0	0	1	0	0	1	0	
Approach	EB			WB			NB			SB			
Opposing Approach	WB			EB			SB			NB			
Opposing Lanes	2			2			1			1			
Conflicting Approach Le	eft SB			NB			EB			WB			
Conflicting Lanes Left	1			1			2			2			
Conflicting Approach R	igh N B			SB			WB			EB			
Conflicting Lanes Right	1			1			2			2			
HCM Control Delay	11.9			11.5			12.3			12.6			
HCM LOS	В			В			В			В			

Lane	NBLn1	EBLn1	EBLn2V	VBLn1V	WBLn2	SBLn1					
Vol Left, %	25%	34%	0%	14%	0%	19%					
Vol Thru, %	61%	66%	83%	86%	79%	59%					
Vol Right, %	13%	0%	17%	0%	21%	21%					
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol by Lane	157	144	115	119	130	175					
LT Vol	40	49	0	16	0	34					
Through Vol	96	95	95	103	103	104					
RT Vol	21	0	20	0	27	37					
Lane Flow Rate	199	182	145	150	164	222					
Geometry Grp	2	7	7	7	7	2					
Degree of Util (X)	0.336	0.328	0.25	0.268	0.283	0.368					
Departure Headway (Hd)	6.091	6.505	6.206	6.424	6.207	5.985					
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes					
Cap	586	550	575	557	575	598					
Service Time	4.171	4.276	3.978	4.197	3.979	4.063					
HCM Lane V/C Ratio	0.34	0.331	0.252	0.269	0.285	0.371					
HCM Control Delay	12.3	12.5	11.1	11.6	11.4	12.6					
HCM Lane LOS	В	В	В	В	В	В					
HCM 95th-tile Q	1.5	1.4	1	1.1	1.2	1.7					

Existing AM Synchro 11 Report REC Synchro 12 Report Page 3

Scenario 1: 1 EX 2022 AM

Intersection Level Of Service Report Intersection 4: Kern Street (SR-33) / 1st Street

Control Type:Two-way stopDelay (sec / veh):13.9Analysis Method:HCM 6th EditionLevel Of Service:BAnalysis Period:15 minutesVolume to Capacity (v/c):0.021

Intersection Setup

Name		1st S	Street			1st S	treet			Kern Stree	et (SR-33))	
Approach		North	bound			South	bound			Eastb	ound		
Lane Configuration		4	-			뒥	→			4	H		
Turning Movement	Left2	Left	Thru	Right	Left2	Left	Thru	Right	Left2	Left	Thru	Right	
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0 0 0 0				0	0	0	0	0	0	0	0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00 100.00 100.00 100.00			100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00 0.00 0.00 0.00				
Speed [mph]	30.00					30.	.00		30.00				
Grade [%]	0.00				0.00				0.00				
Crosswalk		No				No				No			

Volumes

Name		1st S	treet			1st S	Street			Kern Stree	et (SR-33))
Base Volume Input [veh/h]	8	0	5	2	4	6	2	23	20	84	85	23
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	0.00	2.00	0.00	2.00	2.00	2.00	2.00	0.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	8	0	5	2	4	6	2	23	20	84	85	23
Peak Hour Factor	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	2	0	1	1	1	2	1	7	6	24	24	7
Total Analysis Volume [veh/h]	9	0	6	2	5	7	2	26	23	97	98	26
Pedestrian Volume [ped/h]		()	_		()	_	0			

Scenario 1: 1 EX 2022 AM

Intersection Settings

Priority Scheme	Stop	Stop	Free
Flared Lane	No	No	
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance	No	No	
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.02	0.00	0.01	0.00	0.01	0.01	0.00	0.03	0.02	0.06	0.00	0.00	
d_M, Delay for Movement [s/veh]	13.87	14.79	12.82	8.92	12.17	12.22	13.01	8.79	7.57	7.55	0.00	0.00	
Movement LOS	В	В	В	Α	В	В	В	Α	Α	Α	Α	Α	
95th-Percentile Queue Length [veh/ln]	0.11	0.11	0.11	0.11	0.17	0.17	0.17	0.17	0.25	0.25	0.13	0.00	
95th-Percentile Queue Length [ft/ln]	2.80	2.80	2.80	2.80	4.18	4.18	4.18	4.18	6.37	6.37	3.19	0.00	
d_A, Approach Delay [s/veh]		12	.92			10	.02			3.	71		
Approach LOS		E	3			E	3		A				
d_I, Intersection Delay [s/veh]						5.	44						
Intersection LOS	В												

Scenario 1: 1 EX 2022 AM

Intersection Setup

Name		Kern Stre	et (SR-33)			E Kerr	Street			
Approach		West	oound			Southwe	estbound			
Lane Configuration		4	 			Y	Ϋ́			
Turning Movement	Left	Thru	Right	Right2	Left	Thru	Right	Right2		
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00		
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0		
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00		
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0		
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Speed [mph]		30	.00		30.00					
Grade [%]		0.	00		0.00					
Crosswalk		N	lo		No					

Volumes

Name		Kern Stree	et (SR-33)			E Kerr	Street	
Base Volume Input [veh/h]	1	68	1	2	2	5	102	6
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	1	68	1	2	2	5	102	6
Peak Hour Factor	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	20	0	1	1	1	29	2
Total Analysis Volume [veh/h]	1	78	1	2	2	6	117	7
Pedestrian Volume [ped/h])	-			0	



Scenario 1: 1 EX 2022 AM

Intersection Settings

Priority Scheme	Free	Stop
Flared Lane		
Storage Area [veh]	0	0
Two-Stage Gap Acceptance		No
Number of Storage Spaces in Median	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.00	0.00	0.01	0.13	0.01
d_M, Delay for Movement [s/veh]	7.47	0.00	0.00	0.00	12.76	13.41	9.36	9.36
Movement LOS	Α	А	A	A	В	В	А	Α
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.00	0.00	0.27	0.27	0.25	0.24
95th-Percentile Queue Length [ft/ln]	0.05	0.05	0.05	0.05	6.68	6.68	6.31	5.94
d_A, Approach Delay [s/veh]		0.	09			9.	60	
Approach LOS		ļ	4			ļ	4	
d_I, Intersection Delay [s/veh]				5.	44			
Intersection LOS				E	3			

Intersection												
Int Delay, s/veh	0.5											
		EDT	EDD	MDI	WDT	MDD	NDI	NDT	NDD	CDI	CDT	ODE
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	4	_	•	4	•		4	•	•	4	•
Traffic Vol, veh/h	2	0	5	0	0	0	4	82	0	0	92	3
Future Vol, veh/h	2	0	5	0	0	0	4	82	0	0	92	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	2	0	6	0	0	0	5	93	0	0	105	3
Major/Minor	Minor2			Minor1			Major1		1	Major2		
Conflicting Flow All	210	210	107	213	211	93	108	0	0	93	0	0
Stage 1	107	107	-	103	103	7.5	100	-	U	75	U	<u> </u>
Stage 2	107	107	-	110	103	-	-	-			_	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	0.22	6.12	5.52	0.22	4.12	-	-	4.12	-	-
, ,	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2		4.018	3.318	3.518	4.018	3.318	2.218	-		2.218		-
Follow-up Hdwy	3.518							-	-		-	-
Pot Cap-1 Maneuver	747	687	947	744	686	964	1483	-	-	1501	-	-
Stage 1	898	807	-	903	810	-	-	-	-	-	-	-
Stage 2	903	810	-	895	806	-	-	-	-	-	-	-
Platoon blocked, %	7.45	(0)	0.47	707	/00	0//	1400	-	-	1504	-	-
Mov Cap-1 Maneuver	745	684	947	737	683	964	1483	-	-	1501	-	-
Mov Cap-2 Maneuver	745	684	-	737	683	-	-	-	-	-	-	-
Stage 1	894	807	-	899	807	-	-	-	-	-	-	-
Stage 2	899	807	-	890	806	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9.1			0			0.3			0		
HCM LOS	Α			A								
Minor Lane/Major Mvn	nt	NBL	NBT	MRP	EBLn1V	WRI n1	SBL	SBT	SBR			
Capacity (veh/h)	TC .	1483	NDI -	NDIX I	879		1501	301	JUK			
						-		-				
HCM Cantral Dalay (c)		0.003	-	-	0.009	-	-	-	-			
HCM Control Delay (s)		7.4	0	-	9.1	0	0	-	-			
HCM Lane LOS	\	A	Α	-	A	Α	A	-	-			
HCM 95th %tile Q(veh	1)	0	-	-	0	-	0	-	-			

Existing AM Synchro 11 Report REC Page 4

Intersection						
Int Delay, s/veh	0.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		ΦÞ		7	^
Traffic Vol, veh/h	15	16	375	42	30	278
Future Vol, veh/h	15	16	375	42	30	278
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	50	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	76	76	76	76	76	76
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	20	21	493	55	39	366
WWW. Tiow	20	21	170	00	07	000
	Minor1		/lajor1	ſ	Major2	
Conflicting Flow All	782	274	0	0	548	0
Stage 1	521	-	-	-	-	-
Stage 2	261	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	_	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	_	_	2.22	-
Pot Cap-1 Maneuver	331	724	-	-	1018	-
Stage 1	561	-	_	_	-	_
Stage 2	759	_	-	-	-	_
Platoon blocked, %	137					
Mov Cap-1 Maneuver	318	724	-	-	1018	-
	430				1010	_
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	561	-	-	-	-	-
Stage 2	730	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	12.2		0		0.8	
HCM LOS	В				0.0	
110111 200						
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	011	1018	-
HCM Lane V/C Ratio		-	-	0.075	0.039	-
HCM Control Delay (s)		-	-	12.2	8.7	-
HCM Lane LOS		-	-	В	Α	-
HCM 95th %tile Q(veh)		-	-		0.1	-
HUM 95th %tile Q(veh)		-	-	0.2	0.1	-

Existing AM Synchro 11 Report REC Synchro 12 Report Page 5

	٠	→	•	•	←	•	4	†	/	/	Ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	∱ ∱		ሻ	ተ ኈ		7	ተ ኈ		ሻ	^	7
Traffic Volume (veh/h)	114	202	93	73	194	40	101	212	81	79	202	147
Future Volume (veh/h)	114	202	93	73	194	40	101	212	81	79	202	147
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		0.99	1.00		0.99	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	123	217	100	78	209	43	109	228	87	85	217	158
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	184	555	247	158	641	129	178	600	222	164	813	356
Arrive On Green	0.10	0.23	0.23	0.09	0.22	0.22	0.10	0.24	0.24	0.09	0.23	0.23
Sat Flow, veh/h	1781	2388	1061	1781	2942	593	1781	2535	938	1781	3554	1558
Grp Volume(v), veh/h	123	159	158	78	125	127	109	158	157	85	217	158
Grp Sat Flow(s), veh/h/ln	1781	1777	1671	1781	1777	1758	1781	1777	1696	1781	1777	1558
Q Serve(g_s), s	4.7	5.3	5.6	2.9	4.1	4.3	4.1	5.2	5.5	3.2	3.5	6.1
Cycle Q Clear(g_c), s	4.7	5.3	5.6	2.9	4.1	4.3	4.1	5.2	5.5	3.2	3.5	6.1
Prop In Lane	1.00	0.0	0.63	1.00		0.34	1.00	0.2	0.55	1.00	0.0	1.00
Lane Grp Cap(c), veh/h	184	413	389	158	387	383	178	421	402	164	813	356
V/C Ratio(X)	0.67	0.39	0.41	0.49	0.32	0.33	0.61	0.37	0.39	0.52	0.27	0.44
Avail Cap(c_a), veh/h	400	1063	1000	202	866	857	377	833	795	311	1535	673
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	30.4	22.8	22.9	30.5	23.1	23.2	30.3	22.5	22.6	30.5	22.3	23.3
Incr Delay (d2), s/veh	1.6	2.7	3.1	0.9	1.3	1.4	1.3	2.5	2.9	0.9	0.8	4.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.0	2.4	2.4	1.2	1.8	1.8	1.7	2.3	2.4	1.3	1.5	2.5
Unsig. Movement Delay, s/veh		۷.۹	2.4	1.2	1.0	1.0	1.7	2.0	2.4	1.0	1.0	2.5
LnGrp Delay(d),s/veh	31.9	25.5	26.0	31.4	24.4	24.6	31.6	25.0	25.4	31.4	23.1	27.2
LnGrp LOS	C	23.3 C	20.0 C	C C	24.4 C	24.0 C	C C	23.0 C	23.4 C	C C	23.1 C	C C
	C			C	330			424	C	C	460	
Approach Vol, veh/h		440										
Approach LOS		27.5			26.1			26.9			26.1	
Approach LOS		С			С			С			С	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.0	23.2	12.8	22.5	13.0	22.1	12.2	23.1				
Change Period (Y+Rc), s	* 5.7	6.8	* 5.7	6.4	* 5.7	6.8	* 5.7	6.4				
Max Green Setting (Gmax), s	* 8	42.1	* 15	30.4	* 16	34.3	* 12	33.0				
Max Q Clear Time (q_c+l1), s	4.9	7.6	6.1	8.1	6.7	6.3	5.2	7.5				
Green Ext Time (p_c), s	0.0	5.9	0.1	5.4	0.1	3.1	0.0	5.2				
Intersection Summary												
HCM 6th Ctrl Delay			26.7									
HCM 6th LOS			С									
Notes												

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Existing PM Synchro 11 Report REC Page 1

Intersection Delay, s/veh 12	Intersection			
	Intersection Delay, s/veh	12		
Intersection EOS	Intersection LOS	В		

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		414			414			414			đĵ.		
Traffic Vol, veh/h	78	245	17	27	224	42	22	79	28	37	117	79	
Future Vol, veh/h	78	245	17	27	224	42	22	79	28	37	117	79	
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	88	275	19	30	252	47	25	89	31	42	131	89	
Number of Lanes	0	2	0	0	2	0	0	2	0	0	2	0	
Approach	EB			WB			NB			SB			
Opposing Approach	WB			EB			SB			NB			
Opposing Lanes	2			2			2			2			
Conflicting Approach Le	eft SB			NB			EB			WB			
Conflicting Lanes Left	2			2			2			2			
Conflicting Approach R	ighNB			SB			WB			EB			
Conflicting Lanes Right	2			2			2			2			
HCM Control Delay	12.8			11.8			10.8			11.6			
HCM LOS	В			В			В			В			

Lane	NBLn11	NBLn2	EBLn1	EBLn2V	VBLn1\	VBLn2	SBLn1	SBLn2	
Vol Left, %	36%	0%	39%	0%	19%	0%	39%	0%	
Vol Thru, %	64%	59%	61%	88%	81%	73%	61%	43%	
Vol Right, %	0%	41%	0%	12%	0%	27%	0%	57%	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	62	68	201	140	139	154	96	138	
LT Vol	22	0	78	0	27	0	37	0	
Through Vol	40	40	123	123	112	112	59	59	
RT Vol	0	28	0	17	0	42	0	79	
Lane Flow Rate	69	76	225	157	156	173	107	154	
Geometry Grp	7	7	7	7	7	7	7	7	
Degree of Util (X)	0.138	0.142	0.409	0.272	0.283	0.299	0.208	0.274	
Departure Headway (Hd)	7.2	6.72	6.533	6.249	6.523	6.23	6.99	6.382	
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Cap	495	530	550	572	548	574	511	559	
Service Time	4.983	4.504	4.301	4.017	4.295	4.002	4.764	4.157	
HCM Lane V/C Ratio	0.139	0.143	0.409	0.274	0.285	0.301	0.209	0.275	
HCM Control Delay	11.1	10.6	13.8	11.4	11.9	11.7	11.6	11.6	
HCM Lane LOS	В	В	В	В	В	В	В	В	
HCM 95th-tile Q	0.5	0.5	2	1.1	1.2	1.2	0.8	1.1	

Existing PM Synchro 11 Report REC Synchro 2 Page 2

HCM Lane LOS

HCM 95th-tile Q

В

0.9

В

1.3

B B B

1 1.1

1.1

Intersection												
Intersection Delay, s/ve	h10 9											
Intersection LOS	1110.9 B											
Intersection LOS	U											
	EDI	EDT	EDD	MDI	WDT	MDD	NDI	NDT	NDE		CDI.	ODI ODI
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR		SBL	
Lane Configurations	20	413	07	20	4 î	20	27	4	20		٥٦	4
Traffic Vol, veh/h	38	247	27	29	216	39	37	73	20		35	
Future Vol, veh/h	38	247	27	29	216	39	37	73	20		35 0.88	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	Ü.		
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2		2	
Mvmt Flow	43	281	31	33	245	44	42	83	23	40		
Number of Lanes	0	2	0	0	2	0	0	1	0	0		1
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			2			1			1		
Conflicting Approach Le				NB			EB			WB		
Conflicting Lanes Left	1			1			2			2		
Conflicting Approach Ri	•			SB			WB			EB		
Conflicting Lanes Right				1			2			2		
HCM Control Delay	11.1			10.7			10.8			10.8		
HCM LOS	В			В			В			В		
Long		IDI "1	CDI 51	EDI 20	MDI 51\	MDI 50	CDI 51					
Lane	ľ			EBLn2V								
Vol Left, %		28%	24%	0%	21%	0%	27%					
Vol Thru, %		56%	76%	82%	79%	73%	48%					
Vol Right, %		15%	0%	18%	0%	27%	25%					
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol by Lane		130	162	151	137	147	132					
LT Vol		37	38	0	29	0	35					
Through Vol		73	124	124	108	108	64					
RT Vol		20	0	27	0	39	33					
Lane Flow Rate		148	184	171	156	167	150					
Geometry Grp		2	7	7	7	7	2					
Degree of Util (X)		0.242		0.273								
Departure Headway (Ho	d)	5.894	5.998	5.752	6.03	5.734	5.83					
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes					
Сар		609	598	624	595	626	615					
Service Time		3.943	3.74	3.493	3.773	3.477	3.879					
HCM Lane V/C Ratio		0.243	0.308	0.274	0.262	0.267	0.244					
HCM Control Delay		10.8	11.4	10.7	10.9	10.6	10.8					
HOMI		D	D		D	. n	. D					

Existing PM Synchro 11 Report REC Page 3

В

0.9

DOWNTOWN TAFT SPECIFIC PLAN

Scenario 2: 2 EX 2022 PM

Intersection Level Of Service Report Intersection 4: Kern Street (SR-33) / 1st Street

Control Type:Two-way stopDelay (sec / veh):16.0Analysis Method:HCM 6th EditionLevel Of Service:CAnalysis Period:15 minutesVolume to Capacity (v/c):0.006

Intersection Setup

Name		1st Street				1st Street				Kern Street (SR-33)			
Approach		North	bound		Southbound				Eastbound				
Lane Configuration		4	-		+				41+				
Turning Movement	Left2	Left	Thru	Right	Left2	Left	Thru	Right	Left2	Left	Thru	Right	
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	0	0	0	0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Speed [mph]		30	.00	-	30.00				30.00				
Grade [%]		0.	00		0.00				0.00				
Crosswalk		No				No				No			

Volumes

Name	1st Street					1st S	treet		Kern Street (SR-33)				
Base Volume Input [veh/h]	9	2	4	0	2	5	3	17	11	139	84	10	
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	
Heavy Vehicles Percentage [%]	2.00	2.00	0.00	2.00	0.00	2.00	2.00	2.00	2.00	0.00	2.00	2.00	
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	
Total Hourly Volume [veh/h]	9	2	4	0	2	5	3	17	11	139	84	10	
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	
Total 15-Minute Volume [veh/h]	2	1	1	0	1	1	1	5	3	37	22	3	
Total Analysis Volume [veh/h]	10 2 4			0	2	5	3	18	12	148	89	11	
Pedestrian Volume [ped/h]	0				0				0				

DOWNTOWN TAFT SPECIFIC PLAN

Scenario 2: 2 EX 2022 PM

Intersection Settings

Priority Scheme	Stop	Stop	Free
Flared Lane	No	No	
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance	No	No	
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.03	0.01	0.01	0.00	0.00	0.01	0.01	0.02	0.01	0.10	0.00	0.00
d_M, Delay for Movement [s/veh]	15.26	16.00	14.52	9.01	14.00	14.09	14.57	8.94	7.77	7.75	0.00	0.00
Movement LOS	С	С	В	А	В	В	В	Α	Α	Α	Α	Α
95th-Percentile Queue Length [veh/ln]	0.14	0.14	0.14	0.14	0.14	0.14	0.14	0.14	0.37	0.37	0.18	0.00
95th-Percentile Queue Length [ft/ln]	3.38	3.38	3.38	3.38	3.39	3.39	3.39	3.39	9.15	9.15	4.58	0.00
d_A, Approach Delay [s/veh]		15.17			10.82				4.77			
Approach LOS		С				E	3		A			
d_I, Intersection Delay [s/veh]					5.05							
Intersection LOS	С											

DOWNTOWN TAFT SPECIFIC PLAN Scenario 2: 2 EX 2022 PM

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Intersection Setup

Name		Kern Stre	et (SR-33)		E Kern Street					
Approach		West	bound		Southwestbound					
Lane Configuration		H	þ		Υř					
Turning Movement	Left	Thru	Right	Right2	Left	Thru	Right	Right2		
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00		
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0		
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00		
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0		
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Speed [mph]		30	.00		30.00					
Grade [%]		0.	00		0.00					
Crosswalk		N	lo		No					

Volumes

Name		Kern Stree	et (SR-33)		E Kern Street					
Base Volume Input [veh/h]	2	121	11	1	1	6	80	4		
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	0.00	0.00	0.00	0.00	0.00		
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0		
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0		
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0		
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0		
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0		
Other Volume [veh/h]	0	0	0	0	0	0	0	0		
Total Hourly Volume [veh/h]	2	121	11	1	1	6	80	4		
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400		
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		
Total 15-Minute Volume [veh/h]	1	32	3	0	0	2	21	1		
Total Analysis Volume [veh/h]	2 129 12 1				1	6	85	4		
Pedestrian Volume [ped/h]		(0							



DOWNTOWN TAFT SPECIFIC PLAN Scenario 2: 2 EX 2022 PM

Version 2021 (SP 0-6)

Intersection Settings

Priority Scheme	Free	Stop
Flared Lane		
Storage Area [veh]	0	0
Two-Stage Gap Acceptance		No
Number of Storage Spaces in Median	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.00	0.00	0.02	0.10	0.00
d_M, Delay for Movement [s/veh]	7.42	0.00	0.00	0.00	14.51	14.71	9.51	9.55
Movement LOS	Α	А	А	А	В	В	Α	Α
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.00	0.00	0.21	0.21	0.19	0.18
95th-Percentile Queue Length [ft/ln]	0.10	0.10	0.10	0.10	5.29	5.29	4.87	4.46
d_A, Approach Delay [s/veh]		0.10				9.	89	
Approach LOS		ļ	4		A			
d_I, Intersection Delay [s/veh]	5.05							
Intersection LOS	С							

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	LDL	4	LDIN	VVDL	4	WDIX	NDL	4	NDIX	ODL	4	ODIT
Traffic Vol, veh/h	0	0	13	0	1	1	5	119	0	2	78	1
Future Vol, veh/h	0	0	13	0	1	1	5	119	0	2	78	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	2,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	82	82	82	82	82	82	82	82	82	82	82	82
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	16	0	1	1	6	145	0	2	95	1
Major/Minor I	Minor2			Minor1			Major1		1	Major2		
Conflicting Flow All	258	257	96	265	257	145	96	0	0	145	0	0
Stage 1	100	100	-	157	157	-	-	-	-	-	-	-
Stage 2	158	157	-	108	100	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	695	647	960	688	647	902	1498	-	-	1437	-	-
Stage 1	906	812	-	845	768	-	-	-	-	-	-	-
Stage 2	844	768	-	897	812	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	690	644	960	674	644	902	1498	-	-	1437	-	-
Mov Cap-2 Maneuver	690	644	-	674	644	-	-	-	-	-	-	-
Stage 1	902	811	-	842	765	-	-	-	-	-	-	-
Stage 2	838	765	-	881	811	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	8.8			9.8			0.3			0.2		
HCM LOS	Α			Α								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1498	-	-	960	751	1437	-	-			
HCM Lane V/C Ratio		0.004	-	-	0.017			-	_			
HCM Control Delay (s)		7.4	0	-	8.8	9.8	7.5	0	-			
HCM Lane LOS		Α	A	-	Α	Α	A	A	-			
HCM 95th %tile Q(veh))	0	-	-	0.1	0	0	-	-			

Existing PM Synchro 11 Report REC Synchro 12 Report Page 4

Intersection						
Int Delay, s/veh	1.6					
		WED	NET	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		↑ ⊅		ች	^
Traffic Vol, veh/h	27	49	343	51	56	339
Future Vol, veh/h	27	49	343	51	56	339
Conflicting Peds, #/hr	0	1	0	5	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	50	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	29	53	373	55	61	368
Major/Minor N	/linor1	Λ	/lajor1	N	Major2	
					433	0
Conflicting Flow All	712	220	0	0		0
Stage 1	406	-	-	-	-	-
Stage 2	306	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	367	784	-	-	1123	-
Stage 1	641	-	-	-	-	-
Stage 2	720	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	345	780	-	-	1118	-
Mov Cap-2 Maneuver	458	-	-	-	-	-
Stage 1	638	-	-	-	-	-
Stage 2	680	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	11.6		0		1.2	
HCM LOS	В					
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-		1118	-
HCM Lane V/C Ratio		-	_	0.132		-
HCM Control Delay (s)		-	-		8.4	-
HCM Lane LOS		-	-	В	Α	-
			_	0.5	0.2	_
HCM 95th %tile Q(veh)		-	_	(7.1)	(1.7	_

Existing PM Synchro 11 Report REC Synchro 12 Report Page 5

APPENDIX C

ITE TRIP GENERATION RATE SHEETS / NCHRP REPORT 684 INTERNAL CAPTURE WORKSHEETS

Land Use: 210 Single-Family Detached Housing

Description

A single-family detached housing site includes any single-family detached home on an individual lot. A typical site surveyed is a suburban subdivision.

Specialized Land Use

Data have been submitted for several single-family detached housing developments with homes that are commonly referred to as patio homes. A patio home is a detached housing unit that is located on a small lot with little (or no) front or back yard. In some subdivisions, communal maintenance of outside grounds is provided for the patio homes. The three patio home sites total 299 dwelling units with overall weighted average trip generation rates of 5.35 vehicle trips per dwelling unit for weekday, 0.26 for the AM adjacent street peak hour, and 0.47 for the PM adjacent street peak hour. These patio home rates based on a small sample of sites are lower than those for single-family detached housing (Land Use 210), lower than those for single-family attached housing (Land Use 251), and higher than those for senior adult housing -- single-family (Land Use 251). Further analysis of this housing type will be conducted in a future edition of Trip Generation Manual.

Additional Data

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (https://www.ite.org/technical-resources/topics/tripand-parking-generation/).

For 30 of the study sites, data on the number of residents and number of household vehicles are available. The overall averages for the 30 sites are 3.6 residents per dwelling unit and 1.5 vehicles per dwelling unit.

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Arizona, California, Connecticut, Delaware, Illinois, Indiana, Kentucky, Maryland, Massachusetts, Minnesota, Montana, New Jersey, North Carolina, Ohio, Ontario (CAN), Oregon, Pennsylvania, South Carolina, South Dakota, Tennessee, Vermont, Virginia, and West Virginia.

Source Numbers

100, 105, 114, 126, 157, 167, 177, 197, 207, 211, 217, 267, 275, 293, 300, 319, 320, 356, 357, 367, 384, 387, 407, 435, 522, 550, 552, 579, 598, 601, 603, 614, 637, 711, 716, 720, 728, 735, 868, 869, 903, 925, 936, 1005, 1007, 1008, 1010, 1033, 1066, 1077,1078, 1079



Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units On a: Weekday

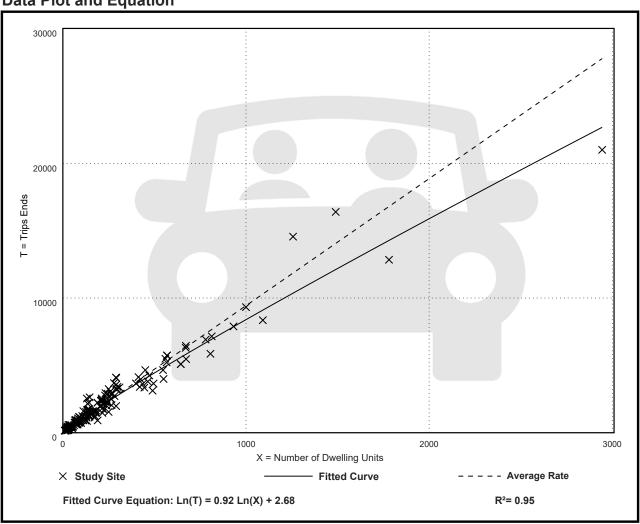
Setting/Location: General Urban/Suburban

Number of Studies: 174 Avg. Num. of Dwelling Units: 246

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
9.43	4.45 - 22.61	2.13





Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

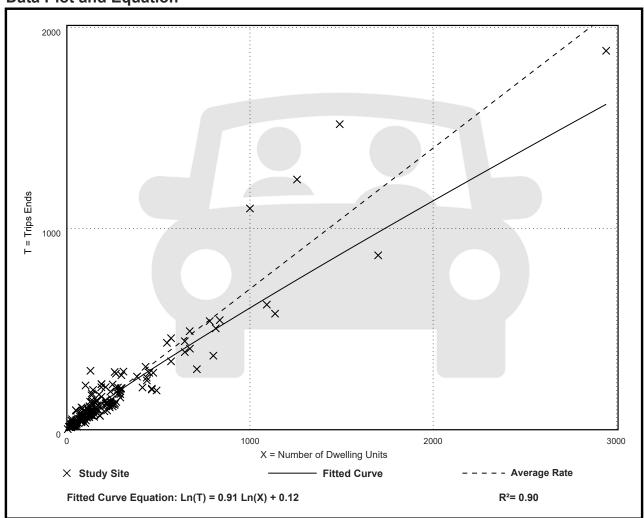
Setting/Location: General Urban/Suburban

Number of Studies: 192 Avg. Num. of Dwelling Units: 226

Directional Distribution: 26% entering, 74% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.70	0.27 - 2.27	0.24





Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

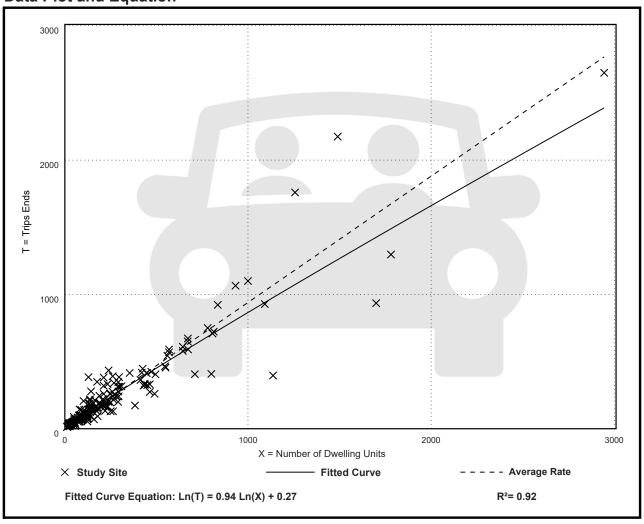
Setting/Location: General Urban/Suburban

Number of Studies: 208 Avg. Num. of Dwelling Units: 248

Directional Distribution: 63% entering, 37% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.94	0.35 - 2.98	0.31





Land Use: 215 Single-Family Attached Housing

Description

Single-family attached housing includes any single-family housing unit that shares a wall with an adjoining dwelling unit, whether the walls are for living space, a vehicle garage, or storage space.

Additional Data

The database for this land use includes duplexes (defined as a single structure with two distinct dwelling units, typically joined side-by-side and each with at least one outside entrance) and townhouses/rowhouses (defined as a single structure with three or more distinct dwelling units, joined side-by-side in a row and each with an outside entrance).

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (https://www.ite.org/technical-resources/topics/tripand-parking-generation/).

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in British Columbia (CAN), California, Georgia, Illinois, Maryland, Massachusetts, Minnesota, New Jersey, Ontario (CAN), Oregon, Pennsylvania, South Dakota, Utah, Virginia, and Wisconsin.

Source Numbers

168, 204, 211, 237, 305, 306, 319, 321, 357, 390, 418, 525, 571, 583, 638, 735, 868, 869, 870, 896, 912, 959, 1009, 1046, 1056, 1058, 1077



Single-Family Attached Housing (215)

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday

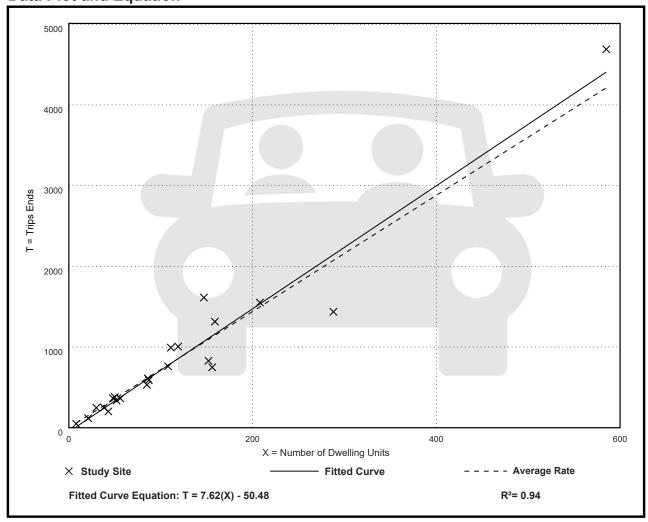
Setting/Location: General Urban/Suburban

Number of Studies: 22 Avg. Num. of Dwelling Units: 120

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
7.20	4.70 - 10.97	1.61





Single-Family Attached Housing (215)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

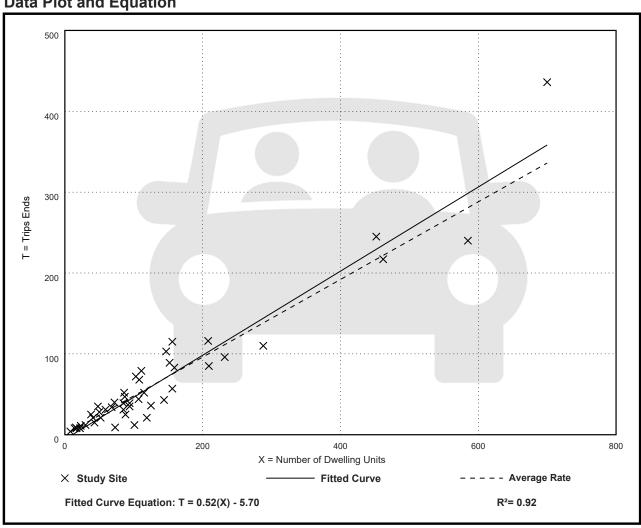
Setting/Location: General Urban/Suburban

Number of Studies: 46 Avg. Num. of Dwelling Units: 135

Directional Distribution: 31% entering, 69% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.48	0.12 - 0.74	0.14





Single-Family Attached Housing (215)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

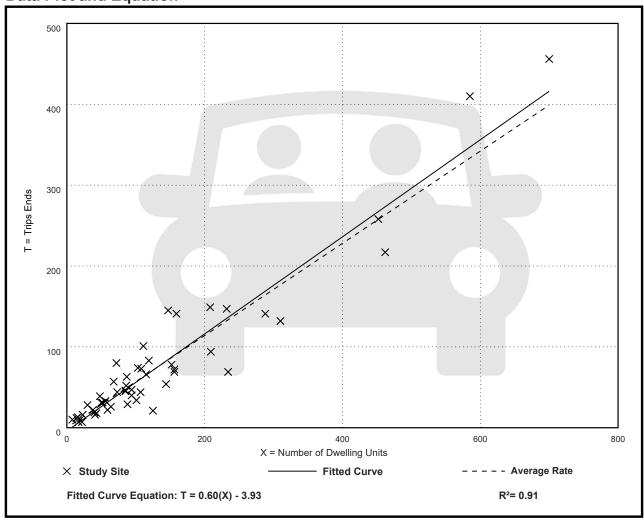
Setting/Location: General Urban/Suburban

Number of Studies: 51 Avg. Num. of Dwelling Units: 136

Directional Distribution: 57% entering, 43% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.57	0.17 - 1.25	0.18





Land Use: 220 **Multifamily Housing (Low-Rise)**

Description

Low-rise multifamily housing includes apartments, townhouses, and condominiums located within the same building with at least three other dwelling units and that have two or three floors (levels). Various configurations fit this description, including walkup apartment, mansion apartment, and stacked townhouse.

- A walkup apartment typically is two or three floors in height with dwelling units that are accessed by a single or multiple entrances with stairways and hallways.
- A mansion apartment is a single structure that contains several apartments within what appears to be a single-family dwelling unit.
- A fourplex is a single two-story structure with two matching dwelling units on the ground and second floors. Access to the individual units is typically internal to the structure and provided through a central entry and stairway.
- A stacked townhouse is designed to match the external appearance of a townhouse. But, unlike a townhouse dwelling unit that only shares walls with an adjoining unit, the stacked townhouse units share both floors and walls. Access to the individual units is typically internal to the structure and provided through a central entry and stairway.

Multifamily housing (mid-rise) (Land Use 221), multifamily housing (high-rise) (Land Use 222), affordable housing (Land Use 223), and off-campus student apartment (low-rise) (Land Use 225) are related land uses.

Land Use Subcategory

Data are presented for two subcategories for this land use: (1) not close to rail transit and (2) close to rail transit. A site is considered close to rail transit if the walking distance between the residential site entrance and the closest rail transit station entrance is 1/2 mile or less.

Additional Data

For the three sites for which both the number of residents and the number of occupied dwelling units were available, there were an average of 2.72 residents per occupied dwelling unit.

For the two sites for which the numbers of both total dwelling units and occupied dwelling units were available, an average of 96.2 percent of the total dwelling units were occupied.

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip



generation resource page on the ITE website (https://www.ite.org/technical-resources/topics/tripand-parking-generation/).

For the three sites for which data were provided for both occupied dwelling units and residents, there was an average of 2.72 residents per occupied dwelling unit.

It is expected that the number of bedrooms and number of residents are likely correlated to the trips generated by a residential site. To assist in future analysis, trip generation studies of all multifamily housing should attempt to obtain information on occupancy rate and on the mix of residential unit sizes (i.e., number of units by number of bedrooms at the site complex).

The sites were surveyed in the 1980s, the 1990s, the 2000s, the 2010s, and the 2020s in British Columbia (CAN), California, Delaware, Florida, Georgia, Illinois, Indiana, Maine, Maryland, Massachusetts, Minnesota, New Jersey, Ontario (CAN), Oregon, Pennsylvania, South Carolina, South Dakota, Tennessee, Texas, Utah, and Washington.

Source Numbers

188, 204, 237, 300, 305, 306, 320, 321, 357, 390, 412, 525, 530, 579, 583, 638, 864, 866, 896, 901, 903, 904, 936, 939, 944, 946, 947, 948, 963, 964, 966, 967, 1012, 1013, 1014, 1036, 1047, 1056, 1071, 1076



Multifamily Housing (Low-Rise) Not Close to Rail Transit (220)

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday

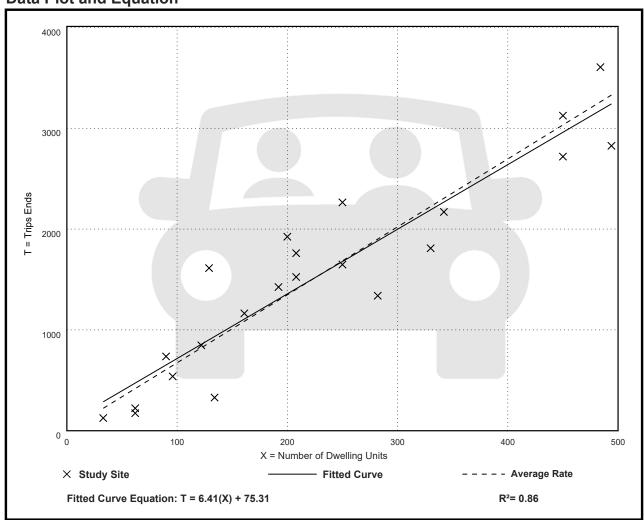
Setting/Location: General Urban/Suburban

Number of Studies: 22 Avg. Num. of Dwelling Units: 229

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
6.74	2.46 - 12.50	1.79





Multifamily Housing (Low-Rise) Not Close to Rail Transit (220)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

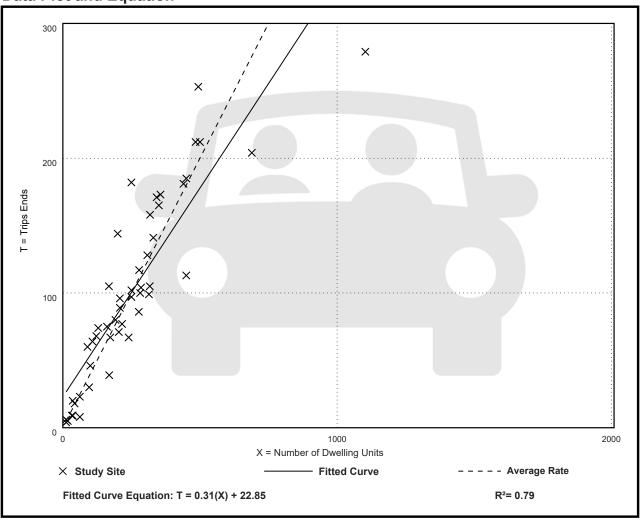
Setting/Location: General Urban/Suburban

Number of Studies: 49 Avg. Num. of Dwelling Units: 249

Directional Distribution: 24% entering, 76% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.40	0.13 - 0.73	0.12





Multifamily Housing (Low-Rise) Not Close to Rail Transit (220)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

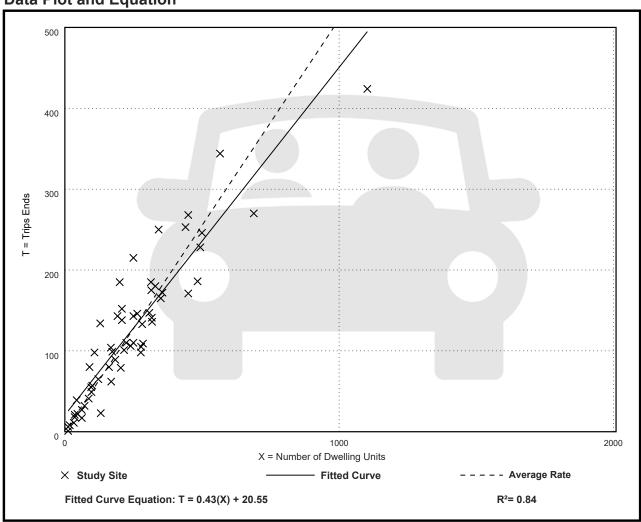
Setting/Location: General Urban/Suburban

Number of Studies: 59 Avg. Num. of Dwelling Units: 241

Directional Distribution: 63% entering, 37% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.51	0.08 - 1.04	0.15





Land Use: 820 **Shopping Center (>150k)**

Description

A shopping center is an integrated group of commercial establishments that is planned, developed, owned, and managed as a unit. Each study site in this land use has at least 150,000 square feet of gross leasable area (GLA). It often has more than one anchor store. Various names can be assigned to a shopping center within this size range, depending on its specific size and tenants, such as community center, regional center, superregional center, fashion center, and power center.

A shopping center of this size typically contains more than retail merchandising facilities. Office space, a movie theater, restaurants, a post office, banks, a health club, and recreational facilities are common tenants.

A shopping center of this size can be enclosed or open-air. The vehicle trips generated at a shopping center are based upon the total GLA of the center. In the case of a smaller center without an enclosed mall or peripheral buildings, the GLA is the same as the gross floor area of the building.

The 150,000 square feet GLA threshold value between community/regional shopping center and shopping plaza (Land Use 821) is based on an examination of trip generation data. For a shopping plaza that is smaller than the threshold value, the presence or absence of a supermarket within the plaza has a measurable effect on site trip generation. For a shopping center that is larger than the threshold value, the trips generated by its other major tenants mask any effects of the presence or absence of an on-site supermarket.

Shopping plaza (40-150k) (Land Use 821), strip retail plaza (<40k) (Land Use 822), and factory outlet center (Land Use 823) are related uses.

Additional Data

Many shopping centers—in addition to the integrated unit of shops in one building or enclosed around a mall—include outparcels (peripheral buildings or pads located on the perimeter of the center adjacent to the streets and major access points). These buildings are typically drive-in banks, retail stores, restaurants, or small offices. Although the data herein do not indicate which of the centers studied include peripheral buildings, it can be assumed that some of the data show their effect.

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (https://www.ite.org/technical-resources/topics/tripand-parking-generation/).

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Alberta (CAN), California, Colorado, Connecticut, Delaware, Florida, Georgia, Illinois, Indiana, Iowa, Kentucky,



Maryland, Massachusetts, Michigan, Minnesota, New Jersey, New York, North Carolina, Ohio, Oklahoma, Pennsylvania, Tennessee, Texas, Vermont, Virginia, Washington, West Virginia, and Wisconsin.

Source Numbers

77, 110, 154, 156, 159, 190, 199, 202, 204, 213, 251, 269, 294, 295, 299, 304, 305, 307, 308, 309, 311, 314, 315, 316, 317, 319, 365, 385, 404, 414, 423, 442, 446, 562, 629, 702, 715, 728, 868, 871, 880, 899, 912, 926, 946, 962, 973, 974, 978, 1034, 1040, 1067



Shopping Center (>150k) (820)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA On a: Weekday

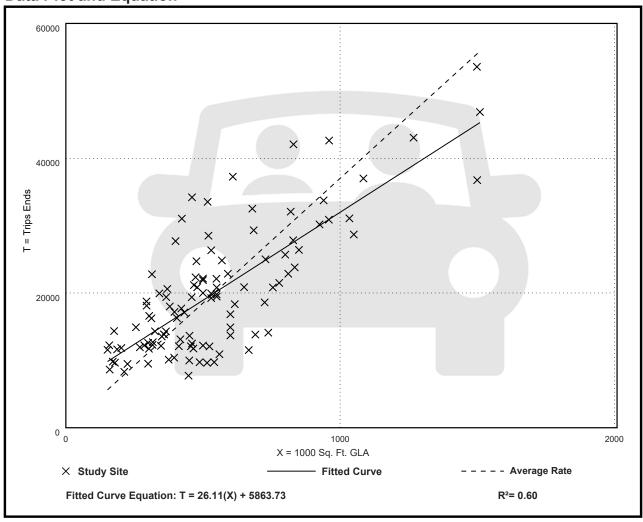
Setting/Location: General Urban/Suburban

Number of Studies: 108 Avg. 1000 Sq. Ft. GLA: 538

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
37.01	17.27 - 81.53	12.79





Shopping Center (>150k) (820)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

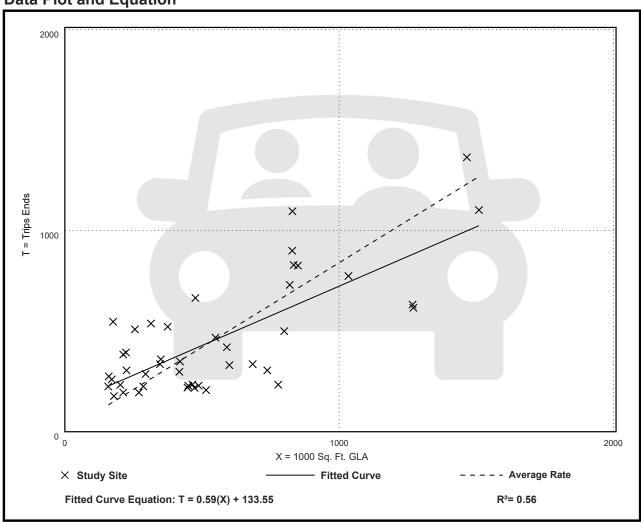
Setting/Location: General Urban/Suburban

Number of Studies: 44 Avg. 1000 Sq. Ft. GLA: 546

Directional Distribution: 62% entering, 38% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
0.84	0.30 - 3.11	0.42





Shopping Center (>150k) (820)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

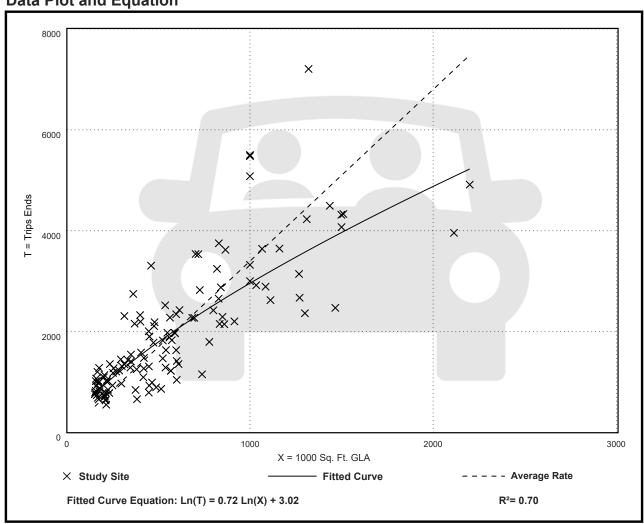
Setting/Location: General Urban/Suburban

Number of Studies: 126 Avg. 1000 Sq. Ft. GLA: 581

Directional Distribution: 48% entering, 52% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
3.40	1.57 - 7.58	1.26





Land Use: 750 **Office Park**

Description

An office park is typically a suburban subdivision or planned unit development that contains general office buildings and support services, such as banks, restaurants, and service stations, arranged in a park- or campus-like atmosphere. General office building (Land Use 710), corporate headquarters building (Land Use 714), single tenant office building (Land Use 715), research and development center (Land Use 760), and business park (Land Use 770) are related uses.

Additional Data

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Alberta (CAN), Connecticut, Georgia, Indiana, Massachusetts, New Jersey, New York, and Pennsylvania.

Source Numbers

160, 161, 184, 185, 253, 300, 301, 356, 550, 618, 912, 972, 973



Office Park (750)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA On a: Weekday

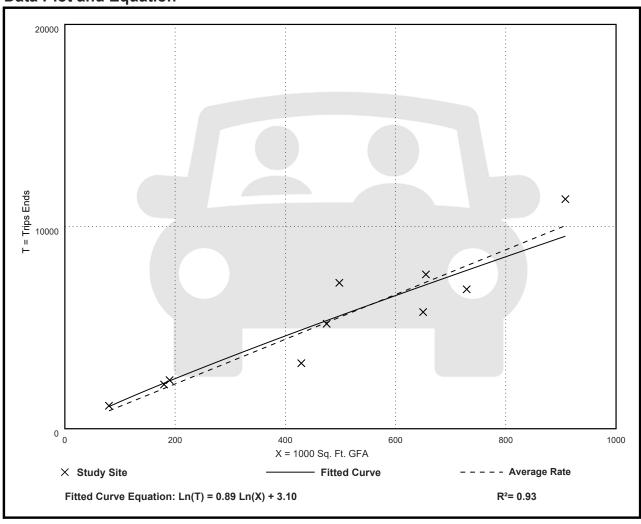
Setting/Location: General Urban/Suburban

Number of Studies: 10 Avg. 1000 Sq. Ft. GFA: 479

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
11.07	7.56 - 14.50	2.14





Office Park (750)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

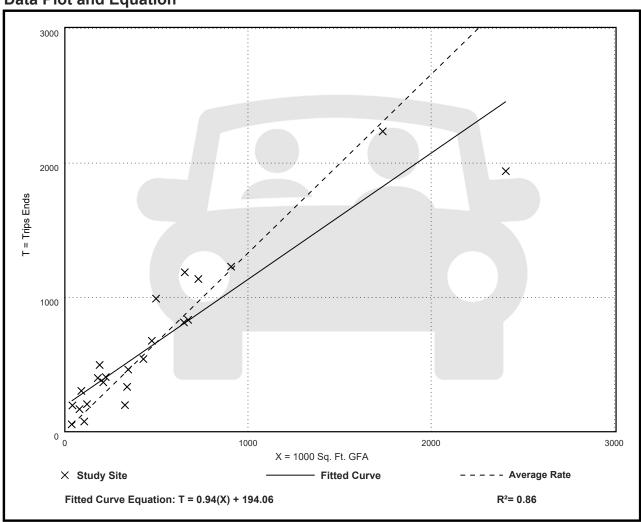
Setting/Location: General Urban/Suburban

Number of Studies: 23 Avg. 1000 Sq. Ft. GFA: 498

Directional Distribution: 89% entering, 11% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.33	0.60 - 4.74	0.51





Office Park (750)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

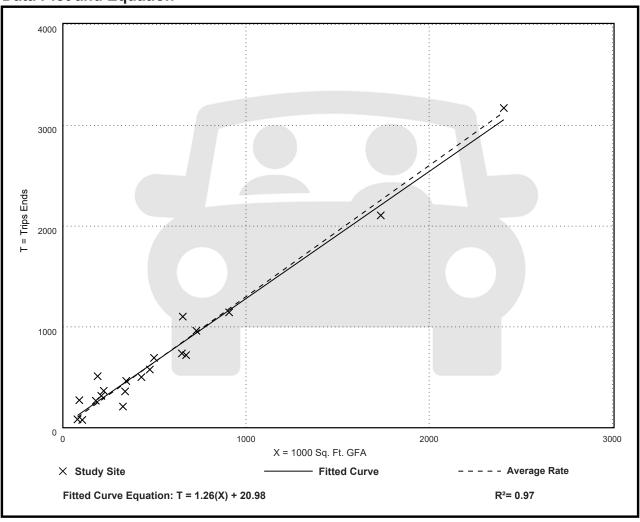
Setting/Location: General Urban/Suburban

Number of Studies: 20 Avg. 1000 Sq. Ft. GFA: 563

Directional Distribution: 14% entering, 86% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.30	0.64 - 3.03	0.32





Land Use: 730 **Government Office Building**

Description

A government office building is an individual building containing either the entire function or simply one agency of a city, county, state, federal, or other governmental unit.

Additional Data

Each study site in the current database serves a municipal or county agency.

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (https://www.ite.org/technical-resources/topics/tripand-parking-generation/).

The sites were surveyed in the 2000s and the 2010s in Oregon and Texas.

Source Numbers

579,889



Government Office Building (730)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday

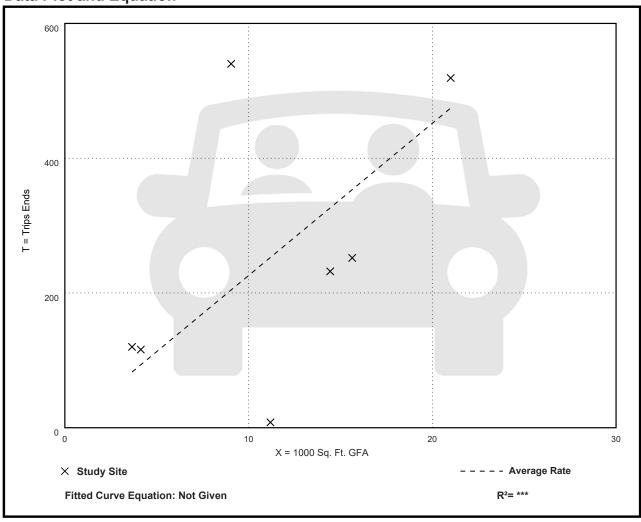
Setting/Location: General Urban/Suburban

Number of Studies: 7 Avg. 1000 Sq. Ft. GFA: 11

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
22.59	0.71 - 59.66	17.03





Government Office Building (730)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

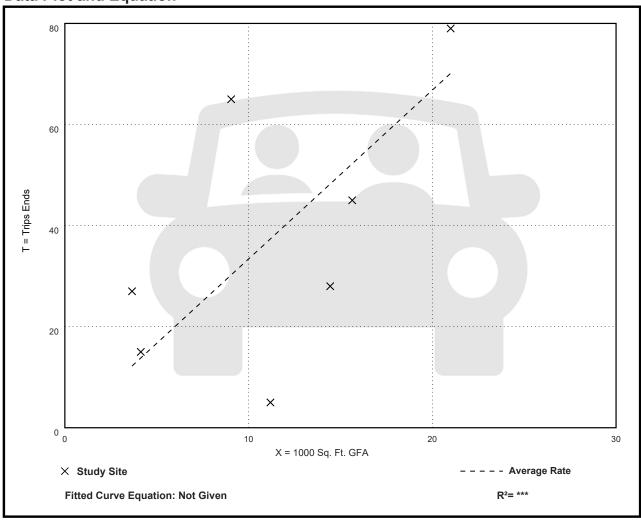
Setting/Location: General Urban/Suburban

Number of Studies: 7 Avg. 1000 Sq. Ft. GFA: 11

Directional Distribution: 75% entering, 25% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
3.34	0.45 - 7.38	2.18





Government Office Building (730)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

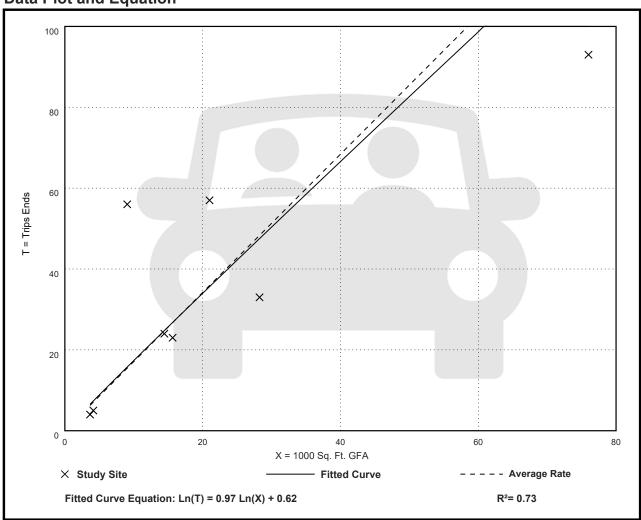
Setting/Location: General Urban/Suburban

Number of Studies: 8 Avg. 1000 Sq. Ft. GFA: 22

Directional Distribution: 25% entering, 75% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.71	1.09 - 6.19	1.24





Land Use: 150 Warehousing

Description

A warehouse is primarily devoted to the storage of materials, but it may also include office and maintenance areas. High-cube transload and short-term storage warehouse (Land Use 154), high-cube fulfillment center warehouse (Land Use 155), high-cube parcel hub warehouse (Land Use 156), and high-cube cold storage warehouse (Land Use 157) are related uses.

Additional Data

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (https://www.ite.org/technical-resources/topics/trip-and-parking-generation/).

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in California, Connecticut, Minnesota, New Jersey, New York, Ohio, Oregon, Pennsylvania, and Texas.

Source Numbers

184, 331, 406, 411, 443, 579, 583, 596, 598, 611, 619, 642, 752, 869, 875, 876, 914, 940, 1050



Warehousing (150)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday

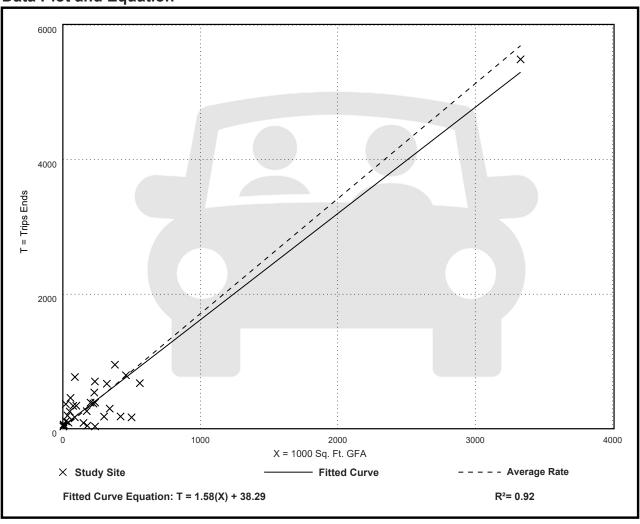
Setting/Location: General Urban/Suburban

Number of Studies: 31 Avg. 1000 Sq. Ft. GFA: 292

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.71	0.15 - 16.93	1.48





Warehousing (150)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

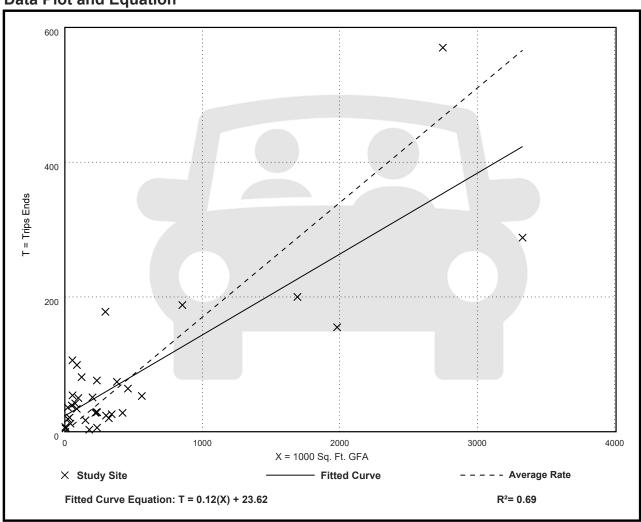
Number of Studies: 36 Avg. 1000 Sq. Ft. GFA: 448

Directional Distribution: 77% entering, 23% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.17	0.02 - 1.93	0.19

Data Plot and Equation





Warehousing (150)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

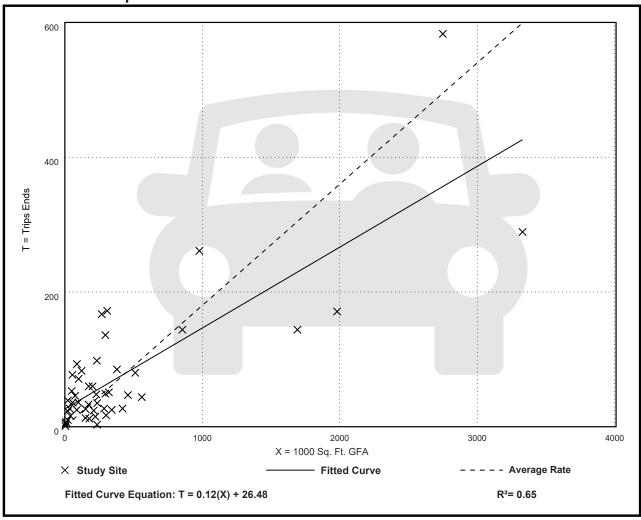
Number of Studies: 49 Avg. 1000 Sq. Ft. GFA: 400

Directional Distribution: 28% entering, 72% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.18	0.01 - 1.80	0.18

Data Plot and Equation





NCHRP 8-51 Internal Trip Capture Estimation Tool							
Project Name:	Downtown Taft Specific Plan		Organization:				
Project Location:	Taft, CA		Performed By:				
Scenario Description:			Date:				
Analysis Year:	Horizon Year 2042		Checked By:				
Analysis Period:	AM Street Peak Hour		Date:				

	Table 1-	A: Base Vehicle	-Trip Generation	Est	timates (Single-Use Sit	te Estimate)	
Land Use	Developme	ent Data (<i>For Info</i>	ormation Only)			Estimated Vehicle-Trips	
Land Use	ITE LUCs1	Quantity	Units		Total	Entering	Exiting
Office				Ī	2007	1682	325
Retail					659	409	250
Restaurant					0		
Cinema/Entertainment					0		
Residential					1543	419	1124
Hotel					0		
All Other Land Uses ²					51	39	12
Total					4260	2549	1711

Table 2-A: Mode Split and Vehicle Occupancy Estimates								
Land Use		Entering Tri	ps		Exiting Trips			
Land Ose	Veh. Occ.	% Transit	% Non-Motorized		Veh. Occ.	% Transit	% Non-Motorized	
Office	1.00				1.00			
Retail	1.00				1.00			
Restaurant	1.00				1.00			
Cinema/Entertainment								
Residential								
Hotel								
All Other Land Uses ²								

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)								
Origin (From)				Destination (To)				
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel		
Office								
Retail								
Restaurant								
Cinema/Entertainment								
Residential								
Hotel								

Table 4-A: Internal Person-Trip Origin-Destination Matrix*								
Origin (Fram)				Destination (To)				
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel		
Office		91	0	0	0	0		
Retail	67		0	0	8	0		
Restaurant	0	0		0	0	0		
Cinema/Entertainment	0	0	0		0	0		
Residential	22	11	0	0		0		
Hotel	0	0	0	0	0			

Table 5-A: Computations Summary						
	Total	Entering	Exiting			
All Person-Trips	4,260	2,549	1,711			
Internal Capture Percentage	9%	8%	12%			
External Vehicle-Trips ³	3,862	2,350	1,512			
External Transit-Trips ⁴	0	0	0			
External Non-Motorized Trips ⁴	0	0	0			

Table 6-A: Internal Trip Capture Percentages by Land Use							
Land Use	Entering Trips	Exiting Trips					
Office	5%	28%					
Retail	25%	30%					
Restaurant	N/A	N/A					
Cinema/Entertainment	N/A	N/A					
Residential	2%	3%					
Hotel	N/A	N/A					

¹Land Use Codes (LUCs) from *Trip Generation Informational Report*, published by the Institute of Transportation Engineers.

²Total estimate for all other land uses at mixed-use development site-not subject to internal trip capture computations in this estimator

³Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A

⁴Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

Estimation Tool Developed by the Texas Transportation Institute

Project Name:	Downtown Taft Specific Plan
Analysis Period:	AM Street Peak Hour

Table 7-A: Conversion of Vehicle-Trip Ends to Person-Trip Ends								
Land Use	Tab	le 7-A (D): Enter	ing Trips			Table 7-A (O): Exiting Trips		
Land OSE	Veh. Occ.	Vehicle-Trips	Person-Trips*		Veh. Occ.	Vehicle-Trips	Person-Trips*	
Office	1.00	1682	1682		1.00	325	325	
Retail	1.00	409	409		1.00	250	250	
Restaurant	1.00	0	0		1.00	0	0	
Cinema/Entertainment	1.00	0	0		1.00	0	0	
Residential	1.00	419	419		1.00	1124	1124	
Hotel	1.00	0	0		1.00	0	0	

Table 8-A (O): Internal Person-Trip Origin-Destination Matrix (Computed at Origin)									
Origin (From)		Destination (To)							
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office		91	205	0	3	0			
Retail	73		33	0	35	0			
Restaurant	0	0		0	0	0			
Cinema/Entertainment	0	0	0		0	0			
Residential	22	11	225	0		0			
Hotel	0	0	0	0	0				

Table 8-A (D): Internal Person-Trip Origin-Destination Matrix (Computed at Destination) Destination (To)								
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel		
Office		131	0	0	0	0		
Retail	67		0	0	8	0		
Restaurant	235	33		0	21	0		
Cinema/Entertainment	0	0	0		0	0		
Residential	50	70	0	0		0		
Hotel	50	16	0	0	0			

	Table 9-A (D): Internal and External Trips Summary (Entering Trips)									
Destination Land Lies	Person-Trip Estimates				External Trips by Mode*					
Destination Land Use	Internal	External	Total		Vehicles ¹	Transit ²	Non-Motorized ²			
Office	89	1593	1682		1593	0	0			
Retail	102	307	409		307	0	0			
Restaurant	0	0	0		0	0	0			
Cinema/Entertainment	0	0	0		0	0	0			
Residential	8	411	419		411	0	0			
Hotel	0	0	0		0	0	0			
All Other Land Uses ³	0	39	39		39	0	0			

	Table 9-A (O): Internal and External Trips Summary (Exiting Trips)									
Origin Land Use	Person-Trip Estimates				External Trips by Mode*					
	Internal	External	Total		Vehicles ¹	Transit ²	Non-Motorized ²			
Office	91	234	325		234	0	0			
Retail	75	175	250		175	0	0			
Restaurant	0	0	0		0	0	0			
Cinema/Entertainment	0	0	0		0	0	0			
Residential	33	1091	1124		1091	0	0			
Hotel	0	0	0		0	0	0			
All Other Land Uses ³	0	12	12		12	0	0			

¹Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A

²Person-Trips

³Total estimate for all other land uses at mixed-use development site-not subject to internal trip capture computations in this estimator *Indicates computation that has been rounded to the nearest whole number.

	NCHRP 8-51 Internal Trip Capture Estimation Tool								
Project Name:	Downtown Taft Specific Plan	Organization:							
Project Location:	Taft, CA		Performed By:						
Scenario Description:			Date:						
Analysis Year:	Horizon Year 2042		Checked By:						
Analysis Period:	PM Street Peak Hour		Date:						

	Table 1-	P: Base Vehicle	-Trip Generation	Est	imates (Single-Use Sit	te Estimate)		
Land Use	Developme	Development Data (For Information Only)			Estimated Vehicle-Trips			
	ITE LUCs1	Quantity	Units		Total	Entering	Exiting	
Office					1803	292	1511	
Retail					2726	1308	1418	
Restaurant					0			
Cinema/Entertainment					0			
Residential					1948	1187	761	
Hotel					0			
All Other Land Uses ²					54	15	39	
Total					6531	2802	3729	

	Table 2-P: Mode Split and Vehicle Occupancy Estimates									
Land Use	Entering Trips				Exiting Trips					
Land Ose	Veh. Occ.	% Transit	% Non-Motorized		Veh. Occ.	% Transit	% Non-Motorized			
Office	1.00				1.00					
Retail	1.00				1.00					
Restaurant	1.00				1.00					
Cinema/Entertainment										
Residential										
Hotel										
All Other Land Uses ²										

	Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)									
Origin (From)		Destination (To)								
Oligili (Floili)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office										
Retail										
Restaurant										
Cinema/Entertainment										
Residential										
Hotel										

Table 4-P: Internal Person-Trip Origin-Destination Matrix*										
Origin (Fram)		Destination (To)								
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		105	0	0	30	0				
Retail	28		0	0	369	0				
Restaurant	0	0		0	0	0				
Cinema/Entertainment	0	0	0		0	0				
Residential	30	131	0	0		0				
Hotel	0	0	0	0	0					

Table 5-P	Table 5-P: Computations Summary								
	Total	Entering	Exiting						
All Person-Trips	6,531	2,802	3,729						
Internal Capture Percentage	21%	25%	19%						
External Vehicle-Trips ³	5,145	2,109	3,036						
External Transit-Trips ⁴	0	0	0						
External Non-Motorized Trips ⁴	0	0	0						

Table 6-P: Interna	Table 6-P: Internal Trip Capture Percentages by Land Use								
Land Use	Entering Trips	Exiting Trips							
Office	20%	9%							
Retail	18%	28%							
Restaurant	N/A	N/A							
Cinema/Entertainment	N/A	N/A							
Residential	34%	21%							
Hotel	N/A	N/A							

¹Land Use Codes (LUCs) from *Trip Generation Informational Report*, published by the Institute of Transportation Engineers.

²Total estimate for all other land uses at mixed-use development site-not subject to internal trip capture computations in this estimator

³Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P

⁴Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

Estimation Tool Developed by the Texas Transportation Institute

Project Name:	Downtown Taft Specific Plan
Analysis Period:	PM Street Peak Hour

	Table 7-P: Conversion of Vehicle-Trip Ends to Person-Trip Ends									
Land Use	Table 7-P (D): Entering Trips				Table 7-P (O): Exiting Trips					
	Veh. Occ.	Vehicle-Trips	Person-Trips*	i	Veh. Occ.	Vehicle-Trips	Person-Trips*			
Office	1.00	292	292		1.00	1511	1511			
Retail	1.00	1308	1308		1.00	1418	1418			
Restaurant	1.00	0	0		1.00	0	0			
Cinema/Entertainment	1.00	0	0		1.00	0	0			
Residential	1.00	1187	1187		1.00	761	761			
Hotel	1.00	0	0		1.00	0	0			

	Table 8-P (D): Internal Pers	on-Trip Origin-De	stination Matrix (Computed	at Origin)	
Origin (From)				Destination (To)		
Oligili (Floili)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		302	60	0	30	0
Retail	28		411	57	369	71
Restaurant	0	0		0	0	0
Cinema/Entertainment	0	0	0		0	0
Residential	30	320	160	0		23
Hotel	0	0	0	0	0	

	Table 8-P (D):	Internal Person	-Trip Origin-Desti	nation Matrix (Computed at	Destination)	
Origin (Franc)				Destination (To)		
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		105	0	0	47	0
Retail	91		0	0	546	0
Restaurant	88	654		0	190	0
Cinema/Entertainment	18	52	0		47	0
Residential	166	131	0	0		0
Hotel	0	26	0	0	0	

	Tal	ole 9-P (D): Interi	nal and External T	rips	Summary (Entering Tr	ips)	
Destination Land Lles	Р	erson-Trip Estima	ites			External Trips by Mode*	
Destination Land Use	Internal	External	Total	Î l	Vehicles ¹	Transit ²	Non-Motorized ²
Office	58	234	292		234	0	0
Retail	236	1072	1308		1072	0	0
Restaurant	0	0	0		0	0	0
Cinema/Entertainment	0	0	0		0	0	0
Residential	399	788	1187		788	0	0
Hotel	0	0	0		0	0	0
All Other Land Uses ³	0	15	15		15	0	0

	Та	ble 9-P (O): Inter	nal and External 1	rip	s Summary (Exiting Trip	os)	
Origin Land Has	Po	erson-Trip Estima	tes			External Trips by Mode*	
Origin Land Use	Internal	External	Total	1	Vehicles ¹	Transit ²	Non-Motorized ²
Office	135	1376	1511		1376	0	0
Retail	397	1021	1418		1021	0	0
Restaurant	0	0	0		0	0	0
Cinema/Entertainment	0	0	0		0	0	0
Residential	161	600	761		600	0	0
Hotel	0	0	0		0	0	0
All Other Land Uses ³	0	39	39		39	0	0

¹Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P

²Person-Trips

³Total estimate for all other land uses at mixed-use development site-not subject to internal trip capture computations in this estimator

*Indicates computation that has been rounded to the nearest whole number.

APPENDIX D

KERN COG HORIZON YEAR 2042 WITH SPECIFIC PLAN REGIONAL MODEL ADT PLOT AND VOLUME POST-PROCESSING WORKSHEETS



EX AM	#1		N/S Stree	et:10th Street			E/W Street:		Kern Street (S	R-33)								
Existing ADT	Existing	EX AM		ting Link lumes		Future	ADT (2042)		Raw Turning ent Volumes	FUTURE Link Volumes		Verify	Minimum 10% Growth	Volume Adj.	Refined Turning Movement Volumes	Refined Link Volumes	Gap Closure	10% Gap
	Northbound	N.of	Kern Str	eet (SR-33)	%				Northbound		N of	Kern Street (SR-33)						
	Left	67	NB	362	53%	NB	602	111	Left	605	NB	605	74		111	605	4	61
9,782	Thru	181	SB	322	47%	SB	535	301	Thru	535	SB	535	199		301	535	0	54
	Right	129	Sum	684		Enter	16255	214	Right		Sum	1140	142		214			
	Southbound	S. of	Kern Str	eet (SR-33)					Southbound		S of	Kern Street (SR-33)						
	Left	57	NB	377	55%	NB	626	95	Left	626	NB	626	63		95	626	0	63
9,390	Thru	174	SB	303	45%	SB	504	289	Thru	506	SB	506	191		289	506	3	51
	Right	91	Sum	680		Enter	15604	151	Right		Sum	1133	100		151			
	Eastbound	E. of	10th Stre	et					Eastbound		E of	10th Street						
	Left	123	EB	416	56%	EB	700	207	Left	696	EB	696	135		207	696	-4	70
9,278	Thru	230	WB	332	44%	WB	559	387	Thru	559	WB	559	253		387	559	0	56
	Right	54	Sum	748		Enter	15612	91	Right		Sum	1255	59		91			
	Westbound	W.of	10th Stre	et					Westbound		W of	10th Street						
	Left	75	EB	407	53%	EB	685	126	Left	685	EB	685	83		126	685	0	68
11,803	Thru	199	WB	357	47%	WB	601	335	Thru	597	WB	597	219		335	597	-3	60
	Right	58	Sum	764		Enter	19861	98	Right		Sum	1282	64		98			
							-		-	-					-		-0.71089	

EX PM N/S Street: 10th Street E/W Street: Kern Street (SR-33) Refined **FUTURE** Minimum Turning Refined Existing Existing Link **FUTURE** Raw Turning Link 10% Volume Movement Link Gap ADT Existing Volumes Volumes Growth Volumes Closure 10% Gap EX PM Future ADT (2042) Movement Volumes Verify Adj. Volumes Northbound N.of Kern Street (SR-33) % Northbound N of Kern Street (SR-3 46% NB 168 611 61 Left 101 NB 366 NB 608 168 Left 611 611 111 Thru 212 SB 428 54% SB 711 352 Thru 711 SB 711 233 352 711 71 Right 794 1625 135 1323 89 135 81 Sum Enter Right Sum Kern Street (SR-33) Southbound S. of Southbound S of Kern Street (SR-3 131 Left 79 NB 394 52% NB 655 131 Left 655 NB 655 87 655 65 202 SB 368 48% SB 612 336 Thru 615 SB 615 222 336 615 61 Thru Righ 147 762 Enter 1560 244 Right Sum 1270 162 244 Sum Eastbound E. of 10th Street Eastbound E of 10th Stree Left EB 362 54% EΒ 609 192 Left 606 EΒ 606 125 192 606 61 Thru 202 WB 307 46% WB 517 340 Thru 517 WB 517 222 340 517 52 Righ 93 Sum 669 Enter 1561 156 Right Sum 1122 102 156 Westbound 10th Street Westbound W of 10th Stree W.of 123 80 688 69 Left 73 EB 409 48% EΒ 688 Left 688 EΒ 688 123 326 WB 213 74 Thru 194 WB 442 52% WB 744 Thru 739 739 326 739 Right 40 851 1427 Sum Enter 67 Right Sum 44 67

-1.84347

EX AM	#2		N/S Street:	6th Street			E/W Street:		Kern Street (S	R-33)								
Existing ADT	Existing	EX AM		ng Link Imes		Future	ADT (2042)		Raw Turning ent Volumes	FUTURE Link Volumes		Verify	Minimum 10% Growth	Volume Adj.	Refined Turning Movement Volumes	Refined Link Volumes	Gap Closure	10% Gap
	Northbound	N.of	Kern Stree	et (SR-33)	%				Northbound		N of	Kern Street (SR-33)						
	Left	17	NB	253	51%	NB	554	37	Left	494	NB	494	19	+20	57	504	-50	50
3,984		135	SB	245	49%	SB	536	296	Thru	536	SB	536	149		296	491	-45	49
3984	Right	21	Sum	498		Enter	8723	46	Right		Sum	1031	23	+15	61			
	Southbound	S. of	Kern Stree	et (SR-33)					Southbound		S of	Kern Street (SR-33)						
_	Left	43	NB	173	53%	NB	379	94	Left	379	NB	379	47	-10	84	414	35	
2,637	Thru		SB	156	47%	SB	342	280	Thru	327	SB	327	141	-20	260	347	6	35
2632	Right	74	Sum	329		Enter	5774	162	Right		Sum	706	81	-15	147			
	Eastbound	E. of	6th Street						Eastbound		E of	6th Street						
	Left	79	EB	291	48%	EB	490	133	Left	522	EB	522	87		133	527	37	53
8,364	<u>.</u>		WB	312	52%	WB	525	382	Thru	525	WB	525	250		382	535	10	54
8364	Right	12	Sum	603		Enter	14074	20	Right		Sum	1047	13	+20	40			
	Westbound	W.of	6th Street				_		Westbound		W of	6th Street						
	Left	16	EB	318	48%	EB	535	27	Left	535	EB	535	18		47	555	20	
9,237	Thru	_	WB	348	52%	WB	586	432	Thru	632	WB	632	283	-20	412	617	31	62
9237	Right	39	Sum	666		Enter	15543	66	Right		Sum	1167	43	+10	76			
				•		•	•		•		•				•		44.567	

EX PM			N/S Stree	et: <mark>6th Street</mark>			E/W Street:		Kern Street (S	R-33)								
Existing ADT	Existing	EX PM		ting Link olumes		Euture	ADT (2042)		Raw Turning	FUTURE Link Volumes		Verify	Minimum 10% Growth	Volume Adi.	Refined Turning Movement Volumes	Refined Link Volumes	Gap	10% Gap
ADT	Northbound	N.of		eet (SR-33)	%	i uture	ADT (2042)	MOVETTIC	Northbound		N of	Kern Street (SR-33)	Olowaii	Auj.	Volunics	Volumes	Ologuic	10 % Gap
	Left	22	NB	199	46%	NB	436	48	Left	375	NB	375	24	+10	58	400	-36	40
3,984	Thru	79	SB	233	54%	SB	510	173	Thru	510	SB	510	87	+15	188	480	-30	48
	Right	28	Sum	432		Enter	8723	61	Right		Sum	885	31	+5	66			
	Southbound	S. of	Kern Str	eet (SR-33)			_		Southbound		S of	Kern Street (SR-33)						
	Left	37	NB	129	44%	NB	282	81	Left	282	NB	282	41	-10	71	312	30	31
2,637	Thru	117	SB	161	56%	SB	353	256	Thru	330	SB	330	129		256	370	18	37
	Right	79	Sum	290		Enter	5774	173	Right		Sum	613	87	-20	153			
	Eastbound	E. of	6th Stree	et					Eastbound		E of	6th Street						
	Left	78	EB	310	51%	EB	522	131	Left	555	EB	555	86		131	550	28	55
8,364	Thru	245	WB	293	49%	WB	493	412	Thru	493	WB	493	270		412	523	30	52
	Right	17	Sum	603		Enter	14074	29	Right		Sum	1048	19	+20	49			
	Westbound	W.of	6th Stree	et					Westbound		W of	6th Street						
	Left	27	EB	340	51%	EB	572	45	Left	572	EB	572	30	+20	65	592	20	59
9,237	Thru	224	WB	325	49%	WB	547	377	Thru	598	WB	598	246		377	588	41	59
	Right	42	Sum	665		Enter	15543	71	Right		Sum	1170	46	+10	81			
•			•			•		•									101.0079	

EX AM	#3		N/S Stree	t: <mark>4th Street</mark>			E/W Street:		Kern Street (S	R-33)								
Existing ADT	Existing	EX AM		ing Link umes		Future	ADT (2042)		Raw Turning ent Volumes	FUTURE Link Volumes		Verify	Minimum 10% Growth	Volume Adj.	Refined Turning Movement Volumes	Refined Link Volumes	Gap Closure	10% Gap
	Northbound	N.of	Kern Stre	et (SR-33)	%				Northbound		N of	Kern Street (SR-33)						
	Left	40	NB	172	50%	NB	435	101	Left	371	NB	371	44	-10	91	396	-39	40
2,776	Thru	96	SB	175	50%	SB	443	243	Thru	443	SB	443	106	+5	248	428	-15	43
2776	Right	21	Sum	347		Enter	7027	53	Right		Sum	814	23		53			
	Southbound	S. of	Kern Stre	et (SR-33)					Southbound		S of	Kern Street (SR-33)						
	Left	34	NB	157	53%	NB	397	86	Left	397	NB	397	37	-5	81	392	-5	39
2,376	Thru	104	SB	140	47%	SB	354	263	Thru	324	SB	324	114		263	364	9	36
2376	Right	37	Sum	297		Enter	6015	94	Right		Sum	721	41	-10	84			
	Eastbound	E. of	4th Street	t					Eastbound		E of	4th Street						
	Left	49	EB	244	50%	EB	411	82	Left	457	EB	457	54	+10	92	452	42	45
7,520	Thru	189	WB	248	50%	WB	417	318	Thru	417	WB	417	208		318	447	30	45
-	Right	20	Sum	492		Enter	12654	34	Right		Sum	875	22	+20	54			
	Westbound	W.of	4th Street	t					Westbound		W of	4th Street						
	Left	16	EB	258	48%	EB	434	27	Left	434	EB	434	18	+20	47	464	30	46
7,672	Thru	205	WB	282	52%	WB	475	345	Thru	540	WB	540	226		345	520	45	52
	Right	27	Sum	540		Enter	12910	45	Right		Sum	974	30	+10	55			
		•		•					•		•	•			•		96.97746	

EX PM			N/S Street:	4th Street			E/W Street:		Kern Street (S	R-33)								
Existing ADT	Existing	EX PM	Existin Volur			Future	ADT (2042)		Raw Turning	FUTURE Link Volumes		Verify	Minimum 10% Growth	Volume Adj.	Refined Turning Movement Volumes	Refined Link Volumes	Gap Closure	10% Gap
Ì	Northbound	N.of	Kern Stree	t (SR-33)	%				Northbound		N of	Kern Street (SR-33)						
	Left	37	NB	150	53%	NB	380	94	Left	314	NB	314	41	-10	84	349	-30	35
2,776	Thru	73	SB	132	47%	SB	334	185	Thru	334	SB	334	80	+15	200	324	-10	32
	Right	20	Sum	282		Enter	7027	51	Right		Sum	649	22		51			
	Southbound	S. of	Kern Stree	t (SR-33)			_		Southbound		S of	Kern Street (SR-33)						
	Left	35	NB	130	52%	NB	329	89	Left	329	NB	329	39		89	334	5	33
2,376	Thru	64	SB	120	48%	SB	304	162	Thru	256	SB	256	70		162	296	-8	30
	Right	33	Sum	250		Enter	6015	84	Right		Sum	585	36	-10	74			
	Eastbound	E. of	4th Street				_		Eastbound		E of	4th Street						
	Left	38	EB	302	52%	EB	508	64	Left	555	EB	555	42	+10	74	555	47	55
7,520	Thru	247	WB	284	48%	WB	478	416	Thru	478	WB	478	272		416	508	30	51
7518	Right	27	Sum	586		Enter	12654	45	Right		Sum	1033	30	+20	65			
	Westbound	W.of	4th Street						Westbound		W of	4th Street						
	Left	29	EB	312	52%	EB	525	49	Left	525	EB	525	32	+20	69	555	30	
7,672	Thru	216	WB _	286	48%	WB	481	363	Thru	541	WB	541	238		363	521	39	52
7672	Right	39	Sum	598		Enter	12910	66	Right		Sum	1066	43	+10	76			

• • • • • • • • • • • • • • • • • • • •	9 5 29 9 0 8 -11 14
ADT	9 5 29 9 0 8
Northbound Nor	9 5 29 9
Left 8	0 8
South Sout	0 8
Right Sum Fight Sum	0 8
Right2 Southbound S. of Kern Street (SR-33)/E. Kern Street Southbound S. of Left 4 Left 4 Left 6 Left 80 NB 80 7 10 80	0 8 -11 14
Southbound S. of Kern Street (SR-33)/E. Kern Street Southbound S. of Left 4	0 8 -11 14
Left 4 Left 2 6 NB 16 34% NB 80 10 Left 80 NB 80 7 10 80 376 Thru 2 SB 31 66% SB 154 3 Thru 52 SB 156 2 +25 28 143 376 Right 23 Sum 47 Enter 1869 37 Right Sum 236 25 37 Eastbound E. of 1st Street Eastbound Left 20 EB 95 45% EB 160 34 Left 176 EB 176 22 -10 24 177 3.081 Thru 84 WB 115 55% WB 194 141 Thru 194 WB 194 92 -5 136 236 Right 85 Sum 210 Enter 5185 143 Right Sum 370 944 143 Right 23 Sum 370 944 143 Right 23 Sum 370 Pight 25 +21 60	0 8
Left2 6 NB 16 34% NB 80 10 Left 80 NB 80 7 10 80 Thru 2 SB 31 66% SB 154 3 Thru 52 SB 156 2 +25 28 143 Right 23 Sum 47 Enter 1869 37 Right Sum 236 25 37 Eastbound E. of 1st Street Left 20 EB 95 45% EB 160 34 Left 176 EB 176 22 -10 24 177 Thru 84 WB 115 55% WB 194 141 Thru 194 WB 194 92 -5 136 236 Right 85 Sum 210 Enter 5185 143 Right Right 23 Right2 23	0 8
376	-11 14
Sum Sum	-11 14
Eastbound E. of 1st Street E	
Left 20 EB 95 45% EB 160 34 Left 176 EB 176 22 -10 24 177 3,081 Thru 84 WB 115 55% WB 194 141 Thru Right 85 Sum 210 Enter 5185 143 Right Right2 Sum 370 94 143 Right2 23 Right2	
3,081 Thru 84 WB 115 55% WB 194 141 Thru 194 WB 194 92 -5 136 236 Right 85 Sum 210 Enter 5185 143 Right Right 2 3 Sum 210 Enter 5185 143 Right Right 2 5 +21 60	17 18
Right 85 Sum 210 Enter 5185 143 Right Sum 370 94 143 Right Right2 23 89 Right2 50 421 60	42 24
Right2 23 Right2 25 +21 60	42 24
	-
Left2 2 3 Left2 2 +7 10	
Left 5 EB 212 51% EB 357 8 Left 357 EB 357 6 +35 43 363	6 36
6,123 Thru 102 WB 201 49% WB 338 172 Thru 363 WB 363 112 172 363	25 36
Right 6 Sum 413 Enter 10303 10 Right Sum 719 7 10	20 00
Northwestbound SE.of 1st Street NWbound SE of 1st Street	
Left2 1 +10 12	
Left 68 SEB 95 57% SEB 160 114 Left 166 SEB 166 75 114 173	13 17
2,929 Thru 1 NWB 72 43% NWB 121 2 Thru 121 NWB 121 1 +8 10 146	25 15
Right 2 Sum 167 Enter 4929 3 Right Sum 287 2 +7 10	
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	

EX PM			N/S Street:	1st Street			E/W Street:		Kern Street (SI	R-33)/E. Kern S	treet							
Existing								FUTURE	Raw Turning	FUTURE Link			Minimum 10%	Volume	Refined Turning Movement	Refined Link	Gap	
ADT	Existing	EX PM	Existing Linl	k Volumes		Future	ADT (2042)	Moveme	ent Volumes	Volumes		Verify	Growth	Adj.	Volumes	Volumes	Closure	10% Gap
Ī	Northbound	N.of	Kern Street	(SR-33)/E	%				Northbound		N of	Kem Street (SR-32)/E. Kem Street						
	Left	9	NB	28	51%	NB	45	45	Left	54	NB	54	10		45	50	5	5
504	Thru	2	SB	27	49%	SB	43	10	Thru	43	SB	43	2		10	67	24	7
	Right	4	Sum	55		Enter	808	20	Right		Sum	97	4		20			
_	Right2	1					•	5	Right2				1	+5	10			
	Southbound		Kern Street	(SR-33)/E.	Kern Str	eet			Southbound		S of	Kem Street (SR-32)/E. Kem Street						1
	Left	2						3	Left2				2	+7	10			
	Left2		NB	16	43%	NB	80	8	Left	80	NB	80	6	+2	10	85	5	8
376	Thru	3	SB	21	57%	SB	104	5	Thru	35	SB	160	3	+15	20	97	-7	10
L	Right	17	Sum	37		Enter	1869	27	Right		Sum	239	19		27			
	Eastbound	E. of	1st Street						Eastbound		E of	1st Street						
	Left	11	EB	146	62%	EB	246		Left	259	EB	259	12	-3	16	273	27	27
3,081	Thru	139	WB _	91	38%	WB	153	234	Thru	151	WB	153	153	-1	233	184	31	18
3081	Right	84	Sum	237		Enter	5185	141	Right		Sum	412	92	-1	140			
L	Right2	10						17	Right2				11	+20	37			
	Westbound		1st Street				-		Westbound		W of	1st Street						
	Left2	1			E00/			2	Left2				1	+8	10			<u> </u>
0.400	Left	6	EB	244	52%	EB	411	10	Left		EB	411	/	+20	30	426	15	
6,123 6123	Thru	80	WB _	227 471	48%	WB	382	135 7	Thru	410	WB	410 821	88	+3	135 10	410	28	41
6123	Right	4	Sum	4/1		Enter	10303		Right		Sum SE of		4	+3	10			
	Northwestbound Left2	SE.of	1st Street					3	NWbound Left2		SE of	1st Street	0	+7	10			
		121	SEB	04	40%	CED	450		Leitz Left	156	CED	450	133	+/	204	170	17	47
2,929	Left Thru	121	NMB	91 135	40% 60%	SEB NWB	153 227	204 19	Leπ Thru	156 227	SEB NWB	156 227	133	-4	204 15	238	17	17 24
2,929		17	_	226	00%	Enter	4929	2	Right	221	Sum	383	12	+8	10	238	- 11	24
2938	Right		Sum	220		⊏nter	4929		rignt		ouill	383	l l	78	10		127.757	
																	121.131	

EX AM	#5		N/S Street	t: <mark>Westside H</mark>	ighway (S	SR-33)	E/W Street:		Center Street									
Existing ADT	Existing	EX AM	Existing Li	ink Volumes		Future	ADT (2042)		Raw Turning	FUTURE Link Volumes		Verify	Minimum 10% Growth	Volume Adj.	Refined Turning Movement Volumes	Refined Link Volumes	Gap Closure	10% Gap
	Northbound	N.of	Center St	reet	%				Northbound		N of	Center Street						
	Left	4	NB	84	47%	NB	141	7	Left	146	NB	146	4	+10	17	151	9	15
2,929	Thru	82	SB	95	53%	SB	160	138	Thru	160	SB	160	90		138	168	8	17
	Right	0	Sum	179		Enter	4929	0	Right		Sum	305	0		0	,		
	Southbound	S. of	Center St	reet					Southbound		S of	Center Street						
	Left	0	NB	86	47%	NB	145	0	Left	145	NB	145	0		0	155	10	15
3,187	Thru	92	SB	97	53%	SB	163	155	Thru	174	SB	174	101		155	174	10	17
	Right	3	Sum	183		Enter	5363	5	Right		Sum	318	3	+8	13			
	Eastbound	E. of	Westside	Highway (SI	R-33)				Eastbound		E of	Westside Highway (SR-33)						
	Left	2	EB	0	0%	EB	0	8	Left	0	EB	0	2	+5	13	0	0	0
48	Thru	0	WB	0	0%	WB	0	0	Thru	0	WB	0	0		0	0	0	0
	Right	5	Sum	0		Enter	72	19	Right		Sum	0	6		19			
	Westbound	W.of	Westside	Highway (SI					Westbound		W of	Westside Highway (SR-33)						
	Left	0	EB	7	50%	EB	26	0	Left	26	EB	26	0		0	31	5	3
331	Thru	0	WB	7	50%	WB	26	0	Thru	12	WB	12	0		0	30	3	3
	Right	0	Sum	14		Enter	1247	0	Right		Sum	38	0		0			
				•						•					•		46.00058	

N/S Street: Westside Highway (SR-33) EX PM E/W Street: Center Street Refined **FUTURE** Minimum Turning Refined Existing **FUTURE** Raw Turning Link 10% Volume Movement Link Gap ADT Existing Volumes Growth Volumes Closure 10% Gap EX PM Existing Link Volumes Future ADT (2042) Movement Volumes Verify Adj. Volumes Northbound N.of Center Street Northbound N of Center Stree 5 NB 121 60% NB 204 206 +10 18 22 Left 8 206 NB 6 216 14 119 SB 81 40% SB 136 200 136 SB 136 131 200 144 Thru Thru 4929 Righ 0 Sum 202 Enter 0 Right Sum 342 0 0 Southbound S. of Center Street Southbound S of Center Stree NB 22 NB 124 NB 209 209 209 2 219 10 Left 2 58% 3 Left 3 SB 17 Thru 78 SB 91 42% SB 153 131 Thru 180 180 86 131 Riah Sum 215 Enter 5363 2 Riaht Sum 389 +8 10 Eastbound E. of Westside Highway (SR-33) Eastbound E of EΒ 2 50% EB 4 Left EΒ +10 14 Lef 3 3 0 WB 2 50% WB WB Thr 0 Thru 3 3 3 Righ Sum 4 Enter 49 Right Sum 6 14 -10 Westbound W.of Westside Highway (SR-33) Westbound W of EΒ 14 EB 53 53 EΒ 53 0 53 Lef 0 67% 0 Left 0 7 Thru WB 33% WB 26 2 Thru 12 WB 12 2 30 21 Right 1247 Right 64 Sum Enter Sum

EX AM	#6		N/S Street	:10th Street			E/W Street:	:	Center Street									
Existing			Existir	ng Link				FUTURE	Raw Turning	FUTURE Link			Minimum 10%	Volume	Refined Turning Movement	Refined Link	Gap	
ADT	Existing	EX AM	Volu	umes		Future	ADT (2042)	Moveme	ent Volumes	Volumes		Verify	Growth	Adj.	Volumes	Volumes	Closure	10% Gap
	Northbound	N.of	Center Sti	reet	%				Northbound		N of	Center Street						
	Left	0	NB	391	56%	NB	650	0	Left	658	NB	658	0		0	658	8	66
9,390	Thru	375	SB	308	44%	SB	512	623	Thru	512	SB	512	413		623	524	12	52
	Right	42	Sum	699		Enter	15604	70	Right		Sum	1169	46	+10	80			
	Southbound	S. of	Center Sti	reet					Southbound		S of	Center Street						
	Left	30	NB	417	59%	NB	693	50	Left	693	NB	693	33	+12	62	703	10	70
9,044	Thru	278	SB	293	41%	SB	487	462	Thru	494	SB	494	306		462	494	7	49
	Right	0	Sum	710		Enter	15029	0	Right		Sum	1187	0		0			
	Eastbound	E. of	10th Stree	et					Eastbound		E of	10th Street						
	Left	0	EB	72	70%	EB	155	0	Left	120	EB	120	0		0	142	-14	14
2,360	Thru	0	WB	31	30%	WB	67	0	Thru	67	WB	67	0		0	67	0	7
-	Right	0	Sum	103		Enter	5086	0	Right		Sum	186	0		0			
	Westbound	W.of	10th Stree	et			_		Westbound		W of	10th Street						
	Left	15	EB	0	0%	EB	0	32	Left	0	EB	0	17		32	0	0	0
	Thru	0	WB	0	0%	WB	0	0	Thru	0	WB	0	0		0	0	0	0
(Right	16	Sum	0		Enter	0	34	Right		Sum	0	18		34			
					•	•			•		•	•				•	23.77362	

EX PM			N/S Stre	et: 10th Street			E/W Street:		Center Street									
Existing ADT	Existing	EX PM		sting Link olumes		Future	ADT (2042)		Raw Turning	FUTURE Link Volumes		Verify	Minimum 10% Growth	Volume Adj.	Refined Turning Movement Volumes	Refined Link Volumes	Gap Closure	10% Gap
	Northbound	N.of	Center S		%	, ataro	7.2. (20.2)		Northbound		N of	Center Street	_	<u>-</u> -				1070 Cup
	Left	0	NB	392	50%	NB	651	0	Left	676	NB	676	0		0	676	24	68
9,390	Thru	343	SB	395	50%	SB	656	570	Thru	656	SB	656	377		570	676	20	68
9365	Right	51	Sum	787		Enter	15604	85	Right		Sum	1332	56	+15	100			
	Southbound		Center S						Southbound		S of	Center Street						
	Left	56	NB	394	52%	NB	655	93	Left	655	NB	655	62	+20	113	670	15	
9,044	Thru	339	SB	366	48%	SB	608	563	Thru	622	SB	622	373		563	622	13	62
9044	Right	0	Sum	760		Enter	15029	0	Right		Sum	1276	0		0			
	Eastbound	E. of	10th Str				_		Eastbound		E of	10th Street						
	Left	0	EB	107	58%	EB	231	0	Left	178	EB	178	0		0	213	-18	21
2,360	Thru	0	WB	76	42%	WB	164	0	Thru	164	WB	164	0		0	164	0	16
2361	Right	0	Sum	183		Enter	5086	0	Right		Sum	342	0		0			l .
	Westbound	W.of	10th Str	eet					Westbound		W of	10th Street						1
	Left	27	EB	0	0%	EB	0	58	Left	0	EB	0	30		58	0	0	0
0	Thru	0	WB	0	0%	WB	0	0	Thru	0	WB	0	0		0	0	0	0
0	Right	49	Sum	0		Enter	0	106	Right		Sum	0	54		106			<u> </u>

APPENDIX E

HORIZON YEAR 2042 WITH PROJECT INTERSECTION OPERATIONS ANALYSIS WORKSHEETS

	۶	→	•	•	←	•	4	†	/	/	ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ť	∱ ∱		ሻ	ተ ኈ		7	ተ ኈ		ሻ	^	7
Traffic Volume (veh/h)	207	387	91	126	335	98	111	301	214	95	289	151
Future Volume (veh/h)	207	387	91	126	335	98	111	301	214	95	289	151
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		0.98	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	269	503	118	164	435	127	144	391	278	123	375	196
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	300	833	194	194	628	182	173	547	384	151	931	414
Arrive On Green	0.17	0.29	0.29	0.11	0.23	0.23	0.10	0.27	0.27	0.08	0.26	0.26
Sat Flow, veh/h	1781	2859	667	1781	2708	783	1781	1991	1399	1781	3554	1580
Grp Volume(v), veh/h	269	312	309	164	284	278	144	348	321	123	375	196
Grp Sat Flow(s), veh/h/ln	1781	1777	1749	1781	1777	1714	1781	1777	1613	1781	1777	1580
Q Serve(g_s), s	15.1	15.4	15.6	9.3	15.0	15.2	8.1	18.1	18.4	7.0	8.9	10.7
Cycle Q Clear(g_c), s	15.1	15.4	15.6	9.3	15.0	15.2	8.1	18.1	18.4	7.0	8.9	10.7
Prop In Lane	1.00		0.38	1.00		0.46	1.00		0.87	1.00		1.00
Lane Grp Cap(c), veh/h	300	517	509	194	412	398	173	488	443	151	931	414
V/C Ratio(X)	0.90	0.60	0.61	0.84	0.69	0.70	0.83	0.71	0.72	0.81	0.40	0.47
Avail Cap(c_a), veh/h	334	711	700	210	588	567	188	573	520	162	1093	486
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	41.7	31.2	31.2	44.8	35.9	36.1	45.4	33.5	33.6	46.1	31.2	31.8
Incr Delay (d2), s/veh	22.6	5.1	5.3	22.6	5.6	6.0	22.4	8.6	9.9	22.9	1.3	3.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	8.4	7.2	7.2	5.2	7.0	6.9	4.6	8.8	8.2	4.0	3.9	4.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	64.3	36.3	36.6	67.3	41.5	42.1	67.8	42.1	43.5	69.0	32.5	35.7
LnGrp LOS	E	D	D	E	D	D	E	D	D	E	С	D
Approach Vol, veh/h		890			726			813			694	
Approach Delay, s/veh		44.9			47.6			47.2			39.8	
Approach LOS		D			T7.0			D			D	
• •											D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	16.9	36.6	15.7	33.2	22.9	30.6	14.4	34.5				
Change Period (Y+Rc), s	* 5.7	6.8	* 5.7	6.4	* 5.7	6.8	* 5.7	6.4				
Max Green Setting (Gmax), s	* 12	41.0	* 11	31.5	* 19	33.9	* 9.3	33.0				
Max Q Clear Time (g_c+I1), s	11.3	17.6	10.1	12.7	17.1	17.2	9.0	20.4				
Green Ext Time (p_c), s	0.0	10.3	0.0	7.8	0.1	6.0	0.0	7.3				
Intersection Summary												
HCM 6th Ctrl Delay			45.0									
HCM 6th LOS			D									
Notes												

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

HCM Control Delay

HCM Lane LOS

HCM 95th-tile Q

68.3

8

65.1 225.5

21.7

7.9

77.5 107.5 136.4

12.2

9.8

F

15

71.5 127.5

8.7 14.1

17.7											
F											
EDI EDT	EDD	WDI	WDT	WDD	MDI	NDT	NIDD	CDI	CDT	CDD	
	EDK	WDL		WDK	INDL		INDK	SDL		SDK	
	40	47		7/	F7		/1	0.4		1 17	
0 2	0	0	2	0	0	2	0	0	2	0	
EB		WB			NB			SB			
WB		EB			SB			NB			
2		2			2			2			
SB		NB			EB			WB			
2		2			2			2			
hNB		SB			WB			EB			
2		2			2			2			
53.9		122.7			66.7			103.1			
F		F			F			F			
NRI n1	MRI n2 l	FRI n1 l	FRI n?\	//RI n1\/	//RI n2 '	SRI n1 9	SRI n2				
					-						
•	•	•			*	•					
10.687		9.735 1.451		9.778 1.138			1.235				
	EBL EBT 133 382 133 382 0.73 0.73 2 2 182 523 0 2 EB WB 2 SB 2 2 nNB 2 2 33.9 F NBLn1 28% 72% 0% Stop 205 57 148 0 281 7 0.896 12.987 Yes 281	EBL EBT EBR 133 382 40 133 382 40 0.73 0.73 0.73 2 2 2 2 182 523 55 0 2 0 EB WB 2 SB 2 NBB 2 SB 2 SB 2 NBB 2 SB 2 SB 2 SB 2 NBB 2 SB 2	EBL EBT EBR WBL 133 382 40 47 133 382 40 47 0.73 0.73 0.73 0.73 2 2 2 2 2 182 523 55 64 0 2 0 0 EB WB WB EB 2 2 2 SB NB 2 2 2 SB NB 2 2 2 SNB SB 2 523 55 64 0 7 7 7 0.896 0.886 1.389 12.987 12.625 12.035 Yes Yes Yes	EBL EBT EBR WBL WBT 133 382 40 47 412 133 382 40 47 412 0.73 0.73 0.73 0.73 0.73 2 2 2 2 2 2 182 523 55 64 564 0 2 0 0 2 EB WB WB EB 2 2 2 SB NB 2 2 2 SB NB 2 2 2 SB NB 2 2 2 SSB NB 2 2 53.9 122.7 F F F NBLn1 NBLn2 EBLn1 EBLn2V 28% 0% 41% 0% 72% 71% 59% 83% 0% 29% 0% 17% Stop Stop Stop Stop 205 209 324 231 57 0 133 0 148 148 191 191 0 61 0 40 281 286 444 316 7 7 7 7 0.896 0.886 1.389 0.962 12.987 12.625 12.035 11.69 Yes Yes Yes Yes 281 289 306 313	EBL EBT EBR WBL WBT WBR 133 382 40 47 412 76 133 382 40 47 412 76 137 0.73 0.73 0.73 0.73 0.73 2 2 2 2 2 2 2 2 182 523 55 64 564 104 0 2 0 0 2 0 EB WB WB EB 2 2 2 SB NB 2 53.9 122.7 F F NBLn1 NBLn2 EBLn1 EBLn2WBLn1V 28% 0% 41% 0% 19% 72% 71% 59% 83% 81% 0% 29% 0% 17% 0% Stop Stop Stop Stop Stop 205 209 324 231 253 57 0 133 0 47 148 148 191 191 206 0 61 0 40 0 281 286 444 316 347 7 7 7 7 7 0.896 0.886 1.389 0.962 1.067 12.987 12.625 12.035 11.69 12.078 1 Yes Yes Yes Yes Yes Yes 281 289 306 313 305	EBL EBT EBR WBL WBT WBR NBL 133 382 40 47 412 76 57 133 382 40 47 412 76 57 133 382 40 47 412 76 57 133 0.73 0.73 0.73 0.73 0.73 0.73 2 2 2 2 2 2 2 2 2 2 182 523 55 64 564 104 78 0 2 0 0 2 0 0 2 0 0 EB WB NB WB EB SB 2 2 2 2 2 2 2 SSB NB EB 2 2 2 2 2 2 SSB NB EB 2 2 2 2 2 2 SSB NB EB 2 52 3 55 64 564 6.7 F F F F F NBLn1 NBLn2 EBLn1 EBLn2WBLn1WBLn2 SB 2 2 2 2 2 53.9 122.7 66.7 F F F F NBLn1 NBLn2 EBLn1 EBLn2WBLn1WBLn2 SB 2 88 0% 41% 0% 19% 0% 72% 71% 59% 83% 81% 73% 0% 29% 0% 17% 0% 27% Stop Stop Stop Stop Stop Stop Stop Stop	EBL EBT EBR WBL WBT WBR NBL NBT 133 382 40 47 412 76 57 296 133 382 40 47 412 76 57 296 133 0.73 0.73 0.73 0.73 0.73 0.73 0.73 2 2 2 2 2 2 2 2 2 2 2 2 2 182 523 55 64 564 104 78 405 0 2 0 0 2 0 0 2 0 0 2 EB WB NB WB EB SB 2 2 2 2 2 2 2 2 SB NB EB 2 2 2 2 2 2 2 2 SB NB EB 2 2 2 2 2 2 2 3 39 TBB SB WB NB WB EB SB 2 2 2 2 2 3 39 TBB SB SB SB 2 3 39 122.7 666.7 F F F F F NBLn1 NBLn2 EBLn1 EBLn2WBLn1WBLn2 SBLn1 SB	EBL EBT EBR WBL WBT WBR NBL NBT NBR 133 382 40 47 412 76 57 296 61 133 382 40 47 412 76 57 296 61 133 382 40 47 412 76 57 296 61 133 382 40 47 412 76 57 296 61 133 382 40 47 412 76 57 296 61 133 382 40 47 412 76 57 296 61 133 382 40 47 412 76 57 296 61 133 382 40 47 412 76 57 296 61 133 382 40 47 412 76 57 296 61 133 382 40 47 412 76 57 296 61 133 382 40 47 412 76 57 296 61 133 382 40 47 412 76 57 296 61 133 382 40 47 412 78 405 84 0	EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL 133 382 40 47 412 76 57 296 61 84 133 382 40 47 412 76 57 296 61 84 133 382 40 47 412 76 57 296 61 84 137 0.73 0.73 0.73 0.73 0.73 0.73 0.73 0.	EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT 133 382 40 47 412 76 57 296 61 84 260 133 382 40 47 412 76 57 296 61 84 260 133 382 40 47 412 76 57 296 61 84 260 133 382 40 47 412 76 57 296 61 84 260 133 382 40 47 412 76 57 296 61 84 260 133 382 40 47 412 76 57 296 61 84 260 133 382 40 47 412 76 57 296 61 84 260 133 382 40 47 412 76 57 296 61 84 260 133 382 40 47 412 76 57 296 61 84 260 133 382 40 47 412 76 57 296 61 84 260 133 382 40 47 412 76 57 296 61 84 260 133 382 40 47 412 76 57 296 61 84 260 133 382 40 47 412 76 57 296 61 84 260 148 148 191 191 206 206 130 130 148 148 191 191 206 206 130 130 148 148 191 191 206 206 130 130 148 148 191 191 206 206 130 130 148 148 191 191 206 206 130 130 148 148 191 191 206 206 130 130 148 148 191 191 206 206 130 130 148 148 191 191 206 206 130 130 148 148 191 191 206 206 130 130 149 281 286 444 316 347 386 293 379 7	EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR 133

Intersection													
Intersection Delay, s/ve	h 18.8												
Intersection LOS	F												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		414			414			4			4		
Traffic Vol, veh/h	92	318	54	47	345	55	91	248	53	81	263	84	
Future Vol, veh/h	92	318	54	47	345	55	91	248	53	81	263	84	
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	116	403	68	59	437	70	115	314	67	103	333	106	
Number of Lanes	0	2	0	0	2	0	0	1	0	0	1	0	
Approach	EB			WB			NB			SB			
Opposing Approach	WB			EB			SB			NB			
Opposing Lanes	2			2			1			1			
Conflicting Approach Le	eft SB			NB			EB			WB			
Conflicting Lanes Left	1			1			2			2			
Conflicting Approach Ri	ghtNB			SB			WB			EB			
Conflicting Lanes Right	1			1			2			2			
HCM Control Delay	46.9			41.3			179.1			222.5			
HCM LOS	Ε			Ε			F			F			
Lane	N	NBLn1 I	EBLn1 I	EBLn2V	VBLn1V	VBLn2	SBLn1						
Vol Left, %		23%	37%	0%	21%	0%	19%						
Vol Thru, %		63%	63%	75%	79%	76%	61%						
Vol Right, %		14%	0%	25%	0%	24%	20%						
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop						
Traffic Vol by Lane		392	251	213	220	228	428						
LT Vol		91	92	0	47	0	81						
Through Vol		248	159	159	173	173	263						
RT Vol		53	0	54	0	55	84						
Lane Flow Rate		496	318	270	278	288	542						
Geometry Grp		2	7	7	7	7	2						
Degree of Util (X)		1.286	0.859	0.702	0.749	0.754	1.397						
Departure Headway (Ho	d) 1					11.108							
Convergence, Y/N		Yes	Yes			Yes	Yes						
Cap		347	321	329	320	328	363						
Service Time						8.808							
HCM Lane V/C Ratio						0.878							
HCM Control Delay		179.1	56.1	36.1	41.5		222.5						
HCM Lane LOS		F	F	Е	Ε	Ε	F						
HCM 95th-tile Q		20.3	7.7	5	5.7	5.8	24.9						

Scenario 3: 3 HY 2042 AM

Intersection Level Of Service Report Intersection 4: Kern Street (SR-33) / 1st Street

Control Type:Two-way stopDelay (sec / veh):47.6Analysis Method:HCM 6th EditionLevel Of Service:EAnalysis Period:15 minutesVolume to Capacity (v/c):0.084

Intersection Setup

Name		1st S	Street			1st S	street		Kern Street (SR-33)					
Approach		North	bound			South	bound			Eastb	ound			
Lane Configuration		+				4	→			41				
Turning Movement	Left2	Left	Thru	Right	Left2	Left	Thru	Right	Left2	Left	Thru	Right		
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00		
No. of Lanes in Entry Pocket	0	0 0 0 0		0	0	0	0	0	0	0	0			
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00		
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0		
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Speed [mph]	30.00			30.00				30.00						
Grade [%]	0.00			0.00				0.00						
Crosswalk	No			No				No						

Volumes

Name	1st Street				1st S	Street			Kern Stree	et (SR-33))	
Base Volume Input [veh/h]	40	10	20	10	10	10	28	37	24	136	143	60
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	0.00	2.00	0.00	2.00	2.00	2.00	2.00	0.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	40	10	20	10	10	10	28	37	24	136	143	60
Peak Hour Factor	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	11	3	6	3	3	3	8	11	7	39	41	17
Total Analysis Volume [veh/h]	46	11	23	11	11	11	32	43	28	156	164	69
Pedestrian Volume [ped/h]	0			0					0			

Scenario 3: 3 HY 2042 AM

Intersection Settings

Priority Scheme	Stop	Stop	Free
Flared Lane	No	No	
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance	No	No	
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.29	0.08	0.08	0.01	0.04	0.04	0.11	0.04	0.02	0.11	0.00	0.00	
d_M, Delay for Movement [s/veh]	43.27	47.65	32.11	24.14	19.44	19.46	20.36	11.27	7.83	7.81	0.00	0.00	
Movement LOS	Е	E	D	С	С	С	С	В	Α	Α	Α	Α	
95th-Percentile Queue Length [veh/ln]	2.24	2.24	2.24	2.24	0.88	0.88	0.88	0.88	0.43	0.43	0.21	0.00	
95th-Percentile Queue Length [ft/ln]	56.00	56.00	56.00	56.00	22.03	22.03	22.03	22.03	10.75	10.75	5.37	0.00	
d_A, Approach Delay [s/veh]		38.	.67			16	.12			3.4	45		
Approach LOS		E				()		A				
d_I, Intersection Delay [s/veh]						10	.34						
Intersection LOS	E												

Scenario 3: 3 HY 2042 AM

Intersection Setup

Name		Kern Stre	et (SR-33)			E Kerr	n Street			
Approach		West	bound			Southwe	estbound			
Lane Configuration		Н	þ			Y	' F			
Turning Movement	Left	Thru	Right	Right2	Left	Thru	Right	Right2		
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00		
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0		
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00		
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0		
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Speed [mph]		30	.00		30.00					
Grade [%]		0.	00		0.00					
Crosswalk		١	lo		No					

Volumes

Name		Kern Stree	et (SR-33)			E Kerr	Street			
Base Volume Input [veh/h]	12	114	10	10	10	43	172	10		
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	0.00	0.00	0.00	0.00	0.00		
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0		
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0		
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0		
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0		
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0		
Other Volume [veh/h]	0	0	0	0	0	0	0	0		
Total Hourly Volume [veh/h]	12	114	10	10	10	43	172	10		
Peak Hour Factor	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700		
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		
Total 15-Minute Volume [veh/h]	3	33	3	3	3	12	49	3		
Total Analysis Volume [veh/h]	14	131	11	11	11	49	198	11		
Pedestrian Volume [ped/h])		0					

Scenario 3: 3 HY 2042 AM

Intersection Settings

Priority Scheme	Free	Stop
Flared Lane		
Storage Area [veh]	0	0
Two-Stage Gap Acceptance		No
Number of Storage Spaces in Median	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.01	0.00	0.00	0.00	0.04	0.20	0.28	0.02		
d_M, Delay for Movement [s/veh]	7.73	0.00	0.00	0.00	23.82	24.86	12.77	11.19		
Movement LOS	Α	Α	А	A	С	С	В	В		
95th-Percentile Queue Length [veh/ln]	0.03	0.03	0.03	0.03	1.55	1.55	1.12	0.70		
95th-Percentile Queue Length [ft/ln]	0.80	0.80	0.80	0.80	38.63	38.63	28.11	17.60		
d_A, Approach Delay [s/veh]		0.0	65			15	.36			
Approach LOS		F	4			(
d_I, Intersection Delay [s/veh]				10	1.34					
Intersection LOS				i	E					

Intersection												
Int Delay, s/veh	1.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	13	0	19	0	0	0	17	138	0	0	155	13
Future Vol, veh/h	13	0	19	0	0	0	17	138	0	0	155	13
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	15	0	22	0	0	0	19	157	0	0	176	15
Major/Minor N	Minor2			Minor1			Major1		1	Major2		
Conflicting Flow All	379	379	184	390	386	157	191	0	0	157	0	0
Stage 1	184	184	-	195	195	-	-	-	-	-	-	-
Stage 2	195	195	-	195	191	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	579	553	858	569	548	889	1383	-	-	1423	-	-
Stage 1	818	747	-	807	739	-	-	-	-	-	-	-
Stage 2	807	739	-	807	742	-	-	-	-	-	-	-
Platoon blocked, %				=	=		1000	-	-		-	-
Mov Cap-1 Maneuver	573	545	858	549	540	889	1383	-	-	1423	-	-
Mov Cap-2 Maneuver	573	545	-	549	540	-	-	-	-	-	-	-
Stage 1	806	747	-	795	728	-	-	-	-	-	-	-
Stage 2	795	728	-	787	742	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	10.3			0			0.8			0		
HCM LOS	В			Α								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1383	-	-		-		-	-			
HCM Lane V/C Ratio		0.014	-	-	0.051	-	-	-	-			
HCM Control Delay (s)		7.6	0	-		0	0	-	-			
HCM Lane LOS		Α	A	-	В	A	A	-	-			
HCM 95th %tile Q(veh))	0	-	-	0.2	-	0	-	-			

Intersection						
Int Delay, s/veh	1.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		↑ ⊅		ነ	^
Traffic Vol, veh/h	32	34	623	80	62	462
Future Vol, veh/h	32	34	623	80	62	462
Conflicting Peds, #/hr	0	0	023	0	02	0
Sign Control		Stop	Free	Free	Free	Free
RT Channelized	Stop	None		None		None
	-		-		- E0	none
Storage Length	0	-	-	-	50	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	76	76	76	76	76	76
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	42	45	820	105	82	608
Major/Minor	Minor1	N	/lajor1	N	/lajor2	
						0
Conflicting Flow All	1341	463	0	0	925	0
Stage 1	873	-	-	-	-	-
Stage 2	468	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	144	546	-	-	734	-
Stage 1	369	-	-	-	-	-
Stage 2	597	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	128	546	-	-	734	-
Mov Cap-2 Maneuver	254	-	_	-	-	_
Stage 1	369	_	_	_	-	_
Stage 2	530	_	_		_	
Jiage 2	330	-		-	-	
Approach	WB		NB		SB	
HCM Control Delay, s	18.6		0		1.2	
HCM LOS	С					
Minor Long (Maior M.		NDT	MDD	VDI 1	CDI	CDT
Minor Lane/Major Mvn	10	NBT		VBLn1	SBL	SBT
Capacity (veh/h)		-	-	351	734	-
HCM Lane V/C Ratio		-	-	0.247		-
HCM Control Delay (s)		-	-	18.6	10.5	-
HCM Lane LOS		-	-	С	В	-
HCM 95th %tile Q(veh)	-	-	1	0.4	-

	۶	→	•	•	←	•	4	†	/	/	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	∱ ∱		ሻ	ተ ኈ		7	ተ ኈ		ሻ	^	7
Traffic Volume (veh/h)	192	340	156	123	326	67	168	352	135	131	336	244
Future Volume (veh/h)	192	340	156	123	326	67	168	352	135	131	336	244
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		0.99	1.00		1.00	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	206	366	168	132	351	72	181	378	145	141	361	262
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	240	659	297	162	686	139	213	704	266	172	910	400
Arrive On Green	0.13	0.28	0.28	0.09	0.23	0.23	0.12	0.28	0.28	0.10	0.26	0.26
Sat Flow, veh/h	1781	2375	1072	1781	2939	596	1781	2519	952	1781	3554	1561
Grp Volume(v), veh/h	206	272	262	132	211	212	181	265	258	141	361	262
Grp Sat Flow(s), veh/h/ln	1781	1777	1670	1781	1777	1758	1781	1777	1694	1781	1777	1561
Q Serve(g_s), s	10.9	12.6	12.9	7.0	9.9	10.1	9.6	12.2	12.4	7.5	8.1	14.4
Cycle Q Clear(g_c), s	10.9	12.6	12.9	7.0	9.9	10.1	9.6	12.2	12.4	7.5	8.1	14.4
Prop In Lane	1.00		0.64	1.00		0.34	1.00		0.56	1.00		1.00
Lane Grp Cap(c), veh/h	240	493	463	162	415	411	213	497	473	172	910	400
V/C Ratio(X)	0.86	0.55	0.56	0.81	0.51	0.52	0.85	0.53	0.54	0.82	0.40	0.66
Avail Cap(c_a), veh/h	337	767	721	191	621	614	246	610	581	196	1119	492
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	40.7	29.7	29.8	42.9	32.0	32.1	41.5	29.3	29.5	42.6	29.6	32.0
Incr Delay (d2), s/veh	11.0	4.4	4.9	17.5	2.6	2.8	18.9	4.1	4.5	18.8	1.3	8.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.4	5.8	5.7	3.8	4.5	4.5	5.2	5.6	5.5	4.1	3.5	6.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	51.7	34.1	34.7	60.4	34.7	34.9	60.3	33.4	33.9	61.4	30.9	40.1
LnGrp LOS	D	С	С	E	С	С	E	С	С	E	С	D
Approach Vol, veh/h		740			555			704			764	
Approach Delay, s/veh		39.2			40.9			40.5			39.7	
Approach LOS		D			D			D			D	
• •											D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	14.5	33.5	17.2	31.0	18.7	29.3	15.0	33.3				
Change Period (Y+Rc), s	* 5.7	6.8	* 5.7	6.4	* 5.7	6.8	* 5.7	6.4				
Max Green Setting (Gmax), s	* 10	41.5	* 13	30.3	* 18	33.6	* 11	33.0				
Max Q Clear Time (g_c+I1), s	9.0	14.9	11.6	16.4	12.9	12.1	9.5	14.4				
Green Ext Time (p_c), s	0.0	9.5	0.0	6.8	0.1	5.0	0.0	7.5				
Intersection Summary												
HCM 6th Ctrl Delay			40.0									
HCM 6th LOS			D									
Notes												

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

HCM 95th-tile Q

2.5

2.6 12.5 5.9

6.2

6.6

3.8

Intersection													
Intersection Delay, s/vel	า42.9												
Intersection LOS	Ε												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		414			414			414			414		
Traffic Vol, veh/h	131	412	49	65	377	81	58	188	66	71	256	153	
Future Vol, veh/h	131	412	49	65	377	81	58	188	66	71	256	153	
Peak Hour Factor		0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	147	463	55	73	424	91	65	211	74	80	288	172	
Number of Lanes	0	2	0	0	2	0	0	2	0	0	2	0	
Approach	EB			WB			NB			SB			
Opposing Approach	WB			EB			SB			NB			
Opposing Lanes	2			2			2			2			
Conflicting Approach Le				NB			EB			WB			
Conflicting Lanes Left	2			2			2			2			
Conflicting Approach Rig				SB			WB			EB			
Conflicting Lanes Right	2			2			2			2			
HCM Control Delay	63.3			38.7			22.4			35.8			
HCM LOS	F			E			С			Е			
Lane	NF	l n1	NRI n2	FRI n1	FBI n2\	WBI n1\	WBI n2	SBLn1:	SBI n2				
Vol Left, %		38%	0%	39%	0%	26%	0%	36%	0%				
Vol Thru, %		52%	59%	61%	81%	74%	70%	64%	46%				
Vol Right, %		0%	41%	0%	19%	0%	30%	0%	54%				
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop				
Traffic Vol by Lane		152	160	337	255	254	270	199	281				
LT Vol		58	0	131	0	65	0	71	0				
Through Vol		94	94	206	206	189	189	128	128				
RT Vol		0	66	0	49	0	81	0	153				
Lane Flow Rate		171	180	379	287	285	303	224	316				
Geometry Grp		7	7	7	7	7	7	7	7				
Degree of Util (X)	0	486	0.492	1.023	0.747	0.765	0.784	0.605	0.812				
Departure Headway (Ho	l) 10	529	10.024	9.728	9.384	9.823	9.468	10.005	9.419				
Convergence, Y/N						V/00	V/00	Yes	Yes				
Convergence, 1711		Yes	Yes	Yes	Yes	Yes	Yes	162	1 62				
Cap		Yes 345	Yes 361	Yes 375	Yes 390	370	385	362	388				
	8			375			385		388				
Сар	0	345 229 496	361 7.724 0.499	375 7.416 1.011	390 7.072 0.736	370 7.523 0.77	385 7.168 0.787	362 7.705 0.619	388 7.119 0.814				
Cap Service Time	0	345 229	361 7.724	375 7.416	390 7.072	370 7.523	385 7.168	362 7.705	388 7.119				

Intersection													
Intersection Delay, s/ve	h55.4												
Intersection LOS	F												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4T>			414			4			4		
Traffic Vol, veh/h	74	416	65	69	363	76	84	200	51	89	162	74	
Future Vol, veh/h	74	416	65	69	363	76	84	200	51	89	162	74	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	84	473	74	78	413	86	95	227	58	101	184	84	
Number of Lanes	0	2	0	0	2	0	0	1	0	0	1	0	
Approach	EB			WB			NB			SB			
Opposing Approach	WB			EB			SB			NB			
Opposing Lanes	2			2			1			1			
Conflicting Approach Le				NB			EB			WB			
Conflicting Lanes Left	1			1			2			2			
Conflicting Approach Ri	ghtNB			SB			WB			EB			
Conflicting Lanes Right	1			1			2			2			
HCM Control Delay	47.6			39			80			69.2			
HCM LOS	Ε			Ε			F			F			
Lane	N	NBLn1	EBLn1	EBLn2V	VBLn1V	VBLn2	SBLn1						
Vol Left, %		25%	26%	0%	28%	0%	27%						
Vol Thru, %		60%	74%	76%	72%	70%	50%						
Vol Right, %		15%	0%	24%	0%	30%	23%						
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop						
Traffic Vol by Lane		335	282	273	251	258	325						
LT Vol		84	74	0	69	0	89						
Through Vol		200	208	208	182	182	162						
RT Vol		51	0	65	0	76	74						
Lane Flow Rate		381	320	310	285	293	369						
Geometry Grp		2	7	7	7	7	2						
Degree of Util (X)		1.008	0.867	0.813	0.775	0.768	0.963						
Departure Headway (Ho	d)	9.534	9.964	9.65	10.03	9.667	9.571						
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes						
Cap		384	365	378	364	377	380						
Service Time		7.497	7.664	7.35		7.367							
HCM Lane V/C Ratio		0.992	0.877	0.82	0.783	0.777	0.971						
HCM Control Delay		80	51.9	43.1	40	38	69.2						
HCM Lane LOS		F	F	Е	Ε	Е	F						
HCM 95th-tile Q		12.2	8.2	7.2	6.3	6.3	10.8						

Scenario 4: 4 HY 2042 PM

Intersection Level Of Service Report Intersection 4: Kern Street (SR-33) / 1st Street

Control Type:Two-way stopDelay (sec / veh):67.2Analysis Method:HCM 6th EditionLevel Of Service:FAnalysis Period:15 minutesVolume to Capacity (v/c):0.100

Intersection Setup

Name		1st S	Street			1st S	Street		Kern Street (SR-33)				
Approach		North	bound			South	bound		Eastbound				
Lane Configuration		4	-			뒥	H			4	F		
Turning Movement	Left2	Left	Thru	Right	Left2	Left	Thru	Right	Left2	Left	Thru	Right	
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	0	0	0	0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00 100.00 100.00 100.00			100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Speed [mph]		30.	.00	-		30	.00	-	30.00				
Grade [%]		0.00				0.	00		0.00				
Crosswalk		No				N	lo		No				

Volumes

Name		1st S	Street			1st S	Street			Kern Stree	et (SR-33))
Base Volume Input [veh/h]	45	10	20	10	10	10	20	27	16	233	140	37
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	0.00	2.00	0.00	2.00	2.00	2.00	2.00	0.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	45	10	20	10	10	10	20	27	16	233	140	37
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	12	3	5	3	3	3	5	7	4	62	37	10
Total Analysis Volume [veh/h]	48	11	21	11	11	11	21	29	17	248	149	39
Pedestrian Volume [ped/h]		()			()			()	

Scenario 4: 4 HY 2042 PM

Intersection Settings

Priority Scheme	Stop	Stop	Free
Flared Lane	No	No	
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance	No	No	
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.38	0.10	0.10	0.01	0.06	0.06	0.11	0.03	0.01	0.18	0.00	0.00
d_M, Delay for Movement [s/veh]	63.19	67.16	51.69	38.34	28.29	28.37	27.47	13.26	8.30	8.27	0.00	0.00
Movement LOS	F	F	F	E	D	D	D	В	Α	Α	Α	Α
95th-Percentile Queue Length [veh/ln]	3.13	3.13	3.13	3.13	0.99	0.99	0.99	0.99	0.72	0.72	0.36	0.00
95th-Percentile Queue Length [ft/ln]	78.17	78.17	78.17	78.17	24.67	24.67	24.67	24.67	18.00	18.00	9.00	0.00
d_A, Approach Delay [s/veh]		58.	.01			22	.01			4.8	34	
Approach LOS		F	=			()			P	4	
d_I, Intersection Delay [s/veh]						11	.79					
Intersection LOS						-	=					

Scenario 4: 4 HY 2042 PM

Intersection Setup

Name		Kern Stre	et (SR-33)			E Kern Street						
Approach		West	bound			Southwe	estbound					
Lane Configuration		Н	þ			74						
Turning Movement	Left	Thru	Right	Right2	Left	Thru	Right	Right2				
Lane Width [ft]	12.00	12.00 12.00 12.00 12.00 12.00 12.00 12.00										
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0				
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00				
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0				
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Speed [mph]		30	.00			30	.00					
Grade [%]		0.	00			0.	.00					
Crosswalk	No No											

Volumes

Name		Kern Stree	et (SR-33)			E Kern Street					
Base Volume Input [veh/h]	10	204	15	10	10	30	135	10			
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000			
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	0.00	0.00	0.00	0.00	0.00			
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000			
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0			
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0			
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0			
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0			
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0			
Other Volume [veh/h]	0	0	0	0	0	0	0	0			
Total Hourly Volume [veh/h]	10	204	15	10	10	30	135	10			
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400			
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000			
Total 15-Minute Volume [veh/h]	3	54	4	3	3	8	36	3			
Total Analysis Volume [veh/h]	11	217	16	11	11	32	144	11			
Pedestrian Volume [ped/h]		()		0						

Scenario 4: 4 HY 2042 PM

Intersection Settings

Priority Scheme	Free	Stop
Flared Lane		
Storage Area [veh]	0	0
Two-Stage Gap Acceptance		No
Number of Storage Spaces in Median	0	0

Movement, Approach, & Intersection Results

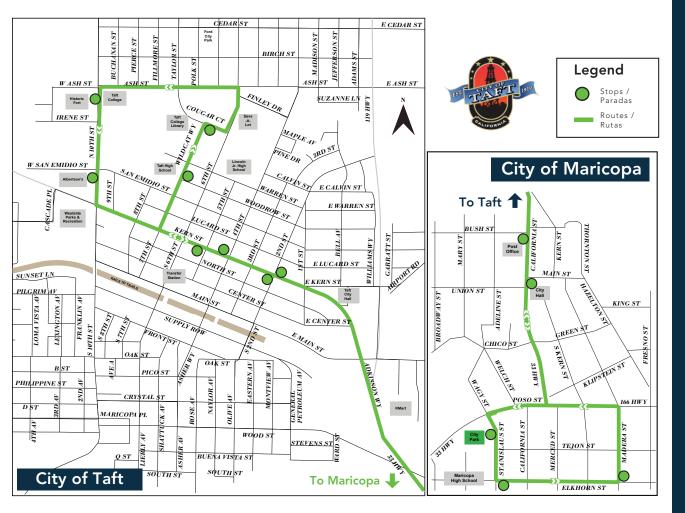
V/C, Movement V/C Ratio	0.01	0.00	0.00	0.00	0.06	0.18	0.23	0.02
d_M, Delay for Movement [s/veh]	7.62	0.00	0.00	0.00	32.24	31.34	13.71	11.63
Movement LOS	Α	Α	Α	Α	D	D	В	В
95th-Percentile Queue Length [veh/ln]	0.02	0.02	0.02	0.02	1.43	1.43	0.99	0.55
95th-Percentile Queue Length [ft/ln]	0.60	0.60	0.60	0.60	35.67	35.67	24.77	13.86
d_A, Approach Delay [s/veh]		0.3	33		17.48			
Approach LOS		F	4		С			
d_I, Intersection Delay [s/veh]	11.79							
Intersection LOS	F							

Intersection												
Int Delay, s/veh	1.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	14	0	39	0	2	2	18	200	0	3	131	10
Future Vol, veh/h	14	0	39	0	2	2	18	200	0	3	131	10
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	82	82	82	82	82	82	82	82	82	82	82	82
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	17	0	48	0	2	2	22	244	0	4	160	12
Major/Minor I	Minor2		l	Minor1		I	Major1		[Major2		
Conflicting Flow All	464	462	166	486	468	244	172	0	0	244	0	0
Stage 1	174	174	-	288	288	-	-	-	-	-	-	-
Stage 2	290	288	-	198	180	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	508	497	878	492	493	795	1405	-	-	1322	-	-
Stage 1	828	755	-	720	674	-	-	-	-	-	-	-
Stage 2	718	674	-	804	750	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	496	487	878	458	483	795	1405	-	-	1322	-	-
Mov Cap-2 Maneuver	496	487	-	458	483	-	-	-	-	-	-	-
Stage 1	813	753	-	707	662	-	-	-	-	-	-	-
Stage 2	700	662	-	758	748	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	10.4			11			0.6			0.2		
HCM LOS	В			В								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1405	_	-		601	1322	_	_			
HCM Lane V/C Ratio		0.016	_			0.008		_	_			
HCM Control Delay (s)		7.6	0	-	10.4	11	7.7	0	-			
HCM Lane LOS		Α.	A	-	В	В	A	A	_			
HCM 95th %tile Q(veh))	0	-	-	0.3	0	0	-	-			

Intersection						
Int Delay, s/veh	2.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		†		<u> </u>	† †
Traffic Vol, veh/h	58	106	570	100	113	563
Future Vol, veh/h	58	106	570	100	113	563
Conflicting Peds, #/hr	0	100	0	5	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Jiop -	None	-	None	-	None
Storage Length	0	-	_	-	50	-
Veh in Median Storage		_	0	_	-	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	63	115	620	109	123	612
IVIVIIIL FIOW	03	113	020	109	123	012
	Minor1		Major1	N	Major2	
Conflicting Flow All	1232	371	0	0	734	0
Stage 1	680	-	-	-	-	-
Stage 2	552	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	169	626	-	-	867	-
Stage 1	465	-	-	-	-	-
Stage 2	541	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	144	622	-	-	863	-
Mov Cap-2 Maneuver	277	-	-	-	-	-
Stage 1	463	-	-	-	-	-
Stage 2	464	_	_	_	_	_
Jiago Z	107					
Approach	WB		NB		SB	
HCM Control Delay, s	19.1		0		1.6	
HCM LOS	С					
Minor Lane/Major Mvm	ıt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_		432	863	_
HCM Lane V/C Ratio		_	_	0.413		_
HCM Control Delay (s)		_	_	19.1	9.9	_
HCM Lane LOS		-	_	C	Α.,	_
HCM 95th %tile Q(veh))	_	_	2	0.5	_
/ 5411 / 54110 (2(1011)				_	5.0	

APPENDIX F

TAFT AREA TRANSIT AND KERN TRANSIT ROUTES AND SCHEDULES



Taft-Maricopa Weekday Route / Ruta de Taft-Maricopa de Entre Semana Stanislaus St Elkhorn St & Post Office City Park Stanislaus St 7:12 AM 7:13 AM 7:14 AM 1:56 PM 1:57 PM 1:35 PM 5:35 PM 5:36 PM 5:37 PM 5:15 PM 5:16 PM

Т	att-Marico	opa Wee	kday Rou	ite (CONTI	NUED) /	Ruta de Ta	att-Mario	opa de E	ntre Sem	ana (CON	ITINUA
	Madera St & Elkhorn St	Maricopa City Hall	Kern St & 2nd St	Chamber of Commerce	Taft High School	Cougar Ct & Wildcat Way	Fort	Albertsons Plaza	Kern St & 5th St	Little Caesars	
1	7:15 AM	7:18 AM	7:29 AM	7:30 AM	7:32 AM	7:37 AM	7:39 AM	7:40 AM	7:41 AM	7:42 AM	
2	1:58 PM	2.01 PM	2:12 PM	2:13 PM	2:15 PM	2:20 PM	2:22 PM	2:23 PM	2:24 PM	2:25 PM	
3	5:38 PM	5:41 PM	5:52 PM	5:53 PM	5:55 PM	6:00 PM	6:02 PM	6:03 PM	6:04 PM	6:05 PM	

Route Hours of Operation

Taft - Maricopa Route:

Monday – Friday 7:12 a.m. to 6:05 p.m.

Taft-Mar
\$2.00
\$2.00
Free
Free

Taft 12-Trip Pass	Taft-Mar
General	\$20.00
ADA / Senior / Youth	\$20.00
N	

Note: Have exact fare. Drivers do not make change.

Horas de Operación

Ruta de Taft - Maricopa:

Lunes - Viernes 7:12 a.m. a 6:05 p.m.

Tarifas	Taft-Mar
General	\$2.00
Certificado de ADA / Mayor de 60 años / Jovenes	\$2.00
Niños menores de 5 años acompañados por pasajero que paga	Gratis
Asistente ADA (con pasajero registrado ADA que paga)	Gratis

Pase de 12-Viajes Taft	Ruta Taft-Mar
General	\$20.00
Certificado ADA / Mayor de 60 años /	\$20.00
Jovenes	
Nota : Tener tarifa exacta. Los conductores no hacen camb	oio.

TAFT-MARICOPA AREA TRANSIT

SYSTEM MAP & SCHEDULE HORARIO Y MAPA DE SISTEMA





661.763.3822 www.cityoftaft.org



EFFECTIVE JANUARY 1, 2019 / EFECTIVO 1 DE ENERO DE 2019

Welcome Aboard! / ¡Bienvenido a Bordo!

Taft Area Transit (TAT) travels throughout the cities of Taft and Maricopa. For additional service information or transportation planning assistance call (661) 763-3822.

Taft Area Transit (TAT) viaja a través de las ciudades de Taft y Maricopa. Para información adicional del servicio o asistencia de planificación de transporte llame al (661) 763-3822.

Passenger Guidelines

See the system map for service and area routes. For additional service information or transportation planning assistance call (661) 763-3822.

Do

- Pay your fare (exact change only).
- Be respectful of your driver and fellow passengers.
- Reserve front seats for the elderly or disabled passengers.
- Keep hands and objects inside vehicle.
- Remain seated until vehicle comes to a complete stop.
- Report any vandalism to driver.

Don't

- · No smoking.
- No offensive language or threatening behavior.
- No litterina.
- No music or excessive noise.
- No food or drinks onboard.
- No marking or defacing vehicle.
- No pets/animals except assistance animals with an official tag.

Note: TAT drivers may refuse service to persons acting inappropriately or causing harm to other customers or driver. Passengers unwilling to abide by these guidelines may be barred from using the City of Taft's transportation services.

Bikes on Bus

Getting around on Taft Area Transit just got easier. Take your bike along when you ride the bus. TAT buses feature easy-to-load bike racks. Motorized bicycles may not be loaded on the TAT bus bike racks.

Guías de Pasajeros

Vea el mapa del sistema para las rutas y áreas de servicio. Para información adicional del servicio o asistencia de planificación de transporte llame al (661) 763-3822.

Hacer:

- Paque su tarifa (cambio exacto solamente).
- Sea respetuoso de su conductor y los demás pasajeros.
- Reserva los asientos delanteros para los pasajeros de edad avanzada o discapacitados.
- Mantenga las manos y los objetos en el interior del vehículo.
- Permanezca sentado hasta que el vehículo se haya detenido por completo.
- Reporte cualquier vandalismo al conductor.

No Hacer:

- No fumar.
- Ningún lenguaje ofensivo o comportamiento amenazante.
- No tirar basura.
- · No música o ruido excesivo.
- · No comida o bebidas a bordo.
- · Ningún marcando o desfigurando del vehículo.
- No mascotas / animales, excepto los animales de asistencia con una etiqueta oficial.

Nota: Los conductores TAT pueden negar el servicio a las personas que actúan de manera inapropiada o causar daño a otros clientes o el conductor. Los pasajeros que no están dispuestos a cumplir con estas guías pueden ser excluidos de usar los servicios de transporte de la ciudad de Taft.

Bicicletas en el Autobús

Moverse en Taft Area Transit ahora es más fácil. Tome su bicicleta junto al montar el autobús. TAT autobuses disponen bastidores de bicicletas fáciles de carga. Bicicletas motorizadas no se pueden cargar en los bastidores de bicicleta de los autobuses TAT.

www.cityoftaft.org 661.763.3822

Dial-A-Ride Information

TAT offers a curb-to-curb, reservation based Dial-A-Ride service that is open to the public. Dial-A-Ride is limited to the City of Taft and does not travel to the City of Maricopa. Reservations must be made a minimum of two hours before you wish to ride by calling (661) 763-3822. Standing reservations may also be made. ADA-certified patrons and seniors (60 + years) may fill out an application to receive discount Dial-A-Ride pricing. Persons with disabilities interested in using Dial-A-Ride must submit an application signed by a physician. A photocopy of a state-issued identification card is also required. To request a Dial-A-Ride application call (661) 763-3822.

Please be ready when the vehicle arrives! Drivers will not wait longer than 2 minutes past the scheduled pick-up time. To schedule a ride call 2 hours prior to pickup time desired (661) 763-3822.

Información Dial-A-Ride

TAT ofrece al público servicio de curva a curva, basado en reserva por medio de Dial-A-Ride. Dial-A-Ride está limitado a la Ciudad de Taft y no viaja a la Ciudad de Maricopa. Reservaciones deben de ser hechas por lo menos dos horas antes de viajar llamando al (661) 763-3822. También se pueden hacer reservaciones permanentes. Patrones certificados ADA y mayores de edad (60 + años) pueden llenar una aplicación para recibir descuento. Personas con discapacidad interesados en utilizar Dial-A-Ride, deberán presentar una aplicación firmada por un médico. También se requiere una fotocopia de una tarjeta de identificación emitida por el estado. Para solicitar una aplicación llame al (661) 763-3822.

Por favor, ¡estar listo cuando llegue el vehículo! Los conductores no esperaran más de 2 minutos después de la hora de recogida programada. Para programar un viaje, por favor llame 2 horas antes de la hora deseada (661) 763-3822.

Holidays

TAT does not operate on New Year's Day, Labor Day, Independence Day, Lincoln's Birthday, Washington's Birthday, Memorial Day, Veteran's Day, Thanksgiving Day, Thanksgiving Day Friday, Christmas Eve, Christmas Day, and New Year's Eve.

Dial-A-Ride Hours of Operation

Monday – Friday 7:15 a.m. to 5:30 p.m. Saturday 10:15 a.m. to 2:30 p.m.

Dial-A-Ride Fares	
One-Way Fare (General Public)	\$2.50
ADA / Senior / Youth	\$1.75
Children under 5 per fare paying adult	Free
12-Trip Pass (General Public)	\$25.00
ADA / Senior / Youth	\$17.50
ADA Attendant (with paid registered ADA patron)	Free

Note: Have exact fare. Drivers do not make change.

Horas de Operación Dial-A-Ride

Lunes – Viernes 7:15 a.m. a 5:30 p.m. Sábado 10:15 a.m. a 2:30 p.m.

Tarifas de Dial-A-Ride	
Viaje Sencillo (Publico General)	\$2.50
ADA / Mayores de Edad / Jovenes	\$1.75
Niños menores de 5 años acompañados por pasajero que paga	Gratis
Pase de12 Viajes (Publico General)	\$25.00
ADA / Mayores de Edad / Jovenes	\$17.50

Nota: Tenga tarifa exacta. Los conductores no hacen el cambio.

Gratis

Días de Fiesta

Asistente ADA (con pasajero

registrado ADA que paga)

TAT no opera en el día de Año Nuevo, Día del Trabajo, Día de la Independencia, el Cumpleaños de Lincoln, el Cumpleaños de Washington, Día Conmemorativo, Día de los Veteranos, Día de Acción de Gracias, el Viernes Después de Día de Acción de Gracias, Víspera de Navidad, el día de Navidad y Fin de Año.

DAYS: DIRECTIONS:

Weekday Saturday Eastbound Westbound

STOP	175237	175084	175248	175238
Taft				
Taft College - 6th St. at Cougar Ct. (WB)	7:40am	10:00am	2:30pm	7:35pm
8th St. at North St. (Heritage Park Senior Complex)	7:44am	10:04am	2:34pm	7:39pm
Taft Transit Center	7:48am	10:08am	2:38pm	7:43pm
Harrison St. at Cedar St.	7:54am	10:14am	2:44pm	7:49pm
Valley Acres (East)	8:01am	10:21am	2:51pm	7:56pm
Dustin Acres East				
Bakersfield				
Stockdale Hwy. at Buena Vista Rd.	8:31am	10:51am	3:21pm	8:26pm
Cal State University Bakersfield	8:37am	10:57am	3:27pm	8:32pm
Stockdale Hwy. at New Stine Rd. (7/11)				
Brundage Ln. at A St. (East)				
Kern County Superior Court - Truxtun Ave.				
Bakersfield Amtrak	8:59am	11:19am	3:49pm	8:54pm
Greyhound/18th St.	9:04am	11:24am	3:54pm	8:59pm
GET Downtown Transit Center	9:07am	11:27am	3:57pm	9:02pm

DAYS: DIRECTIONS:
Weekday Saturday Eastbound Westbound

STOP	175071	175099	175066	175244
Bakersfield				
Bakersfield Amtrak	<u> </u>	8:22am	12:47pm	5:47pm
Greyhound/18th St.	, See	8:27am	12:52pm	5:52pm
GET Downtown Transit Center	6:10am	8:30am	12:55pm	5:55pm
Chester Ave. at Truxtun Ave City Hall				
Brundage Ln. at A St.				
Stockdale Hwy. at New Stine Rd.				
Cal State University Bakersfield	6:22am	8:52am	1:17pm	6:17pm
Stockdale Hwy. at Buena Vista Rd.	6:27am	8:57am	1:22pm	6:22pm
Taft				
Dustin Acres West				
Valley Acres (West)	6:57am	9:25am	1:50pm	6:50pm
Harrison St. at Cedar St. (WB)	7:07am	9:35am	2:00pm	7:00pm
Taft College - 6th St. at Cougar Ct. (WB)	7:09am	9:37am	2:02pm	7:02pm
8th St. at North St. (Heritage Park Senior Complex)	7:13am	9:41am	2:06pm	7:06pm
Taft Transit Center	7:16am	9:44am	2:09pm	7:09pm

DAYS: DIRECTIONS:
Weekday Saturday Eastbound Westbound

STOP	175252	175300	175129
Taft			
Taft College - 6th St. at Cougar Ct. (WB)	7:50am	12:12pm	5:30pm
8th St. at North St. (Heritage Park Senior Complex)	7:54am	12: 1 6pm	5:34pm
Taft Transit Center	7:58am	12:20pm	5:38pm
Harrison St. at Cedar St.	8:04am	12:26pm	5:44pm
Valley Acres (East)	8:11am	12:33pm	5:51pm
Dustin Acres East			
Bakersfield			
Stockdale Hwy. at Buena Vista Rd.	8:41am	1:03pm	6:21pm
Cal State University Bakersfield	8:47am	1:09pm	6:27pm
Stockdale Hwy. at New Stine Rd. (7/11)			
Brundage Ln. at A St. (East)			
Kern County Superior Court - Truxtun Ave.			
Bakersfield Amtrak	9:09am	1:31pm	6:49pm
Greyhound/18th St.	9:14am	1:36pm	6:54pm
GET Downtown Transit Center	9:17am	1:39pm	6:57pm

DAYS:

DIRECTIONS:

Weekday Saturday

Eastbound Westbound

STOP	175139	175130
Bakersfield		
Bakersfield Amtrak	10:27am	3:42pm
Greyhound/18th St.	10:32am	3:47pm
GET Downtown Transit Center	10:35am	3:50pm
Chester Ave. at Truxtun Ave City Hall		
Brundage Ln. at A St.		
Stockdale Hwy. at New Stine Rd.		
Cal State University Bakersfield	10:57am	4:12pm
Stockdale Hwy. at Buena Vista Rd.	11:02am	4:17pm
Taft		
Dustin Acres West		
Valley Acres (West)	11:30am	4:45pm
Harrison St. at Cedar St. (WB)	11:40am	4:55pm
Taft College - 6th St. at Cougar Ct. (WB)	11:42am	4:57pm
8th St. at North St. (Heritage Park Senior Complex)	11:46am	5:01pm
Taft Transit Center	11:49am	5:04pm

Route Map Q Enter address or location U Uit Field ± BFL Buttonwillow Lamont Weedpatch 223 (223) (33) (a) merebox

Route Map 5th S Q Enter address or location Taft State Park San Emidio St Lucard St North St Kern St 33 Center St North St Center St Franklin Ave S 10th St Main St FrontSt Cypress Ln © Mapbox © OpenStreetMap Improve this map (Description)

APPENDIX G

CA MUTCD PEAK HOUR WARRANT (WARRANT 3) WORKSHEETS

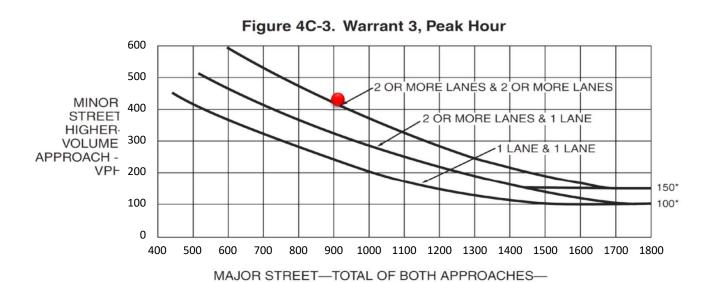
Minor Street: Kern Street Number of Lanes on minor street: 1 **Major Street** 4th Street Number of Lanes on major street: 2 **Total Number of Vehicles Entering:** Scenario: Horizon Year 2042 1731 **AM Peak** Number of Approaches: Intersection total delay (seconds): 118.8 Minor St approach total delay (veh-hr): 14.1

Part A

-		Criteria Met?
1	Minor St approach total stopped time delay	YES
2	Volume on the same minor street approach (one directional only)	YES
3	Total volume entering the intersection during the peak hour	YES

Part B

Volume on Major St (Σ of both Approaches)	Volumes on Minor St (higher approach)
911	428



*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

VEHICLES PER HOUR (VPH)

Does the Intersection Meet Peak Hour Signal Warrants?:		
PART A	YES	
PART B	YES	

Minor Street: Kern Street Number of Lanes on minor street: 1 **Major Street** 4th Street Number of Lanes on major street: 2 Scenario: Horizon Year 2042 **Total Number of Vehicles Entering:** 1723 PM Peak Number of Approaches: Intersection total delay (seconds): 55.4 Minor St approach total delay (veh-hr): 5.2

Part A

		Criteria Met?
1	Minor St approach total stopped time delay	YES
2	Volume on the same minor street approach (one directional only)	YES
3	Total volume entering the intersection during the peak hour	YES

Part B

Volume on Major St (Σ of both Approaches)	Volumes on Minor St (higher approach)
1063	335



*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

VEHICLES PER HOUR (VPH)

Does the Intersection Meet Peak Hour Signal Warrants?:		
PART A	YES	
PART B	YES	

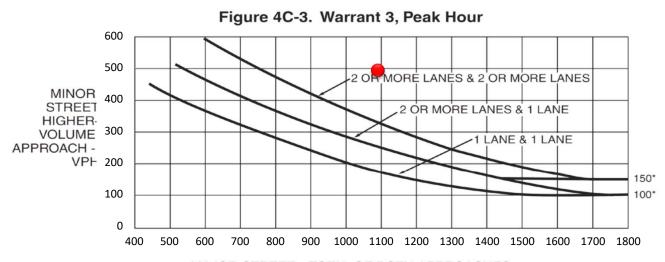
Minor Street: Kern Street Number of Lanes on minor street: 2 **Major Street** 6th Street Number of Lanes on major street: 2 Scenario: Horizon Year 2042 **Total Number of Vehicles Entering:** 1995 **AM Peak** Number of Approaches: Intersection total delay (seconds): 117.7 Minor St approach total delay (veh-hr): 16.1

Part A

		Criteria Met?
1	Minor St approach total stopped time delay	YES
2	Volume on the same minor street approach (one directional only)	YES
3	Total volume entering the intersection during the peak hour	YES

Part B

Volume on Major St (Σ of both Approaches)	Volumes on Minor St (higher approach)
1090	491



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Does the Intersection Meet Peak Hour Signal Warrants?:									
PART A	YES								
PART B	YES								

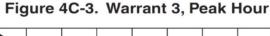
Minor Street: Kern Street Number of Lanes on minor street: 2 **Major Street** 6th Street Number of Lanes on major street: 2 Scenario: Horizon Year 2042 **Total Number of Vehicles Entering:** 1907 PM Peak Number of Approaches: Intersection total delay (seconds): 42.9 Minor St approach total delay (veh-hr): 5.7

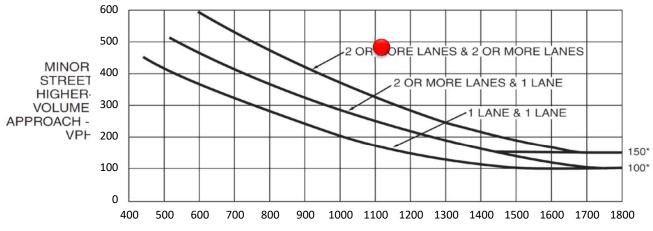
Part A

		Criteria Met?
1	Minor St approach total stopped time delay	YES
2	Volume on the same minor street approach (one directional only)	YES
3	Total volume entering the intersection during the peak hour	YES

Part B

Volume on	Major St (Σ of both Approaches)	Volumes on Minor St (higher approach)
	1115	480





MAJOR STREET-TOTAL OF BOTH APPROACHES-VEHICLES PER HOUR (VPH)

*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Does the Intersection Meet Peak Hour Signal Warrants?:									
PART A	YES								
PART B	YES								

APPENDIX H

HORIZON YEAR 2042 WITH PROJECT INTERSECTION OPERATIONS WITH RECOMMENDED IMPROVEMENTS ANALYSIS WORKSHEETS

	۶	→	•	•	←	•	4	†	/	/	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	ተ ኈ		ሻ	ተ ኈ		7	↑	7	ሻ	↑	7
Traffic Volume (veh/h)	133	382	40	47	412	76	57	296	61	84	260	147
Future Volume (veh/h)	133	382	40	47	412	76	57	296	61	84	260	147
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		0.99	1.00		0.97	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	182	523	55	64	564	104	78	405	84	115	356	201
Peak Hour Factor	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	212	924	97	129	714	131	139	530	434	154	545	453
Arrive On Green	0.12	0.28	0.28	0.07	0.24	0.24	0.08	0.28	0.28	0.09	0.29	0.29
Sat Flow, veh/h	1781	3244	340	1781	2994	550	1781	1870	1534	1781	1870	1554
Grp Volume(v), veh/h	182	286	292	64	334	334	78	405	84	115	356	201
Grp Sat Flow(s), veh/h/ln	1781	1777	1807	1781	1777	1767	1781	1870	1534	1781	1870	1554
Q Serve(g_s), s	8.7	11.9	11.9	3.0	15.3	15.4	3.7	17.1	3.6	5.5	14.4	9.1
Cycle Q Clear(g_c), s	8.7	11.9	11.9	3.0	15.3	15.4	3.7	17.1	3.6	5.5	14.4	9.1
Prop In Lane	1.00		0.19	1.00		0.31	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	212	506	515	129	424	422	139	530	434	154	545	453
V/C Ratio(X)	0.86	0.56	0.57	0.50	0.79	0.79	0.56	0.76	0.19	0.75	0.65	0.44
Avail Cap(c_a), veh/h	212	577	587	189	554	551	165	672	551	165	672	558
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.4	26.4	26.4	38.6	30.9	30.9	38.5	28.4	23.5	38.6	26.8	25.0
Incr Delay (d2), s/veh	27.9	1.0	1.0	2.9	5.6	5.9	3.5	4.0	0.2	15.8	1.6	0.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.3	4.9	5.1	1.4	6.9	6.9	1.7	7.9	1.3	3.0	6.4	3.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	65.4	27.4	27.4	41.5	36.5	36.8	41.9	32.4	23.7	54.5	28.4	25.6
LnGrp LOS	E	С	С	D	D	D	D	С	С	D	С	С
Approach Vol, veh/h		760			732			567			672	
Approach Delay, s/veh		36.5			37.1			32.4			32.1	
Approach LOS		D			D D			C			C	
•											<u> </u>	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.2	30.9	12.0	31.5	11.5	31.6	16.0	27.5				
Change Period (Y+Rc), s	* 4.7	6.4	* 5.7	6.8	* 4.7	6.4	* 5.7	6.8				
Max Green Setting (Gmax), s	* 8	31.1	* 9.2	28.1	* 8	31.1	* 10	27.0				
Max Q Clear Time (g_c+l1), s	7.5	19.1	5.0	13.9	5.7	16.4	10.7	17.4				
Green Ext Time (p_c), s	0.0	2.1	0.0	2.9	0.0	2.4	0.0	2.8				
Intersection Summary												
HCM 6th Ctrl Delay			34.7									
HCM 6th LOS			С									
Notes												

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

	۶	→	•	•	←	•	4	†	/	/	Ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	∱ ∱		ሻ	∱ ∱		7	₽		7	ĵ∍	
Traffic Volume (veh/h)	92	318	54	47	345	55	91	248	53	81	263	84
Future Volume (veh/h)	92	318	54	47	345	55	91	248	53	81	263	84
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		0.97	1.00		0.99	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	116	403	68	59	437	70	115	314	67	103	333	106
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	169	639	107	132	580	92	168	457	98	164	412	131
Arrive On Green	0.09	0.21	0.21	0.07	0.19	0.19	0.09	0.31	0.31	0.09	0.30	0.30
Sat Flow, veh/h	1781	3042	509	1781	3056	486	1781	1491	318	1781	1356	432
Grp Volume(v), veh/h	116	234	237	59	253	254	115	0	381	103	0	439
Grp Sat Flow(s), veh/h/ln	1781	1777	1774	1781	1777	1765	1781	0	1810	1781	0	1788
Q Serve(g_s), s	4.9	9.3	9.4	2.5	10.4	10.6	4.8	0.0	14.3	4.3	0.0	17.6
Cycle Q Clear(g_c), s	4.9	9.3	9.4	2.5	10.4	10.6	4.8	0.0	14.3	4.3	0.0	17.6
Prop In Lane	1.00	7.0	0.29	1.00		0.28	1.00	0.0	0.18	1.00	0.0	0.24
Lane Grp Cap(c), veh/h	169	373	373	132	337	335	168	0	555	164	0	543
V/C Ratio(X)	0.69	0.63	0.64	0.45	0.75	0.76	0.68	0.00	0.69	0.63	0.00	0.81
Avail Cap(c_a), veh/h	237	495	494	204	463	460	237	0	831	214	0	798
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	34.0	27.9	27.9	34.4	29.7	29.7	34.0	0.0	23.6	33.9	0.0	24.9
Incr Delay (d2), s/veh	4.9	1.7	1.8	2.3	4.4	4.9	4.8	0.0	1.5	3.9	0.0	3.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.3	3.9	4.0	1.1	4.6	4.7	2.3	0.0	6.2	2.0	0.0	7.8
Unsig. Movement Delay, s/veh		0.7	1.0	• • •	1.0	1.7	2.0	0.0	0.2	2.0	0.0	7.0
LnGrp Delay(d),s/veh	38.9	29.6	29.7	36.7	34.1	34.6	38.8	0.0	25.1	37.9	0.0	28.8
LnGrp LOS	D	C	C	D	С	C	D	A	C	D	A	C
Approach Vol, veh/h		587			566			496			542	
Approach Delay, s/veh		31.5			34.6			28.3			30.5	
Approach LOS		31.3 C			34.0 C			20.3 C			30.5 C	
Approach EO3		C			C			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.8	30.2	11.5	23.1	13.0	30.0	13.0	21.5				
Change Period (Y+Rc), s	* 5.7	6.4	* 5.7	6.8	* 5.7	6.4	* 5.7	6.8				
Max Green Setting (Gmax), s	* 9.3	35.6	* 8.9	21.6	* 10	34.6	* 10	20.2				
Max Q Clear Time (q_c+l1), s	6.3	16.3	4.5	11.4	6.8	19.6	6.9	12.6				
Green Ext Time (p_c), s	0.1	2.4	0.0	1.9	0.1	2.6	0.1	1.8				
Intersection Summary												
HCM 6th Ctrl Delay			31.3									
HCM 6th LOS			С									
Notes												

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

DOWNTOWN TAFT SPECIFIC PLAN

Scenario 5: 5 HY 2042 AM - MITI

Intersection Level Of Service Report Intersection 4: Kern Street (SR-33) / 1st Street

Control Type:All-way stopDelay (sec / veh):12.3Analysis Method:HCM 6th EditionLevel Of Service:BAnalysis Period:15 minutesVolume to Capacity (v/c):0.399

Intersection Setup

Name		1st S	Street			1st S	treet			Kern Stre	et (SR-33))
Approach		North	bound			South	bound			Eastb	ound	
Lane Configuration		4	-		#				414			
Turning Movement	Left2	Left	Thru	Right	Left2	Left	Thru	Right	Left2	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]		30	.00	-		30	.00	-	30.00			
Grade [%]	0.00			0.00				0.00				
Crosswalk		N	lo			N	lo			N	lo	

Volumes

Name		1st S	Street			1st S	street			Kern Stre	et (SR-33)	
Base Volume Input [veh/h]	40	10	20	10	10	10	28	37	24	136	143	60
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	0.00	2.00	0.00	2.00	2.00	2.00	2.00	0.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	40	10	20	10	10	10	28	37	24	136	143	60
Peak Hour Factor	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	11	3	6	3	3	3	8	11	7	39	41	17
Total Analysis Volume [veh/h]	46	11	23	11	11	11	32	43	28	156	164	69
Pedestrian Volume [ped/h]		()			()			()	



DOWNTOWN TAFT SPECIFIC PLAN

 Version 2021 (SP 0-6)
 Scenario 5: 5 HY 2042 AM - MITI

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Lanes										
Capacity per Entry Lane [veh/h]	496	550	523	579						
Degree of Utilization, x	0.18	0.18	0.40	0.36						
Movement, Approach, & Intersection Results										
95th-Percentile Queue Length [veh]	0.67	0.64	1.90	1.63						
95th-Percentile Queue Length [ft]	16.63	15.88	47.43	40.84						
Approach Delay [s/veh]	11.88	10.94	13	.23						
Approach LOS	В	В		В						
Intersection Delay [s/veh]		12.29	•							
Intersection LOS		В								

DOWNTOWN TAFT SPECIFIC PLAN Scenario 5: 5 HY 2042 AM - MITI

Intersection Setup

Name		Kern Stre	et (SR-33)		E Kern Street					
Approach		West	bound		Southwestbound					
Lane Configuration		+	H		Ϋ́F					
Turning Movement	Left	Thru	Right	Right2	Left	Thru	Right	Right2		
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00		
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0		
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00		
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0		
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Speed [mph]		30	.00			30	.00			
Grade [%]		0.	00		0.00					
Crosswalk		Ν	lo			N	lo			

Volumes

Name		Kern Stree	et (SR-33)			E Kern	Street	
Base Volume Input [veh/h]	12	114	10	10	10	43	172	10
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	12	114	10	10	10	43	172	10
Peak Hour Factor	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	3	33	3	3	3	12	49	3
Total Analysis Volume [veh/h]	14	131	11	11	11	49	198	11
Pedestrian Volume [ped/h]		()			()	



DOWNTOWN TAFT SPECIFIC PLAN Scenario 5: 5 HY 2042 AM - MITI

Intersection Settings

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Capacity per Entry Lane [veh/h]	524	554	586
Degree of Utilization, x	0.32	0.24	0.23

Movement, Approach, & Intersection Results

95th-Percentile Queue Length [veh]	1.36	0.94	0.88				
95th-Percentile Queue Length [ft]	34.11	23.60	21.99				
Approach Delay [s/veh]	13.07	10.96					
Approach LOS	В	В					
Intersection Delay [s/veh]	12.	29					
Intersection LOS	E	В					

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Į.	∱ β		¥	∱ β		Ĭ		7	ň		7
Traffic Volume (veh/h)	131	412	49	65	377	81	58	188	66	71	256	153
Future Volume (veh/h)	131	412	49	65	377	81	58	188	66	71	256	153
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		0.99	1.00		0.98	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	147	463	55	73	424	91	65	211	74	80	288	172
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	198	763	90	158	628	134	149	430	356	164	446	374
Arrive On Green	0.11	0.24	0.24	0.09	0.22	0.22	0.08	0.23	0.23	0.09	0.24	0.24
Sat Flow, veh/h	1781	3198	378	1781	2911	620	1781	1870	1546	1781	1870	1567
Grp Volume(v), veh/h	147	256	262	73	257	258	65	211	74	80	288	172
Grp Sat Flow(s), veh/h/ln	1781	1777	1799	1781	1777	1754	1781	1870	1546	1781	1870	1567
Q Serve(g_s), s	5.4	8.6	8.7	2.6	8.9	9.1	2.3	6.6	2.6	2.9	9.3	6.3
Cycle Q Clear(g_c), s	5.4	8.6	8.7	2.6	8.9	9.1	2.3	6.6	2.6	2.9	9.3	6.3
Prop In Lane	1.00	0.0	0.21	1.00	0.7	0.35	1.00	0.0	1.00	1.00	7.3	1.00
Lane Grp Cap(c), veh/h	198	424	429	158	383	378	149	430	356	1.00	446	374
V/C Ratio(X)	0.74	0.60	0.61	0.46	0.67	0.68	0.44	0.49	0.21	0.49	0.65	0.46
	273	729	738	257	713	704	212	864	715	212	864	724
Avail Cap(c_a), veh/h HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	29.0	22.8	22.8	29.1	24.2	24.3	29.3	22.5	20.9	29.0	23.1	21.9
Incr Delay (d2), s/veh	6.8	1.4	1.4	2.1	2.0	2.2	2.0	0.9	0.3	2.2	1.6	0.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.5	3.5	3.6	1.2	3.7	3.7	1.0	2.8	0.9	1.3	4.0	2.2
Unsig. Movement Delay, s/veh		24.2	24.2	21.0	2/2	0/ 4	04.0	22.2	21.2	21.2	24.7	22.0
LnGrp Delay(d),s/veh	35.8	24.2	24.2	31.3	26.2	26.4	31.3	23.3	21.2	31.3	24.6	22.8
LnGrp LOS	D	С	С	С	С	С	С	С	С	С	С	<u>C</u>
Approach Vol, veh/h		665			588			350			540	
Approach Delay, s/veh		26.8			26.9			24.4			25.0	
Approach LOS		С			С			С			С	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.9	21.9	11.7	22.8	10.3	22.5	13.2	21.3				
Change Period (Y+Rc), s	* 4.7	6.4	* 5.7	6.8	* 4.7	6.4	* 5.7	6.8				
Max Green Setting (Gmax), s	* 8	31.1	* 9.7	27.6	* 8	31.1	* 10	27.0				
Max Q Clear Time (g_c+l1), s	4.9	8.6	4.6	10.7	4.3	11.3	7.4	11.1				
Green Ext Time (p_c), s	0.0	1.4	0.1	2.8	0.0	2.1	0.1	2.7				
* .	0.0	1.7	0.1	2.0	0.0	۷, ۱	0.1	2.1				
Intersection Summary			26.0									
HCM 6th Ctrl Delay			26.0									
HCM 6th LOS			С									
Notes												

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	∱ β		ሻ	∱ β		ሻ	₽		ሻ	₽	
Traffic Volume (veh/h)	74	416	65	69	363	76	84	200	51	89	162	74
Future Volume (veh/h)	74	416	65	69	363	76	84	200	51	89	162	74
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	84	473	74	78	412	86	95	227	58	101	184	84
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	171	663	103	166	621	129	180	302	77	184	258	118
Arrive On Green	0.10	0.22	0.22	0.09	0.21	0.21	0.10	0.21	0.21	0.10	0.21	0.21
Sat Flow, veh/h	1781	3079	479	1781	2929	606	1781	1437	367	1781	1215	555
Grp Volume(v), veh/h	84	272	275	78	249	249	95	0	285	101	0	268
Grp Sat Flow(s), veh/h/ln	1781	1777	1781	1781	1777	1758	1781	0	1804	1781	0	1770
Q Serve(g_s), s	2.9	9.2	9.3	2.7	8.3	8.5	3.3	0.0	9.6	3.5	0.0	9.1
Cycle Q Clear(g_c), s	2.9	9.2	9.3	2.7	8.3	8.5	3.3	0.0	9.6	3.5	0.0	9.1
Prop In Lane	1.00		0.27	1.00		0.34	1.00		0.20	1.00		0.31
Lane Grp Cap(c), veh/h	171	382	383	166	377	373	180	0	379	184	0	375
V/C Ratio(X)	0.49	0.71	0.72	0.47	0.66	0.67	0.53	0.00	0.75	0.55	0.00	0.71
Avail Cap(c_a), veh/h	269	618	620	266	615	609	318	0	872	321	0	858
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	27.9	23.6	23.7	27.9	23.4	23.5	27.7	0.0	24.1	27.7	0.0	23.8
Incr Delay (d2), s/veh	2.2	2.5	2.5	2.1	2.0	2.1	2.4	0.0	3.0	2.5	0.0	2.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.3	3.8	3.9	1.2	3.4	3.4	1.5	0.0	4.2	1.6	0.0	3.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	30.0	26.1	26.2	30.0	25.4	25.6	30.1	0.0	27.1	30.2	0.0	26.3
LnGrp LOS	С	С	С	С	С	С	С	Α	С	С	А	С
Approach Vol, veh/h		631			576			380			369	
Approach Delay, s/veh		26.7			26.1			27.9			27.4	
Approach LOS		C C			C C			C C			C C	
											C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.4	20.0	11.7	20.8	12.3	20.2	11.9	20.6				
Change Period (Y+Rc), s	* 5.7	6.4	* 5.7	6.8	* 5.7	6.4	* 5.7	6.8				
Max Green Setting (Gmax), s	* 12	31.4	* 9.7	22.6	* 12	31.5	* 9.8	22.5				
Max Q Clear Time (g_c+l1), s	5.5	11.6	4.7	11.3	5.3	11.1	4.9	10.5				
Green Ext Time (p_c), s	0.1	1.7	0.1	2.4	0.1	1.6	0.1	2.3				
Intersection Summary												
HCM 6th Ctrl Delay			26.9									
HCM 6th LOS			С									
Notes												

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

DOWNTOWN TAFT SPECIFIC PLAN

Scenario 6: 6 HY 2042 PM - MITI

Intersection Level Of Service Report Intersection 4: Kern Street (SR-33) / 1st Street

Control Type:All-way stopDelay (sec / veh):13.9Analysis Method:HCM 6th EditionLevel Of Service:BAnalysis Period:15 minutesVolume to Capacity (v/c):0.515

Intersection Setup

Name		1st S	Street			1st S	street		Kern Street (SR-33)			
Approach	Northbound				Southbound				Eastbound			
Lane Configuration	+			+					41			
Turning Movement	Left2	Left	Thru	Right	Left2	Left	Thru	Right	Left2	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]		30	.00	-		30	.00	-		30	.00	
Grade [%]		0.00			0.00			0.00				
Crosswalk		N	lo			N	lo			N	lo	

Volumes

Name		1st S	treet			1st S	Street			Kern Stree	et (SR-33))
Base Volume Input [veh/h]	45	10	20	10	10	10	20	27	16	233	140	37
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	0.00	2.00	0.00	2.00	2.00	2.00	2.00	0.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	45	10	20	10	10	10	20	27	16	233	140	37
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	12	3	5	3	3	3	5	7	4	62	37	10
Total Analysis Volume [veh/h]	48	11	21	11	11	11	21	29	17	248	149	39
Pedestrian Volume [ped/h]		()			()			()	



DOWNTOWN TAFT SPECIFIC PLAN

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Intersection Settings

Lanes				
Capacity per Entry Lane [veh/h]	482	532	515	566
Degree of Utilization, x	0.19	0.14	0.51	0.33
Movement, Approach, & Intersection Result	s			
95th-Percentile Queue Length [veh]	0.69	0.47	2.91	1.45
95th-Percentile Queue Length [ft]	17.23	11.64	72.78	36.23
Approach Delay [s/veh]	12.21	10.82	14	1.94
Approach LOS	В	В		В
Intersection Delay [s/veh]		13.92	•	
Intersection LOS		В		

DOWNTOWN TAFT SPECIFIC PLAN Scenario 6: 6 HY 2042 PM - MITI

Intersection Setup

Name		Kern Stre	et (SR-33)		E Kern Street				
Approach		West	bound		Southwestbound				
Lane Configuration		H	þ		YF				
Turning Movement	Left	Left Thru Right Right2				Thru	Right	Right2	
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Speed [mph]		30	.00			30	.00		
Grade [%]		0.	00		0.00				
Crosswalk		N	lo		No				

Volumes

Name		Kern Street (SR-33)				E Kern Street				
Base Volume Input [veh/h]	10	204	15	10	10	30	135	10		
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	0.00	0.00	0.00	0.00	0.00		
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0		
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0		
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0		
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0		
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0		
Other Volume [veh/h]	0	0	0	0	0	0	0	0		
Total Hourly Volume [veh/h]	10	204	15	10	10	30	135	10		
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400		
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		
Total 15-Minute Volume [veh/h]	3	54	4	3	3	8	36	3		
Total Analysis Volume [veh/h]	11	217	16	11	11	32	144	11		
Pedestrian Volume [ped/h]		0				0				



DOWNTOWN TAFT SPECIFIC PLAN Scenario 6: 6 HY 2042 PM - MITI

Version 2021 (SP 0-6)

Intersection Settings

<u> </u>						
Lanes						
Capacity per Entry Lane [veh/h]	530	524	553			
Degree of Utilization, x	0.48	0.19	0.18			
Movement, Approach, & Intersection Results		·				
95th-Percentile Queue Length [veh]	2.59	0.69	0.65			
95th-Percentile Queue Length [ft]	64.79	17.25	16.17			
Approach Delay [s/veh]	15.95	10).90			
Approach LOS	С		В			
Intersection Delay [s/veh]	13.92					
Intersection LOS	В					