

Red Bluff Apartments Traffic Impact Analysis

Red Bluff, California

April 6, 2022

Prepared by:



TJW ENGINEERING, INC.

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Irvine, CA 92618

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TJW ENGINEERING, INC.
TRAFFIC ENGINEERING &
TRANSPORTATION PLANNING
CONSULTANTS

April 6, 2022

Mr. Mitch Slagerman
PALM COMMUNITIES
100 Pacifica, Suite 203
Irvine, CA 92618

Subject: Red Bluff Apartments Traffic Impact Analysis, City of Red Bluff

Dear Mr. Slagerman:

TJW ENGINEERING, INC. (TJW) is pleased to present you with this traffic impact analysis for the proposed Red Bluff Apartments project located at 321 South Jackson Street in the City of Red Bluff.

This traffic study has been prepared to meet the traffic study requirements for the City of Red Bluff and assesses the forecast traffic operations associated with the proposed project and its impact on the local street network. This report is being submitted to you for review and forwarding to the City of Red Bluff.

Please contact us at (949) 878-3509 if you have any questions regarding this analysis.

Sincerely,

A handwritten signature in black ink, appearing to read 'TH Wheat'.

Thomas Wheat, PE, TE
President

A handwritten signature in black ink, appearing to read 'David Chew'.

David Chew, PTP
Transportation Planner

Registered Civil Engineer #69467
Registered Traffic Engineer #2565

A handwritten signature in black ink, appearing to read 'Brandon Alvarado'.

Brandon Alvarado, EIT
Transportation Planner



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1.0 EXECUTIVE SUMMARY

This traffic impact analysis (TIA) analyzes the projected traffic operations associated with the development of the Red Bluff Apartments project located at 321 South Jackson Street in the City of Red Bluff. The purpose of this TIA is to evaluate potential circulation system deficiencies that may result from development of the proposed project, and to recommend improvements to achieve acceptable operations, if applicable. This analysis has been prepared in coordination with the City of Red Bluff via a scoping agreement (See **Appendix B**).

The proposed project consists of constructing 61 affordable apartments. Site access is planned via a full access driveway on Jackson Street (aligned with Olive Street). The proposed project is anticipated to be fully built and generating trips in 2025. To account for ambient growth on roadways, *Existing* traffic volumes have been increased by a growth rate of 2% per year over a three-year period for Project Opening Year conditions.

The proposed project is projected to generate 293 daily trips, 22 AM peak hour trips, and 28 PM peak hour trips.

The following 5 intersections in the vicinity of the project site have been included in the intersection level of service (LOS) analysis:

1. Jackson Street / Aloha Street
2. Jackson Street / Lay Avenue
3. Jackson Street / Reeds Avenue
4. Jackson Street / Luther Road
5. Jackson Street / Olive Street - Project Driveway

The following roadway segment in the vicinity of the project site has been included in the roadway segment LOS analysis:

1. Jackson Street from Lay Avenue to Reeds Avenue

The study intersections and roadway segments have been analyzed for the following study scenarios:

- Existing Year 2022 Traffic Conditions ("*Existing*")
- Opening Year 2025 Without Project Conditions ("*Opening Year Without Project*")
 - Existing + Ambient
- Opening Year 2025 With Project Conditions ("*Opening Year With Project*")
 - Existing + Ambient + Project



1.1 SUMMARY OF LEVEL OF SERVICE ANALYSIS RESULTS

The table below summarizes the results of the intersection LOS analysis based on the City of San Bernardino thresholds of significance for analyzing transportation deficiencies.

Summary of Transportation Deficiencies at Study Intersections

Intersection			Opening Year With Project
1	Jackson Street	Aloha Street	No Deficiencies
2	Jackson Street	Lay Avenue	No Deficiencies
3	Jackson Street	Reeds Avenue	No Deficiencies
4	Jackson Street	Luther Road	No Deficiencies
5	Jackson Street	Olive Street – Project Driveway	No Deficiencies

Existing Conditions

The study intersections are projected to operate at an acceptable LOS during the AM and PM peak hours for *Existing* conditions.

Opening Year Without Project Conditions

The study intersections are projected to operate at an acceptable LOS during the AM and PM peak hours for *Opening Year Without Project* conditions.

Opening Year With Project Conditions

The study intersections are projected to operate at an acceptable LOS during the AM and PM peak hours for *Opening Year With Project* conditions.

1.2 ON-SITE ROADWAY AND SITE ACCESS IMPROVEMENTS

Wherever necessary, roadways adjacent to the proposed project site and site access points will be constructed in compliance with recommended roadway classifications and respective cross-sections in the City of Red Bluff General Plan or as directed by the City Engineer.

Sight distance at each project access point should be reviewed with respect to standard City sight distance standards at the time of final grading, landscaping and street improvement plans.

Signing/stripping should be implemented in conjunction with detailed construction plans for the project site.



2.0 INTRODUCTION

This traffic impact analysis (TIA) analyzes the projected traffic operations associated with the development of the Red Bluff Apartments project located at 321 South Jackson Street in the City of Red Bluff. The purpose of this TIA is to evaluate potential circulation system deficiencies that may result from development of the proposed project, and to recommend improvements to achieve acceptable operations, if applicable. This analysis has been prepared in coordination with the City of Red Bluff via a scoping agreement (See **Appendix B**).

2.1 PROJECT DESCRIPTION

The proposed project consists of constructing 61 affordable apartments. Site access is planned via a full access driveway on Jackson Street (aligned with Olive Street). The proposed project is anticipated to be fully built and generating trips in 2025. To account for ambient growth on roadways, *Existing* traffic volumes have been increased by a growth rate of 2% per year over a three-year period for Project Opening Year conditions.

Exhibit 1 shows the project site location and study area. **Exhibit 2** shows the proposed project site plan.

2.2 STUDY AREA

The following 5 intersections in the vicinity of the project site have been included in the intersection level of service (LOS) analysis:

1. Jackson Street / Aloha Street
2. Jackson Street / Lay Avenue
3. Jackson Street / Reeds Avenue
4. Jackson Street / Luther Road
5. Jackson Street / Olive Street - Project Driveway

The following roadway segment in the vicinity of the project site has been included in the roadway segment LOS analysis:

1. Jackson Street from Lay Avenue to Reeds Avenue

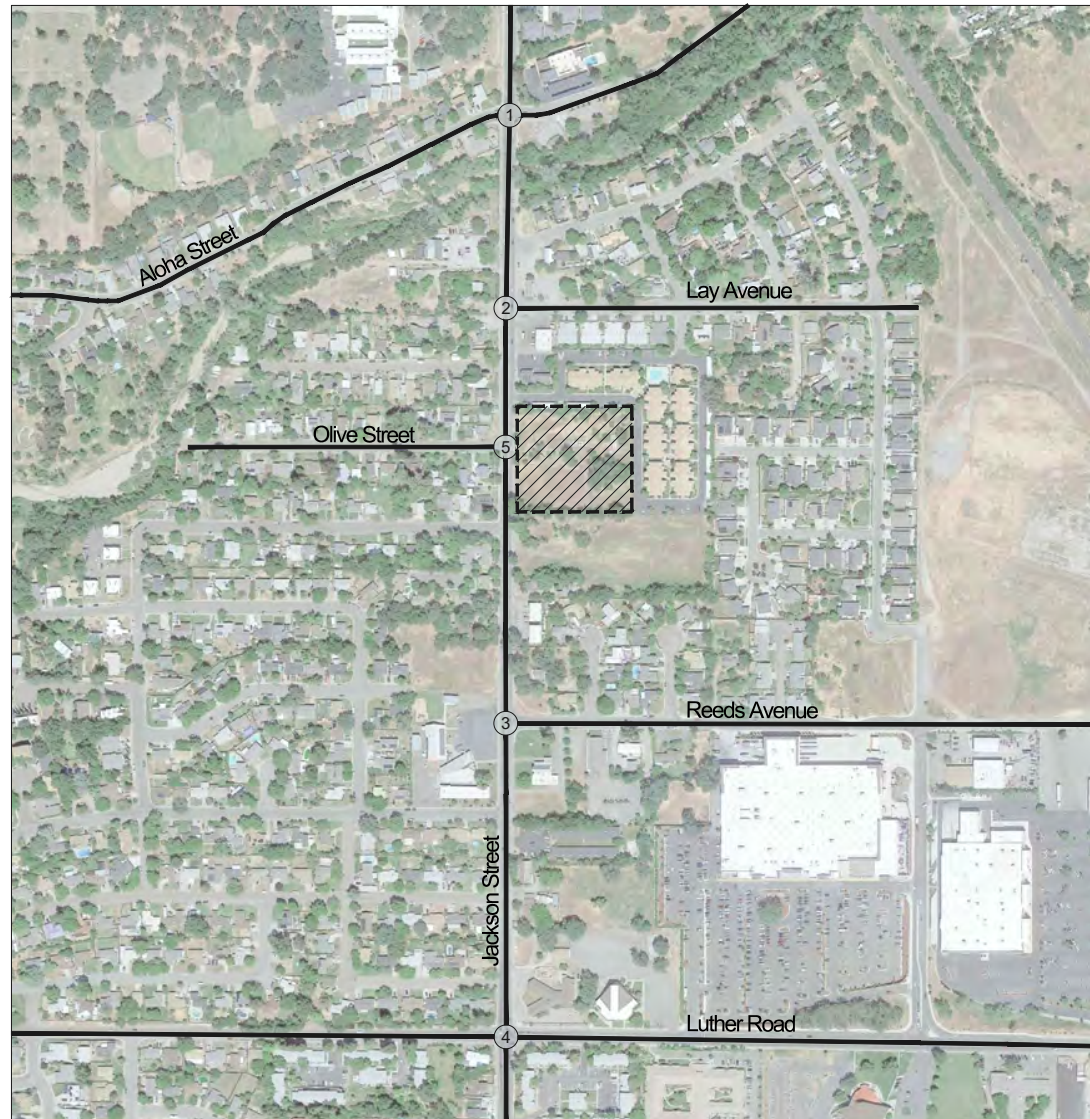
Exhibit 1 shows the location of the study intersections and roadway segments which are analyzed for the following study scenarios:



- Existing Year 2022 Traffic Conditions (“Existing”)
- Opening Year 2025 Without Project Conditions (“Opening Year Without Project”)
 - Existing + Ambient
- Opening Year 2025 With Project Conditions (“Opening Year With Project”)
 - Existing + Ambient + Project

Traffic operations are evaluated for the following time periods:

- Weekday AM Peak Hour occurring within 7:00 AM to 9:00 AM
- Weekday PM Peak Hour occurring within 4:00 PM to 6:00 PM



Legend:

- Project Site
- Ⓜ Study Intersection Location

Exhibit 1: Project Location

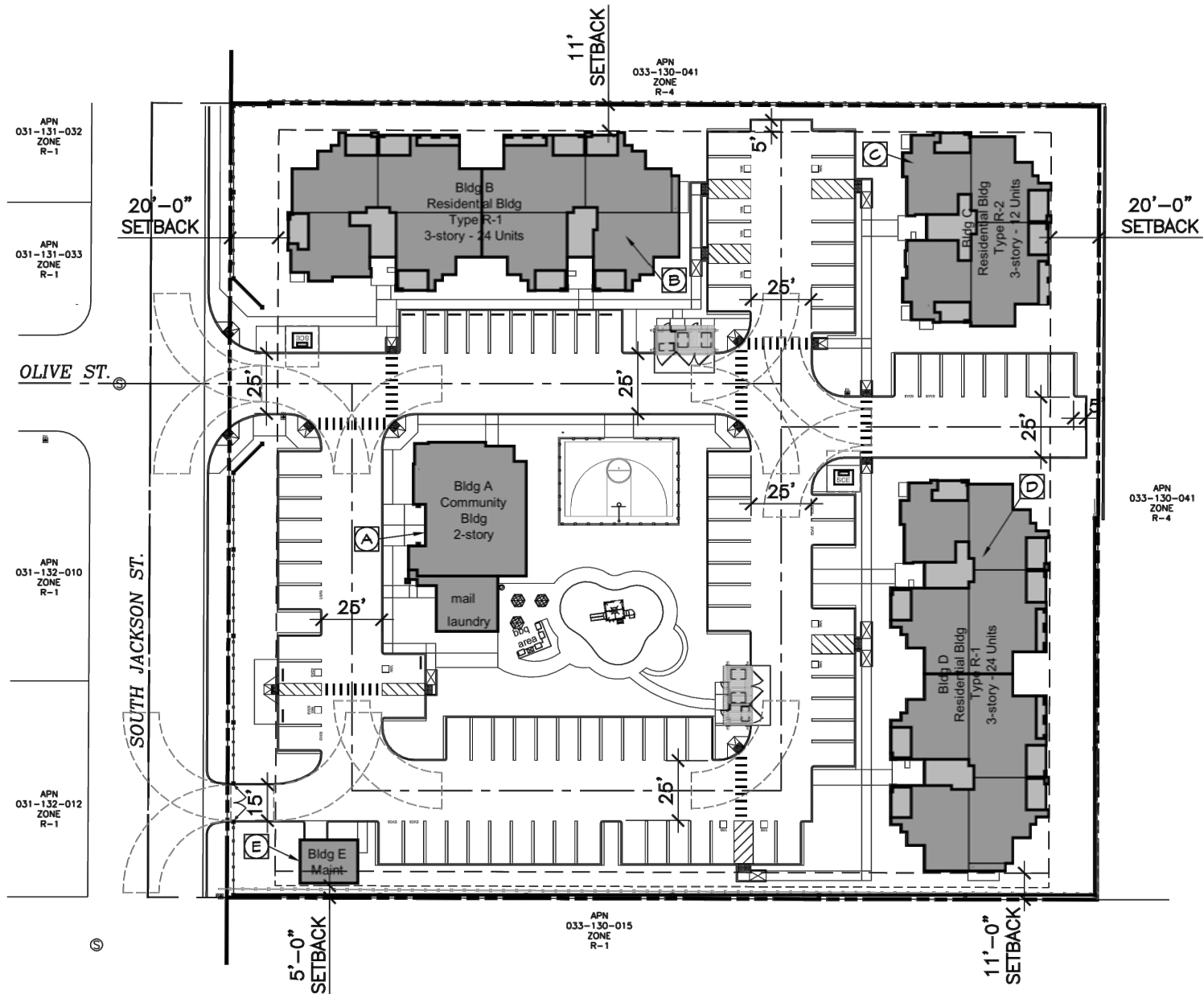


Exhibit 2: Proposed Project Site Plan

3.0 METHODOLOGY

3.1 INTERSECTION ANALYSIS METHODOLOGY

Level of Service (LOS) is commonly used to describe the quality of flow on roadways and at intersections using a range of LOS from LOS A (free flow with little congestion) to LOS F (severely congested conditions). The definitions for LOS for interruption of traffic flow differ depending on the type of traffic control (traffic signal, unsignalized intersection with side street stops, unsignalized intersection with all-way stops).

Analysis of signalized intersections within the City of Red Bluff is based on the Intersection Capacity Utilization (ICU) methodology. The ICU methodology compares the traffic volume using the intersection to the capacity of the intersection. The resulting volume-to-capacity ratio represents that portion of the total hourly capacity required to provide sufficient capacity to accommodate all intersection traffic if all approaches operate at capacity.

The table below describes the general characteristics of traffic flow and accompanying delay ranges at signalized intersections.

ICU – LOS & V/C Ranges – Signalized Intersections

Level of Service	Description	Volume/Capacity Ratio
A	Represents free flow. Individual users are virtually unaffected by the presence of others in the traffic stream. Freedom to select desired speeds and to maneuver within the traffic stream is extremely high. Intersection delays are very short, less than 5 seconds. The general level of comfort and convenience provided to the motorist, passenger, or pedestrian is excellent.	≤ 0.60
B	Stable flow, but the presence of other users in the traffic stream begins to be noticeable. Freedom to select desired speeds is relatively unaffected, but there is a slight decline in the freedom to maneuver within the traffic stream from LOS A. Intersection delays are somewhat increased, to between 5 and 15 seconds. The level of comfort and convenience provided is somewhat less than at LOS A because the presence of others in the traffic stream begins to affect individual behavior.	0.61 – 0.70
C	Stable flow, but marks the beginning of the range of flow in which the operation of individual users becomes significantly affected by interactions with others in the traffic stream. The selection of speed is now affected by the presence of others, and maneuvering within the traffic stream requires substantial vigilance on the part of the user. Intersection delays are in the range of 15 to 25 seconds. The general level of comfort and convenience declines noticeably at this level.	0.71 – 0.80

D	High-density but stable flow. Speed and freedom to maneuver are severely restricted, and the driver or pedestrian experiences a generally poor level of comfort and convenience. Intersection delays are between 25 and 40 seconds. Small increases in traffic flow will generally cause operational problems at this level.	0.81 – 0.90
E	Represents operating conditions at or near the capacity level. All speeds are reduced to a low, but relatively uniform value. Freedom to maneuver within the traffic stream is extremely difficult, and it is generally accomplished by forcing a vehicle or pedestrian to 'give way' to accommodate such maneuvers. Intersection delays range between 40 to 60 seconds. Comfort and convenience levels are extremely poor, and driver or pedestrian frustration is generally high. Operations at this level are usually unstable, because small increases in flow and minor perturbations within the traffic stream will cause breakdowns.	0.91 – 1.00
F	Forced or breakdown flow. This condition exists wherever the amount of traffic approaching a point exceeds the amount which can transverse the point. Queues form behind such locations. Operations within the queue are characterized by stop-and-go waves, and they are extremely unstable. Vehicles may progress at reasonable speeds for several hundred feet or more, then be required to stop in a cyclic fashion. Intersection delays are greater than 60 seconds. Level-of-service F is used to describe the operating conditions within the queue, as well as the point of the breakdown. It should be noted, however, that in many cases, operating conditions of vehicles or pedestrians discharged from the queue may be quite good. Nevertheless, it is the point at which arrival flow exceeds discharge flow, which causes the queue to form, and level-of-service F is an appropriate designation for such points.	≥ 1.01

Source: City of Red Bluff, *Circulation Element Intent*, August 1991.

Collected peak hour traffic volumes have been adjusted using a peak hour factor (PHF) to reflect peak 15-minute volumes. It is a common practice in LOS analysis to conservatively use a peak 15-minute flow rate applied to the entire hour to derive flow rates in vehicles per hour that are used in the LOS analysis. The PHF is the relationship between the peak 15-minute flow rate and the full hourly volume.

$$\text{PHF} = [\text{Hourly Volume}] / [4 * \text{Peak 15-Minute Volume}]$$

The use of a 15-minute PHF produces a more detailed and conservative analysis compared to analyzing vehicles per hour. Existing PHFs obtained from the existing traffic counts have been used for all analysis scenarios in this study.

The *Highway Capacity Manual (HCM) 6* (Transportation Research Board, 2016) methodology expresses the LOS of an intersection in terms of delay time for the intersection approaches. The HCM methodology utilizes different procedures for different types of intersection control. Unsignalized intersection operations have been analyzed utilizing the HCM 6th Edition methodology. Intersection operation for unsignalized intersections is based on the weighted average control delay expressed in seconds per vehicle.



At a two-way or side-street stop-controlled intersections, LOS is calculated for each stop-controlled minor street movement, for the left-turn movement(s) from the major street, and for the intersection as a whole. For approaches consisting of a single lane, the delay is calculated as the average of all movements in that lane. For all-way stop-controlled intersection, LOS is computed for the intersection as a whole.

The table below describes the general characteristics of traffic flow and accompanying delay ranges at unsignalized intersections.

HCM – LOS & Delay Ranges – Unsignalized Intersections

Level Of Service	Description	Delay (in seconds)
A	Little or no delays.	0 – 10.00
B	Short traffic delays.	10.01 – 15.00
C	Average traffic delays.	15.01 – 25.00
D	Long traffic delays. Multiple vehicles in queue.	25.01 – 35.00
E	Very long delays. Demand approaching capacity of intersection	35.01 – 50.00
F	Very constrained flow with extreme delays and intersection capacity exceeded.	> 50.01

Source: Transportation Research Board, *Highway Capacity Manual*, HCM6 Edition (Washington D.C., 2016).

Intersection analysis has been performed utilizing *PTV Vistro 2022* analysis software for all signalized and unsignalized intersections.

3.2 ROADWAY SEGMENT ANALYSIS METHODOLOGY

LOS for roadway segments is based on volume/capacity ratio (V/C). Roadway capacities for various types of facilities have been referenced from the *Tehama County General Plan Update 2009-2029* (March 2009). The capacities for each type of facility are defined below.

County of Tehama Roadway Capacities

Facility Type	Capacity
Local/Minor	2,000 Vehicles Per Day
Collector	12,000 Vehicles Per Day
Minor Arterial	23,500 Vehicles Per Day
Major Arterial	35,000 Vehicles Per Day

The table below describes the LOS and V/C ranges for roadway segments.

LOS & V/C Ranges – Roadway Segments

Level of Service	Volume/Capacity Ratio
A	≤ 0.60
B	0.61 – 0.70
C	0.71 – 0.80
D	0.81 – 0.90
E	0.91 – 1.00
F	≥ 1.01

3.3 CITY OF RED BLUFF PERFORMANCE CRITERIA

The City of Red Bluff has established level of service “C” or better as acceptable LOS for all intersections along the designated street and highway system in the City’s General Plan Circulation Element. As such, intersections operating at LOS D, E, or F will be considered deficient.

A transportation deficiency occurs if the addition of Project trips to an intersection that is operating at an acceptable LOS (i.e., LOS A, B, or C) causes the intersection to operate at an unacceptable LOS (i.e., LOS D, E, or F).

4.0 EXISTING CONDITIONS

4.1 EXISTING CIRCULATION NETWORK/STUDY AREA CONDITIONS

The characteristics of the roadway system in the vicinity of the proposed project are described in **Table 1**.

Exhibit 3 shows *Existing* conditions lane geometry and intersection controls.

4.2 CITY OF RED BLUFF GENERAL PLAN CIRCULATION PLAN

The proposed project site is located within the City of Red Bluff. **Appendix C** contains the current *City of Red Bluff General Plan Circulation Plan* and an explanation of roadway cross sections.

4.3 EXISTING PUBLIC TRANSIT SERVICES

The City of Red Bluff is served by Tehama Rural Area eXpress (TRAX), which is a fixed route bus service connecting Red Bluff, Corning, Los Molinos, Gerber, Tehama, and places in between. **Appendix C** shows the TRAX routes in the vicinity of the project site.

TRAX Route 2 runs along Jackson Street and Reeds Avenue within the study area. The nearest Route 2 stop to the project site is located north of the intersection of Jackson Street/Lay Avenue. This stop is approximately 0.12 miles away from the proposed project.

4.4 EXISTING TRAFFIC VOLUMES

To determine the existing operation of the study intersections, AM and PM peak period traffic volumes were estimated based on new traffic counts collected on Tuesday March 1, 2022 and Tuesday March 15, 2022. Detailed traffic count data is provided in **Appendix D**.

Exhibit 4 and **Exhibit 5** show *Existing* AM and PM peak hour volumes at the study intersections.

4.5 EXISTING CONDITIONS INTERSECTION LEVEL OF SERVICE ANALYSIS

Existing conditions AM and PM peak hour intersection analysis is shown in **Table 2**. Calculations are based on the existing geometrics at the study area intersections as shown in **Exhibit 3**. HCM analysis sheets are provided in **Appendix E**.

As shown in **Table 2**, the study intersections are currently operating at an acceptable LOS during the AM and PM peak hours for *Existing* conditions.



4.6 EXISTING CONDITIONS ROADWAY SEGMENT LEVEL OF SERVICE ANALYSIS

Table 3 summarizes *Existing* conditions roadway segment analysis based on the LOS capacities referenced in the *Tehama County General Plan Update 2009-2029* (March 2009).

As shown in **Table 3**, the study roadway segments are currently operating at an acceptable LOS.

Table 1
Roadway Characteristics Within Study Area

Roadway	Classification ¹	Jurisdiction	Direction	Existing Travel Lanes	Median Type ²	Speed Limit (mph)	On-Street Parking
Jackson	Minor Arterial	City of Red Bluff	North-South	2	NM/ TWLTL	35	No
Aloha	Collector	City of Red Bluff	East-West	2	NM	25/30	Yes/No
Lay	Local	City of Red Bluff	East-West	2	NM	25	Yes
Olive	Local	City of Red Bluff	East-West	2	NM	25	Yes
Reeds	Local	City of Red Bluff	East-West	2	NM	25	Yes
Luther	Minor Arterial	City of Red Bluff	East-West	2	NM	35	Yes/No

1: Source: City of Red Bluff Circulation Element Existing/Proposed Circulation Map (February 4, 2008).

2: NM = No Median; TWLTL = Two-Way Left-Turn Lane.

Table 2
Intersection Analysis - Existing Conditions

Intersection			Control Type ¹	Peak Hour	Existing	
					Delay ² / [V/C] ³	LOS
1	Jackson Street	Aloha Street	AWSC	AM	19.0	C
				PM	14.6	B
2	Jackson Street	Lay Avenue	TWSC	AM	12.7	B
				PM	11.4	B
3	Jackson Street	Reeds Avenue	TWSC	AM	12.0	B
				PM	12.4	B
4	Jackson Street	Luther Road	Signal	AM	[0.743]	C
				PM	[0.719]	C
5	Jackson Street	Olive Street	TWSC	AM	15.5	C
				PM	14.1	B

1: AWSC = All-Way Stop Controlled; TWSC = Two-Way Stop Controlled.

2: Delay is shown in seconds per vehicle. Per the Highway Capacity Manual 6th Edition, overall average delay and LOS are shown for signalized and all-way stop-controlled intersections. For intersections with one-or-two-way stop-control, the delay and LOS for the worst individual movement is shown.

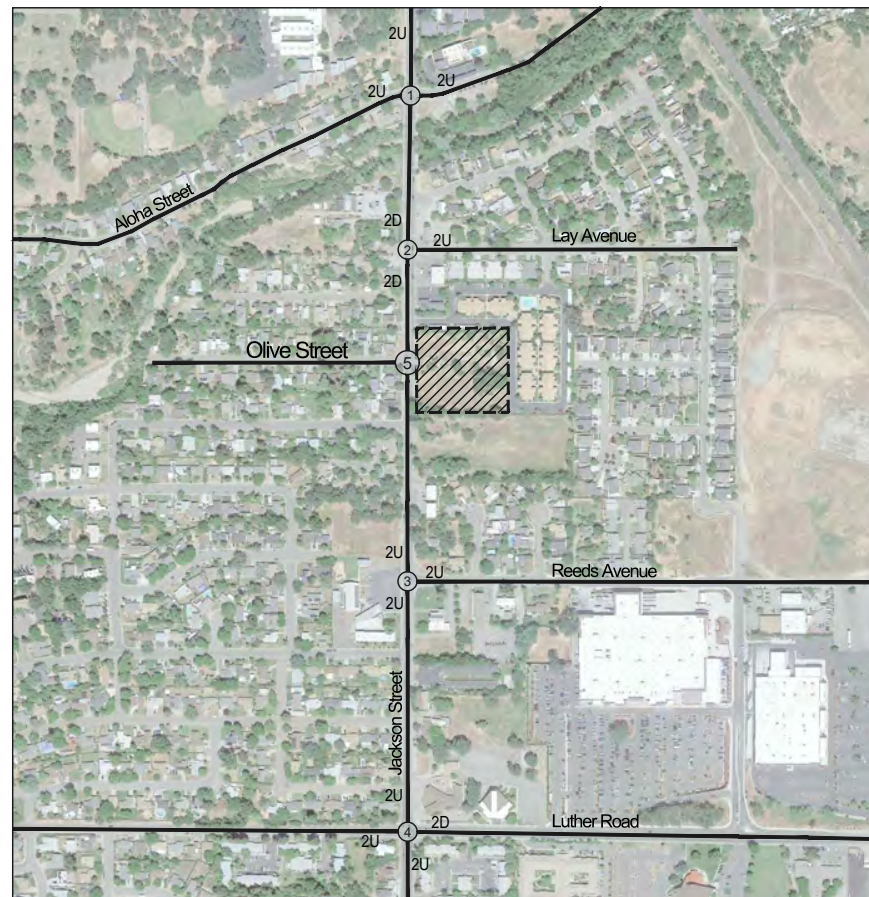
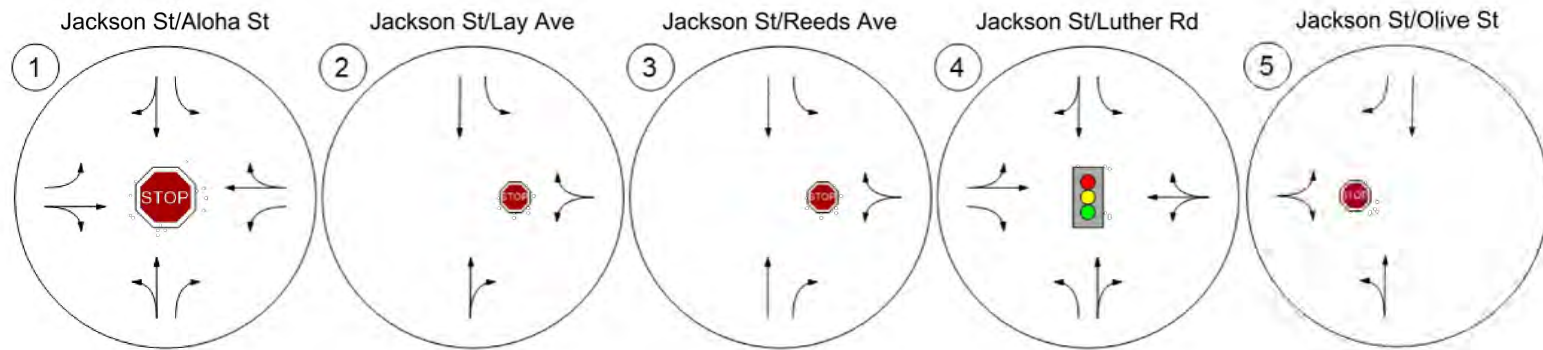
3: V/C = Volume/Capacity.

Table 3
Roadway Segment Analysis - Existing Conditions

Roadway Segment	Cross Section ¹	Capacity ²	Existing		
			ADT	V/C	LOS
Jackson Street from Lay Avenue to Reeds Avenue	2U	23,500	9,586	0.41	A

1: 2U = two-lane undivided roadway.

2: Capacity based on Tehama County General Plan Update 2009-2029 (March 2009).



- Legend:**
- Project Site
 - 2U 2-Lane Undivided Roadway
 - 2D 2-Lane Divided Roadway

Exhibit 3: Lane Geometry and Intersection Controls

AM Peak Hour Trips

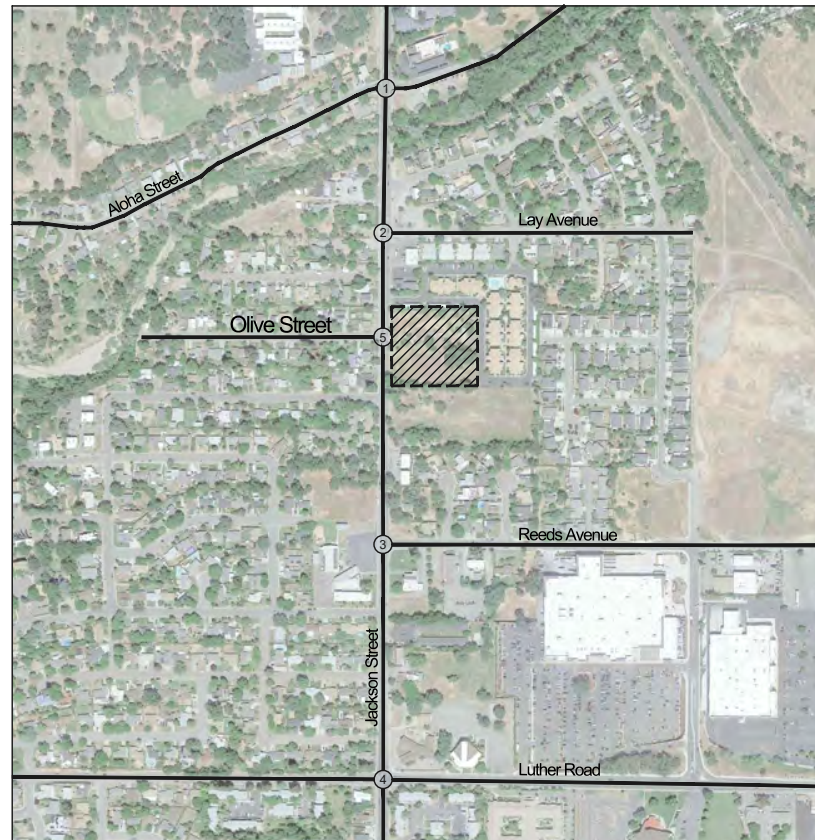
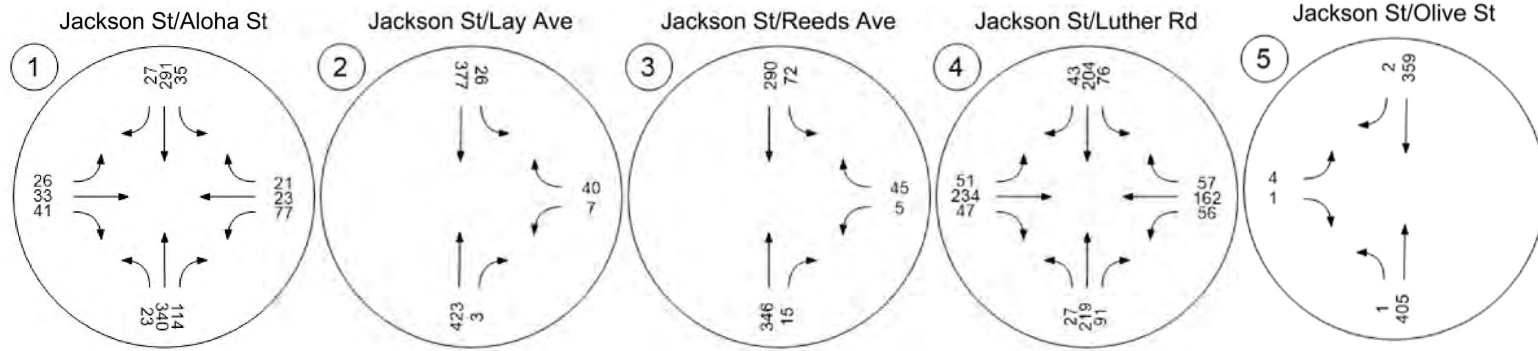


Exhibit 4: Existing AM Peak Hour Volumes

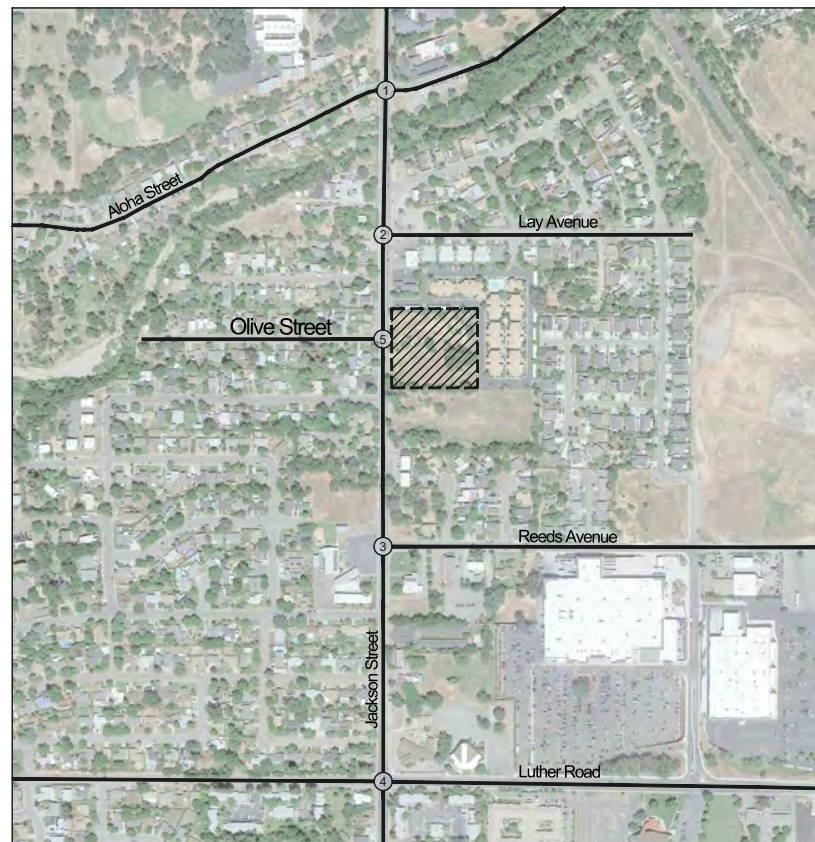
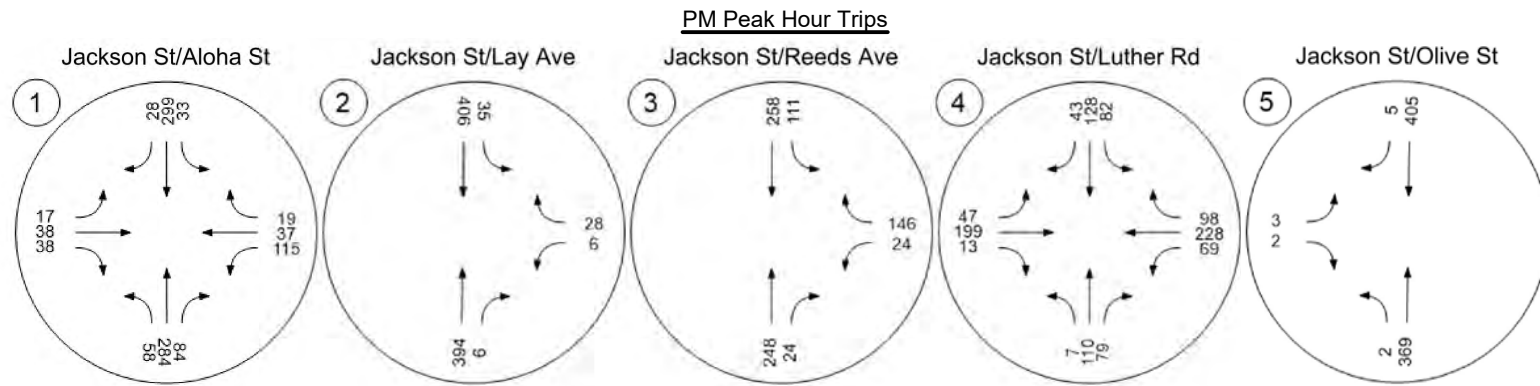


Exhibit 5: Existing PM Peak Hour Volumes

5.0 PROPOSED PROJECT

5.1 PROJECT DESCRIPTION

The proposed project consists of constructing 61 affordable apartments. Site access is planned via a full access driveway on Jackson Street (aligned with Olive Street). A conceptual striping plan showing site access upon project completion is presented in **Exhibit 6**.

The proposed project is anticipated to be fully built and generating trips in 2025. To account for ambient growth on roadways, *Existing* traffic volumes have been increased by a growth rate of 2% per year over a three-year period for Project Opening Year conditions.

5.2 PROJECT TRIP GENERATION

Trip generation represents the amount of traffic, both inbound and outbound, produced by a development. Determining trip generation for a proposed project is based on projecting the amount of traffic that the specific land uses being proposed will produce. Industry standard *Institute of Transportation Engineers (ITE) Trip Generation Manual (11th Edition, 2021)* trip generation rates were used to determine trip generation of for the proposed project land use.

Table 4 summarizes the projected AM peak hour, PM peak hour and daily trip generation of the proposed project. The proposed project is projected to generate 293 daily trips, 22 AM peak hour trips, and 28 PM peak hour trips.

5.3 PROJECT TRIP DISTRIBUTION

Projecting trip distribution involves the process of identifying probable destinations and traffic routes that will be utilized by the proposed project's traffic. The potential interaction between the proposed land use and surrounding regional access routes are considered to identify the probable routes onto which project traffic would distribute. The projected trip distribution for the proposed project is based on anticipated travel patterns to and from the project site.

Exhibit 7 shows the projected trip distribution of proposed project trips. **Exhibit 8** and **Exhibit 9** show the AM and PM peak hour project trip assignment.

Table 4
Project Trip Generation

Proposed Land Use ¹	ITE Code	Qty	Unit ²	Daily		AM Peak Hour					PM Peak Hour				
				Rate	Volume	Rate	In:Out Split	Volume			Rate	In:Out Split	Volume		
								In	Out	Total			In	Out	Total
Affordable Housing	223	61	DU	4.81	293	0.36	29:71	6	16	22	0.46	59:41	17	11	28

1: Trip generation and pass-by rates from ITE Trip Generation (11th Edition, 2021).

2: DU = Dwelling Units.



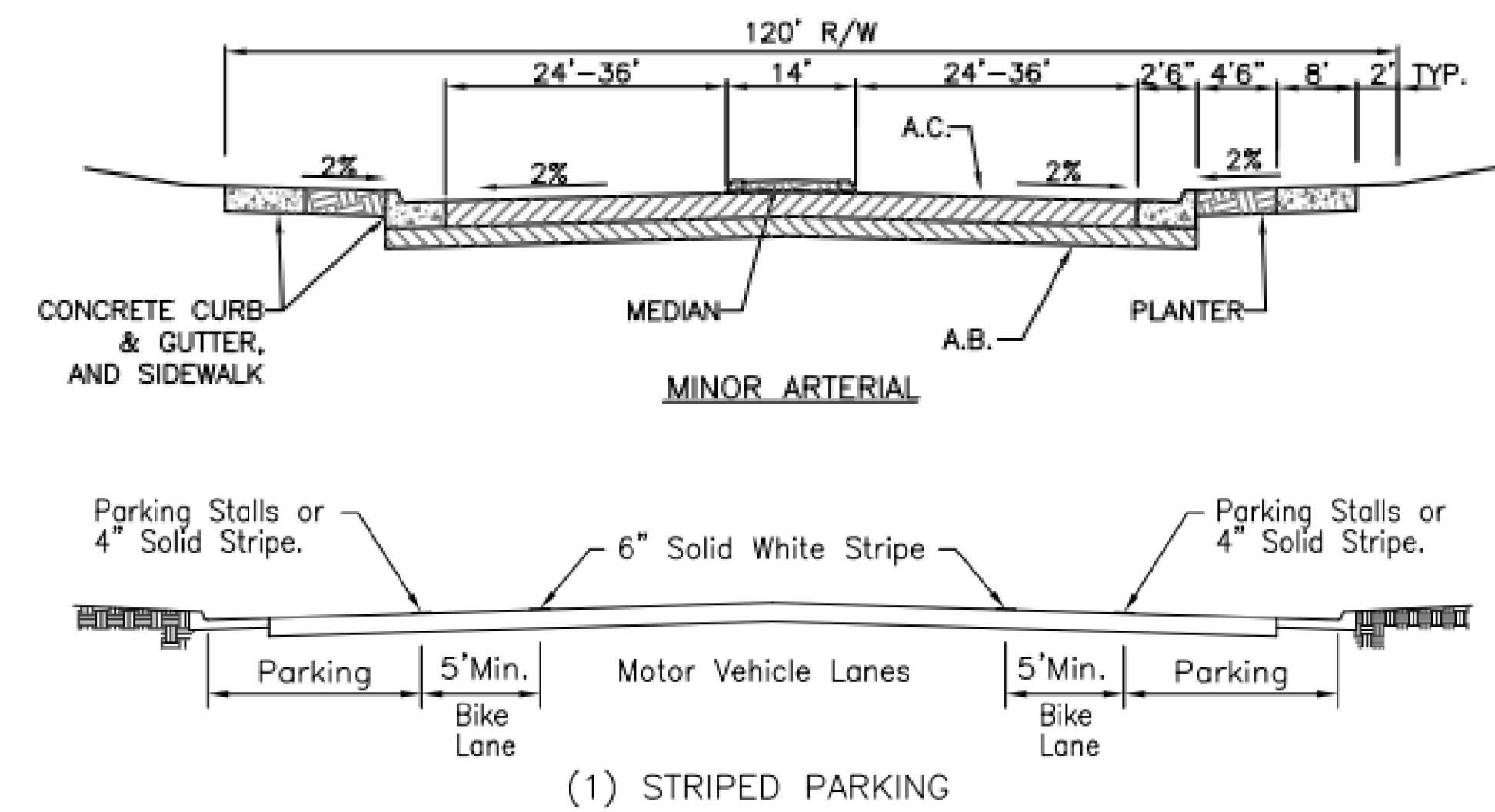
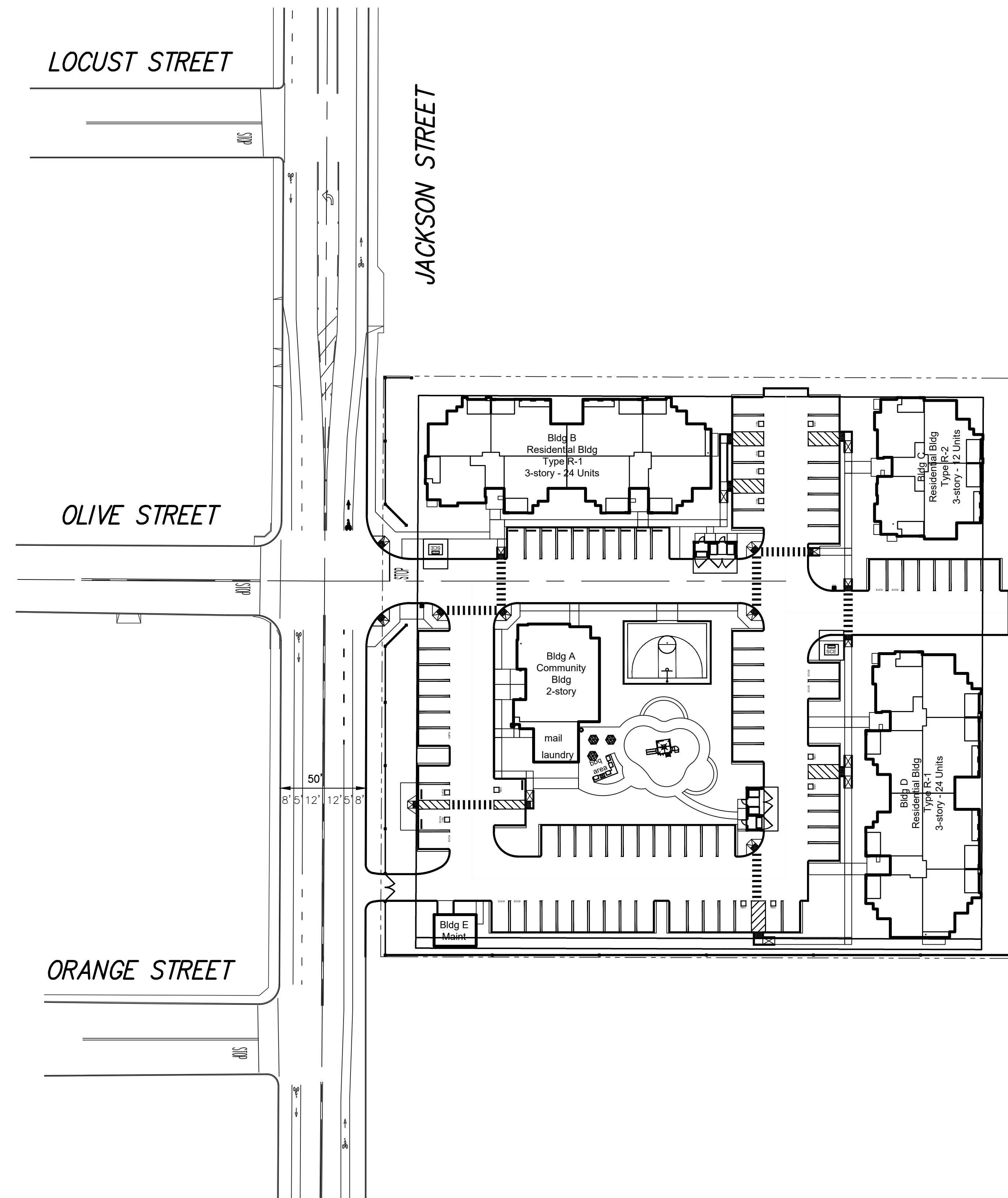
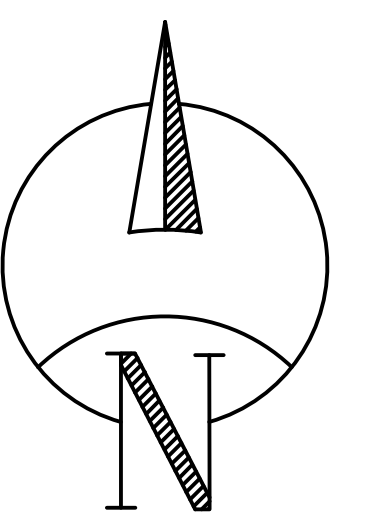


EXHIBIT 6: CONCEPTUAL STRIPING PLAN
JACKSON STREET

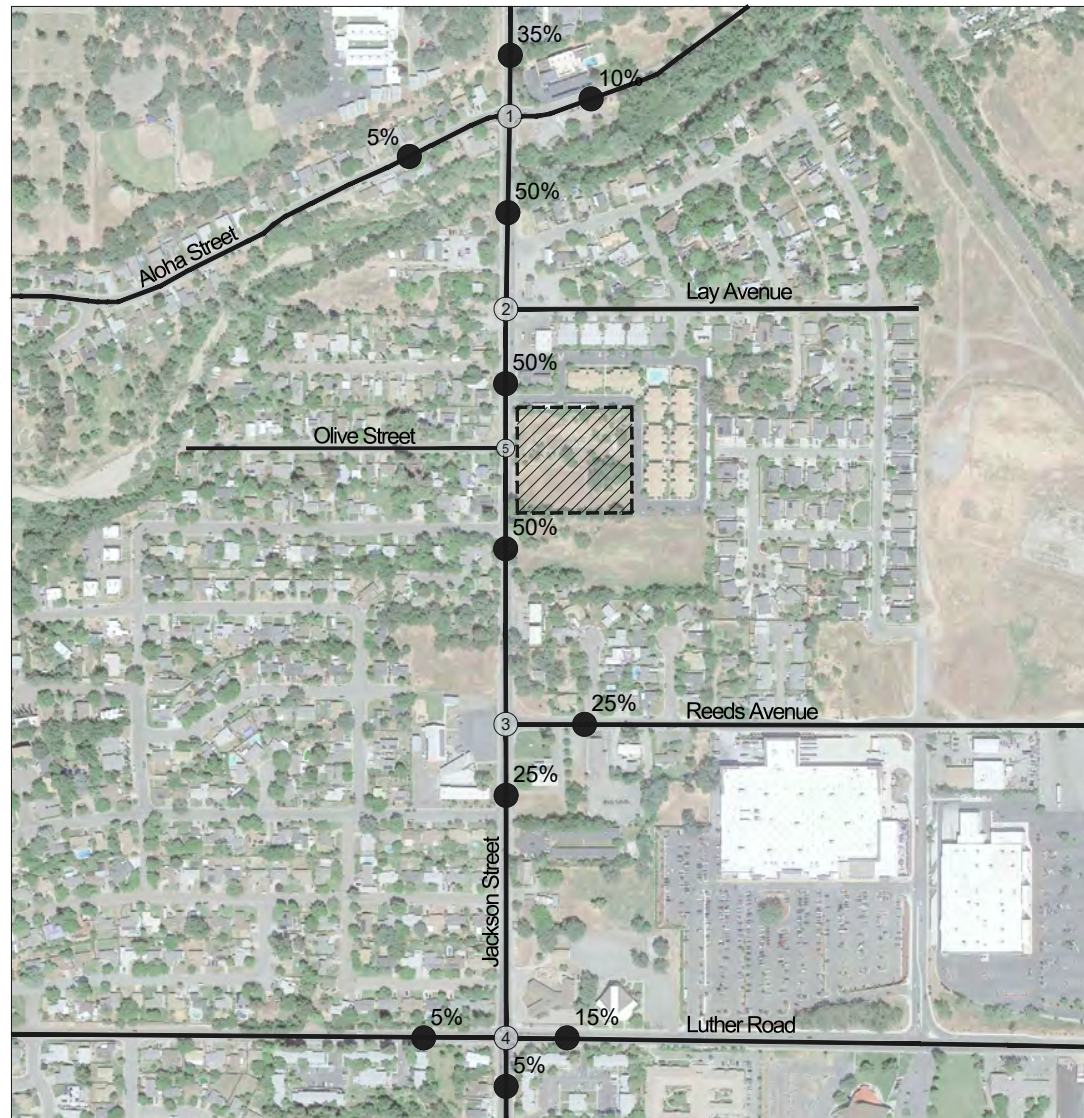
Date: 04/05/22



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SCALE: 1" = 40'
20 0 20 40 60
24x36 PLOT



Legend:

- Project Site
- XX% Percent Trip Distribution
- Ⓢ Study Intersection Location

Exhibit 7: Project Trip Distribution

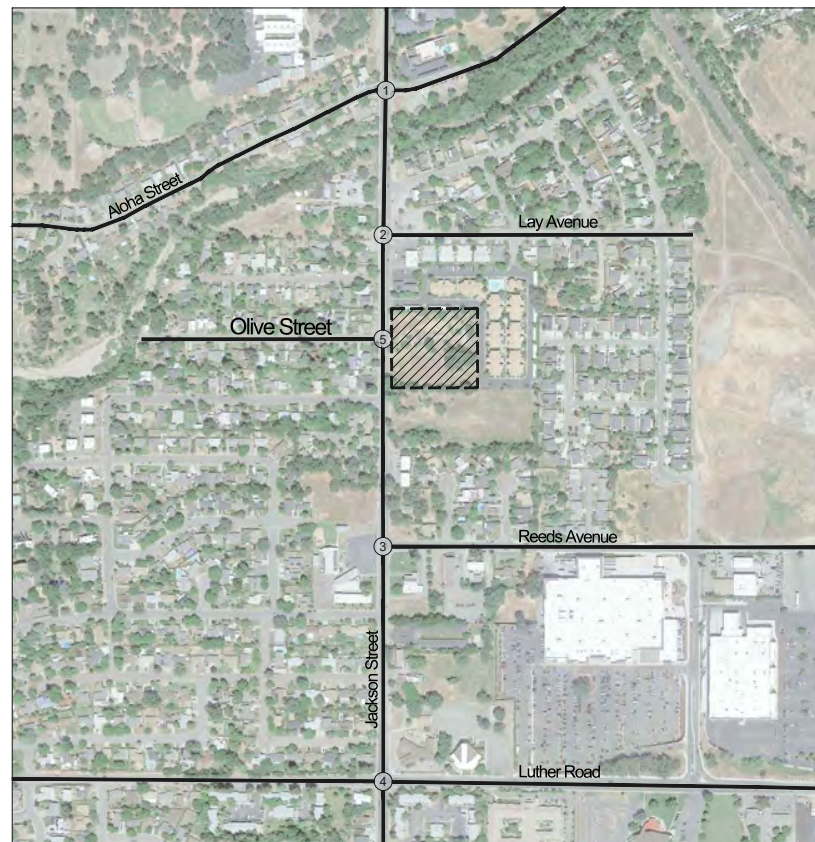
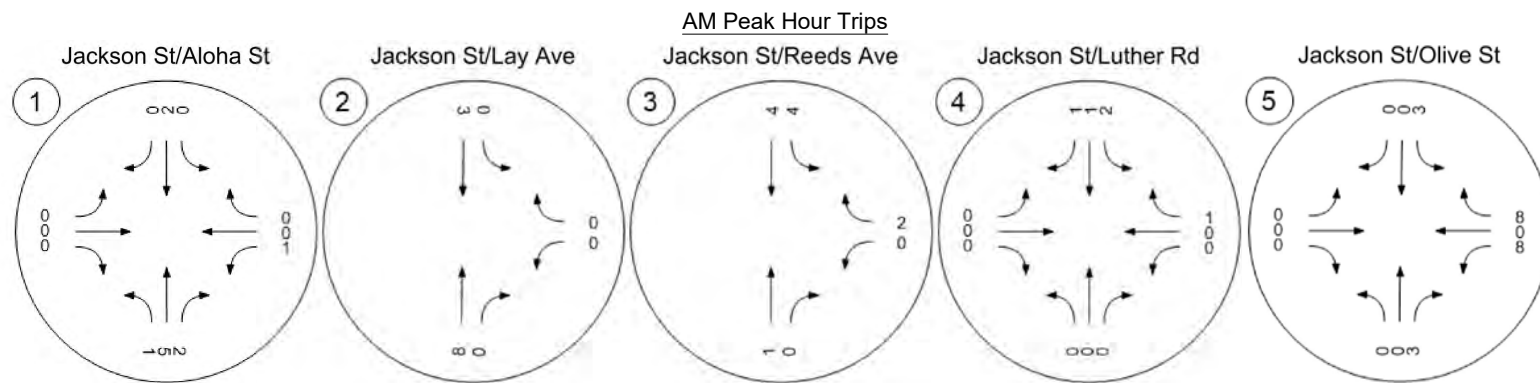


Exhibit 8: Project AM Peak Hour Volumes

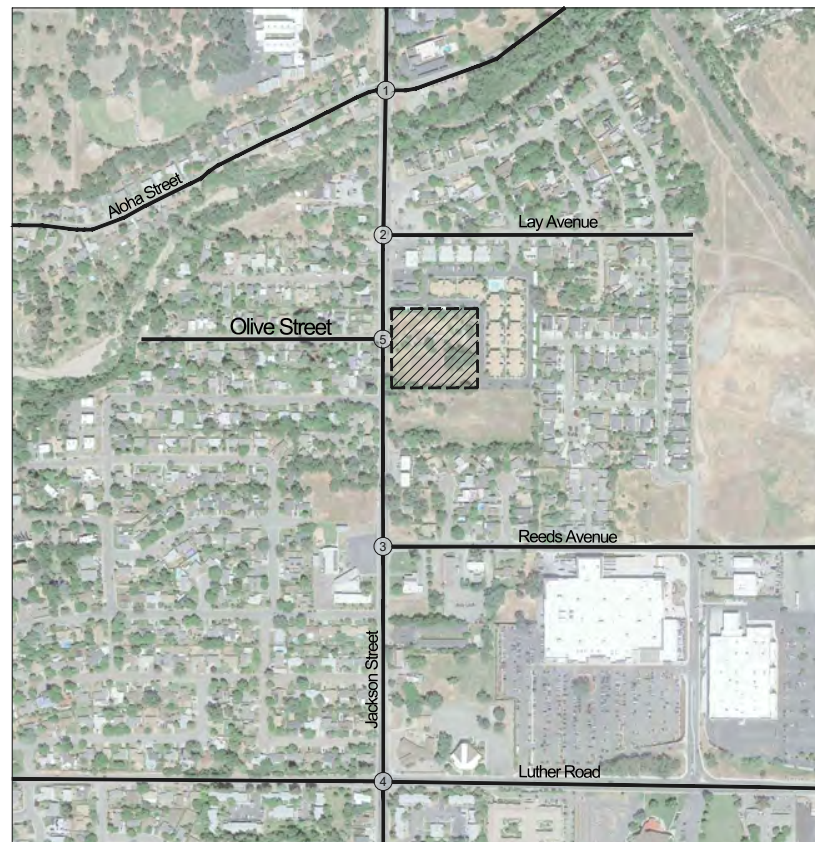
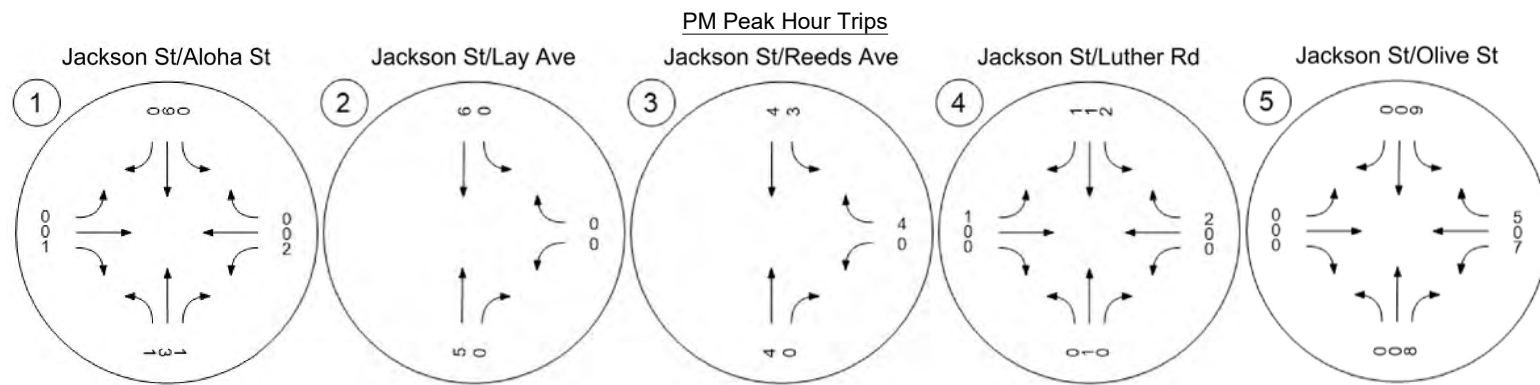


Exhibit 9: Project PM Peak Hour Volumes

6.0 OPENING YEAR WITHOUT PROJECT CONDITIONS

Opening Year Without Project traffic conditions analysis is intended to identify baseline conditions in the near-term without the proposed project and without ambient growth.

6.1 ROADWAY IMPROVEMENTS

The lane configurations and traffic controls assumed to be in place for the *Opening Year Without Project* scenario are consistent with those previously shown in **Exhibit 3**.

6.2 OPENING YEAR WITHOUT PROJECT TRAFFIC VOLUMES

Opening Year Without Project volumes include background traffic. Since the proposed project is expected to be built and generating trips in 2025, *Opening Year Without Project* volumes include a growth rate of 2% per year for three years applied to *Existing* volumes.

$$\text{Opening Year Without Project Volumes} = \text{Existing (2022) Counts} * 1.02^3$$

Exhibit 10 and **Exhibit 11** show *Opening Year Without Project* AM and PM peak hour volumes at the study intersections.

6.3 OPENING YEAR WITHOUT PROJECT LEVEL OF SERVICE ANALYSIS

Opening Year Without Project AM and PM peak hour intersection analysis is shown in **Table 5**. HCM analysis sheets are provided in **Appendix E**.

As shown in **Table 5**, the study intersections are projected to continue to operate at an acceptable LOS during the AM and PM peak hours for *Opening Year* conditions.

6.4 OPENING YEAR WITHOUT PROJECT ROADWAY SEGMENT LEVEL OF SERVICE ANALYSIS

Table 6 summarizes *Opening Year Without Project* conditions roadway segment analysis based on the LOS capacities referenced in the *Tehama County General Plan Update 2009-2029* (March 2009).

As shown in **Table 6**, the study roadway segments are projected to continue operating at an acceptable LOS.

Table 5
Intersection Analysis - Opening Year Without Project Conditions

Intersection			Control Type ¹	Peak Hour	Opening Year Without Project	
					Delay ² / [V/C] ³	LOS
1	Jackson Street	Aloha Street	AWSC	AM	22.6	C
				PM	16.1	C
2	Jackson Street	Lay Avenue	TWSC	AM	13.1	B
				PM	11.6	B
3	Jackson Street	Reeds Avenue	TWSC	AM	12.4	B
				PM	12.9	B
4	Jackson Street	Luther Road	Signal	AM	[0.780]	C
				PM	[0.756]	C
5	Jackson Street	Olive Street	TWSC	AM	16.3	C
				PM	14.7	B

1: AWSC = All-Way Stop Controlled; TWSC = Two-Way Stop Controlled.

2: Delay is shown in seconds per vehicle. Per the Highway Capacity Manual 6th Edition, overall average delay and LOS are shown for signalized and all-way stop-controlled intersections. For intersections with one-or-two-way stop-control, the delay and LOS for the worst individual movement is shown.

3: V/C = Volume/Capacity.

Table 6
Roadway Segment Analysis - Opening Year Without Project Conditions

Roadway Segment	Cross Section ¹	Capacity ²	Opening Year Without Project		
			ADT	V/C	LOS
Jackson Street from Lay Avenue to Reeds Avenue	2U	23,500	10,173	0.43	A

1: 2U = two-lane undivided roadway.

2: Capacity based on Tehama County General Plan Update 2009-2029 (March 2009).

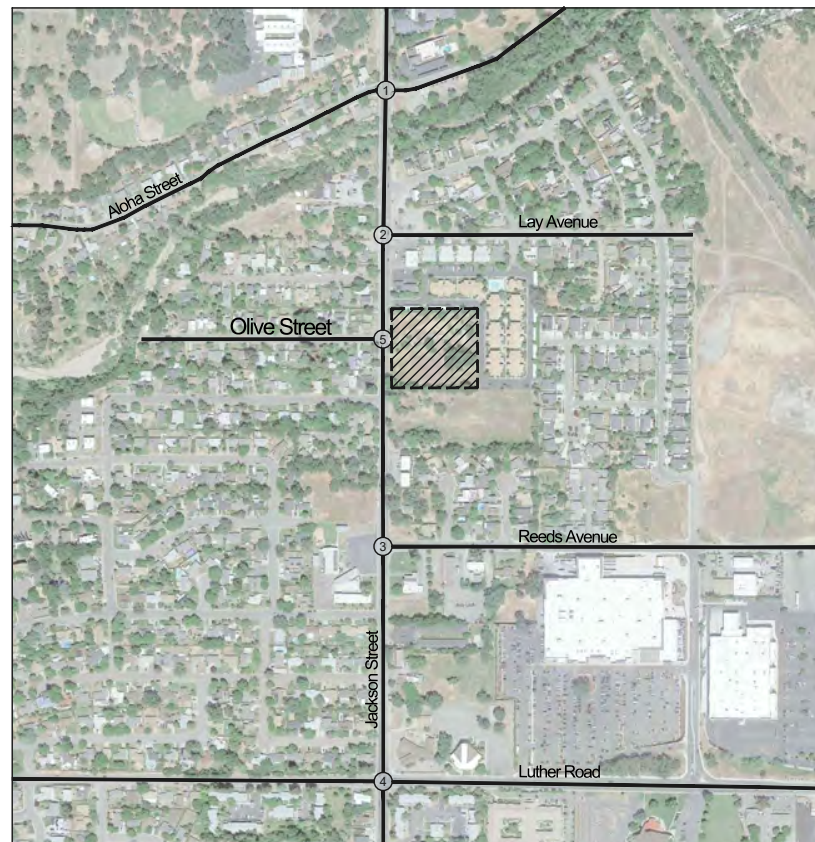
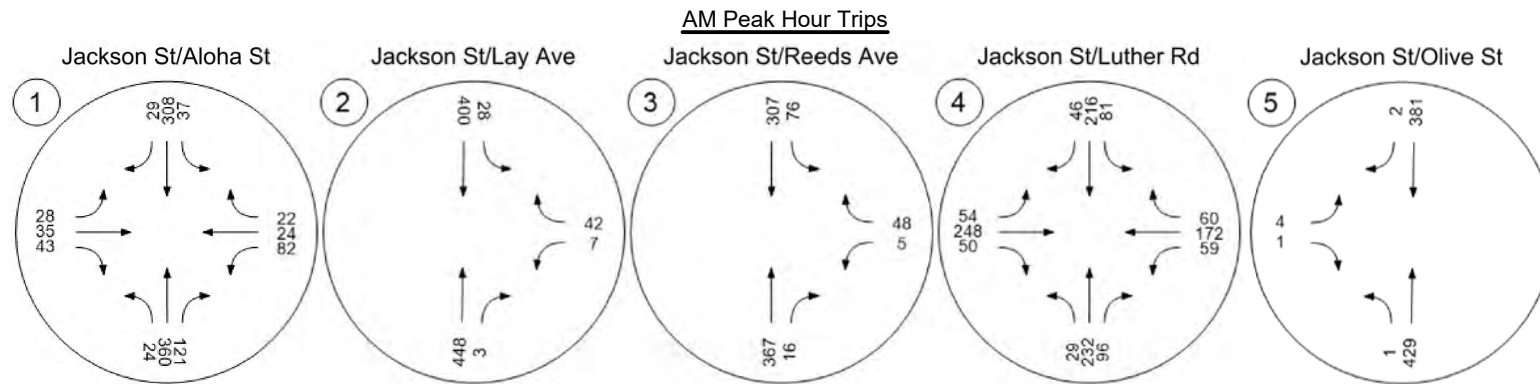


Exhibit 10: Opening Year Without Project AM Peak Hour Volumes

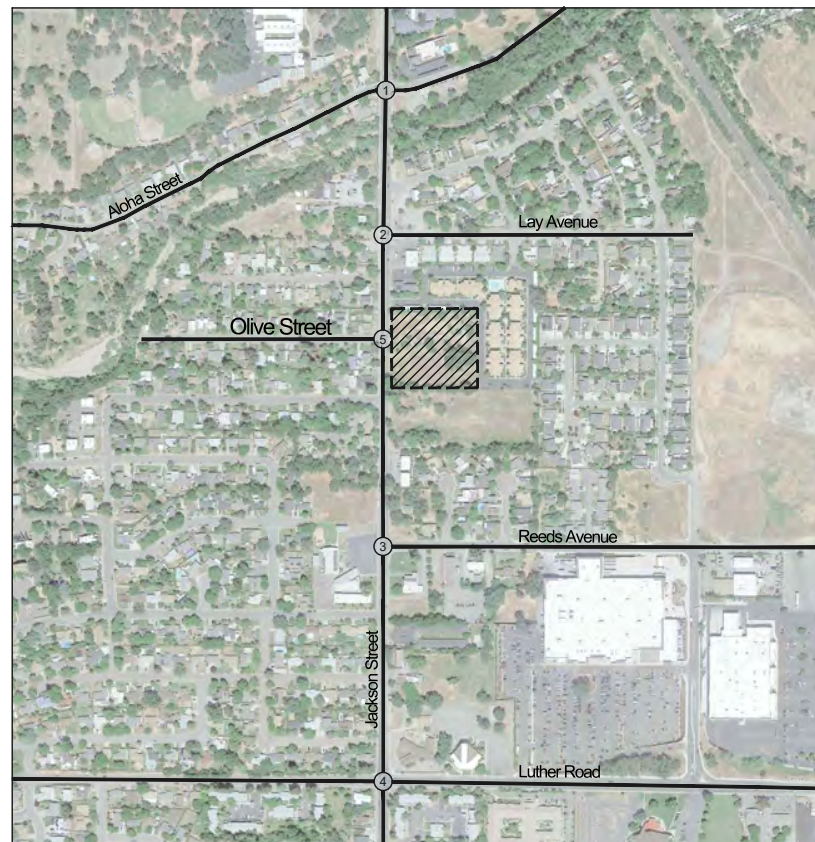
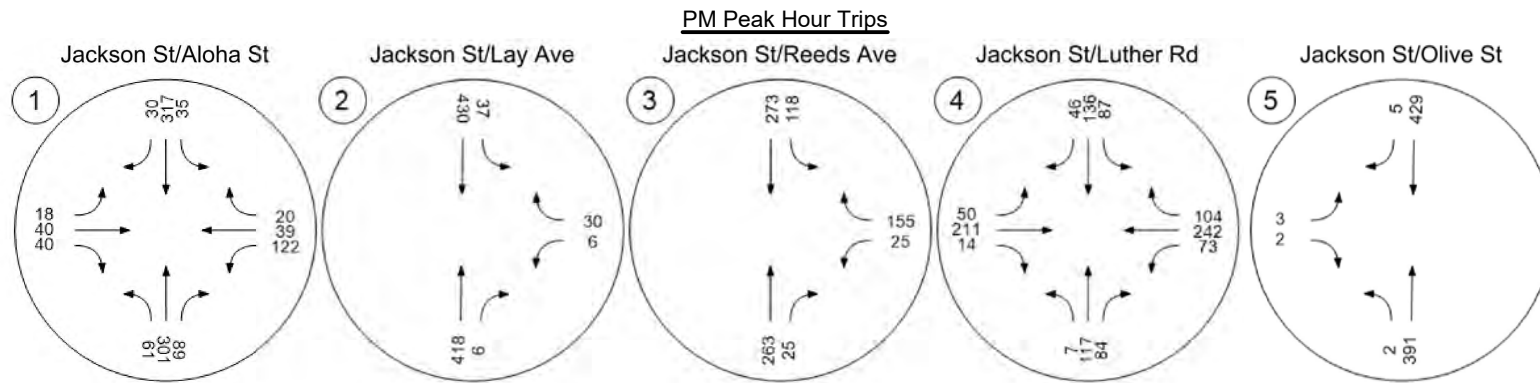


Exhibit 11: Opening Year Without Project PM Peak Hour Volumes

7.0 OPENING YEAR WITH PROJECT CONDITIONS

Opening Year With Project conditions analysis is intended to identify the project-related impacts on the planned near-term circulation system.

7.1 ROADWAY IMPROVEMENTS

The lane configurations and traffic controls assumed to be in place for the *Opening Year With Project* scenario are consistent with those previously shown in **Exhibit 3**. The project driveway will align with Olive Street and will be stop controlled.

7.2 OPENING YEAR WITH PROJECT TRAFFIC VOLUMES

Opening Year With Project volumes include background traffic plus the addition of the traffic projected to be generated by the proposed project. Since the proposed project is expected to be fully completed and generating trips in 2025, *Opening Year With Project* volumes include an ambient growth rate of 2% per year for three years applied to *Existing* volumes.

$$\text{Opening Year With Project Volumes} = (\text{Existing (2022) Counts} * 1.02^3) + \text{Project Volume}$$

Exhibit 12 and **Exhibit 13** show *Opening Year With Project* AM and PM peak hour volumes at the study intersections.

7.3 OPENING YEAR WITH PROJECT INTERSECTION LEVEL OF SERVICE ANALYSIS

Opening Year With Project conditions AM and PM peak hour intersection analysis is shown in **Table 7**. HCM analysis sheets are provided in **Appendix E**.

As shown in **Table 7**, the study intersections are projected to continue to operate at an acceptable LOS during the AM and PM peak hours for *Opening Year With Project* conditions.

A transportation deficiency occurs if the addition of Project trips to an intersection that is operating at an acceptable LOS (i.e., LOS A, B, or C) causes the intersection to operate at an unacceptable LOS (i.e., LOS D, E, or F). All study area intersections are projected to continue to operate at an acceptable LOS; therefore, the project does not cause a deficiency and no improvements are required.



7.4 OPENING YEAR WITH PROJECT ROADWAY SEGMENT LEVEL OF SERVICE ANALYSIS

Table 8 summarizes *Opening Year With Project* conditions roadway segment analysis based on the LOS capacities referenced in the *Tehama County General Plan Update 2009-2029* (March 2009).

As shown in **Table 8**, the study roadway segments are projected to continue operating at an acceptable LOS.

Table 7
Intersection Analysis - Opening Year With Project Conditions

Intersection			Control Type ¹	Peak Hour	Opening Year Without Project		Opening Year With Project		Change	Impact?
					Delay ² / [V/C] ³	LOS	Delay ² / [V/C] ³	LOS		
1	Jackson Street	Aloha Street	AWSC	AM	22.6	C	23.5	C	0.9	No
				PM	16.1	C	16.5	C	0.4	No
2	Jackson Street	Lay Avenue	TWSC	AM	13.1	B	13.2	B	0.1	No
				PM	11.6	B	11.7	B	0.1	No
3	Jackson Street	Reeds Avenue	TWSC	AM	12.4	B	12.4	B	0.0	No
				PM	12.9	B	13.1	B	0.1	No
4	Jackson Street	Luther Road	Signal	AM	[0.780]	C	[0.782]	C	0.0	No
				PM	[0.756]	C	[0.761]	C	0.0	No
5	Jackson Street	Olive Street	TWSC	AM	16.3	C	18.9	C	2.6	No
				PM	14.7	B	16.7	C	2.0	No

1: AWSC = All-Way Stop Controlled; TWSC = Two-Way Stop Controlled.

2: Delay is shown in seconds per vehicle. Per the Highway Capacity Manual 6th Edition, overall average delay and LOS are shown for signalized and all-way stop-controlled intersections. For intersections with one-or-two-way stop-control, the delay and LOS for the worst individual movement is shown.

3: V/C = Volume/Capacity.

Table 8
Roadway Segment Analysis - Opening Year With Project Conditions

Roadway Segment	Cross Section ¹	Capacity ²	Opening Year Without Project			Opening Year With Project		
			ADT	V/C	LOS	ADT	V/C	LOS
Jackson Street from Lay Avenue to Reeds Avenue	2U	23,500	10,173	0.43	A	10,320	0.44	A

1: 2U = two-lane undivided roadway.

2: Capacity based on Tehama County General Plan Update 2009-2029 (March 2009).



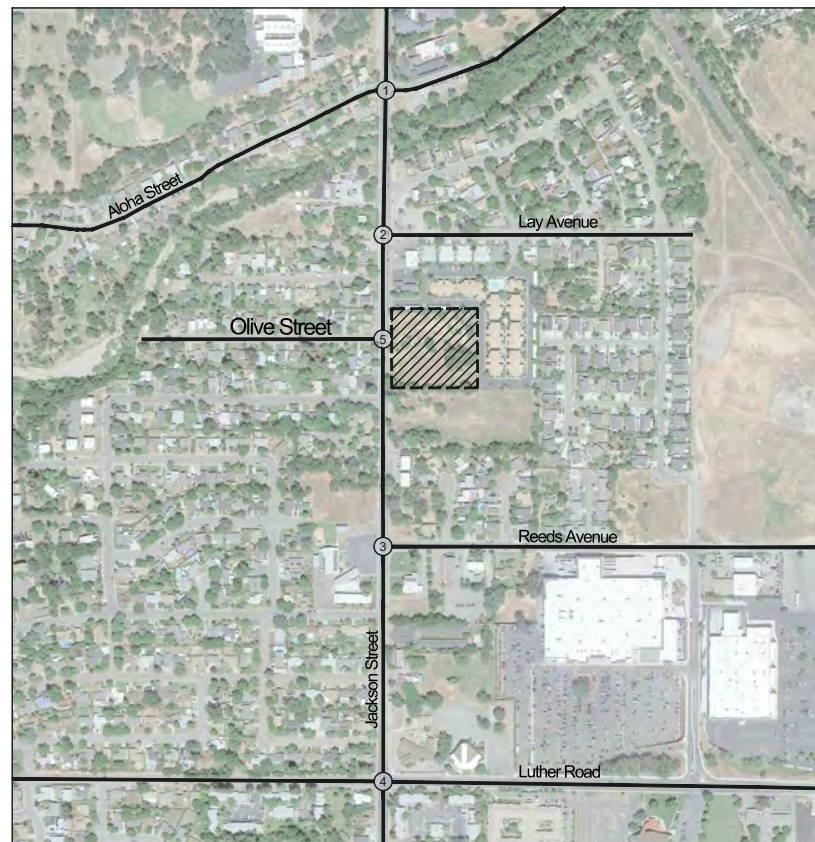
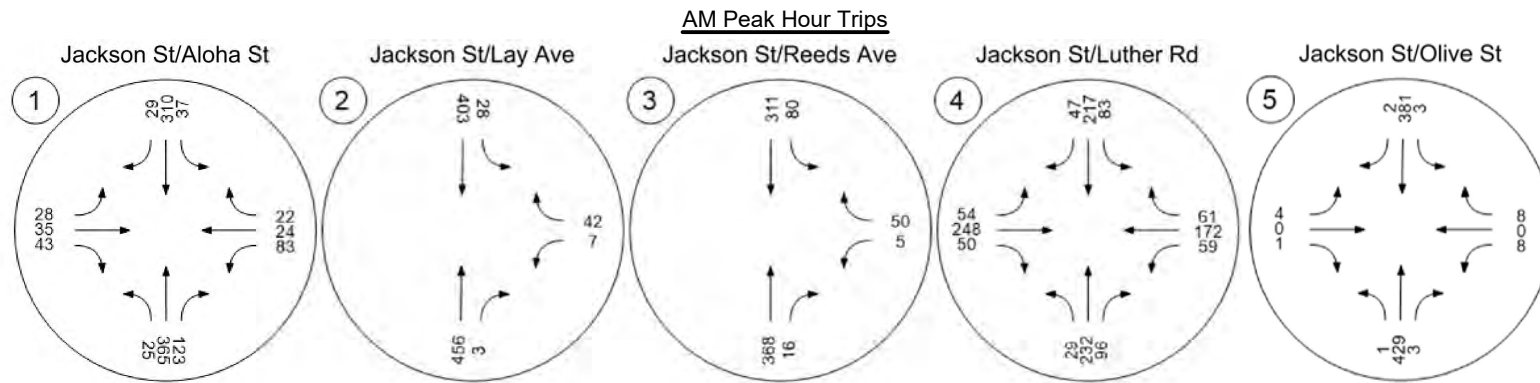


Exhibit 12: Opening Year With Project AM Peak Hour Volumes

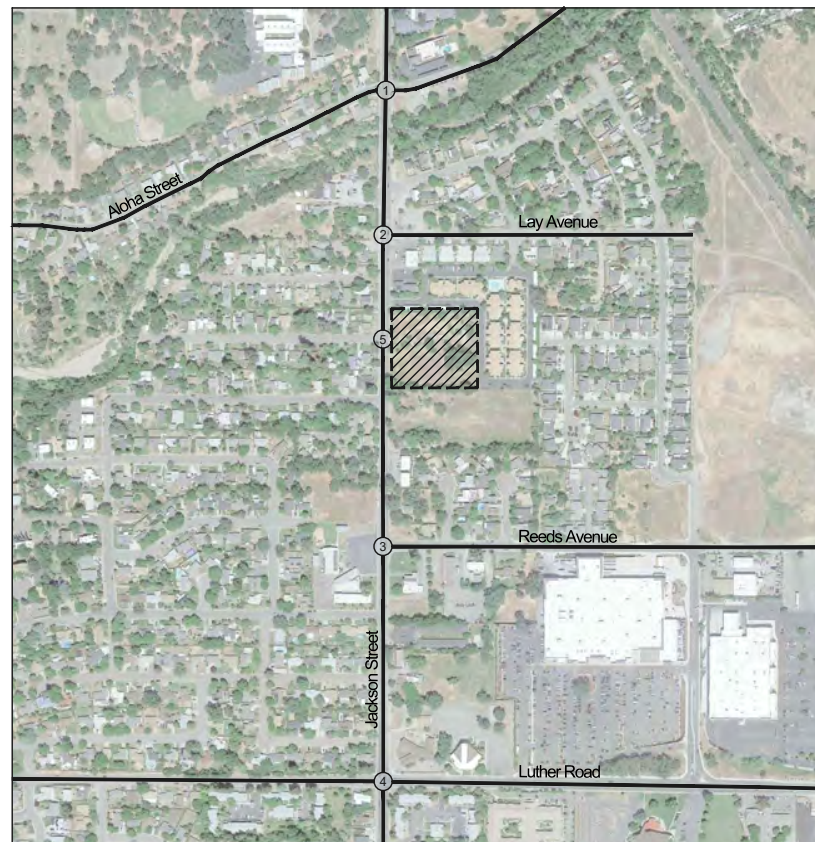
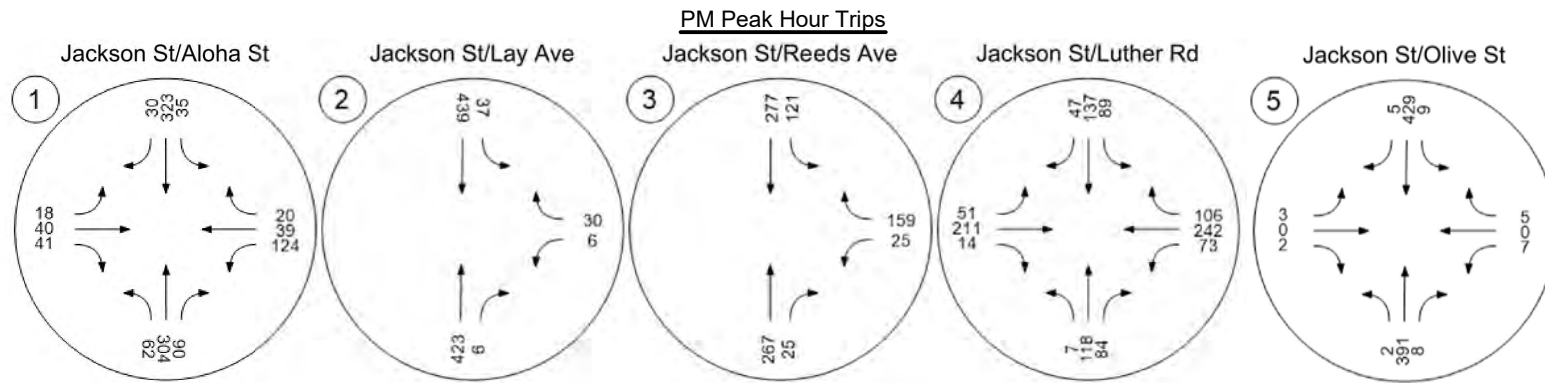


Exhibit 13: Opening Year With Project PM Peak Hour Volumes

APPENDIX

APPENDIX A

GLOSSARY OF TERMS

Glossary of Terminology

ACRONYMS:

AC	Acres
ADT	Average Daily Traffic
Caltrans	California Department of Transportation
DU	Dwelling Unit
ICU	Intersection Capacity Utilization
LOS	Level of Service
TSF	Thousand Square Feet
V/C	Volume/Capacity
VMT	Vehicle Miles Traveled

TERMS

AVERAGE DAILY TRAFFIC – *The average 24-hour volume for a stated period divided by the number of days in that period. For example, Annual Average Daily Traffic is the total volume during a year divided by 365 days.*

CAPACITY – *The maximum number of vehicles that can be reasonably expected to pass over a given section of a lane or a roadway in a given time period.*

CORNER SIGHT DISTANCE – *The minimum sight distance required by the driver of a vehicle to cross or enter the lanes of the major roadway without requiring approaching traffic travelling at a given speed to radically alter their speed or trajectory. Corner sight distance is measured from the driver's eye at 42 inches above the pavement to an object height of 36 inches above the pavement in the center of the nearest approach lane.*

CYCLE LENGTH – *The time period in seconds required for a traffic signal to complete one full cycle of indications.*

CUL-DE-SAC – *A local street open at one end only and with special provisions for turning around.*

DAILY CAPACITY – *A theoretical value representing the daily traffic volume that will typically result in a peak hour volume equal to the capacity of the roadway.*

DELAY – *The time consumed while traffic is impeded in its movement by some element over which it has no control, usually expressed in seconds per vehicle.*

DENSITY – *The number of vehicles occupying in a unit length of the through traffic lanes of a roadway at any given instant. Usually expressed in vehicles per mile.*

DESIGN SPEED – *A speed selected for purposes of design. Features of a highway, such as curvature, superelevation, and sight distance (upon which the safe operation of vehicles is dependent) are correlated to design speed.*

DIRECTIONAL SPLIT – *The percent of traffic in the peak direction at any point in time.*

FREE FLOW – *Volumes are well below capacity. Vehicles can maneuver freely and travel is unimpeded by other traffic.*

HEADWAY – *Time or distance spacing between successive vehicles in a traffic stream, front bumper to front bumper.*

LEVEL OF SERVICE – *A qualitative measure of a number of factors, which include speed and travel time, traffic interruptions, freedom to maneuver, safety, driving comfort and convenience, and operating costs.*

LOOP DETECTOR – *A vehicle detector consisting of a loop of wire embedded in the roadway, energized by alternating current and producing an output circuit closure when passed over by a vehicle.*

PASSENGER CAR EQUIVALENT (PCE) – *A metric used to assess the impact of larger vehicles, such as trucks, recreational vehicles, and buses, by converting the traffic volume of larger vehicles to an equivalent number of passenger cars.*

PEAK HOUR – *The 60 consecutive minutes with the highest number of vehicles.*

QUEUE LENGTH – *The length of vehicle queue, typically expressed in feet, waiting at a service area such as a Traffic signal, stop sign, or access gate.*

SIGHT DISTANCE – *The continuous length of roadway visible to a driver or roadway user.*

SIGNAL CYCLE – *The time period in seconds required for one complete sequence of signal indications.*

SIGNAL PHASE – *The part of the signal cycle allocated to one or more traffic movements.*

STARTING DELAY – *The delay experienced in initiating the movement of queued traffic from a stop to an average running speed through an intersection.*

STOPPING SIGHT DISTANCE – *The minimum distance required by the driver of a vehicle on the major roadway travelling at a given speed to bring the vehicle to a stop after an object on the road becomes visible. Stopping sight distance is measured from the driver's eye at 42 inches above the pavement to an object height of 6 inches above the pavement.*

TRAFFIC-ACTUATED SIGNAL – *A type of traffic signal that directs traffic to stop and go in accordance with the demands of traffic, as registered by the actuation of detectors.*

TRIP – *The movement of a person or vehicle from one location (origin) to another (destination). For example, from home to store to home is two trips, not one.*

TRIP GENERATION RATE – *The quantity of trips produced and/or attracted by a specific land use stated in terms of units such as per dwelling, per acre, and per 1,000 square feet of floor space.*

TURNING RADIUS – *The circular arc formed by the smallest turning path radius of the front outside tire of a vehicle, such as that performed by a U-turn maneuver. This is based on the length and width of the wheel base as well as the steering mechanism of the vehicle.*

VEHICLE MILES OF TRAVEL – *A measure of the amount of usage of a section of highway, obtained by multiplying the average daily traffic by length of facility in miles.*

APPENDIX B

SCOPING AGREEMENT

Scoping Agreement for Traffic Impact Analysis

This form acknowledges the requirements for the traffic impact analysis of the following project. The analysis will follow the local jurisdiction's traffic impact analysis guidelines.

Project Name: Red Bluff Apartments
Project Address: 321 South Jackson Street, Red Bluff, CA
Project Description: Affordable Apartments (61 Dwelling Units)

	<u>Consultant</u>	<u>Developer</u>
Name:	TJW Engineering	PALM COMMUNITIES
Address:	9841 Irvine Center Drive, Suite 200 Irvine, CA 92618	100 Pacifica, Suite 203 Irvine, CA 92618
Telephone:	949-878-3509	949-878-9373
Email:	daniel@tjwengineering.com	mslagerman@palmcommunities.com

Trip Generation Source: ITE Trip Generation Manual, 11th Edition (2021)

Current GP Land Use:	R-L/R-M	Proposed Land Use:	R-L/R-M
Current Zoning:	R-1/R-3	Proposed Zoning:	R-1/R-3

Is the project screened from LOS analysis? ☐ Yes ☒ No

Justification:	
----------------	--

Is the project screened from VMT analysis? ☒ Yes ☐ No

Justification:	Since the City does not have established guidelines, the Office of Planning and Research (OPR) guidelines will be used. Since the proposed project is proposing affordable housing, it is anticipated a Vehicle Miles Traveled (VMT) analysis will not be needed.
----------------	---



	Existing Trip Generation			Proposed Trip Generation		
	In	Out	Total	In	Out	Total
AM Trips				6	16	22
PM Trips				16	12	28
Daily						293

Internal Trip Capture: ☐ Yes ☒ No _____ % Trip Discount

Pass-By Allowance: ☐ Yes ☒ No _____ % Trip Discount

Trip Distribution: See attached exhibit

Project Build-out Year: 2025 Annual Ambient Growth Rate: 2%

Study Intersections:

1. Jackson St/Aloha St 6. _____
2. Jackson St/Lay Ave 7. _____
3. Jackson St/Reeds Ave 8. _____
4. Jackson St/Luther Rd 9. _____
5. _____ 10. _____

Study Roadway Segments:

1. Jackson St between Lay Ave and Reeds Ave 3. _____
2. _____ 4. _____

Analysis Scenarios:

1. Existing Traffic Conditions (existing)
2. Opening Year Without Project Conditions (Existing + Ambient + Cumulative)
3. Opening Year With Project Conditions (Existing + Ambient + Cumulative + Project)
4. _____
5. _____

Other Jurisdiction Analyzed? ☐ Yes ☒ No Name of Jurisdiction: City of Red Bluff

Date of Traffic Counts: New Traffic Counts



Additional Items to be Addressed:	TJW will prepare a concept striping plan for the proposed project access along Jackson Street. The striping plan would incorporate the anticipated driveway and roadway configuration demonstrating project site's access to and from the adjacent roadway.
-----------------------------------	---

Additional Notes:	City to provide the most recent cumulative project list so that we may add to the analysis.
-------------------	---

Signatures:



Consultant's Representative

02/17/22

Date

City (Approved By)

Date

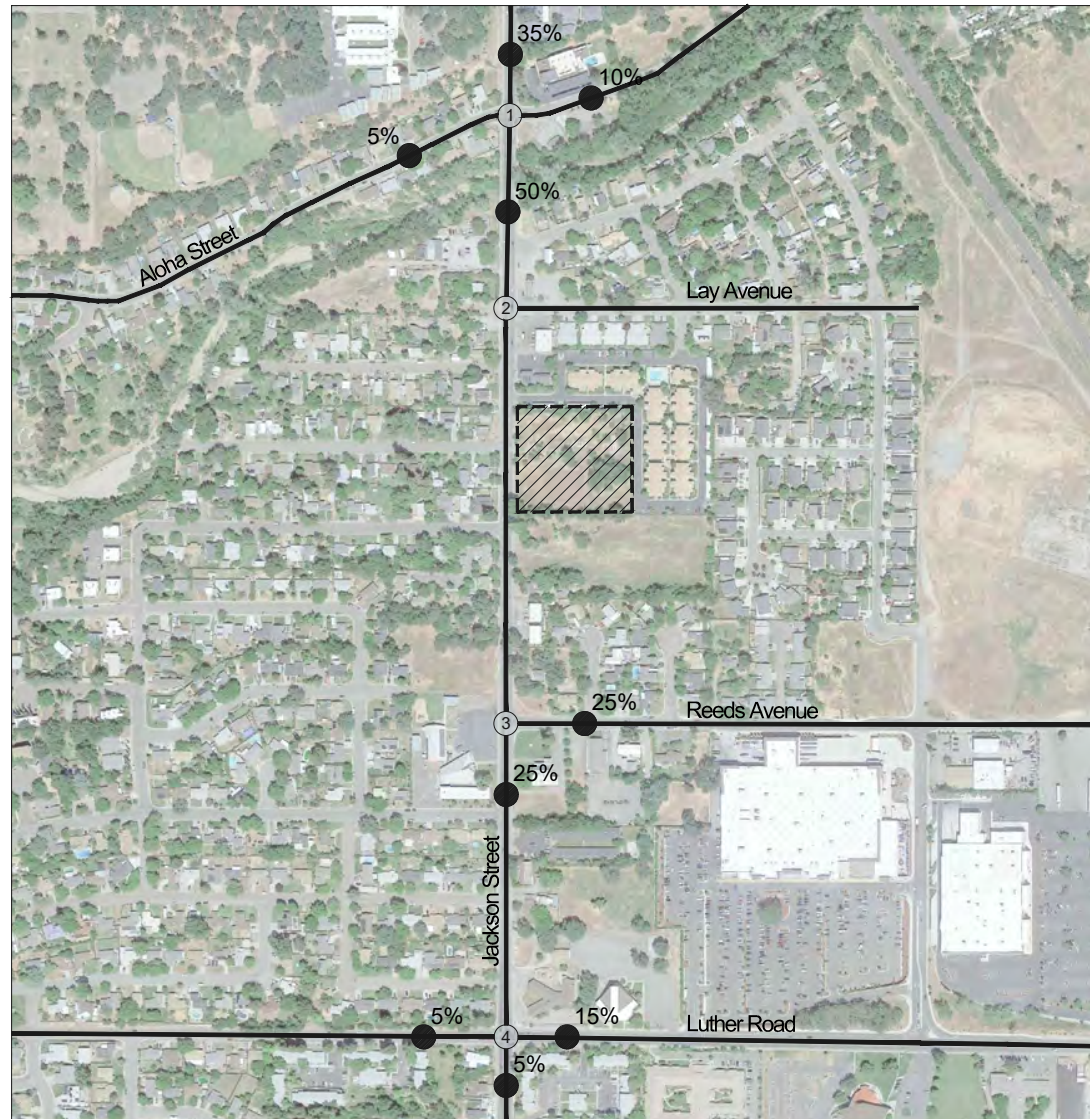


Trip Generation Table

Land Use ¹	Qty	Unit ²	Daily Trips (ADTs)		AM Peak Hour					PM Peak Hour				
			Rate	Volume	Rate	In:Out Split	Volume			Rate	In:Out Split	Volume		
							In	Out	Total			In	Out	Total
Affordable Housing (223)	61	DU	4.81	293	0.36	29:71	6	16	22	0.46	59:41	16	12	28
Total				293			6	16	22			16	12	28

1: Rates from ITE Trip Generation (11th Edition, 2021).

2: DU = Dwelling Unit.



Legend:

- Project Site
- Ⓝ Study Intersection Location
- XX% Percent Trip Distribution

Project Location



528 W. Washington Avenue Conversion Project Traffic Impact Analysis

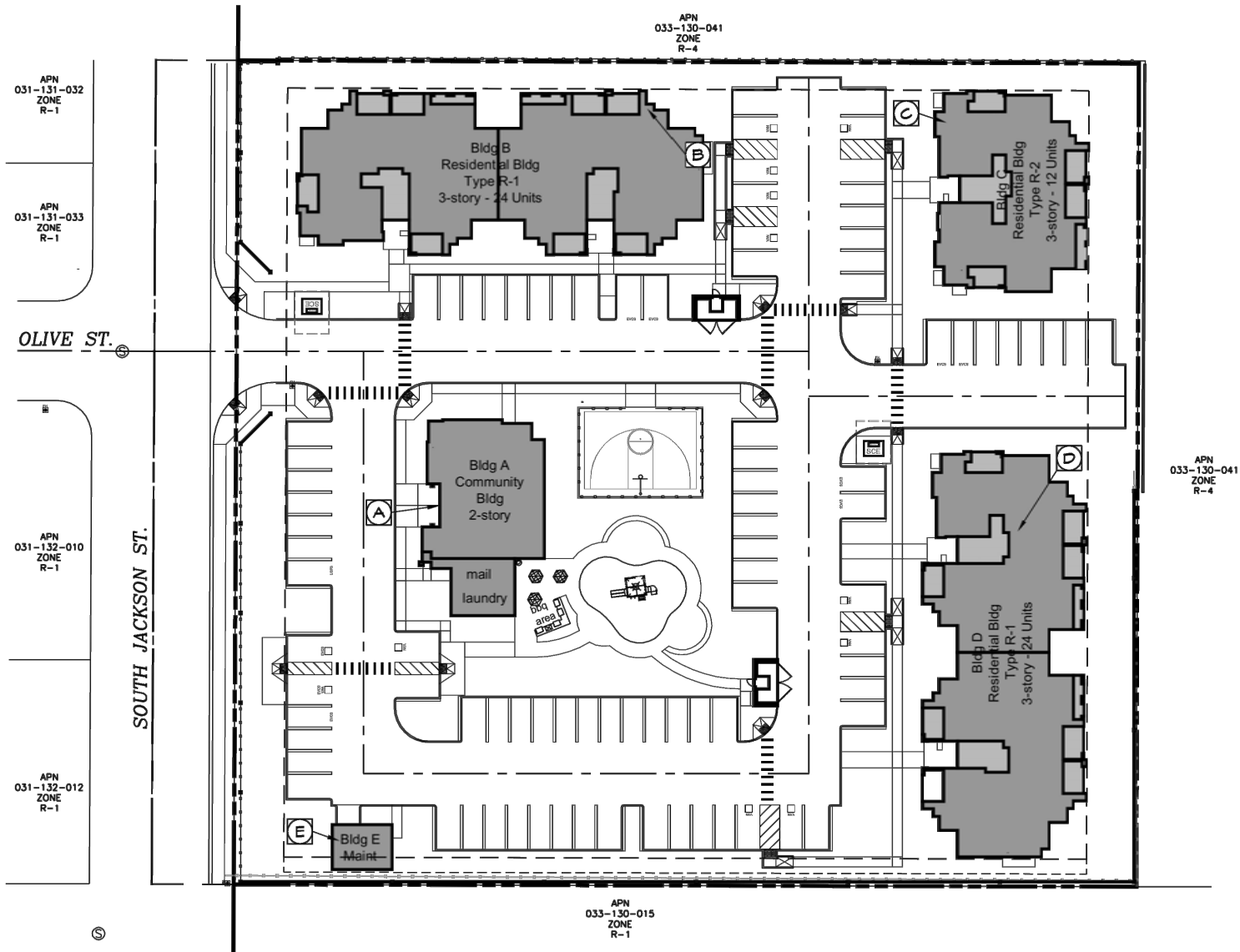


VGI-22-001



Not to Scale





to the project site



528 W. Washington Avenue Conversion Project Traffic Impact Analysis

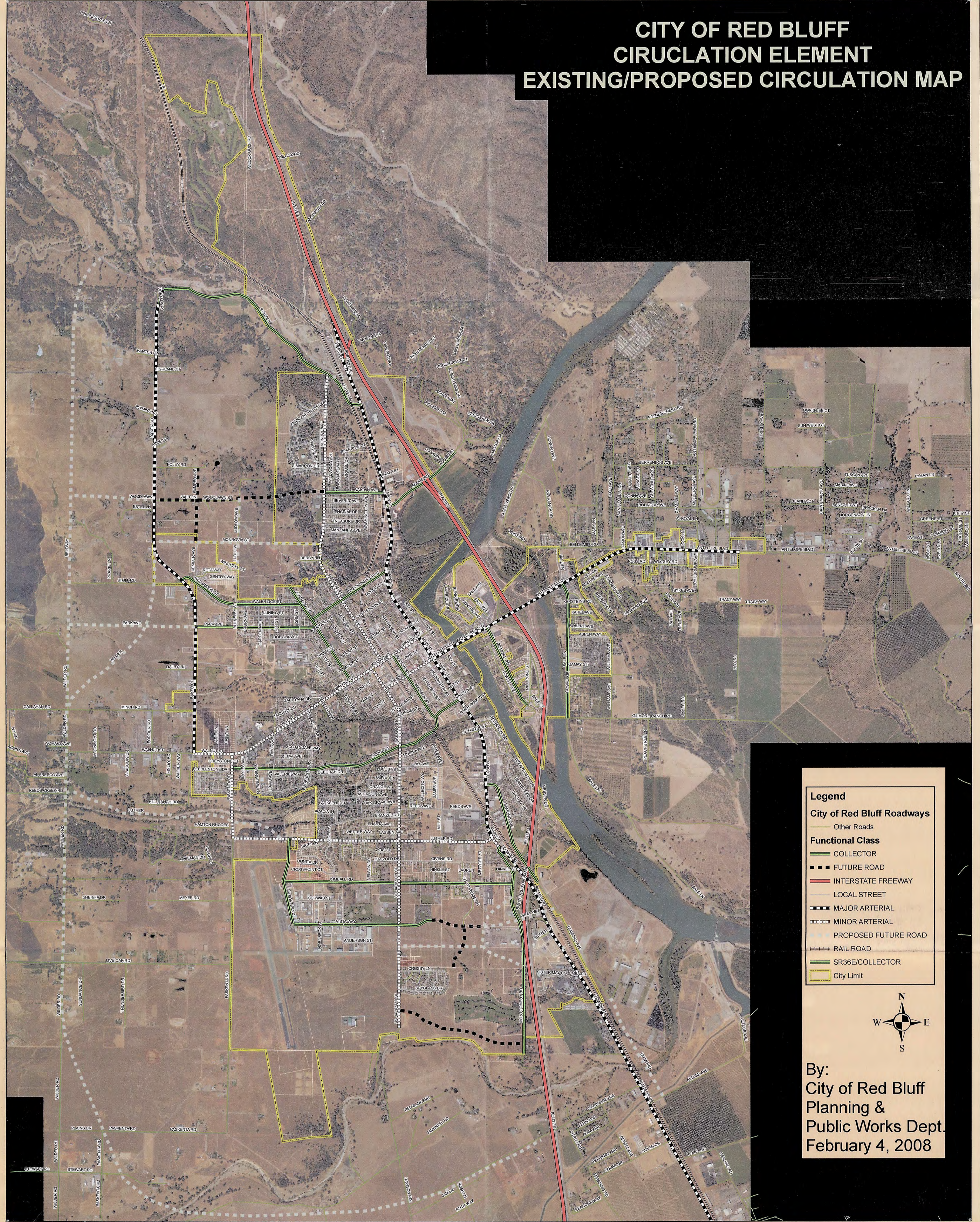
VGI-22-001



APPENDIX C

CITY OF RED BLUFF PLANS AND TRANSIT

CITY OF RED BLUFF CIRCUCLATION ELEMENT EXISTING/PROPOSED CIRCULATION MAP



Legend

City of Red Bluff Roadways

- Other Roads
- Functional Class**
- COLLECTOR
- FUTURE ROAD
- INTERSTATE FREEWAY
- LOCAL STREET
- MAJOR ARTERIAL
- MINOR ARTERIAL
- PROPOSED FUTURE ROAD
- RAIL ROAD
- SR36E/COLLECTOR
- City Limit

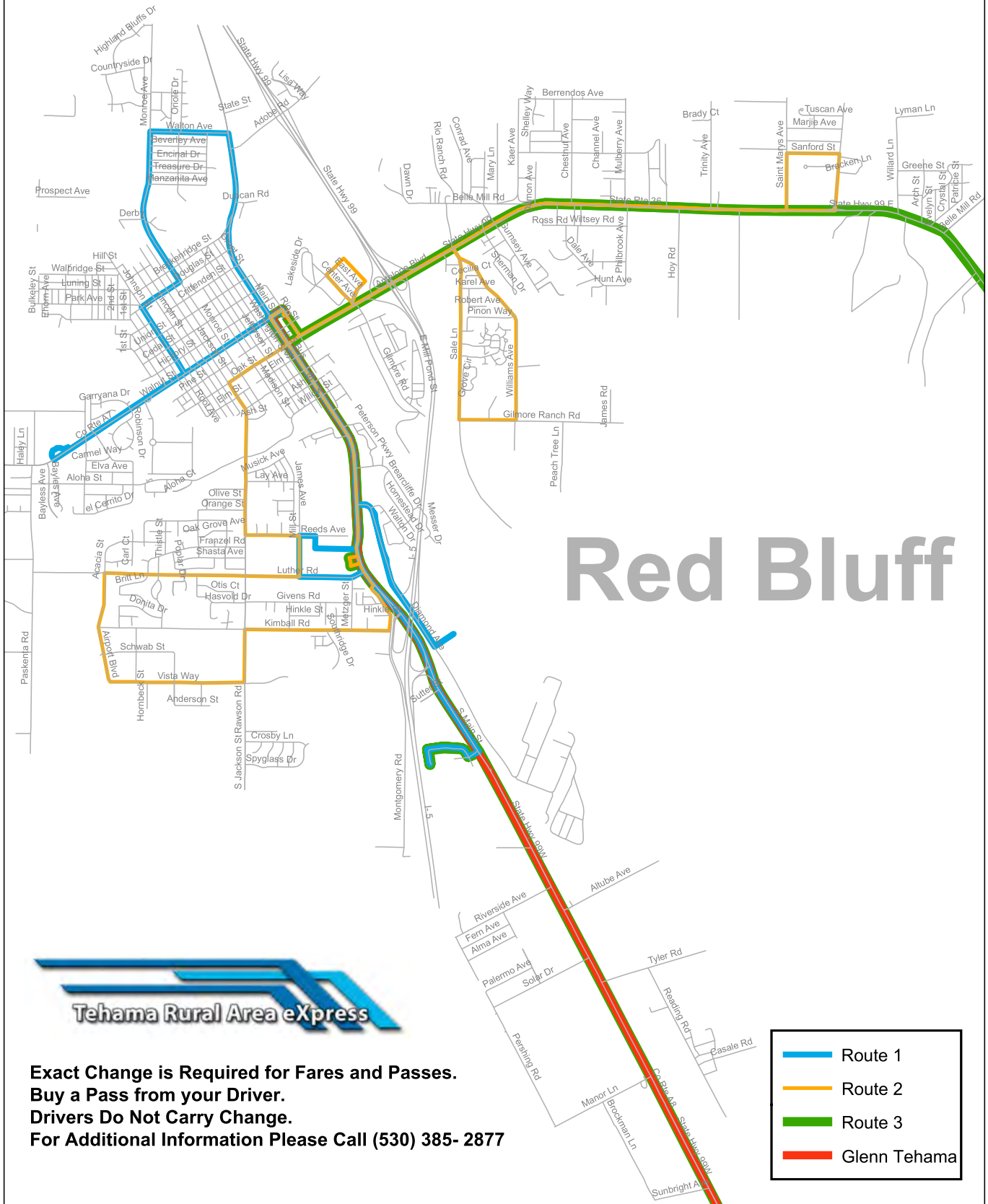


By:
City of Red Bluff
Planning &
Public Works Dept.
February 4, 2008

0 0.1 0.2 0.4 0.6 0.8
Miles



TRAX Routes



APPENDIX D

EXISTING TRAFFIC COUNTS

City of Red Bluff
N/S: Jackson Street
E/W: Aloha Street
Weather: Clear

File Name : 01_RBF_Jackson_Aloha AM
Site Code : 23622164
Start Date : 3/1/2022
Page No : 1

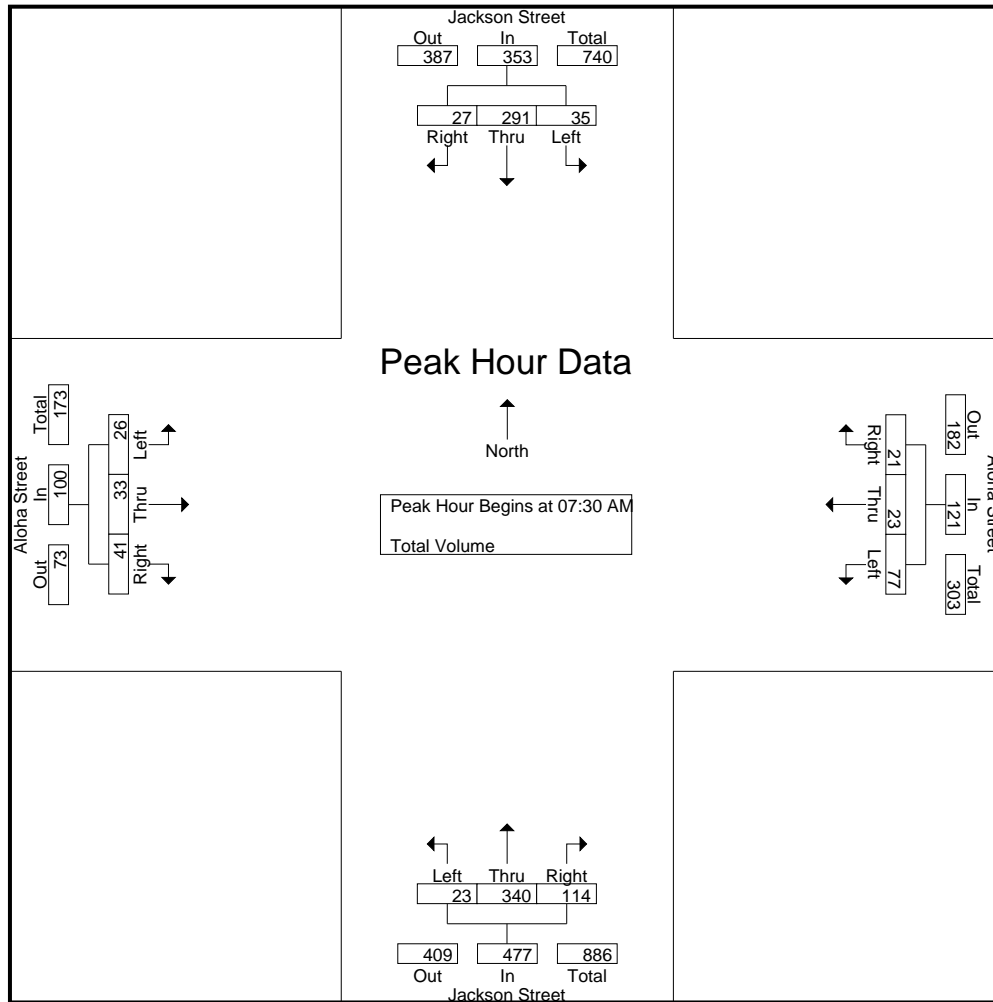
Groups Printed- Total Volume

	Jackson Street Southbound				Aloha Street Westbound				Jackson Street Northbound				Aloha Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	1	22	3	26	4	3	0	7	2	27	10	39	4	6	8	18	90
07:15 AM	4	30	1	35	11	3	0	14	6	48	13	67	2	13	4	19	135
07:30 AM	8	59	2	69	13	4	7	24	5	92	29	126	7	14	10	31	250
07:45 AM	6	92	9	107	21	9	9	39	7	104	41	152	5	12	11	28	326
Total	19	203	15	237	49	19	16	84	20	271	93	384	18	45	33	96	801
08:00 AM	14	86	11	111	25	4	3	32	5	78	21	104	9	2	10	21	268
08:15 AM	7	54	5	66	18	6	2	26	6	66	23	95	5	5	10	20	207
08:30 AM	3	48	1	52	13	4	2	19	6	42	12	60	5	5	12	22	153
08:45 AM	7	42	3	52	23	5	5	33	5	40	28	73	6	12	10	28	186
Total	31	230	20	281	79	19	12	110	22	226	84	332	25	24	42	91	814
Grand Total	50	433	35	518	128	38	28	194	42	497	177	716	43	69	75	187	1615
Apprch %	9.7	83.6	6.8		66	19.6	14.4		5.9	69.4	24.7		23	36.9	40.1		
Total %	3.1	26.8	2.2	32.1	7.9	2.4	1.7	12	2.6	30.8	11	44.3	2.7	4.3	4.6	11.6	

	Jackson Street Southbound				Aloha Street Westbound				Jackson Street Northbound				Aloha Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	8	59	2	69	13	4	7	24	5	92	29	126	7	14	10	31	250
07:45 AM	6	92	9	107	21	9	9	39	7	104	41	152	5	12	11	28	326
08:00 AM	14	86	11	111	25	4	3	32	5	78	21	104	9	2	10	21	268
08:15 AM	7	54	5	66	18	6	2	26	6	66	23	95	5	5	10	20	207
Total Volume	35	291	27	353	77	23	21	121	23	340	114	477	26	33	41	100	1051
% App. Total	9.9	82.4	7.6		63.6	19	17.4		4.8	71.3	23.9		26	33	41		
PHF	.625	.791	.614	.795	.770	.639	.583	.776	.821	.817	.695	.785	.722	.589	.932	.806	.806

City of Red Bluff
N/S: Jackson Street
E/W: Aloha Street
Weather: Clear

File Name : 01_RBF_Jackson_Aloha AM
Site Code : 23622164
Start Date : 3/1/2022
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:30 AM				07:30 AM				07:30 AM				07:30 AM			
+0 mins.	8	59	2	69	13	4	7	24	5	92	29	126	7	14	10	31
+15 mins.	6	92	9	107	21	9	9	39	7	104	41	152	5	12	11	28
+30 mins.	14	86	11	111	25	4	3	32	5	78	21	104	9	2	10	21
+45 mins.	7	54	5	66	18	6	2	26	6	66	23	95	5	5	10	20
Total Volume	35	291	27	353	77	23	21	121	23	340	114	477	26	33	41	100
% App. Total	9.9	82.4	7.6		63.6	19	17.4		4.8	71.3	23.9		26	33	41	
PHF	.625	.791	.614	.795	.770	.639	.583	.776	.821	.817	.695	.785	.722	.589	.932	.806

City of Red Bluff
N/S: Jackson Street
E/W: Aloha Street
Weather: Clear

File Name : 01_RBF_Jackson_Aloha PM
Site Code : 23622164
Start Date : 3/1/2022
Page No : 1

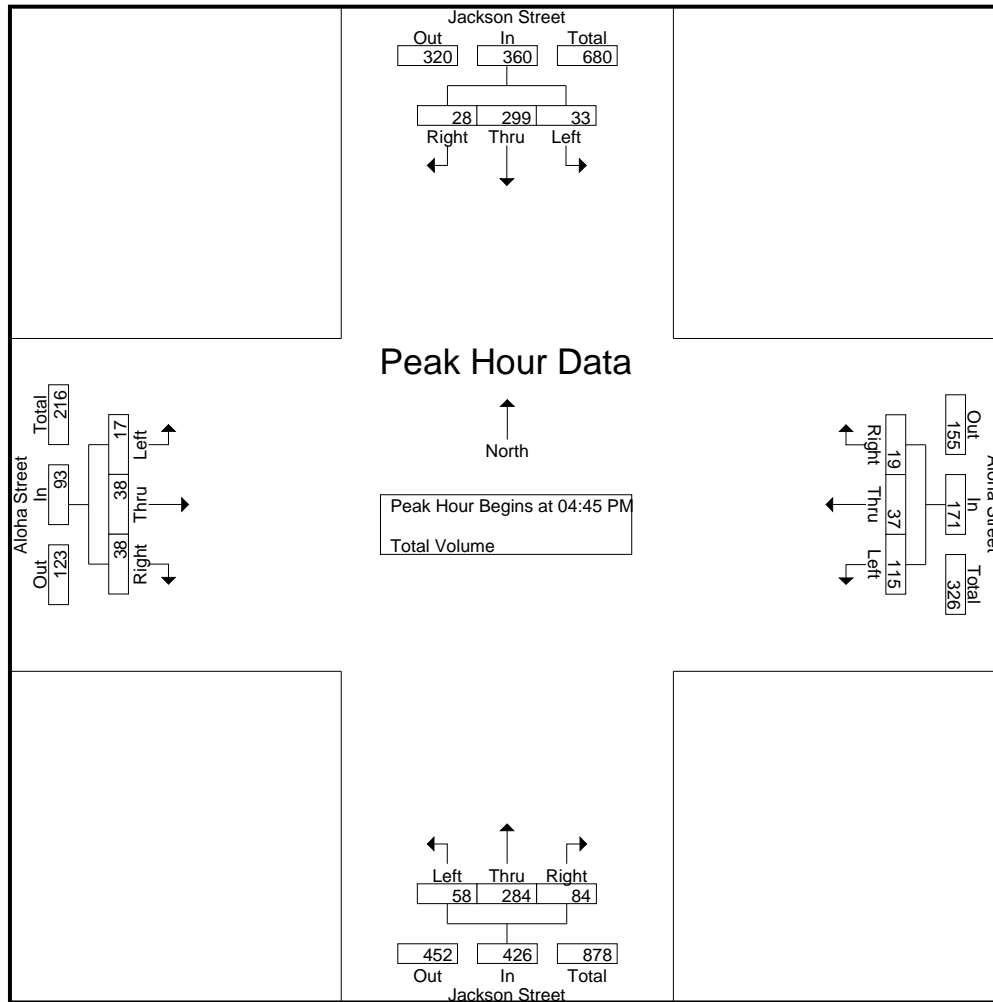
Groups Printed- Total Volume

	Jackson Street Southbound				Aloha Street Westbound				Jackson Street Northbound				Aloha Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	8	85	5	98	27	5	7	39	22	71	29	122	2	6	7	15	274
04:15 PM	10	66	10	86	20	9	4	33	15	75	25	115	3	10	9	22	256
04:30 PM	8	66	4	78	28	18	8	54	12	68	20	100	4	4	9	17	249
04:45 PM	8	62	6	76	26	5	2	33	13	75	26	114	2	10	8	20	243
Total	34	279	25	338	101	37	21	159	62	289	100	451	11	30	33	74	1022
05:00 PM	12	86	5	103	31	13	8	52	9	67	19	95	7	13	8	28	278
05:15 PM	7	63	8	78	37	9	6	52	17	74	20	111	4	6	12	22	263
05:30 PM	6	88	9	103	21	10	3	34	19	68	19	106	4	9	10	23	266
05:45 PM	4	65	7	76	27	1	5	33	10	58	22	90	3	8	10	21	220
Total	29	302	29	360	116	33	22	171	55	267	80	402	18	36	40	94	1027
Grand Total	63	581	54	698	217	70	43	330	117	556	180	853	29	66	73	168	2049
Apprch %	9	83.2	7.7		65.8	21.2	13		13.7	65.2	21.1		17.3	39.3	43.5		
Total %	3.1	28.4	2.6	34.1	10.6	3.4	2.1	16.1	5.7	27.1	8.8	41.6	1.4	3.2	3.6	8.2	

	Jackson Street Southbound				Aloha Street Westbound				Jackson Street Northbound				Aloha Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	8	62	6	76	26	5	2	33	13	75	26	114	2	10	8	20	243
05:00 PM	12	86	5	103	31	13	8	52	9	67	19	95	7	13	8	28	278
05:15 PM	7	63	8	78	37	9	6	52	17	74	20	111	4	6	12	22	263
05:30 PM	6	88	9	103	21	10	3	34	19	68	19	106	4	9	10	23	266
Total Volume	33	299	28	360	115	37	19	171	58	284	84	426	17	38	38	93	1050
% App. Total	9.2	83.1	7.8		67.3	21.6	11.1		13.6	66.7	19.7		18.3	40.9	40.9		
PHF	.688	.849	.778	.874	.777	.712	.594	.822	.763	.947	.808	.934	.607	.731	.792	.830	.944

City of Red Bluff
N/S: Jackson Street
E/W: Aloha Street
Weather: Clear

File Name : 01_RBF_Jackson_Aloha PM
Site Code : 23622164
Start Date : 3/1/2022
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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:45 PM				04:30 PM				04:00 PM				05:00 PM			
+0 mins.	8	62	6	76	28	18	8	54	22	71	29	122	7	13	8	28
+15 mins.	12	86	5	103	26	5	2	33	15	75	25	115	4	6	12	22
+30 mins.	7	63	8	78	31	13	8	52	12	68	20	100	4	9	10	23
+45 mins.	6	88	9	103	37	9	6	52	13	75	26	114	3	8	10	21
Total Volume	33	299	28	360	122	45	24	191	62	289	100	451	18	36	40	94
% App. Total	9.2	83.1	7.8		63.9	23.6	12.6		13.7	64.1	22.2		19.1	38.3	42.6	
PHF	.688	.849	.778	.874	.824	.625	.750	.884	.705	.963	.862	.924	.643	.692	.833	.839

City of Red Bluff
N/S: Jackson Street
E/W: Lay Avenue
Weather: Clear

File Name : 02_RBF_Jackson_Lay AM
Site Code : 23622164
Start Date : 3/1/2022
Page No : 1

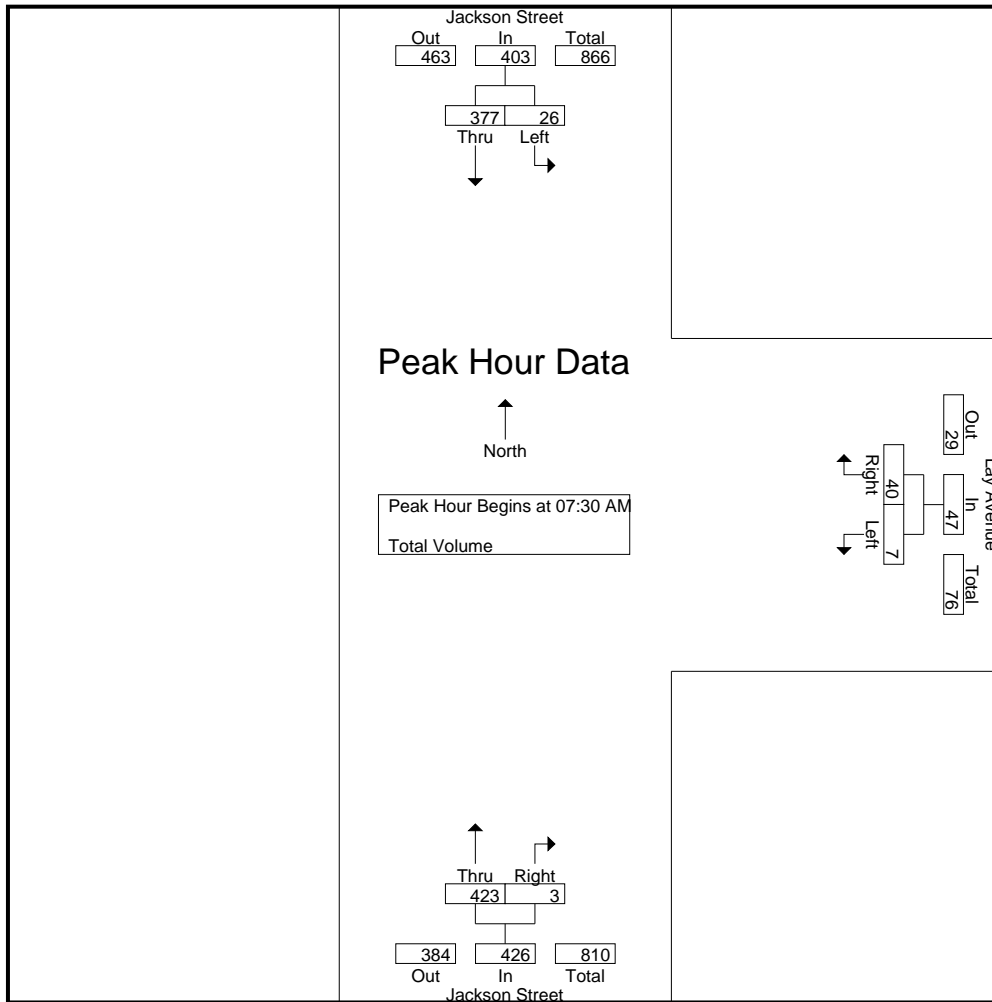
Groups Printed- Total Volume

	Jackson Street Southbound			Lay Avenue Westbound			Jackson Street Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
07:00 AM	2	29	31	1	3	4	36	0	36	71
07:15 AM	1	44	45	0	10	10	56	0	56	111
07:30 AM	4	73	77	1	8	9	124	0	124	210
07:45 AM	6	117	123	3	19	22	120	1	121	266
Total	13	263	276	5	40	45	336	1	337	658
08:00 AM	10	108	118	3	10	13	96	1	97	228
08:15 AM	6	79	85	0	3	3	83	1	84	172
08:30 AM	3	67	70	0	3	3	60	0	60	133
08:45 AM	3	67	70	2	8	10	50	0	50	130
Total	22	321	343	5	24	29	289	2	291	663
Grand Total	35	584	619	10	64	74	625	3	628	1321
Apprch %	5.7	94.3		13.5	86.5		99.5	0.5		
Total %	2.6	44.2	46.9	0.8	4.8	5.6	47.3	0.2	47.5	

	Jackson Street Southbound			Lay Avenue Westbound			Jackson Street Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:30 AM										
07:30 AM	4	73	77	1	8	9	124	0	124	210
07:45 AM	6	117	123	3	19	22	120	1	121	266
08:00 AM	10	108	118	3	10	13	96	1	97	228
08:15 AM	6	79	85	0	3	3	83	1	84	172
Total Volume	26	377	403	7	40	47	423	3	426	876
% App. Total	6.5	93.5		14.9	85.1		99.3	0.7		
PHF	.650	.806	.819	.583	.526	.534	.853	.750	.859	.823

City of Red Bluff
N/S: Jackson Street
E/W: Lay Avenue
Weather: Clear

File Name : 02_RBF_Jackson_Lay AM
Site Code : 23622164
Start Date : 3/1/2022
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:30 AM			07:15 AM			07:30 AM		
+0 mins.	4	73	77	0	10	10	124	0	124
+15 mins.	6	117	123	1	8	9	120	1	121
+30 mins.	10	108	118	3	19	22	96	1	97
+45 mins.	6	79	85	3	10	13	83	1	84
Total Volume	26	377	403	7	47	54	423	3	426
% App. Total	6.5	93.5		13	87		99.3	0.7	
PHF	.650	.806	.819	.583	.618	.614	.853	.750	.859

City of Red Bluff
N/S: Jackson Street
E/W: Lay Avenue
Weather: Clear

File Name : 02_RBF_Jackson_Lay PM
Site Code : 23622164
Start Date : 3/1/2022
Page No : 1

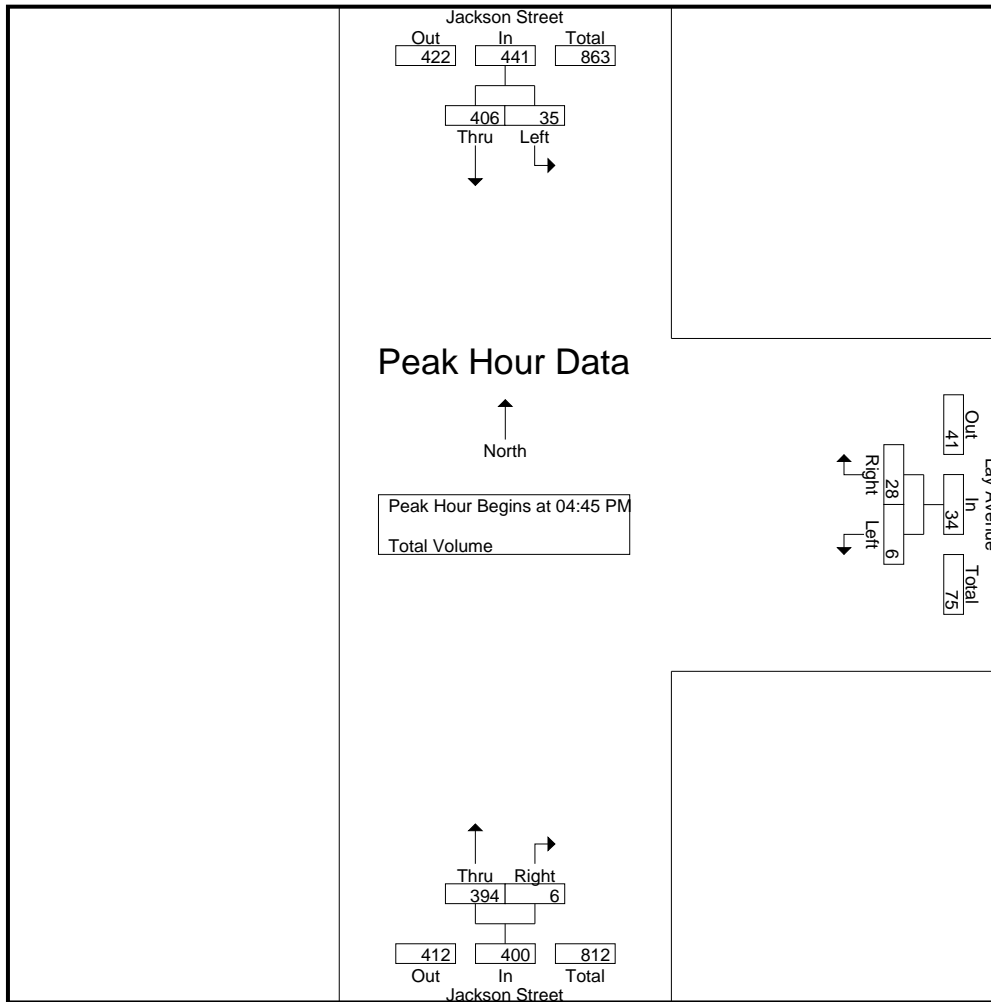
Groups Printed- Total Volume

	Jackson Street Southbound			Lay Avenue Westbound			Jackson Street Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
04:00 PM	5	113	118	2	5	7	107	1	108	233
04:15 PM	6	85	91	1	12	13	105	1	106	210
04:30 PM	10	85	95	4	7	11	88	3	91	197
04:45 PM	8	89	97	2	9	11	103	2	105	213
Total	29	372	401	9	33	42	403	7	410	853
05:00 PM	6	116	122	2	9	11	89	2	91	224
05:15 PM	10	102	112	1	3	4	104	1	105	221
05:30 PM	11	99	110	1	7	8	98	1	99	217
05:45 PM	8	98	106	2	6	8	78	2	80	194
Total	35	415	450	6	25	31	369	6	375	856
Grand Total	64	787	851	15	58	73	772	13	785	1709
Apprch %	7.5	92.5		20.5	79.5		98.3	1.7		
Total %	3.7	46.1	49.8	0.9	3.4	4.3	45.2	0.8	45.9	

	Jackson Street Southbound			Lay Avenue Westbound			Jackson Street Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 04:45 PM										
04:45 PM	8	89	97	2	9	11	103	2	105	213
05:00 PM	6	116	122	2	9	11	89	2	91	224
05:15 PM	10	102	112	1	3	4	104	1	105	221
05:30 PM	11	99	110	1	7	8	98	1	99	217
Total Volume	35	406	441	6	28	34	394	6	400	875
% App. Total	7.9	92.1		17.6	82.4		98.5	1.5		
PHF	.795	.875	.904	.750	.778	.773	.947	.750	.952	.977

City of Red Bluff
N/S: Jackson Street
E/W: Lay Avenue
Weather: Clear

File Name : 02_RBF_Jackson_Lay PM
Site Code : 23622164
Start Date : 3/1/2022
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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	05:00 PM			04:15 PM			04:00 PM		
+0 mins.	6	116	122	1	12	13	107	1	108
+15 mins.	10	102	112	4	7	11	105	1	106
+30 mins.	11	99	110	2	9	11	88	3	91
+45 mins.	8	98	106	2	9	11	103	2	105
Total Volume	35	415	450	9	37	46	403	7	410
% App. Total	7.8	92.2		19.6	80.4		98.3	1.7	
PHF	.795	.894	.922	.563	.771	.885	.942	.583	.949

City of Red Bluff
N/S: Jackson Street
E/W: Reeds Avenue
Weather: Clear

File Name : 03_RBF_Jackson_Reeds AM
Site Code : 23622164
Start Date : 3/1/2022
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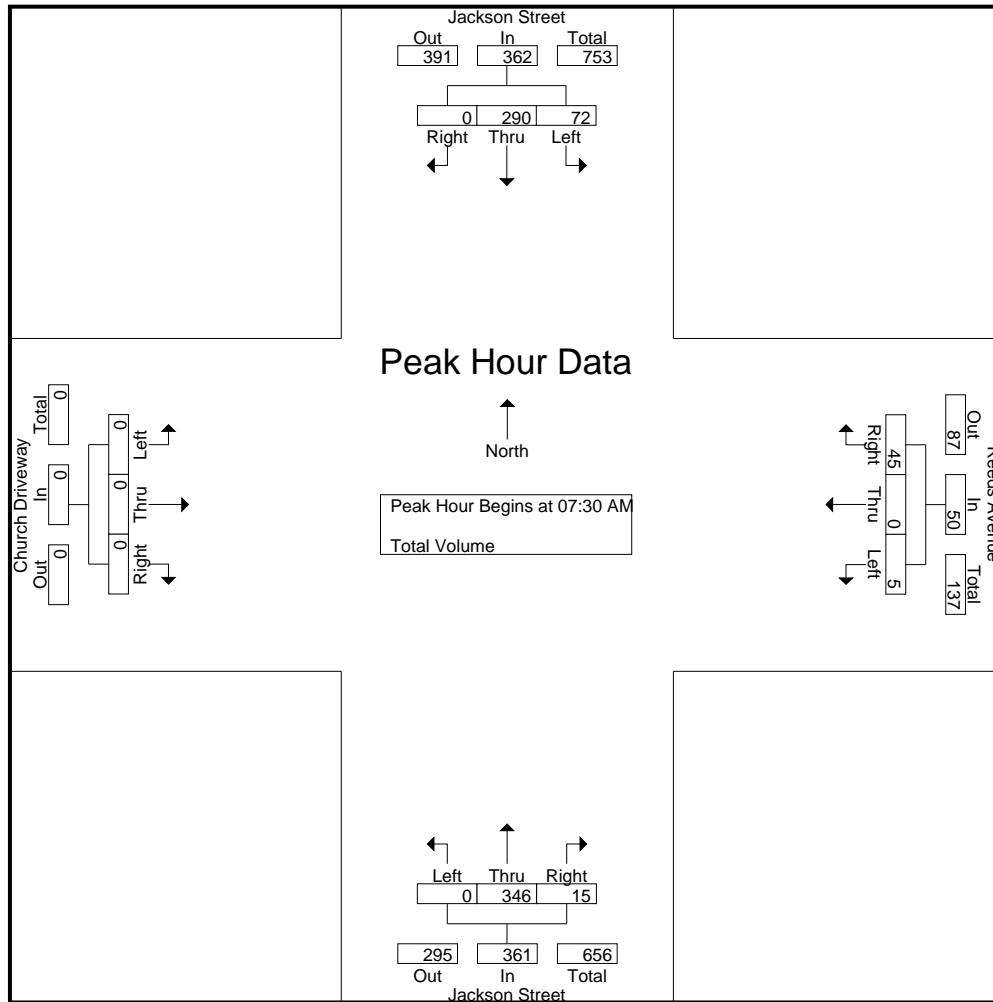
Groups Printed- Total Volume

	Jackson Street Southbound				Reeds Avenue Westbound				Jackson Street Northbound				Church Driveway Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	3	28	0	31	1	0	9	10	0	29	0	29	0	0	0	0	70
07:15 AM	7	39	0	46	0	0	8	8	0	48	1	49	0	0	0	0	103
07:30 AM	10	64	0	74	3	0	19	22	0	90	2	92	0	0	0	0	188
07:45 AM	24	87	0	111	1	0	15	16	0	96	5	101	0	0	0	0	228
Total	44	218	0	262	5	0	51	56	0	263	8	271	0	0	0	0	589
08:00 AM	16	84	0	100	1	0	4	5	0	84	5	89	0	0	0	0	194
08:15 AM	22	55	0	77	0	0	7	7	0	76	3	79	0	0	0	0	163
08:30 AM	16	49	0	65	5	0	12	17	0	47	7	54	0	0	0	0	136
08:45 AM	17	56	0	73	2	0	15	17	0	57	4	61	0	0	0	0	151
Total	71	244	0	315	8	0	38	46	0	264	19	283	0	0	0	0	644
Grand Total	115	462	0	577	13	0	89	102	0	527	27	554	0	0	0	0	1233
Apprch %	19.9	80.1	0		12.7	0	87.3		0	95.1	4.9		0	0	0		
Total %	9.3	37.5	0	46.8	1.1	0	7.2	8.3	0	42.7	2.2	44.9	0	0	0	0	

	Jackson Street Southbound				Reeds Avenue Westbound				Jackson Street Northbound				Church Driveway Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	10	64	0	74	3	0	19	22	0	90	2	92	0	0	0	0	188
07:45 AM	24	87	0	111	1	0	15	16	0	96	5	101	0	0	0	0	228
08:00 AM	16	84	0	100	1	0	4	5	0	84	5	89	0	0	0	0	194
08:15 AM	22	55	0	77	0	0	7	7	0	76	3	79	0	0	0	0	163
Total Volume	72	290	0	362	5	0	45	50	0	346	15	361	0	0	0	0	773
% App. Total	19.9	80.1	0		10	0	90		0	95.8	4.2		0	0	0		
PHF	.750	.833	.000	.815	.417	.000	.592	.568	.000	.901	.750	.894	.000	.000	.000	.000	.848

City of Red Bluff
N/S: Jackson Street
E/W: Reeds Avenue
Weather: Clear

File Name : 03_RBF_Jackson_Reeds AM
Site Code : 23622164
Start Date : 3/1/2022
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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:30 AM				07:00 AM				07:30 AM				07:00 AM			
+0 mins.	10	64	0	74	1	0	9	10	0	90	2	92	0	0	0	0
+15 mins.	24	87	0	111	0	0	8	8	0	96	5	101	0	0	0	0
+30 mins.	16	84	0	100	3	0	19	22	0	84	5	89	0	0	0	0
+45 mins.	22	55	0	77	1	0	15	16	0	76	3	79	0	0	0	0
Total Volume	72	290	0	362	5	0	51	56	0	346	15	361	0	0	0	0
% App. Total	19.9	80.1	0		8.9	0	91.1		0	95.8	4.2		0	0	0	
PHF	.750	.833	.000	.815	.417	.000	.671	.636	.000	.901	.750	.894	.000	.000	.000	.000

City of Red Bluff
N/S: Jackson Street
E/W: Reeds Avenue
Weather: Clear

File Name : 03_RBF_Jackson_Reeds PM
Site Code : 23622164
Start Date : 3/1/2022
Page No : 1

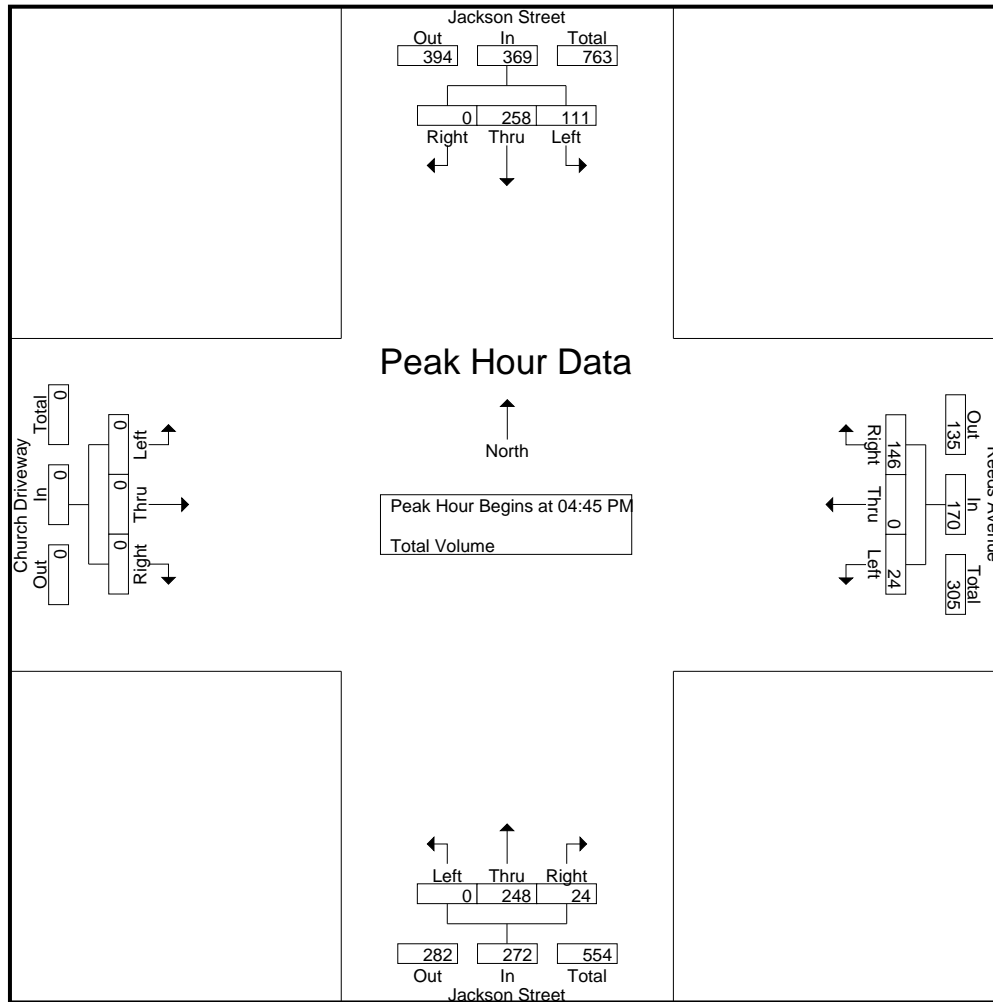
Groups Printed- Total Volume

	Jackson Street Southbound				Reeds Avenue Westbound				Jackson Street Northbound				Church Driveway Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	23	85	0	108	4	0	34	38	0	64	5	69	0	0	0	0	215
04:15 PM	22	55	0	77	6	0	35	41	0	72	6	78	0	0	0	0	196
04:30 PM	30	51	0	81	9	0	31	40	0	59	2	61	0	0	0	0	182
04:45 PM	20	59	0	79	8	0	32	40	0	62	3	65	0	0	0	0	184
Total	95	250	0	345	27	0	132	159	0	257	16	273	0	0	0	0	777
05:00 PM	33	74	0	107	8	0	31	39	0	52	10	62	0	0	0	0	208
05:15 PM	34	58	0	92	3	0	45	48	0	68	5	73	0	0	0	0	213
05:30 PM	24	67	0	91	5	0	38	43	0	66	6	72	0	0	0	0	206
05:45 PM	28	73	0	101	4	0	27	31	0	49	2	51	0	0	0	0	183
Total	119	272	0	391	20	0	141	161	0	235	23	258	0	0	0	0	810
Grand Total	214	522	0	736	47	0	273	320	0	492	39	531	0	0	0	0	1587
Apprch %	29.1	70.9	0		14.7	0	85.3		0	92.7	7.3		0	0	0		
Total %	13.5	32.9	0	46.4	3	0	17.2	20.2	0	31	2.5	33.5	0	0	0	0	

	Jackson Street Southbound				Reeds Avenue Westbound				Jackson Street Northbound				Church Driveway Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	20	59	0	79	8	0	32	40	0	62	3	65	0	0	0	0	184
05:00 PM	33	74	0	107	8	0	31	39	0	52	10	62	0	0	0	0	208
05:15 PM	34	58	0	92	3	0	45	48	0	68	5	73	0	0	0	0	213
05:30 PM	24	67	0	91	5	0	38	43	0	66	6	72	0	0	0	0	206
Total Volume	111	258	0	369	24	0	146	170	0	248	24	272	0	0	0	0	811
% App. Total	30.1	69.9	0		14.1	0	85.9		0	91.2	8.8		0	0	0		
PHF	.816	.872	.000	.862	.750	.000	.811	.885	.000	.912	.600	.932	.000	.000	.000	.000	.952

City of Red Bluff
N/S: Jackson Street
E/W: Reeds Avenue
Weather: Clear

File Name : 03_RBF_Jackson_Reeds PM
Site Code : 23622164
Start Date : 3/1/2022
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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	05:00 PM				04:45 PM				04:00 PM				04:00 PM			
+0 mins.	33	74	0	107	8	0	32	40	0	64	5	69	0	0	0	0
+15 mins.	34	58	0	92	8	0	31	39	0	72	6	78	0	0	0	0
+30 mins.	24	67	0	91	3	0	45	48	0	59	2	61	0	0	0	0
+45 mins.	28	73	0	101	5	0	38	43	0	62	3	65	0	0	0	0
Total Volume	119	272	0	391	24	0	146	170	0	257	16	273	0	0	0	0
% App. Total	30.4	69.6	0		14.1	0	85.9		0	94.1	5.9		0	0	0	
PHF	.875	.919	.000	.914	.750	.000	.811	.885	.000	.892	.667	.875	.000	.000	.000	.000

City of Red Bluff
N/S: Jackson Street
E/W: Luther Road
Weather: Clear

File Name : 04_RBF_Jackson_Luther AM
Site Code : 23622164
Start Date : 3/1/2022
Page No : 1

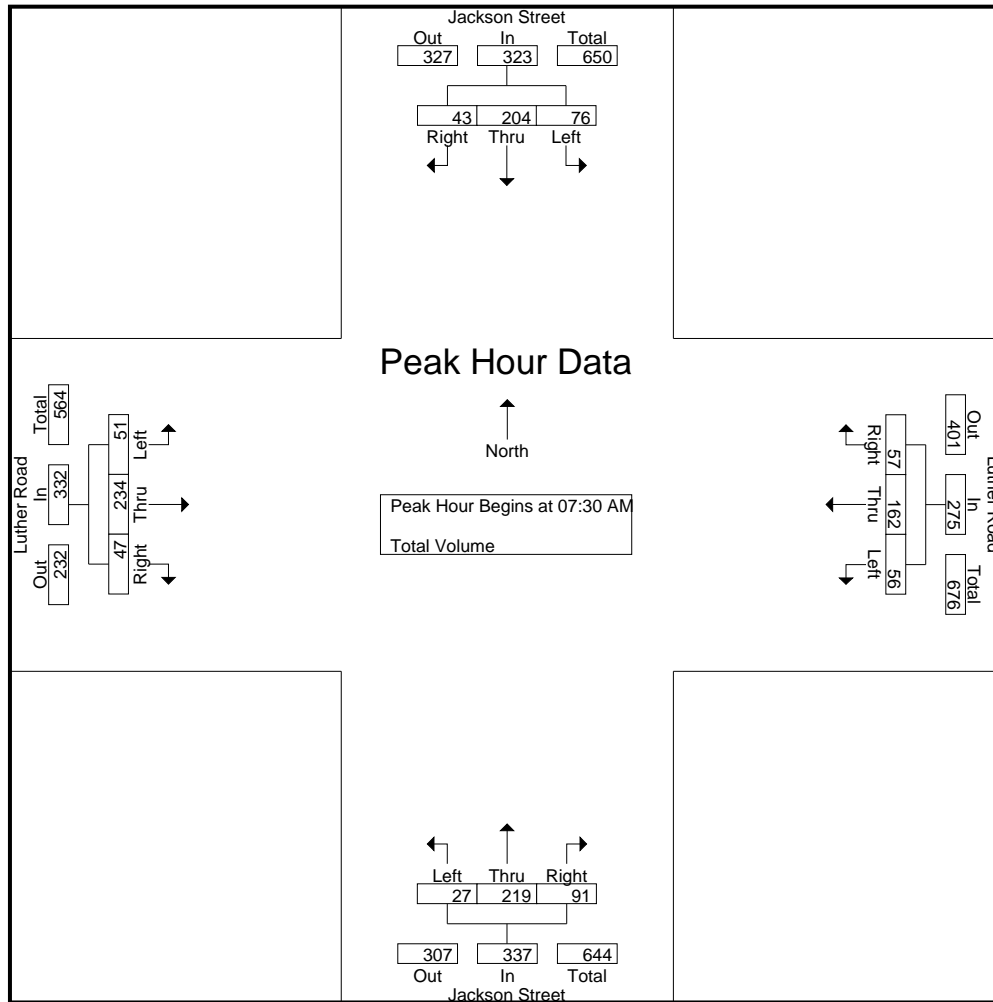
Groups Printed- Total Volume

	Jackson Street Southbound				Luther Road Westbound				Jackson Street Northbound				Luther Road Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	13	9	6	28	7	21	8	36	0	11	6	17	8	39	3	50	131
07:15 AM	15	26	4	45	7	23	5	35	2	26	10	38	10	54	0	64	182
07:30 AM	20	45	10	75	7	37	14	58	5	55	10	70	9	62	11	82	285
07:45 AM	17	68	6	91	15	53	12	80	6	54	20	80	15	60	18	93	344
Total	65	148	26	239	36	134	39	209	13	146	46	205	42	215	32	289	942
08:00 AM	15	65	13	93	20	37	14	71	9	62	34	105	14	38	12	64	333
08:15 AM	24	26	14	64	14	35	17	66	7	48	27	82	13	74	6	93	305
08:30 AM	22	20	13	55	8	28	12	48	1	24	17	42	13	50	4	67	212
08:45 AM	17	26	12	55	15	31	20	66	2	23	14	39	14	36	0	50	210
Total	78	137	52	267	57	131	63	251	19	157	92	268	54	198	22	274	1060
Grand Total	143	285	78	506	93	265	102	460	32	303	138	473	96	413	54	563	2002
Apprch %	28.3	56.3	15.4		20.2	57.6	22.2		6.8	64.1	29.2		17.1	73.4	9.6		
Total %	7.1	14.2	3.9	25.3	4.6	13.2	5.1	23	1.6	15.1	6.9	23.6	4.8	20.6	2.7	28.1	

	Jackson Street Southbound				Luther Road Westbound				Jackson Street Northbound				Luther Road Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	20	45	10	75	7	37	14	58	5	55	10	70	9	62	11	82	285
07:45 AM	17	68	6	91	15	53	12	80	6	54	20	80	15	60	18	93	344
08:00 AM	15	65	13	93	20	37	14	71	9	62	34	105	14	38	12	64	333
08:15 AM	24	26	14	64	14	35	17	66	7	48	27	82	13	74	6	93	305
Total Volume	76	204	43	323	56	162	57	275	27	219	91	337	51	234	47	332	1267
% App. Total	23.5	63.2	13.3		20.4	58.9	20.7		8	65	27		15.4	70.5	14.2		
PHF	.792	.750	.768	.868	.700	.764	.838	.859	.750	.883	.669	.802	.850	.791	.653	.892	.921

City of Red Bluff
N/S: Jackson Street
E/W: Luther Road
Weather: Clear

File Name : 04_RBF_Jackson_Luther AM
Site Code : 23622164
Start Date : 3/1/2022
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:30 AM				07:30 AM				07:30 AM				07:30 AM			
+0 mins.	20	45	10	75	7	37	14	58	5	55	10	70	9	62	11	82
+15 mins.	17	68	6	91	15	53	12	80	6	54	20	80	15	60	18	93
+30 mins.	15	65	13	93	20	37	14	71	9	62	34	105	14	38	12	64
+45 mins.	24	26	14	64	14	35	17	66	7	48	27	82	13	74	6	93
Total Volume	76	204	43	323	56	162	57	275	27	219	91	337	51	234	47	332
% App. Total	23.5	63.2	13.3		20.4	58.9	20.7		8	65	27		15.4	70.5	14.2	
PHF	.792	.750	.768	.868	.700	.764	.838	.859	.750	.883	.669	.802	.850	.791	.653	.892

City of Red Bluff
N/S: Jackson Street
E/W: Luther Road
Weather: Clear

File Name : 04_RBF_Jackson_Luther PM
Site Code : 23622164
Start Date : 3/1/2022
Page No : 1

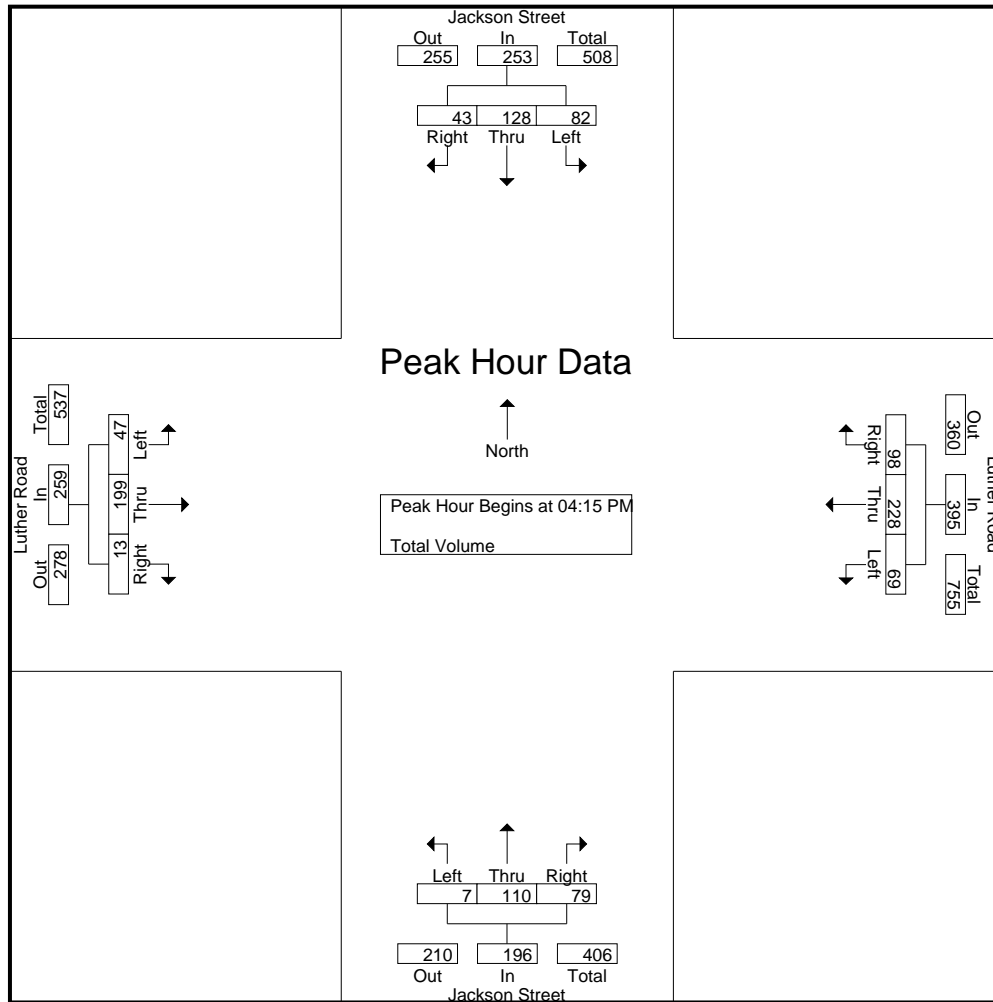
Groups Printed- Total Volume

	Jackson Street Southbound				Luther Road Westbound				Jackson Street Northbound				Luther Road Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	21	40	14	75	19	56	21	96	4	38	23	65	11	51	5	67	303
04:15 PM	17	29	15	61	16	56	29	101	0	34	18	52	8	46	2	56	270
04:30 PM	24	25	7	56	18	58	32	108	2	24	9	35	8	58	5	71	270
04:45 PM	17	33	12	62	15	46	18	79	3	28	29	60	15	35	5	55	256
Total	79	127	48	254	68	216	100	384	9	124	79	212	42	190	17	249	1099
05:00 PM	24	41	9	74	20	68	19	107	2	24	23	49	16	60	1	77	307
05:15 PM	18	18	15	51	21	57	27	105	1	44	20	65	7	21	3	31	252
05:30 PM	18	31	9	58	23	62	28	113	1	32	23	56	12	36	4	52	279
05:45 PM	23	31	14	68	20	50	18	88	0	21	20	41	4	35	5	44	241
Total	83	121	47	251	84	237	92	413	4	121	86	211	39	152	13	204	1079
Grand Total	162	248	95	505	152	453	192	797	13	245	165	423	81	342	30	453	2178
Apprch %	32.1	49.1	18.8		19.1	56.8	24.1		3.1	57.9	39		17.9	75.5	6.6		
Total %	7.4	11.4	4.4	23.2	7	20.8	8.8	36.6	0.6	11.2	7.6	19.4	3.7	15.7	1.4	20.8	

	Jackson Street Southbound				Luther Road Westbound				Jackson Street Northbound				Luther Road Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:15 PM																	
04:15 PM	17	29	15	61	16	56	29	101	0	34	18	52	8	46	2	56	270
04:30 PM	24	25	7	56	18	58	32	108	2	24	9	35	8	58	5	71	270
04:45 PM	17	33	12	62	15	46	18	79	3	28	29	60	15	35	5	55	256
05:00 PM	24	41	9	74	20	68	19	107	2	24	23	49	16	60	1	77	307
Total Volume	82	128	43	253	69	228	98	395	7	110	79	196	47	199	13	259	1103
% App. Total	32.4	50.6	17		17.5	57.7	24.8		3.6	56.1	40.3		18.1	76.8	5		
PHF	.854	.780	.717	.855	.863	.838	.766	.914	.583	.809	.681	.817	.734	.829	.650	.841	.898

City of Red Bluff
N/S: Jackson Street
E/W: Luther Road
Weather: Clear

File Name : 04_RBF_Jackson_Luther PM
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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:00 PM				05:00 PM				04:45 PM				04:15 PM			
+0 mins.	21	40	14	75	20	68	19	107	3	28	29	60	8	46	2	56
+15 mins.	17	29	15	61	21	57	27	105	2	24	23	49	8	58	5	71
+30 mins.	24	25	7	56	23	62	28	113	1	44	20	65	15	35	5	55
+45 mins.	17	33	12	62	20	50	18	88	1	32	23	56	16	60	1	77
Total Volume	79	127	48	254	84	237	92	413	7	128	95	230	47	199	13	259
% App. Total	31.1	50	18.9		20.3	57.4	22.3		3	55.7	41.3		18.1	76.8	5	
PHF	.823	.794	.800	.847	.913	.871	.821	.914	.583	.727	.819	.885	.734	.829	.650	.841

City of Red Bluff
N/S: Jackson Street
E/W: Olive Street
Weather: Clear

File Name : 05_RBF_Jackson_Olive AM
Site Code : 23622164
Start Date : 3/15/2022
Page No : 1

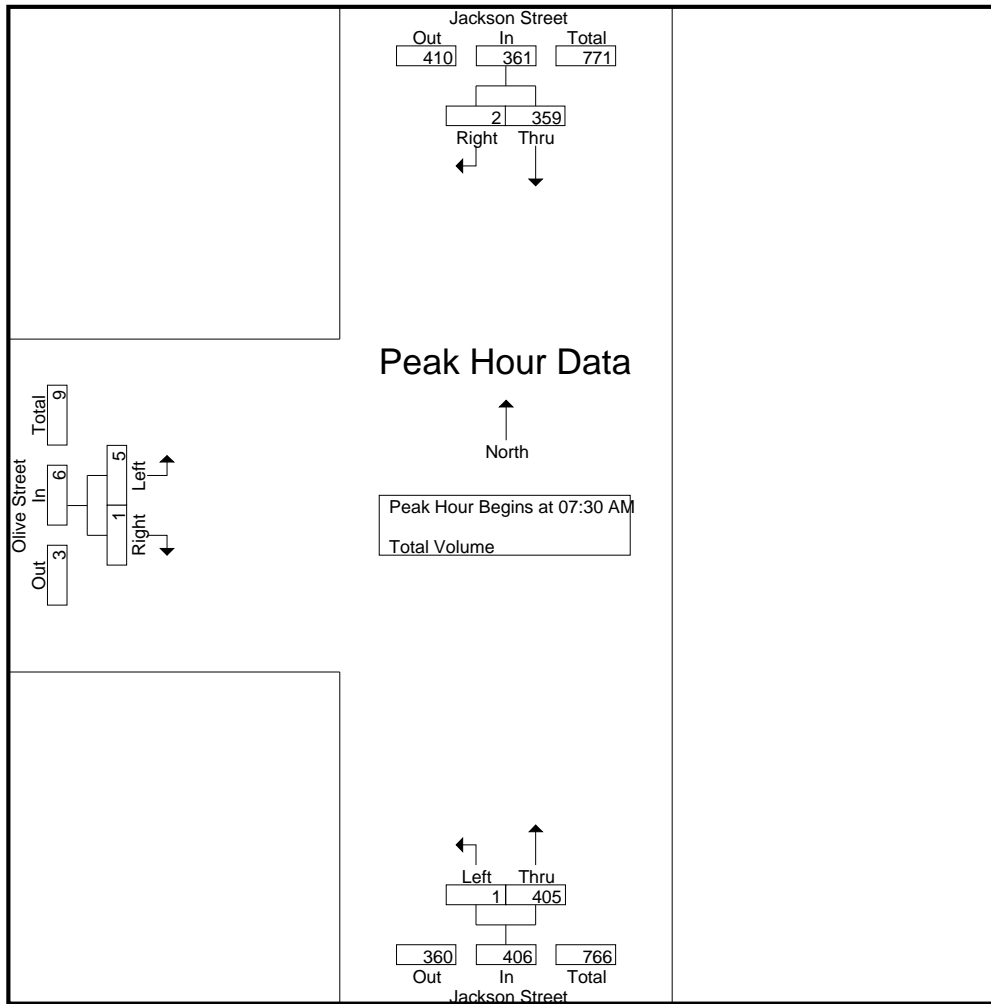
Groups Printed- Total Volume

	Jackson Street Southbound			Jackson Street Northbound			Olive Street Eastbound			
Start Time	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	Int. Total
07:00 AM	33	1	34	0	40	40	0	1	1	75
07:15 AM	53	2	55	0	71	71	0	1	1	127
07:30 AM	70	0	70	1	105	106	0	0	0	176
07:45 AM	103	1	104	0	116	116	3	0	3	223
Total	259	4	263	1	332	333	3	2	5	601
08:00 AM	110	1	111	0	104	104	2	0	2	217
08:15 AM	76	0	76	0	80	80	0	1	1	157
08:30 AM	53	1	54	1	66	67	1	1	2	123
08:45 AM	76	1	77	1	51	52	1	0	1	130
Total	315	3	318	2	301	303	4	2	6	627
Grand Total	574	7	581	3	633	636	7	4	11	1228
Apprch %	98.8	1.2		0.5	99.5		63.6	36.4		
Total %	46.7	0.6	47.3	0.2	51.5	51.8	0.6	0.3	0.9	

	Jackson Street Southbound			Jackson Street Northbound			Olive Street Eastbound			
Start Time	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:30 AM										
07:30 AM	70	0	70	1	105	106	0	0	0	176
07:45 AM	103	1	104	0	116	116	3	0	3	223
08:00 AM	110	1	111	0	104	104	2	0	2	217
08:15 AM	76	0	76	0	80	80	0	1	1	157
Total Volume	359	2	361	1	405	406	5	1	6	773
% App. Total	99.4	0.6		0.2	99.8		83.3	16.7		
PHF	.816	.500	.813	.250	.873	.875	.417	.250	.500	.867

City of Red Bluff
 N/S: Jackson Street
 E/W: Olive Street
 Weather: Clear

File Name : 05_RBF_Jackson_Olive AM
 Site Code : 23622164
 Start Date : 3/15/2022
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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

	07:30 AM			07:30 AM			07:45 AM		
+0 mins.	70	0	70	1	105	106	3	0	3
+15 mins.	103	1	104	0	116	116	2	0	2
+30 mins.	110	1	111	0	104	104	0	1	1
+45 mins.	76	0	76	0	80	80	1	1	2
Total Volume	359	2	361	1	405	406	6	2	8
% App. Total	99.4	0.6		0.2	99.8		75	25	
PHF	.816	.500	.813	.250	.873	.875	.500	.500	.667

City of Red Bluff
 N/S: Jackson Street
 E/W: Olive Street
 Weather: Clear

File Name : 05_RBF_Jackson_Olive PM
 Site Code : 23622164
 Start Date : 3/15/2022
 Page No : 1

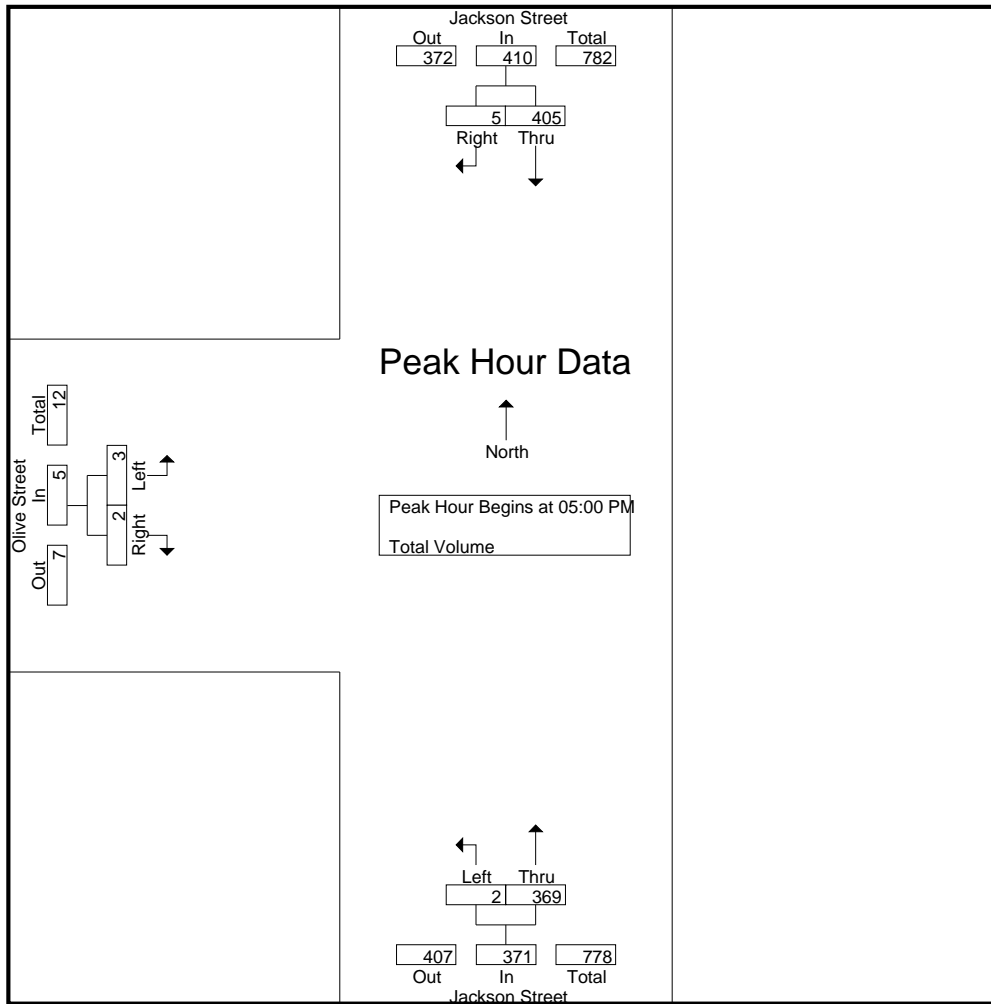
Groups Printed- Total Volume

	Jackson Street Southbound			Jackson Street Northbound			Olive Street Eastbound			
Start Time	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	Int. Total
04:00 PM	118	1	119	0	109	109	2	0	2	230
04:15 PM	92	2	94	2	101	103	2	0	2	199
04:30 PM	79	2	81	1	90	91	0	0	0	172
04:45 PM	84	1	85	1	88	89	2	0	2	176
Total	373	6	379	4	388	392	6	0	6	777
05:00 PM	105	2	107	0	84	84	1	0	1	192
05:15 PM	106	0	106	1	112	113	0	0	0	219
05:30 PM	90	1	91	0	97	97	0	1	1	189
05:45 PM	104	2	106	1	76	77	2	1	3	186
Total	405	5	410	2	369	371	3	2	5	786
Grand Total	778	11	789	6	757	763	9	2	11	1563
Apprch %	98.6	1.4		0.8	99.2		81.8	18.2		
Total %	49.8	0.7	50.5	0.4	48.4	48.8	0.6	0.1	0.7	

	Jackson Street Southbound			Jackson Street Northbound			Olive Street Eastbound			
Start Time	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 05:00 PM										
05:00 PM	105	2	107	0	84	84	1	0	1	192
05:15 PM	106	0	106	1	112	113	0	0	0	219
05:30 PM	90	1	91	0	97	97	0	1	1	189
05:45 PM	104	2	106	1	76	77	2	1	3	186
Total Volume	405	5	410	2	369	371	3	2	5	786
% App. Total	98.8	1.2		0.5	99.5		60	40		
PHF	.955	.625	.958	.500	.824	.821	.375	.500	.417	.897

City of Red Bluff
 N/S: Jackson Street
 E/W: Olive Street
 Weather: Clear

File Name : 05_RBF_Jackson_Olive PM
 Site Code : 23622164
 Start Date : 3/15/2022
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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

	05:00 PM			04:00 PM			04:00 PM		
+0 mins.	105	2	107	0	109	109	2	0	2
+15 mins.	106	0	106	2	101	103	2	0	2
+30 mins.	90	1	91	1	90	91	0	0	0
+45 mins.	104	2	106	1	88	89	2	0	2
Total Volume	405	5	410	4	388	392	6	0	6
% App. Total	98.8	1.2		1	99		100	0	
PHF	.955	.625	.958	.500	.890	.899	.750	.000	.750

Counts Unlimited, Inc.

City of Red Bluff
Jackson Street
B/ Lay Avenue - Reeds Avenue
24 Hour Directional Volume Count

PO Box 1178
Corona, CA 92878
Phone: (951) 268-6268
email: counts@countsunlimited.com

RBFJALARE
Site Code: 236-22164

Start Time	3/1/2022 Tue	Northbound		Hour Totals		Southbound		Hour Totals		Combined Totals	
		Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
12:00		13	117			6	98				
12:15		2	90			8	84				
12:30		2	91			4	93				
12:45		4	102	21	400	3	82	21	357	42	757
01:00		1	76			3	68				
01:15		6	79			5	79				
01:30		4	64			1	74				
01:45		2	91	13	310	7	74	16	295	29	605
02:00		6	80			1	84				
02:15		4	80			3	102				
02:30		3	110			2	107				
02:45		2	91	15	361	10	91	16	384	31	745
03:00		3	142			1	116				
03:15		5	104			3	116				
03:30		4	90			3	121				
03:45		2	106	14	442	10	91	17	444	31	886
04:00		2	94			11	110				
04:15		5	110			8	77				
04:30		12	94			15	81				
04:45		7	105	26	403	24	80	58	348	84	751
05:00		12	82			10	108				
05:15		12	114			16	96				
05:30		8	104			16	94				
05:45		11	76	43	376	35	102	77	400	120	776
06:00		13	77			28	70				
06:15		21	72			27	66				
06:30		28	66			30	63				
06:45		35	58	97	273	46	62	131	261	228	534
07:00		38	41			35	63				
07:15		58	48			48	50				
07:30		109	36			76	40				
07:45		106	44	311	169	113	38	272	191	583	360
08:00		88	45			104	26				
08:15		82	23			81	29				
08:30		58	29			64	26				
08:45		74	22	302	119	75	26	324	107	626	226
09:00		64	24			55	31				
09:15		64	24			54	24				
09:30		53	18			65	27				
09:45		78	24	259	90	80	25	254	107	513	197
10:00		72	21			67	18				
10:15		78	18			80	19				
10:30		90	8			72	13				
10:45		74	15	314	62	68	8	287	58	601	120
11:00		84	13			70	10				
11:15		80	8			80	10				
11:30		72	11			71	10				
11:45		102	8	338	40	104	8	325	38	663	78
Total		1753	3045	1753	3045	1798	2990	1798	2990	3551	6035
Combined Total		4798		4798		4788		4788		9586	
AM Peak	-	07:30	-	-	-	07:30	-	-	-	-	-
Vol.	-	385	-	-	-	374	-	-	-	-	-
P.H.F.		0.883				0.827					
PM Peak	-	-	02:30	-	-	-	02:45	-	-	-	-
Vol.	-	-	447	-	-	-	444	-	-	-	-
P.H.F.			0.787				0.917				
Percentage		36.5%	63.5%			37.6%	62.4%				
ADT/AADT		ADT 9,586		AADT 9,586							

APPENDIX E

INTERSECTION ANALYSIS WORKSHEETS

EXISTING (2022) CONDITIONS





Intersection Level Of Service Report

Intersection 1: Jackson St/Aloha St

Control Type: All-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 19.0
 Level Of Service: C
 Volume to Capacity (v/c): 0.764

Intersection Setup

Name	Jackson Street			Jackson Street			Aloha Street			Aloha Street		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	1	0	0	1	0	0	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	120.00	100.00	100.00	75.00	100.00	100.00	70.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00			35.00			25.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name	Jackson Street			Jackson Street			Aloha Street			Aloha Street		
Base Volume Input [veh/h]	23	340	114	35	291	27	26	33	41	77	23	21
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	23	340	114	35	291	27	26	33	41	77	23	21
Peak Hour Factor	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	7	105	35	11	90	8	8	10	13	24	7	7
Total Analysis Volume [veh/h]	29	422	141	43	361	33	32	41	51	96	29	26
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings**Lanes**

Capacity per Entry Lane [veh/h]	591	672	535	583	454	513	459	514
Degree of Utilization, x	0.76	0.21	0.08	0.68	0.07	0.18	0.21	0.11

Movement, Approach, & Intersection Results

95th-Percentile Queue Length [veh]	6.94	0.79	0.26	5.13	0.23	0.65	0.78	0.36
95th-Percentile Queue Length [ft]	173.40	19.70	6.53	128.29	5.66	16.21	19.52	8.94
Approach Delay [s/veh]	21.90		19.70		11.25		11.86	
Approach LOS	C		C		B		B	
Intersection Delay [s/veh]	18.98							
Intersection LOS	C							

Intersection Level Of Service Report**Intersection 2: Jackson St/Lay Ave**

Control Type: Two-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 15.2
 Level Of Service: C
 Volume to Capacity (v/c): 0.023

Intersection Setup

Name	Jackson Street		Jackson Street		Lay Avenue	
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Lay Avenue	
Base Volume Input [veh/h]	423	3	26	377	7	40
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	423	3	26	377	7	40
Peak Hour Factor	0.8230	0.8230	0.8230	0.8230	0.8230	0.8230
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	128	1	8	115	2	12
Total Analysis Volume [veh/h]	514	4	32	458	9	49
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			Yes
Number of Storage Spaces in Median	0	0	1

Movement, Approach, & Intersection Results



V/C, Movement V/C Ratio	0.01	0.00	0.03	0.00	0.02	0.09
d_M, Delay for Movement [s/veh]	0.00	0.00	8.51	0.00	15.22	12.24
Movement LOS	A	A	A	A	C	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.09	0.00	0.37	0.37
95th-Percentile Queue Length [ft/ln]	0.00	0.00	2.34	0.00	9.25	9.25
d_A, Approach Delay [s/veh]	0.00		0.56		12.70	
Approach LOS	A		A		B	
d_I, Intersection Delay [s/veh]	0.95					
Intersection LOS	C					

Intersection Level Of Service Report
Intersection 3: Jackson St/Reeds Ave

Control Type: Two-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 18.5
 Level Of Service: C
 Volume to Capacity (v/c): 0.021

Intersection Setup

Name	Jackson Street		Jackson Street		Reeds Avenue	
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	1	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Reeds Avenue	
Base Volume Input [veh/h]	346	15	72	290	5	45
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	346	15	72	290	5	45
Peak Hour Factor	0.8480	0.8480	0.8480	0.8480	0.8480	0.8480
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	102	4	21	85	1	13
Total Analysis Volume [veh/h]	408	18	85	342	6	53
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.07	0.00	0.02	0.08
d_M, Delay for Movement [s/veh]	0.00	0.00	8.40	0.00	18.55	11.28
Movement LOS	A	A	A	A	C	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.24	0.00	0.34	0.34
95th-Percentile Queue Length [ft/ln]	0.00	0.00	6.01	0.00	8.59	8.59
d_A, Approach Delay [s/veh]	0.00		1.67		12.02	
Approach LOS	A		A		B	
d_I, Intersection Delay [s/veh]	1.56					
Intersection LOS	C					





Intersection Level Of Service Report

Intersection 4: Jackson St/Luther Rd

Control Type: Signalized
 Analysis Method: ICU 1
 Analysis Period: 15 minutes

Delay (sec / veh): -
 Level Of Service: C
 Volume to Capacity (v/c): 0.743

Intersection Setup

Name	Jackson Street			Jackson Street			Luther Road			Luther Road		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	0	0	1	0	0	0
Entry Pocket Length [ft]	105.00	100.00	100.00	85.00	100.00	100.00	100.00	100.00	250.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00			35.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name	Jackson Street			Jackson Street			Luther Road			Luther Road		
Base Volume Input [veh/h]	27	219	91	76	204	43	51	234	47	56	162	57
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	27	219	91	76	204	43	51	234	47	56	162	57
Peak Hour Factor	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	7	59	25	21	55	12	14	64	13	15	44	15
Total Analysis Volume [veh/h]	29	238	99	83	221	47	55	254	51	61	176	62
Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	0			0			0			0		

Intersection Settings

Cycle Length [s]	100
Lost time [s]	10.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Split	Split	Split	Split	Split	Split
Signal Group	1	6	0	5	2	0	0	8	0	0	4	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	-	-	-	-	-	-

Movement, Approach, & Intersection Results




V/C, Movement V/C Ratio	0.02	0.21	0.21	0.05	0.17	0.17	0.03	0.19	0.03	0.04	0.19	0.19
Intersection LOS	C											
Intersection V/C	0.743											

Intersection Level Of Service Report
Intersection 5: Jackson St/Olive St

Control Type: Two-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 16.5
 Level Of Service: C
 Volume to Capacity (v/c): 0.016

Intersection Setup

Name	Jackson Street		Jackson Street		Olive Street	
Approach	Northbound		Southbound		Eastbound	
Lane Configuration						
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Olive Street	
Base Volume Input [veh/h]	1	405	359	2	4	1
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	1	405	359	2	4	1
Peak Hour Factor	0.8670	0.8670	0.8670	0.8670	0.8670	0.8670
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	117	104	1	1	0
Total Analysis Volume [veh/h]	1	467	414	2	5	1
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.00	0.02	0.00
d_M, Delay for Movement [s/veh]	8.12	0.00	0.00	0.00	16.48	10.78
Movement LOS	A	A	A	A	C	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.00	0.00	0.05	0.05
95th-Percentile Queue Length [ft/ln]	0.07	0.07	0.00	0.00	1.32	1.32
d_A, Approach Delay [s/veh]	0.02		0.00		15.53	
Approach LOS	A		A		C	
d_I, Intersection Delay [s/veh]	0.11					
Intersection LOS	C					





Intersection Level Of Service Report

Intersection 1: Jackson St/Aloha St

Control Type: All-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 14.6
 Level Of Service: B
 Volume to Capacity (v/c): 0.605

Intersection Setup

Name	Jackson Street			Jackson Street			Aloha Street			Aloha Street		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	1	0	0	1	0	0	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	120.00	100.00	100.00	75.00	100.00	100.00	70.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00			35.00			25.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name	Jackson Street			Jackson Street			Aloha Street			Aloha Street		
Base Volume Input [veh/h]	58	284	84	33	299	28	17	38	38	115	37	19
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	58	284	84	33	299	28	17	38	38	115	37	19
Peak Hour Factor	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	15	75	22	9	79	7	5	10	10	30	10	5
Total Analysis Volume [veh/h]	61	301	89	35	317	30	18	40	40	122	39	20
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings**Lanes**

Capacity per Entry Lane [veh/h]	598	688	552	605	478	539	490	545
Degree of Utilization, x	0.61	0.13	0.06	0.57	0.04	0.15	0.25	0.11

Movement, Approach, & Intersection Results

95th-Percentile Queue Length [veh]	4.05	0.44	0.20	3.63	0.12	0.52	0.97	0.36
95th-Percentile Queue Length [ft]	101.20	11.08	5.05	90.69	2.93	12.95	24.36	9.05
Approach Delay [s/veh]	15.79		15.73		10.53		11.70	
Approach LOS	C		C		B		B	
Intersection Delay [s/veh]	14.64							
Intersection LOS	B							



Intersection Level Of Service Report

Intersection 2: Jackson St/Lay Ave

Control Type: Two-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 13.8
 Level Of Service: B
 Volume to Capacity (v/c): 0.014

Intersection Setup

Name	Jackson Street		Jackson Street		Lay Avenue	
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Lay Avenue	
Base Volume Input [veh/h]	394	6	35	406	6	28
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	394	6	35	406	6	28
Peak Hour Factor	0.9770	0.9770	0.9770	0.9770	0.9770	0.9770
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	101	2	9	104	2	7
Total Analysis Volume [veh/h]	403	6	36	416	6	29
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			Yes
Number of Storage Spaces in Median	0	0	1

Movement, Approach, & Intersection Results




V/C, Movement V/C Ratio	0.00	0.00	0.03	0.00	0.01	0.04
d_M, Delay for Movement [s/veh]	0.00	0.00	8.20	0.00	13.77	10.92
Movement LOS	A	A	A	A	B	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.10	0.00	0.19	0.19
95th-Percentile Queue Length [ft/ln]	0.00	0.00	2.40	0.00	4.66	4.66
d_A, Approach Delay [s/veh]	0.00		0.65		11.41	
Approach LOS	A		A		B	
d_I, Intersection Delay [s/veh]	0.78					
Intersection LOS	B					

Intersection Level Of Service Report
Intersection 3: Jackson St/Reeds Ave

Control Type: Two-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 17.6
 Level Of Service: C
 Volume to Capacity (v/c): 0.074

Intersection Setup

Name	Jackson Street		Jackson Street		Reeds Avenue	
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	1	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Reeds Avenue	
Base Volume Input [veh/h]	248	24	111	258	24	146
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	248	24	111	258	24	146
Peak Hour Factor	0.9520	0.9520	0.9520	0.9520	0.9520	0.9520
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	65	6	29	68	6	38
Total Analysis Volume [veh/h]	261	25	117	271	25	153
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.09	0.00	0.07	0.20
d_M, Delay for Movement [s/veh]	0.00	0.00	8.07	0.00	17.59	11.59
Movement LOS	A	A	A	A	C	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.30	0.00	1.09	1.09
95th-Percentile Queue Length [ft/ln]	0.00	0.00	7.48	0.00	27.13	27.13
d_A, Approach Delay [s/veh]	0.00		2.43		12.43	
Approach LOS	A		A		B	
d_I, Intersection Delay [s/veh]	3.71					
Intersection LOS	C					





Intersection Level Of Service Report

Intersection 4: Jackson St/Luther Rd

Control Type: Signalized
 Analysis Method: ICU 1
 Analysis Period: 15 minutes

Delay (sec / veh): -
 Level Of Service: C
 Volume to Capacity (v/c): 0.719

Intersection Setup

Name	Jackson Street			Jackson Street			Luther Road			Luther Road		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	0	0	1	0	0	0
Entry Pocket Length [ft]	105.00	100.00	100.00	85.00	100.00	100.00	100.00	100.00	250.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00			35.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name	Jackson Street			Jackson Street			Luther Road			Luther Road		
Base Volume Input [veh/h]	7	110	79	82	128	43	47	199	13	69	228	98
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	7	110	79	82	128	43	47	199	13	69	228	98
Peak Hour Factor	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	2	30	21	22	35	12	13	54	4	19	62	27
Total Analysis Volume [veh/h]	8	119	86	89	139	47	51	216	14	75	248	106
Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	0			0			0			0		

Intersection Settings

Cycle Length [s]	100
Lost time [s]	10.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Split	Split	Split	Split	Split	Split
Signal Group	1	6	0	5	2	0	0	8	0	0	4	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	-	-	-	-	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.01	0.13	0.13	0.06	0.12	0.12	0.03	0.17	0.01	0.05	0.27	0.27
Intersection LOS	C											
Intersection V/C	0.719											




Intersection Level Of Service Report

Intersection 5: Jackson St/Olive St

Control Type: Two-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 16.2
 Level Of Service: C
 Volume to Capacity (v/c): 0.009

Intersection Setup

Name	Jackson Street		Jackson Street		Olive Street	
Approach	Northbound		Southbound		Eastbound	
Lane Configuration						
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Olive Street	
Base Volume Input [veh/h]	2	369	405	5	3	2
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	2	369	405	5	3	2
Peak Hour Factor	0.8970	0.8970	0.8970	0.8970	0.8970	0.8970
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	1	103	113	1	1	1
Total Analysis Volume [veh/h]	2	411	452	6	3	2
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.00	0.01	0.00
d_M, Delay for Movement [s/veh]	8.24	0.00	0.00	0.00	16.19	11.00
Movement LOS	A	A	A	A	C	B
95th-Percentile Queue Length [veh/ln]	0.01	0.01	0.00	0.00	0.04	0.04
95th-Percentile Queue Length [ft/ln]	0.13	0.13	0.00	0.00	0.95	0.95
d_A, Approach Delay [s/veh]	0.04		0.00		14.11	
Approach LOS	A		A		B	
d_I, Intersection Delay [s/veh]	0.10					
Intersection LOS	C					

OPENING YEAR (2025) CONDITIONS





Intersection Level Of Service Report

Intersection 1: Jackson St/Aloha St

Control Type: All-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 22.6
 Level Of Service: C
 Volume to Capacity (v/c): 0.829

Intersection Setup

Name	Jackson Street			Jackson Street			Aloha Street			Aloha Street		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	1	0	0	1	0	0	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	120.00	100.00	100.00	75.00	100.00	100.00	70.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00			35.00			25.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name	Jackson Street			Jackson Street			Aloha Street			Aloha Street		
Base Volume Input [veh/h]	23	340	114	35	291	27	26	33	41	77	23	21
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	24	360	121	37	308	29	28	35	43	82	24	22
Peak Hour Factor	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	7	112	38	11	96	9	9	11	13	25	7	7
Total Analysis Volume [veh/h]	30	447	150	46	382	36	35	43	53	102	30	27
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings**Lanes**

Capacity per Entry Lane [veh/h]	575	652	521	567	441	495	445	497
Degree of Utilization, x	0.83	0.23	0.09	0.74	0.08	0.19	0.23	0.11

Movement, Approach, & Intersection Results




95th-Percentile Queue Length [veh]	8.58	0.88	0.29	6.30	0.26	0.71	0.87	0.39
95th-Percentile Queue Length [ft]	214.58	22.12	7.23	157.38	6.44	17.81	21.86	9.66
Approach Delay [s/veh]	26.94		23.35		11.68		12.36	
Approach LOS	D		C		B		B	
Intersection Delay [s/veh]	22.61							
Intersection LOS	C							

Intersection Level Of Service Report**Intersection 2: Jackson St/Lay Ave**

Control Type: Two-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 15.8
 Level Of Service: C
 Volume to Capacity (v/c): 0.025

Intersection Setup

Name	Jackson Street		Jackson Street		Lay Avenue	
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Lay Avenue	
Base Volume Input [veh/h]	423	3	26	377	7	40
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	448	3	28	400	7	42
Peak Hour Factor	0.8230	0.8230	0.8230	0.8230	0.8230	0.8230
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	136	1	9	122	2	13
Total Analysis Volume [veh/h]	544	4	34	486	9	51
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			Yes
Number of Storage Spaces in Median	0	0	1

Movement, Approach, & Intersection Results




V/C, Movement V/C Ratio	0.01	0.00	0.03	0.00	0.02	0.09
d_M, Delay for Movement [s/veh]	0.00	0.00	8.61	0.00	15.80	12.61
Movement LOS	A	A	A	A	C	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.10	0.00	0.40	0.40
95th-Percentile Queue Length [ft/ln]	0.00	0.00	2.55	0.00	10.04	10.04
d_A, Approach Delay [s/veh]	0.00		0.56		13.09	
Approach LOS	A		A		B	
d_I, Intersection Delay [s/veh]	0.96					
Intersection LOS	C					

Intersection Level Of Service Report Intersection 3: Jackson St/Reeds Ave

Control Type: Two-way stop
Analysis Method: HCM 6th Edition
Analysis Period: 15 minutes

Delay (sec / veh): 19.8
Level Of Service: C
Volume to Capacity (v/c): 0.023

Intersection Setup

Name	Jackson Street		Jackson Street		Reeds Avenue	
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	1	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Reeds Avenue	
Base Volume Input [veh/h]	346	15	72	290	5	45
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	367	16	76	307	5	48
Peak Hour Factor	0.8480	0.8480	0.8480	0.8480	0.8480	0.8480
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	108	5	22	91	1	14
Total Analysis Volume [veh/h]	433	19	90	362	6	57
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.08	0.00	0.02	0.09
d_M, Delay for Movement [s/veh]	0.00	0.00	8.50	0.00	19.76	11.58
Movement LOS	A	A	A	A	C	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.26	0.00	0.38	0.38
95th-Percentile Queue Length [ft/ln]	0.00	0.00	6.55	0.00	9.60	9.60
d_A, Approach Delay [s/veh]	0.00		1.69		12.36	
Approach LOS	A		A		B	
d_I, Intersection Delay [s/veh]	1.60					
Intersection LOS	C					





Intersection Level Of Service Report

Intersection 4: Jackson St/Luther Rd

Control Type: Signalized
 Analysis Method: ICU 1
 Analysis Period: 15 minutes

Delay (sec / veh): -
 Level Of Service: C
 Volume to Capacity (v/c): 0.780

Intersection Setup

Name	Jackson Street			Jackson Street			Luther Road			Luther Road		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	0	0	1	0	0	0
Entry Pocket Length [ft]	105.00	100.00	100.00	85.00	100.00	100.00	100.00	100.00	250.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00			35.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name	Jackson Street			Jackson Street			Luther Road			Luther Road		
Base Volume Input [veh/h]	27	219	91	76	204	43	51	234	47	56	162	57
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	29	232	96	81	216	46	54	248	50	59	172	60
Peak Hour Factor	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	8	63	26	22	59	12	15	67	14	16	47	16
Total Analysis Volume [veh/h]	31	252	104	88	235	50	59	269	54	64	187	65
Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	0			0			0			0		

Intersection Settings

Cycle Length [s]	100
Lost time [s]	10.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Split	Split	Split	Split	Split	Split
Signal Group	1	6	0	5	2	0	0	8	0	0	4	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	-	-	-	-	-	-

Movement, Approach, & Intersection Results




V/C, Movement V/C Ratio	0.02	0.22	0.22	0.06	0.18	0.18	0.04	0.21	0.03	0.04	0.20	0.20
Intersection LOS	C											
Intersection V/C	0.780											

Intersection Level Of Service Report Intersection 5: Jackson St/Olive St

Control Type: Two-way stop
Analysis Method: HCM 6th Edition
Analysis Period: 15 minutes

Delay (sec / veh): 17.4
Level Of Service: C
Volume to Capacity (v/c): 0.017

Intersection Setup

Name	Jackson Street		Jackson Street		Olive Street	
Approach	Northbound		Southbound		Eastbound	
Lane Configuration						
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Olive Street	
Base Volume Input [veh/h]	1	405	359	2	4	1
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	1	429	381	2	4	1
Peak Hour Factor	0.8670	0.8670	0.8670	0.8670	0.8670	0.8670
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	124	110	1	1	0
Total Analysis Volume [veh/h]	1	495	439	2	5	1
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.00	0.02	0.00
d_M, Delay for Movement [s/veh]	8.19	0.00	0.00	0.00	17.36	11.00
Movement LOS	A	A	A	A	C	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.00	0.00	0.06	0.06
95th-Percentile Queue Length [ft/ln]	0.07	0.07	0.00	0.00	1.41	1.41
d_A, Approach Delay [s/veh]	0.02		0.00		16.30	
Approach LOS	A		A		C	
d_I, Intersection Delay [s/veh]	0.11					
Intersection LOS	C					





Intersection Level Of Service Report

Intersection 1: Jackson St/Aloha St

Control Type: All-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 16.1
 Level Of Service: C
 Volume to Capacity (v/c): 0.655

Intersection Setup

Name	Jackson Street			Jackson Street			Aloha Street			Aloha Street		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	1	0	0	1	0	0	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	120.00	100.00	100.00	75.00	100.00	100.00	70.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00			35.00			25.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name	Jackson Street			Jackson Street			Aloha Street			Aloha Street		
Base Volume Input [veh/h]	58	284	84	33	299	28	17	38	38	115	37	19
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	61	301	89	35	317	30	18	40	40	122	39	20
Peak Hour Factor	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	16	80	24	9	84	8	5	11	11	32	10	5
Total Analysis Volume [veh/h]	65	319	94	37	336	32	19	42	42	129	41	21
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings**Lanes**

Capacity per Entry Lane [veh/h]	586	672	542	593	466	524	479	531
Degree of Utilization, x	0.66	0.14	0.07	0.62	0.04	0.16	0.27	0.12

Movement, Approach, & Intersection Results

95th-Percentile Queue Length [veh]	4.79	0.48	0.22	4.27	0.13	0.57	1.08	0.39
95th-Percentile Queue Length [ft]	119.85	12.11	5.47	106.77	3.18	14.16	26.99	9.84
Approach Delay [s/veh]	17.63		17.46		10.85		12.12	
Approach LOS	C		C		B		B	
Intersection Delay [s/veh]	16.08							
Intersection LOS	C							

Intersection Level Of Service Report

Intersection 2: Jackson St/Lay Ave

Control Type: Two-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 14.2
 Level Of Service: B
 Volume to Capacity (v/c): 0.015

Intersection Setup

Name	Jackson Street		Jackson Street		Lay Avenue	
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Lay Avenue	
Base Volume Input [veh/h]	394	6	35	406	6	28
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	418	6	37	430	6	30
Peak Hour Factor	0.9770	0.9770	0.9770	0.9770	0.9770	0.9770
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	107	2	9	110	2	8
Total Analysis Volume [veh/h]	428	6	38	440	6	31
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			Yes
Number of Storage Spaces in Median	0	0	1

Movement, Approach, & Intersection Results



V/C, Movement V/C Ratio	0.00	0.00	0.03	0.00	0.01	0.05
d_M, Delay for Movement [s/veh]	0.00	0.00	8.28	0.00	14.19	11.15
Movement LOS	A	A	A	A	B	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.10	0.00	0.20	0.20
95th-Percentile Queue Length [ft/ln]	0.00	0.00	2.59	0.00	5.11	5.11
d_A, Approach Delay [s/veh]	0.00		0.66		11.64	
Approach LOS	A		A		B	
d_I, Intersection Delay [s/veh]	0.79					
Intersection LOS	B					

Intersection Level Of Service Report Intersection 3: Jackson St/Reeds Ave

Control Type: Two-way stop
Analysis Method: HCM 6th Edition
Analysis Period: 15 minutes

Delay (sec / veh): 18.7
Level Of Service: C
Volume to Capacity (v/c): 0.082

Intersection Setup

Name	Jackson Street		Jackson Street		Reeds Avenue	
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	1	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Reeds Avenue	
Base Volume Input [veh/h]	248	24	111	258	24	146
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	263	25	118	273	25	155
Peak Hour Factor	0.9520	0.9520	0.9520	0.9520	0.9520	0.9520
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	69	7	31	72	7	41
Total Analysis Volume [veh/h]	276	26	124	287	26	163
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results





V/C, Movement V/C Ratio	0.00	0.00	0.10	0.00	0.08	0.21
d_M, Delay for Movement [s/veh]	0.00	0.00	8.14	0.00	18.67	12.01
Movement LOS	A	A	A	A	C	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.32	0.00	1.22	1.22
95th-Percentile Queue Length [ft/ln]	0.00	0.00	8.09	0.00	30.61	30.61
d_A, Approach Delay [s/veh]	0.00		2.46		12.93	
Approach LOS	A		A		B	
d_I, Intersection Delay [s/veh]	3.83					
Intersection LOS	C					

Intersection Level Of Service Report
Intersection 4: Jackson St/Luther Rd

Control Type: Signalized
 Analysis Method: ICU 1
 Analysis Period: 15 minutes

Delay (sec / veh): -
 Level Of Service: C
 Volume to Capacity (v/c): 0.756

Intersection Setup

Name	Jackson Street			Jackson Street			Luther Road			Luther Road		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	0	0	1	0	0	0
Entry Pocket Length [ft]	105.00	100.00	100.00	85.00	100.00	100.00	100.00	100.00	250.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00			35.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name	Jackson Street			Jackson Street			Luther Road			Luther Road		
Base Volume Input [veh/h]	7	110	79	82	128	43	47	199	13	69	228	98
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	7	117	84	87	136	46	50	211	14	73	242	104
Peak Hour Factor	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	2	32	23	24	37	12	14	57	4	20	66	28
Total Analysis Volume [veh/h]	8	127	91	94	148	50	54	229	15	79	263	113
Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	0			0			0			0		

Intersection Settings

Cycle Length [s]	100
Lost time [s]	10.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Split	Split	Split	Split	Split	Split
Signal Group	1	6	0	5	2	0	0	8	0	0	4	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	-	-	-	-	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.01	0.14	0.14	0.06	0.12	0.12	0.03	0.18	0.01	0.05	0.28	0.28
Intersection LOS	C											
Intersection V/C	0.756											



Intersection Level Of Service Report

Intersection 5: Jackson St/Olive St

Control Type: Two-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 17.0
 Level Of Service: C
 Volume to Capacity (v/c): 0.010

Intersection Setup

Name	Jackson Street		Jackson Street		Olive Street	
Approach	Northbound		Southbound		Eastbound	
Lane Configuration						
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Olive Street	
Base Volume Input [veh/h]	2	369	405	5	3	2
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	2	391	429	5	3	2
Peak Hour Factor	0.8970	0.8970	0.8970	0.8970	0.8970	0.8970
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	1	109	120	1	1	1
Total Analysis Volume [veh/h]	2	436	478	6	3	2
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.00	0.01	0.00
d_M, Delay for Movement [s/veh]	8.31	0.00	0.00	0.00	17.00	11.21
Movement LOS	A	A	A	A	C	B
95th-Percentile Queue Length [veh/ln]	0.01	0.01	0.00	0.00	0.04	0.04
95th-Percentile Queue Length [ft/ln]	0.14	0.14	0.00	0.00	1.01	1.01
d_A, Approach Delay [s/veh]	0.04		0.00		14.68	
Approach LOS	A		A		B	
d_I, Intersection Delay [s/veh]	0.10					
Intersection LOS	C					

OPENING YEAR (2025) WITH PROJECT CONDITIONS





Intersection Level Of Service Report

Intersection 1: Jackson St/Aloha St

Control Type: All-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 23.5
 Level Of Service: C
 Volume to Capacity (v/c): 0.844

Intersection Setup

Name	Jackson Street			Jackson Street			Aloha Street			Aloha Street		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	1	0	0	1	0	0	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	120.00	100.00	100.00	75.00	100.00	100.00	70.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00			35.00			25.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name	Jackson Street			Jackson Street			Aloha Street			Aloha Street		
Base Volume Input [veh/h]	23	340	114	35	291	27	26	33	41	77	23	21
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	1	5	2	0	2	0	0	0	0	1	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	25	365	123	37	310	29	28	35	43	83	24	22
Peak Hour Factor	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060	0.8060
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	8	113	38	11	96	9	9	11	13	26	7	7
Total Analysis Volume [veh/h]	31	453	153	46	385	36	35	43	53	103	30	27
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings**Lanes**

Capacity per Entry Lane [veh/h]	573	649	519	565	438	492	443	494
Degree of Utilization, x	0.84	0.24	0.09	0.75	0.08	0.20	0.23	0.12

Movement, Approach, & Intersection Results

95th-Percentile Queue Length [veh]	9.00	0.91	0.29	6.47	0.26	0.72	0.89	0.39
95th-Percentile Queue Length [ft]	224.99	22.79	7.26	161.87	6.48	17.93	22.27	9.71
Approach Delay [s/veh]	28.25		23.98		11.74		12.44	
Approach LOS	D		C		B		B	
Intersection Delay [s/veh]	23.46							
Intersection LOS	C							



Intersection Level Of Service Report

Intersection 2: Jackson St/Lay Ave

Control Type: Two-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 15.9
 Level Of Service: C
 Volume to Capacity (v/c): 0.025

Intersection Setup

Name	Jackson Street		Jackson Street		Lay Avenue	
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Lay Avenue	
Base Volume Input [veh/h]	423	3	26	377	7	40
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	8	0	0	3	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	456	3	28	403	7	42
Peak Hour Factor	0.8230	0.8230	0.8230	0.8230	0.8230	0.8230
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	139	1	9	122	2	13
Total Analysis Volume [veh/h]	554	4	34	490	9	51
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			Yes
Number of Storage Spaces in Median	0	0	1

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.01	0.00	0.03	0.00	0.02	0.10
d_M, Delay for Movement [s/veh]	0.00	0.00	8.64	0.00	15.94	12.72
Movement LOS	A	A	A	A	C	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.10	0.00	0.41	0.41
95th-Percentile Queue Length [ft/ln]	0.00	0.00	2.58	0.00	10.18	10.18
d_A, Approach Delay [s/veh]	0.00		0.56		13.20	
Approach LOS	A		A		B	
d_I, Intersection Delay [s/veh]	0.95					
Intersection LOS	C					




Intersection Level Of Service Report

Intersection 3: Jackson St/Reeds Ave

Control Type: Two-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 20.1
 Level Of Service: C
 Volume to Capacity (v/c): 0.024

Intersection Setup

Name	Jackson Street		Jackson Street		Reeds Avenue	
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	1	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Reeds Avenue	
Base Volume Input [veh/h]	346	15	72	290	5	45
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	1	0	4	4	0	2
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	368	16	80	311	5	50
Peak Hour Factor	0.8480	0.8480	0.8480	0.8480	0.8480	0.8480
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	108	5	24	92	1	15
Total Analysis Volume [veh/h]	434	19	94	367	6	59
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.08	0.00	0.02	0.09
d_M, Delay for Movement [s/veh]	0.00	0.00	8.51	0.00	20.11	11.62
Movement LOS	A	A	A	A	C	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.27	0.00	0.40	0.40
95th-Percentile Queue Length [ft/ln]	0.00	0.00	6.87	0.00	9.96	9.96
d_A, Approach Delay [s/veh]	0.00		1.74		12.40	
Approach LOS	A		A		B	
d_I, Intersection Delay [s/veh]	1.64					
Intersection LOS	C					





Intersection Level Of Service Report

Intersection 4: Jackson St/Luther Rd

Control Type: Signalized
 Analysis Method: ICU 1
 Analysis Period: 15 minutes

Delay (sec / veh): -
 Level Of Service: C
 Volume to Capacity (v/c): 0.782

Intersection Setup

Name	Jackson Street			Jackson Street			Luther Road			Luther Road		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	0	0	1	0	0	0
Entry Pocket Length [ft]	105.00	100.00	100.00	85.00	100.00	100.00	100.00	100.00	250.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00			35.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name	Jackson Street			Jackson Street			Luther Road			Luther Road		
Base Volume Input [veh/h]	27	219	91	76	204	43	51	234	47	56	162	57
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	2	1	1	0	0	0	0	0	1
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	29	232	96	83	217	47	54	248	50	59	172	61
Peak Hour Factor	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	8	63	26	23	59	13	15	67	14	16	47	17
Total Analysis Volume [veh/h]	31	252	104	90	236	51	59	269	54	64	187	66
Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	0			0			0			0		

Intersection Settings

Cycle Length [s]	100
Lost time [s]	10.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Split	Split	Split	Split	Split	Split
Signal Group	1	6	0	5	2	0	0	8	0	0	4	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	-	-	-	-	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.02	0.22	0.22	0.06	0.18	0.18	0.04	0.21	0.03	0.04	0.20	0.20
Intersection LOS	C											
Intersection V/C	0.782											





Intersection Level Of Service Report

Intersection 5: Jackson St/Olive St

Control Type: Two-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 20.4
 Level Of Service: C
 Volume to Capacity (v/c): 0.021

Intersection Setup

Name	Jackson Street			Jackson Street			Olive Street			Olive Street		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00			35.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	No			No			Yes			Yes		

Volumes

Name	Jackson Street			Jackson Street			Olive Street			Olive Street		
Base Volume Input [veh/h]	1	405	0	0	359	2	4	0	1	0	0	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	3	3	0	0	0	0	0	8	0	8
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	1	429	3	3	381	2	4	0	1	8	0	8
Peak Hour Factor	0.8670	0.8670	0.8670	0.8670	0.8670	0.8670	0.8670	0.8670	0.8670	0.8670	0.8670	0.8670
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	124	1	1	110	1	1	0	0	2	0	2
Total Analysis Volume [veh/h]	1	495	3	3	439	2	5	0	1	9	0	9
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings

Priority Scheme	Free	Free	Stop	Stop
Flared Lane			No	No
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance			No	No
Number of Storage Spaces in Median	0	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.00	0.00	0.00	0.02	0.00	0.00	0.04	0.00	0.02
d_M, Delay for Movement [s/veh]	8.19	0.00	0.00	8.35	0.00	0.00	20.44	19.02	11.10	20.39	19.27	11.80
Movement LOS	A	A	A	A	A	A	C	C	B	C	C	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.00	0.01	0.01	0.00	0.07	0.07	0.07	0.17	0.17	0.17
95th-Percentile Queue Length [ft/ln]	0.07	0.07	0.00	0.21	0.21	0.00	1.73	1.73	1.73	4.14	4.14	4.14
d_A, Approach Delay [s/veh]	0.02			0.06			18.88			16.10		
Approach LOS	A			A			C			C		
d_I, Intersection Delay [s/veh]	0.45											
Intersection LOS	C											





Intersection Level Of Service Report

Intersection 1: Jackson St/Aloha St

Control Type: All-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 16.5
 Level Of Service: C
 Volume to Capacity (v/c): 0.665

Intersection Setup

Name	Jackson Street			Jackson Street			Aloha Street			Aloha Street		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	1	0	0	1	0	0	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	120.00	100.00	100.00	75.00	100.00	100.00	70.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00			35.00			25.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name	Jackson Street			Jackson Street			Aloha Street			Aloha Street		
Base Volume Input [veh/h]	58	284	84	33	299	28	17	38	38	115	37	19
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	1	3	1	0	6	0	0	0	1	2	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	62	304	90	35	323	30	18	40	41	124	39	20
Peak Hour Factor	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440	0.9440
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	16	81	24	9	86	8	5	11	11	33	10	5
Total Analysis Volume [veh/h]	66	322	95	37	342	32	19	42	43	131	41	21
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings**Lanes**

Capacity per Entry Lane [veh/h]	583	669	539	590	464	521	476	528
Degree of Utilization, x	0.67	0.14	0.07	0.63	0.04	0.16	0.27	0.12

Movement, Approach, & Intersection Results

95th-Percentile Queue Length [veh]	4.96	0.49	0.22	4.46	0.13	0.58	1.11	0.40
95th-Percentile Queue Length [ft]	123.93	12.34	5.50	111.61	3.20	14.46	27.73	9.90
Approach Delay [s/veh]	18.06		17.99		10.92		12.24	
Approach LOS	C		C		B		B	
Intersection Delay [s/veh]	16.47							
Intersection LOS	C							




Intersection Level Of Service Report

Intersection 2: Jackson St/Lay Ave

Control Type: Two-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 14.3
 Level Of Service: B
 Volume to Capacity (v/c): 0.015

Intersection Setup

Name	Jackson Street		Jackson Street		Lay Avenue	
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Lay Avenue	
Base Volume Input [veh/h]	394	6	35	406	6	28
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	5	0	0	9	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	423	6	37	439	6	30
Peak Hour Factor	0.9770	0.9770	0.9770	0.9770	0.9770	0.9770
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	108	2	9	112	2	8
Total Analysis Volume [veh/h]	433	6	38	449	6	31
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			Yes
Number of Storage Spaces in Median	0	0	1

Movement, Approach, & Intersection Results


V/C, Movement V/C Ratio	0.00	0.00	0.03	0.00	0.01	0.05
d_M, Delay for Movement [s/veh]	0.00	0.00	8.29	0.00	14.30	11.20
Movement LOS	A	A	A	A	B	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.10	0.00	0.21	0.21
95th-Percentile Queue Length [ft/ln]	0.00	0.00	2.60	0.00	5.15	5.15
d_A, Approach Delay [s/veh]	0.00		0.65		11.70	
Approach LOS	A		A		B	
d_I, Intersection Delay [s/veh]	0.78					
Intersection LOS	B					

Intersection Level Of Service Report Intersection 3: Jackson St/Reeds Ave

Control Type: Two-way stop
Analysis Method: HCM 6th Edition
Analysis Period: 15 minutes

Delay (sec / veh): 19.0
Level Of Service: C
Volume to Capacity (v/c): 0.084

Intersection Setup

Name	Jackson Street		Jackson Street		Reeds Avenue	
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	1	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		No		Yes	

Volumes

Name	Jackson Street		Jackson Street		Reeds Avenue	
Base Volume Input [veh/h]	248	24	111	258	24	146
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	4	0	3	4	0	4
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	267	25	121	277	25	159
Peak Hour Factor	0.9520	0.9520	0.9520	0.9520	0.9520	0.9520
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	70	7	32	73	7	42
Total Analysis Volume [veh/h]	280	26	127	291	26	167
Pedestrian Volume [ped/h]	0		0		0	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.10	0.00	0.08	0.22
d_M, Delay for Movement [s/veh]	0.00	0.00	8.16	0.00	19.02	12.14
Movement LOS	A	A	A	A	C	B
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.33	0.00	1.27	1.27
95th-Percentile Queue Length [ft/ln]	0.00	0.00	8.34	0.00	31.79	31.79
d_A, Approach Delay [s/veh]	0.00		2.48		13.07	
Approach LOS	A		A		B	
d_I, Intersection Delay [s/veh]	3.88					
Intersection LOS	C					





Intersection Level Of Service Report

Intersection 4: Jackson St/Luther Rd

Control Type: Signalized
 Analysis Method: ICU 1
 Analysis Period: 15 minutes

Delay (sec / veh): -
 Level Of Service: C
 Volume to Capacity (v/c): 0.761

Intersection Setup

Name	Jackson Street			Jackson Street			Luther Road			Luther Road		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	0	0	1	0	0	0
Entry Pocket Length [ft]	105.00	100.00	100.00	85.00	100.00	100.00	100.00	100.00	250.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00			35.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name	Jackson Street			Jackson Street			Luther Road			Luther Road		
Base Volume Input [veh/h]	7	110	79	82	128	43	47	199	13	69	228	98
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	1	0	2	1	1	1	0	0	0	0	2
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	7	118	84	89	137	47	51	211	14	73	242	106
Peak Hour Factor	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210	0.9210
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	2	32	23	24	37	13	14	57	4	20	66	29
Total Analysis Volume [veh/h]	8	128	91	97	149	51	55	229	15	79	263	115
Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	0			0			0			0		

Intersection Settings

Cycle Length [s]	100
Lost time [s]	10.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Split	Split	Split	Split	Split	Split
Signal Group	1	6	0	5	2	0	0	8	0	0	4	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	-	-	-	-	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.01	0.14	0.14	0.06	0.13	0.13	0.03	0.18	0.01	0.05	0.29	0.29
Intersection LOS	C											
Intersection V/C	0.761											





Intersection Level Of Service Report

Intersection 5: Jackson St/Olive St

Control Type: Two-way stop
 Analysis Method: HCM 6th Edition
 Analysis Period: 15 minutes

Delay (sec / veh): 20.4
 Level Of Service: C
 Volume to Capacity (v/c): 0.033

Intersection Setup

Name	Jackson Street			Jackson Street			Olive Street			Olive Street		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	35.00			35.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	No			No			Yes			Yes		

Volumes

Name	Jackson Street			Jackson Street			Olive Street			Olive Street		
Base Volume Input [veh/h]	2	369	0	0	405	5	3	0	2	0	0	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Factor	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600	1.0600
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	8	9	0	0	0	0	0	7	0	5
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	2	391	8	9	429	5	3	0	2	7	0	5
Peak Hour Factor	0.8970	0.8970	0.8970	0.8970	0.8970	0.8970	0.8970	0.8970	0.8970	0.8970	0.8970	0.8970
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	1	109	2	3	120	1	1	0	1	2	0	1
Total Analysis Volume [veh/h]	2	436	9	10	478	6	3	0	2	8	0	6
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings

Priority Scheme	Free	Free	Stop	Stop
Flared Lane			No	No
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance			No	No
Number of Storage Spaces in Median	0	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.01	0.00	0.00	0.01	0.00	0.00	0.03	0.00	0.01
d_M, Delay for Movement [s/veh]	8.31	0.00	0.00	8.23	0.00	0.00	20.27	19.06	11.27	20.40	19.31	11.25
Movement LOS	A	A	A	A	A	A	C	C	B	C	C	B
95th-Percentile Queue Length [veh/ln]	0.01	0.01	0.00	0.03	0.03	0.00	0.05	0.05	0.05	0.13	0.13	0.13
95th-Percentile Queue Length [ft/ln]	0.14	0.14	0.00	0.67	0.67	0.00	1.21	1.21	1.21	3.34	3.34	3.34
d_A, Approach Delay [s/veh]	0.04			0.17			16.67			16.48		
Approach LOS	A			A			C			C		
d_I, Intersection Delay [s/veh]	0.43											
Intersection LOS	C											