Schaaf & Wheeler

CONSULTING CIVIL ENGINEERS

MEMORANDUM

SUBJECT:	100-year Base Flood Elevation Determina	tion – Dry (Creek
FROM:	Katie Hogan, PE, CFM	JOB#:	AABC.01
TO:	Mike Muelrath, PE	DATE:	12/16/2020

Introduction

The property owner at APN 034-190-040 in Napa County, California is planning construction onsite. The site is generally located in the unincorporated Napa County west of Highway 29 and south of Darms Lane, as shown in Figure 1. The property boundary borders the Dry Creek stream centerline within the Napa River watershed.

A portion of the lot is located within a FEMA Special Flood Hazard Area (SFHA) Zone A floodplain on the Federal Insurance Rate Map (FIRM) panel number 06055C0505F, effective September 28, 2010, as shown in Figure 2.

The project is subject to County floodplain ordinances per the National Flood Insurance Program (NFIP), which requires that the building be elevated at least 1 foot above the base flood elevation (BFE). We completed a hydrologic and hydraulic analysis to calculate the BFE along Dry Creek near the project site.

This memorandum documents the methodology and results of the analysis.

Site Conditions

The project site is adjacent to Dry Creek, which is upstream of the Highway 29 and Solano Avenue bridge crossings. The property mostly consists of vineyards. Dry Creek conveys runoff from the Enchanted Hills east towards the Napa River.

Neighboring properties have obtained approved Letters of Map Amendment (LOMAs). LOMA case number 14-09-0308A for APN 034-212-011 across the creek from the subject property lists the 100-year base flood elevation as 106.2 feet, NAVD 88. This property is indicated in Figure 3.

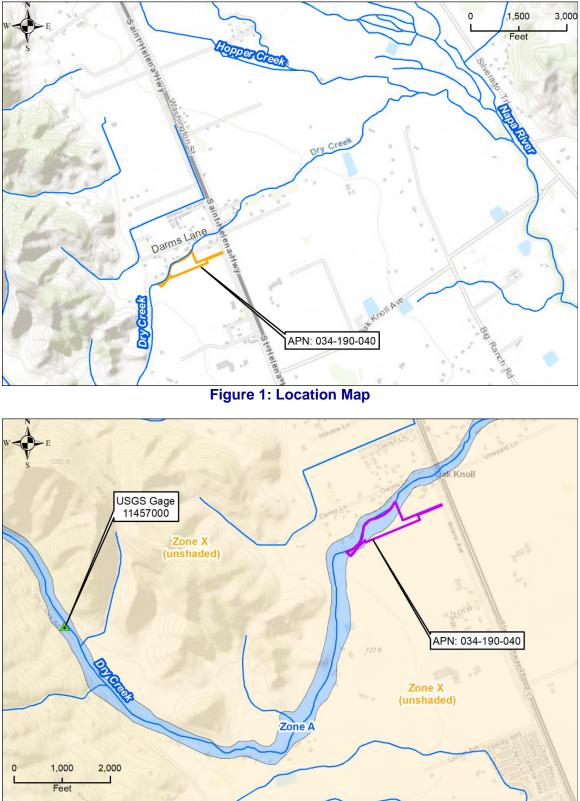


Figure 2: FEMA Effective Special Flood Hazard Areas

Hydrology

The United States Geological Survey (USGS) stream gage 11457000 (Figure 2) collected flowrate information along Dry Creek, which is upstream of the project site. The USGS Stream Statistics (StreamStats) program used this gage data to calculate a 100-year flowrate of 4,170 cfs. We used this flowrate in the hydraulic model to calculate 100-year BFEs along the subject property.

The StreamStats gage information is included as an attachment to this memorandum.

As-built plans from the California Department of Transportation (CalTrans) indicate a 100-year flowrate of 4,000 cfs at the Highway 29 bridges. While the flowrates are similar, we used the USGS gage data for this analysis because it is more conservative.

Hydraulics

We created a HEC-RAS model for Dry Creek to calculate 100-year water surface elevations. The model extends along Dry Creek from just downstream of Washington Street to approximately 800 linear feet upstream of the subject property. We developed cross sections geometry using the 2014 digital elevation model (DEM) for Napa County. We also used an onsite survey performed in 2014 to supplement the LiDAR data. The survey is included as an attachment.

We used as-built bridge plans provided by CalTrans to model the Solano Avenue, Highway 29, and Washington Street Bridges, which are attached to this memorandum.

We estimated the railroad bridge height and pier information based on available data in the CalTrans plans and the available topographic information. We used a conversion of +2.66 feet to translate the elevations (NGVD 29) in the bridge plans in order to reference the NAVD 88 vertical datum.

The elevation conversion is based on the Napa County benchmarks 723-C and 724-C along Solano Avenue near the project site. The vertical datum conversion from NGVD 29 to NAVD 88 was not available for Dry Creek in the FEMA Flood Insurance Study (FIS).

We used a normal depth of 0.7 percent slope as the downstream boundary condition measured using the DEM. There are no available, effective base flood elevations along Dry Creek. We assigned Manning's roughness values based on aerial imagery and Google Street View photos.

Figure 4 shows an image of Dry Creek looking upstream from Solano Avenue. We used a roughness value of 0.04 for the channel, and 0.08 was used for the overbanks to represent the trees, development, and vineyards in the overbanks. We reduced channel roughness to 0.035 beneath the bridges since vegetation is minimal.

Ineffective flow areas represent the expansion and contraction of the floodplain through the bridges at a 1:1 ratio upstream and 4:1 downstream. We adjusted the contraction and expansion coefficients to 0.3 and 0.5, respectively, at the cross sections upstream and downstream of the bridges.

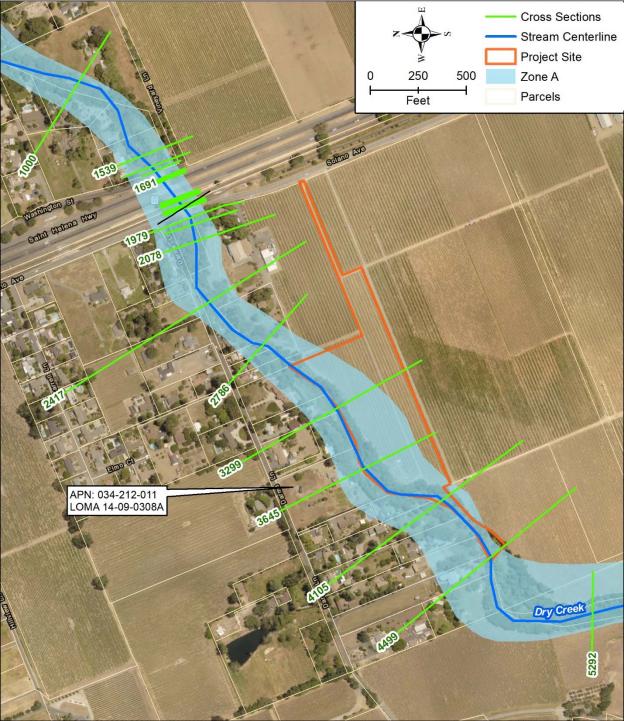


Figure 3: Hydraulic Work Map Dry Creek

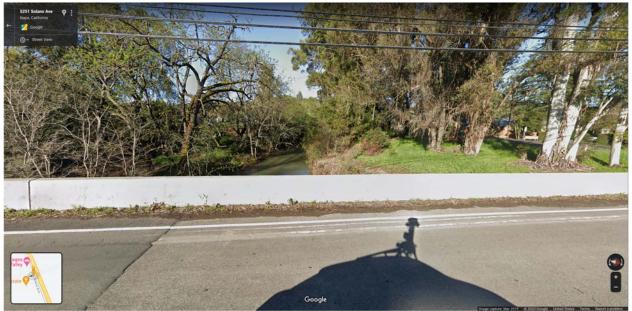


Figure 4: Google Street View on Solano Avenue Looking West (Upstream) at Dry Creek

Results

The results of the analysis indicate a 100-year base flood elevation of 109.51 feet, NAVD 88 at the upstream end of the property and 105.3 feet, NAVD 88, at the downstream end of the property.

The sections through the property site are highlighted in green in Table 1. Note that the calculated 100-year water surface elevation of 106.2 feet, NAVD 88, matches the base flood elevation determined as part of the LOMA for property APN 034-212-111.

The 100-year water surface elevation is generally contained within the channel banks of Dry Creek near the project site, as shown in Figure 5.

Table 1: Dry Creek 100-year Water Surface Elevations (it, NAVD 88)							
	WSE (ft,		WSE (ft,		WSE (ft,		
River Station	NAV D88)	River Station	NAV D88)	River Station	NAV D88)		
5292	114.89	1886	100.08	1703	97.18		
4499	110.21	1881	100.09	1700	97.28		
4105	109.51	1876	100.09	1697	97.40		
3645	106.17	1870	100.10	1691	97.6		
3299	105.30	1857	Railroad		Washington		
2786	103.52	1844	99.82	1674	Street		
2417	101.68	1840	99.80	1645	97.64		
2078	100.77	1836	99.73	1615	97.22		
2017	100.36	1832	99.66	1539	96.67		
1979	100.42	1827	99.53	1000	92.98		
1928	Solano Ave	1765	HWY 29				
1892	100.07	1709	96.94				

Table 1: Dry Creek 100-year Water Surface Elevations (ft, NAVD 88)

*Highlighted areas represent the project site

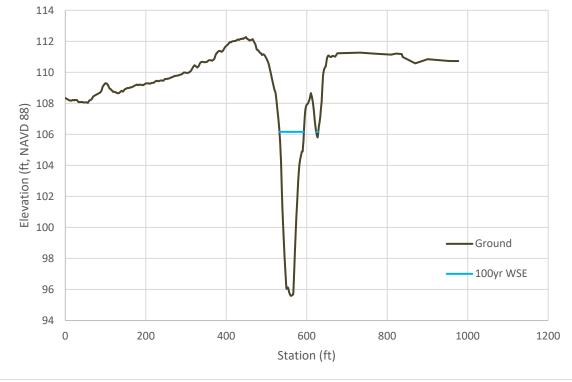


Figure 5: Cross Section 3645 100-year Water Surface Elevations

Recommendations and Conclusions

The results indicate that the 100-year flowrate calculated at the USGS gage on Dry Creek is contained within the channel banks where the floodplain is located within the subject property. The water surface elevation varies across the property site.

Note that the LiDAR data used for the channel geometry may not fully capture the channel flowline due to standing water and vegetation. A survey of the channel would provide additional detail and may change the calculations.

Structures constructed within the Zone A floodplain are required to elevate the finished floor elevation 1 foot above the base flood elevation to meet County NFIP regulations. Flood insurance rates may still apply. A LOMA or Letter of Map Revision Based on Fill (LOMR-F) would need to be submitted to FEMA to remove any structures or portions of property from the Zone A floodplain.

The lowest adjacent grade to any structure or lowest elevation within a defined portion of land must be at or above the BFE to remove the structure or area from the Zone A designation. Flood insurance requirements are subject to the lender and NFIP regulations.

Attachments:

- 1. USGS StreamStats Results
- 2. CalTrans As-built Bridge Plans
- 3. 2014 Onsite Survey
- 4. LOMA 14-09-0308A-06025
- 5. HEC-RAS Cross Sections
- 6. HEC-RAS Stream Profile

Attachment 1: USGS StreamStats Gage Data



StreamStats Data-Collection Station Report

USGS Station Number 11457000 Station Name DRY C NR NAPA CA

Click here to link to available data on NWIS-Web for this site.

Descriptive Information

Station Type	Streamgage, continuous record
Location	
Gage	
Regulation and Diversions	
Regulated?	False
Period of Record	
Remarks	
Latitude (degrees NAD83)	38.35630164
Longitude (degrees NAD83)	-122.36497702
Hydrologic unit code	18050002
County	-
HCDN2009	No

Physical Characteristics

Characteristic Name	Value	Units	Citation Number
Descriptive Information			
High_Flow_Region_Code	1	dimensionless	<u>230</u>
High_Flow_Regression_Equation	Y	Yes or No	<u>230</u>
Datum_of_Latitude_Longitude	NAD83	dimensionless	<u>30</u>
District_Code	06	dimensionless	<u>30</u>
Begin_date_of_record	4/1/1951	days	<u>41</u>
End_date_of_record	9/30/1966	days	<u>41</u>
Number_of_days_of_record	5662	days	<u>41</u>
Number_of_days_GT_0 Precipitation Statistics	4465	days	<u>41</u>
24_Hour_2_Year_Precipitation	3.3000	inches	<u>31</u>
Mean_Annual_Precipitation Climate Characteristics	40.72	inches	<u>230</u>
Mean_Annual_Lake_Evaporation Temperature Statistics	49.000	inches	<u>31</u>
Mean_Min_January_Temperature Topographical Characteristics	36.000	degrees F	<u>31</u>
Elevation_of_10_and_85_points	1200.00	feet	<u>31</u>
Percent_above_5000_ft	0.0000	percent	<u>31</u>
Mean_Basin_Elevation Land Cover Characteristics	1139	feet	<u>230</u>
Percent_Forest	29.000	percent	<u>31</u>
Percent_Storage Stream Channel Properties	0.0000	percent	<u>31</u>
Main_Channel_Length	10.800	miles	<u>31</u>
Stream_Slope_10_and_85_Method Basin Dimensional Characteristics	72.000	feet per mi	<u>31</u>
Contributing_Drainage_Area	17.400	square miles	<u>31</u>

11/30/2020

StreamStats Data-Collection Station Repo	rt
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Drainage_Area	17.3	square miles	<u>230</u>
0 =			

Streamflow Statistics

Statistic Name	Value	Units	Citation Number F	Preferred?	Years of Record	Standard Error, percent	Variance log- 10	Lower 95% Upper 95% Confidence Confidenc Interval Interval
Deals Flow Statistics								
Peak-Flow Statistics Mean_Annual_Flood	617.000	cubic feet per second	<u>31</u>	Y				
2_Year_Peak_Flood	1580	cubic feet per second	<u>230</u>	Y	15	13.31	0.00331149995	5
5_Year_Peak_Flood	2350	cubic feet per	<u>230</u>	Y	15	13.43	0.00337069994	ı
		second	_					
10_Year_Peak_Flood	2830	cubic feet per second	<u>230</u>	Y	15	14.88	0.00412950013	3
25_Year_Peak_Flood	3400	cubic feet per second	<u>230</u>	Y	15	17.42	0.00564080011	
50_Year_Peak_Flood	3800	cubic feet per second	<u>230</u>	Y	15	19.62	0.00712289987	,
100_Year_Peak_Flood	4170	cubic feet per second	<u>230</u>	Y	15	21.96	0.00888239965	5

200_Year_Peak_Flood	4530	cubic feet per second	<u>230</u>	Y	15	24.4	0.0109099997
500_Year_Peak_Flood	4980	cubic feet per second	<u>230</u>	Y	15	27.75	0.0139880003
Regression_2_Year_Peak_Flood	917	cubic feet per	<u>230</u>	Y		58.14	0.0549179986
Regression_5_Year_Peak_Flood	1780	second cubic feet per	230	Y		47.03	0.0376829989
Regression_10_Year_Peak_Flood	2400	second cubic feet per	230	Y		44.01	0.0333979987
Regression_25_Year_Peak_Flood	3230	second cubic feet per	230	Y		42.29	0.0310299993
Regression_50_Year_Peak_Flood	3850	second cubic feet per	<u>230</u>	Y		42.37	0.0311450008
		second					
Regression_100_Year_Peak_Flood	4510	cubic feet per second	<u>230</u>	Y		43.88	0.0332069993
Regression_200_Year_Peak_Flood	5120	cubic feet per second	<u>230</u>	Y		44.07	0.0334799998
Regression_500_Year_Peak_Flood	5950	cubic feet per second	<u>230</u>	Y		45.63	0.0356810018
Weighted_5_Year_Peak_Flood	2300	cubic feet per second	<u>230</u>	Y		12.86	0.00309389993
Weighted_10_Year_Peak_Flood	2780	cubic feet per second	<u>230</u>	Y		14.03	0.00367509993
Weighted_25_Year_Peak_Flood	3370	cubic feet per second	<u>230</u>	Y		16.01	0.00477309991
Weighted_50_Year_Peak_Flood	3810	cubic feet per second	<u>230</u>	Y		17.67	0.00579709979
Weighted_100_Year_Peak_Flood	4240	cubic feet per second	<u>230</u>	Y		19.46	0.00700790016
Weighted_200_Year_Peak_Flood	4670	cubic feet per second	<u>230</u>	Y		21.117165828988	1 0.00822880026
Weighted_500_Year_Peak_Flood	5230	cubic feet per second	<u>230</u>	Y		23.39	0.0100490004
Peak_years_with_historic_adjustment	15	years	<u>230</u>	N			
Weighted_2_Year_Peak_Flood	1530	cubic feet per second	<u>230</u>	Y		12.92	0.00312320003
Flood-Volume Statistics							
1_Day_2_Year_Maximum	596.000	cubic feet per second	<u>31</u>	Y			
1_Day_50_Year_Maximum	1683.00	cubic feet per second	<u>31</u>	Y			
3_Day_2_Year_Maximum	394.000	cubic feet per	<u>31</u>	Y			

second

11/30/2020		Strea	amStats Da	ta-Collect	tion Station Report
3_Day_50_Year_Maximum	1108.00	cubic feet per second	<u>31</u>	Y	
7_Day_2_Year_Maximum	234.000	cubic feet per second	<u>31</u>	Y	
7_Day_50_Year_Maximum	809.000	cubic feet per second	<u>31</u>	Y	
15_Day_2_Year_Maximum	149.000	cubic feet per second	<u>31</u>	Y	
15_Day_50_Year_Maximum	526.000	cubic feet per second	<u>31</u>	Y	
Flow-Duration Statistics					
1_Percent_Duration	316	cubic feet per second	<u>325</u>	Y	15
2_Percent_Duration	199	cubic feet per second	<u>325</u>	Y	15
3_Percent_Duration	144	cubic feet per second	<u>325</u>	Y	15
5_Percent_Duration	89	cubic feet per second	<u>325</u>	Y	15
10_Percent_Duration	42	cubic feet per second	<u>325</u>	Y	15
15_Percent_Duration	24	cubic feet per second	<u>325</u>	Y	15
20_Percent_Duration	14	cubic feet per second	<u>325</u>	Y	15
25_Percent_Duration	9.9	cubic feet per second	<u>325</u>	Y	15
30_Percent_Duration	7.3	cubic feet per second	<u>325</u>	Y	15
35_Percent_Duration	5.3	cubic feet per second	<u>325</u>	Y	15
40_Percent_Duration	4	cubic feet per second	<u>325</u>	Y	15
45_Percent_Duration	2.7	cubic feet per second	<u>325</u>	Y	15
50_Percent_Duration	1.9	cubic feet per second	<u>325</u>	Y	15
55_Percent_Duration	1.4	cubic feet per second	<u>325</u>	Y	15
60_Percent_Duration	0.9	cubic feet per second	<u>325</u>	Y	15
65_Percent_Duration	0.5	cubic feet per second	<u>325</u>	Y	15
70_Percent_Duration	0.2	cubic feet per second	<u>325</u>	Y	15
75_Percent_Duration	0.1	cubic feet per second	<u>325</u>	Y	15
80_Percent_Duration	0	cubic feet per second	<u>325</u>	Y	15
85_Percent_Duration	0	cubic feet per second	<u>325</u>	Y	15
90_Percent_Duration	0	cubic feet per second	<u>325</u>	Y	15
95_Percent_Duration	0	cubic feet per second	<u>325</u>	Y	15
97_Percent_Duration	0	cubic feet per second	<u>325</u>	Y	15
98_Percent_Duration	0	cubic feet per second	<u>325</u>	Y	15
99_Percent_Duration	0	cubic feet per second	<u>325</u>	Y	15
Annual Flow Statistics					
Mean_Annual_Flow	20	cubic feet per second	<u>325</u>	Y	15

11/30/2020		Stre	amStats Da	ata-Collect	ion Station Report
Stand_Dev_of_Mean_Annual_Flow	14	cubic feet per	<u>325</u>	Y	15
Maximum_Annual_Mean_Flow	48	second cubic feet per second	<u>325</u>	Y	15
Minimum_Annual_Mean_Flow	5.4	cubic feet per second	<u>325</u>	Y	15
Monthly Flow Statistics					
January_Mean_Flow	59.950	cubic feet per second	<u>31</u>	Y	
January_STD	56.320	cubic feet per second	<u>31</u>	Y	
February_Mean_Flow	67.600	cubic feet per second	<u>31</u>	Y	
February_STD	62.210	cubic feet per second	<u>31</u>	Y	
March_Mean_Flow	35.510	cubic feet per second	<u>31</u>	Y	
March_STD	29.130	cubic feet per second	<u>31</u>	Y	
April_Mean_Flow	26.460	cubic feet per second	<u>31</u>	Y	
April_STD	39.280	cubic feet per second	<u>31</u>	Y	
May_Mean_Flow	5.8300	cubic feet per second	<u>31</u>	Y	
May_STD	4.0000	cubic feet per second	<u>31</u>	Y	
June_Mean_Flow	1.7800	cubic feet per second	<u>31</u>	Y	
June_STD	1.4800	cubic feet per second	<u>31</u>	Y	
October_Mean_Flow	3.0600	cubic feet per second	<u>31</u>	Y	
October_STD	9.9600	cubic feet per second	<u>31</u>	Y	
November_Mean_Flow	3.1100	cubic feet per	<u>31</u>	Y	
November_STD	3.2300	cubic feet per second	<u>31</u>	Y	
December_Mean_Flow	35.640	cubic feet per second	<u>31</u>	Y	
December_STD	57.720	cubic feet per second	<u>31</u>	Y	
General Flow Statistics					
Minimum_daily_flow	0	cubic feet per second	<u>325</u>	Y	15
Maximum_daily_flow	1490	cubic feet per second	<u>325</u>	Y	15
Std_Dev_of_daily_flows	70	cubic feet per second	<u>325</u>	Y	15
Average_daily_streamflow	19.117	cubic feet per second	<u>41</u>	Y	15
Harmonic_Mean_Streamflow	0.54	cubic feet per second	<u>325</u>	Y	15
Mean_of_Logs_of_Daily_Values	0.569818	Log base 10	<u>325</u>	Y	15
Std_Dev_of_Logs_of_Daily_Values		Log base 10	325	Y	15
Skew_of_Logs_of_Daily_Values		Log base 10	325	Y	15
Non_Zero_Adjusted_Harmonic_Mean_Flow		cubic feet per	325	Y	15
		second			
Base Flow Statistics					
Number_of_years_to_compute_BFI	15	years	<u>42</u>	Y	15
Average_BFI_value	0.301	dimensionless	<u>42</u>	Y	15
Std_dev_of_annual_BFI_values	0.096	dimensionless	<u>42</u>	Y	15

11/30/2020

Probabillity Statistics

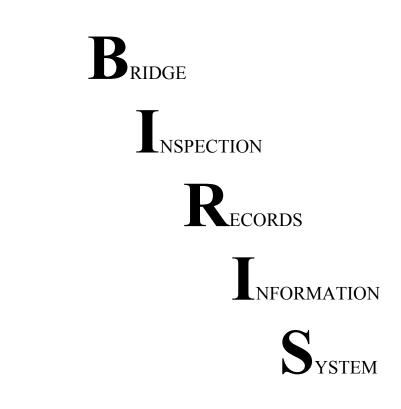
Probability_flow_durations_are_zero	0 20752	dimensionless	325	Y	15
Trobability_liow_durations_are_zero	0.207 52	unnensioniess	323		10

Citations	
Citation Number	Citation Name and URL
31	Imported from Basin Characteristics file
41	Wolock, D.M., 2003, Flow characteristics at U.S. Geological Survey streamgages in the conterminous United States: U.S. Geological Survey Open-File Report 03-146, digital data set
42	Wolock, D.M., 2003, Base-flow index grid for the conterminous United States: U.S. Geological Survey Open-File Report 03-263, digital data set
230	<u>Gotvald, A.J., Barth, N.A., Veilleux, A.G., and Parrett, Charles, 2012, Methods for determining magnitude and frequency of floods in California, based on data through water year 2006: U.S. Geological Survey Scientific Investigations Report 2012 • 5113, 38 p., 1 pl.</u>
325	Granato G.E., Ries, K.G., III, and Steeves, P.A., 2017, Compilation of streamflow statistics calculated from daily mean streamflow data collected during water years 1901 2015 for selected U.S. Geological Survey streamgages: U.S. Geological Survey Open-File Report 2017-1108, 17 p.

Attachment 2: CalTrans As-Built Bridge Plans

California Department of Transportation Division of Maintenance

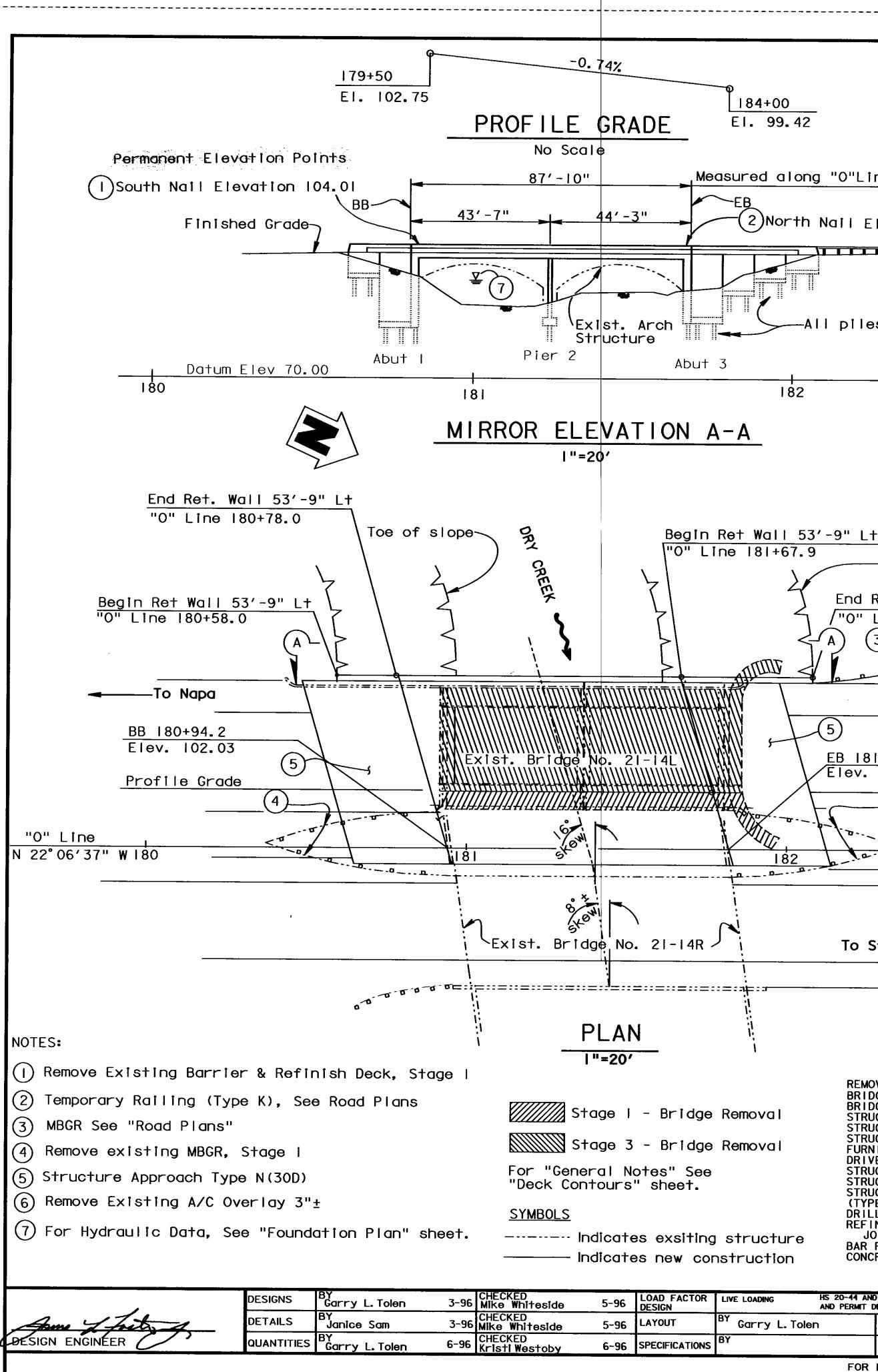
Structure Maintenance and Investigations



The requested documents have been generated by BIRIS.

These documents are the property of the California Department of Transportation and should be handled in accordance with Deputy Directive 55 and the State Administrative Manual.

Records for "Confidential" bridges may only be released outside the Department of Transportation upon execution of a confidentiality agreement.



PILE DATA

Measured along "O"Line

North Nail Elevation 103.41

piles not shown

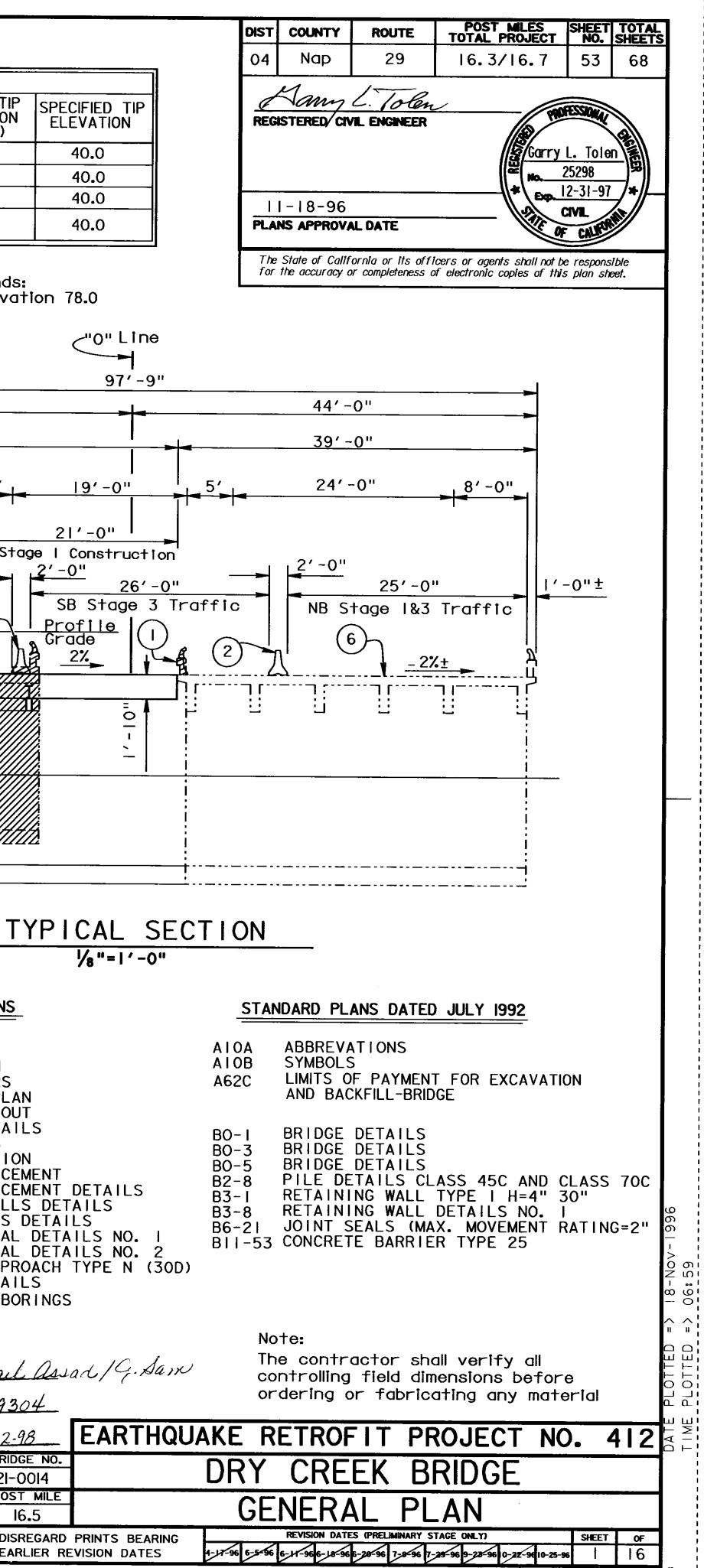
		CLASS 450	C PILE		
LOCATION	DESIGN LOADING	NOMINAL RES	ISTANCE	DESIGN TIP	SPECIFIE
	(SERVICE)	COMPRESSION	TENSION	(a), (c)&(d)	ELEVAT
Abut I	45 tons	180 kips	0 kips	40.0	40.0
Pier 2	45 tons	180 kips	90 kips	40.0	40.0
Abut 3	45 tons	180 kips	0 kips	40.0	40.0
Retaining Wall	45 tons	180 kips	0 kips	40.0	40.0

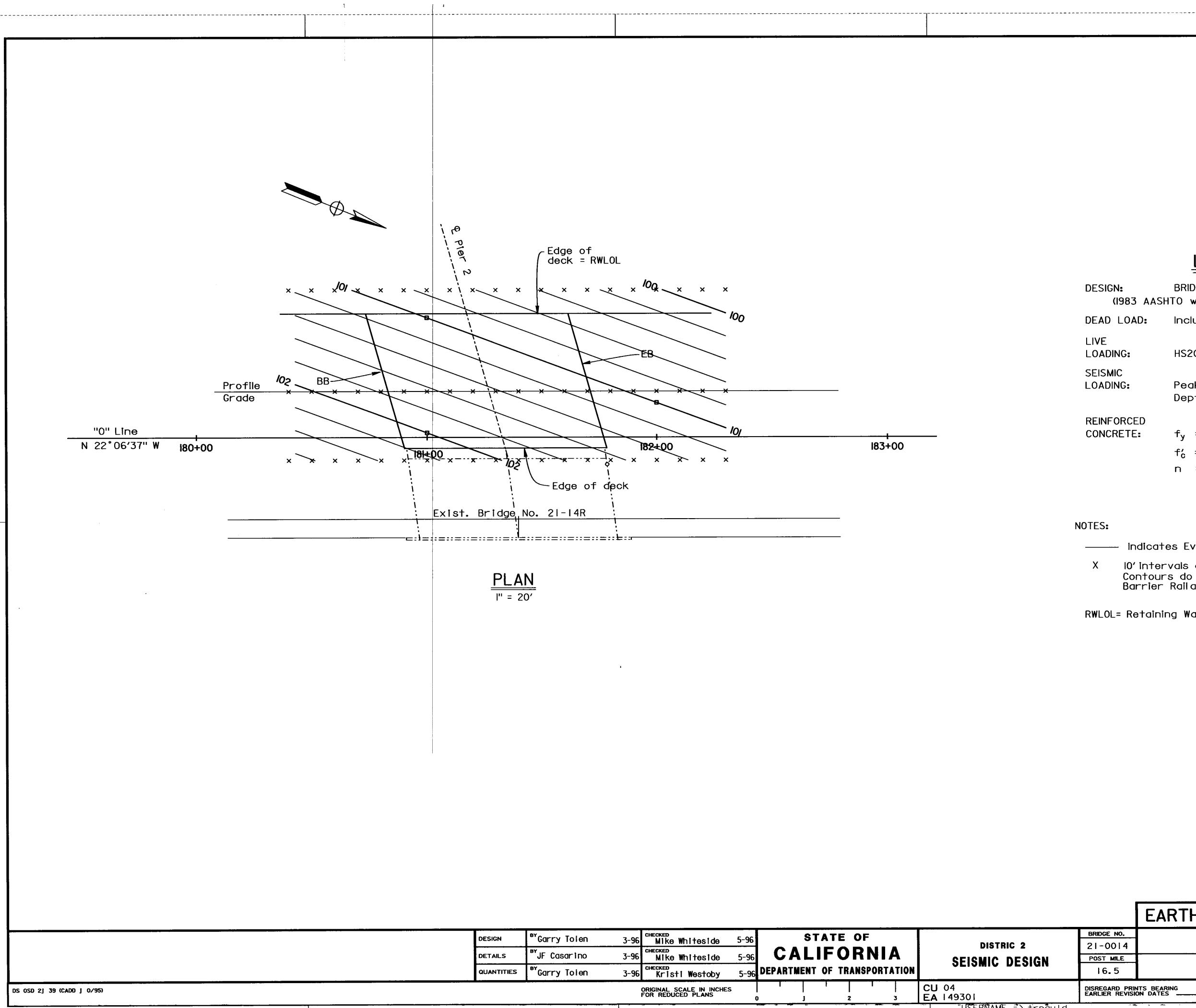
Design tip elevation is controlled by the following demands: (a) Compression; (c) Lateral Loads; (d) Scour potential to Elevation 78.0

53'-9" 182 58'-9" Widen 1'-9' 24'-0" 10'-0" 2'-0" 26'-0" 21'-0" SB Stage | Traffic Stage | Construction 2'-0" Type 25 37'-9" Barrier Stage 3 Construction Railing Top of slope /See Detail "B" Profile "Typical Section" sheet Grade End Ret Wall 53'-9" Lt 2% RC Slab-"O" Line 182+07.9 0Gʻ <u>EB 181+82.0</u> \mathbf{N} Elev. 101.38 (4 19'-0"____ in INDEX TO PLANS SHEET NO. TITLE To St. Helena — GENERAL PLAN DECK CONTOURS FOUNDATION PLAN ABUTMENT LAYOUT ABUTMENT DETAILS PIER DETAILS Ω TYPICAL SECTION SLAB REINFORCEMENT QUANTITIES SLAB REINFORCEMENT DETAILS RETAINING WALLS DETAILS 10. REMOVE ASPHALT CONCRETE SURFACING 353 SQYD MISCELLANEOUS DETAILS BRIDGE REMOVAL ||. LUMP SUM BRIDGE REMOVAL DETAILS NO. BRIDGE REMOVAL (PORTION) 12. LUMP SUM STRUCTURE EXCAVATION (BRIDGE) 128 CY 664 CY 395 CY BRIDGE REMOVAL DETAILS NO. 2 13. STRUCTURE EXCAVATION (TYPE D) STRUCTURE APPROACH TYPE N (30D) 14. STRUCTURE BACKFILL (BRIDGE) DRAINAGE DETAILS FURNISH PILING (CLASS 45C) 15. 2,933 LF DRIVE PILE (CLASS 45C) LOG OF TEST BORINGS 68 EA 16. STRUCTURAL CONCRETE, BRIDGE FOOTING 80 CY STRUCTURAL CONCRETE, BRIDGE STRUCTURAL CONCRETE, APPROACH SLAB 582 CY AS BUILT 127 CY (TYPE N) DRILL AND BOND DOWEL 94 LF 172 SQFT CORRECTIONS BY Michael Assad 19. Sam REFINISH BRIDGE DECK JOINT SEAL (MR/2") BAR REINFORCING STEEL (BRIDGE) II8 LF 128,200 LB 148 LF CONTRACT NO. 0-4-149304 CONCRETE BARRIER (TYPE 25)

3-2-98 DATE 12-08-97 HS 20-44 AND ALTERNATIVE AND PERMIT DESIGN LOAD BRIDGE NO. STATE OF DISTRICT 2 21-0014 IFORNIA CHECKED CAL 5-96 Mike Whiteside SEISMIC DESIGN POST MILE PLANS AND SPECS COMPARED DEPARTMENT OF TRANSPORTATION 16.5 FOR REDUCED PLANS CU 04 EA 149301 DISREGARD PRINTS BEARING ORIGINAL SCALE IS IN INCHES EARLIER REVISION DATES USERNAME => trpauld

 $DCN EHE = \sum [ESC_0E]aaap0 pc$





ClibPDF - www.fastio.com

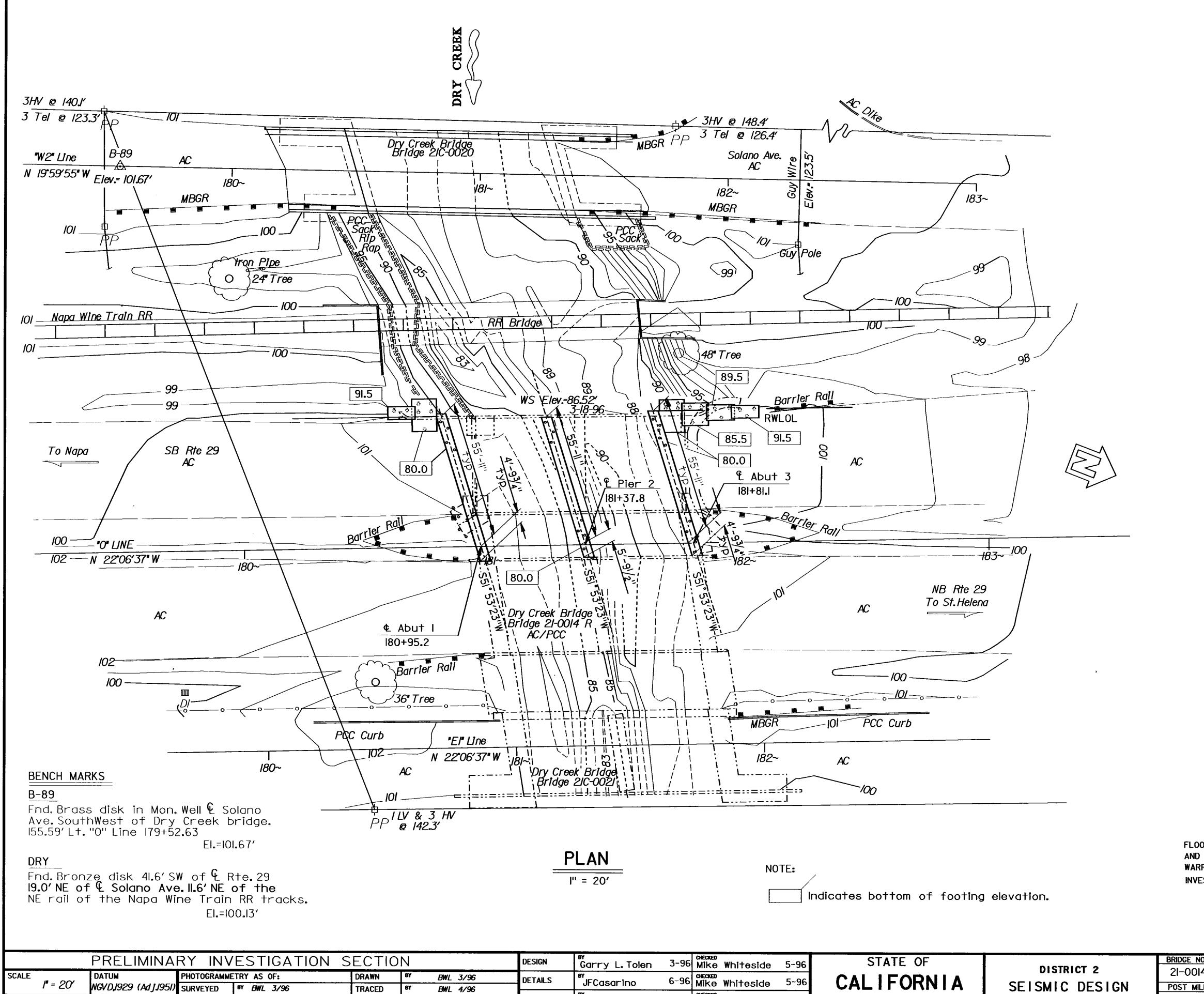
_ _ _ _ _ _ _ _

											EARTH	QUA
	^{BY} Garry Tolen	3-96	снескер Mike Whiteside	5-96	STA	TE	OF			BRIDGE NO.		
 c	BY JF Casarino	3-96	CHECKED			FΟ	RN	ΙΔ	DISTRIC 2	21-0014		
J			Mike Whiteside	5-96					SEISMIC DESIGN	POST MILE		
ITIES	^{BY} Garry Tolen	3-96	Kristi Westoby	5-96	DEPARTMENT O		ANSPUK	TATION		16.5		
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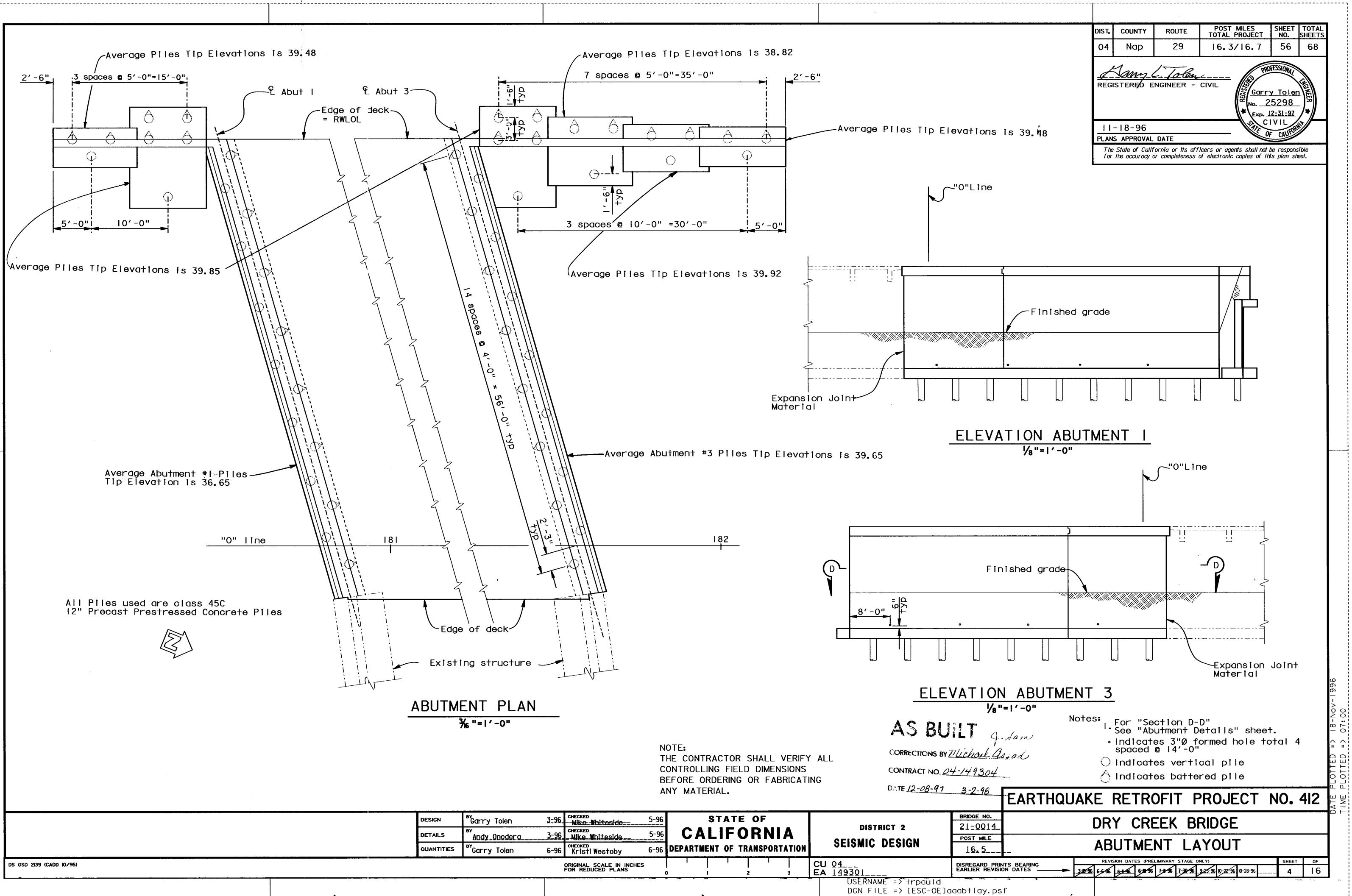
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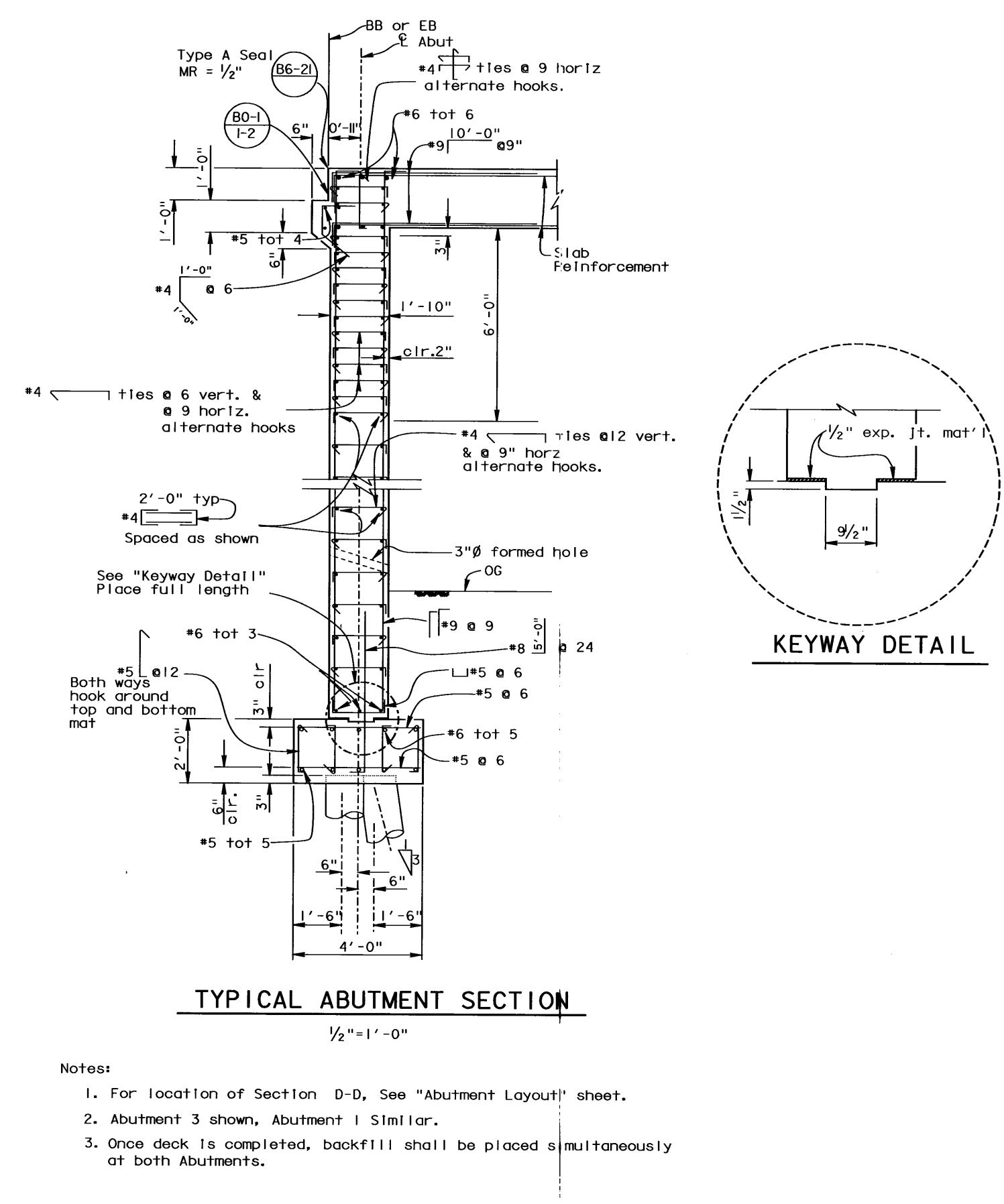
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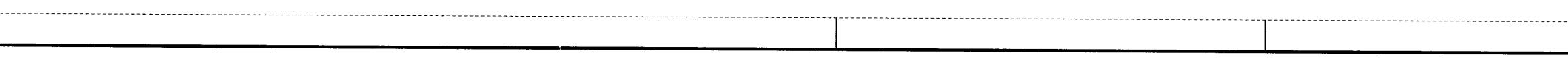
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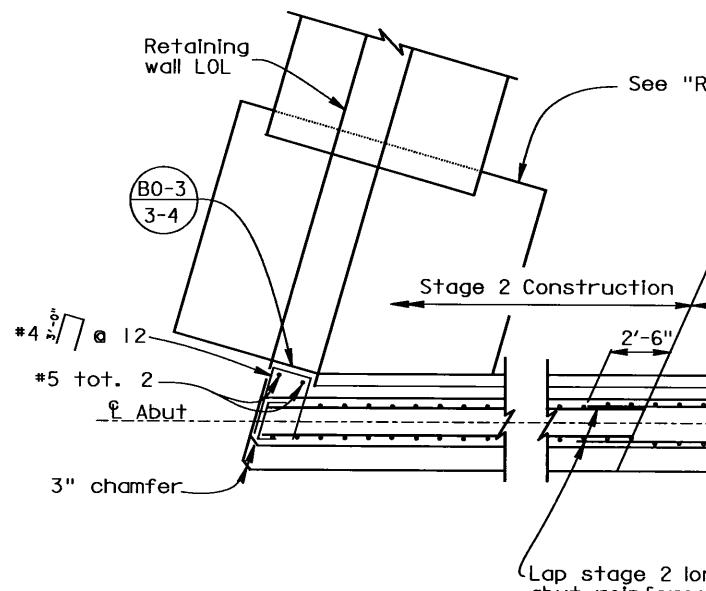
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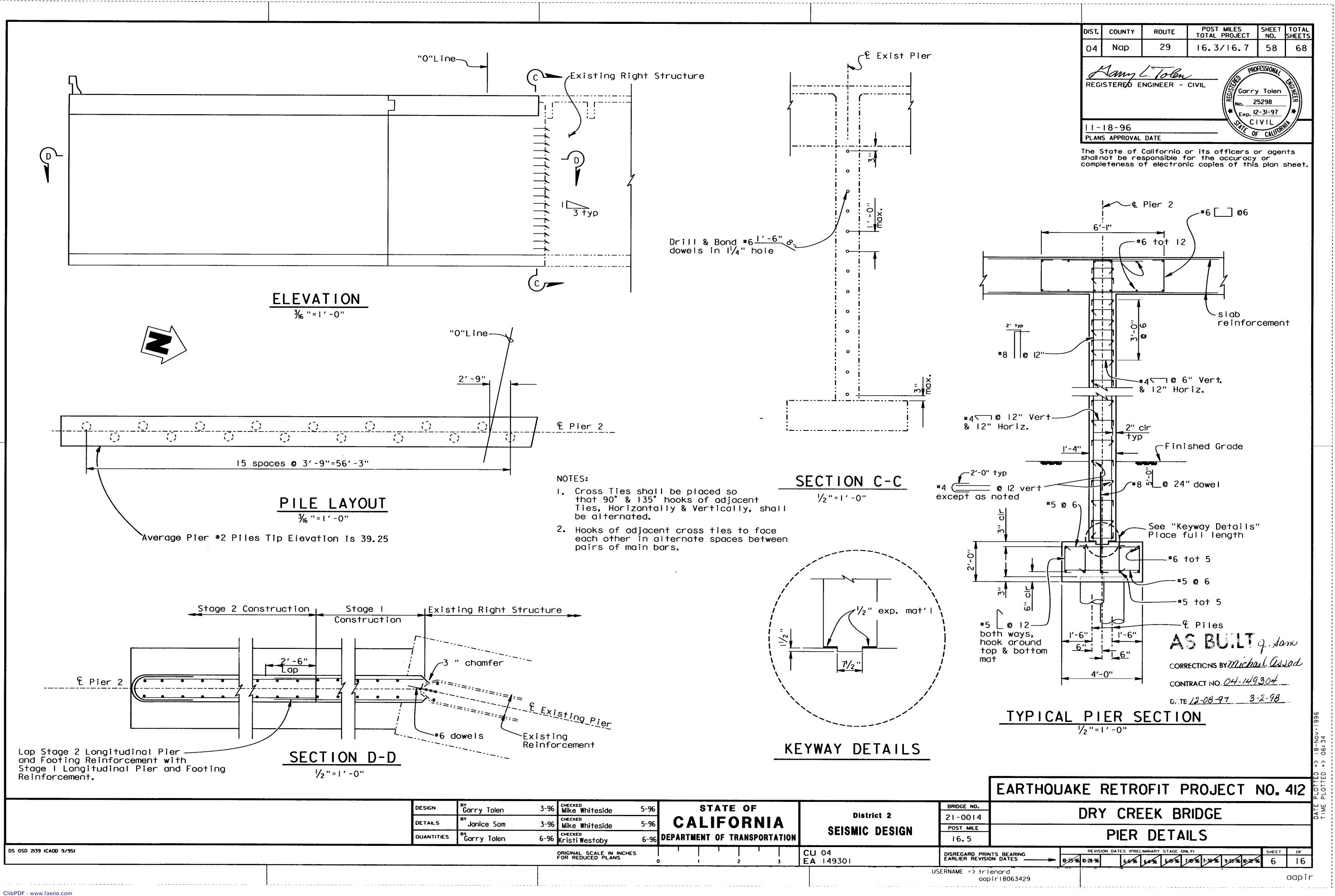


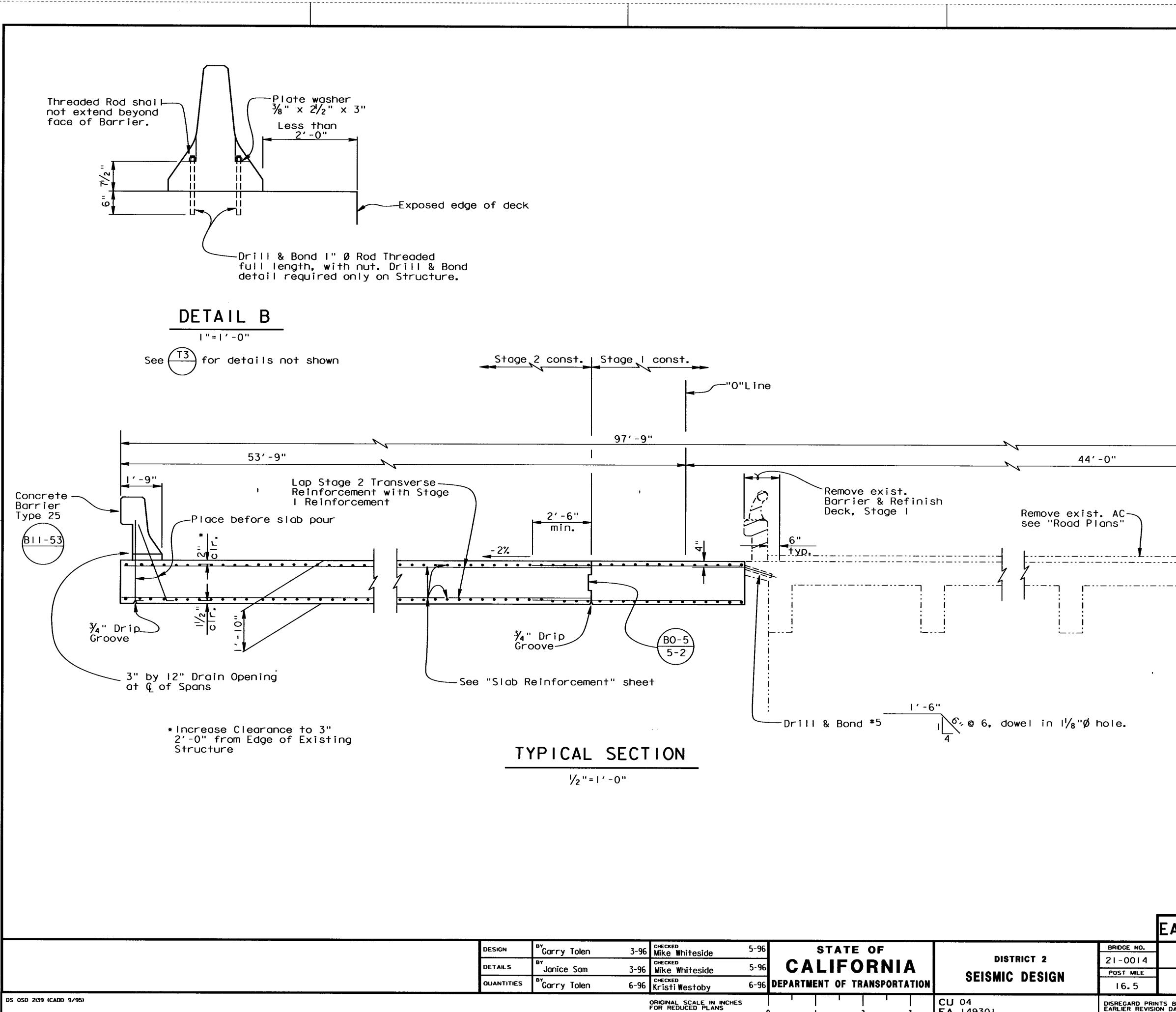
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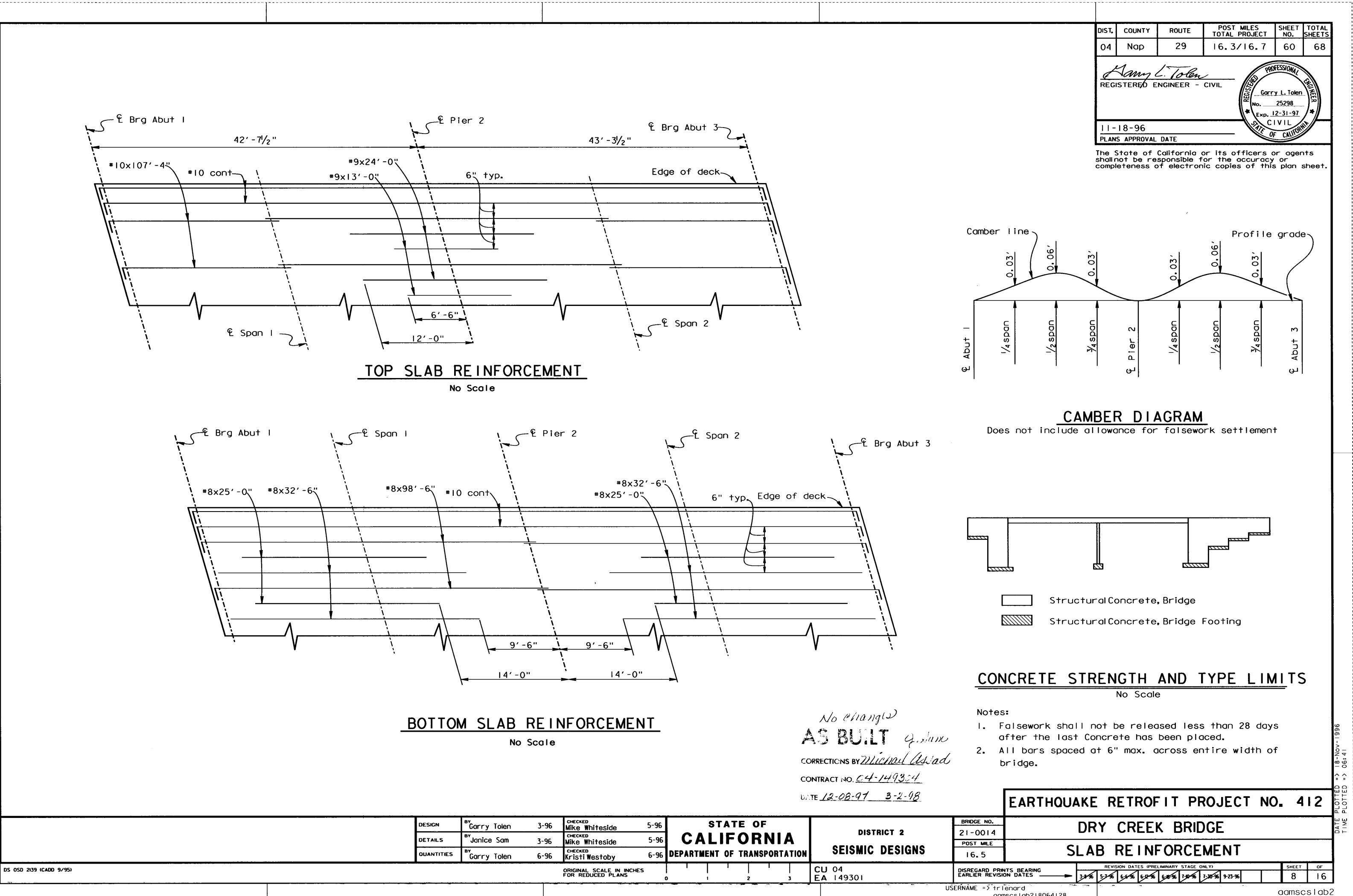


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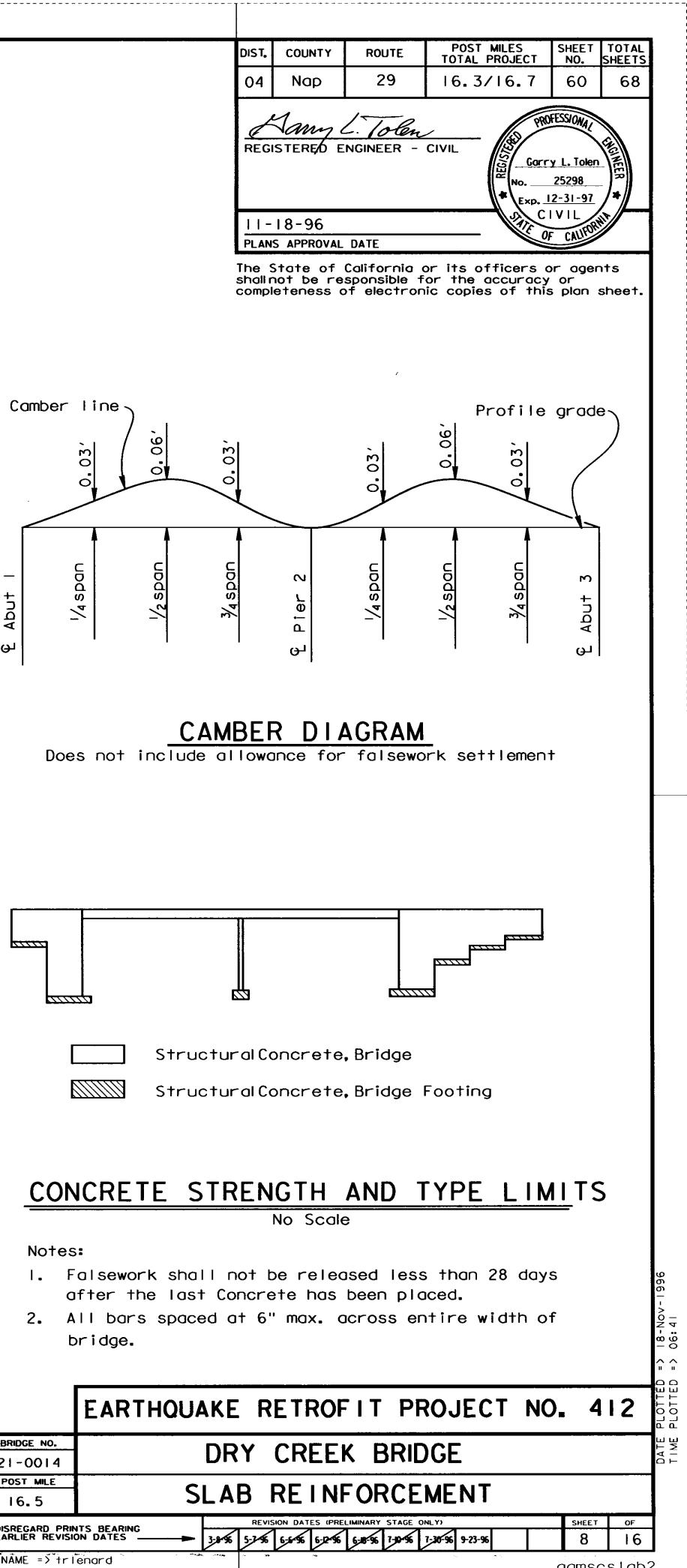
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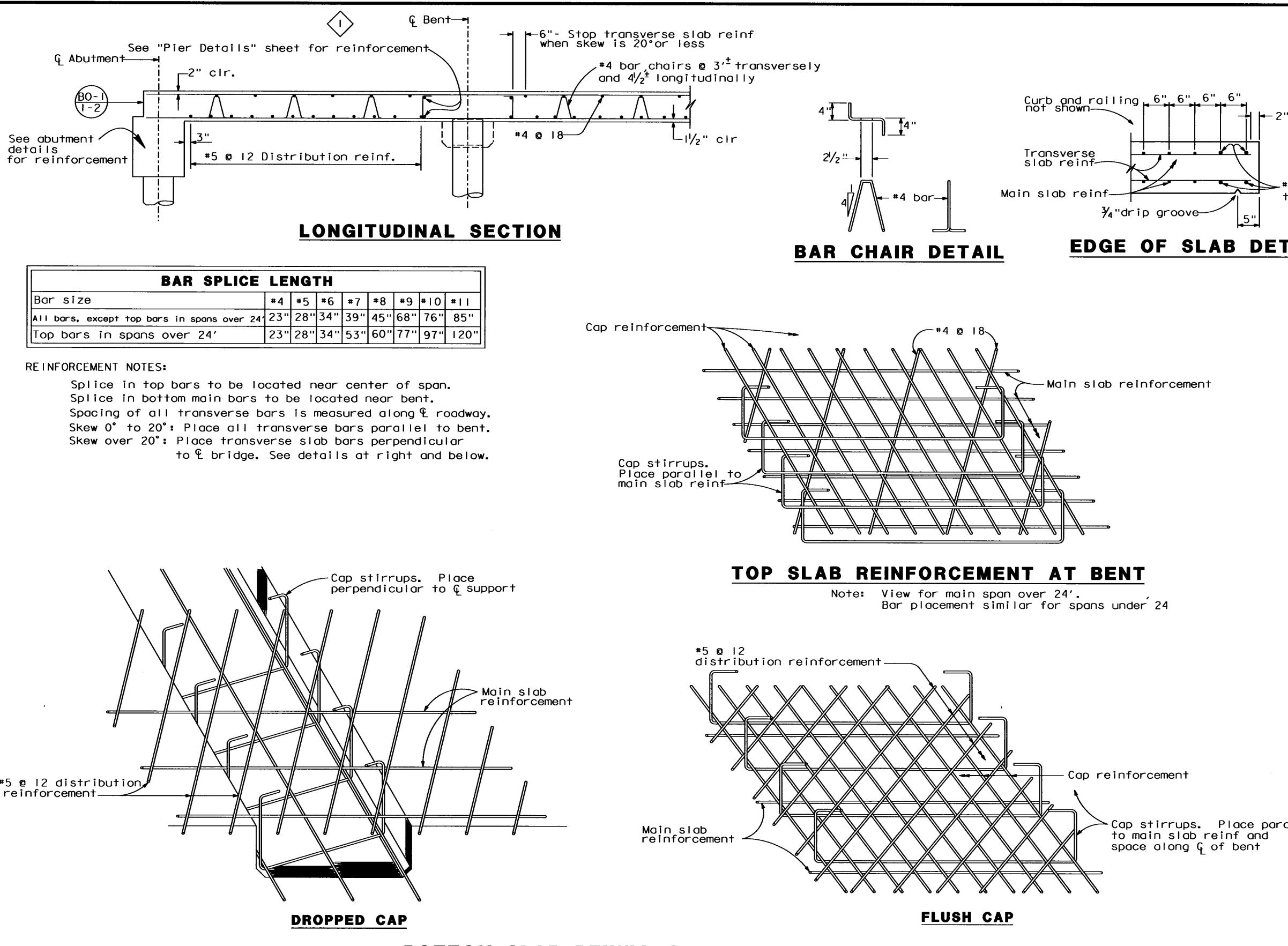
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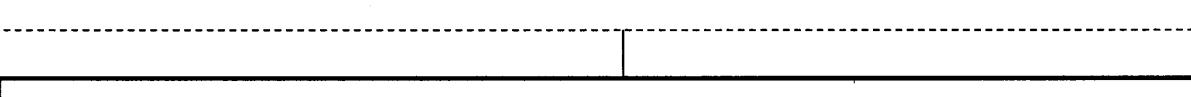
BAR SPLICE LENGTH												
Bar size	#4	#5	# 6	#7	#8	#9	# 0	#				
All bars, except top bars in spans over 24'	23"	28''	34"	39"	45"	68"	76"	85"				
Top bars in spans over 24'	23"	28"	34"	53"	60"	77"	97"	120"				

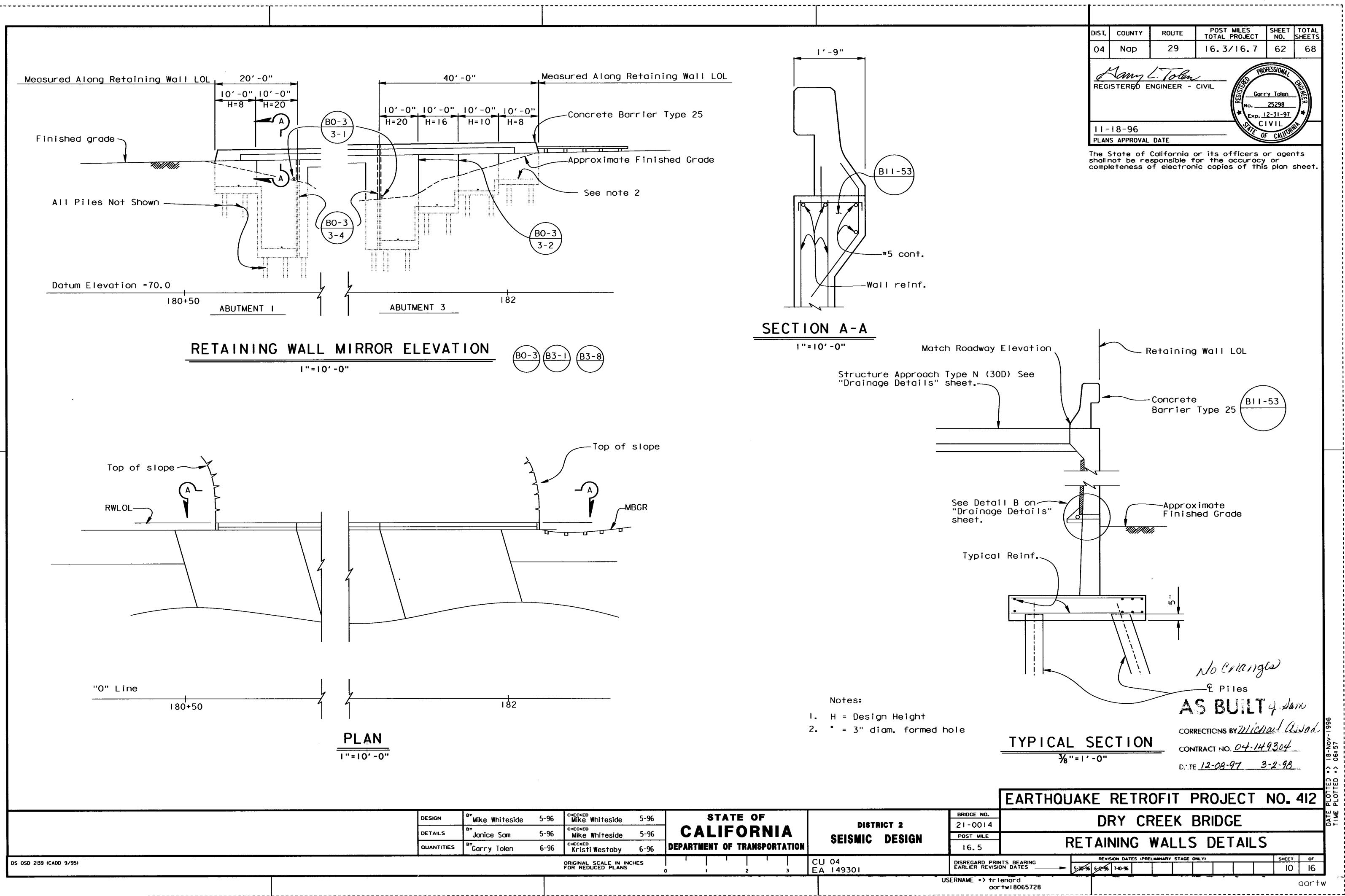
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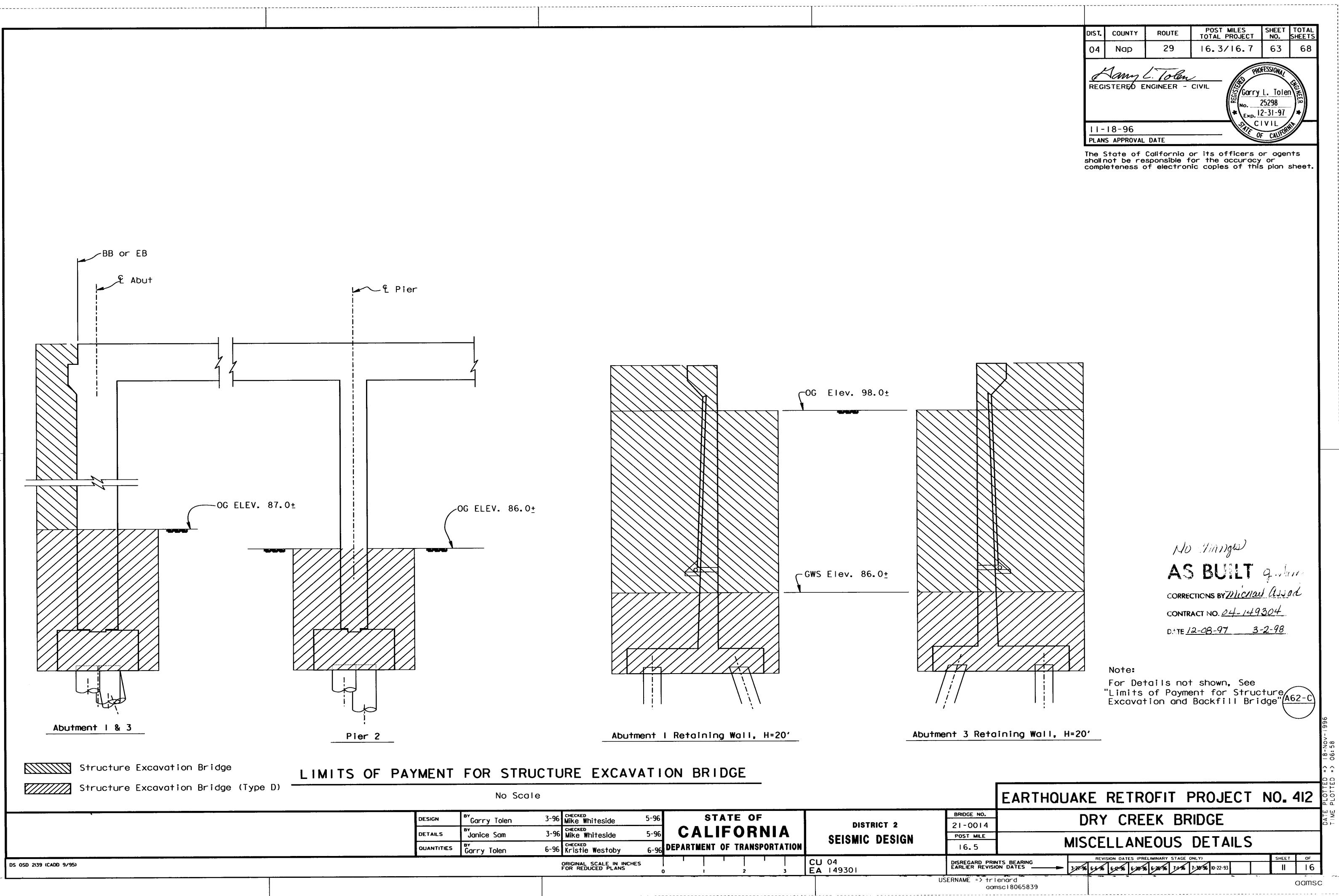
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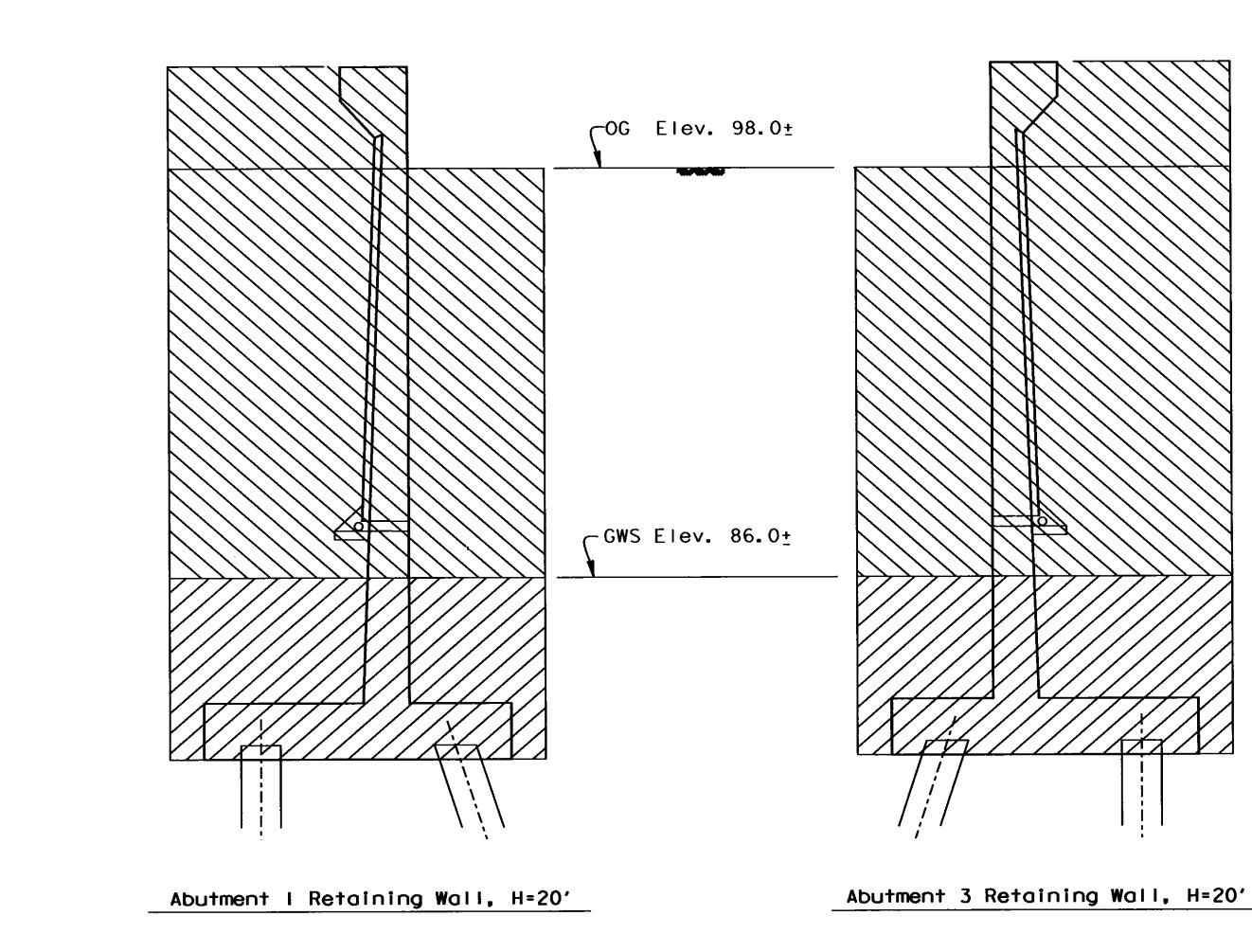


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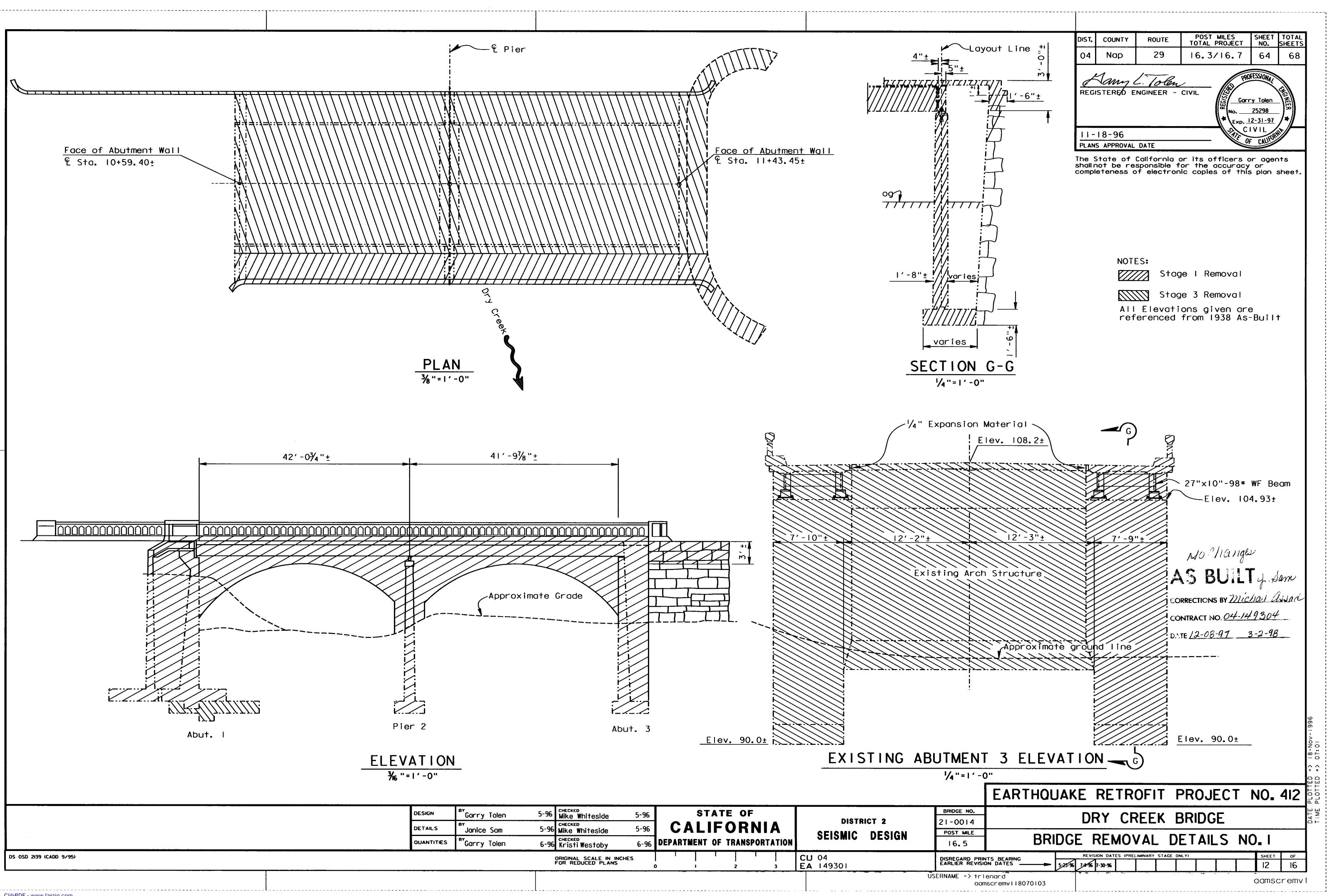


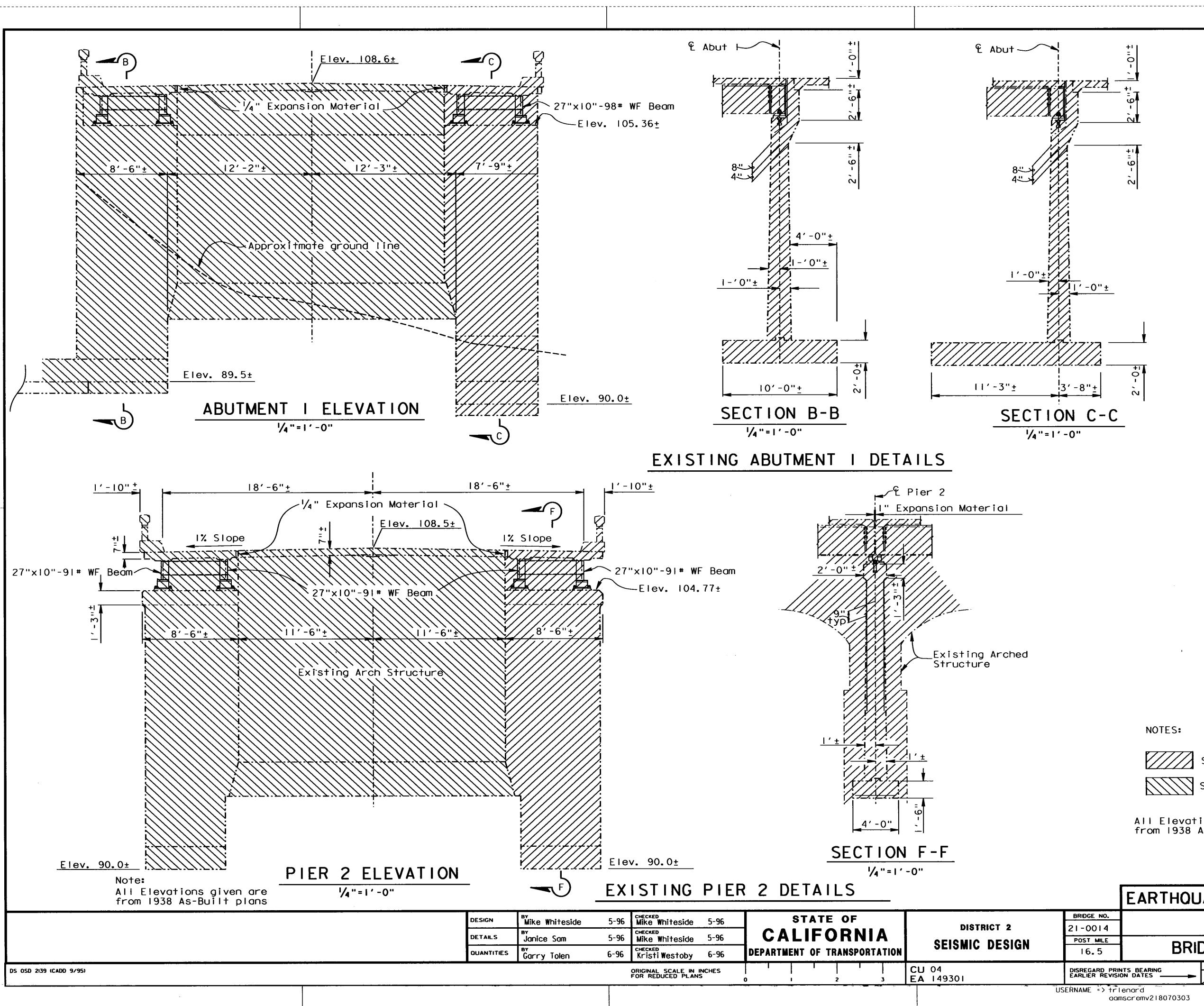
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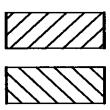




DIST,	COUNTY	ROUTE	POST MILES TOTAL PROJECT	SHEET NO.	TOTAL SHEETS				
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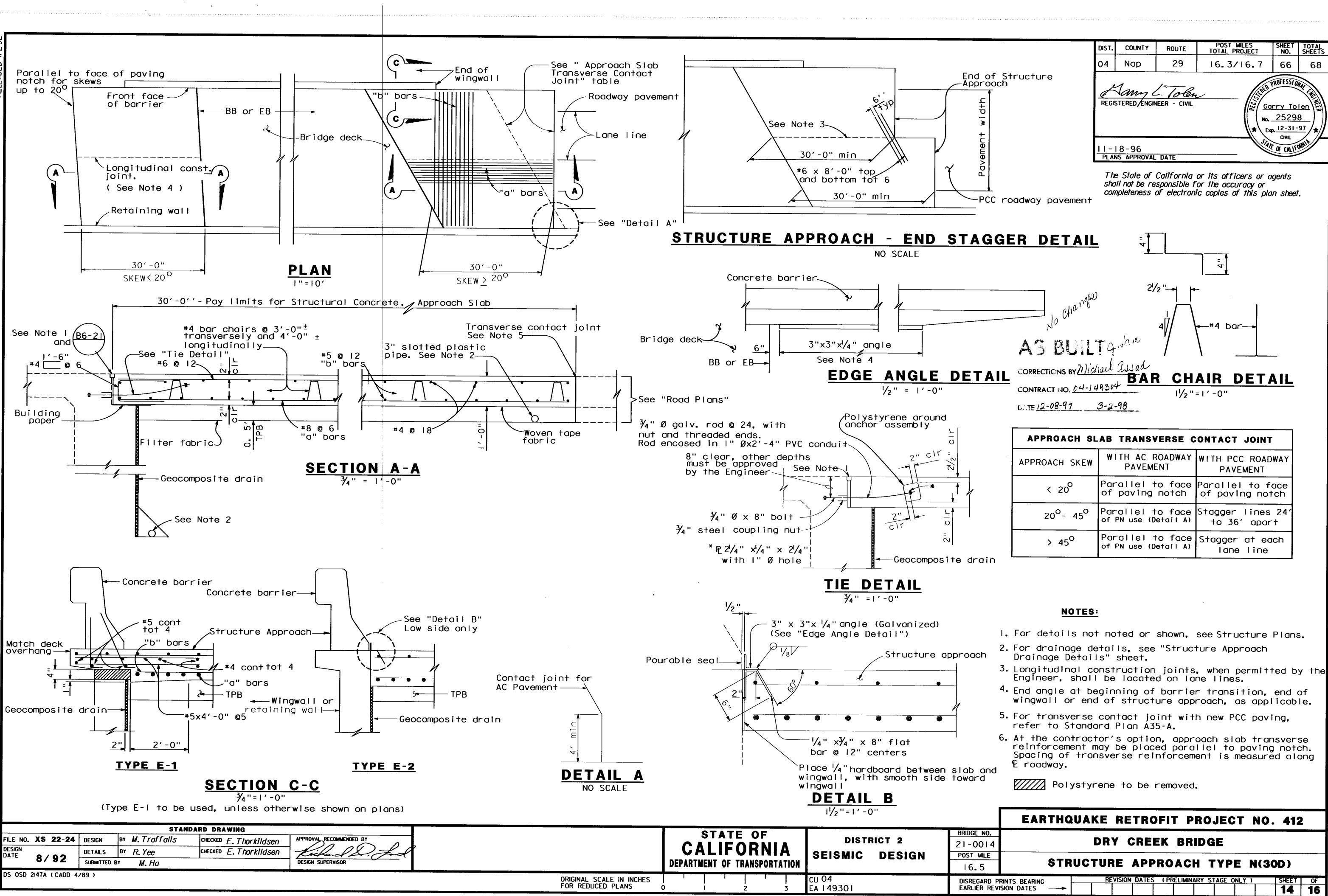
All Elevations given are from 1938 As-Built plans

Stage I Removal No (Manger) Stage 3 Removal ASBUILT 9. Janu

CORRECTIONS BY Michael asiad CONTRACT NO. 04-149304 DATE 12-08-91 3-2-98

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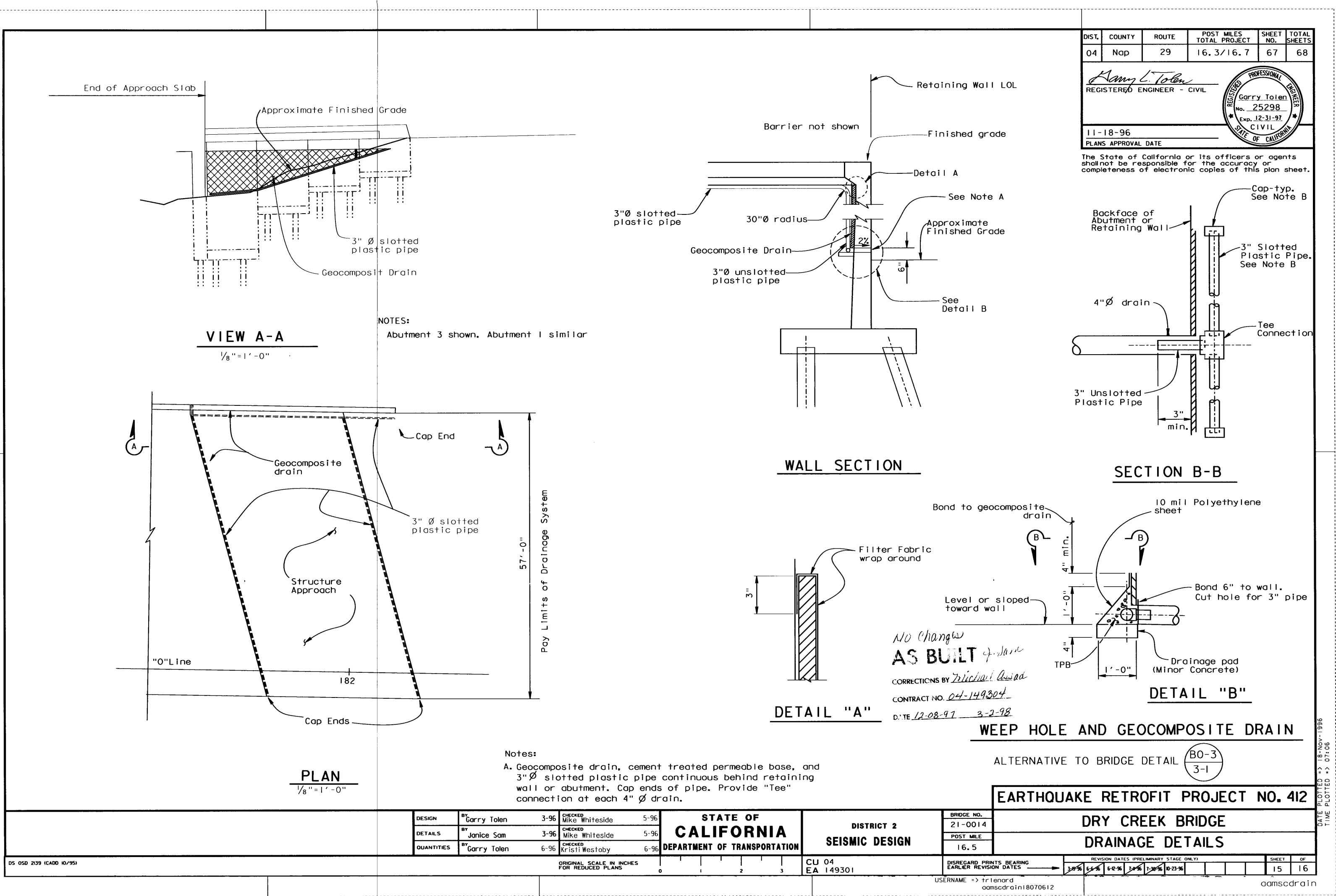
shall not be responsible for the accuracy or completeness of electronic copies of this plan sheet.

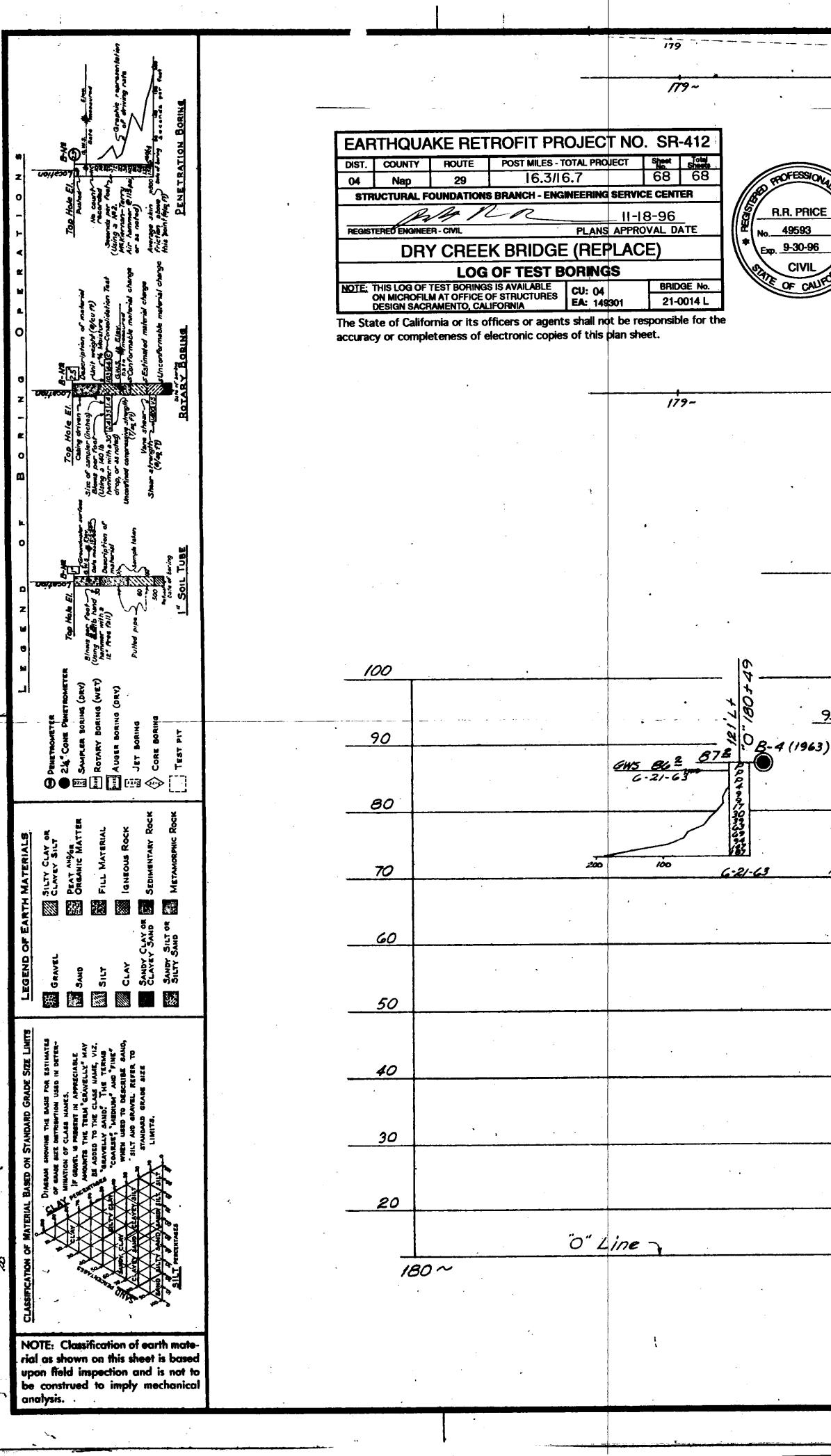
APPROACH SLAB TRANSVERSE CONTACT JOINT										
APPROACH SKEW	WITH AC ROADWAY PAVEMENT	WITH PCC ROADWAY PAVEMENT								
< 20 ⁰	Parallel to face of paving notch	Parallel to face of paving notch								
20 ⁰ - 45 ⁰	Parallel to face of PN use (Detail A)	Stagger lines 24' to 36' apart								
> 45 ⁰	Parallel to face of PN use (Detail A)	Stagger at each Iane line								

wingwall or end of structure approach, as applicable.

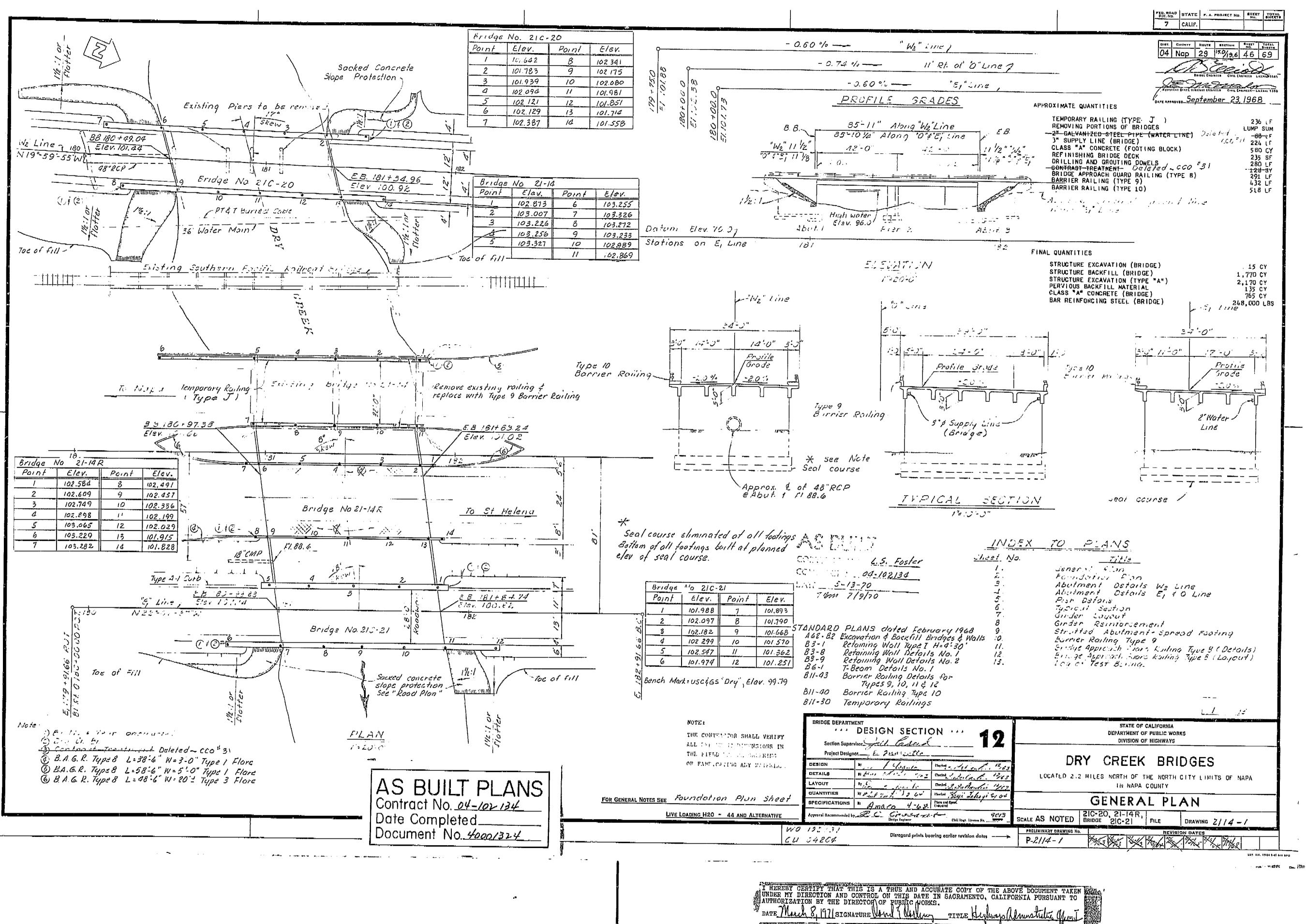
EARTHQUAKE RETROFIT PROJECT NO. 412

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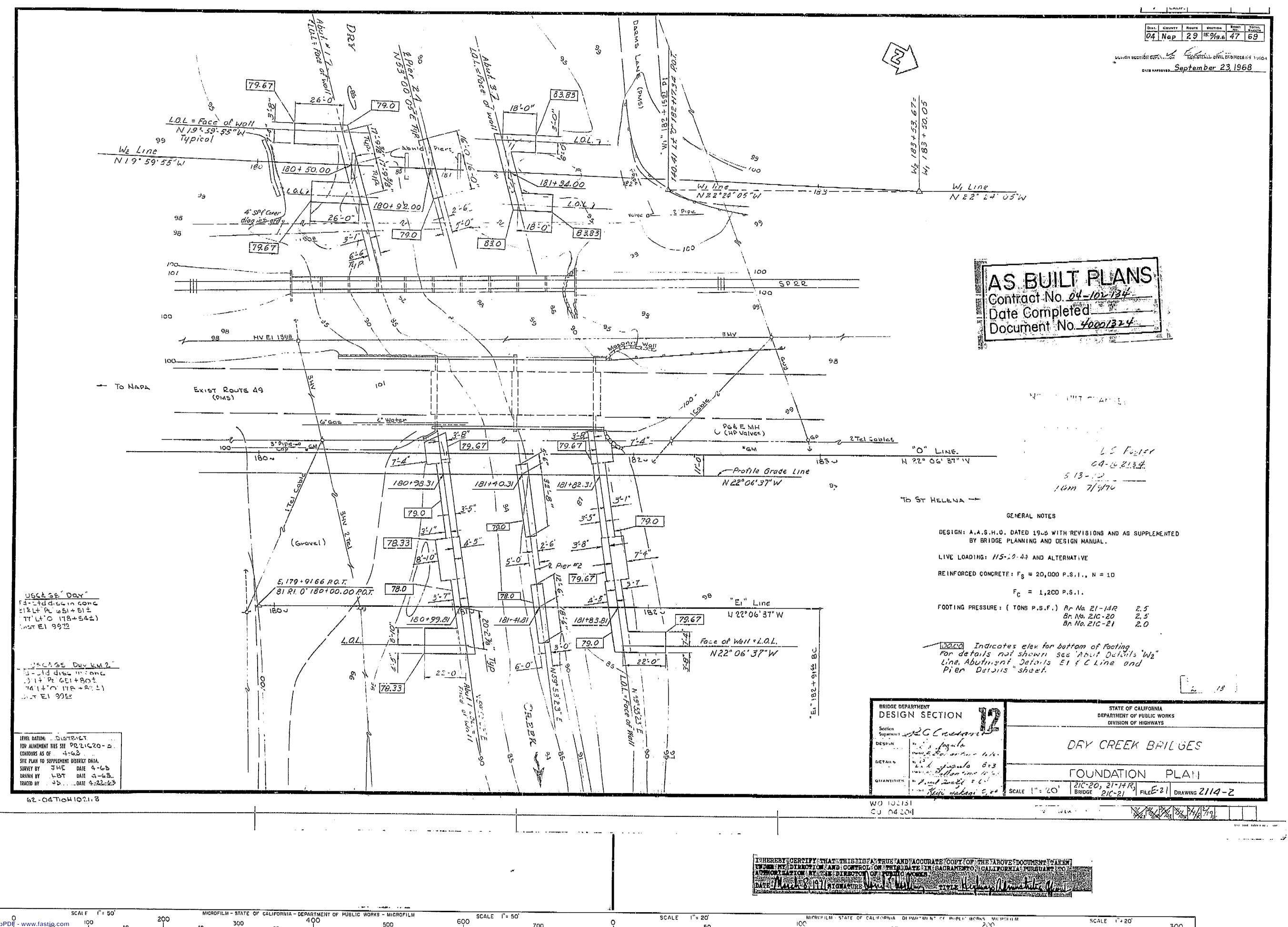


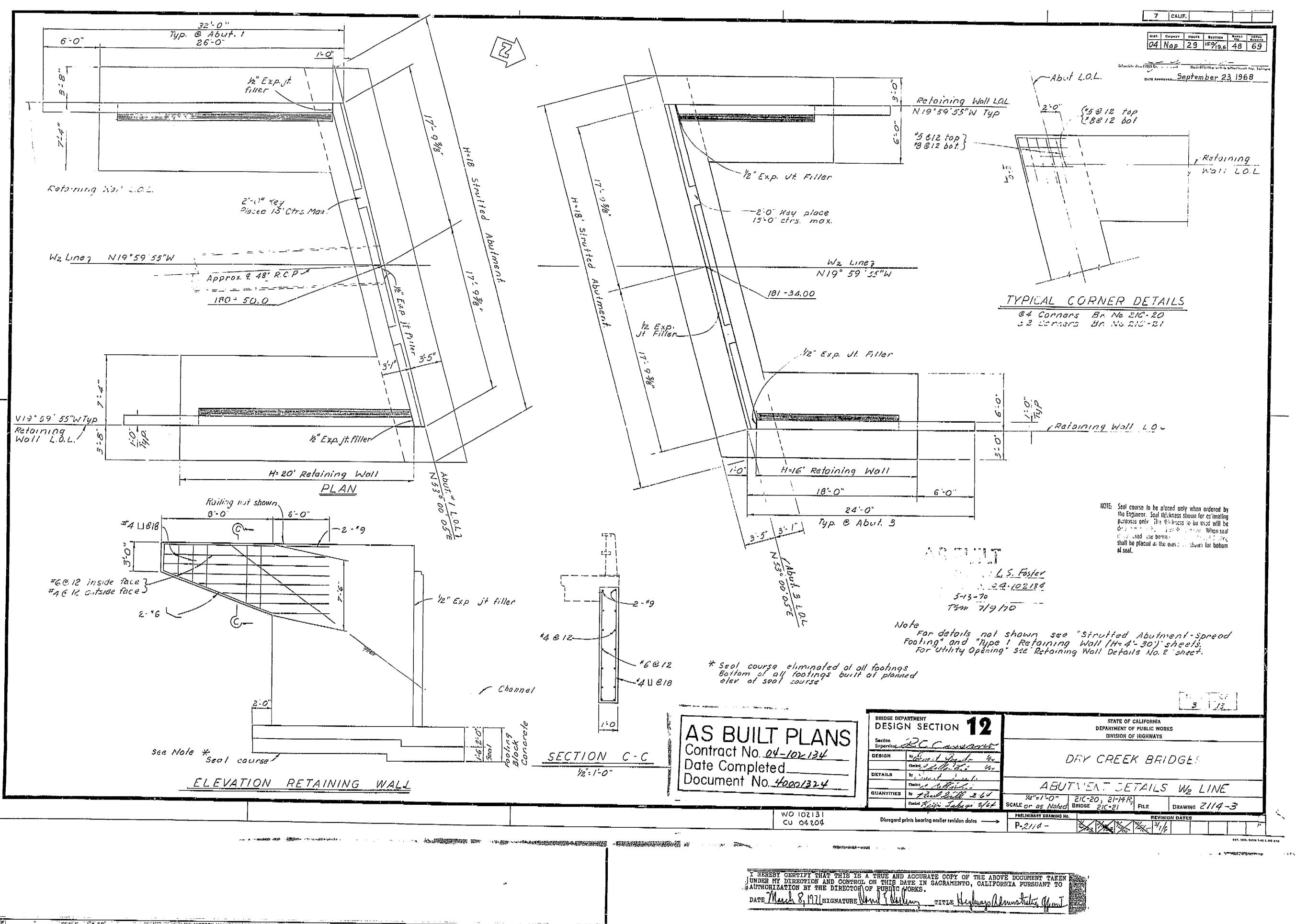


7 CAUF. 04 Nap 29 966 We Line? "W," Line . 183 ----181~ 25 183~ 182~ 180~ B-4 (1963) September 23, 19 DATE AN < <u>B.M.</u> USCEGS "Dry Sta disc in concrete 77'L+ "D" 178 + 54 ± No change. Elev. 99.79 "O" Line 2 AS BUILT 181~ 183~ 182~ 180~ B-2 (1957) B-1(1957) B-3 CORRECTIONS BY Michael asy (1957) CONTRACT NO. 04-149304 No As- Built Changes DATE 12-08-91 3-2-98 AS DUILT CORRECTIONS BY C.S. foster (1963) B-3 B-2 (1\$63) E, Line 181~ CONTRACT NO. 04-102134 182~ 180~ DATE______S-13-70 <u>PLAN</u> Scale: 1": 30" 100 B-3(1957) B-1 (1963) sightly compact B-2(1357) B-1(1957) Compart to Very dense Very dense Very dense Very dense Very dense Loose grey cobble and pebble gravel Dense brown gravel silt, silt, silty gravel Ko B-3(1963) 25 Loose brown B-2 (1963) Compact to very dense sand, gravel, & cobbles. 010 to compact 87: brown silt. Gws 851 86 85° 250 Very dense yellow-brown Dortially iron cemented 80 1000 sandy silt E some small 8-4-57 36 1.4-2 - 21 gravel 6-20-63 6-20-63 Compoct red brown Silve and sondy silt. claver silt-bound Very dense ycitow-brown alamp, iron stained sandy silt e small gravel. 250 sand & small grovel. Refusq/ 8-4-57 people graver. 401.4 Slightly compact to compact brown sandy silt with some Dense to very dense plus yellow - brown silt. 12 Kin Loose to slightly 70 1 14 compact brown Reflixe scottered grovel.-8-4-57 silty sand and Loose brown silt, clayey silt, and silty clayey sand with some small _ Talta gravel. 17774 60 Dense to compost gravelly silt 26 Ax brown silty lenses. gravel. Slightly compact brown interbedded layers of medium sand and silty sand gravel. 50 Compact brown silt. 22 04 107 69 78 108 Stiff brown plastic & 6-21-63 111 Tank organic silt. 6.20-63 Very still to hard REEX brown & tan clayey 30 silt with scattered 1 2 24 cloyey gravel lenses. HOLE X 6-20-63 20 · SHEET STATE OF CALIFORNIA т. Та DEPARTMENT OF PUBLIC WORKS 181~ 182~ DIVISION OF HIGHWAYS CREEK BRIDGES DRY PROFILE Scale: 1" = 10' LOG OF TEST BORINGS SCALE AS Noted BRIDGE 21C-21, FILE DRAWING 2/ 14 - 14 WO 102131 PREL DRAWING NO. PR-2/1450 % Disregard prints bearing earlier number 1.40 CU 04204



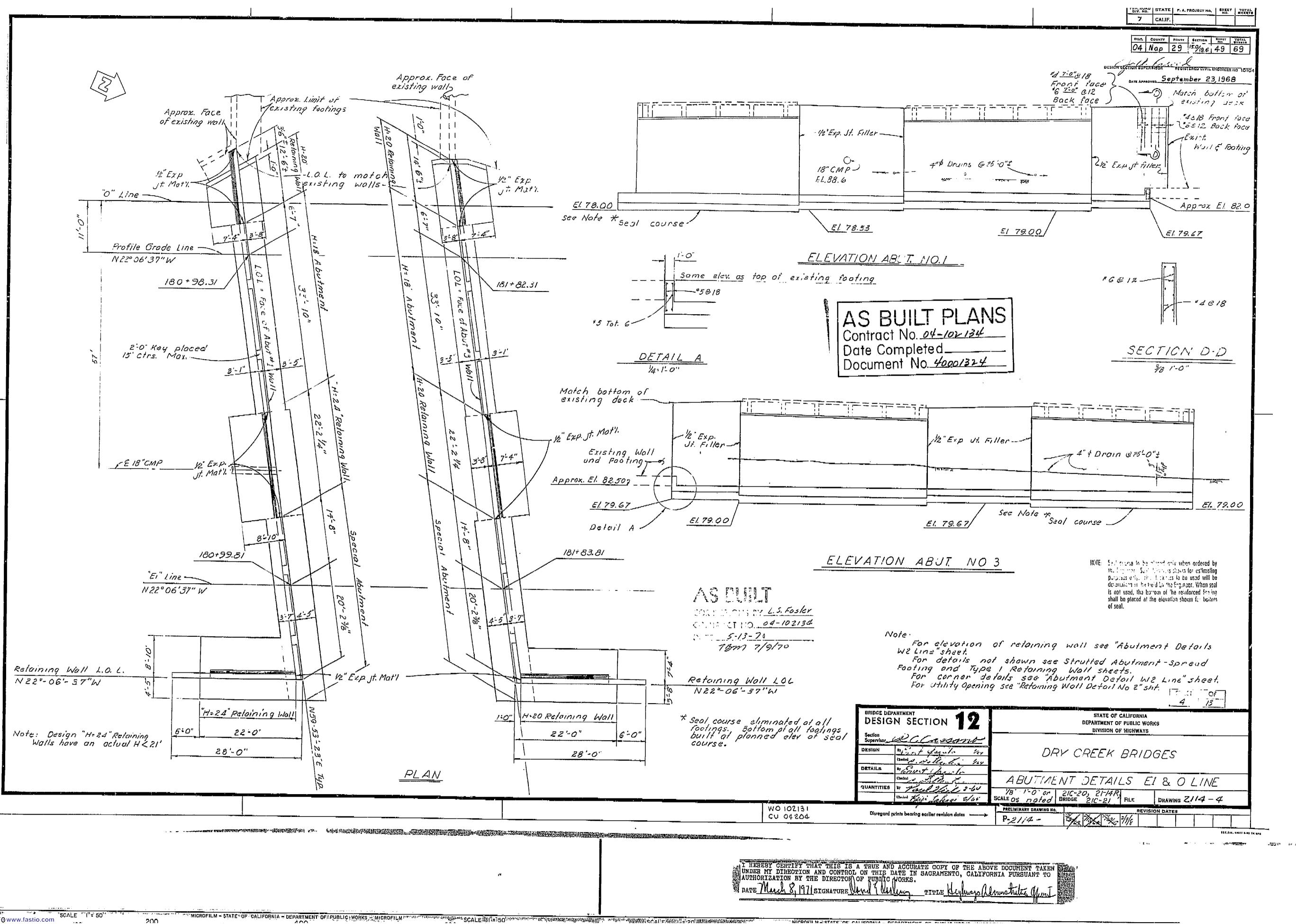
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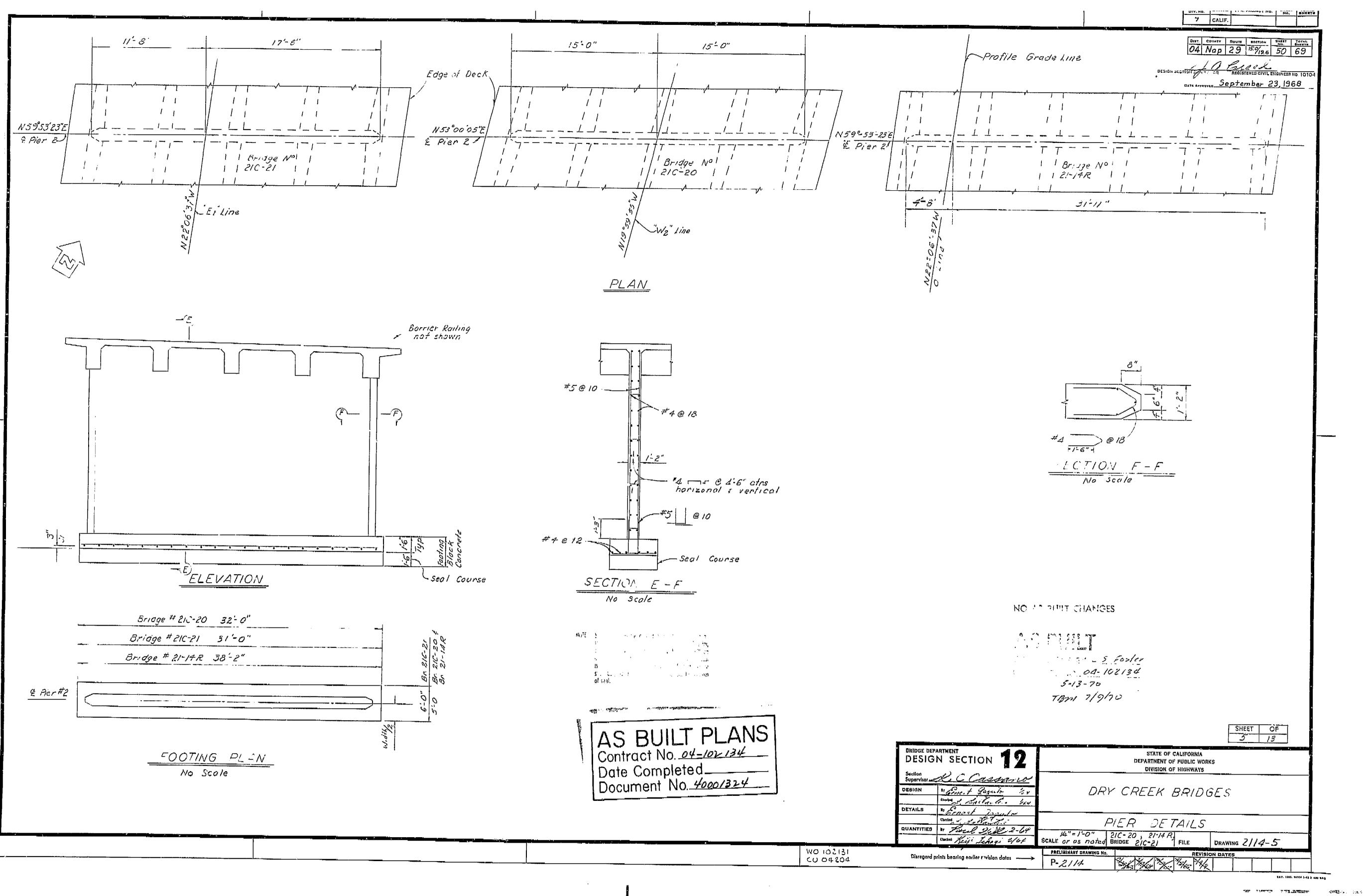
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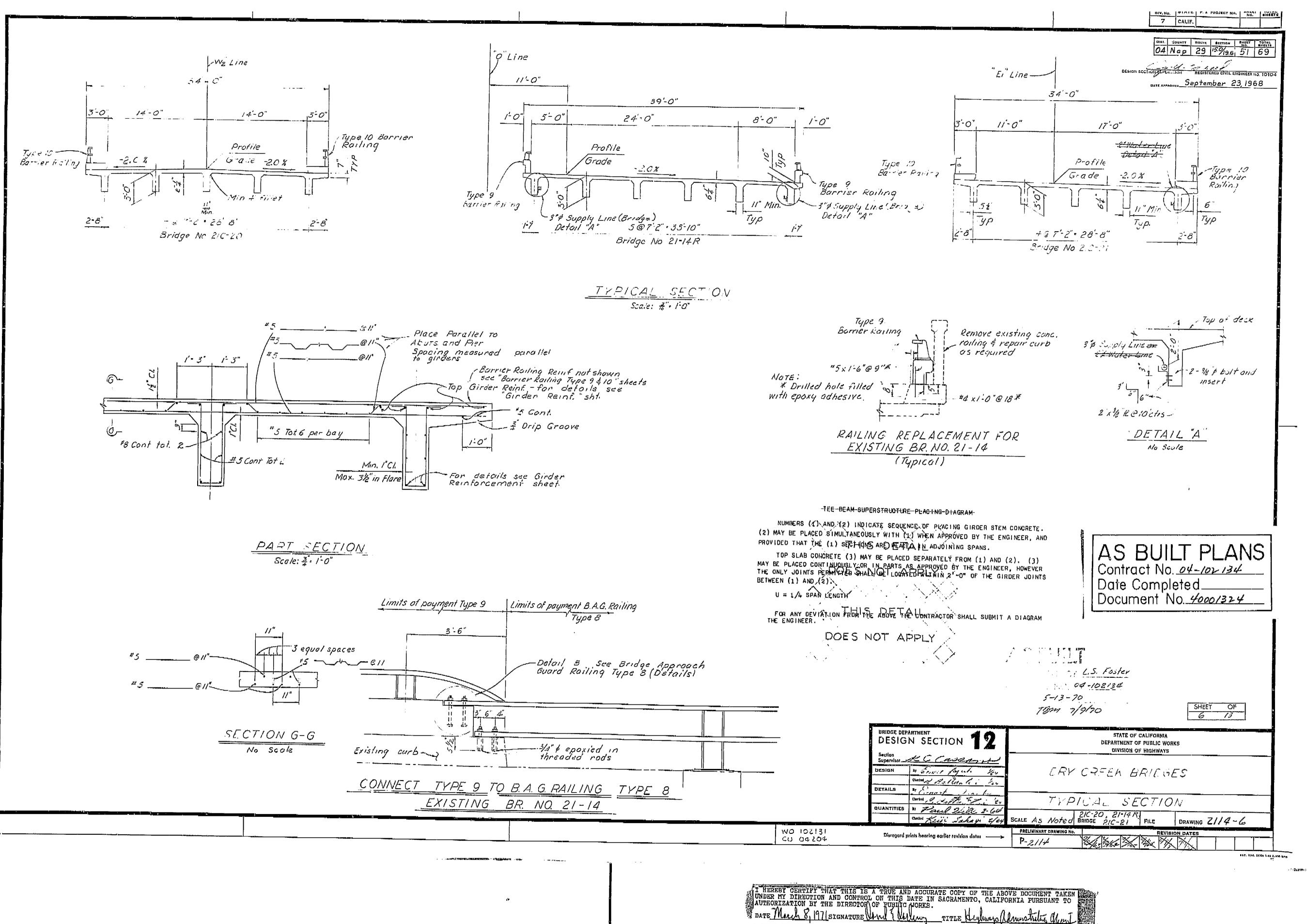


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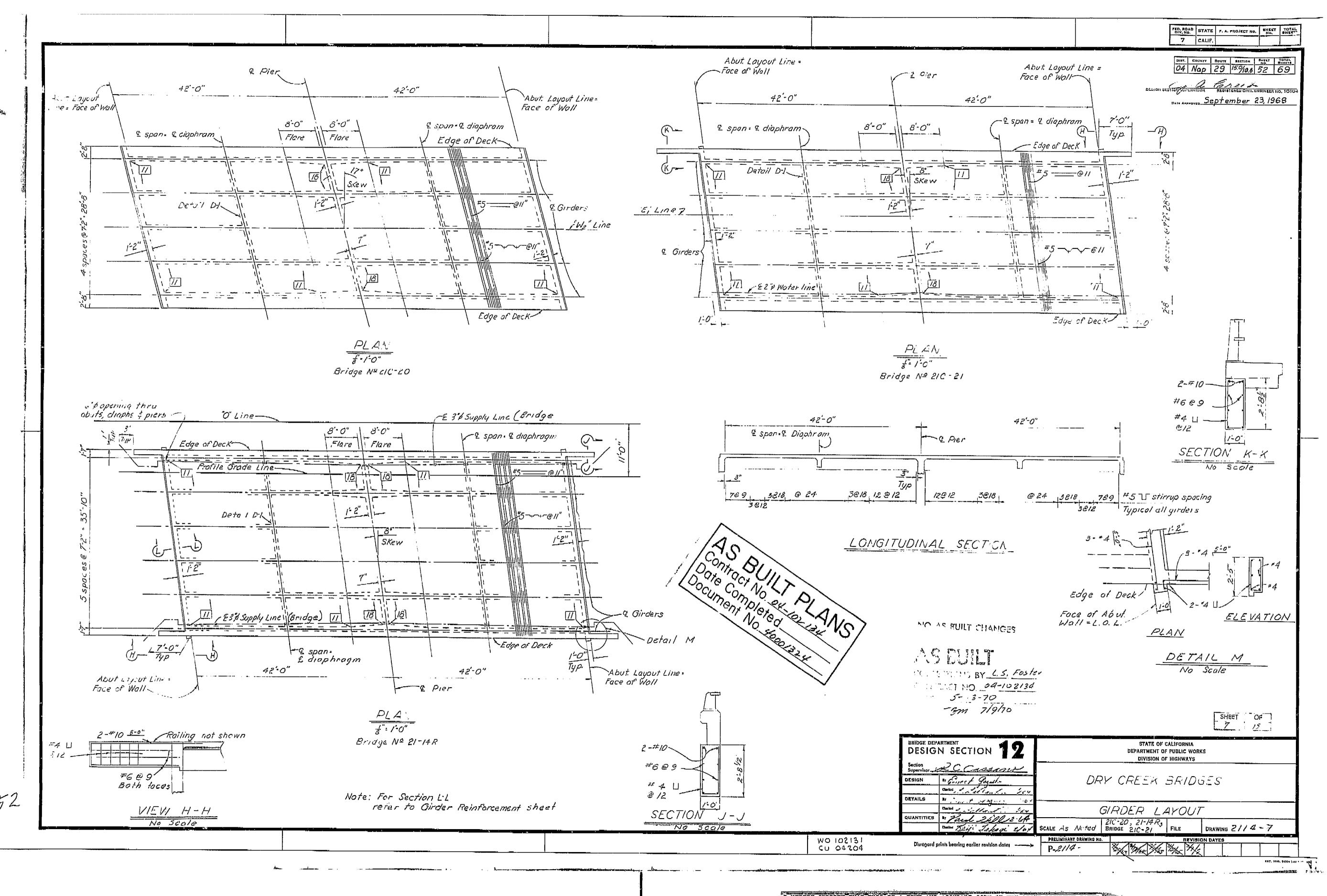
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ment No. 4000/324		DESIGN Br Grine, + Gagaile 1/4 V DESIGN Br Grine, + Gagaile 1/4 V DETAILS Br Grinest Justice 1/4 V	DRY CREEK B
		QUANTITIES & Farral State 2-64 (backed Keips Jaka gi 2/64	PIER DETA 44"=1"-0" 21C+20, 21-14 R, SCALE or 05 noted BRIDGE 21C-21
	WO 102131 CU 04204	Disregard prints bearing earlier rivision dates	PREI WINARY TRAWING No.

I HEREBY CERTIFY THAT THIS IS A TRUE AND ACCURATE COPY OF THE ABOVE DOCUMENT TAKEN UNDER MY DIRECTION AND CONTROL ON THIS DATE IN SACRAMENTO, CALIFORNIA PURSUANT TO AUTHORIZATION BY THE DIRECTOR OF PUBLIC WORKS. DATE MALL 8, 1971 SIGNATURE UNIT WILLING TITLE HUMMAN Admintation (Muni-



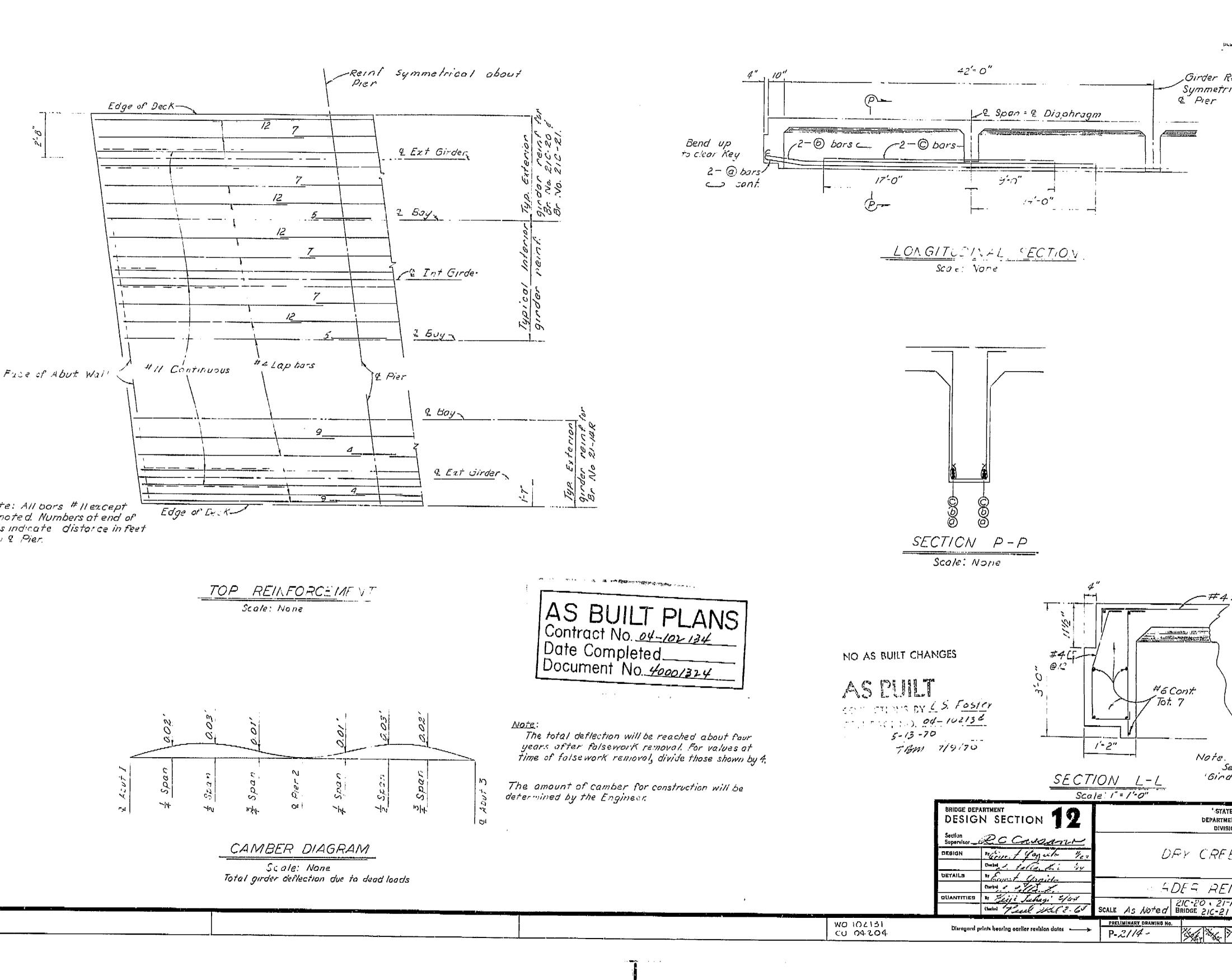
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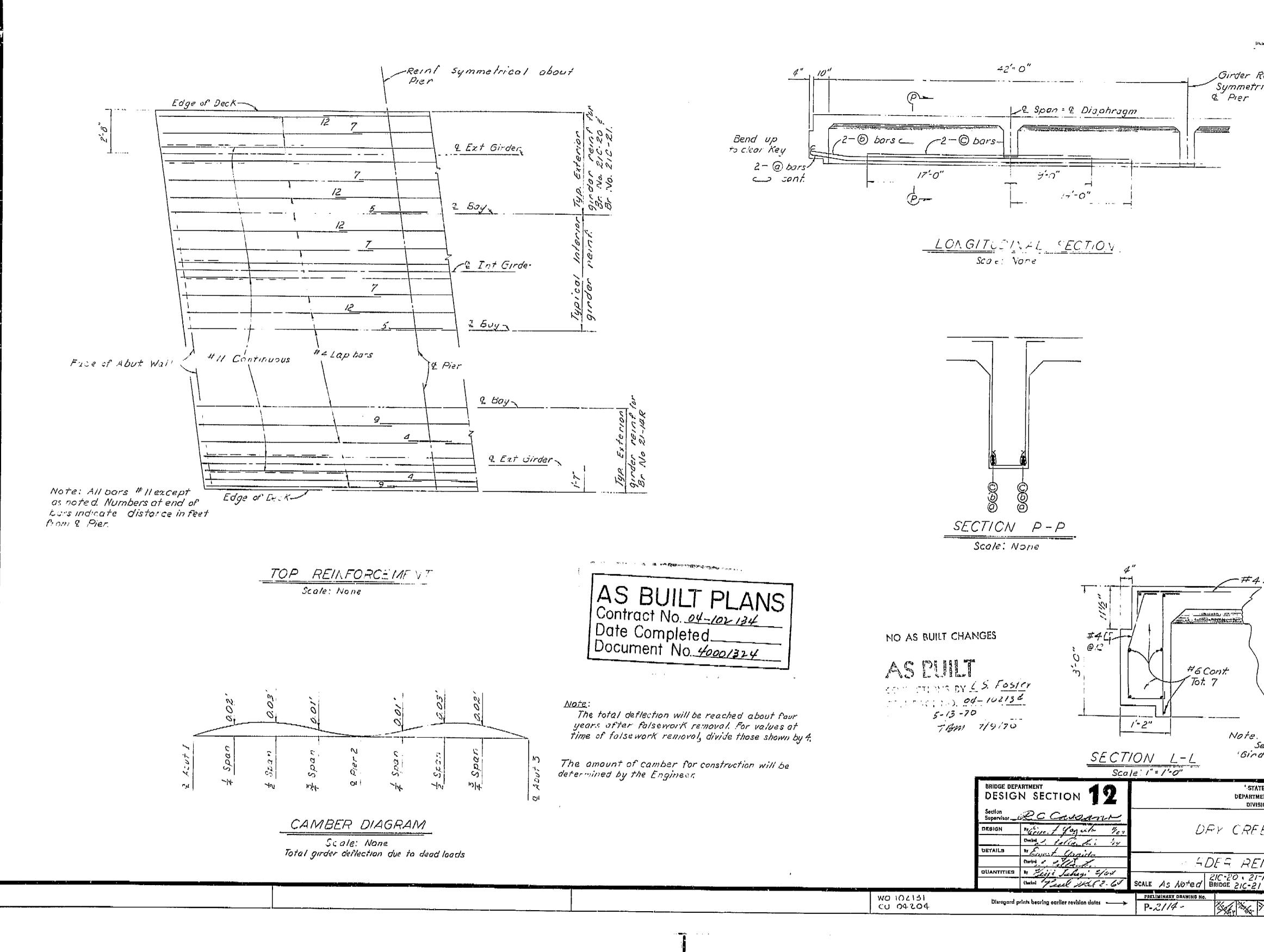
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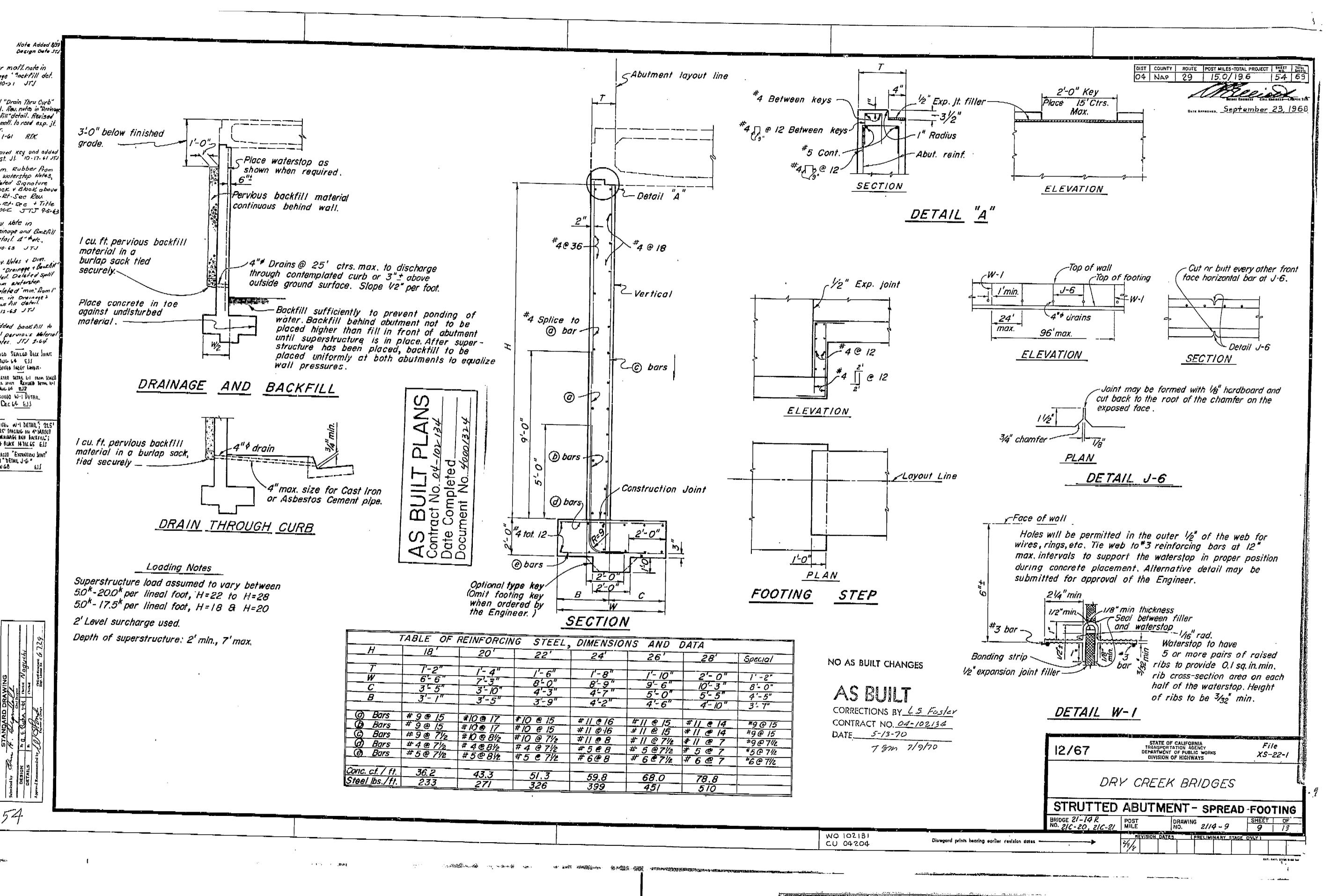
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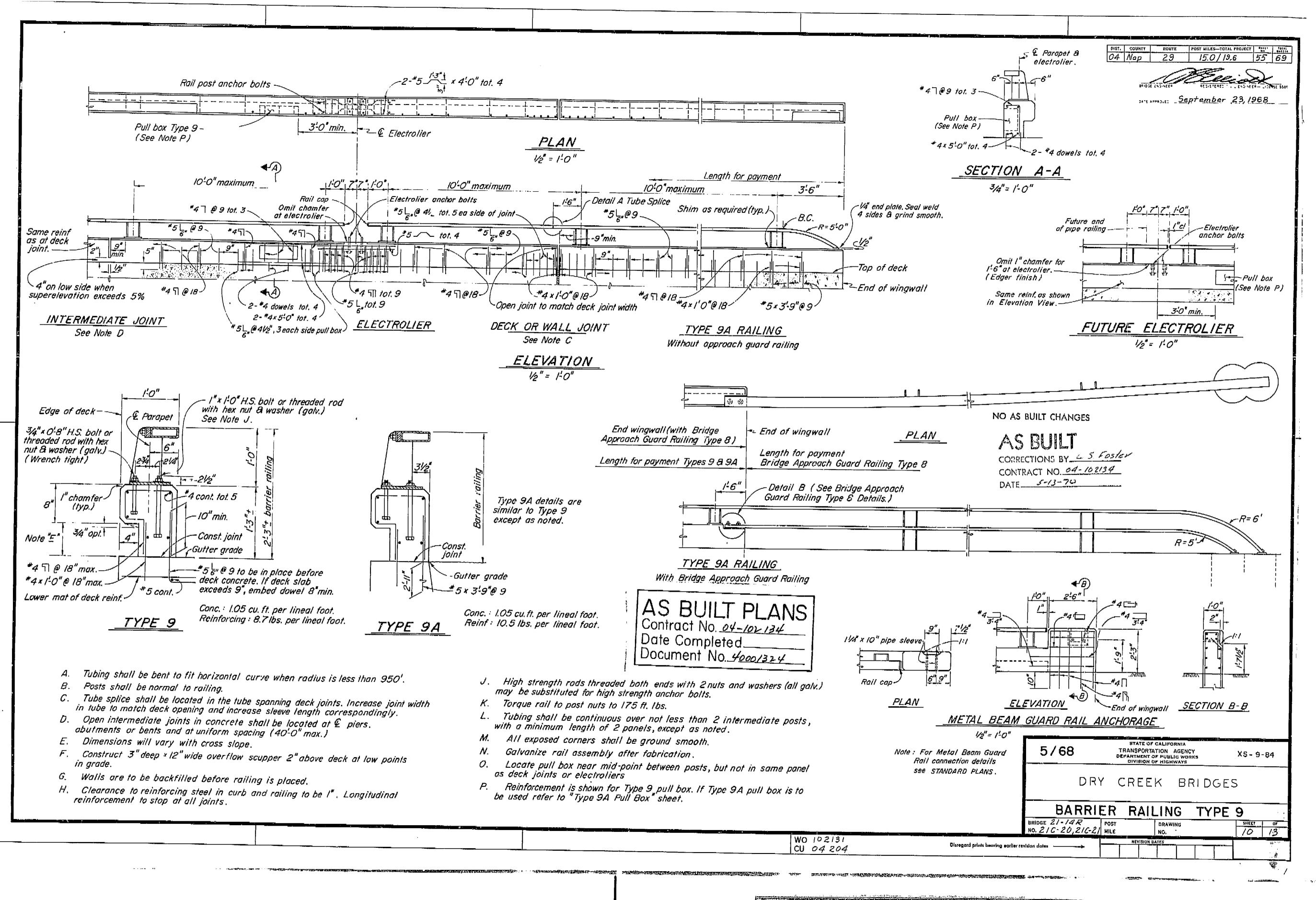
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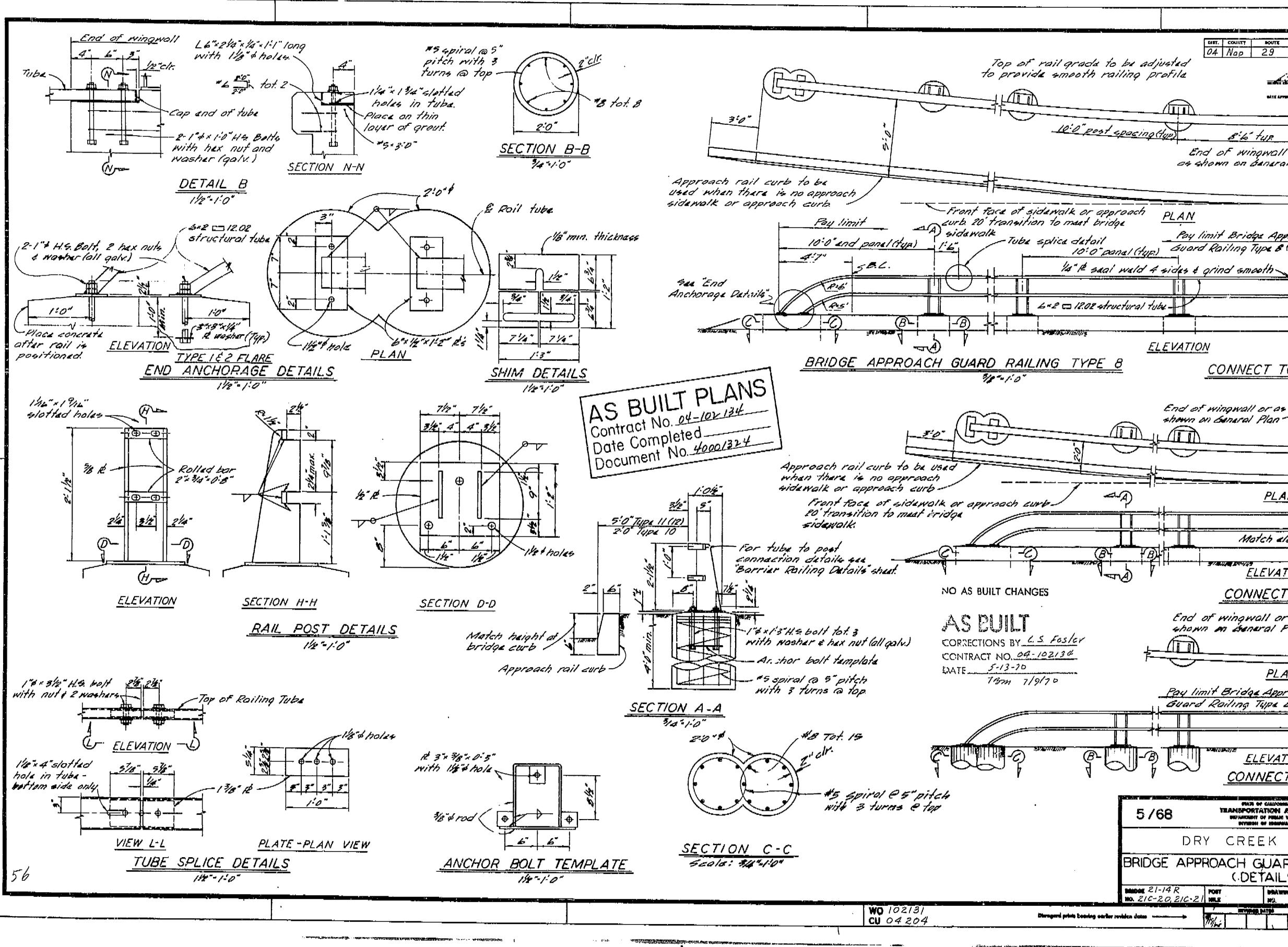


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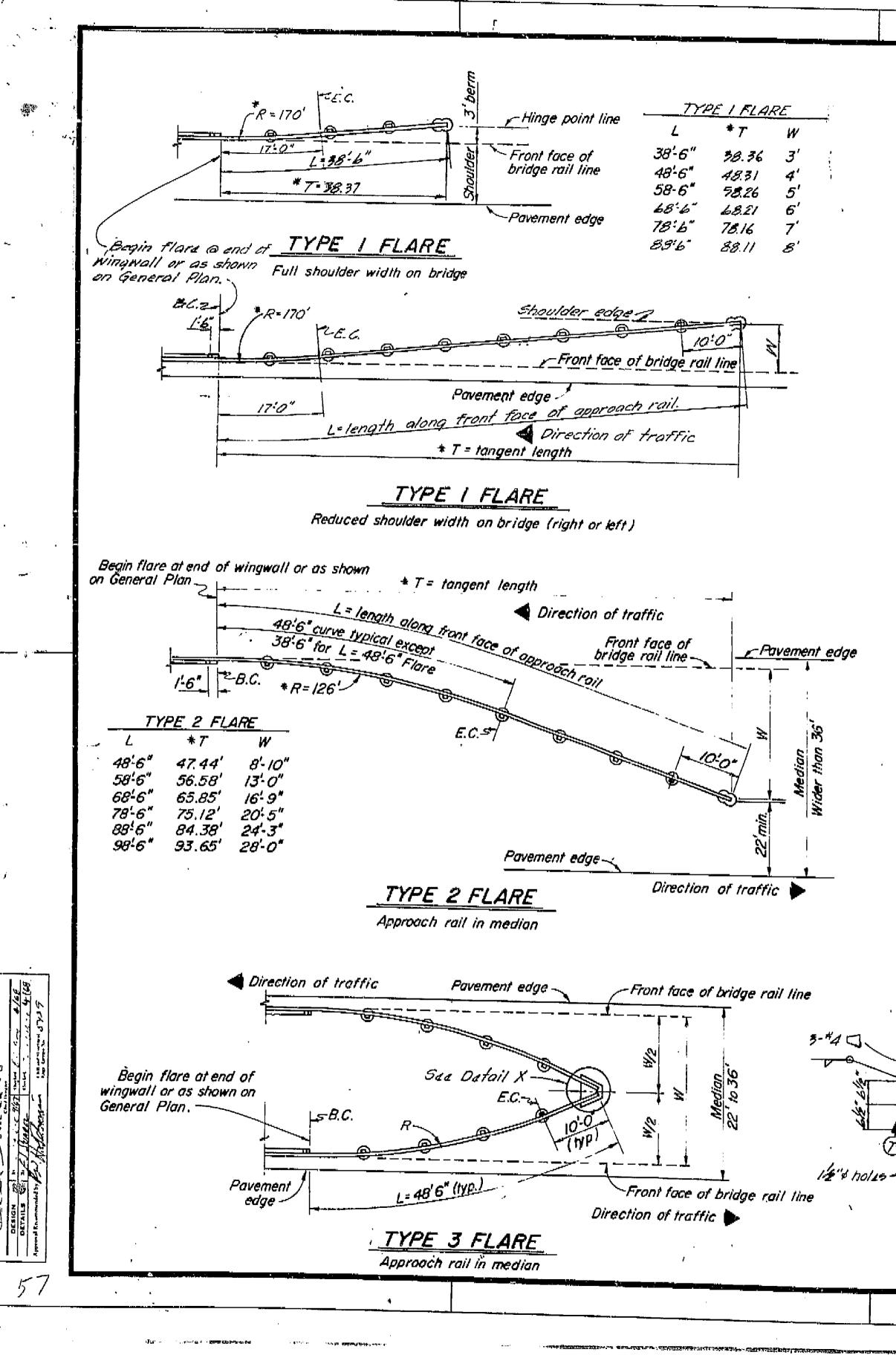


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I HEREBY CERTIFY THAT THIS IS A TRUE AND ACCURATE COPY OF THE ABOVE DOCUMENT TAKEN UNDER MY DIRECTION AND CONTROL ON THIS DATE IN SACRAMENTO, CALIFORNIA PURSUANT TO AUTHORIZATION BY THE DIRECTOR OF PUBLIC MORES.



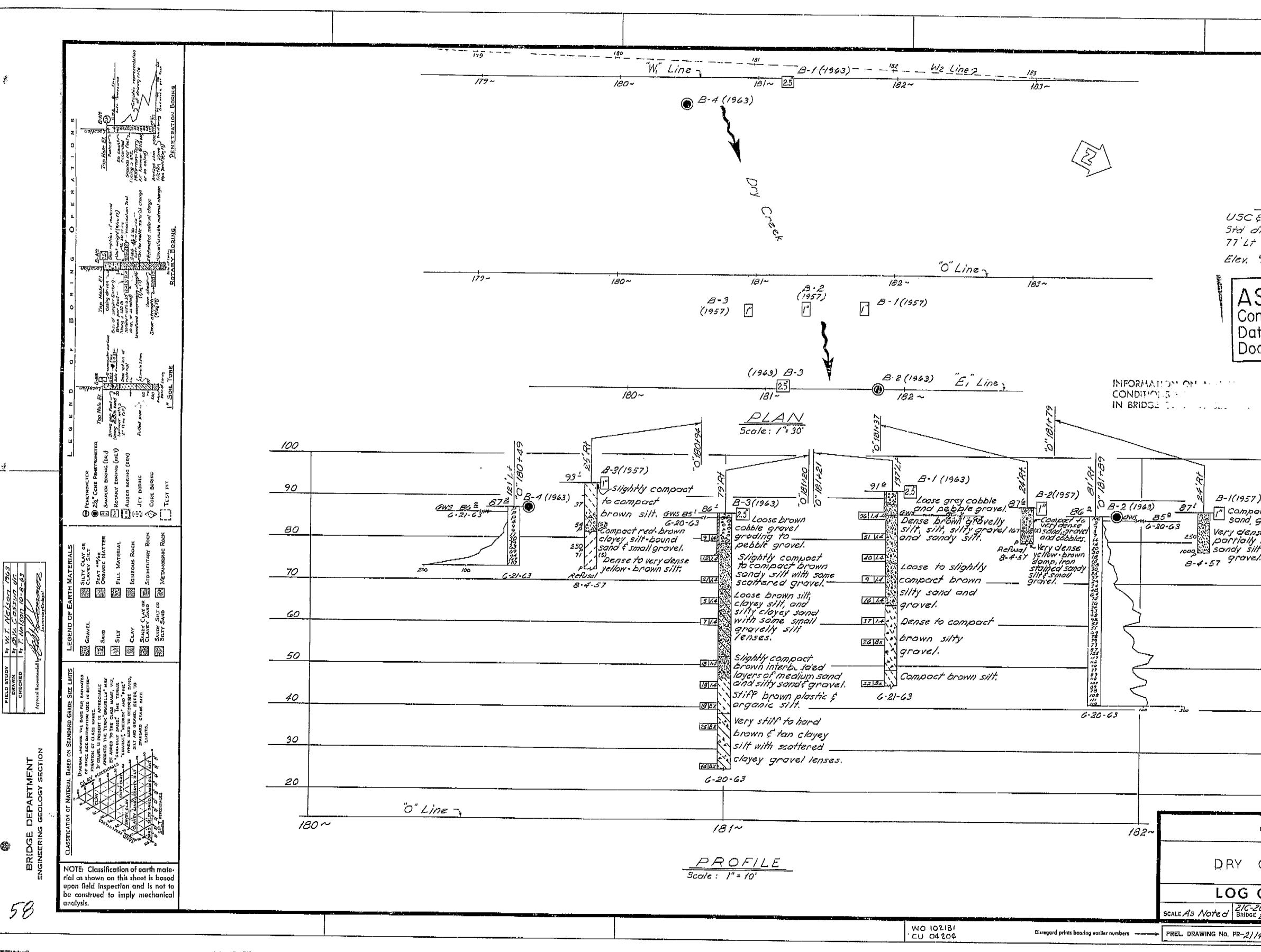
DIAT. COMMETT BOATS FOAT HILLES-TOTAL PROJECT BUT FOAT 04 Nap 29 15.0/19.6 56 69 Seind September 23, 1968 8:6 typ ŀ`&`` End of wingwall or Top tube not of shown on beneral Plans used with chain link fance (Type 4) Pay limit Bridge Approach, Pay limit Type 11 (2) Guard Railing Type & (typ) R-5-0* - Retail B Match dlavation CONNECT TO TYPE II (12) Pay limit Type 10 <u>PLAN</u> Motch elevations ELEVATION CONNECT TO TYPE 10 End of wingwall or as shown m General Plan -8.8 PLAN Pay limit Bridge Approach Pay limit Type 9 Guard Railing Type 8 Detail B ELEVATION CONNECT TO TYPE 9 отал от сладонна. Теанброктатиона Афенсу окуляти от реши учеся отумани от состануя XS-9-83 DRY CREEK BRIDGES BRIDGE APPROACH GUARD RAILING TYPES (DETAILS) INERT 💕 // //31 -----



Direction of traffic Pavement edge ~ <u> 48'6</u>* - Front face of bridge rail line The second secon Variable offset, when more than IO'use Type 3 Flare. For offset see "General Plan." -*-*2*B.C*. Front face of bridge rail line -Pavement edge See Detail X -Begin flare at end of Direction of traffic 🍉 wingwall or as shown on General Plan. TYPE 4 FLARE Approach rail in median , Tangent length dependent upon Flore type <u>__</u> R (modified) <u> TUBE -</u> -Front face of bridge rail line T (modified) FLARE LAYOUT ON CURVED ROADWAY Note: W is radial to front face of bridge rail line Notes : Railing shown for roadway curve to left A. Tubing shall be shop bent or fabricated to fit B. Posts shall be vertical and spaced at 10'-0" Top of rail elevations to be determined by AS BUILT PLANS D. Torque rail to post nuts to 175 ft. lbs. E. No more than one tube splice per panel is Contract No. <u>04-102 134</u> F. All exposed edges shall be ground smooth Date Completed G. Galvanize rail assembly after fubrication. Document No. 4000/324 H. For details not shown see "Barrier Railing" J. Front face of bridge rail line is parallel of K. Dimensions marked * are valid for tangent curved roadway. , Place concrate 2:0" min. 3:6 L. R for Type 3 & 4 Flare must be colculate into undisturbed moterial after roll is positioned. NO AS #4 tot.8 -10" 12" 10/4" R. Clip corner. AS it required for deargnes to-of 2-14 + 1.3" H.S. Bolto, 2 hax nuts & Washar (all galv.) --6×2 🗖 12.02 structural tuba CORREC" CONTRA DATE..... Vary Alavation to >6×2 = 12.02 structural tube fit railing 3" x 3 "x 1/4" R^t 1105 traves (Typ) -CE Rail tube f0--01 -72 Tor 5/68 SECTION T-T DR <u>PL AN</u> BRIDGE APPI DETAIL X No Scola BRIDGE 21-14R No. 216-20, 216-21 WO 102131 Discogend prime boaring earlier revision dates -----CU 04204

I HEREBY CERTIFY THAT THIS IS A TRUE AND ACCURATE COPY OF THE ABOVE DOCUMENT TAKEN UNDER MY DIRECTION AND CONTROL ON THIS DATE IN SACRAMENTO, CALIFORNIA PURSUANT TO AUTHORIZATION BY THE DIRECTOR OF PUBLIC WORKS.

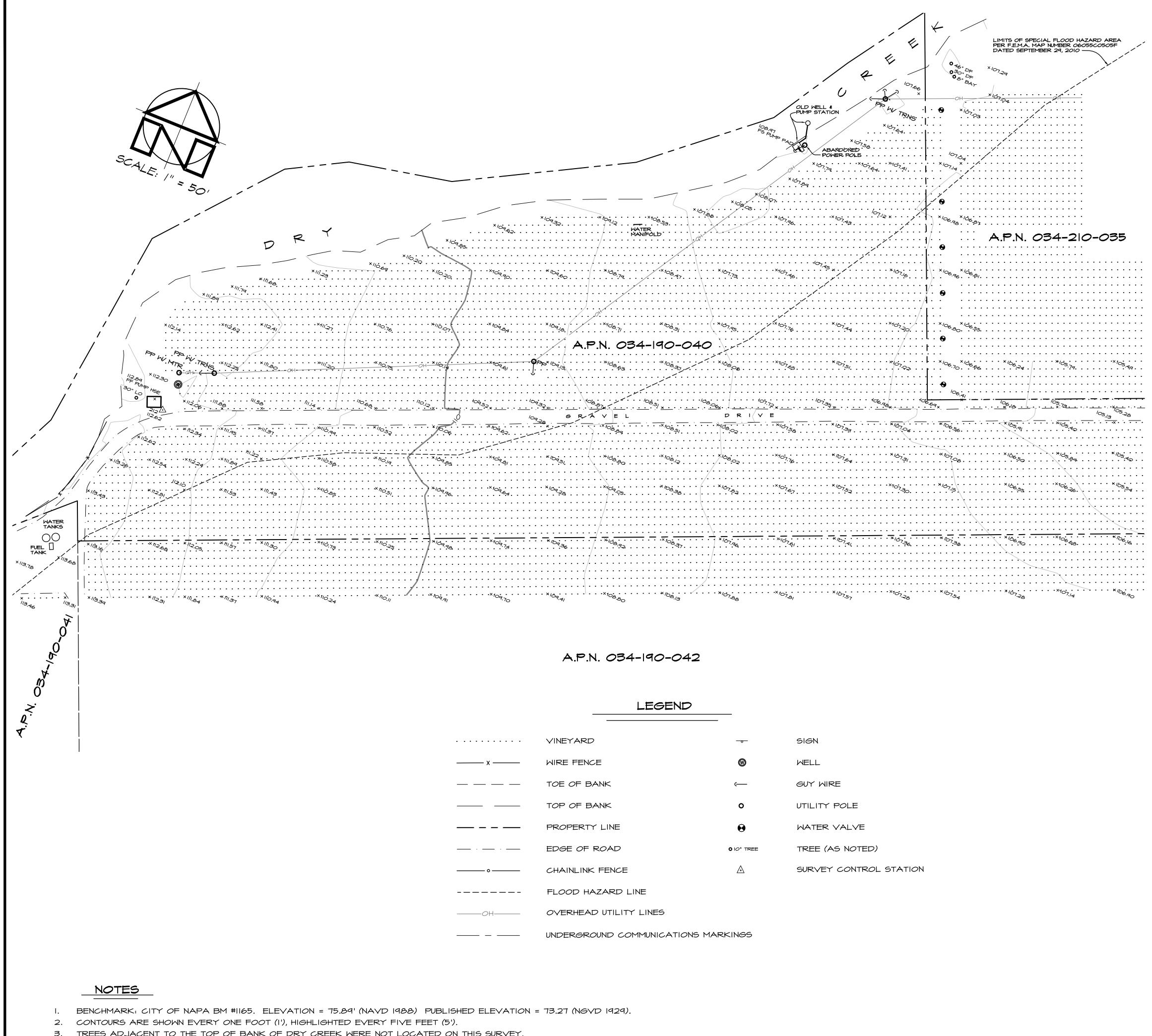
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the Engineer unless shown on the plans.	•q [°]
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or concentric to pavement edge . t roadway alignment only. Modify for	
ed for each offset.	•
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BUILT	
TIONS BY L. S. Foster	
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STATE OF GALIFORNIA TRANSPORTATION AGENCY	
DEPARTMENT OF FUELIC WORKS XS-9-87	
Y CREEK BRIDGES	
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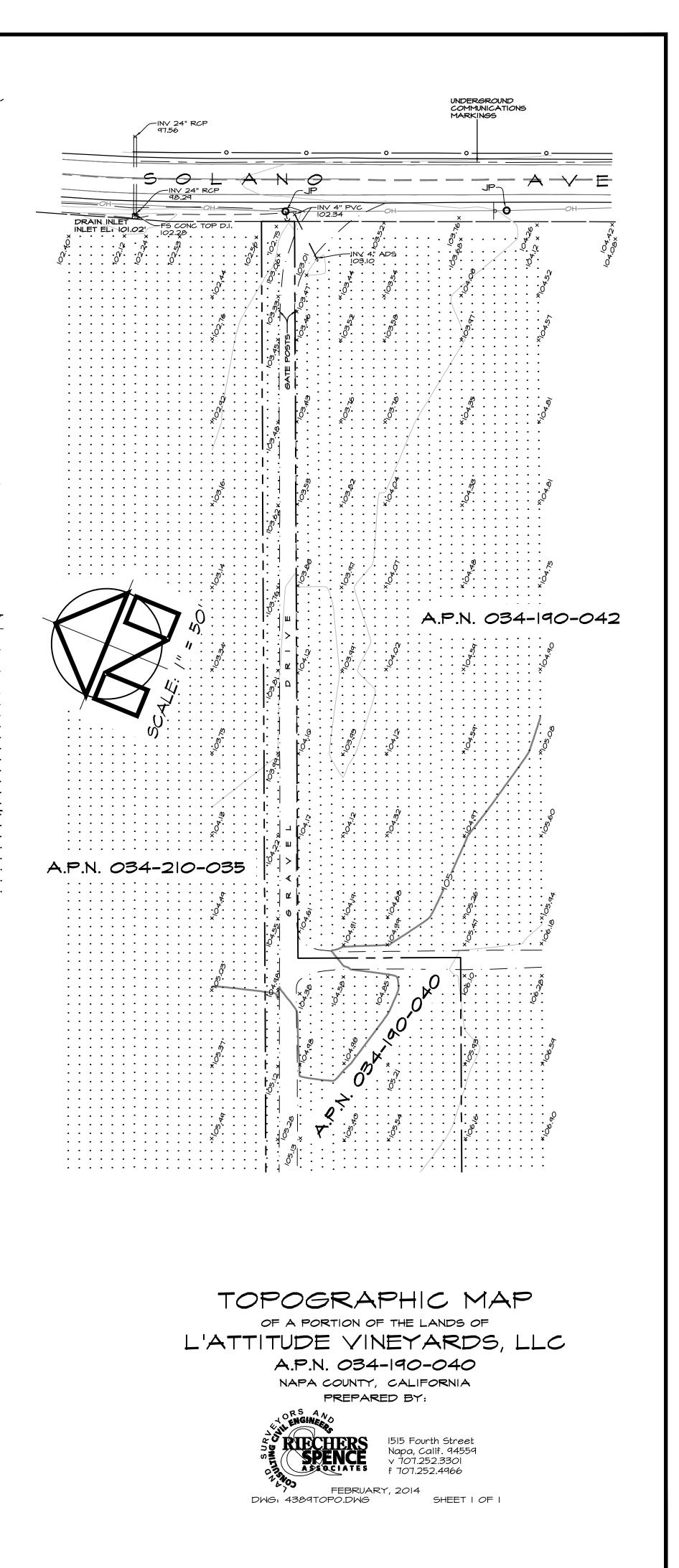
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STATE OF CALIFORNIA DEPARTMENT OF PUBLIC WORKS
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Attachment 3: 2014 Onsite Survey



TREES ADJACENT TO THE TOP OF BANK OF DRY CREEK WERE NOT LOCATED ON THIS SURVEY.

LEGEND		
	_	
VINEYARD	- 0 -	SIGN
WIRE FENCE	W	WELL
TOE OF BANK	(GUY WIRE
TOP OF BANK	ο	UTILITY POLE
PROPERTY LINE	Θ	WATER VALVE
EDGE OF ROAD	O IO" TREE	TREE (AS NOTED)
CHAINLINK FENCE		SURVEY CONTROL STATION
FLOOD HAZARD LINE		



Attachment 4: LOMA 14-09-0308A-06025

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Date: December 17, 2013

Case No.: 14-09-0308A

LOMA

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Federal Emergency Management Agency

			AND SECURI		Washingto	n, D.C. 20472			
			DET	LETTER C	OF MAP AN				
С	OMMUN		AND MAP PANEL	INFORMATION		LEGAL P	ROPERTY DESC		
NAPA COUNTY, CALIFORNIA (Unincorporated Areas) COMMUNITY		A parcel of land, as described in the Grant Deed recorded as Document No. 2013-0004244, in the Office of the Recorder, Napa County, California (APN:034-212-011)							
		CON	IMUNITY NO.: 060	205					
AFFE MAP P		NUN	IBER: 06055C050	5F					
		DAT	E: 9/29/2010						
ECODINO COONCE: DINI ONEEN					APPROXIMATE LATI SOURCE OF LAT & L				3 DATUM: NAD 83
					DETERMINATIO	DN			
LOT	BLOC SECTI		SUBDIVISION	STREET	OUTCOME WHAT IS REMOVED FROM THE SFHA	FLOOD ZONE	1% ANNUAL CHANCE FLOOD ELEVATION (NAVD 88)	LOWEST ADJACENT GRADE ELEVATION (NAVD 88)	LOWEST LOT ELEVATION (NAVD 88)
-				1088 Darms Lane	Structure (Residence)	X (unshaded)	106.2 feet	110.5 feet	
-			a rd Area (SFHA) d in any given year	· The SFHA is an a (base flood).	area that would be	inundated by	the flood havin	ng a 1-percent c	hance of being
				ease refer to the appropria	te section on Attachmo	ent 1 for the addi	tional consideration	ns listed below.)	
	ONS REN		BLE (CONTINUED) IN THE SFHA						
the pro determin being e the SFI lender	operty de ned that qualed o HA locat has the	escrib the or ex ted c optic	ed above. Using structure(s) on the ceeded in any giv on the effective N on to continue the	Emergency Managemen the information subm property(ies) is/are not en year (base flood). IFIP map; therefore, th flood insurance requir SFHA. Information about	itted and the effect located in the SFH. This document amen ne Federal mandatoo rement to protect its	tive National F A, an area inu ds the effective ry flood insura financial risk	Flood Insurance ndated by the flo e NFIP map to r nce requirement on the loan. A	Program (NFIP) od having a 1-pe remove the subjec does not apply.	map, we have rcent chance of ct property from However, the

This determination is based on the flood data presently available. The enclosed documents provide additional information regarding this determination. If you have any questions about this document, please contact the FEMA Map Assistance Center toll free at (877) 336-2627 (877-FEMA MAP) or by letter addressed to the Federal Emergency Management Agency, LOMC Clearinghouse, 847 South Pickett Street, Alexandria, VA 22304-4605.

(into

Luis Rodriguez, P.E., Chief Engineering Management Branch Federal Insurance and Mitigation Administration Page 2 of 2

Date: December 17, 2013 Case No.: 14

Case No.: 14-09-0308A

LOMA



Federal Emergency Management Agency

Washington, D.C. 20472

LETTER OF MAP AMENDMENT DETERMINATION DOCUMENT (REMOVAL)

ATTACHMENT 1 (ADDITIONAL CONSIDERATIONS)

DETERMINATION TABLE (CONTINUED)

LOT	BLOCK/ SECTION	SUBDIVISION	STREET	OUTCOME WHAT IS REMOVED FROM THE SFHA	FLOOD ZONE	1% ANNUAL CHANCE FLOOD ELEVATION (NAVD 88)	LOWEST ADJACENT GRADE ELEVATION (NAVD 88)	LOWEST LOT ELEVATION (NAVD 88)	
			1088 Darms Lane	Structure (Garage)	X (unshaded)	106.2 feet	111.3 feet		

PORTIONS OF THE PROPERTY REMAIN IN THE SFHA (This Additional Consideration applies to the preceding 2 Properties.)

Portions of this property, but not the subject of the Determination/Comment document, may remain in the Special Flood Hazard Area. Therefore, any future construction or substantial improvement on the property remains subject to Federal, State/Commonwealth, and local regulations for floodplain management.

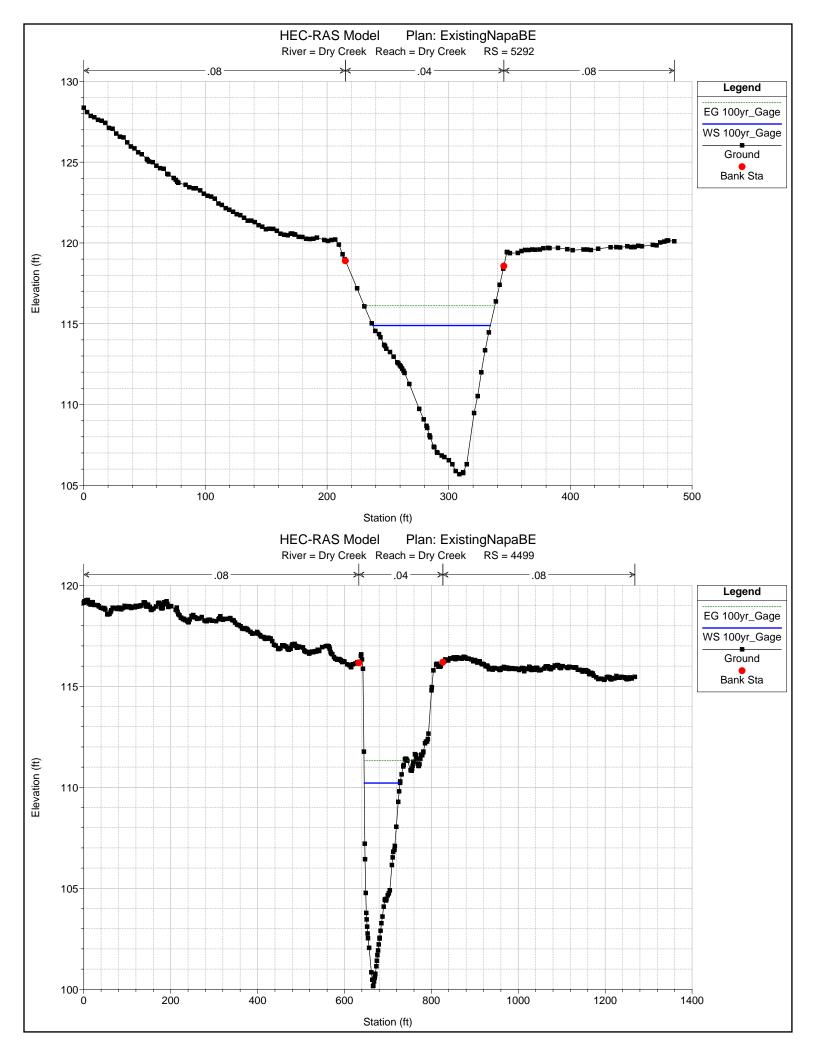
ZONE A (This Additional Consideration applies to the preceding 2 Properties.)

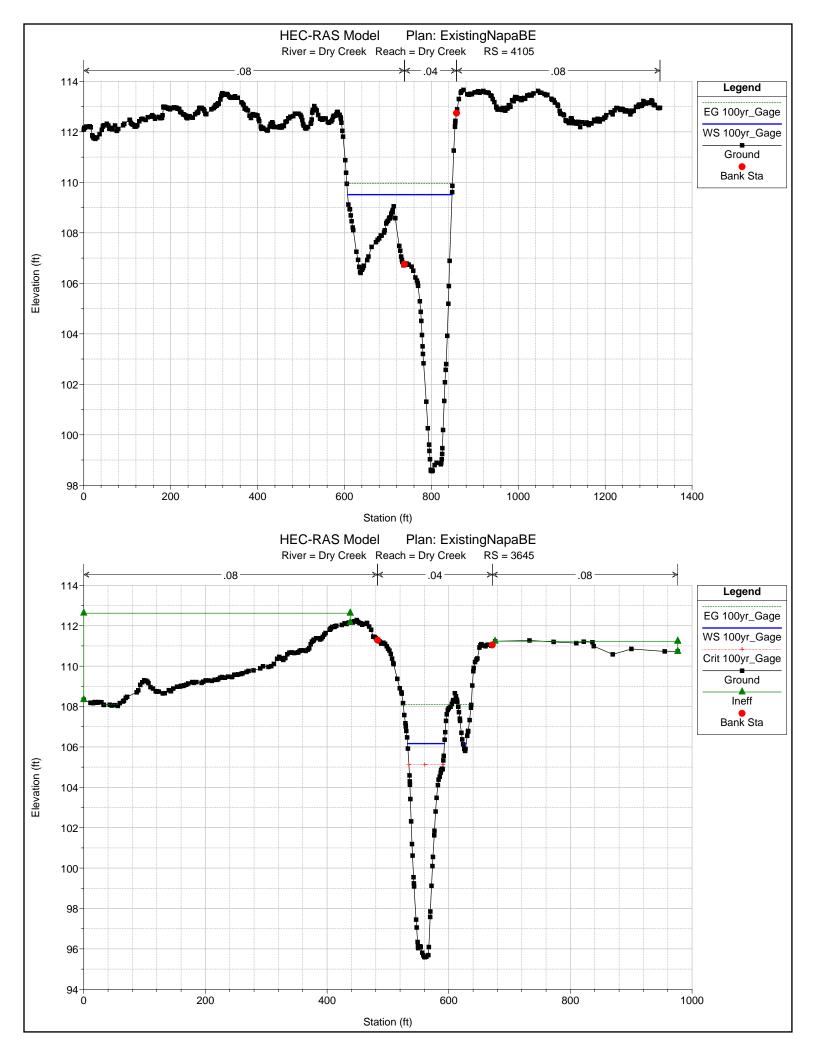
The National Flood Insurance Program map affecting this property depicts a Special Flood Hazard Area that was determined using the best flood hazard data available to FEMA, but without performing a detailed engineering analysis. The flood elevation used to make this determination is based on approximate methods and has not been formalized through the standard process for establishing base flood elevations published in the Flood Insurance Study. This flood elevation is subject to change.

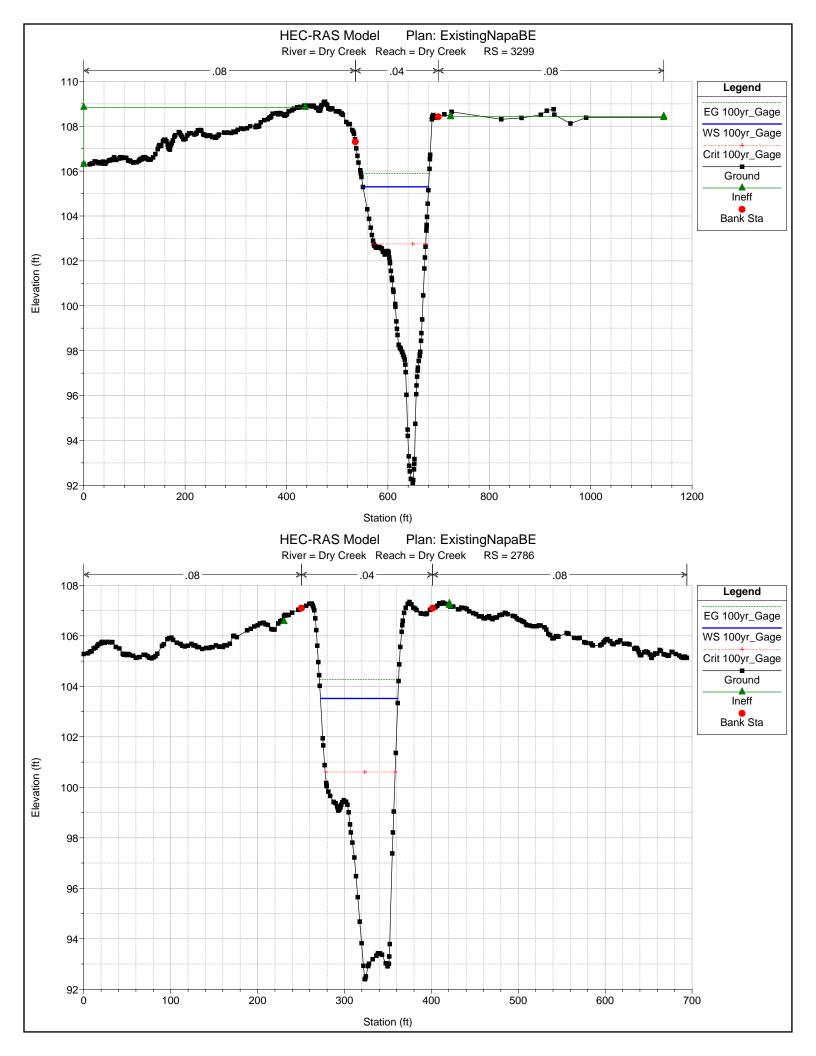
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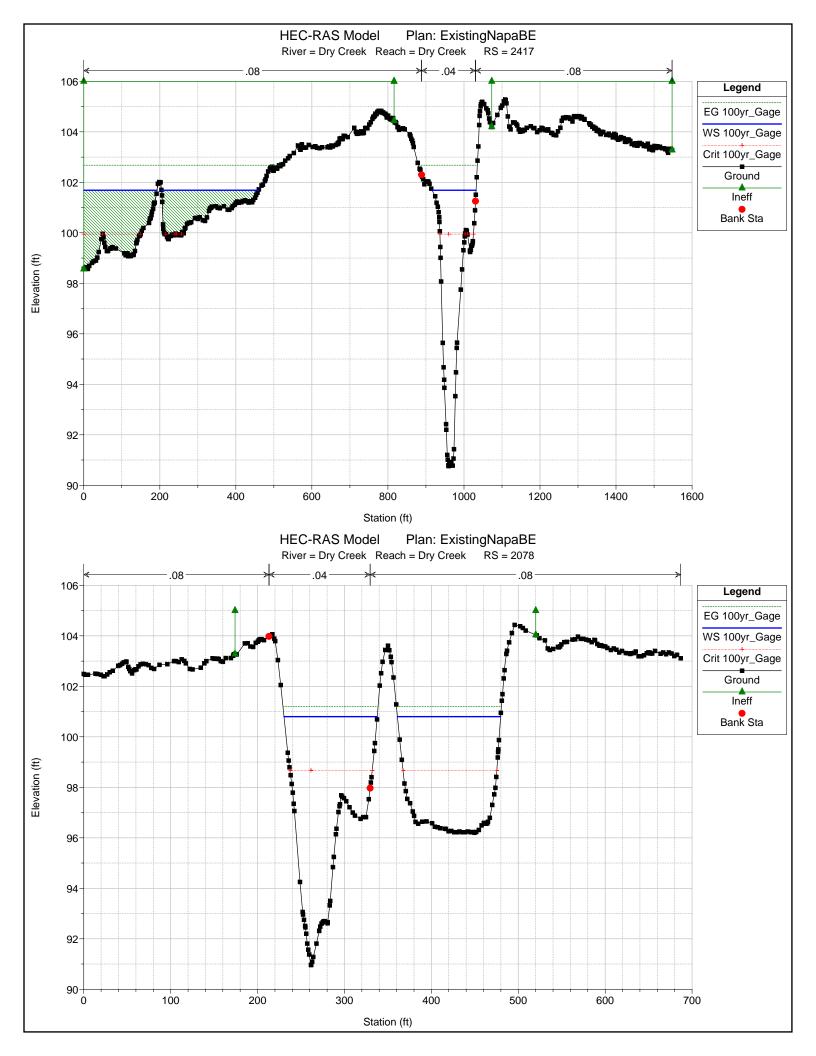
Luis Rodriguez, P.E., Chief Engineering Management Branch Federal Insurance and Mitigation Administration

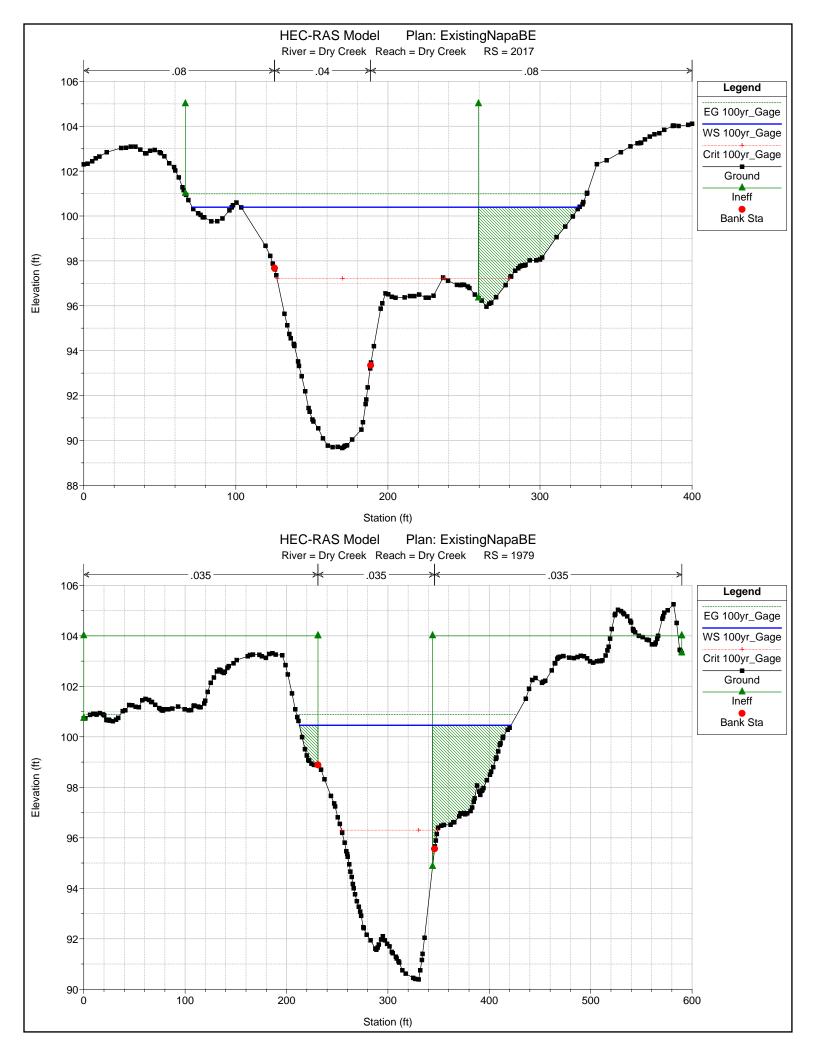
Attachment 5: HEC-RAS Cross Sections

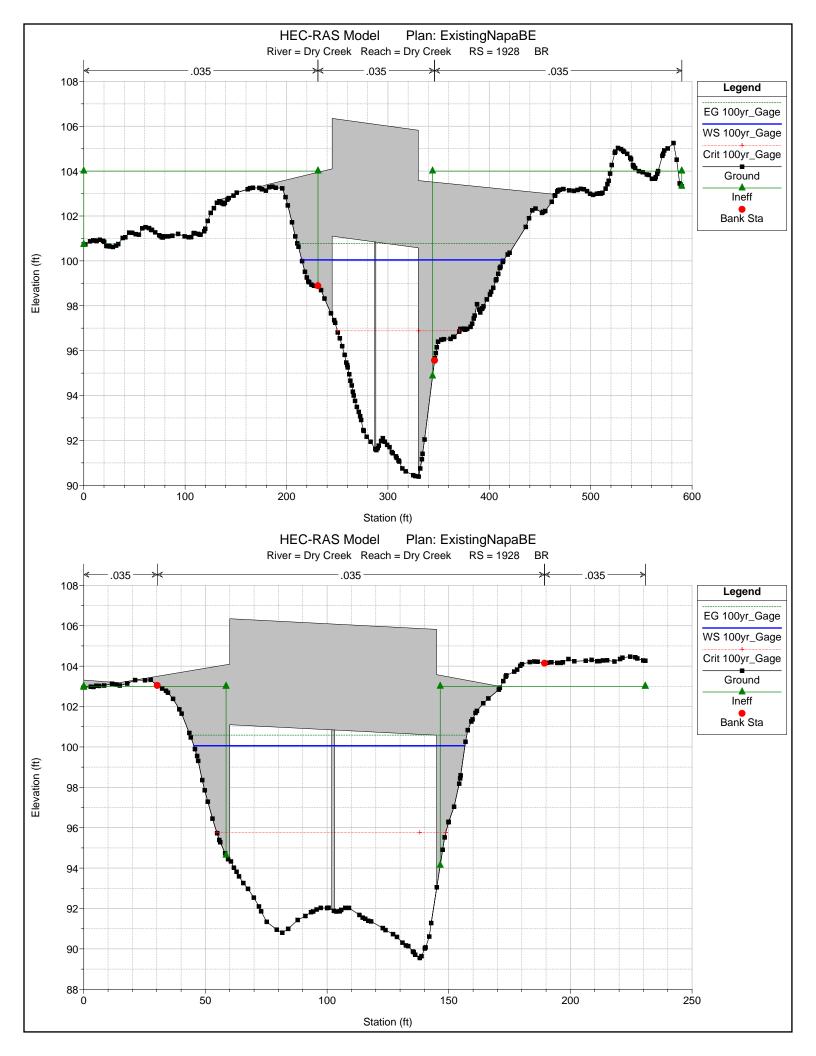


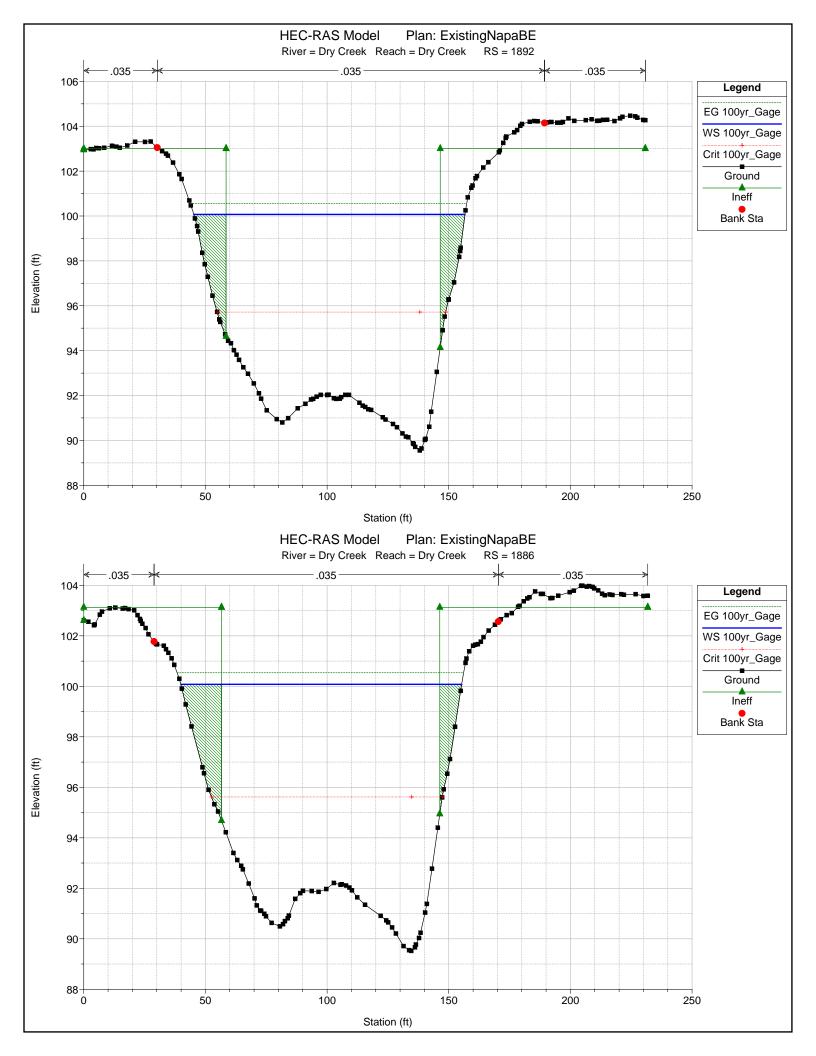


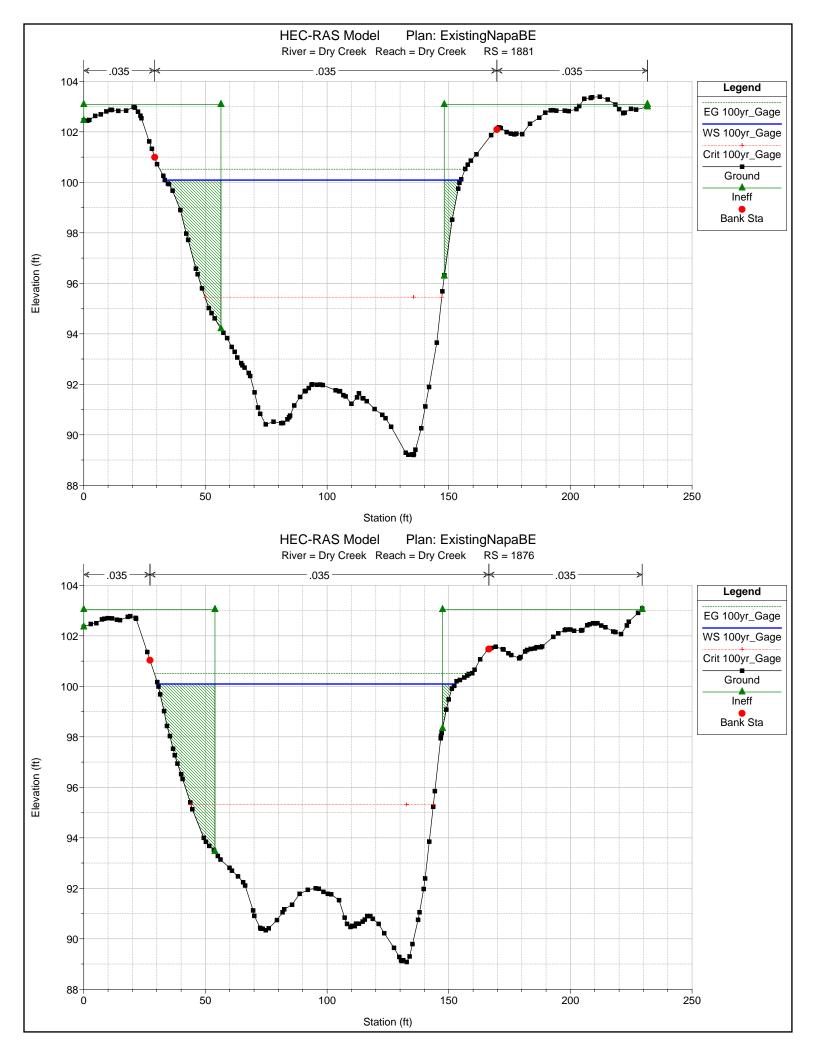


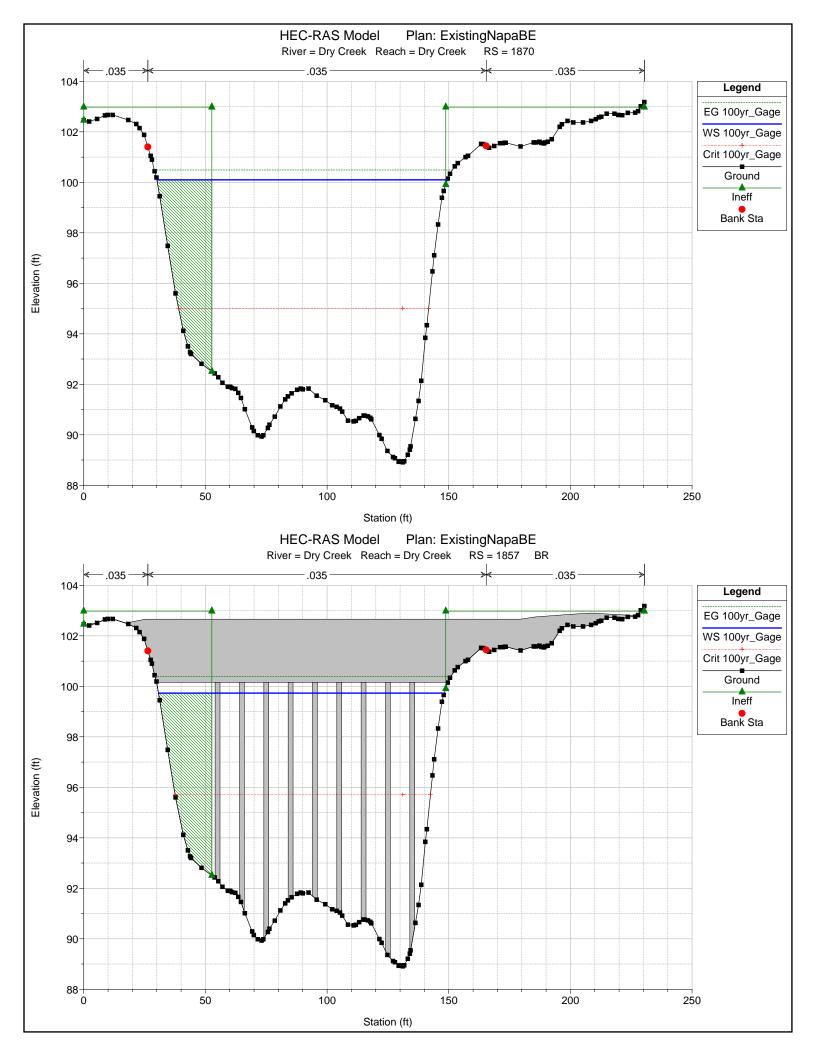


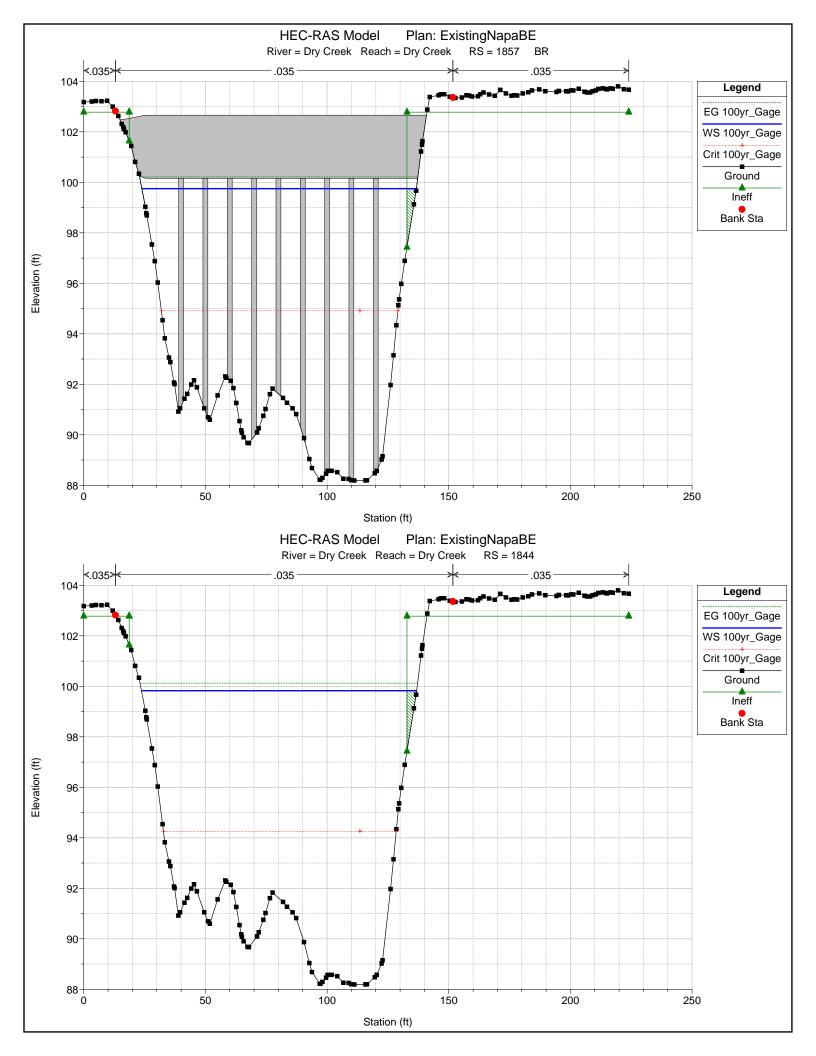


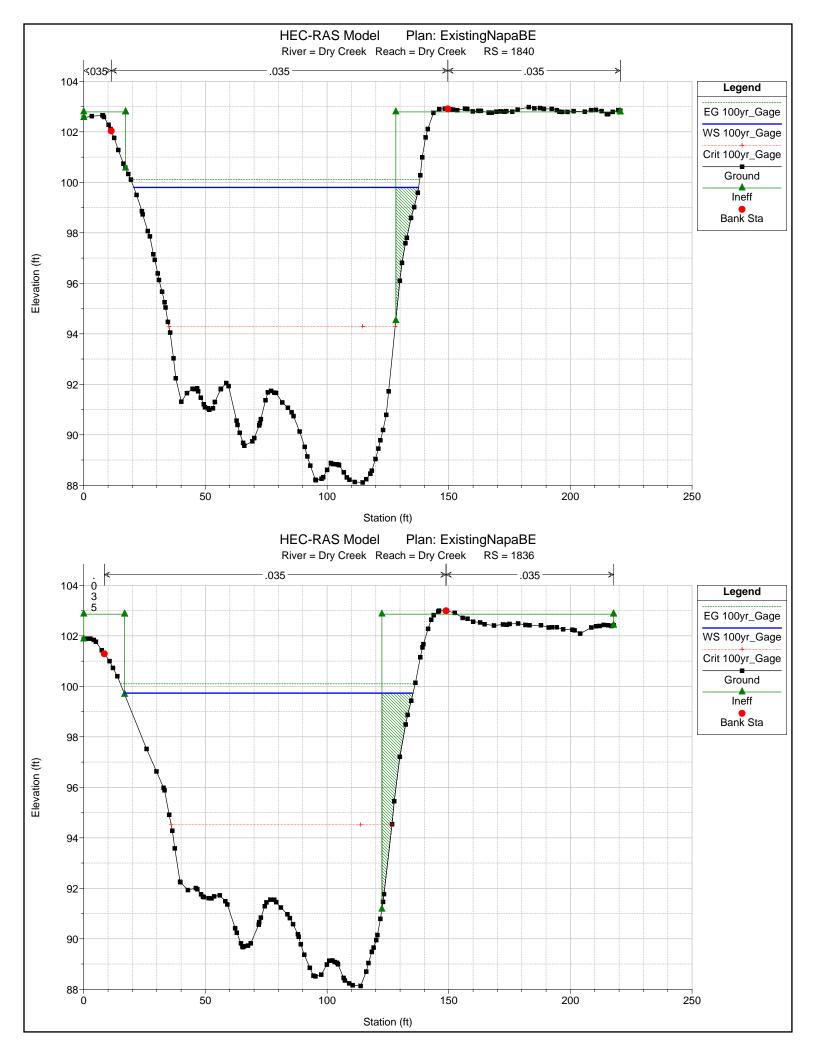


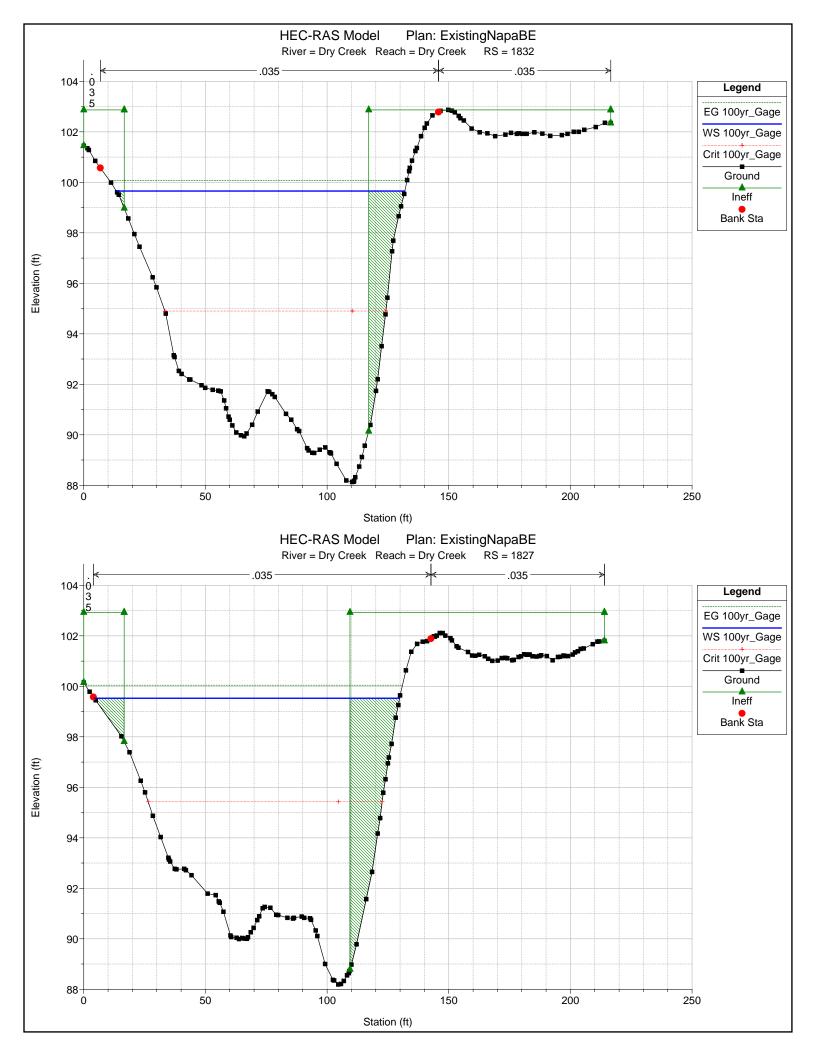


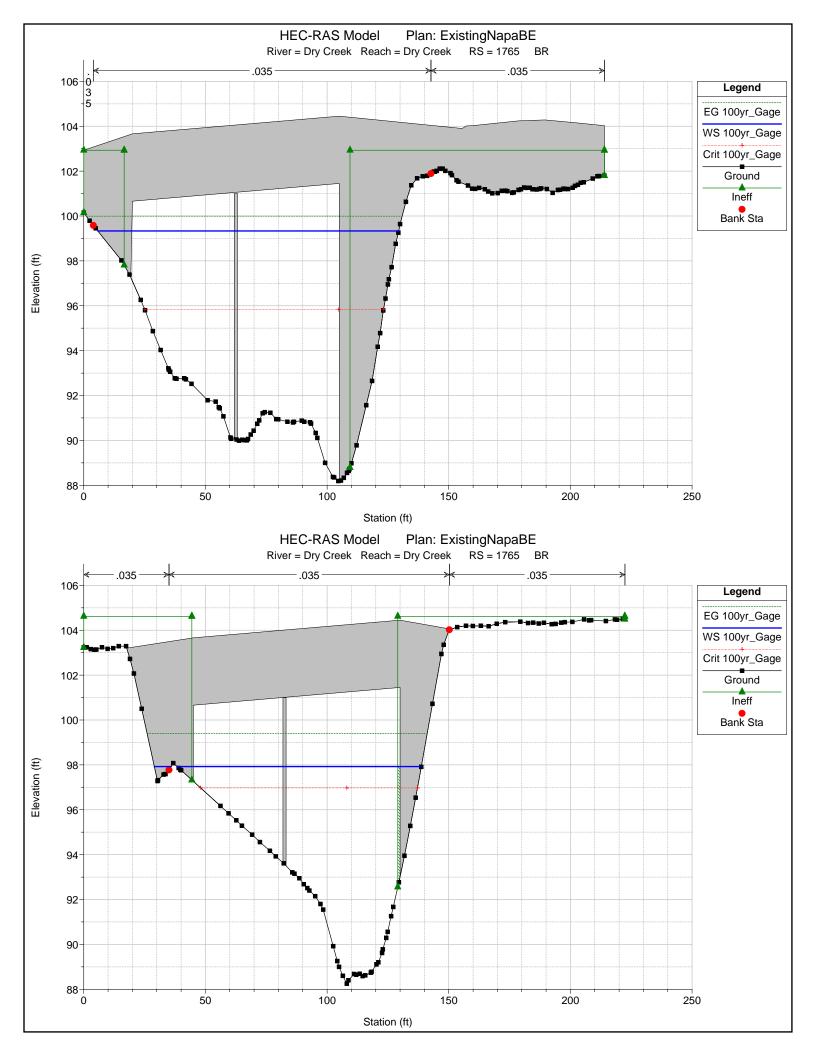


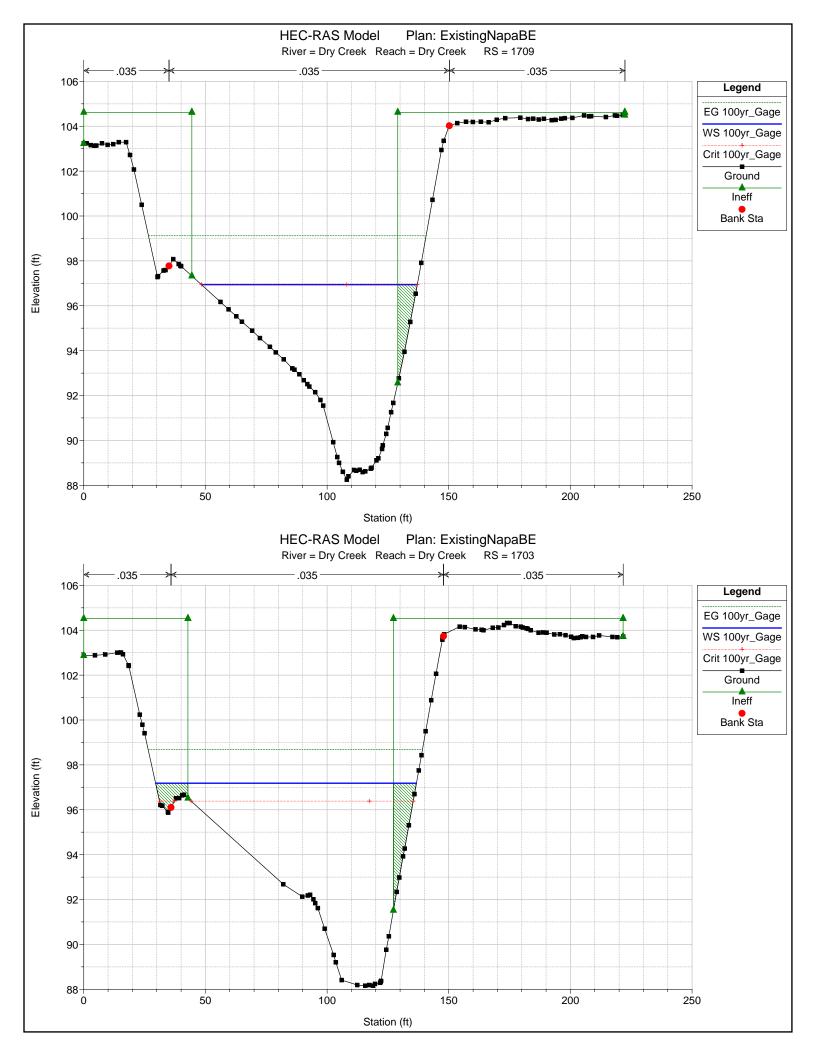


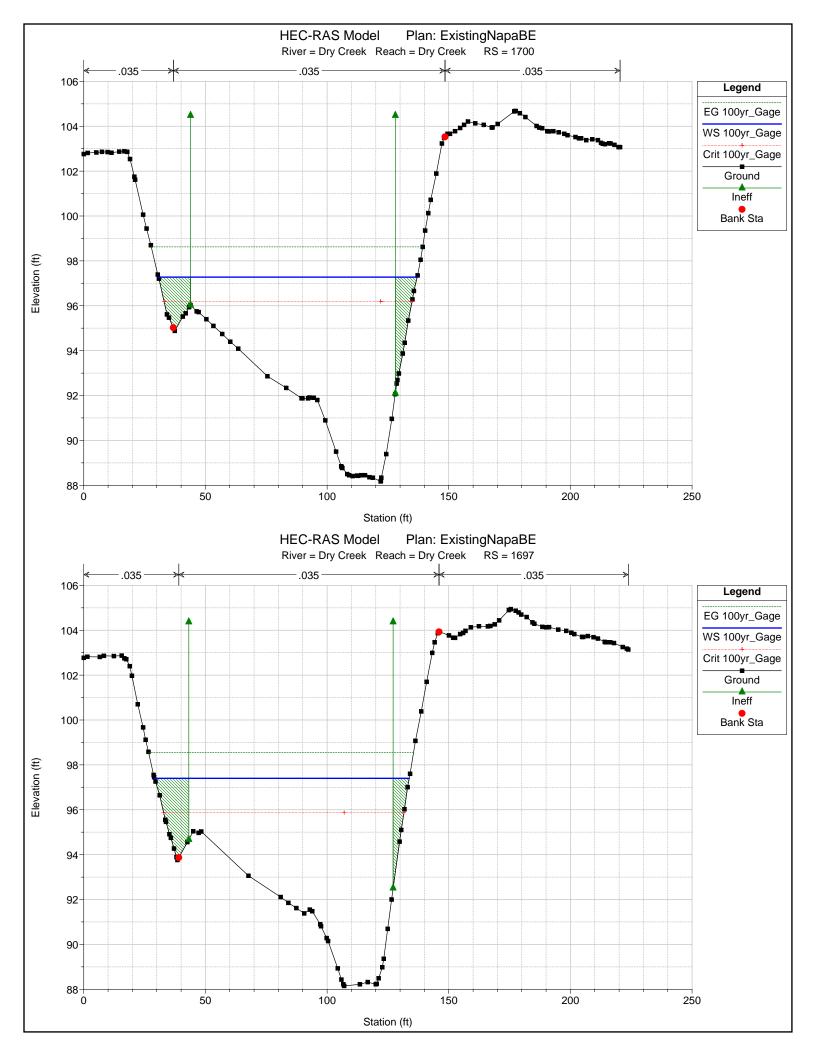


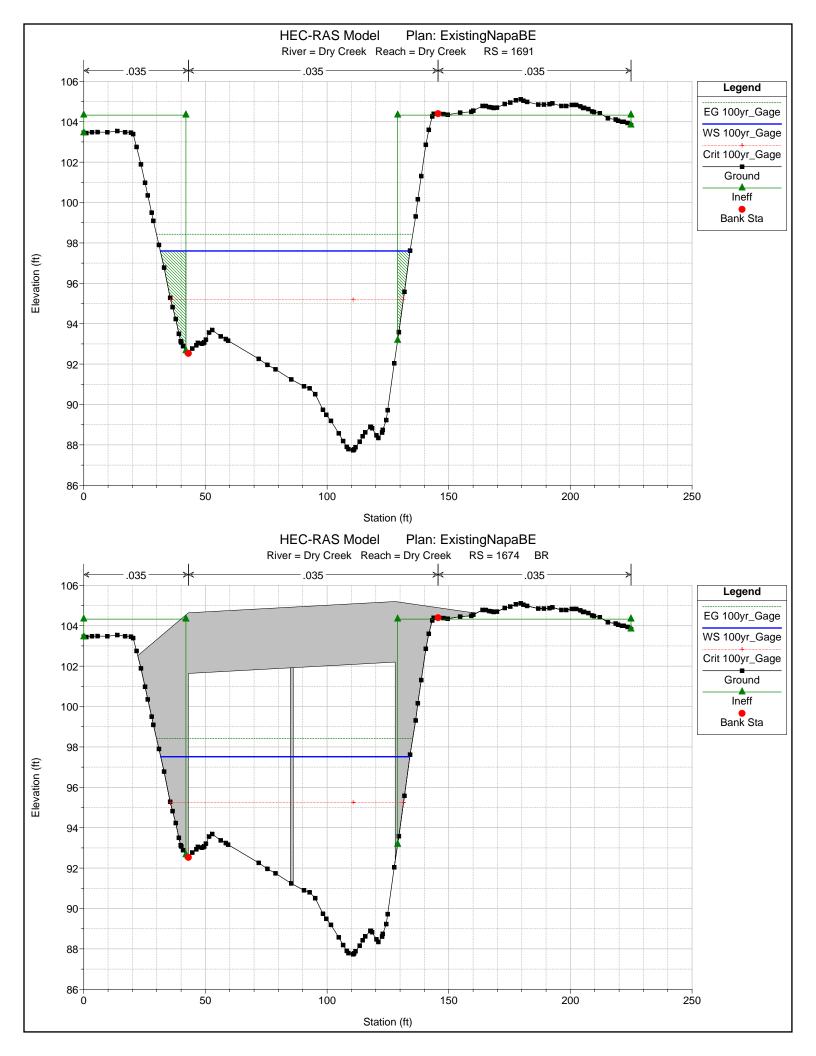


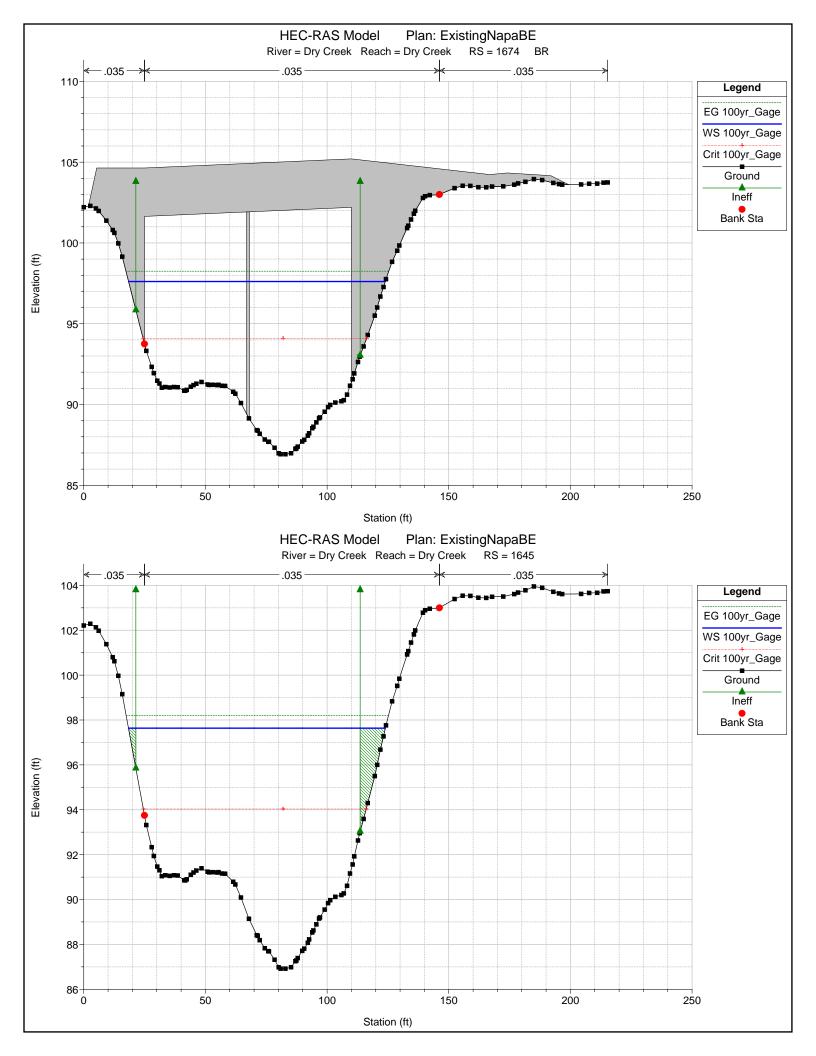


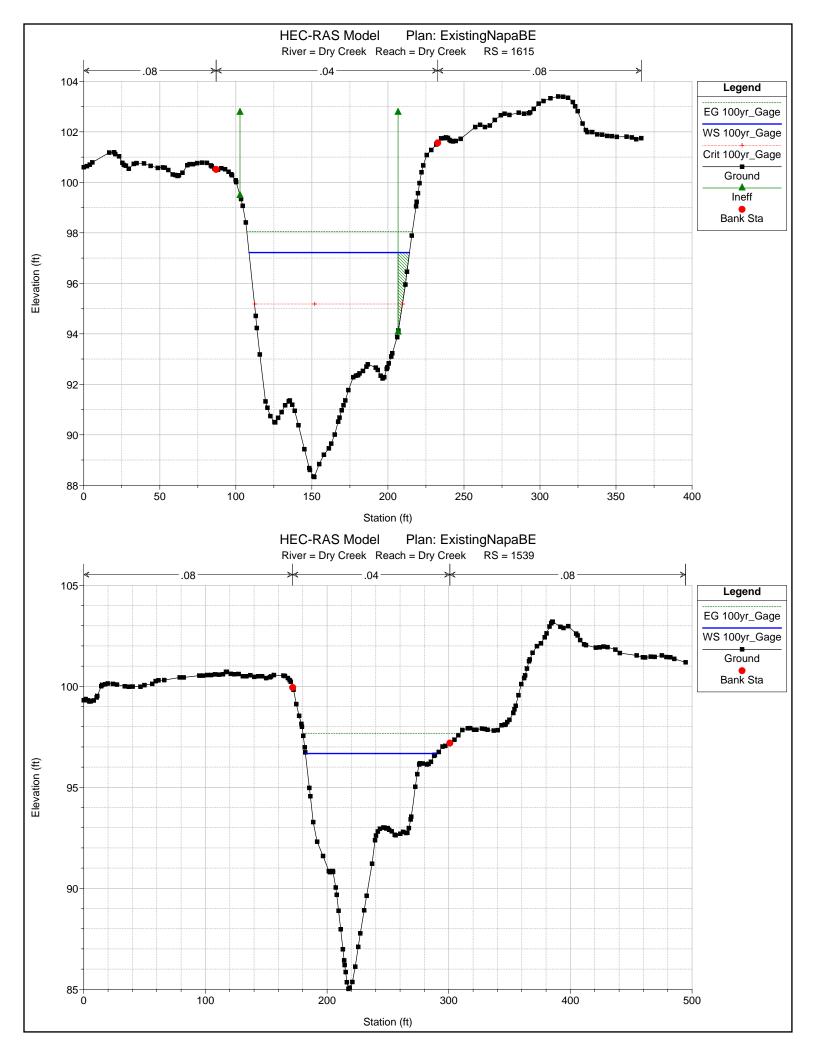


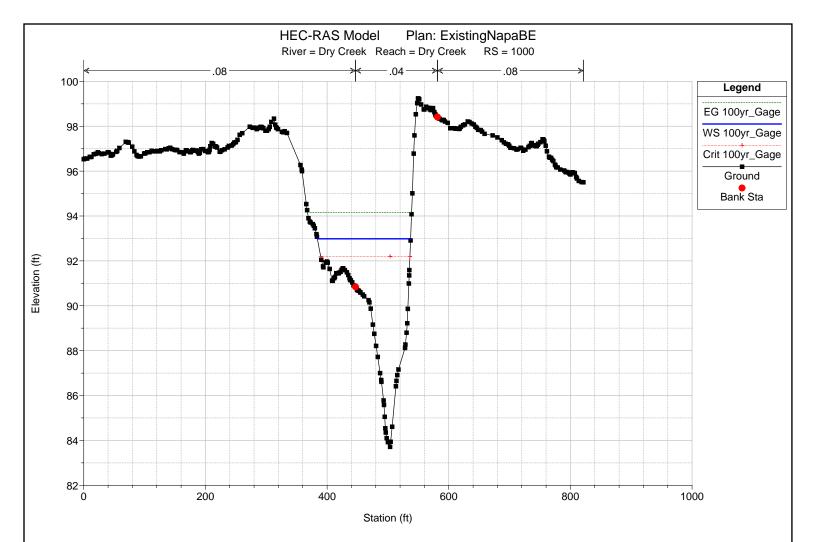












Attachment 6: HEC-RAS Stream Profile

