

February 21, 2017

TO:

Dirk Lovett, P.E., City Engineer

City of Hidden Hills

FROM:

Jack Rydell, P.E., T.E.

Senior Traffic Engineer

## LONG VALLEY ROAD WEST OF VALLEY CIRCLE BOULEVARD TRAFFIC EVALUATION

As requested, I have analyzed the portion of Long Valley Road west of Valley Circle Boulevard to evaluate the effects of adding a 3<sup>rd</sup> westbound travel lane on vehicle queuing and delay. The effects of adding a 3<sup>rd</sup> lane are illustrated in Attachment 1. The conceptual layout used for this analysis is included as Attachment 2. Traffic counts used for this analysis were taken on May 26, 2016 and the analysis was performed using Synchro 9. The pertinent Synchro summary reports are included as Attachment 3.

Under existing traffic conditions and geometry, the westbound direction of Long Valley Road experiences an average queue of approximately 791 feet measured from the existing stop control limit line during the AM peak hour. This backup extends through the intersection with Valley Circle Boulevard. Approach vehicle delay, as measured from the stop sign, is 70.6 seconds. Adding the 3<sup>rd</sup> westbound lane reduces the queue to approximately 84 feet from the stop control limit line and reduces the approach vehicle delay to 46.0 seconds. This action would be expected to eliminate the current vehicle backup that affects Valley Circle Boulevard.

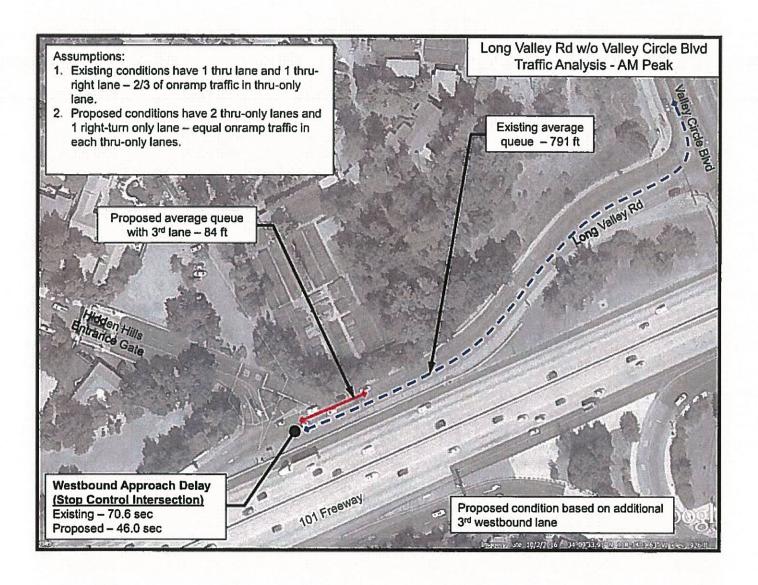
Due to significantly lower PM peak hour volume entering the freeway onramp (1,239 vehicles per hour vs. 690 vehicles per hour), there is negligible benefit to PM peak hour operations by adding the 3<sup>rd</sup> lane.

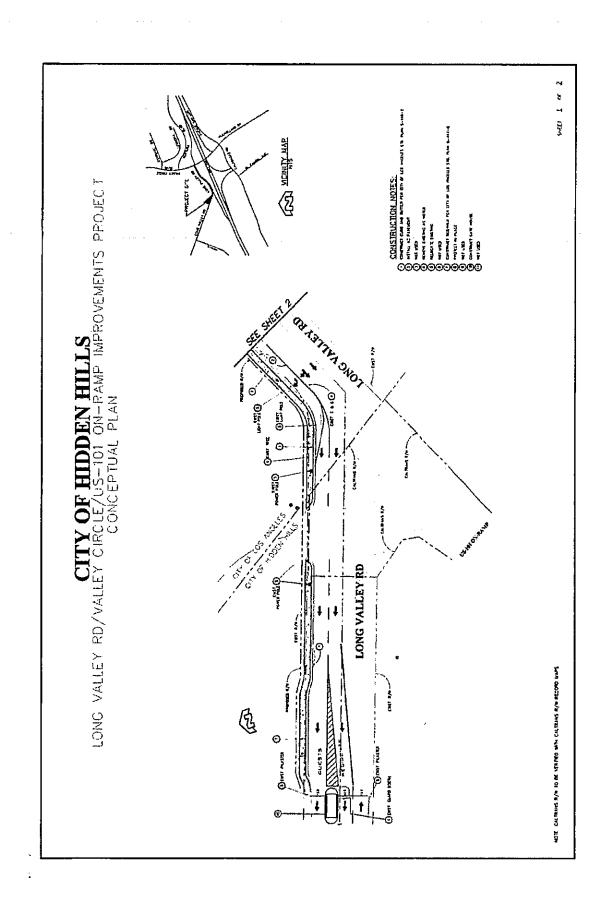
JR: Long Valley Rd w-o Valley Circle Blvd Traffic Evaluation - 2-21-17

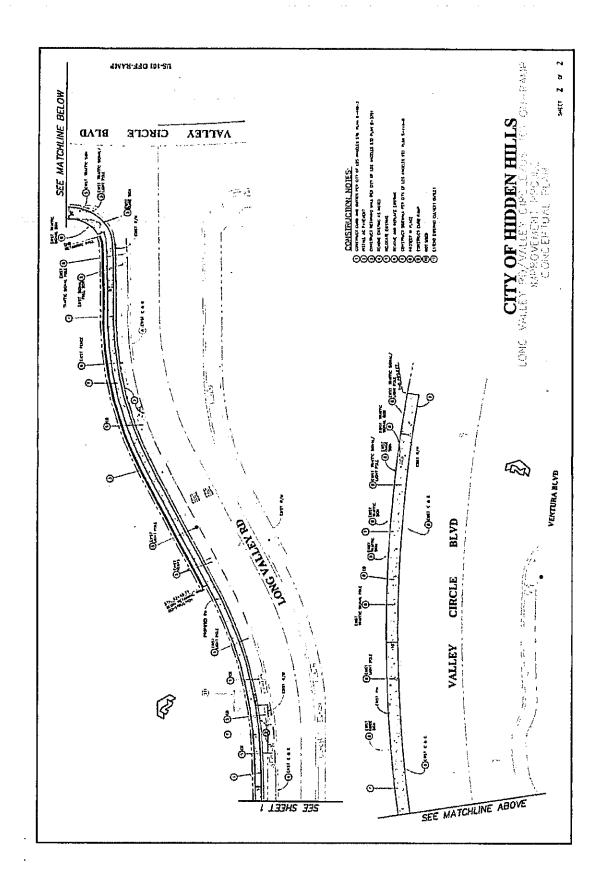
Attachment 1 - AM peak hour traffic analysis results

Attachment 2 - Conceptual 3rd lane layout

Attachment 3 – AM peak hour SYNCHRO summary reports







	1	-	4	4	1	1	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations			44			7	
Sign Control		Stop	Stop		Stop		
Volume (vph)	0	0	826	524	72	28	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	
Hourly flow rate (vph)	0	0	949	602	83	32	
Direction, Lane #	WB 1	WB 2	SB 1	SB 2			
Volume Total (vph)	633	919	83	32		(y)	
Volume Left (vph)	0	0	83	0			
Volume Right (vph)	0	602	0	32			
Hadj (s)	0.03	-0.42	0.53	-0.67			
Departure Headway (s)	5.0	4.5	7.6	6.4			
Degree Utilization, x	0.87	1.0	0.17	0.06			
Capacity (veh/h)	716	807	462	546			The state of the s
Control Delay (s)	30.6	98.2	10.9	8.6			
Approach Delay (s)	70.6		10.3				The second secon
Approach LOS	F		В	produced by one a			
ntersection Summary							
Delay			66.4				
Level of Service			F				
Intersection Capacity Utiliza	ation		50.3%	IC	U Level o	f Service	A
Analysis Period (min)			15		and Committee of the Committee		

## Intersection: 3: N 101 On Ramp & W Long Valley Rd

Movement	WB	WB	SB	SB	
Directions Served	T	TR	L	R	
Maximum Queue (ft)	1514	1495	32	45	
Average Queue (ft)	791	847	26	22	
95th Queue (ft)	1371	1432	41	46	
Link Distance (ft)	1480	1480	32	32	
Upstream Blk Time (%)	1	2	7	2	
Queuing Penalty (veh)	0	0	3	1	
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

EX AM

1	>	4	*	1	1				
EBL	EBT	WBT	WBR	SBL	SBR				
		44	_		THE RESERVE OF THE PERSON NAMED IN COLUMN TWO IS NOT THE OWNER.				
	Stop			Stop					
0	0	1239	111	72	28				Charles Andrews
0.87	0.87	0.87	0.87	0.87	0.87				
0	0	1424	128	83	32				NO CONTRACTOR
WB 1	WB 2	WB3	SB 1	SB 2					
712	712	128	83	32				Name of the latest of the late	
0	0	0	83	0					
0	0	128	0	32		ISPC/03/NEACHGROUPS	CONTRACTOR US CONTRACTOR	NET ENRICED COLD	Delete St. of Sanda
0.03	0.03	-0.67	0.53	-0.67					
5.0	5.0	3.2	7.7	6.5		adjust on Little grows At Couly	AND DESCRIPTION OF THE PROPERTY OF THE PROPERT	and the same of the same of	Market Control
0.98	0.98	0.11	0.18	0.06					
717	722	1121	462	546		and the same of the same of the same of	-Annabas adecasion	CONTRACTOR DE LA CONTRACTOR DE	THE RESERVE AND ADDRESS OF
49.6	49.6	5.4	11.1	8.7					
46.0			10.4	ACTION AND ADDRESS OF THE PARTY				NO PERSONAL PROPERTY.	NAME OF TAXABLE PARTY.
E			В						
STABLE PERSON									
		43.5							
		Е							
Intersection Capacity Utilization 44.9%		IC	U Level o	f Service		Α			
Analysis Period (min) 15			1000						
	0 0.87 0 WB 1 712 0 0.03 5.0 0.98 717 49.6 46.0 E	Stop 0 0 0.87 0.87 0 0 0 0.87 0.87 0 0 0 WB 1 WB 2 712 712 0 0 0 0.03 0.03 5.0 5.0 0.98 0.98 717 722 49.6 49.6 46.0 E	Stop Stop 0 0 1239 0.87 0.87 0.87 0.87 0 0 1424  WB 1 WB 2 WB 3  712 712 128 0 0 0 128 0.03 0.03 -0.67 5.0 5.0 3.2 0.98 0.98 0.11 717 722 1121 49.6 49.6 5.4 46.0 E  43.5 E on 44.9%	Stop Stop  0 0 1239 111  0.87 0.87 0.87 0.87 0.87  0 0 1424 128  WB 1 WB 2 WB 3 SB 1  712 712 128 83  0 0 0 83  0 0 128 0  0.03 0.03 -0.67 0.53  5.0 5.0 3.2 7.7  0.98 0.98 0.11 0.18  717 722 1121 462  49.6 49.6 5.4 11.1  46.0 10.4  E B  43.5  E  on 44.9% IC	Stop   Stop   Stop	Stop Stop Stop  0 0 1239 111 72 28  0.87 0.87 0.87 0.87 0.87 0.87  0 0 1424 128 83 32  WB 1 WB 2 WB 3 SB 1 SB 2  712 712 128 83 32  0 0 0 83 0  0 0 128 0 32  0.03 0.03 -0.67 0.53 -0.67  5.0 5.0 3.2 7.7 6.5  0.98 0.98 0.11 0.18 0.06  717 722 1121 462 546  49.6 49.6 5.4 11.1 8.7  46.0 10.4  E B  43.5  E  On 44.9% ICU Level of Service	Stop Stop Stop  0 0 1239 111 72 28  0.87 0.87 0.87 0.87 0.87 0.87  0 0 1424 128 83 32  WB 1 WB 2 WB 3 SB 1 SB 2  712 712 128 83 32  0 0 0 83 0  0 0 128 0 32  0.03 0.03 -0.67 0.53 -0.67  5.0 5.0 3.2 7.7 6.5  0.98 0.98 0.11 0.18 0.06  717 722 1121 462 546  49.6 49.6 5.4 11.1 8.7  46.0 10.4  E B  43.5  E  On 44.9% ICU Level of Service	Stop Stop Stop  0 0 1239 111 72 28  0.87 0.87 0.87 0.87 0.87 0.87  0 0 1424 128 83 32  WB 1 WB 2 WB 3 SB 1 SB 2  712 712 128 83 32  0 0 0 83 0  0 0 128 0 32  0.03 0.03 -0.67 0.53 -0.67  5.0 5.0 3.2 7.7 6.5  0.98 0.98 0.11 0.18 0.06  717 722 1121 462 546  49.6 49.6 5.4 11.1 8.7  46.0 10.4  E B	Stop Stop Stop  0 0 1239 111 72 28  0.87 0.87 0.87 0.87 0.87 0.87  0 0 1424 128 83 32  WB 1 WB 2 WB 3 SB 1 SB 2  712 712 128 83 32  0 0 0 83 0  0 0 128 0 32  0.03 0.03 -0.67 0.53 -0.67  5.0 5.0 3.2 7.7 6.5  0.98 0.98 0.11 0.18 0.06  717 722 1121 462 546  49.6 49.6 5.4 11.1 8.7  46.0 10.4  E B

## Intersection: 3: N 101 On Ramp & W Long Valley Rd

Movement	WB	WB	WB	SB	SB	
Directions Served	Т	Т	R	L	R	
Maximum Queue (ft)	137	140	80	52	51	
Average Queue (ft)	84	83	9	27	18	
95th Queue (ft)	118	119	39	44	47	
Link Distance (ft)	1481	1481	1481	33	33	
Upstream Blk Time (%)				7	2	
Queuing Penalty (veh)		-		3	1	
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

EX AM (LANE ADD)

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