

APPENDIX D

Geotechnical Investigation



Geotechnical
Environmental
Hydrogeology
Material Testing
Construction Inspection

May 8, 2020

Project No. 20-7009

Robert Hoff
Hilltop 3 Development, LLC
3875 Crest Drive
Yorba Linda, CA 92886

Subject: Preliminary Geotechnical Investigation Report, Tentative Parcel Map No. 2020-125, Yorba Linda, California

Robert,

In accordance with your request and authorization, TGR Geotechnical, Inc. (TGR) has performed a preliminary geotechnical investigation for the proposed development at the subject site. This report presents the findings of our limited geotechnical investigation, including site seismicity and provides preliminary design geotechnical recommendations for the proposed development. The work was performed in general accordance with our proposal dated February 5, 2020 and your subsequent authorization to proceed.

It is our understanding that the proposed development will consist of the construction of a single-family residence. The proposed development is suitable from a geotechnical viewpoint provided the recommendations presented in this report are incorporated during design and construction.

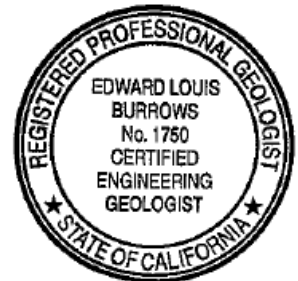
If you have any questions regarding this report, please do not hesitate to contact this office. We appreciate this opportunity to be of service.

Respectfully submitted,

TGR GEOTECHNICAL, INC.



Sanjay Govil, PhD, PE, GE 2382
Principal Geotechnical Engineer



Edward L. Burrows, M.S, PG, CEG 1750
Principal Engineering Geologist

Distribution: (4) Addressee

Attachments

Plate 1 – Geotechnical Map
Plate 2 – Geotechnical Cross-Section A-A', B-B' and C-C'
Figure 1 – Site Location Map
Figure 2 – Geologic Map
Figure 3 – Historic High Groundwater Map
Figure 4 – Regional Fault Map
Figure 5 – Seismic Hazard Zone Map

Appendix A – References
Appendix B – Log of Borings
Appendix C – Laboratory Test Results
Appendix D – Slope Stability Analysis
Appendix E – Earthwork and Grading Guidelines

INTRODUCTION

Site Description and Proposed Project Development

It is our understanding that the proposed development consists of a single-family residence located off of Fairmont Boulevard, Yorba Linda, California. The subject property is an ascending slope and hilltop. The site is bounded on the north by the Yorba Linda Water District (YLWD) Little Canyon Reservoir; on the south by residential development and Fallen Leaf Road; on the east by residential development and Fairmont Blvd; and on the west by undeveloped land. The approximate site location is presented in the Site Location Map (Figure 1).

The proposed residential development will consist of a single building pad at the northeast portion of the site with a pad elevation of 965 feet above mean sea level (amsl). A large fill/cut slope ranging up to approximately 185 feet in height is proposed to the south and west of the building pad. Access to the pad area will be via a road constructed within the slope area. The maximum anticipated cut is approximately 50 feet at the southwest portion of the site. The maximum anticipated fill is approximately 50 feet in the central portion of the site. The maximum proposed cut slope is 185 feet and the maximum proposed fill slope is 150 feet. Fill and cut slope gradients are 2H:1V (horizontal:vertical).

Reports by Others

A geotechnical investigation report was previously prepared by Converse Ward Davis Dixon, Inc. (Converse, 1981) for the existing YLWD Little Canyon Reservoir which is located directly north of the subject site. TGR reviewed Converse's report in preparing this report.

Scope of Work

The scope of work for this geotechnical investigation included the following:

- Site reconnaissance.
- Review of readily available geotechnical and geologic reports for nearby properties.
- Sampling and logging three (3) large diameter (24") borings utilizing a bucket auger drill rig to depths ranging from 31 to 75 feet at the subject site to evaluate subsurface soil conditions. The borings were backfilled with cuttings and any excess soil was disposed onsite.
- Laboratory testing of selected samples to include in-situ moisture density, maximum density and optimum moisture content, shear, expansion, and sulfate.
- Engineering analysis including static slope stability, seismic slope stability including evaluation of seismic displacement, foundation and retaining wall design.
- Preparation of this appropriately illustrated report, including geologic cross-sections. The report summarizes the results of the geologic and geotechnical investigation, evaluate subsurface soil condition, site seismicity, and slope stability.

Field Investigation

Field exploration was performed on April 20 and 21, 2020 by a certified engineering geologist from our firm who logged the borings and obtained representative samples, which were subsequently transported to the laboratory for further review and testing. The approximate locations of the borings are indicated on the enclosed Geotechnical Map (Plate 1).

The subsurface conditions were explored by drilling, sampling, and down-hole logging three borings with a truck mounted bucket auger drill rig to approximate depths ranging from thirty-one (31) to seventy-five (75) feet below existing grade. Subsequent to drilling and logging, all borings were backfilled with cuttings. Soil descriptions were entered on the logs in general accordance with the Unified Soil Classification System (USCS). The logs of borings presenting soil conditions and descriptions are given in Appendix B.

The drill rig was equipped with a sampling apparatus to allow for recovery of driven modified California Ring Sampler (CRS), 3-inch outside diameter, and 2.42-inch inside diameter. Driven samples and bulk samples of the earth materials encountered at selected intervals were recovered from the borings.

The samples were driven using a Kelly bar falling freely from a height of 30 inches. The weight of the Kelly bar changed with increasing depth and was 3300 pounds in the upper 25 feet below ground surface, 2200 pounds between 25 and 50 feet below ground surface and 1100 pounds between 50 and 75 feet below ground surface. The locations and depths of the soil samples recovered are indicated on the logs in Appendix B.

Laboratory Testing

Laboratory tests were performed on representative samples to verify the field classification of the recovered samples and to evaluate the geotechnical properties of the subsurface soils. The following tests were performed:

- In-situ moisture content (ASTM D2216) and dry density (ASTM D7263);
- Maximum Dry Density and Optimum Moisture Content (ASTM D1557);
- Direct Shear Strength (ASTM D3080);
- Expansion Potential (ASTM D4829);
- Atterberg (ASTM D423); and
- Soluble Sulfate (CAL.417A)

Laboratory tests for geotechnical characteristics were performed in general accordance with the ASTM procedures. The results of the in-situ moisture content and density tests are shown on the borings logs. The results of the laboratory tests are presented in Appendix C.

GEOTECHNICAL FINDINGS

Earth Units/Geology

The Bedrock exposed at the surface and underlying the general site area is part of the late Miocene aged Puente Formation. According to the USGS Professional Paper 420-B, Geologic Map of the Prado Dam and Yorba Linda Quadrangles, (Dunham and Yerkes, 1964), the bedrock at the subject site is the La Vida member of the Puente Formation which generally consists of interbedded siltstone and sandstone.

Groundwater

During the field exploration, groundwater was not encountered to the maximum depth drilled (approximately 75 feet). This site is underlain by bedrock and a review of the seismic hazard zone maps for the Yorba Linda quadrangle does not show groundwater mapped beneath the subject site.

Generally, seasonal and long-term fluctuations in the groundwater may occur as a result of variations in subsurface conditions, rainfall, run-off conditions and other factors. Therefore, variations from our observations may occur.

Faulting and Seismicity

We consider the most significant geologic hazard to be the potential for moderate to strong seismic shaking that is likely to occur at the subject site. The subject site is located in the highly seismic Southern California region within the influence of several faults that are considered to be active or potentially active. An active fault is defined by the State of California as a "sufficiently active and well-defined fault" that has exhibited surface displacement within the Holocene time (about the last 11,000 years). A potentially active fault is defined by the State as a fault with a history of movement within Pleistocene time (between 11,000 and 1.6 million years ago).

These active and potentially active faults are capable of producing potentially damaging seismic shaking at the site. It is anticipated that the subject site will periodically experience ground acceleration as a result of small to moderate magnitude earthquakes. Other active faults without surface expression (blind faults) or other potentially active seismic sources that are not currently zoned and may be capable of generating an earthquake are known to be present under the region.

Based on our review of the referenced geologic maps, as well as our field reconnaissance, the subject site is not included within any Earthquake Fault Zones as created by the Alquist-Priolo Earthquake Fault Zoning Act (Hart, 1997). Our review of geologic literature pertaining to the site area indicates that there are no known active or potentially active faults located within or immediately adjacent to the subject property. The nearest fault is the Whittier Fault located approximately 0.5 miles from the subject site. Other faults close to the site are Chino Fault (5.0 miles away), and Peralta Hills Fault (5.5 miles away). The Regional Fault Map (Figure 4) presents the location of the site with respect to the regional faults

Surface Fault Rupture and Ground Shaking

Since no known faults are located within the site, surface fault rupture is not anticipated. However, due to the close proximity of known active and potentially active faults, severe ground shaking should be expected during the life of the proposed structures.

Liquefaction

Liquefaction is a seismic phenomenon in which loose, saturated, fine-grained granular soils behave similarly to a fluid when subjected to high-intensity ground shaking. Liquefaction occurs when these ground conditions exist: 1) Shallow groundwater; 2) Low density, fine, clean sandy soils; and 3) High-intensity ground motion. Effects of liquefaction can include sand boils, settlement, and bearing capacity failures below foundations.

The site is underlain by bedrock. A review of the seismic hazard zone map of the Yorba Linda Quadrangle indicates that the subject site is not located within an area having a potential for earthquake induced liquefaction (Figure 5). As such, the potential for liquefaction at the subject site is considered to be negligible.

Seismically Induced Settlement

Ground accelerations generated from a seismic event can produce settlements in sands or in granular earth materials both above and below the groundwater table. This phenomenon is often referred to as seismic settlement and is most common in relatively clean sands, although it can also occur in other soil materials. Based on the nature of the soils (bedrock) underlying the site the potential for seismically induced settlement is considered negligible.

Lateral Spreading

Seismically induced lateral spreading involves primarily movement of earth materials due to earth shaking. Lateral spreading is demonstrated by near-vertical cracks with predominantly horizontal movement of the soil mass involved. Based on the nature of the soils (bedrock) underlying the site the potential for lateral spreading at the subject site is considered negligible.

Earthquake Induced Landsliding

A review of the seismic hazard zone map of the Yorba Linda Quadrangle indicates that the subject site is not located within an area having a potential for earthquake induced landsliding (Figure 5). The site is underlain by bedrock. The bedrock is overlain by approximately 2 to 3.5 feet of topsoil/colluvium. Evidence of ancient or recent landsliding was not observed on the property at the time of our field study and site visit. In addition, the property did not reveal the presence of past surficial slope failures.

Slope Stability

Slope stability calculations are presented in Appendix D on geologic cross-sections A-A', B-B' and C-C'. Based on the calculations, site slopes are considered grossly stable. Seismic displacement was calculated by evaluating the yield acceleration that corresponds to a safety factor of 1.0 and calculating seismic displacement per Bray and Travarasou (2007; 2009), Rathje and Antonakos (2011) and Song and Marek (2015) and taking the average of the two. The design minimum factors of safety under static loading conditions is 1.5. The acceptable seismic displacement is 5 cm for slide planes going through structures, 15 cm otherwise.

Presented below are the results of our slope stability analyses for cross sections A-A', B-B' and C-C':

Cross Section	Static Safety Factor	Ky (g)	Displacement (cm)	Seismic Safety Factor
A – A'	1.62	0.30	3.75	1.00
B – B'	1.70	0.36	1.22	1.01
C-C'	1.61	0.35	2.25	1.02

The soil shear strength parameters utilized in the slope stability calculations are presented below.

Geologic Unit	Unit Weight (pcf)	Static Stability (Ultimate)		Seismic Stability (Ultimate)	
		Cohesion (Psf)	Friction Angle (degree)	Cohesion (Psf)	Friction Angle (degree)
Fill (95%)	130	275	28	460	30
(Tpl) Bedrock: Across Bedding	130	198	36	428	47
(Tpl) Bedrock: Along Bedding	130	336	18	336	18

The surficial stability analysis indicated an adequate factor of safety. Calculations are presented in Appendix D.

DISCUSSIONS AND CONCLUSIONS

General

Based on our field exploration by others and geotechnical engineering analysis, the proposed development is considered suitable from a geotechnical viewpoint, provided the recommendations contained in this report are incorporated into the design and construction phases of the project. It is our opinion that the proposed development and proposed grading will be safe against hazard from landslide, settlement, or slippage, and the proposed construction will have no adverse effect on the geologic stability of the adjacent properties provided our recommendations presented in this report are followed.

Conclusions

Based on our findings and analyses, the subject site is likely to be subjected to moderate to severe ground shaking due to the proximity of known active and potentially active faults. This may reasonably be expected during the life of the structure and should be designed accordingly.

The engineering evaluation performed concerning site preparation and the recommendations presented are based on information provided to us and obtained by us during our office and fieldwork. This report is prepared for the proposed residential development at the subject site. In the event that any significant changes are made to the proposed development, the conclusions and recommendations contained in this report shall not be considered valid unless the changes are reviewed, and the recommendations of this report are verified or modified in writing by TGR.

RECOMMENDATIONS

Seismic Design Parameters

When reviewing the 2019 California Building Code, the following data should be incorporated into the design:

Latitude (degree)	33.908
Longitude (degree)	-117.769
Site Class	C
Site Coefficient, F_a	1.2
Site Coefficient, F_v	1.4
Mapped Spectral Acceleration at 0.2-sec Period, S_s	1.986 g
Mapped Spectral Acceleration at 1.0-sec Period, S_1	0.694 g
Spectral Acceleration at 0.2-sec Period Adjusted for Site Class, S_{MS}	2.383 g
Spectral Acceleration at 1.0-sec Period Adjusted for Site Class, S_{M1}	0.972 g
Design Spectral Acceleration at 0.2-sec Period, S_{DS}	1.589 g
Design Spectral Acceleration at 1.0-sec Period, S_{D1}	0.648 g

The structural consultant should review the above parameters and the 2019 California Building Code to evaluate the seismic design.

Conformance to the criteria presented in the above table for seismic design does not constitute any type of guarantee or assurance that significant structural damage or ground failure will not occur during a large earthquake event. The intent of the code is “life safety” and not to completely prevent damage of the structure, since such design may be economically prohibitive.

Shallow Foundation Design Recommendations

Shallow foundations may be utilized to support the proposed development. Continuous or pad footings shall be supported on a minimum of three feet of engineered fill. It is recommended that the continuous footings be a minimum of 18 inches wide and a minimum of 24 inches deep. An allowable bearing capacity of 2500 psf is recommended for both pad and continuous footings. The minimum horizontal foundation setback for descending slopes is $H/3$ (maximum 40 feet), where H is the height of the slope. Deepened footings may be required to meet the setback requirement. The minimum horizontal setback for ascending slopes is $H/2$ (maximum 15 feet).

Fill slopes and pad areas consisting of expansive soils are expected to have some degree of slope creep and lot stretching/lateral fill extension. Slope creep and lot stretching/lateral fill extension are expected to be particularly prevalent within the outer fifteen (15) feet of the slope face. The amount of movement is difficult to determine and is dependent to some extent upon the height and angle of the slope, the degree of the expansion potential, and irrigation practices.

A minimum foundation setback of 15 feet from the outer edge of the top of slope for free-sanding walls to the slope face daylight is recommended.

The total settlement is not expected to exceed 1 inch. The differential settlement between columns is estimated as 1/2 inch.

The above values may be increased by one-third when considering short duration seismic or wind loads. The reinforcement should be designed by the project structural engineer.

Resistance to lateral loads including wind and seismic forces may be provided by frictional resistance between the bottom of concrete and the underlying fill soils and by passive pressure against the sides of the foundations. A coefficient of friction of 0.40 may be used between concrete foundation and underlying soil. The recommended passive pressure of the engineered fill may be taken as an equivalent fluid pressure of 250 pounds per cubic foot (2,500 psf max).

Retaining Wall Design Recommendations

Proposed retaining walls may be supported on shallow foundations or drilled piers as described below. The following soil parameters may be used for the design of retaining walls up to 10 feet high with level backfill or 2:1 backfill:

<u>Conditions</u>	<u>Equivalent Fluid Pressure</u> <u>(psf/ft)</u>
Active (Level)	45
Active (2:1 Backfill)	65
At Rest (Level)	60
Allowable Passive	275 (maximum 2750)

- An allowable coefficient of friction between on-site soil and concrete of 0.35 may be used with the dead-load forces.
- Passive pressure and frictional resistance could be combined in determining the total lateral resistance. However, one of them shall be reduced by 50 percent.
- Walls near ascending slopes should be designed for creep loading.
- The passive pressure of the soil within the creep zone shall be ignored in the design.
- Retaining walls over 6 feet high shall be designed for a seismic lateral load of $17H^2$ pounds. The seismic load shall be applied at a distance of $0.6H$ above the base of the wall.
- Drilled piers should be designed for the passive pressure presented above.

Retaining structures should be provided with a drainage system to prevent buildup of hydrostatic pressure behind the walls unless the wall is designed for the added hydrostatic pressure. Provisions should be made to collect and dispose of excess water away from the wall. Wall drainage may be provided by a perforated pipe encased in gravel or crushed rock and enclosed by geo-synthetic filter fabric. We do not recommend omitting the drains behind walls.

In addition to the above lateral forces due to retained earth, surcharge due to improvements, such as an adjacent walkway, should be considered in the design of the retaining wall. A minimum vertical surcharge load of 300 psf should be used in design of walls due to adjacent traffic unless the traffic is kept at least 10 feet from the walls. Loads applied within a 1:1 projection from any surcharging structure on the stem of the wall shall be considered as lateral surcharge. For uniform lateral surcharge conditions applied to free-to-deflect walls we recommend utilizing a minimum horizontal load equal to 33 percent of the vertical load and should be applied uniformly over the entire height of the wall.

Cement Type and Corrosion

Preliminary testing indicates that concrete used should be designed in accordance with the provisions of ACI 318 for Exposure Class S0. However, due to the presence of gypsum within the bedrock, testing of soils at the completion of grading should be performed to determine sulfate content.

TGR does not practice corrosion engineering. If needed, a qualified specialist should review the site conditions and evaluate the corrosion potential of the site soil to the proposed improvements and to provide the appropriate corrosion mitigations for the project.

Expansive Soils

The near surface site soils have an expansion index of 41-58, which correlates to a low to medium expansion potential.

Shrinkage/Bulking

Removal and recompaction of the near surface soils (topsoil/colluvium) is estimated to result in shrinkage ranging from 10 to 15 percent. Removal and recompaction of the bedrock is estimated to result in bulking ranging from 0 to 5 percent.

Excavatability of Onsite Bedrock

The major portion of the proposed excavation is expected to be within bedrock. This material is expected to be excavatable by conventional earth-moving equipment. It is anticipated that, within the expected depth of excavation, no blasting will be required; However, some well cemented discontinuous zones which might require heavy ripping might be encountered during excavation. Zones of well cemented sandstone were encountered in boring BA-1 at depths ranging from 31 to 32 feet, 38 to 41 feet and 48 to 51 feet.

Slabs on Grade

Interior slabs may be supported on grade. The slab should be a minimum of 5-inches thick and reinforced with a minimum of No. 4 rebars at 12-inches on center. The slabs shall be supported on a minimum of three feet of engineered fill. All the interior slabs should be underlain by a minimum of fifteen- (15) mil thick Visqueen vapor barrier with all laps sealed. Two (2) inches of sand should be placed over the membrane as well as under the membrane. The subgrade material should be compacted to a minimum of 95 percent of the maximum laboratory dry density (ASTM 1557). Prior to placement of concrete, the subgrade soils should be well moistened to at least optimum moisture content to a depth of 18 inches. The slab should also be designed in accordance with 2019 CBC to include the expansive nature of the soil.

The structural details, such as slab thickness, concrete strength, amount and type of reinforcements, joint spacing, etc., should be established by the structural engineer.

Depending on the location of the building footprint, the slab may be underlain by a differential fill thickness. As such, consideration should be given to the use of a post-tensioned slab or interior grade beams to limit differential movement. Geotechnical recommendations can be provided upon request.

Flatwork Design

Flatwork should be a minimum of 4-inches thick should be reinforced with a minimum of No. 3 reinforcing bar on 12-inch centers in two horizontally perpendicular directions. Reinforcing should be properly supported to ensure placement near the vertical midpoint of the slab. "Hooking" of the reinforcement is not considered an acceptable method of positioning the steel. The slab should not be structurally connected to the buildings. The actual thickness, reinforcement, and spacing of control joints of the slab shall be designed by the structural engineer and should include the anticipated loading condition. The subgrade material should be compacted to a minimum of 95 percent of the maximum laboratory dry density (ASTM 1557) to a minimum depth of 12-inches. Prior to placement of concrete, the subgrade soils should be well moistened to at least optimum moisture content and verified by our field representative.

Site Development Recommendations

General

During earthwork construction, all site preparation and the general procedures of the contractor should be observed, and the fill selectively tested by a representative of TGR. If unusual or unexpected conditions are exposed in the field, they should be reviewed by this office and if warranted, modified and/or additional recommendations will be offered.

Grading

All grading should conform to the guidelines presented in the Appendix E, California Building Code (2019 edition), except where specifically superseded in the text of this report. Prior to grading, TGR's representative should be present at the pre-construction meeting to provide grading guidelines, if needed, and review any earthwork.

Site Preparation

Prior to initiating grading operations, any existing vegetation, debris, oversized materials (greater than 4 inches), and other deleterious materials within fill areas should be removed.

Surficial Soil Removals

The existing fill, topsoil, colluvium and weathered bedrock materials should be removed to expose competent bedrock as directed by the project geotechnical engineer or engineering geologist. Based on our subsurface exploration the upper approximately 3 to 9 feet of onsite materials will require removal. The actual removal depths should be determined in the field as conditions are exposed.

Treatment of Removal Bottoms

Soils exposed within areas approved for fill placement should be scarified to a depth of 6 inches, conditioned to near-optimum moisture content, then compacted in-place to project standards.

Fill Placement

The onsite soils may be used as compacted fill provided, they are free of organic materials and debris. Fills should be placed in relatively thin lifts, brought to near optimum moisture content, then compacted to at least 95 percent relative compaction based on laboratory standard ASTM D-1557.

Compaction

Prior to fill placement, the exposed surface should be scarified to a minimum depth of six (6) inches, fill placed in six (6) inches loose lifts, moisture conditioned to at least optimum moisture content, and compacted to a minimum relative compaction of ninety-five (95) percent in accordance with ASTM D 1557.

Fill Slopes

Permanent fill slopes should be constructed no steeper than 2H:1V and keyed and benched into approved bedrock materials. A minimum relative compaction of 95 percent out to the finish slope face for fill slopes is required. This may be achieved by either overbuilding the slope and cutting back to the compacted core, or by direct compaction of the slope face with suitable equipment, or by any other procedure which produces the required compaction.

The fill slopes shall have a subdrain system. Vertical spacing, outlet spacing, pipe and filter material should be in accordance with the recommendations presented in Appendix E – Earthwork and Grading Guidelines, Typical Stabilization and Buttress Fill Subdrain Detail.

Fill Keys and Benching

Fill keys should be constructed at the toe of the fill slope at the southeast portion of the site near the site entrance off Fairmont Blvd. See Plate 1, Geotechnical Map, for proposed fill key locations. The fill keys should have a minimum width of 25 feet and a minimum depth of 5 feet into competent bedrock. All fill keys should be observed and approved by the project geotechnical consultant prior to placing fill. Fills placed on slopes steeper than 5H:1V should be keyed and benched into competent bedrock as the fills are placed. See Appendix E. Earthwork and grading Guidelines (Typical Fill over Natural Slope detail) for benching recommendations.

Cut Slopes

The proposed cut slope located in the west-southwest portion of the site is anticipated to expose bedrock with bedding favorable to slope stability. As such, this slope is anticipated to be grossly stable. The cut slope should be observed during excavation to verify that existing conditions are consistent with those anticipated. If adverse bedrock conditions are encountered, appropriate remedial recommendations shall be provided by the geotechnical consultant.

Cut/Fill Transition

The building pad will have a cut/fill transition. Typical recommendations for over-excavation of the cut portion of the cut/fill transition are presented in Appendix E, Earthwork and grading Guidelines (Typical Overexcavation of Daylight Line detail). However, due to the anticipated differential pad fill thickness, TGR recommends that the cut portion of the cut fill transition be overexcavated a minimum of 10 feet below pad grade.

Trenching

All excavations should conform to CAL-OSHA and local safety codes.

Drainage

Positive site drainage should be maintained at all times. Drainage should not flow uncontrolled down any descending slope or retaining wall. Water should be directed away from foundations and not allowed to pond and/or seep into the ground. Pad drainage should be directed towards the street/parking or other approved area. Roof gutters and down spouts should be utilized to control roof drainage. Down spouts should outlet a minimum of 5 feet from the proposed structure or into an approved subsurface drainage system. We would recommend that any proposed open-bottom planters adjacent to proposed structures be eliminated for a minimum distance of 10 feet. As an alternative, closed-bottom type planters could be utilized. An outlet placed in the bottom of the planter could be installed to direct drainage away from structures or any exterior concrete flatwork.

Utility Trench Backfill

All utility trench backfills in structural areas and beneath hardscape features should be brought to at least optimum moisture content and compacted to a minimum relative compaction of 90 percent of the laboratory standard. Flooding/jetting is not recommended.

Sand backfill, (unless trench excavation material), should not be allowed in parallel exterior trenches adjacent to and within an area extending below a 1:1 plane projected from the outside bottom edge of the footing. All trench excavations should minimally conform to CAL-OSHA and local safety codes. Soils generated from utility trench excavations may be used provided it is moisture conditioned and compacted to 90 percent minimum relative compaction.

Temporary Excavation Recommendations

Soils may be cut vertically without shoring to a depth of approximately four (4) feet below adjacent grade. For deeper cuts, the slopes should be properly shored or sloped back to at least 1H:1V or flatter. The exposed slope face should be kept moist (but not saturated) during construction to reduce local sloughing. No surcharge loads should be permitted within a horizontal distance equal to the height of cut from the toe of excavation unless the cut is properly shored. Excavations that extend below an imaginary plane inclined at 45 degrees below the edge of any nearby adjacent existing site facilities should be properly shored to maintain foundation support for the adjacent structures.

Preventative Slope Maintenance

For the slopes, it is important to reduce the risk of problems relating to slope instability. It is recommended that a program be implemented for aggressive slope maintenance to include; annual cleanout of drains, elimination of burrowing rodents, maintaining drought and fire resistant, deep-rooted ground cover, and proper irrigation.

Hillside properties are typically subject to potential geotechnical hazards including mudslides, spalling of slopes, erosion, and concentrated flows. It must be emphasized that responsible maintenance of these slopes, and the property in general, by the owner, using proper methods, can reduce the risk of these hazards significantly.

Geotechnical Review of Plans

All grading and foundation plans should be reviewed and accepted by the geotechnical consultant prior to construction. If significant time elapses since preparation of this report, the geotechnical consultant should verify the current site conditions, and provide any additional recommendations (if necessary) prior to construction.

Geotechnical Observation/Testing During Construction

Per sections 1705.6 and table 1705.6 of the 2019 California Building Code, periodic special inspection shall be performed to:

- Verify materials below shallow foundations are adequate to achieve the design bearing capacity;
- Verify excavations are extended to the proper depth and have reached proper material;
- Verify classification and test compacted materials; and
- Prior to placement of compacted fill, inspect subgrade and verify that the site has been prepared properly

Per sections 1705.6 and table 1705.6 of the 2019 California Building Code, continuous special inspection shall be performed to:

- Verify use of proper materials, densities and lift thickness during placement and compaction of compacted fill.

The geotechnical consultant should also perform observation and/or testing at the following stages:

- During any grading and fill placement;
- Prior to pouring foundation or flatwork concrete;
- During trench excavation;
- Excavation bottom;
- Placement of bedding material;
- During trench backfill;
- Subgrade for flatwork;
- When any unusual soil conditions are encountered during any construction operation subsequent to issuance of this report.

Limitations

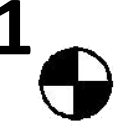
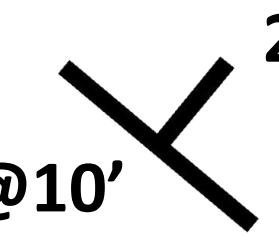
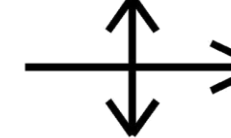
This report was prepared for a specific client and a specific project, based on the client's needs, directions and requirements at the time.

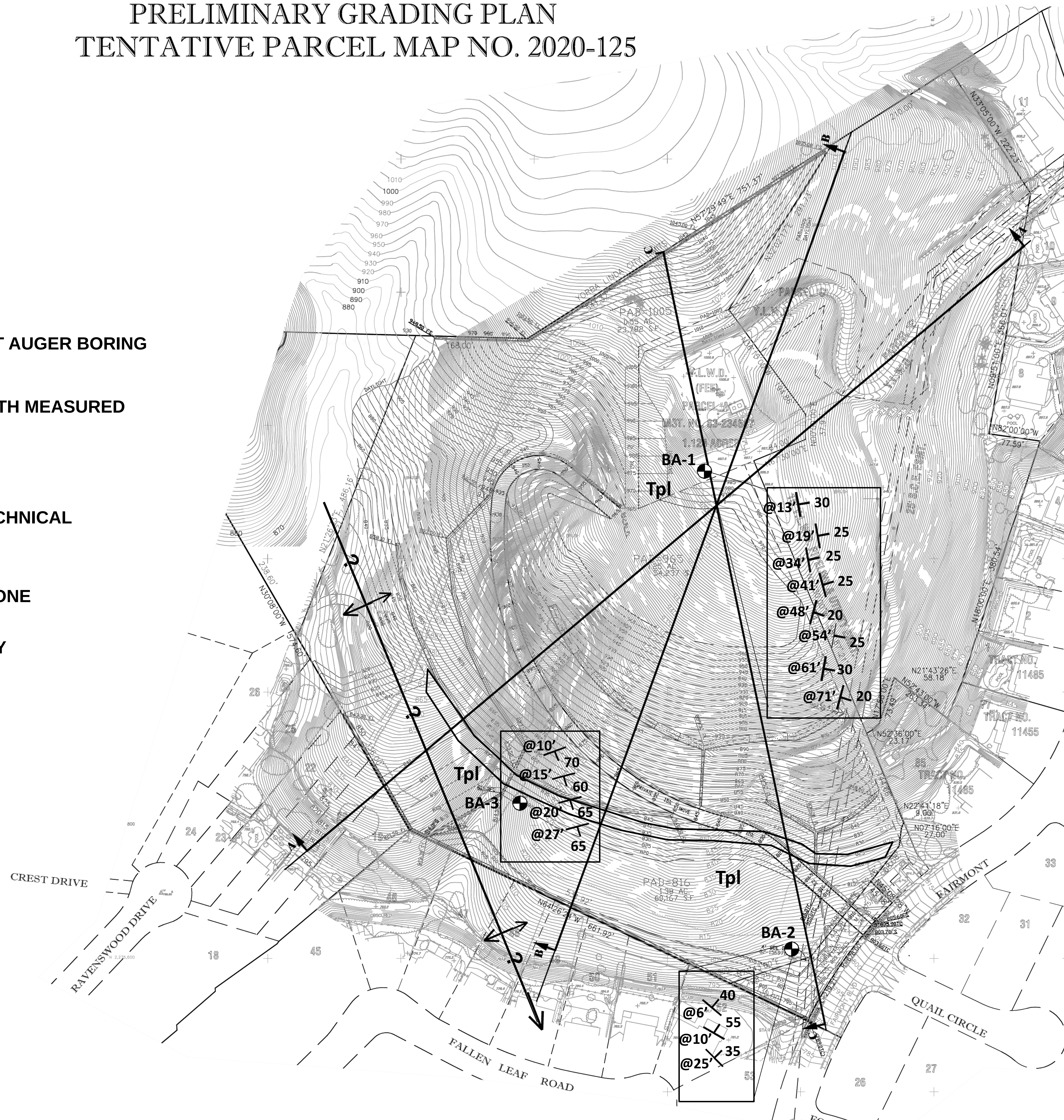
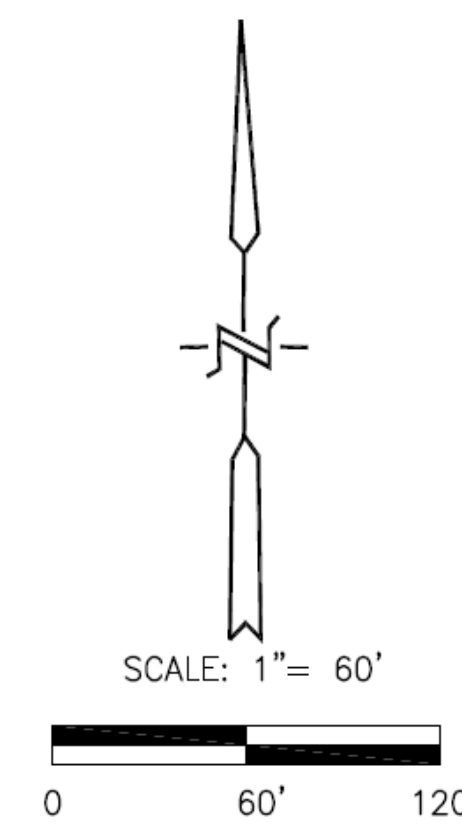
This report was necessarily based in part upon data obtained from a limited number of observances, site visits, soil and/or other samples, tests, analyses, histories of occurrences, spaced subsurface exploration and limited information on historical events and observations. Such information is necessarily incomplete. Variations can be experienced within small distances and under various climatic conditions. Changes in subsurface conditions can and do occur over time.

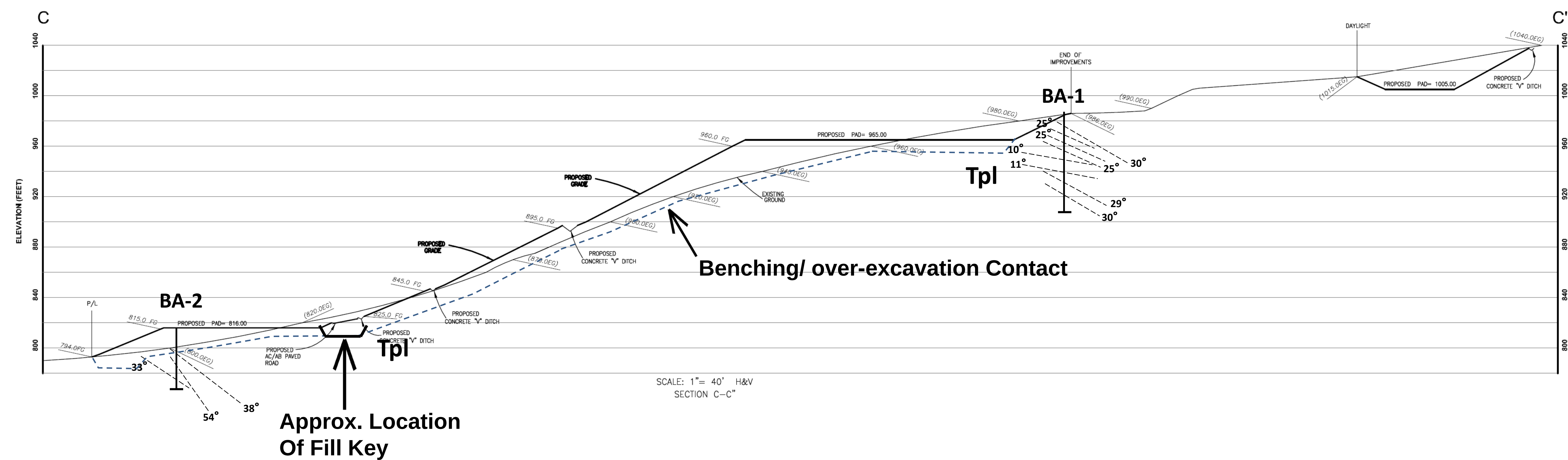
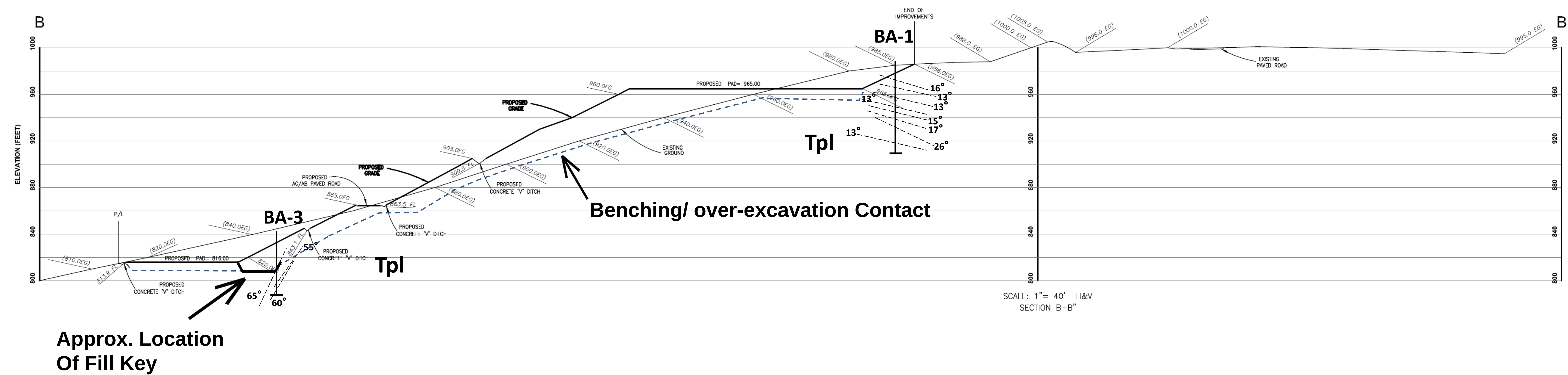
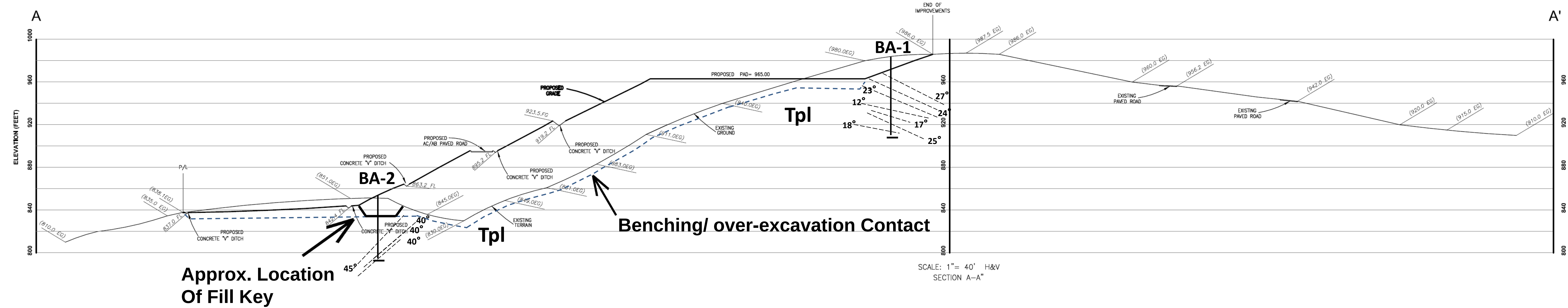
This report is not authorized for use by and is not to be relied upon by any party except the client with whom TGR contracted for the work. Use or reliance on this report by any other party is that party's sole risk. Unauthorized use of or reliance on this report constitutes and agreement to defend and indemnify TGR from and against any liability which may arise as a result of such use or reliance, regardless of any fault, negligence, or strict liability of TGR.

PRELIMINARY GRADING PLAN
TENTATIVE PARCEL MAP NO. 2020-125

EXPLANATION:

- BA-1  APPROXIMATE LOCATION OF BUCKET AUGER BORING BY TGR
- @10'  STRIKE AND DIP OF BEDDING, At DEPTH MEASURED
-  ANTICLINAL FOLD AXIS
-  APPROXIMATE LOCATION OF GEOTECHNICAL BORINGS
- Tpl PUENTE FORMATION, SILTY SANDSTONE
-  APPROXIMATE LOCATION OF FILL KEY





SEE PLATE 1 FOR EXPLANATION

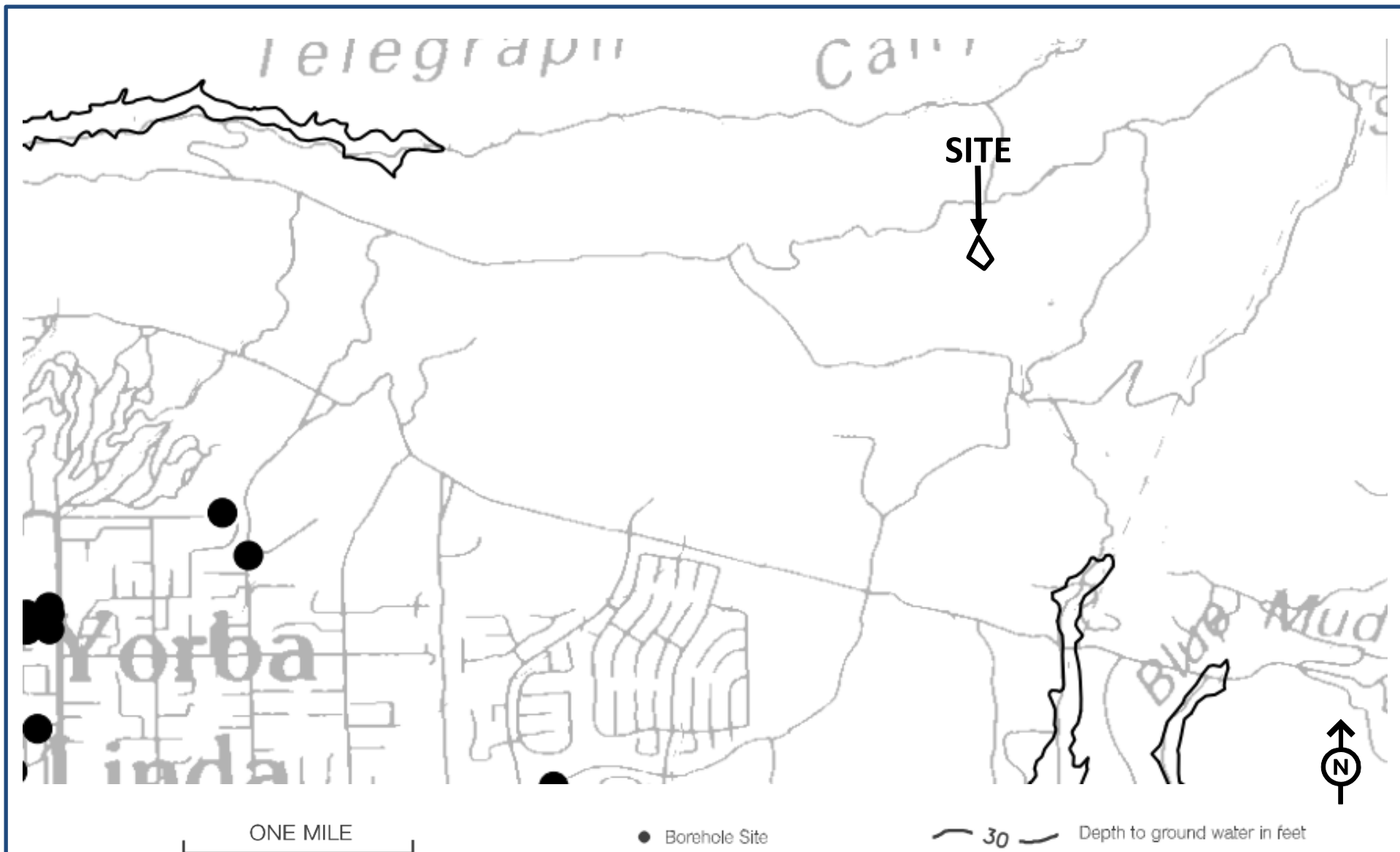


Geotechnical
Environmental
Hydrogeology
Material Testing
Construction Inspection

**SITE LOCATION MAP
HOFF PROPERTY
FAIRMONT BOULEVARD, YORBA LINDA, CALIFORNIA**

PROJECT NO. 20-7009

FIGURE 1

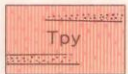


Modified From: California Department of Conservation, Division of Mines and Geology, 2005, Seismic Hazard Zone Report for the Yorba Linda 7.5-Minute Quadrangles, Los Angeles, Orange and San Bernardino Counties, California, Report 010.



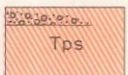
Sycamore Canyon member

Light-yellowish-brown to brown pebble conglomerate and conglomeratic sandstone; light-yellowish-brown fine- to medium-grained thin-bedded to massive feldspathic sandstone; and light-gray fairly well bedded to massive siltstone. Rapid lateral gradations in lithology. In Prado Dam quadrangle, uppermost part is white sandstone, gravel, and siltstone, possibly of Pliocene age in part. Sandstone and conglomerate units shown by lithologic symbols



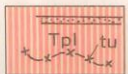
Yorba member

Dark-brown to pinkish-gray poorly bedded siltstone with hackly fracture; light-gray to white platy siltstone; soft brownish-gray paper-thin siltstone; light-gray punky diatomaceous siltstone; locally with brownish-gray medium- to coarse-grained sandstone beds. Sandstone units shown by lithologic symbol



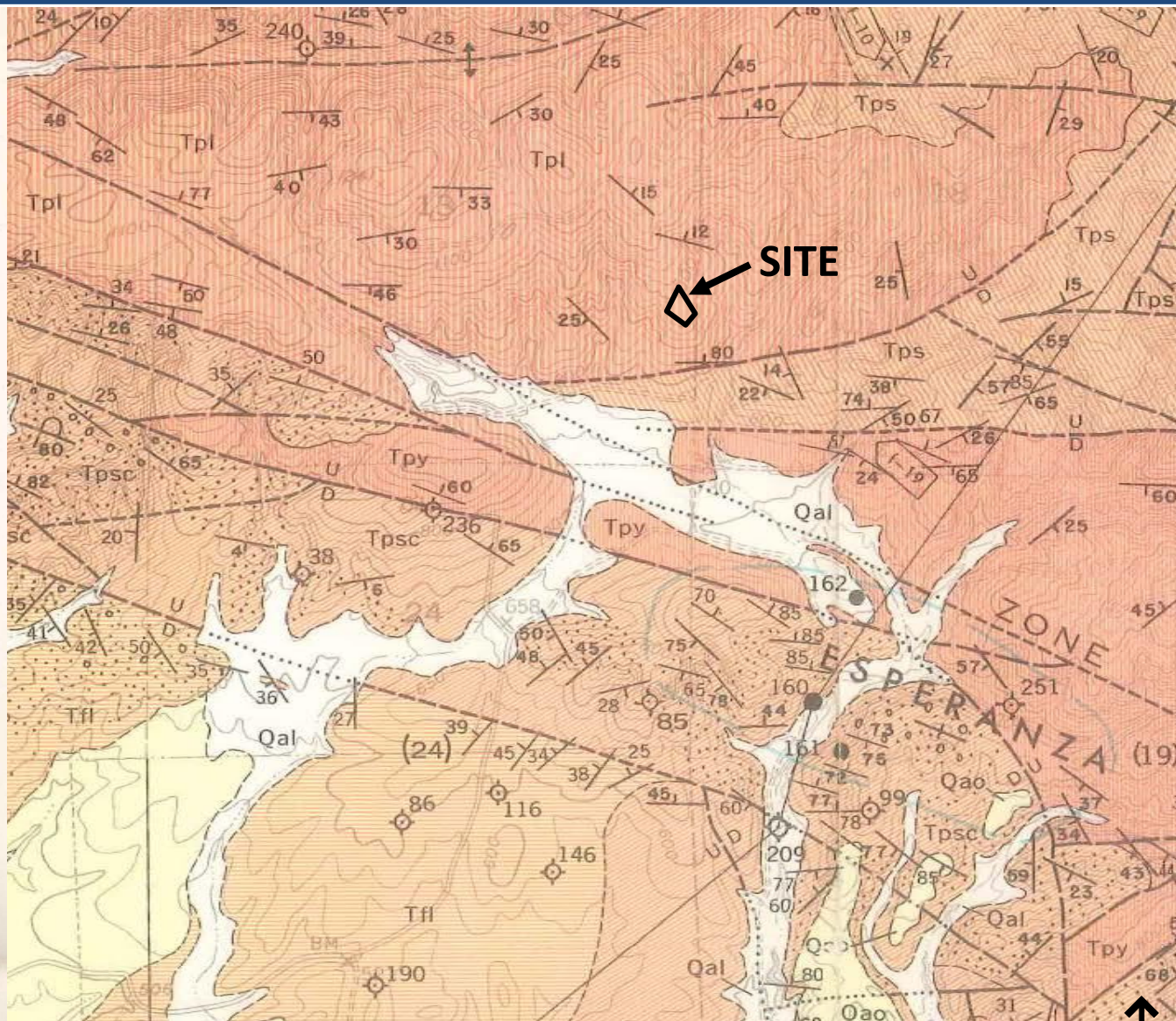
Soquel member

Upper part: light-gray to light-yellowish-brown medium-grained to pebbly feldspathic sandstone with interbedded light-gray to light-yellowish-brown siltstone; numerous 2- to 12-foot rounded boulders of granitic rock along northern border of Yorba Linda quadrangle; lower part: light-gray to light-yellowish-brown thick-bedded to massive medium- to coarse-grained and pebbly feldspathic sandstone, commonly with large concretions; minor amounts of interbedded siltstone. Sandstone and conglomerate units shown by lithologic symbols



La Vida member

Gray to white platy siltstone with white limy concretions and brownish-gray to light-gray soft micaceous siltstone; thin interbedded light-gray sandstone; tan andesitic tuff, tu. Sandstone units shown by lithologic symbol



D.L. Durham and R.F. Yerkes, Geologic map of the Prado Dam and Yorba Linda (eastern Puente Hills), Los Angeles, Orange, Riverside and San Bernardino Counties, California: U.S. Department of the Interior Geological Survey, Professional Paper 420-B, Plate 1, scale 1:24,000.

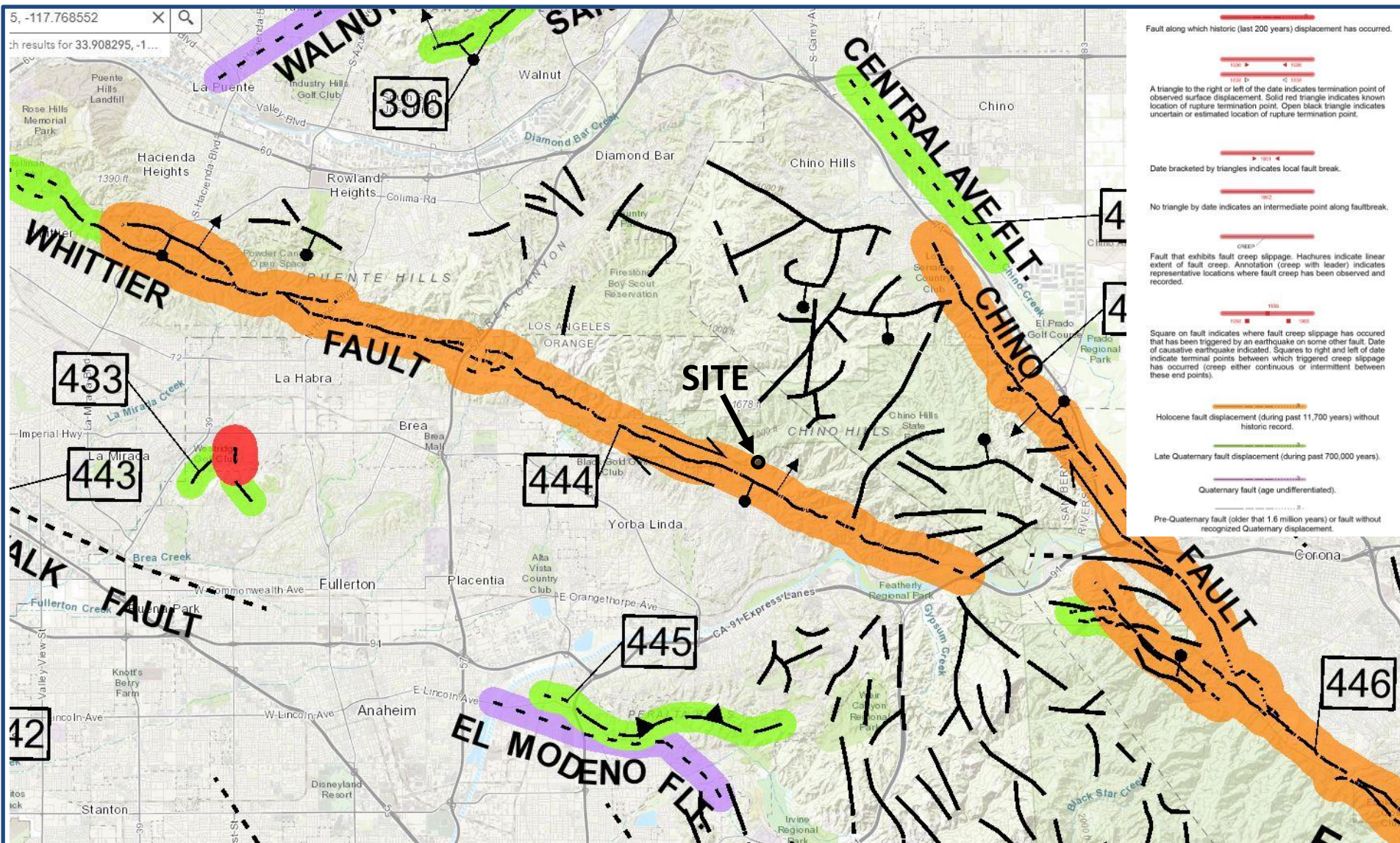


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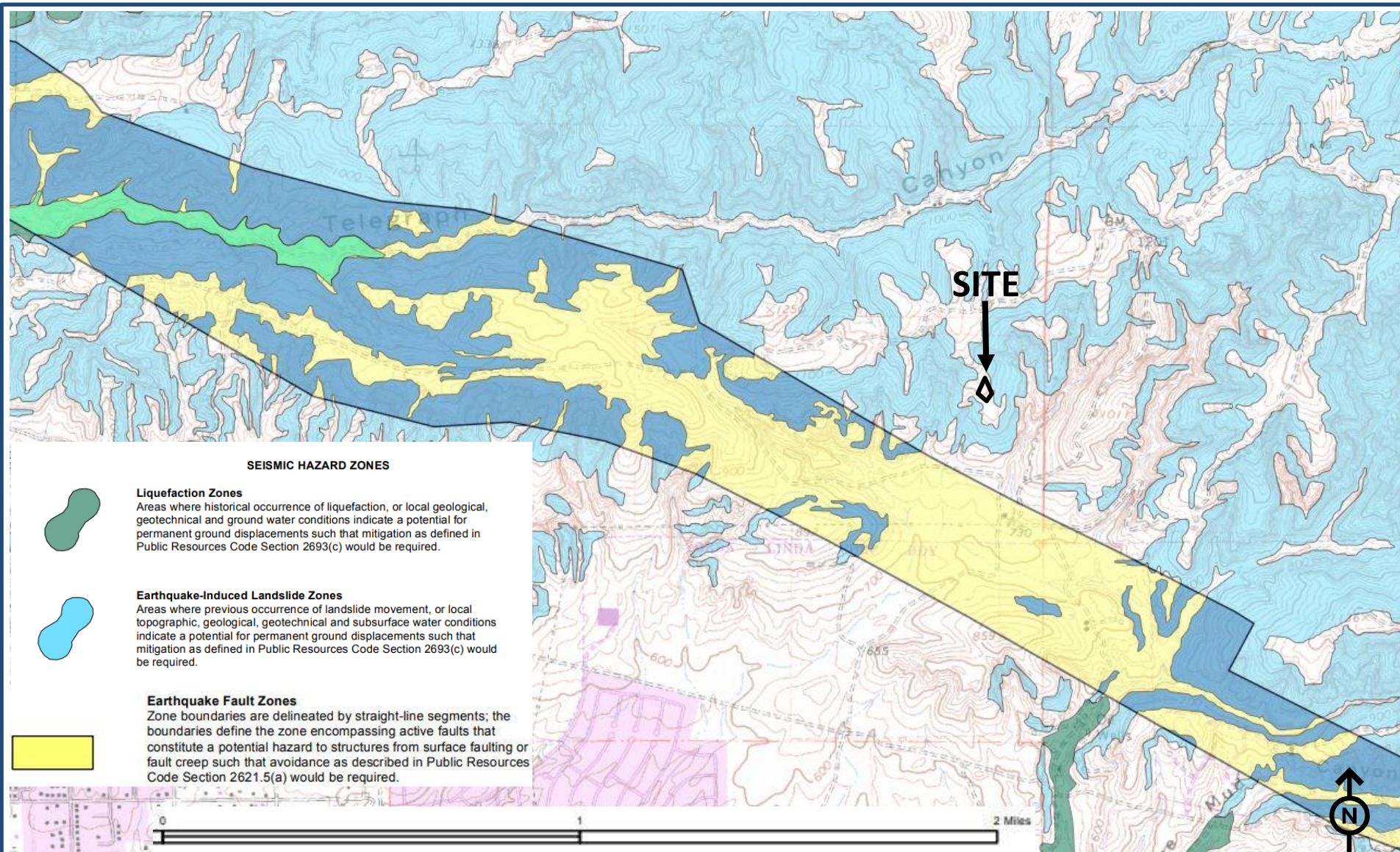
REGIONAL GEOLOGY MAP HOFF PROPERTY FAIRMONT BOULEVARD, YORBA LINDA, CALIFORNIA

PROJECT NO. 19-6969

FIGURE 3



Modified From: Jennings, C. W., 2010, Fault Activity Map of California and Adjacent Areas, California Division of Mines and Geology, Geologic Data Map Series, No. 6, Scale 1:750,000.



Modified From: State of California Division of Mines and Geology, Earthquake Zones of Required Investigation, Yorba Linda Quadrangle
Released December 4, 2015, scale 1:24,000

APPENDIX A REFERENCES

APPENDIX A

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Hart, E. W., 2007, Fault-Rupture Hazard Zones in California, Alquist-Priolo Earthquake Fault Zoning with Index to Special Study Zones Maps: Department of Conservation, Division of Mines and Geology, Special Publication 42.

International Conference of Building Officials, 2019, California Building Code, Part 1 and 2.

Fault Activity Map of California, 2010, California Geological Survey, Geologic Data Map No. 6
Compilation and Interpretation by: Charles W. Jennings and William A. Bryant, Graphics by: Milind Patel, Ellen Sander, Jim Thompson, Barbara Wanish and Milton Fonseca

**APPENDIX B
LOG OF BORINGS**

THE FOLLOWING DESCRIBES THE TERMS AND SYMBOLS USED ON THE LOG OF BORINGS TO SUMMARIZE THE RESULTS OBTAINED IN THE FIELD INVESTIGATION AND SUBSEQUENT LABORATORY TESTING.

DENSITY AND CONSISTENCY

The consistency of fine grained soils and the density of coarse grained soils are described on the basis of the Standard Penetration Test as follows:

COARSE GRAINED SOILS		Estimated Unconfined Compressive Strength (Tsf)	FINE GRAINED SOILS	
Very Loose	< 4	< 0.25	Very soft	< 2
Loose	4 – 10	0.35 – 0.50	Soft	2 – 4
Medium	10 – 30	0.50 – 1.0	Firm (medium)	4 – 8
Dense	30 – 50	1.0 – 2.0	Stiff	8 – 15
Very dense	> 50	2.0 – 4.0	Very stiff	15 – 30
		> 4.0	Hard	> 30

PARTICULE SIZE DEFINITION (As per ASTM D2487 And D422)

Boulder	⇒ Larger than 12 inches	Coarse Sands	⇒ No. 10 to No. 4 sieve
Cobbles	⇒ 3 to 12 inches	Medium Sands	⇒ No. 40 to No. 10 sieve
Coarse Gravel	⇒ 3/4 to 3 inches	Fine Sands	⇒ No. 200 to 40 sieve
Fine Gravel	⇒ No. 4 to 3/4 inches	Silt	⇒ 5µm to No. 200 sieve
		Clay	⇒ Smaller than 5µm

SOIL CLASSIFICATION

Soils and bedrock are classified and described based on their engineering properties and characteristics and using ASTM D2487 and D2488.

Percentage description of minor components

Trace	1-10 %	Some	20-35 %
Little	10-20 %	And or y	35-50 %

Stratified soils description

Parting	0 to 1/16 inch thick	Layer	½ to 12 inches thick
Seam	1/16 to ½ inch thick	Stratum	> 12 inches thick



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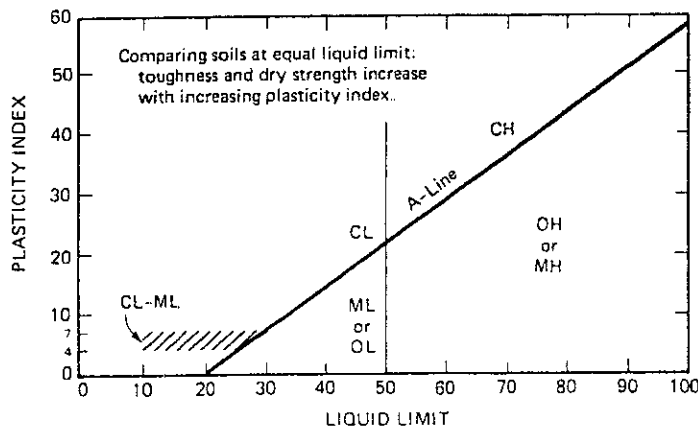
LOG OF BORING EXPLANATION

Page 1 of 2

PLATE 1

SOIL CLASSIFICATION CHART

MAJOR DIVISIONS			SYMBOLS		TYPICAL DESCRIPTIONS	CLASSIFICATION CRITERIA			
			GRAPH	LETTER					
COARSE GRAINED SOILS	GRAVEL AND GRAVELLY SOILS	CLEAN GRAVELS		GW	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES	$C_u \frac{D_{60}}{D_{10}} > 4$ $C_u \frac{(D_{30})^2}{D_{10} \times D_{60}} = 1 \text{ to } 3$			
		(LITTLE OR NO FINES)		GP	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES	Not meeting all above requirements			
		GRAVELS WITH FINES		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES	Atterberg limits below "A" line or $I_p < 4$			
		(APPRECIABLE AMOUNT OF FINES)		GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURES	Atterberg limits above "A" line or $I_p > 7$			
	SAND AND SANDY SOILS	CLEAN SANDS		SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	$C_u \frac{D_{60}}{D_{10}} > 6$ $C_u \frac{(D_{30})^2}{D_{10} \times D_{60}} = 1 \text{ to } 3$			
		(LITTLE OR NO FINES)		SP	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES	Not meeting all above requirements			
		SANDS WITH FINES		SM	SILTY SANDS, SAND - SILT MIXTURES	Atterberg limits below "A" line or $I_p < 4$			
		(APPRECIABLE AMOUNT OF FINES)		SC	CLAYEY SANDS, SAND - CLAY MIXTURES	Atterberg limits above "A" line or $I_p > 7$			
FINE GRAINED SOILS	SILTS AND CLAYS	LIQUID LIMIT LESS THAN 50		ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY	$W_L < 50$			
				CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAM CLAYS	$W_L < 30$			
				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY	$W_L < 50$			
	SILTS AND CLAYS	LIQUID LIMIT GREATER THAN 50		MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS	$W_L > 50$			
				CH	INORGANIC CLAYS OF HIGH PLASTICITY	$W_L > 50$			
				OH	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS	$W_L > 50$			
			HIGHLY ORGANIC SOILS				PT	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS	Strong color or odor and often fibrous texture



LABORATORY TEST DESIGNATIONS

BR	Bearing Ratio	PP	Pocket Penetrometer
C	Consolidation	R	R-Value
COR	Corrosion Test	S	Direct Shear
Ch	Water Soluble Chlorides	SA	Sieve Analysis
EI	Expansive Index	SE	Sand Equivalent
ER	Electrical Resistivity	Sg	Specific Gravity
K	Permeability	SO	Water Soluble Sulfates
MC	Moisture Content	SV	Shear Vane
OC	Organic Content	TC	Triaxial Compression
PH	PH Test	UC	Unconfined Compression

ADDITIONAL SOIL CLASSIFICATION

Fill	Ss Sandstone	Ms Siltstone	Bdr Bedrock

PARTICLE SIZE LIMITS

SILT OR CLAY	SAND			GRAVEL		COBBLES	BOULDERS
	FINE	MEDIUM	COARSE	FINE	COARSE		
	NO. 200	NO. 40	NO. 10	NO. 4	3/4 in.	3 in.	(12 in.)
	U. S. STANDARD SIEVE SIZE						



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LOG OF BORING EXPLANATION

Page 2 of 2

PLATE 1

LOG OF EXPLORATORY BORING BA-1

Sheet 1 of 3

Project Number: **20-7009**
 Project Name: **Hoff Property, Yorba Linda**
 Date Drilled: **4/20/20 - 4/20/20**
 Ground Elev:

Logged By: **ELB**
 Project Engineer: **SG**
 Drill Type: **Bucket Auger**
 Drive Wt & Drop: **140lbs / 30in**

Depth (ft)	Graphic Log	FIELD RESULTS					LAB RESULTS		
		Bulk Sample	Drive Sample	SPT blows/ft (or equivalent N)	Pocket Pen (tsf)	USCS	Moisture Content (%)	Dry Density, (pcf)	Other Tests
Shelby Tube Standard Split Spoon No recovery Modified California Water Table ATD							SUMMARY OF SUBSURFACE CONDITIONS		
Fill/ Slopewash/ Colluvium: Sandy Silt- brown, moist, loose									
5									
10				2			20	109	
15									
20				2			20	105	
25									
30				4			21	107	Shear
BEDROCK: Puente Formation(Tpl)- Silty Sandstone- light brown-tan, slightly moist, medium dense, upper 5 feet highly weathered, bedrock contact N70W, 15SW, moderately fractured, orange oxidation									
@13': Sandstone and siltstone beds, bedding attitude- N10W, 30NE									
@15: increase from medium dense to dense									
@19': bedding attitude- N10W, 25NE									
@20': Silty Sandstone- light brown to tan, slightly moist, dense, moderately fractured, orange oxidation									
@30': bedding attitude- N10E, 20SE									
@ 31-32'- well cemented sandstone layer									
@34': Sandstone and siltstone beds- moderately fractured, dense, moderately oxidized, bedding attitude- N10W, 25NE									

This Boring Log should be evaluated in conjunction with the complete geotechnical report. This Boring Log represents conditions observed at the specific location and date indicated, it is not warranted to be representative of subsurface conditions at other locations and times.

PLATE 2

TGR GEOTECHNICAL, INC.

This Boring Log should be evaluated in conjunction with the complete geotechnical report. This Boring Log represents conditions observed at the specific location and date indicated, it is not warranted to be representative of subsurface conditions at other locations and times.

PLATE 2



TGR GEOTECHNICAL, INC.

LOG OF BORING 20-7009 HOFF PROPERTY, YORBA LINDA.GPJ TGR GEOTECH.GDT 5/8/20

LOG OF EXPLORATORY BORING BA-1

Sheet 2 of 3

Project Number: **20-7009**
 Project Name: **Hoff Property, Yorba Linda**
 Date Drilled: **4/20/20 - 4/20/20**
 Ground Elev:

Logged By: **ELB**
 Project Engineer: **SG**
 Drill Type: **Bucket Auger**
 Drive Wt & Drop: **140lbs / 30in**

Depth (ft)	Graphic Log	FIELD RESULTS					LAB RESULTS		
		Bulk Sample	Drive Sample	SPT blows/ft (or equivalent N)	Pocket Pen (tsf)	USCS	Moisture Content (%)	Dry Density, (pcf)	Other Tests
SUMMARY OF SUBSURFACE CONDITIONS									
						@38-41': Sandstone- well cemented			
40				9		@41': bedding attitude- N15W, 25NE	10	125	
45									El, Max, Remolded Shear
						@48-51': Sandstone layer- well cemented, gypsum along fractures, bedding attitude @ 48'- N15E, 20SE			
50				12			18	117	Shear
						@54': bedding attitude- N10E, 25SE			
55						@56': Siltstone- gray to dark gray, slightly moist, hard, minor fracturing, minor oxidation, gypsum along fracture, bedding			
						@61': bedding attitude- N10W, 30NE			
60				22			14	115	Shear
65						---increase in hardness			

LOG OF BORING: 20-7009 HOFF PROPERTY, YORBA LINDA.GPJ TGR GEOTECH.GDT 5/8/20

This Boring Log should be evaluated in conjunction with the complete geotechnical report. This Boring Log represents conditions observed at the specific location and date indicated, it is not warranted to be representative of subsurface conditions at other locations and times.

PLATE 3



TGR GEOTECHNICAL, INC.

LOG OF EXPLORATORY BORING BA-1

Sheet 3 of 3

Project Number: **20-7009**
 Project Name: **Hoff Property, Yorba Linda**
 Date Drilled: **4/20/20 - 4/20/20**
 Ground Elev:

Logged By: **ELB**
 Project Engineer: **SG**
 Drill Type: **Bucket Auger**
 Drive Wt & Drop: **140lbs / 30in**

Depth (ft)	Graphic Log	FIELD RESULTS					LAB RESULTS						
		Bulk Sample	Drive Sample	SPT blows/ft (or equivalent N)	Pocket Pen (tsf)	USCS	Shelby Tube Modified California	Standard Split Spoon Water Table ATD	No recovery	Moisture Content (%)	Dry Density, (pcf)	Other Tests	
SUMMARY OF SUBSURFACE CONDITIONS													
75			23			@71': bedding attitude- N15W, 20NE Total Depth: 75' No groundwater No caving Hole backfilled with cuttings Kelly Weight: 0'-25': 3300lbs, 25'-50': 2200lbs, 50'-75': 1100lbs					11	131	Shear

LOG OF BORING: 20-7009 HOFF PROPERTY, YORBA LINDA.GPJ TGR GEOTECH.GDT 5/8/20

This Boring Log should be evaluated in conjunction with the complete geotechnical report. This Boring Log represents conditions observed at the specific location and date indicated, it is not warranted to be representative of subsurface conditions at other locations and times.

PLATE 4



TGR GEOTECHNICAL, INC.

LOG OF EXPLORATORY BORING BA-2

Sheet 1 of 1

Project Number: **20-7009**
 Project Name: **Hoff Property, Yorba Linda**
 Date Drilled: **4/21/20 - 4/21/20**
 Ground Elev: _____

Logged By: **ELB**
 Project Engineer: **SG**
 Drill Type: **Bucket Auger**
 Drive Wt & Drop: **140lbs / 30in**

Depth (ft)	Graphic Log	FIELD RESULTS					LAB RESULTS		
		Bulk Sample	Drive Sample	SPT blows/ft (or equivalent N)	Pocket Pen (tsf)	USCS	Shelby Tube	Standard Split Spoon	No recovery
					Modified California	Water Table ATD	Moisture Content (%)	Dry Density, (pcf)	Other Tests
SUMMARY OF SUBSURFACE CONDITIONS									
Topsoil: Sandy Silt- light brown, moist, loose									
BEDROCK: Puente Formation(Tpl)- Silty Sandstone- light brown-tan, slightly moist, dense, moderately fractured, orange oxidation									
5						@6': bedding attitude- N50W, 40NE			
10				4		@10': Sandstone and siltstone beds, light brown to dark gray, slightly moist, dense, moderately fractured, orange oxidation, bedding attitude- N60W, 55NE	12	115	Max, Remolded Shear
15									
20				5		Same as above	13	125	
25						@25': bedding attitude- N45W, 35NE			
30				10			14	117	
Total Depth: 31' No Groundwater. No caving. Hole backfilled with cuttings Kelly Weight: 0'-25': 3300lbs, 25'-50': 2200lbs, 50'-75': 1100lbs									

LOG OF BORING: 20-7009 HOFF PROPERTY, YORBA LINDA.GPJ TGR GEOTECH.GDT 5/8/20

This Boring Log should be evaluated in conjunction with the complete geotechnical report. This Boring Log represents conditions observed at the specific location and date indicated, it is not warranted to be representative of subsurface conditions at other locations and times.

PLATE 5



TGR GEOTECHNICAL, INC.

LOG OF EXPLORATORY BORING BA-3

Sheet 1 of 1

Project Number: **20-7009**
 Project Name: **Hoff Property, Yorba Linda**
 Date Drilled: **4/21/20 - 4/21/20**
 Ground Elev:

Logged By: **ELB**
 Project Engineer: **SG**
 Drill Type: **Bucket Auger**
 Drive Wt & Drop: **140lbs / 30in**

Depth (ft)	Graphic Log	FIELD RESULTS					LAB RESULTS					
		Bulk Sample	Drive Sample	SPT blows/ft (or equivalent N)	Pocket Pen (tsf)	USCS	 Shelby Tube	 Standard Split Spoon	 No recovery			
										 Modified California	 Water Table ATD	
SUMMARY OF SUBSURFACE CONDITIONS					Moisture Content (%)	Dry Density, (pcf)	Other Tests					
							Topsoil: Sandy Silt- light brown, moist, loose					
5							BEDROCK: Puente Formation(Tpl)- Silty Sandstone- light brown, slightly moist, dense, moderately fractured, orange oxidation					
10					3		Sandstone- siltstone beds, light brown/tan- light gray/ gray, slightly moist, moderately fractured, orange oxidation, gypsum along bedding, @10': bedding attitude- N70E, 70SE			17	111	
15							@15': bedding attitude- N75E, 60SE					
20					8/6"		@20': Sandstone, light brown- tan, slightly moist, dense, moderately fractured, orange oxidation, bedding attitude- N75E, 65SE			8	106	
25							@27': Sandstone- siltstone beds, light brown/tan-gray, slightly moist, moderately fractured, orange oxidation, bedding attitude- N75E, 65SE					
30					9					18	116	
							Total Depth: 31 feet No Ground water. No caving. Hole backfilled with cuttings Kelly Weight: 0'-25': 3300lbs, 25'-50': 2200lbs, 50'-75': 1100lbs					

LOG OF BORING: 20-7009 HOFF PROPERTY, YORBA LINDA.GPJ TGR GEOTECH.GDT 5/8/20

This Boring Log should be evaluated in conjunction with the complete geotechnical report. This Boring Log represents conditions observed at the specific location and date indicated, it is not warranted to be representative of subsurface conditions at other locations and times.

PLATE 6



TGR GEOTECHNICAL, INC.

APPENDIX C LABORATORY TEST RESULTS

APPENDIX C

Laboratory Testing Procedures and Results

Moisture and Density Determination Tests: Moisture content and dry density determinations were performed on relatively undisturbed samples obtained from the test borings. The results of these tests are presented in the boring logs. Where applicable, only moisture content was determined from "undisturbed" or disturbed samples.

Maximum Density Tests: The maximum dry density and optimum moisture content of typical materials were determined in accordance with ASTM Test Method D1557. The results of these tests are presented in the test data and in the table below:

Sample Location	Sample Description	Maximum Dry Density (Pcf)	Optimum Moisture Content (%)
BA-1 @ 0-5 feet	Sandy Silt	116.0	15.0
BA-1@ 45 feet	Sandstone	111.0	17.5
BA-2 @ 10 feet	Sandstone and siltstone	108.0	20.0

Direct Shear Tests: Direct shear test was performed on selected remolded and/or undisturbed sample, which was soaked for a minimum of 24 hours under a surcharge equal to the applied normal force during testing. After transfer of the sample to the shear box, and reloading the sample, pore pressures set up in the sample due to the transfer were allowed to dissipate for a period of approximately 1-hour prior to application of shearing force. The sample was tested under various normal loads, a motor-driven, strain-controlled, direct-shear testing apparatus at a strain rate of less than 0.001 to 0.5 inches per minute (depending upon the soil type). The test results are presented in the test data:

Soluble Sulfates: The soluble sulfate content of selected sample was determined by standard geochemical methods. The test result is presented in the table below:

Sample Location	Sample Description	Water Soluble Sulfate in Soil, (% by Weight)	Sulfate Content (ppm)	Exposure Class*
BA-1 @ 0-5 feet	Sandy Silt	0.0765	765	S0

* Based on the current version of ACI 318-14 Building Code, Table No. 19.3.1.1; Exposure Categories and Classes.

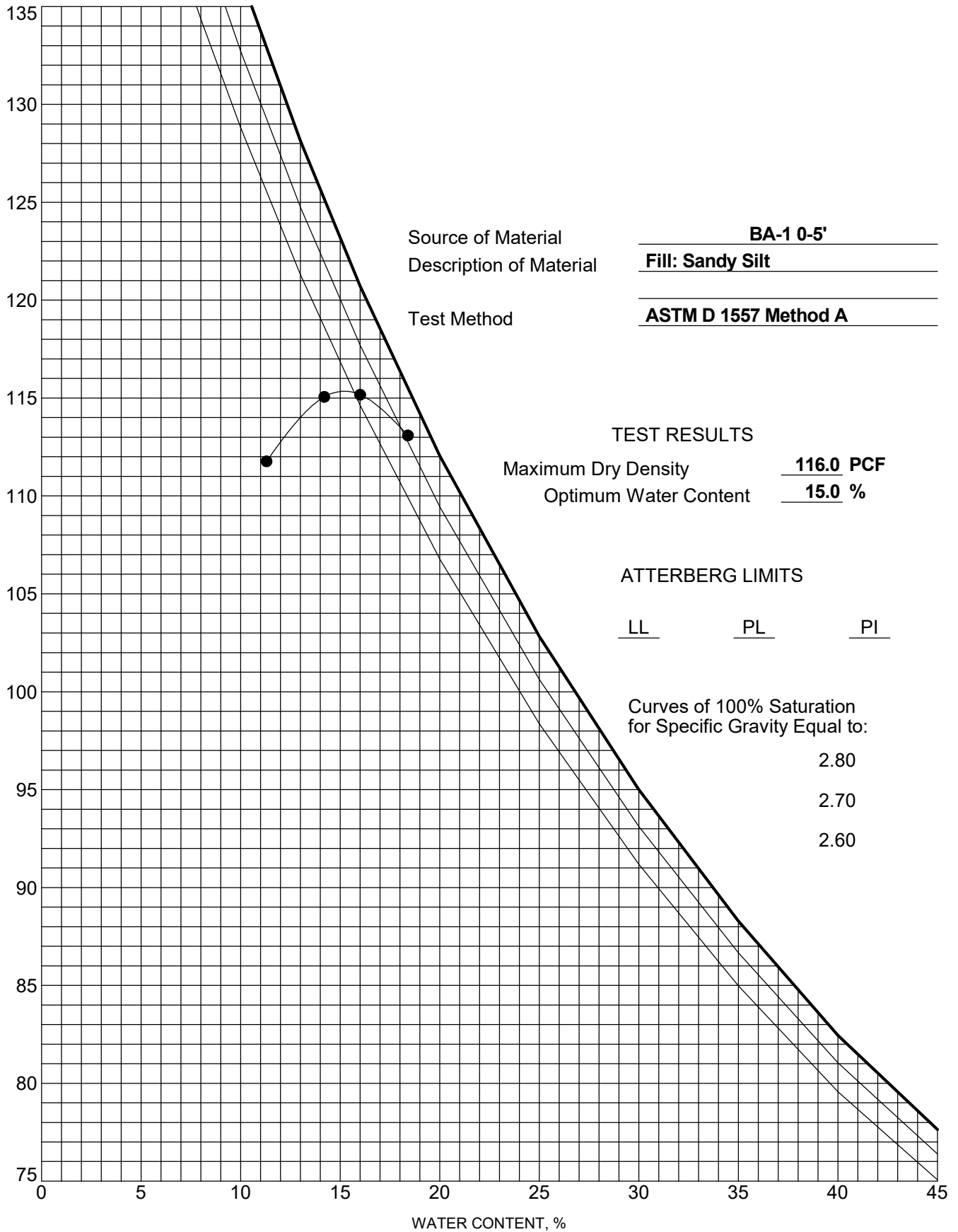
Expansion Index Tests: The expansion potential of selected materials was evaluated by the Expansion Index Test, ASTM D4829. Specimens are molded under a given compactive energy to approximately the optimum moisture content and approximately 50 percent saturation or approximately 90 percent relative compaction. The prepared 1-inch thick by 4-inch diameter

20-7009

specimens are loaded to an equivalent 144 psf surcharge and are inundated with tap water until volumetric equilibrium is reached. The results of these tests are presented in the table below:

Sample Location	Sample Description	Expansion Index	Expansion Potential
BA-1 @ 0-5 feet	Sandy Silt	58	Medium
BA-1 @ 45 feet	Siltstone	41	Low

DRY DENSITY, pcf



TGR GEOTECHNICAL, INC. Fax:

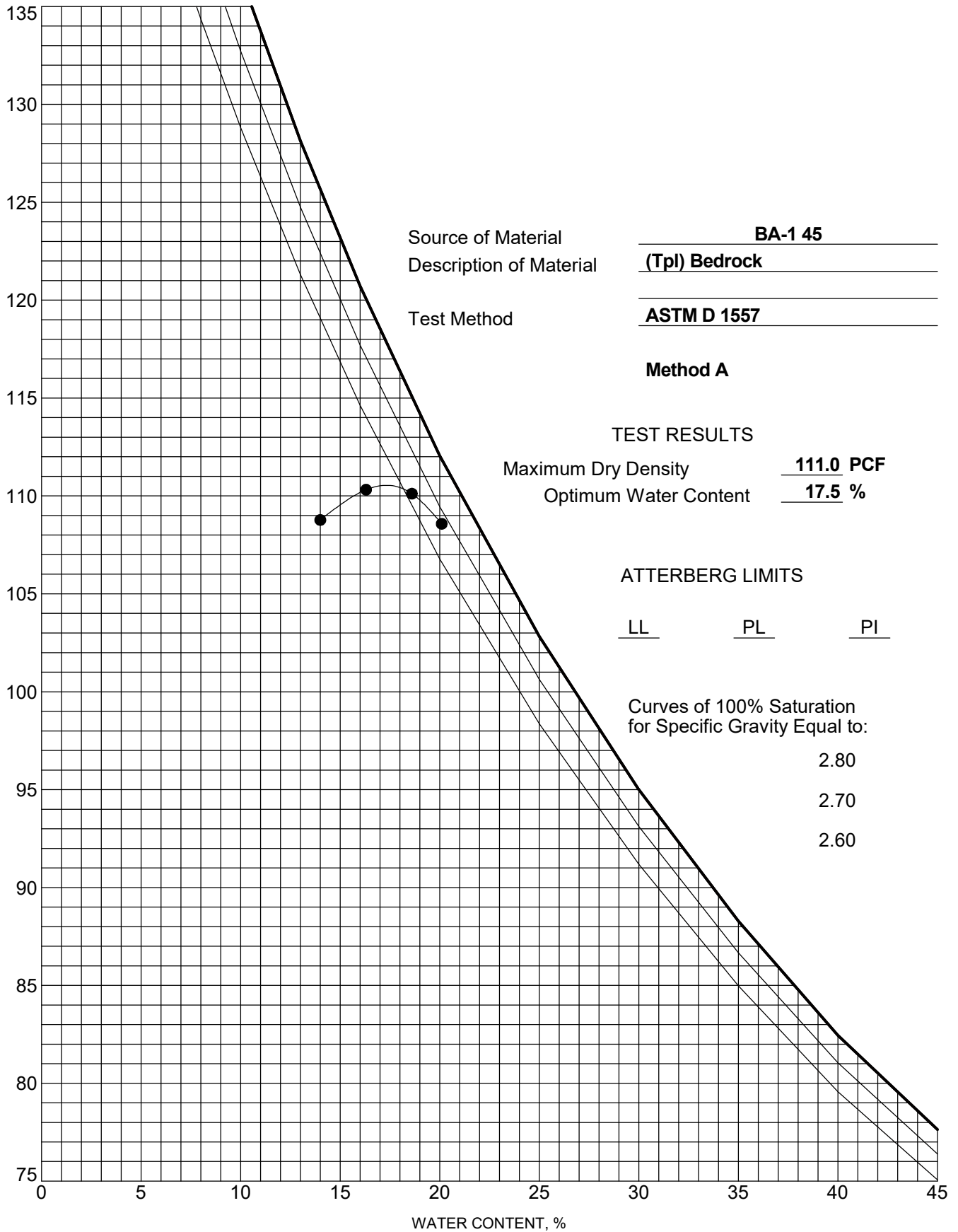
3037 S. Harbor Blvd
Santa Ana, CA 92704
Telephone: 714-641-7189

MOISTURE-DENSITY RELATIONSHIP

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

DRY DENSITY, pcf



WATER CONTENT, %



TGR GEOTECHNICAL, INC. Fax:

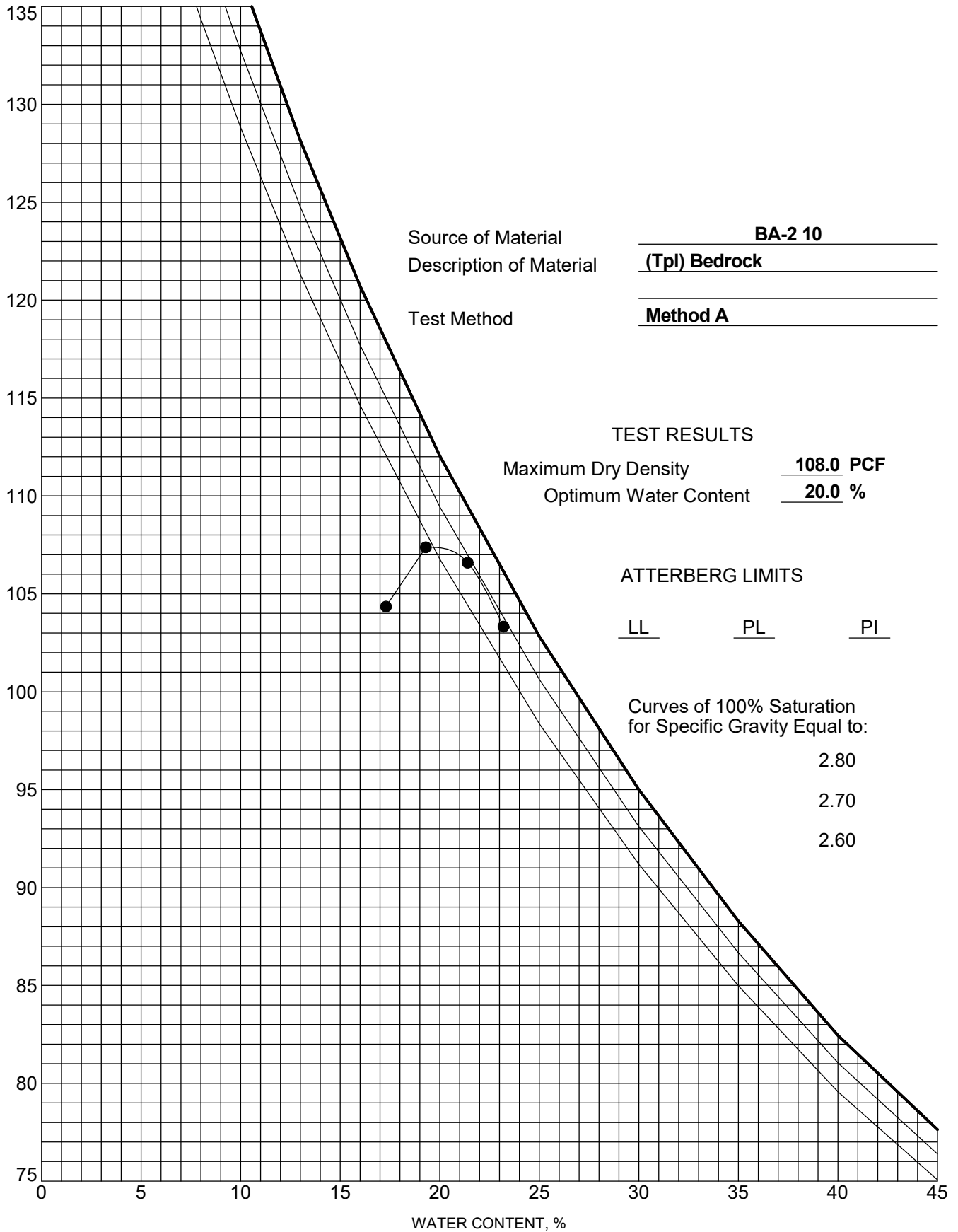
3037 S. Harbor Blvd
Santa Ana, CA 92704
Telephone: 714-641-7189

MOISTURE-DENSITY RELATIONSHIP

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

DRY DENSITY, pcf



TGR GEOTECHNICAL, INC. Fax:

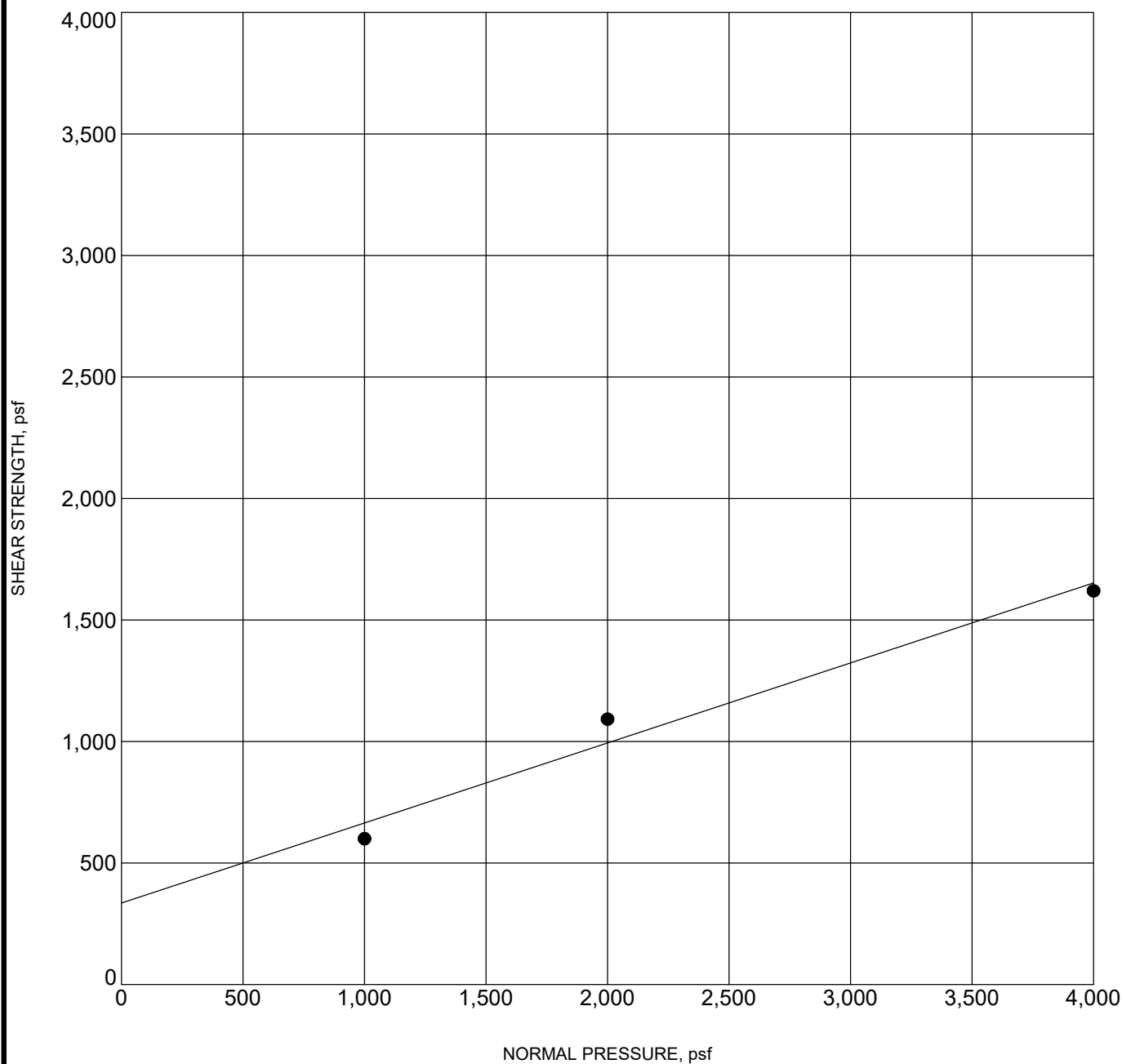
3037 S. Harbor Blvd
Santa Ana, CA 92704
Telephone: 714-641-7189

MOISTURE-DENSITY RELATIONSHIP

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

US DIRECT SHEAR 20-7009 HOFF PROPERTY, YORBA LINDA.GPJ TGR GEOTECH.GDT 5/7/20



Specimen Identification		Classification	γ_d	MC%	c	ϕ
●	BA-1 30	(Tpl) Bedrock, Resheared, Saturated	107	21	336	18



TGR GEOTECHNICAL, INC.

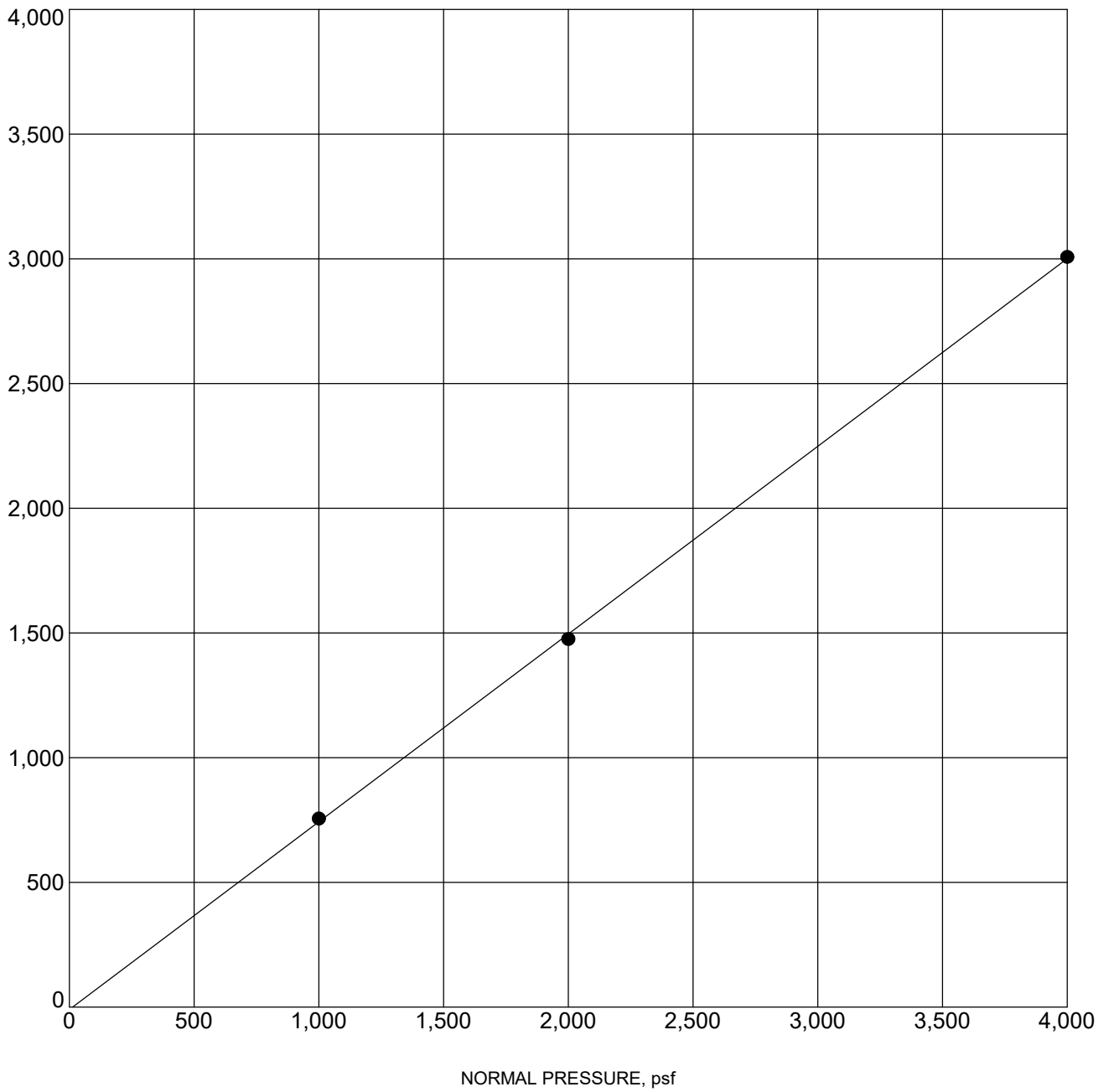
3037 S. Harbor Blvd
 Santa Ana, CA 92704
 Telephone: 714-641-7189
 Fax:

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

SHEAR STRENGTH, psf



NORMAL PRESSURE, psf

Specimen Identification		Classification	γ_d	MC%	c	ϕ
●	BA-1 60	(Tpl) Bedrock, Resheared, Saturated	115	14	0	37



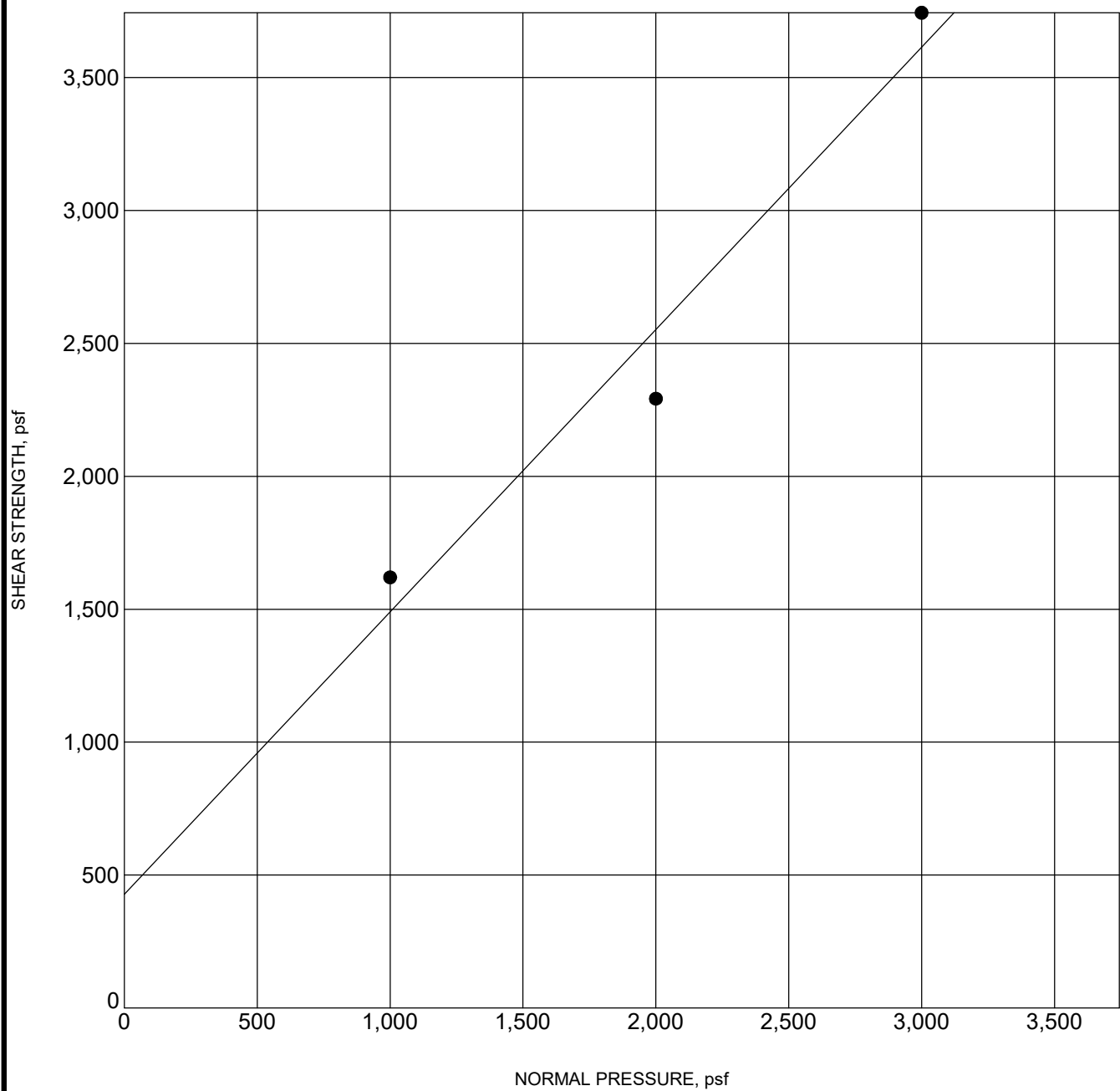
TGR GEOTECHNICAL, INC. Fax:

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 Santa Ana, CA 92704
 Telephone: 714-641-7189

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda



Specimen Identification	Classification	γ_d	MC%	c	ϕ
● BA-1 60	(Tpl) Bedrock, Peak, Saturated	115	14	428	47



TGR GEOTECHNICAL, INC.

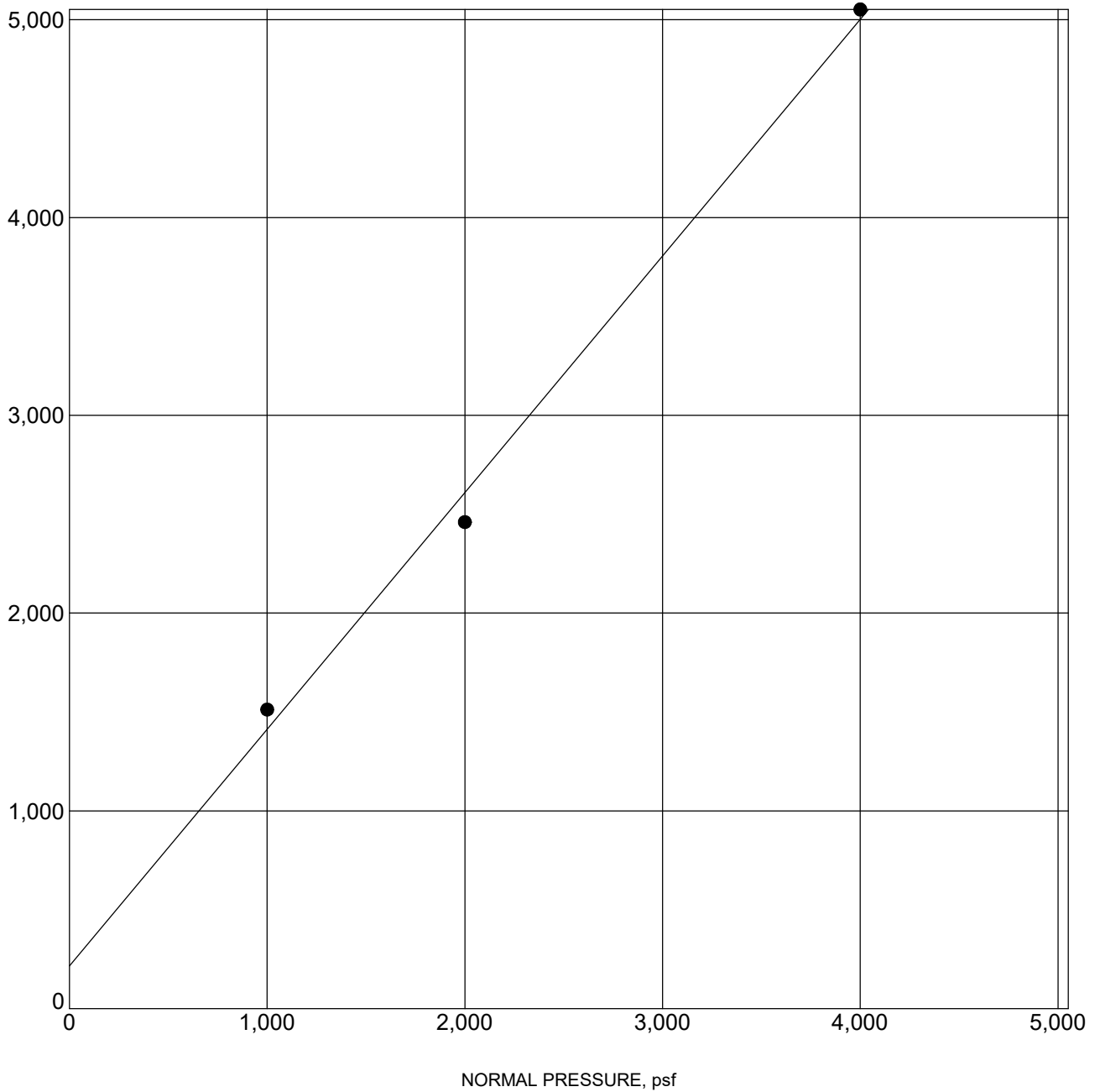
3037 S. Harbor Blvd
 Santa Ana, CA 92704
 Telephone: 714-641-7189
 Fax:

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

SHEAR STRENGTH, psf



NORMAL PRESSURE, psf

Specimen Identification		Classification	γ_d	MC%	c	ϕ
●	BA-1 30	(Tpl) Bedrock, Peak, Saturated	107	21	216	50



TGR GEOTECHNICAL, INC. Fax:

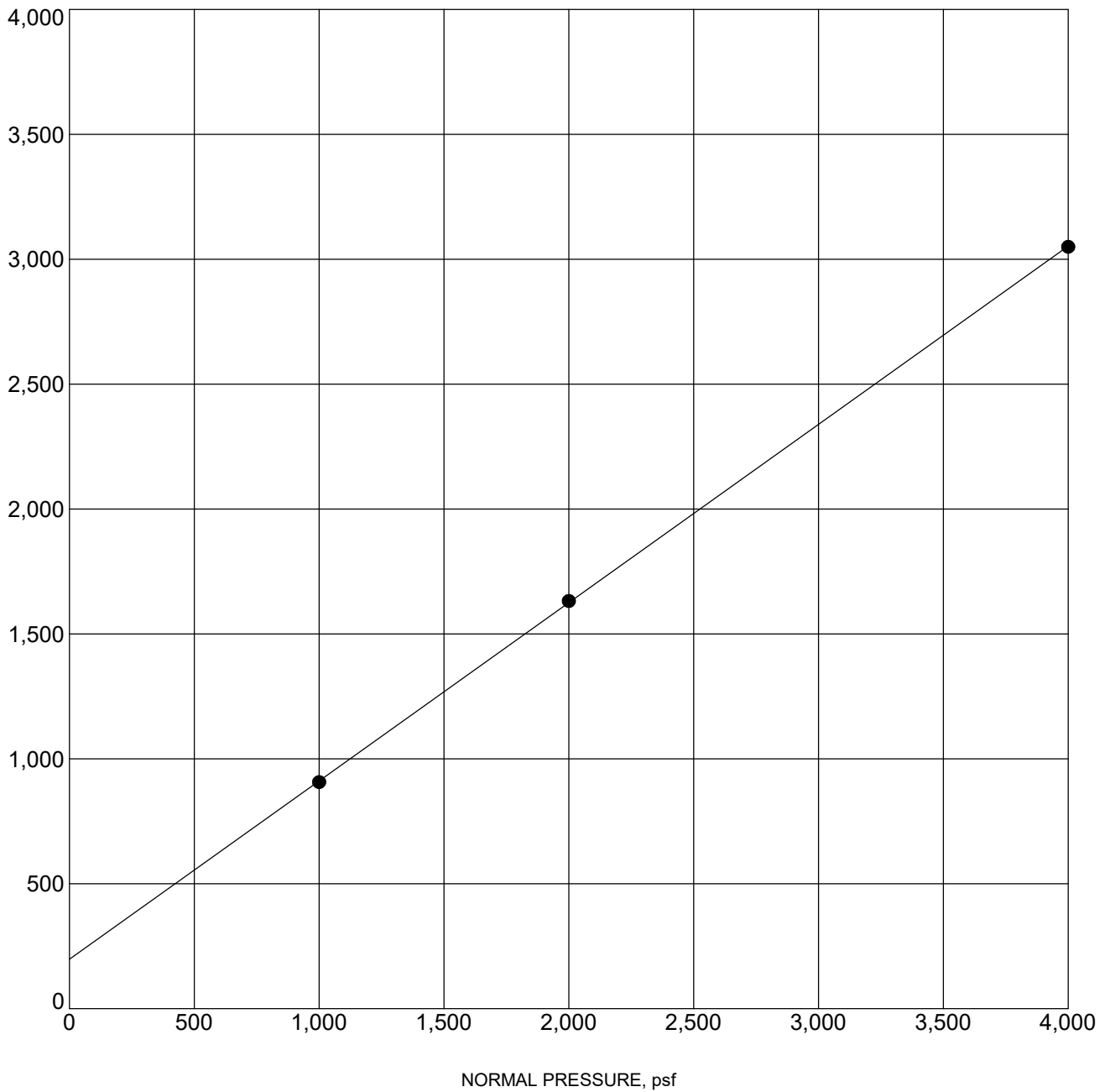
3037 S. Harbor Blvd
 Santa Ana, CA 92704
 Telephone: 714-641-7189

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

SHEAR STRENGTH, psf



NORMAL PRESSURE, psf

Specimen Identification		Classification	γ_d	MC%	c	ϕ
●	BA-1 60	(Tpl) Bedrock, Ultimate, Saturated	115	14	198	36



TGR GEOTECHNICAL, INC. Fax:

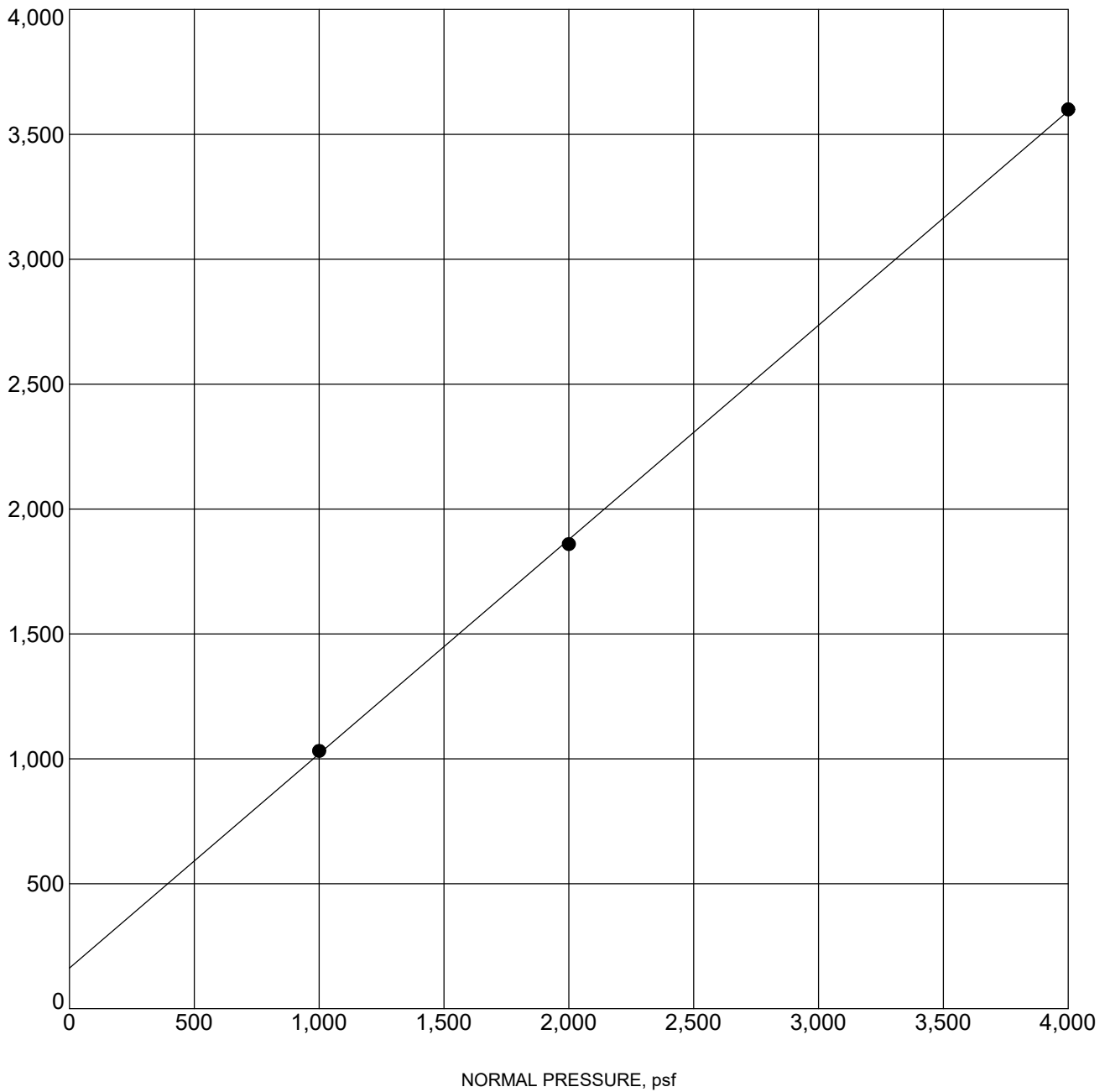
3037 S. Harbor Blvd
 Santa Ana, CA 92704
 Telephone: 714-641-7189

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

SHEAR STRENGTH, psf



Specimen Identification		Classification	γ_d	MC%	c	ϕ
●	BA-1 30	(Tpl) Bedrock, Ultimate, Saturated	107	21	162	41



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 Fax:

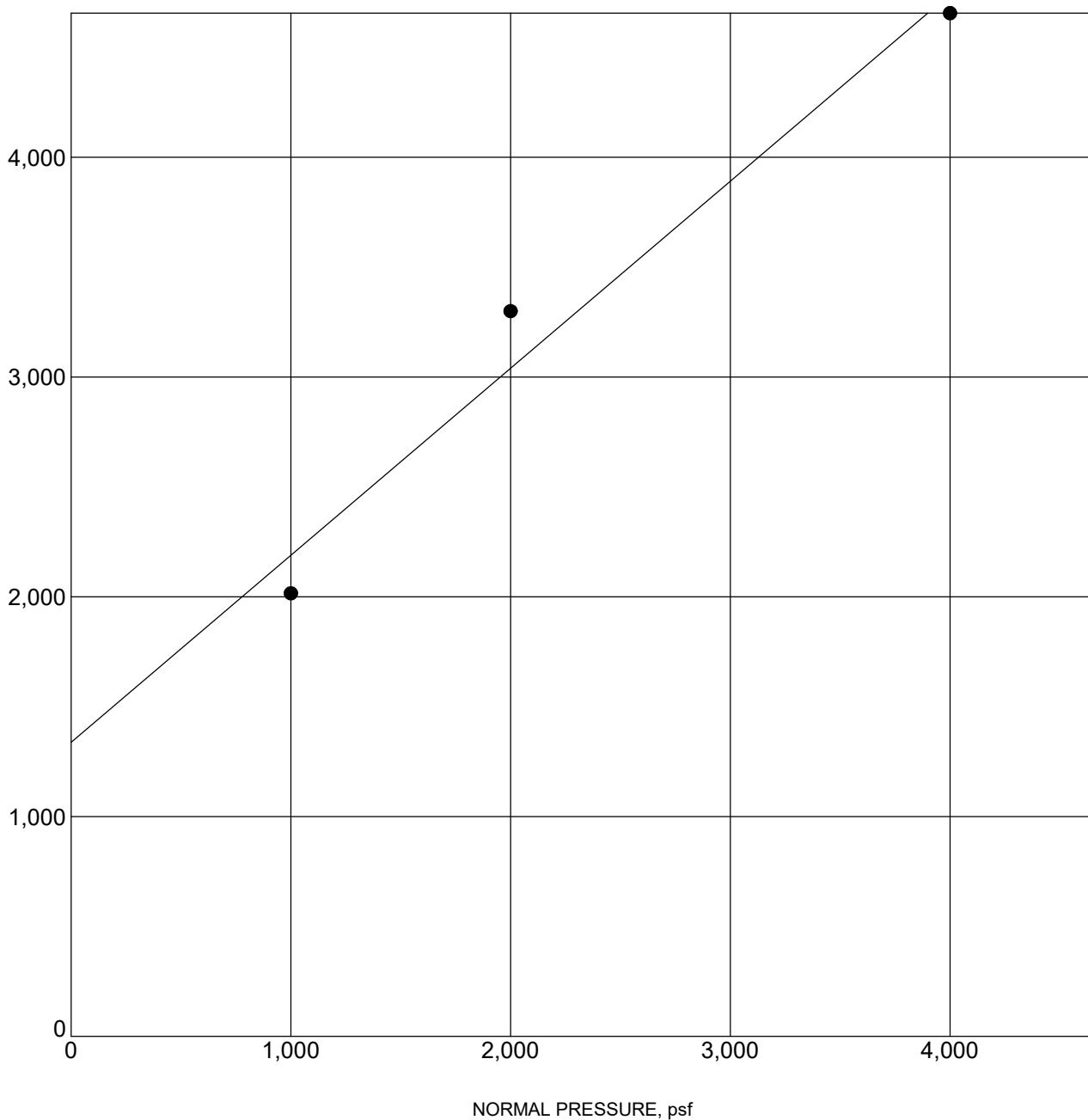
DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

US DIRECT SHEAR 20-7009 HOFF PROPERTY, YORBA LINDA.GPJ TGR GEOTECH.GDT 5/7/20

SHEAR STRENGTH, psf



Specimen Identification	Classification	γ_d	MC%	c	ϕ
● BA-1 50	(Tpl) Bedrock, Peak, Natural	117	10	1338	40



TGR GEOTECHNICAL, INC.

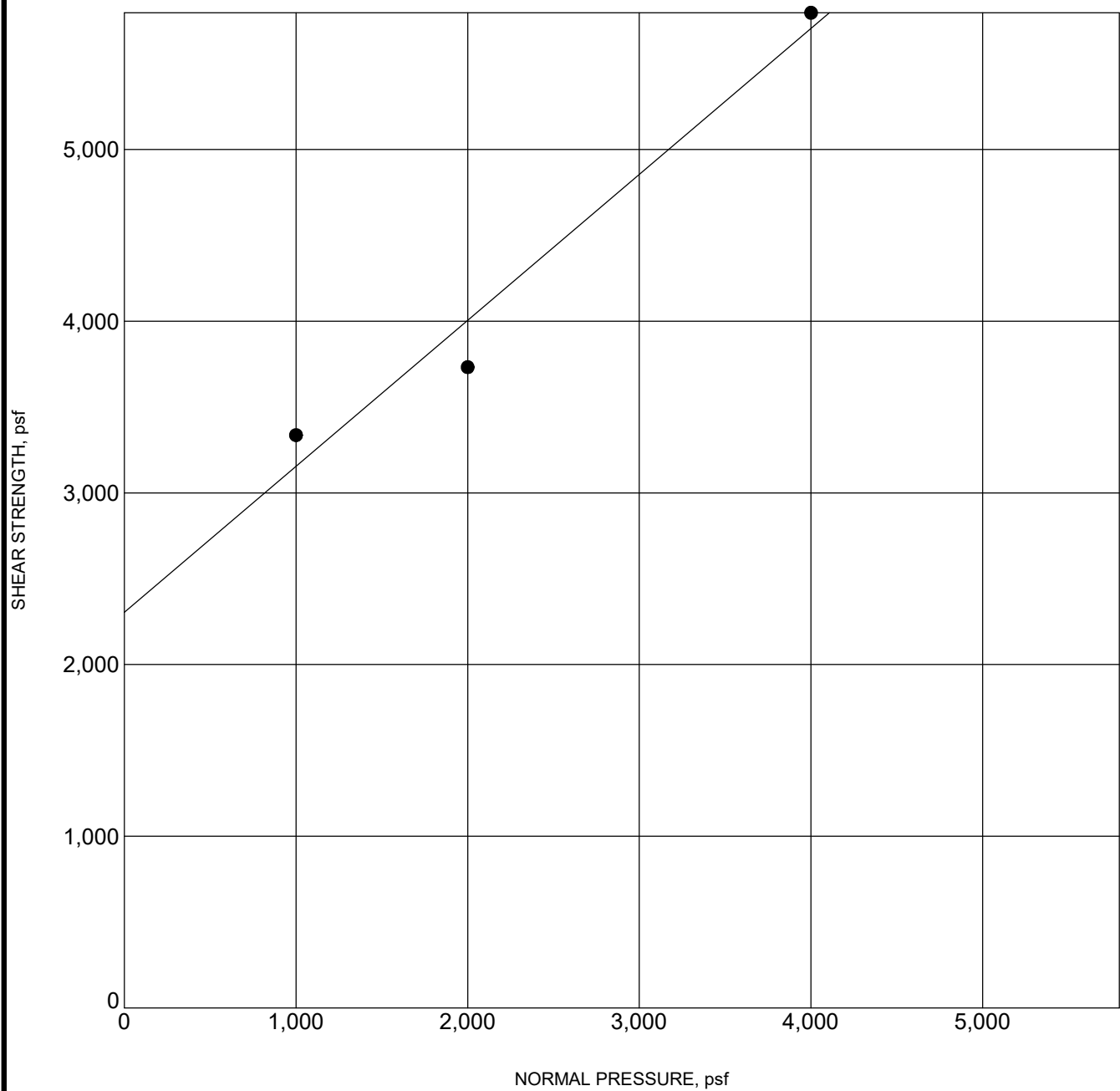
3037 S. Harbor Blvd
 Santa Ana, CA 92704
 Telephone: 714-641-7189
 Fax:

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

US DIRECT SHEAR 20-7009 HOFF PROPERTY, YORBA LINDA.GPJ TGR GEOTECH.GDT 5/7/20



Specimen Identification		Classification	γ_d	MC%	c	ϕ
●	BA-1 70	(Tpl) Bedrock, Peak, Natural	131	11	2304	40



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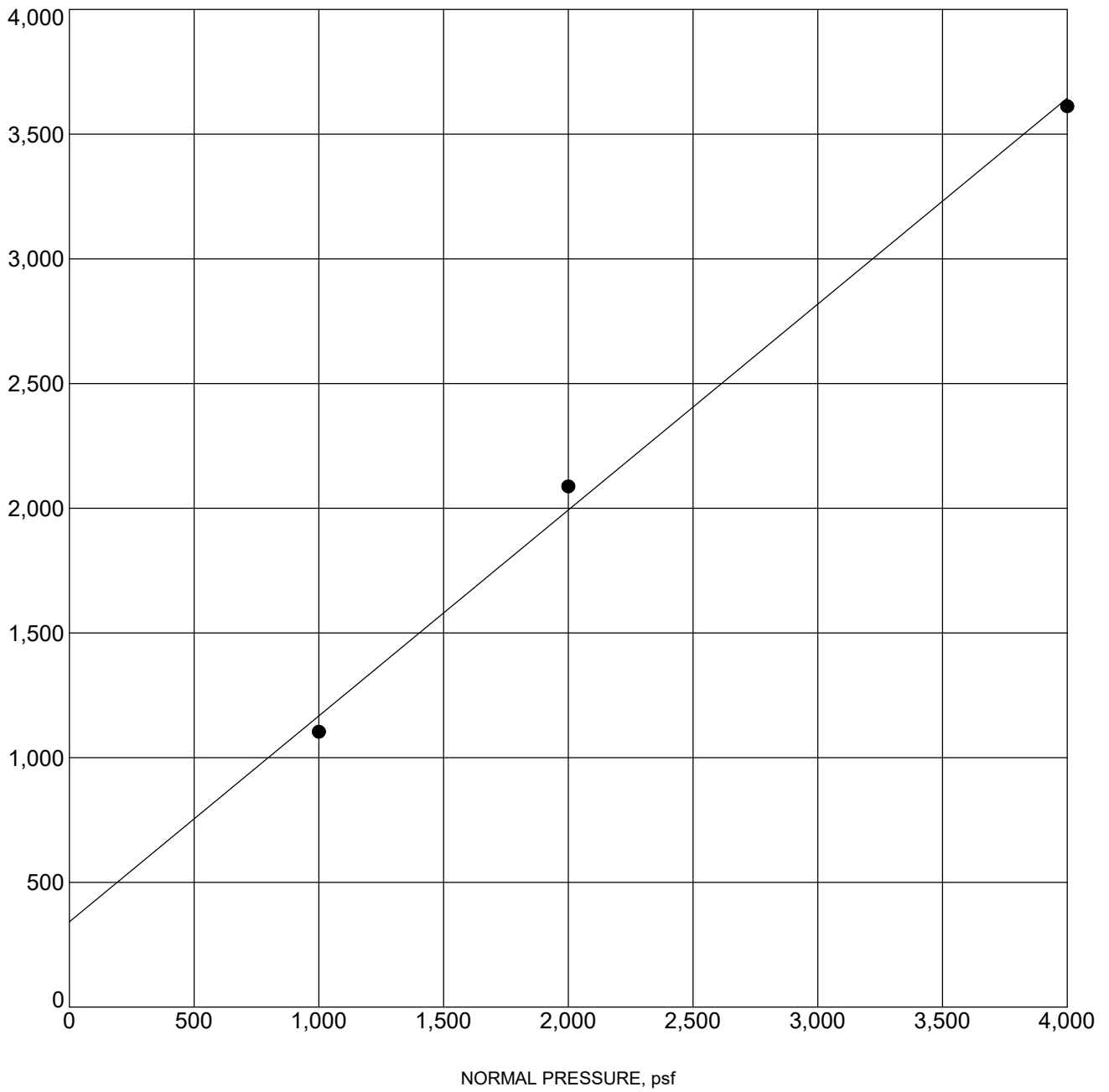
3037 S. Harbor Blvd
 Santa Ana, CA 92704
 Telephone: 714-641-7189
 Fax:

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

SHEAR STRENGTH, psf



NORMAL PRESSURE, psf

Specimen Identification		Classification	γ_d	MC%	c	ϕ
●	BA-1 50	(Tpl) Bedrock, Ultimate, Natural	117	10	342	40



TGR GEOTECHNICAL, INC. Fax:

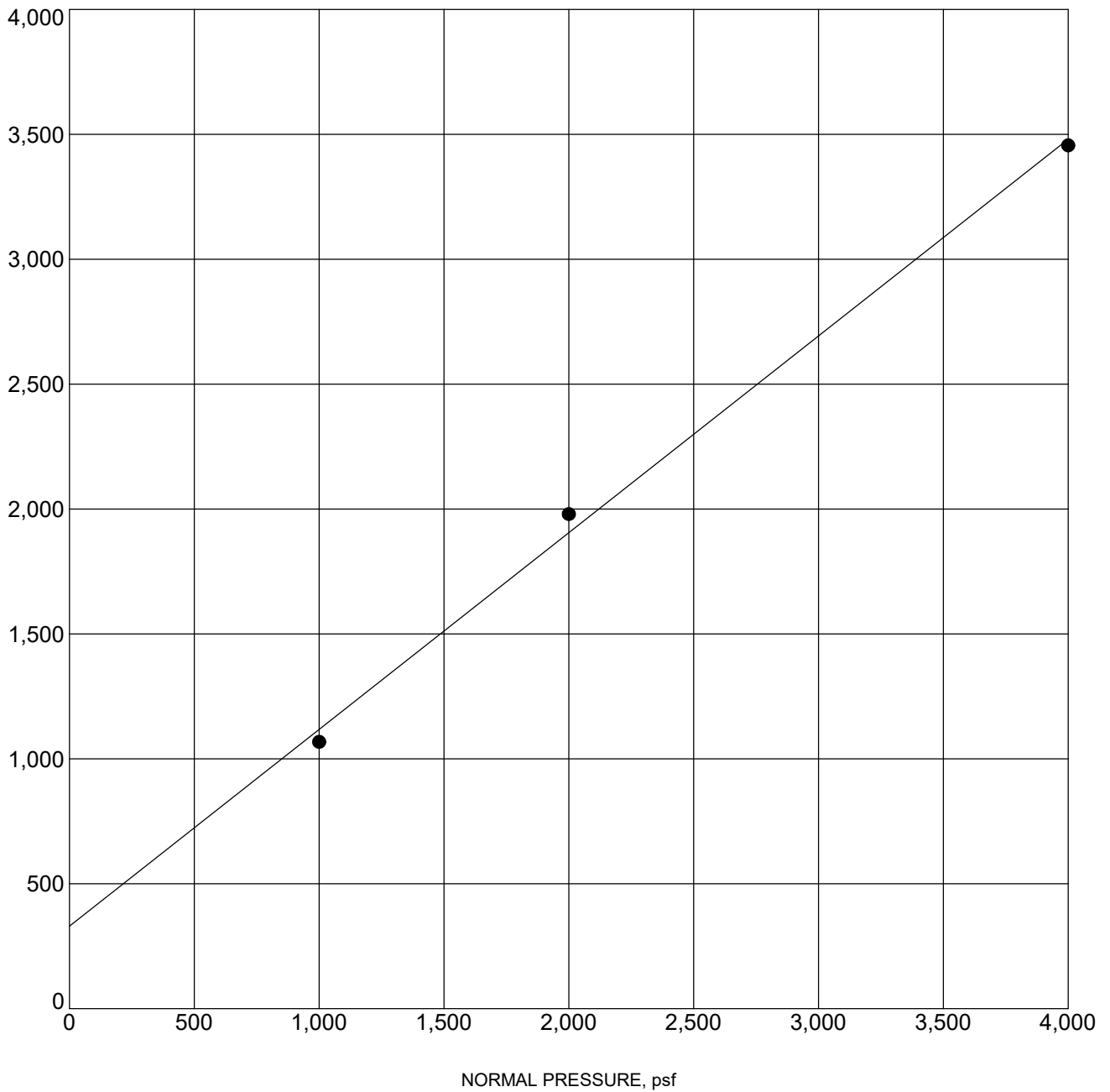
3037 S. Harbor Blvd
 Santa Ana, CA 92704
 Telephone: 714-641-7189

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

SHEAR STRENGTH, psf



NORMAL PRESSURE, psf

Specimen Identification		Classification	γ_d	MC%	c	ϕ
●	BA-1 70	(Tpl) Bedrock, Ultimate, Natural	131	11	330	38



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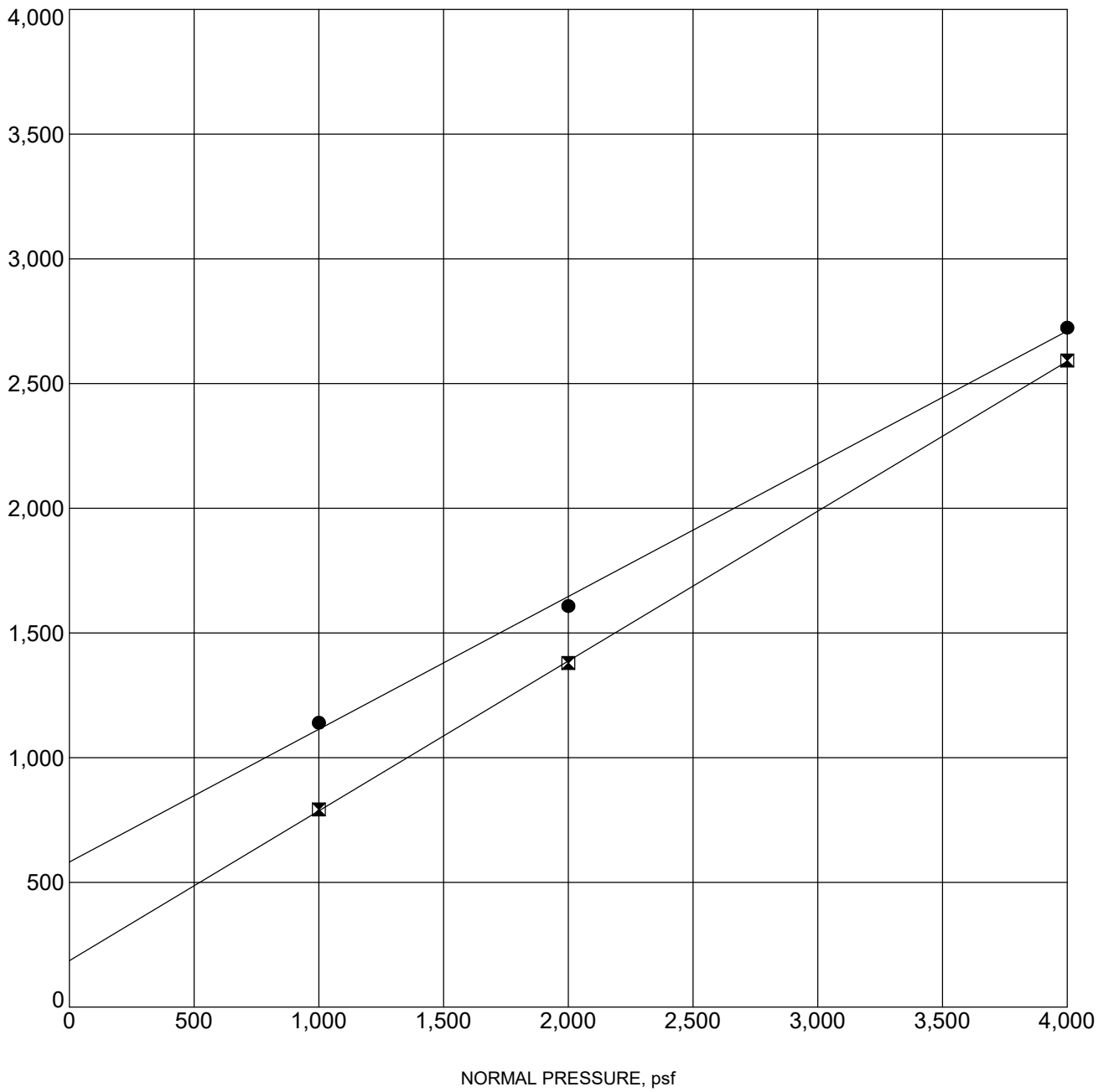
3037 S. Harbor Blvd
 Santa Ana, CA 92704
 Telephone: 714-641-7189
 Fax:

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

SHEAR STRENGTH, psf



NORMAL PRESSURE, psf

Specimen Identification	Classification		γ_d	MC%	c	ϕ
● BA-1 0-5	Fill: Sandy Silt- Remolded Peak, 95% Compaction		116	15	582	28
☒ BA-1 0-5	Fill: Sandy Silt- Remolded Ultimate, 95% Compaction		116	15	186	31



TGR GEOTECHNICAL, INC. Fax:

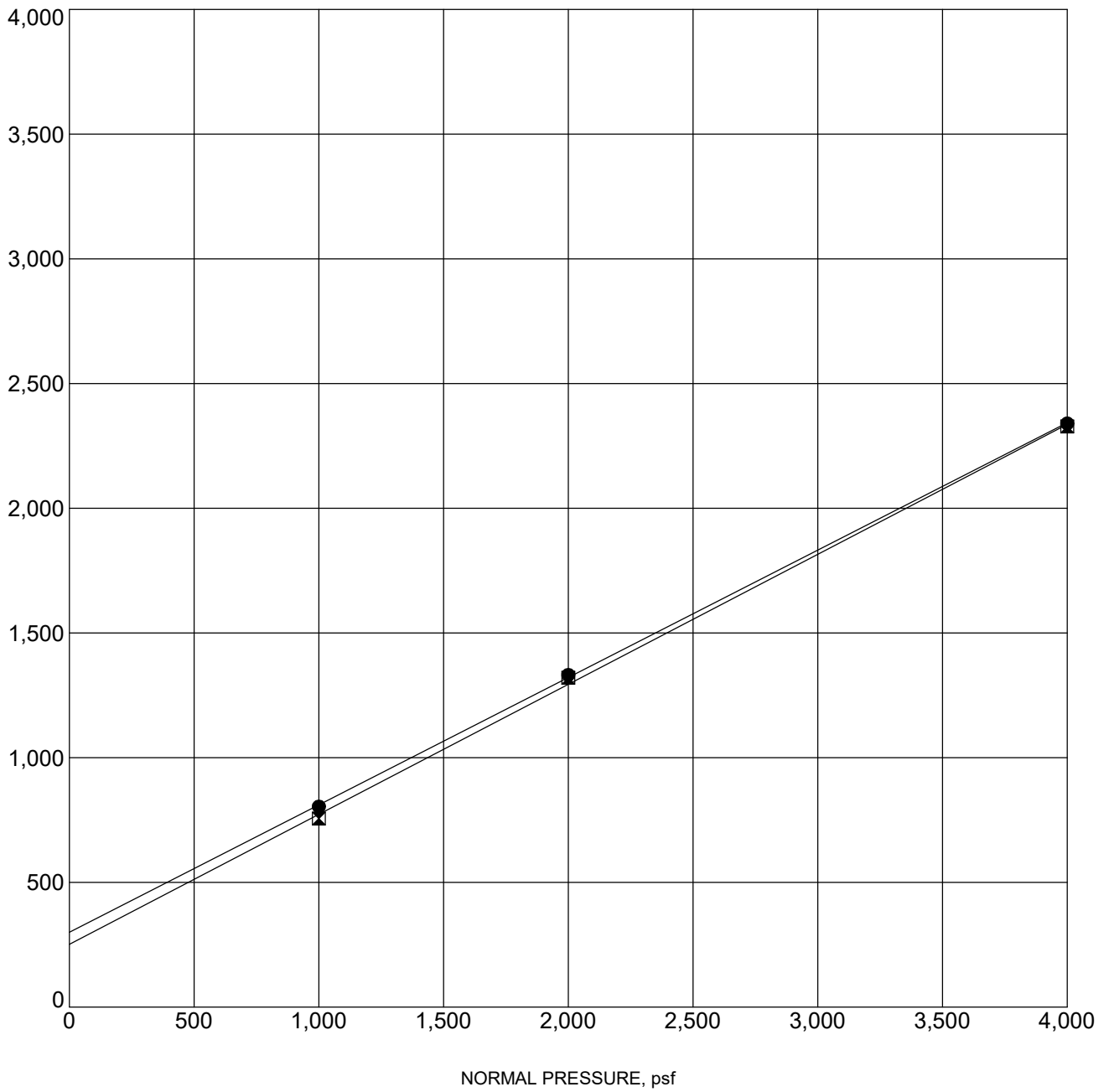
3037 S. Harbor Blvd
 Santa Ana, CA 92704
 Telephone: 714-641-7189

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

SHEAR STRENGTH, psf



NORMAL PRESSURE, psf

Specimen Identification	Classification		γ_d	MC%	c	ϕ
● BA-1 0-5	Fill: Sandy Silt- Remolded Peak, 90% Compaction		116	15	300	27
▣ BA-1 0-5	Fill: Sandy Silt- Remolded Ultimate, 90% Compaction		116	15	252	28



TGR GEOTECHNICAL, INC. Fax:

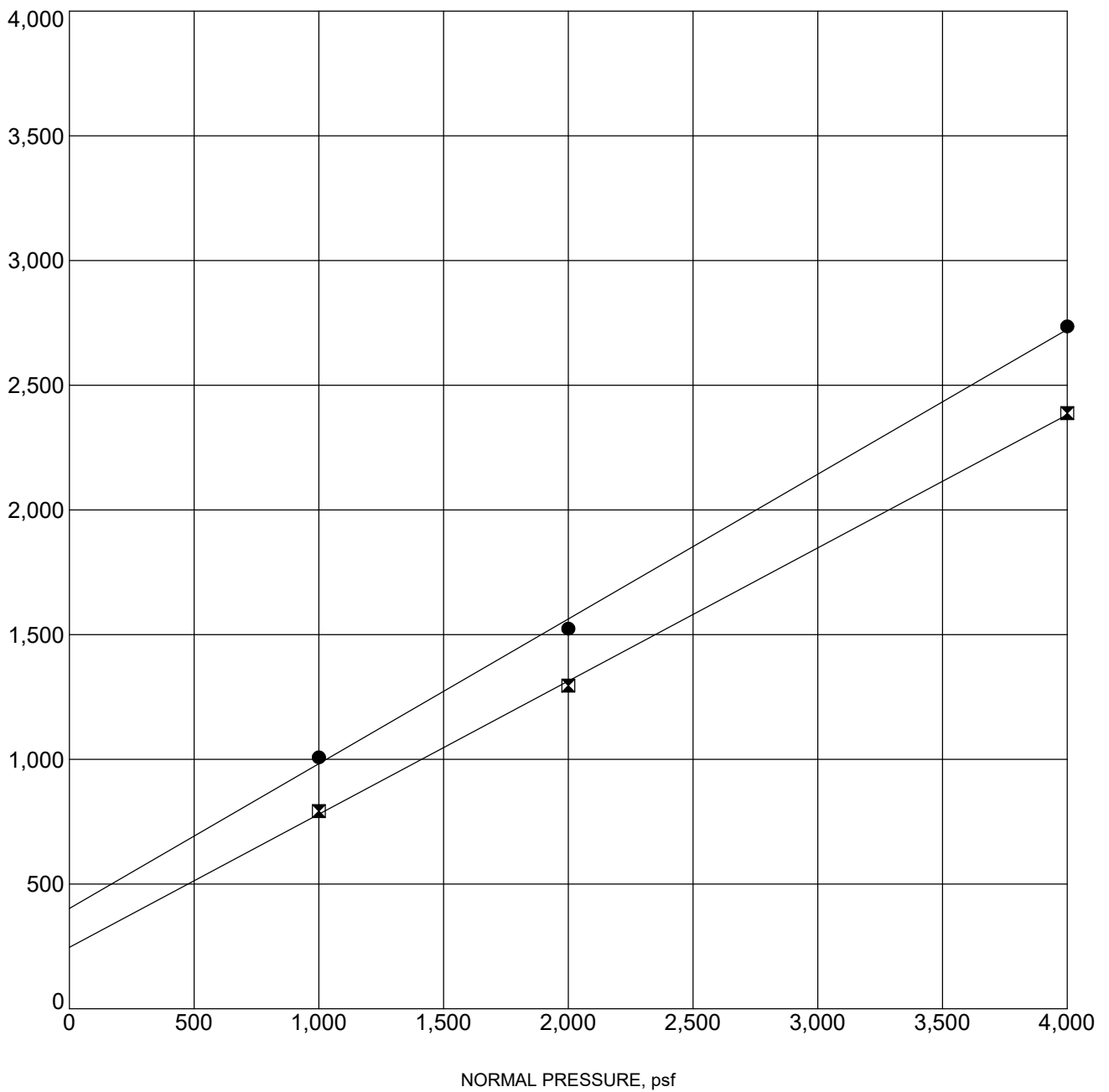
3037 S. Harbor Blvd
 Santa Ana, CA 92704
 Telephone: 714-641-7189

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

SHEAR STRENGTH, psf



Specimen Identification			Classification	γ_d	MC%	c	ϕ
●	BA-1	45	(Tpl) Bedrock- Remolded Peak, 95% Compaction	111	18	402	30
⊠	BA-1	45	(Tpl) Bedrock- Remolded Ultimate, 95% Compaction	111	18	246	28



TGR GEOTECHNICAL, INC. Fax:

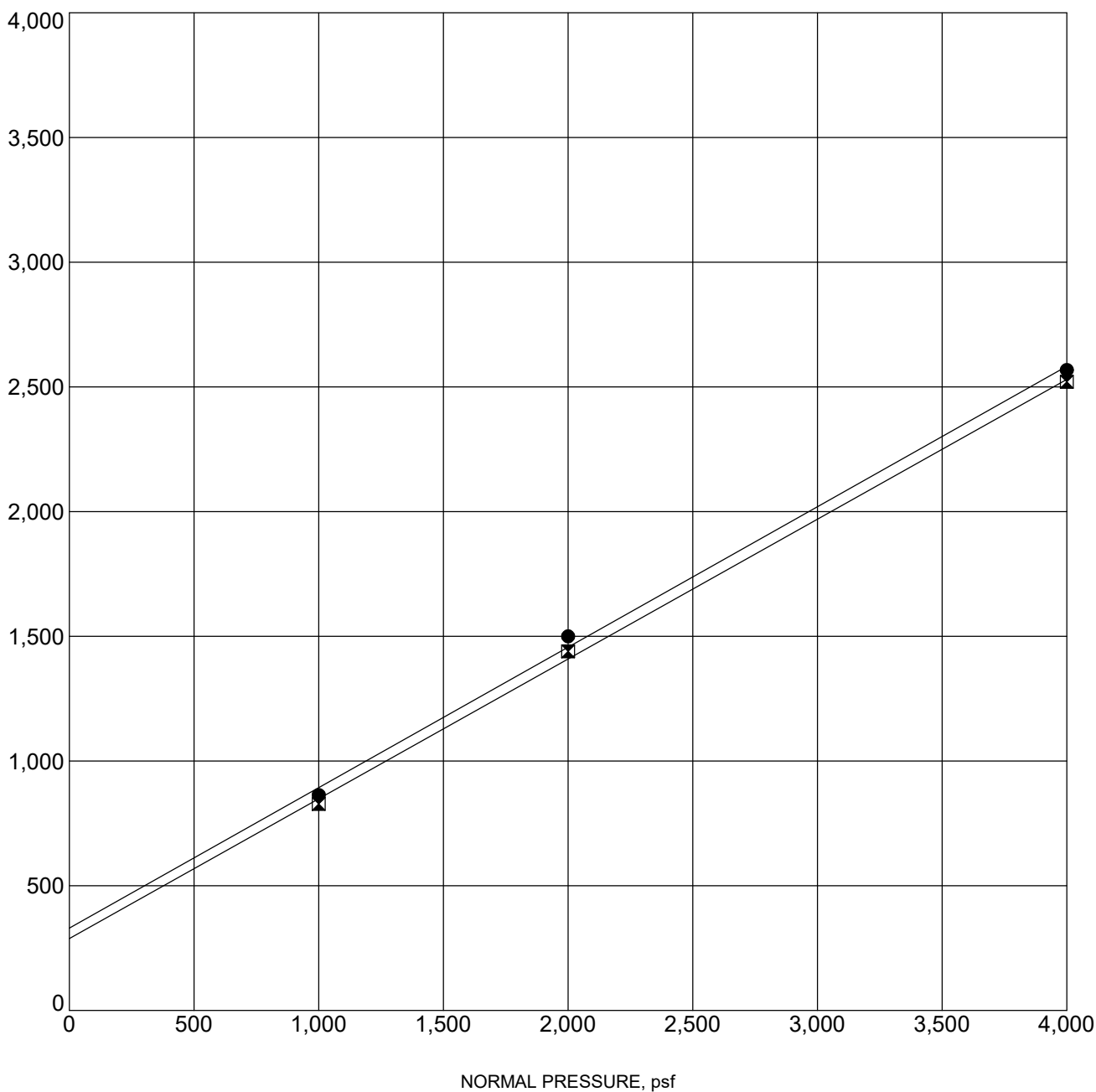
3037 S. Harbor Blvd
 Santa Ana, CA 92704
 Telephone: 714-641-7189

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

SHEAR STRENGTH, psf



Specimen Identification			Classification	γ_d	MC%	c	ϕ
●	BA-1	45	(Tpl) Bedrock- Remolded Peak, 90% Compaction	111	18	330	29
⊠	BA-1	45	(Tpl) Bedrock- Remolded Ultimate, 90% Compaction	111	18	288	29



TGR GEOTECHNICAL, INC. Fax:

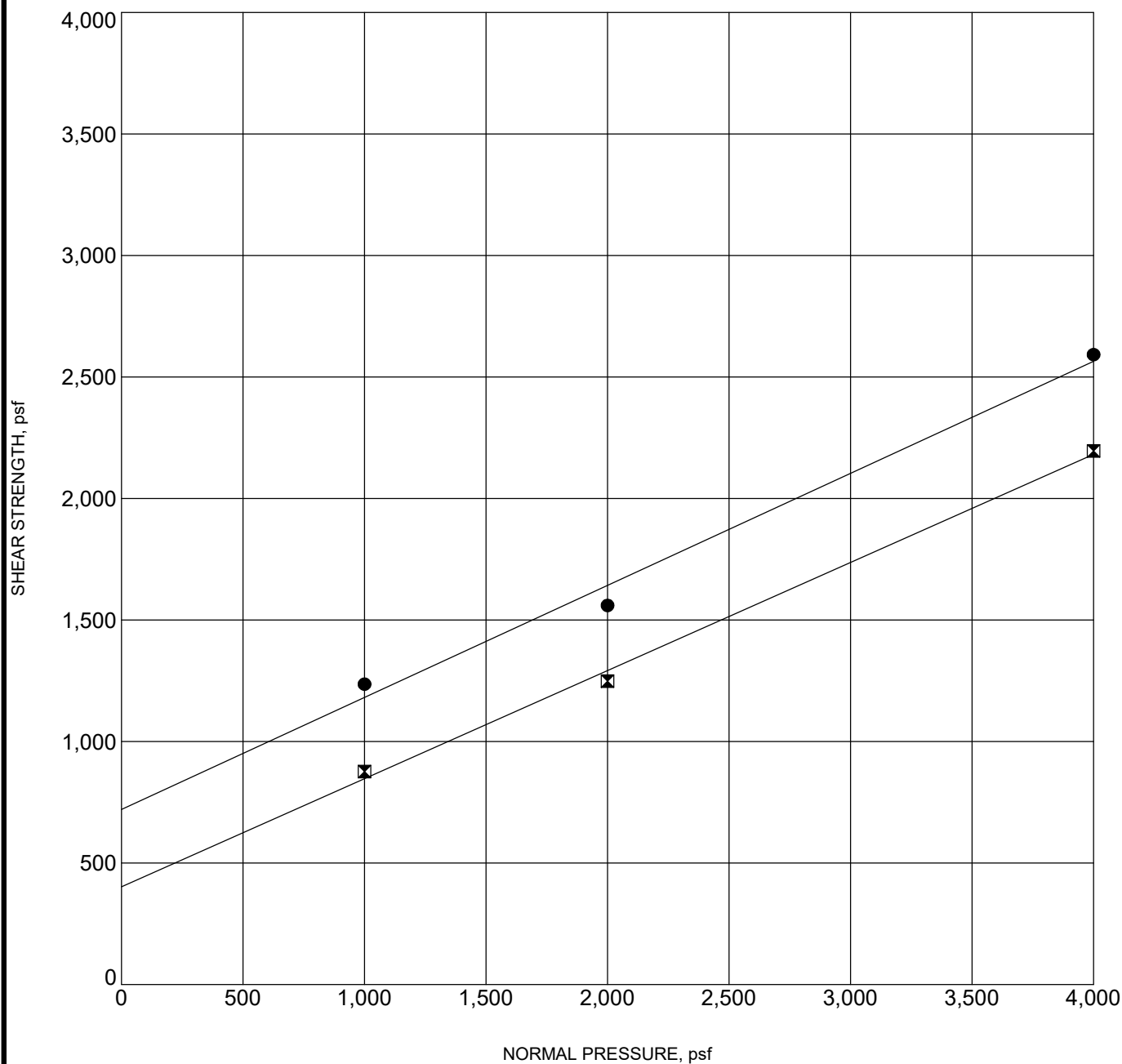
3037 S. Harbor Blvd
Santa Ana, CA 92704
Telephone: 714-641-7189

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

US DIRECT SHEAR 20-7009 HOFF PROPERTY, YORBA LINDA.GPJ TGR GEOTECH.GDT 5/7/20



Specimen Identification			Classification	γ_d	MC%	c	ϕ
●	BA-2	10	(Tpl) Bedrock- Remolded Peak, 95% Compaction	108	20	720	25
⊠	BA-2	10	(Tpl) Bedrock- Remolded Ultimate, 95% Compaction	108	20	402	24



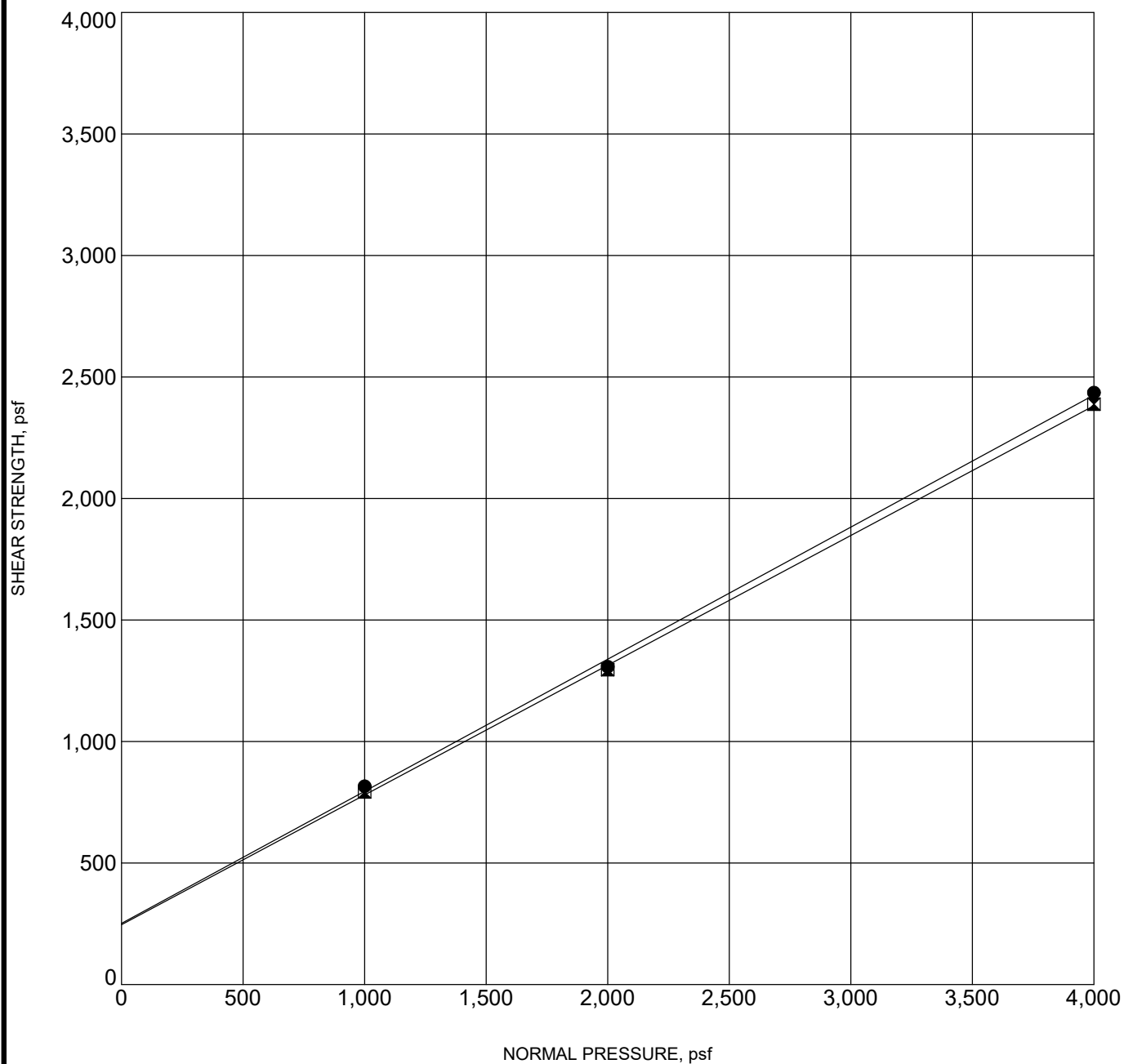
TGR GEOTECHNICAL, INC.

3037 S. Harbor Blvd
 Santa Ana, CA 92704
 Telephone: 714-641-7189
 Fax:

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda



Specimen Identification	Classification		γ_d	MC%	c	ϕ
● BA-2 10	(Tpl) Bedrock- Remolded Peak, 90% Compaction		108	20	252	29
☒ BA-2 10	(Tpl) Bedrock- Remolded Ultimate, 90% Compaction		108	20	246	28



TGR GEOTECHNICAL, INC. Fax:

3037 S. Harbor Blvd
Santa Ana, CA 92704
Telephone: 714-641-7189

DIRECT SHEAR TEST

Project Number: 20-7009

Project Name: Hoff Property, Yorba Linda

ANAHEIM TEST LAB, INC.

196 Technology Drive, Unit D
Irvine, CA 92618
Phone (949)336-6544

TO:

TGR GEOTECHNICAL
3037 S. HARBOR BLVD.
SANTA ANA, CA. 92704

DATE: 04/28/2020

P.O. NO: VERBAL

LAB NO: C-3771

SPECIFICATION: CTM-417

MATERIAL: Soil

Project No.: 20-7009
Project: Huff Property
Yorba Linda, CA
Sample ID: B-1 @ 0'-5', Bulk

ANALYTICAL REPORT

SOLUBLE SULFATES

per CT. 417
ppm

765

RESPECTFULLY SUBMITTED



WES BRIDGER LAB MANAGER

**APPENDIX D
SLOPE STABILITY**

SURFICIAL SLOPE STABILITY ANALYSIS

SLOPE DESCRIPTION: Fill slope @ 2:1

Friction Angle (degrees)=	28	FACTORS OF SAFETY(FOS)	
(radians)=	0.4887		
Cohesion (psf)=	275	<u>Depth of Saturation (ft)</u>	<u>FOS</u>
Saturated Unit Weight (pcf)=	140	1.0	5.5
Bouyant Unit Weight (pcf)=	77.6	2.0	3.0
Slope angle (degrees)=	26.6	3.0	2.2
(radians)=	0.4643	4.0	1.8

DATA FOR SEISMIC SLOPE DISPLACEMENT ANALYSES

Project: 20-7009, HOFF Property, Sec A

INPUT:	Slide h (ft):	31.09	
	Slide Vs (ft/sec):	1200	For slide mass
	Depth to Rx (km):	0.02	(Rx=Vs>5000 ft/s)
	Site Vs(30) (m/s):	360	Material below slide
	Ave. Site Class:	D	
	Mean Magnitude:	6.57	
	Mean Dist. (km):	10.28	
	ky:	0.300	Insert from slope stability

Average Height Calculation Using CAD Data:		
Slide Mass Area =	9944.48	Insert from slope stability
Horiz.Length of Slide Mas	319.88	Insert from slope stability
Average Slide H (ft) =	31.09	

Ts: 4h/Vs
Tm: Rathje et al. (2004)
PGV: Watson-Lamprey and Abrahamson (2006)
Duration: Bommer et al. (2009)

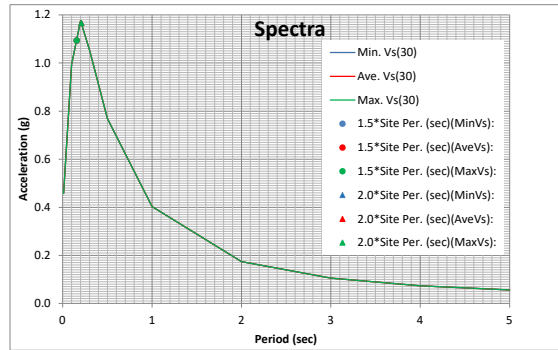
RESULTS:

Based on Min. Vs(30)		Based on Ave. Vs(30)		Based on Max. Vs(30)	
Site Period, Ts (sec):	0.1036	Site Period, Ts (sec):	0.1036	Site Period, Ts (sec):	0.1036
1.5*Site Per. (sec)(MinVs):	0.1554	1.5*Site Per. (sec)(AveVs):	0.1554	1.5*Site Per. (sec)(MaxVs):	0.1554
2.0*Site Per. (sec)(MinVs):	0.2073	2.0*Site Per. (sec)(AveVs):	0.2073	2.0*Site Per. (sec)(MaxVs):	0.2073
EQ Mean Period, Tm (sec):	0.556	EQ Mean Period, Tm (sec):	0.556	EQ Mean Period, Tm (sec):	0.556
Ts/Tm:	0.19	Ts/Tm:	0.19	Ts/Tm:	0.19
Estimated PGV (cm/sec):	48.86	Estimated PGV (cm/sec):	48.86	Estimated PGV (cm/sec):	48.86
Duration (D ₅ -D ₉₅) (sec):	12.7	Duration (D ₅ -D ₉₅) (sec):	12.7	Duration (D ₅ -D ₉₅) (sec):	12.7
Sa(1.5Ts) Min Vs(30)	1.0941	Sa(1.5Ts) Ave Vs(30)	1.0941	Sa(1.5Ts) Max Vs(30)	1.0941
Sa(2.0Ts) Min Vs(30)	1.1663	Sa(2.0Ts) Ave Vs(30)	1.1663	Sa(2.0Ts) Max Vs(30)	1.1663
Mean Mag at Site Per Min	6.57	Mean Mag at Site Per Ave	6.57	Mean Mag at Site Per Ave	6.57
Mean Dist at Site Per Min	10.28	Mean Dist at Site Per Ave	10.28	Mean Dist at Site Per Ave	10.28

Largest Sa(1.5Ts):	1.0941	These values flow to input table at upper left and B&T, R&A, and S&R-M calculations on next pages.
Corresponding Sa(2.0Ts):	1.1663	
Corresponding PGA:	0.4577	
Corresponding PGV:	48.86	
Corresponding Mean Mag:	6.57	
Corresponding Mean Dist:	10.28	

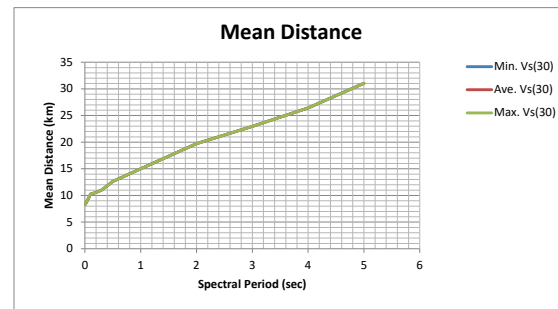
5-Percent Damped Spectra

Vs(30)	480	480	480
Period	Min. Vs(30)	Ave. Vs(30)	Max. Vs(30)
0.01	0.4577	0.4577	0.4577
0.1	0.9933	0.9933	0.9933
0.2	1.1751	1.1751	1.1751
0.3	1.0542	1.0542	1.0542
0.5	0.7690	0.7690	0.7690
1.0	0.4031	0.4031	0.4031
2.0	0.17402	0.17402	0.17402
3.0	0.10542	0.10542	0.10542
4.0	0.07383	0.07383	0.07383
5.0	0.05620	0.05620	0.05620



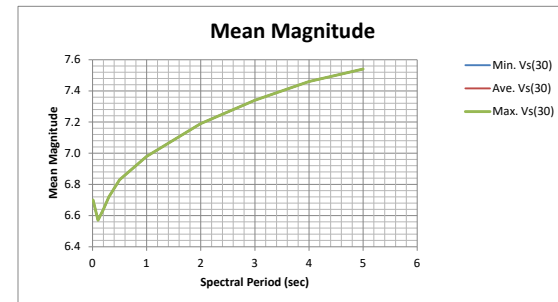
Mean Distance (km)

Period	Min. Vs(30)	Ave. Vs(30)	Max. Vs(30)
0.01	8.4	8.4	8.4
0.1	10.3	10.3	10.3
0.2	10.6	10.6	10.6
0.3	11.0	11.0	11.0
0.5	12.6	12.6	12.6
1.0	15.0	15.0	15.0
2.0	19.7	19.7	19.7
3.0	23.0	23.0	23.0
4.0	26.4	26.4	26.4
5.0	31.0	31.0	31.0



Mean Magnitude

Period	Min. Vs(30)	Ave. Vs(30)	Max. Vs(30)
0.01	6.70	6.70	6.70
0.1	6.57	6.57	6.57
0.2	6.64	6.64	6.64
0.3	6.72	6.72	6.72
0.5	6.83	6.83	6.83
1.0	6.98	6.98	6.98
2.0	7.19	7.19	7.19
3.0	7.34	7.34	7.34
4.0	7.46	7.46	7.46
5.0	7.54	7.54	7.54



Average Seismic Displacement

Seismic Displacement (Bray and Travararou) = 50% Probability of Exceedance (D2)	7.89 cm	3.11 inch
--	---------	-----------

1cm = 0.393701 in.

 Highlight indicates < 1 cm was estimated

Seismic Displacement (Rathje and Antonakos) = [kmax, k-velmax]	1.20 cm	0.47 inch
---	---------	-----------

Seismic Displacement (Song and Rodriguez-Marek) = 50% Probability of Exceedance (D2)	2.16 cm	0.85 inch
---	---------	-----------

Average Displacement (B&T and R&A) =	4.55 cm	1.79 inch
--------------------------------------	---------	-----------

Average Displacement (B&T, R&A, and S&R-A) =	3.75 cm	1.48 inch
--	---------	-----------

DATA FOR SEISMIC SLOPE DISPLACEMENT ANALYSES

Project: 20-7009, HOFF Property, Sec B

INPUT:

Slide h (ft):	15.26
Slide Vs (ft/sec):	1200
Depth to Rx (km):	0
Site Vs(30) (m/s):	760
Ave. Site Class:	C
Mean Magnitude:	6.64
Mean Dist. (km):	9.25
ky:	0.360

Average Height Calculation Using CAD Data:

Slide Mass Area =	3693.71
Horiz.Length of Slide Mass	242.04
Average Slide H (ft) =	15.26

Ts: 4h/Vs
Tm: Rathje et al. (2004)
PGV: Watson-Lamprey and Abrahamson (2006)
Duration: Bommer et al. (2009)

For slide mass
(Rx=Vs>5000 ft/s)
Material below slide

Insert from slope stability

Insert from slope stability
Insert from slope stability

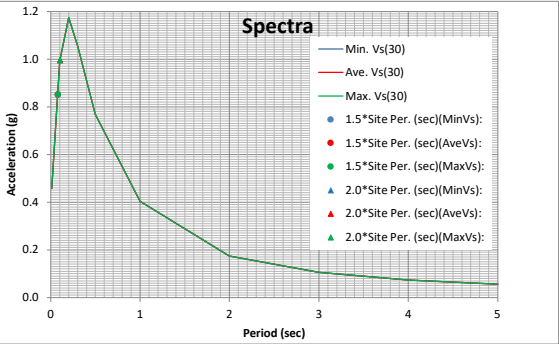
RESULTS:

Based on Min. Vs(30)		Based on Ave. Vs(30)		Based on Max. Vs(30)	
Site Period, Ts (sec):	0.0509	Site Period, Ts (sec):	0.0509	Site Period, Ts (sec):	0.0509
1.5*Site Per. (sec)(MinVs):	0.0763	1.5*Site Per. (sec)(AveVs):	0.0763	1.5*Site Per. (sec)(MaxVs):	0.0763
2.0*Site Per. (sec)(MinVs):	0.1017	2.0*Site Per. (sec)(AveVs):	0.1017	2.0*Site Per. (sec)(MaxVs):	0.1017
EQ Mean Period, Tm (sec):	0.462	EQ Mean Period, Tm (sec):	0.462	EQ Mean Period, Tm (sec):	0.462
Ts/Tm:	0.11	Ts/Tm:	0.11	Ts/Tm:	0.11
Estimated PGV (cm/sec):	49.15	Estimated PGV (cm/sec):	49.15	Estimated PGV (cm/sec):	49.15
Duration (Ds-D95) (sec):	9.9	Duration (Ds-D95) (sec):	9.9	Duration (Ds-D95) (sec):	9.9
Sa(1.5Ts) Min Vs(30)	0.8523	Sa(1.5Ts) Ave Vs(30)	0.8523	Sa(1.5Ts) Max Vs(30)	0.8523
Sa(2.0Ts) Min Vs(30)	0.9965	Sa(2.0Ts) Ave Vs(30)	0.9965	Sa(2.0Ts) Max Vs(30)	0.9965
Mean Mag at Site Per Min	6.64	Mean Mag at Site Per Ave	6.64	Mean Mag at Site Per Ave	6.64
Mean Dist at Site Per Min	9.25	Mean Dist at Site Per Ave	9.25	Mean Dist at Site Per Ave	9.25

Largest Sa(1.5Ts): 0.8523
Corresponding Sa(2.0Ts): 0.9965
Corresponding PGA: 0.4577
Corresponding PGV: 49.15
Corresponding Mean Mag: 6.64
Corresponding Mean Dist: 9.25

These values flow to
input table at upper left and
B&T, R&A, and S&R-M calculations
on next pages.

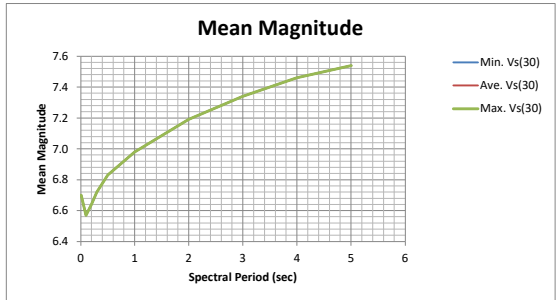
5-Percent Damped Spectra			
Vs(30)	480	480	480
Period	Min. Vs(30)	Ave. Vs(30)	Max. Vs(30)
0.01	0.4577	0.4577	0.4577
0.1	0.9933	0.9933	0.9933
0.2	1.1751	1.1751	1.1751
0.3	1.0542	1.0542	1.0542
0.5	0.7690	0.7690	0.7690
1.0	0.4031	0.4031	0.4031
2.0	0.17402	0.17402	0.17402
3.0	0.10542	0.10542	0.10542
4.0	0.07383	0.07383	0.07383
5.0	0.05620	0.05620	0.05620



Mean Distance (km)			
Period	Min. Vs(30)	Ave. Vs(30)	Max. Vs(30)
0.01	8.4	8.4	8.4
0.1	10.3	10.3	10.3
0.2	10.6	10.6	10.6
0.3	11.0	11.0	11.0
0.5	12.6	12.6	12.6
1.0	15.0	15.0	15.0
2.0	19.7	19.7	19.7
3.0	23.0	23.0	23.0
4.0	26.4	26.4	26.4
5.0	31.0	31.0	31.0



Mean Magnitude			
Period	Min. Vs(30)	Ave. Vs(30)	Max. Vs(30)
0.01	6.70	6.70	6.70
0.1	6.57	6.57	6.57
0.2	6.64	6.64	6.64
0.3	6.72	6.72	6.72
0.5	6.83	6.83	6.83
1.0	6.98	6.98	6.98
2.0	7.19	7.19	7.19
3.0	7.34	7.34	7.34
4.0	7.46	7.46	7.46
5.0	7.54	7.54	7.54



Average Seismic Displacement

Seismic Displacement (Bray and Travararou) = 50% Probability of Exceedance (D2)	2.36 cm	0.93 inch
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1cm = 0.393701 in.

 Highlight indicates < 1 cm was estimated

Seismic Displacement (Rathje and Antonakos) = [kmax, k-velmax]	0.29 cm	0.11 inch
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Seismic Displacement (Song and Rodriguez-Marek) = 50% Probability of Exceedance (D2)	1.00 cm	0.39 inch
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Average Displacement (B&T and R&A) =	1.33 cm	0.52 inch
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Average Displacement (B&T, R&A, and S&R-A) =	1.22 cm	0.48 inch
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DATA FOR SEISMIC SLOPE DISPLACEMENT ANALYSES

Project: 20-7009, HOFF Property, Sec C

INPUT:

Slide h (ft):	14.17
Slide Vs (ft/sec):	1200
Depth to Rx (km):	0
Site Vs(30) (m/s):	760
Ave. Site Class:	C
Mean Magnitude:	6.65
Mean Dist. (km):	9.17
ky:	0.350

Average Height Calculation Using CAD Data:

Slide Mass Area =	3451.00
Horiz.Length of Slide Mass	243.50
Average Slide H (ft) =	14.17

Ts: 4h/Vs
Tm: Rathje et al. (2004)
PGV: Watson-Lamprey and Abrahamson (2006)
Duration: Bommer et al. (2009)

For slide mass
(Rx=Vs>5000 ft/s)
Material below slide

Insert from slope stability

Insert from slope stability
Insert from slope stability

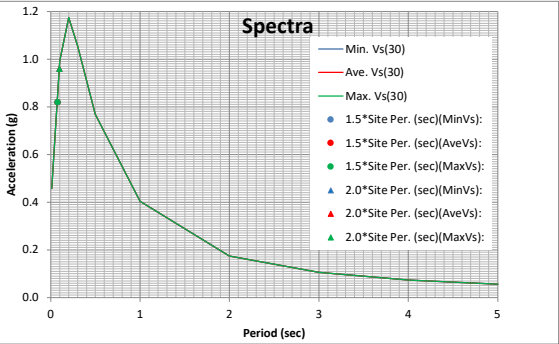
RESULTS:

Based on Min. Vs(30)		Based on Ave. Vs(30)		Based on Max. Vs(30)	
Site Period, Ts (sec):	0.0472	Site Period, Ts (sec):	0.0472	Site Period, Ts (sec):	0.0472
1.5*Site Per. (sec)(MinVs):	0.0709	1.5*Site Per. (sec)(AveVs):	0.0709	1.5*Site Per. (sec)(MaxVs):	0.0709
2.0*Site Per. (sec)(MinVs):	0.0945	2.0*Site Per. (sec)(AveVs):	0.0945	2.0*Site Per. (sec)(MaxVs):	0.0945
EQ Mean Period, Tm (sec):	0.463	EQ Mean Period, Tm (sec):	0.463	EQ Mean Period, Tm (sec):	0.463
Ts/Tm:	0.10	Ts/Tm:	0.10	Ts/Tm:	0.10
Estimated PGV (cm/sec):	49.18	Estimated PGV (cm/sec):	49.18	Estimated PGV (cm/sec):	49.18
Duration (Ds-D99) (sec):	9.9	Duration (Ds-D99) (sec):	9.9	Duration (Ds-D99) (sec):	9.9
Sa(1.5Ts) Min Vs(30)	0.8199	Sa(1.5Ts) Ave Vs(30)	0.8199	Sa(1.5Ts) Max Vs(30)	0.8199
Sa(2.0Ts) Min Vs(30)	0.9605	Sa(2.0Ts) Ave Vs(30)	0.9605	Sa(2.0Ts) Max Vs(30)	0.9605
Mean Mag at Site Per Min	6.65	Mean Mag at Site Per Ave	6.65	Mean Mag at Site Per Ave	6.65
Mean Dist at Site Per Min	9.17	Mean Dist at Site Per Ave	9.17	Mean Dist at Site Per Ave	9.17

Largest Sa(1.5Ts): 0.8199
Corresponding Sa(2.0Ts): 0.9605
Corresponding PGA: 0.4577
Corresponding PGV: 49.18
Corresponding Mean Mag: 6.65
Corresponding Mean Dist: 9.17

These values flow to input table at upper left and B&T, R&A, and S&R-M calculations on next pages.

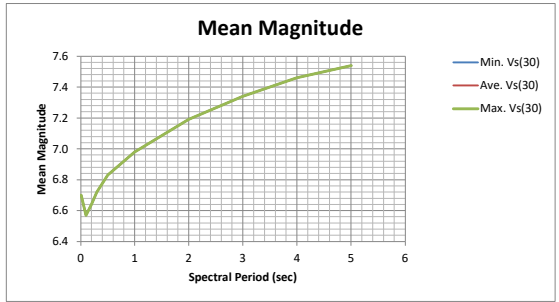
5-Percent Damped Spectra			
Vs(30)	480	480	480
Period	Min. Vs(30)	Ave. Vs(30)	Max. Vs(30)
0.01	0.4577	0.4577	0.4577
0.1	0.9933	0.9933	0.9933
0.2	1.1751	1.1751	1.1751
0.3	1.0542	1.0542	1.0542
0.5	0.7690	0.7690	0.7690
1.0	0.4031	0.4031	0.4031
2.0	0.17402	0.17402	0.17402
3.0	0.10542	0.10542	0.10542
4.0	0.07383	0.07383	0.07383
5.0	0.05620	0.05620	0.05620



Mean Distance (km)			
Period	Min. Vs(30)	Ave. Vs(30)	Max. Vs(30)
0.01	8.4	8.4	8.4
0.1	10.3	10.3	10.3
0.2	10.6	10.6	10.6
0.3	11.0	11.0	11.0
0.5	12.6	12.6	12.6
1.0	15.0	15.0	15.0
2.0	19.7	19.7	19.7
3.0	23.0	23.0	23.0
4.0	26.4	26.4	26.4
5.0	31.0	31.0	31.0



Mean Magnitude			
Period	Min. Vs(30)	Ave. Vs(30)	Max. Vs(30)
0.01	6.70	6.70	6.70
0.1	6.57	6.57	6.57
0.2	6.64	6.64	6.64
0.3	6.72	6.72	6.72
0.5	6.83	6.83	6.83
1.0	6.98	6.98	6.98
2.0	7.19	7.19	7.19
3.0	7.34	7.34	7.34
4.0	7.46	7.46	7.46
5.0	7.54	7.54	7.54



Average Seismic Displacement

Seismic Displacement (Bray and Travararou) = 50% Probability of Exceedance (D2)	5.40 cm	2.13 inch
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1cm = 0.393701 in.

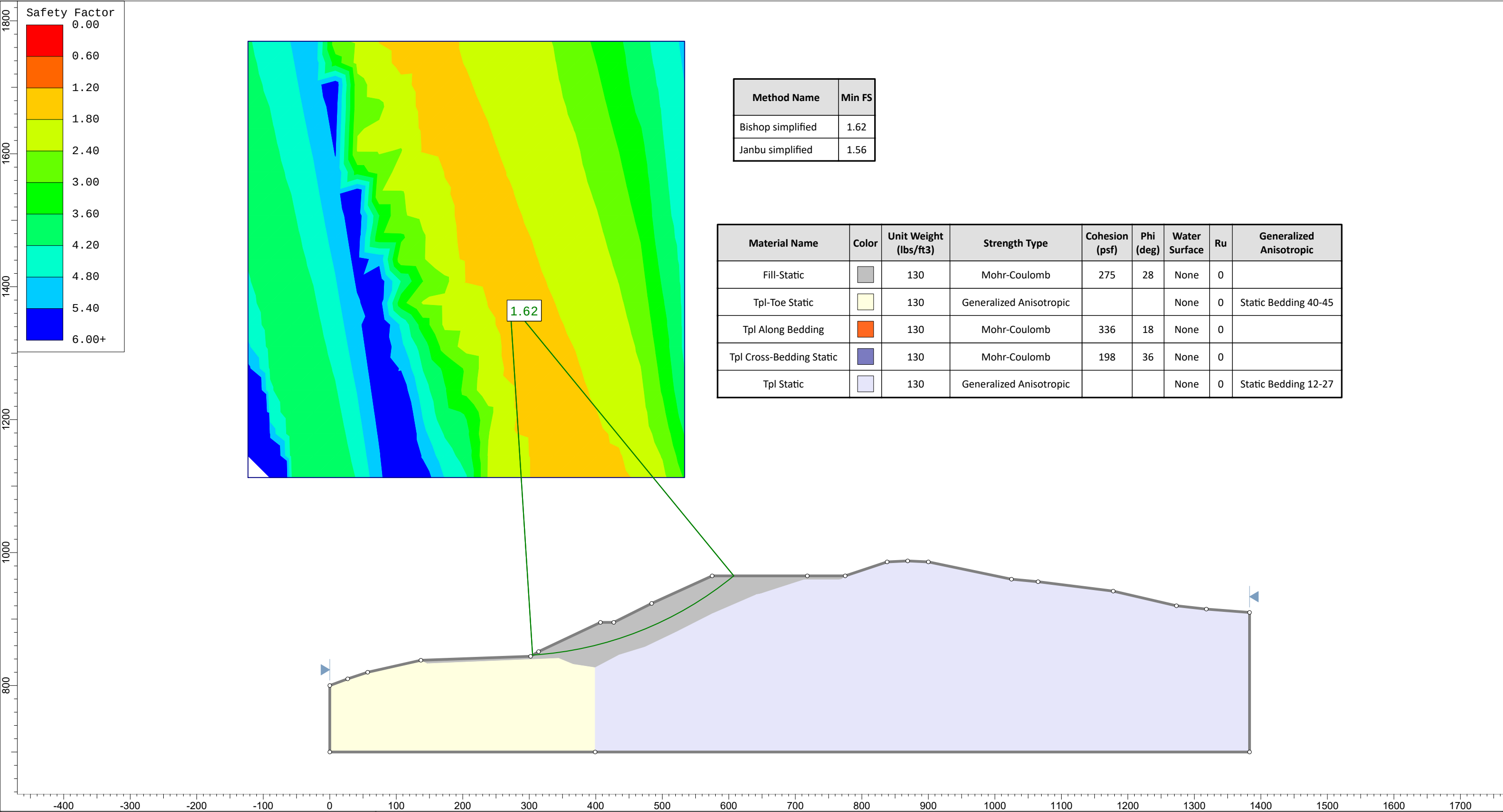
 Highlight indicates < 1 cm was estimated

Seismic Displacement (Rathje and Antonakos) = [kmax, k-velmax]	0.35 cm	0.14 inch
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Seismic Displacement (Song and Rodriguez-Marek) = 50% Probability of Exceedance (D2)	1.00 cm	0.39 inch
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Average Displacement (B&T and R&A) =	2.88 cm	1.13 inch
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Average Displacement (B&T, R&A, and S&R-A) =	2.25 cm	0.89 inch
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Method Name	Min FS
Bishop simplified	1.62
Janbu simplified	1.56

Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (deg)	Water Surface	Ru	Generalized Anisotropic
Fill-Static		130	Mohr-Coulomb	275	28	None	0	
Tpl-Toe Static		130	Generalized Anisotropic			None	0	Static Bedding 40-45
Tpl Along Bedding		130	Mohr-Coulomb	336	18	None	0	
Tpl Cross-Bedding Static		130	Mohr-Coulomb	198	36	None	0	
Tpl Static		130	Generalized Anisotropic			None	0	Static Bedding 12-27

Slide Analysis Information

HOFF Property, Yorba Linda, CA

Project Summary

File Name: Cross-section A-A' Static
 Slide Modeler Version: 7.038
 Project Title: HOFF Property, Yorba Linda, CA
 Analysis: Cross-section A-A' Static
 Company: TGR Geotechnical, Inc.
 Date Created: 4/15/20

General Settings

Units of Measurement: Imperial Units
 Time Units: days
 Permeability Units: feet/second
 Failure Direction: Right to Left
 Data Output: Standard
 Maximum Material Properties: 20
 Maximum Support Properties: 20

Analysis Options

Slices Type: Vertical

Analysis Methods Used

Bishop simplified
 Janbu simplified

Number of slices: 50
 Tolerance: 0.005
 Maximum number of iterations: 75
 Check $m\alpha < 0.2$: Yes
 Create Interslice boundaries at intersections with water tables and piezos: Yes
 Initial trial value of FS: 1
 Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
 Pore Fluid Unit Weight [lbs/ft³]: 62.4
 Use negative pore pressure cutoff: Yes
 Maximum negative pore pressure [psf]: 0
 Advanced Groundwater Method: None

Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3

Surface Options

Surface Type: Circular
 Search Method: Grid Search
 Radius Increment: 10
 Composite Surfaces: Disabled
 Reverse Curvature: Invalid Surfaces
 Minimum Elevation: Not Defined
 Minimum Depth: Not Defined
 Minimum Area: Not Defined
 Minimum Weight: Not Defined

Seismic

Advanced seismic analysis: No
 Staged pseudostatic analysis: No

Material Properties

Property	Fill-Static	Tpl-Toe Static	Tpl Static
Color			
Strength Type	Mohr-Coulomb	Generalized Anisotropic	Generalized Anisotropic
Unit Weight [lbs/ft3]	130	130	130
Cohesion [psf]	275		
Friction Angle [deg]	28		
Water Surface	None	None	None
Ru Value	0	0	0

Generalized Anisotropic Functions

Name: Static Bedding 40-45

Angle From	Angle To	Material
40	-90	Tpl Cross-Bedding Static
45	40	Tpl Along Bedding
90	45	Tpl Cross-Bedding Static

Name: Static Bedding 12-27

Angle From	Angle To	Material
-27	-90	Tpl Cross-Bedding Static
-12	-27	Tpl Along Bedding
90	-12	Tpl Cross-Bedding Static

Global Minimums

Method: bishop simplified

FS	1.618140
Center:	270.990, 1375.482
Radius:	530.809
Left Slip Surface Endpoint:	305.025, 845.765
Right Slip Surface Endpoint:	607.536, 965.000
Resisting Moment:	3.08296e+008 lb-ft
Driving Moment:	1.90525e+008 lb-ft
Total Slice Area:	7521.42 ft ²
Surface Horizontal Width:	302.511 ft
Surface Average Height:	24.8633 ft

Method: janbu simplified

FS	1.562180
Center:	303.811, 1244.195
Radius:	404.150
Left Slip Surface Endpoint:	259.686, 842.461
Right Slip Surface Endpoint:	596.022, 965.000
Resisting Horizontal Force:	636385 lb
Driving Horizontal Force:	407369 lb
Total Slice Area:	9099.64 ft ²
Surface Horizontal Width:	336.336 ft
Surface Average Height:	27.0552 ft

Valid / Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4601
Number of Invalid Surfaces: 250

Error Codes:

Error Code -103 reported for 232 surfaces
Error Code -106 reported for 7 surfaces
Error Code -1000 reported for 11 surfaces

Method: janbu simplified

Number of Valid Surfaces: 4596
Number of Invalid Surfaces: 255

Error Codes:

Error Code -103 reported for 232 surfaces
Error Code -106 reported for 7 surfaces
Error Code -108 reported for 5 surfaces
Error Code -1000 reported for 11 surfaces

Error Codes

The following errors were encountered during the computation:

- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 106 = Average slice width is less than 0.0001 * (maximum horizontal extent of soil region). This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

Slice Data

Global Minimum Query (bishop simplified) - Safety Factor: 1.61814

Slice Number	Width [ft]	Weight [lbs]	Angle of Slice Base [degrees]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]	Base Vertical Stress [psf]	Effective Vertical Stress [psf]
1	6.05022	1221.42	4.00363	Fill-Static	275	28	230.975	373.75	185.722	0	185.722	201.888	201.888
2	6.05022	3566.92	4.65858	Fill-Static	275	28	354.192	573.132	560.704	0	560.704	589.566	589.566
3	6.05022	5464.42	5.31414	Fill-Static	275	28	452.89	732.84	861.071	0	861.071	903.197	903.197
4	6.05022	7245.68	5.9704	Fill-Static	275	28	544.757	881.493	1140.65	0	1140.65	1197.62	1197.62
5	6.05022	8971.81	6.62744	Fill-Static	275	28	633.058	1024.38	1409.37	0	1409.37	1482.93	1482.93
6	6.05022	10642.6	7.28536	Fill-Static	275	28	717.817	1161.53	1667.33	0	1667.33	1759.1	1759.1
7	6.05022	12257.8	7.94425	Fill-Static	275	28	799.059	1292.99	1914.55	0	1914.55	2026.06	2026.06
8	6.05022	13817.2	8.6042	Fill-Static	275	28	876.797	1418.78	2151.14	0	2151.14	2283.81	2283.81
9	6.05022	15320.4	9.2653	Fill-Static	275	28	951.055	1538.94	2377.13	0	2377.13	2532.28	2532.28
10	6.05022	16767.2	9.92765	Fill-Static	275	28	1021.85	1653.5	2592.58	0	2592.58	2771.43	2771.43
11	6.05022	18157.3	10.5913	Fill-Static	275	28	1089.2	1762.47	2797.52	0	2797.52	3001.19	3001.19
12	6.05022	19490.2	11.2565	Fill-Static	275	28	1153.11	1865.89	2992.03	0	2992.03	3221.53	3221.53
13	6.05022	20765.7	11.9231	Fill-Static	275	28	1213.59	1963.76	3176.1	0	3176.1	3432.35	3432.35
14	6.05022	21983.2	12.5915	Fill-Static	275	28	1270.66	2056.11	3349.77	0	3349.77	3633.6	3633.6
15	6.05022	23142.4	13.2615	Fill-Static	275	28	1324.32	2142.94	3513.09	0	3513.09	3825.21	3825.21
16	6.05022	24242.7	13.9334	Fill-Static	275	28	1374.59	2224.28	3666.06	0	3666.06	4007.09	4007.09
17	6.05022	25260	14.6073	Fill-Static	275	28	1420.28	2298.21	3805.1	0	3805.1	4175.25	4175.25
18	6.05022	24812.2	15.2832	Fill-Static	275	28	1392.55	2253.34	3720.72	0	3720.72	4101.24	4101.24
19	6.05022	23481.5	15.9614	Fill-Static	275	28	1321.15	2137.81	3503.43	0	3503.43	3881.3	3881.3
20	6.05022	22089.8	16.6418	Fill-Static	275	28	1247.23	2018.19	3278.47	0	3278.47	3651.27	3651.27
21	6.05022	21475	17.3247	Fill-Static	275	28	1212.1	1961.34	3171.55	0	3171.55	3549.65	3549.65
22	6.05022	22307.9	18.0101	Fill-Static	275	28	1248.23	2019.81	3281.51	0	3281.51	3687.33	3687.33
23	6.05022	23108.4	18.6982	Fill-Static	275	28	1282.44	2075.17	3385.62	0	3385.62	3819.65	3819.65
24	6.05022	23845.1	19.3891	Fill-Static	275	28	1313.21	2124.95	3479.26	0	3479.26	3941.43	3941.43
25	6.05022	24517.1	20.0829	Fill-Static	275	28	1340.53	2169.17	3562.41	0	3562.41	4052.52	4052.52
26	6.05022	25123.7	20.7798	Fill-Static	275	28	1364.4	2207.79	3635.05	0	3635.05	4152.78	4152.78
27	6.05022	25663.9	21.48	Fill-Static	275	28	1384.81	2240.81	3697.16	0	3697.16	4242.09	4242.09
28	6.05022	26136.7	22.1835	Fill-Static	275	28	1401.74	2268.21	3748.69	0	3748.69	4320.25	4320.25
29	6.05022	26541.3	22.8906	Fill-	275	28	1415.19	2289.97	3789.61	0	3789.61	4387.13	4387.13

30	6.05022	26858.1	23.6014	Static Fill-Static	275	28	1424.26	2304.65	3817.22	0	3817.22	4439.5	4439.5
31	6.05022	26949	24.3161	Static Fill-Static	275	28	1422.47	2301.76	3811.78	0	3811.78	4454.54	4454.54
32	6.05022	26932.8	25.0348	Static Fill-Static	275	28	1415.56	2290.57	3790.73	0	3790.73	4451.87	4451.87
33	6.05022	26843.6	25.7577	Static Fill-Static	275	28	1405.18	2273.77	3759.13	0	3759.13	4437.14	4437.14
34	6.05022	26680.2	26.4851	Static Fill-Static	275	28	1391.3	2251.32	3716.91	0	3716.91	4410.14	4410.14
35	6.05022	26441.1	27.2171	Static Fill-Static	275	28	1373.92	2223.19	3664.02	0	3664.02	4370.64	4370.64
36	6.05022	26124.9	27.954	Static Fill-Static	275	28	1353	2189.35	3600.37	0	3600.37	4318.38	4318.38
37	6.05022	25730	28.6959	Static Fill-Static	275	28	1328.53	2149.74	3525.88	0	3525.88	4253.1	4253.1
38	6.05022	25254.7	29.4431	Static Fill-Static	275	28	1300.47	2104.34	3440.49	0	3440.49	4174.55	4174.55
39	6.05022	24697.2	30.1959	Static Fill-Static	275	28	1268.79	2053.08	3344.08	0	3344.08	4082.41	4082.41
40	6.05022	24055.7	30.9544	Static Fill-Static	275	28	1233.47	1995.92	3236.57	0	3236.57	3976.38	3976.38
41	6.05022	23328.2	31.7191	Static Fill-Static	275	28	1194.46	1932.8	3117.88	0	3117.88	3856.14	3856.14
42	6.05022	22512.6	32.4901	Static Fill-Static	275	28	1151.74	1863.67	2987.86	0	2987.86	3721.32	3721.32
43	6.05022	21606.5	33.2677	Static Fill-Static	275	28	1105.26	1788.47	2846.42	0	2846.42	3571.55	3571.55
44	6.05022	20607.6	34.0524	Static Fill-Static	275	28	1055	1707.13	2693.45	0	2693.45	3406.45	3406.45
45	6.05022	19358.5	34.8444	Static Fill-Static	275	28	994.049	1608.51	2507.96	0	2507.96	3199.99	3199.99
46	6.05022	16416.2	35.644	Static Fill-Static	275	28	859.178	1390.27	2097.52	0	2097.52	2713.63	2713.63
47	6.05022	12952.5	36.4518	Static Fill-Static	275	28	702.89	1137.37	1621.9	0	1621.9	2141.09	2141.09
48	6.05022	9384.45	37.2681	Static Fill-Static	275	28	543.743	879.852	1137.56	0	1137.56	1551.3	1551.3
49	6.05022	5708.79	38.0933	Static Fill-Static	275	28	381.721	617.678	644.483	0	644.483	943.719	943.719
50	6.05022	1921.8	38.928	Static Fill-Static	275	28	216.81	350.828	142.612	0	142.612	317.73	317.73

Global Minimum Query (janbu simplified) - Safety Factor: 1.56218

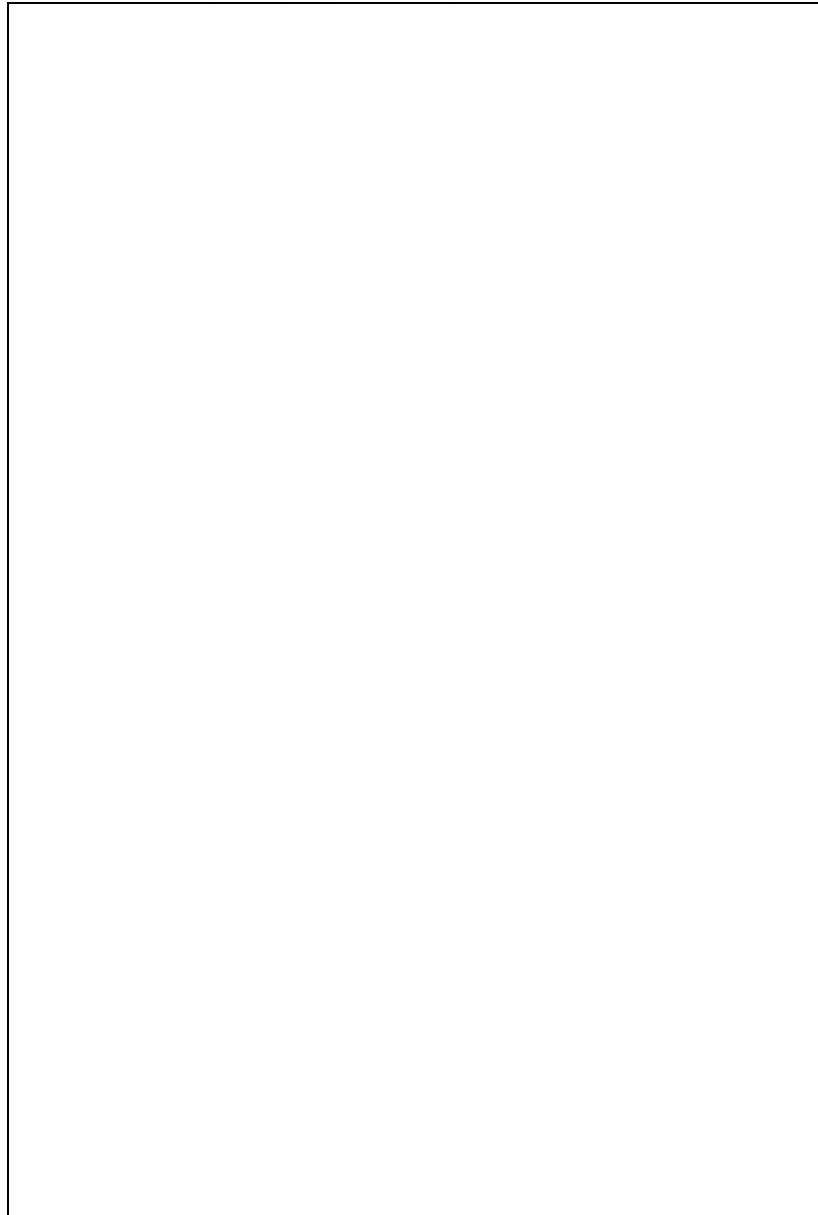
Slice Number	Width [ft]	Weight [lbs]	Angle of Slice Base [degrees]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]	Base Vertical Stress [psf]	Effective Vertical Stress [psf]
1	6.72673	405.127	-5.78885	Fill-Static	275	28	203.551	317.983	80.8387	0	80.8387	60.2028	60.2028
2	6.72673	1165.79	-4.83105	Fill-Static	275	28	241.977	378.011	193.735	0	193.735	173.284	173.284
3	6.72673	1827.48	-3.87461	Fill-Static	275	28	274.832	429.337	290.267	0	290.267	271.653	271.653
4	6.72673	2390.56	-2.91925	Fill-Static	275	28	302.235	472.146	370.777	0	370.777	355.365	355.365
5	6.72673	2855.35	-1.9647	Fill-Static	275	28	324.295	506.607	435.59	0	435.59	424.466	424.466
6	6.72673	3222.03	-1.01069	Fill-Static	275	28	341.112	532.879	485	0	485	478.982	478.982
7	6.72673	4300.64	-0.0569686	Fill-Static	275	28	393.776	615.149	639.727	0	639.727	639.335	639.335
8	6.72673	7553.17	0.89674	Fill-Static	275	28	555.262	867.419	1114.18	0	1114.18	1122.87	1122.87
9	6.72673	10555.7	1.9507	Fill-Static	275	28	702.827	1089.1	1549.02	0	1549.02	1570.74	1570.74

10	6.72673	13111.4	2.80517	Static Fill-Static	275	28	825.705	1289.9	1908.74	0	1908.74	1949.2	1949.2
11	6.72673	15557.1	3.76042	Static Fill-Static	275	28	942.151	1471.81	2250.88	0	2250.88	2312.8	2312.8
12	6.72673	17904.1	4.71672	Static Fill-Static	275	28	1052.44	1644.1	2574.9	0	2574.9	2661.73	2661.73
13	6.72673	20152.3	5.67434	Static Fill-Static	275	28	1156.65	1806.89	2881.05	0	2881.05	2995.98	2995.98
14	6.72673	22301	6.63355	Static Fill-Static	275	28	1254.82	1960.26	3169.52	0	3169.52	3315.45	3315.45
15	6.72673	24349.9	7.59464	Static Fill-Static	275	28	1347.05	2104.33	3440.47	0	3440.47	3620.07	3620.07
16	6.72673	26298.2	8.55787	Static Fill-Static	275	28	1433.36	2239.16	3694.06	0	3694.06	3909.75	3909.75
17	6.72673	28145.2	9.52356	Static Fill-Static	275	28	1513.81	2364.84	3930.41	0	3930.41	4184.38	4184.38
18	6.72673	29890.1	10.492	Static Fill-Static	275	28	1588.43	2481.41	4149.66	0	4149.66	4443.83	4443.83
19	6.72673	31532	11.4634	Static Fill-Static	275	28	1657.26	2588.94	4351.88	0	4351.88	4687.96	4687.96
20	6.72673	33069.9	12.4383	Static Fill-Static	275	28	1720.33	2687.47	4537.19	0	4537.19	4916.63	4916.63
21	6.72673	34502.7	13.4168	Static Fill-Static	275	28	1777.66	2777.03	4705.63	0	4705.63	5129.68	5129.68
22	6.72673	35815	14.3992	Static Fill-Static	275	28	1828.62	2856.63	4855.35	0	4855.35	5324.84	5324.84
23	6.72673	35377.3	15.3861	Static Fill-Static	275	28	1797.88	2808.61	4765.04	0	4765.04	5259.79	5259.79
24	6.72673	33703.5	16.3776	Static Fill-Static	275	28	1710.49	2672.09	4508.28	0	4508.28	5010.98	5010.98
25	6.72673	31942.6	17.3742	Static Fill-Static	275	28	1619.97	2530.68	4242.32	0	4242.32	4749.19	4749.19
26	6.72673	31865.5	18.3763	Static Fill-Static	275	28	1606.91	2510.28	4203.96	0	4203.96	4737.77	4737.77
27	6.72673	32794.8	19.3842	Static Fill-Static	275	28	1639.32	2560.91	4299.17	0	4299.17	4875.96	4875.96
28	6.72673	33607.4	20.3984	Static Fill-Static	275	28	1665.92	2602.46	4377.32	0	4377.32	4996.81	4996.81
29	6.72673	34301.1	21.4194	Static Fill-Static	275	28	1686.68	2634.9	4438.33	0	4438.33	5100	5100
30	6.72673	34873.4	22.4475	Static Fill-Static	275	28	1701.58	2658.18	4482.11	0	4482.11	5185.1	5185.1
31	6.72673	35321.6	23.4833	Static Fill-Static	275	28	1710.6	2672.26	4508.59	0	4508.59	5251.79	5251.79
32	6.72673	35642.9	24.5273	Static Fill-Static	275	28	1713.68	2677.07	4517.64	0	4517.64	5299.59	5299.59
33	6.72673	35834.1	25.5801	Static Fill-Static	275	28	1710.78	2672.54	4509.13	0	4509.13	5328.07	5328.07
34	6.72673	35836.6	26.6422	Static Fill-Static	275	28	1699.46	2654.87	4475.88	0	4475.88	5328.47	5328.47
35	6.72673	35514.1	27.7143	Static Fill-Static	275	28	1674.03	2615.14	4401.16	0	4401.16	5280.57	5280.57
36	6.72673	35034.9	28.797	Static Fill-Static	275	28	1641.92	2564.98	4306.82	0	4306.82	5209.37	5209.37
37	6.72673	34410.2	29.8911	Static Fill-Static	275	28	1603.74	2505.33	4194.64	0	4194.64	5116.5	5116.5
38	6.72673	33635.1	30.9974	Static Fill-Static	275	28	1559.41	2436.08	4064.41	0	4064.41	5001.3	5001.3
39	6.72673	32704.5	32.1166	Static Fill-Static	275	28	1508.85	2357.1	3915.85	0	3915.85	4862.96	4862.96
40	6.72673	31612.6	33.2498	Static Fill-Static	275	28	1451.96	2268.22	3748.71	0	3748.71	4700.64	4700.64
41	6.72673	30353.2	34.3978	Static Fill-Static	275	28	1388.64	2169.31	3562.67	0	3562.67	4513.42	4513.42
42	6.72673	29010.4	35.5619	Static Fill-Static	275	28	1319.70	2060.19	3357.43	0	3357.43	4300.26	4300.26

43	6.72673	27303.6	36.743	Static Fill- Static	275	28	1242.27	1940.65	3132.63	0	3132.63	4060.04	4060.04
44	6.72673	25497.3	37.9427	Fill- Static	275	28	1158.96	1810.51	2887.87	0	2887.87	3791.49	3791.49
45	6.72673	23491.2	39.1623	Fill- Static	275	28	1068.72	1669.54	2622.75	0	2622.75	3493.21	3493.21
46	6.72673	21274.8	40.4035	Fill- Static	275	28	971.412	1517.52	2336.84	0	2336.84	3163.68	3163.68
47	6.72673	18815.4	41.668	Fill- Static	275	28	866.04	1352.91	2027.26	0	2027.26	2798	2798
48	6.72673	14485.8	42.9578	Fill- Static	275	28	690.433	1078.58	1511.31	0	1511.31	2154.2	2154.2
49	6.72673	8879.41	44.2753	Fill- Static	275	28	469.646	733.671	862.635	0	862.635	1320.55	1320.55
50	6.72673	3005.85	45.6231	Fill- Static	275	28	243.519	380.42	198.266	0	198.266	447.14	447.14

Interslice Data

Global Minimum Query (bishop simplified) - Safety Factor: 1.61814



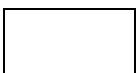
Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	305.025	845.765	0	0	0
2	311.075	846.188	1318.09	0	0
3	317.125	846.681	3183.51	0	0
4	323.176	847.244	5437.62	0	0
5	329.226	847.877	8010.11	0	0
6	335.276	848.579	10847.6	0	0
7	341.326	849.353	13898.7	0	0
8	347.376	850.197	17114.2	0	0
9	353.427	851.113	20447.1	0	0
10	359.477	852.1	23852	0	0
11	365.527	853.159	27285.9	0	0
12	371.577	854.29	30707.5	0	0
13	377.628	855.494	34077.6	0	0
14	383.678	856.772	37358.8	0	0
15	389.728	858.123	40515.7	0	0
16	395.778	859.549	43514.7	0	0
17	401.828	861.05	46324.2	0	0
18	407.879	862.627	48913	0	0
19	413.929	864.28	51182.6	0	0
20	419.979	866.011	53109.3	0	0
21	426.029	867.819	54722.5	0	0
22	432.08	869.706	56066.6	0	0
23	438.13	871.673	57160	0	0
24	444.18	873.721	57982.5	0	0
25	450.23	875.85	58515.2	0	0
26	456.28	878.062	58741.4	0	0
27	462.331	880.358	58646.7	0	0
28	468.381	882.739	58218.7	0	0
29	474.431	885.206	57447.1	0	0
30	480.481	887.761	56324.2	0	0
31	486.532	890.404	54846.3	0	0
32	492.582	893.138	53027.4	0	0
33	498.632	895.964	50875.9	0	0
34	504.682	898.883	48399.2	0	0
35	510.732	901.897	45607.7	0	0
36	516.783	905.009	42514.8	0	0
37	522.833	908.22	39136.8	0	0
38	528.883	911.532	35493.5	0	0
39	534.933	914.947	31607.9	0	0
40	540.984	918.468	27506.9	0	0
41	547.034	922.096	23220.9	0	0
42	553.084	925.836	18784.8	0	0
43	559.134	929.689	14237.5	0	0
44	565.184	933.658	9622.63	0	0
45	571.235	937.747	4988.91	0	0
46	577.285	941.959	436.6	0	0
47	583.335	946.298	-3468.06	0	0
48	589.385	950.767	-6465.9	0	0
49	595.436	955.37	-8414.81	0	0
50	601.486	960.113	-9163.17	0	0
51	607.536	965	0	0	0

Global Minimum Query (janbu simplified) - Safety Factor: 1.56218

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	259.686	842.461	0	0	0
2	266.412	841.779	1422.78	0	0
3	273.139	841.211	3158.75	0	0
4	279.866	840.755	5137.58	0	0
5	286.593	840.412	7295.47	0	0
6	293.319	840.181	9574.91	0	0
7	300.046	840.063	11924.4	0	0
8	306.773	840.056	14574.4	0	0
9	313.5	840.161	18187.9	0	0
10	320.226	840.379	22574.4	0	0
11	326.953	840.708	27493.1	0	0
12	333.68	841.15	32828.3	0	0
13	340.406	841.705	38470.5	0	0
14	347.133	842.374	44316.3	0	0
15	353.86	843.156	50267.9	0	0
16	360.587	844.053	56232.9	0	0
17	367.313	845.065	62124.2	0	0
18	374.04	846.194	67859.9	0	0
19	380.767	847.44	73363.1	0	0
20	387.494	848.804	78561.7	0	0
21	394.22	850.287	83388.9	0	0
22	400.947	851.892	87782.2	0	0
23	407.674	853.619	91683.3	0	0
24	414.4	855.47	94942.6	0	0
25	421.127	857.447	97522.8	0	0
26	427.854	859.552	99478.5	0	0
27	434.581	861.786	100881	0	0
28	441.307	864.153	101720	0	0
29	448.034	866.655	101964	0	0
30	454.761	869.293	101585	0	0
31	461.488	872.072	100562	0	0
32	468.214	874.995	98878.6	0	0
33	474.941	878.064	96526.2	0	0
34	481.668	881.284	93501.4	0	0
35	488.394	884.659	89815.3	0	0
36	495.121	888.193	85510.4	0	0
37	501.848	891.89	80617.6	0	0
38	508.575	895.757	75173.9	0	0
39	515.301	899.798	69225.6	0	0
40	522.028	904.021	62829.2	0	0
41	528.755	908.431	56052.4	0	0
42	535.482	913.037	48974.7	0	0
43	542.208	917.846	41689.3	0	0
44	548.935	922.867	34304.6	0	0
45	555.662	928.112	26945.7	0	0
46	562.389	933.591	19756.8	0	0
47	569.115	939.317	12903.8	0	0
48	575.842	945.303	6586.46	0	0
49	582.569	951.567	1759.3	0	0
50	589.295	958.125	-742.923	0	0
51	596.022	965	0	0	0

List Of Coordinates

External Boundary



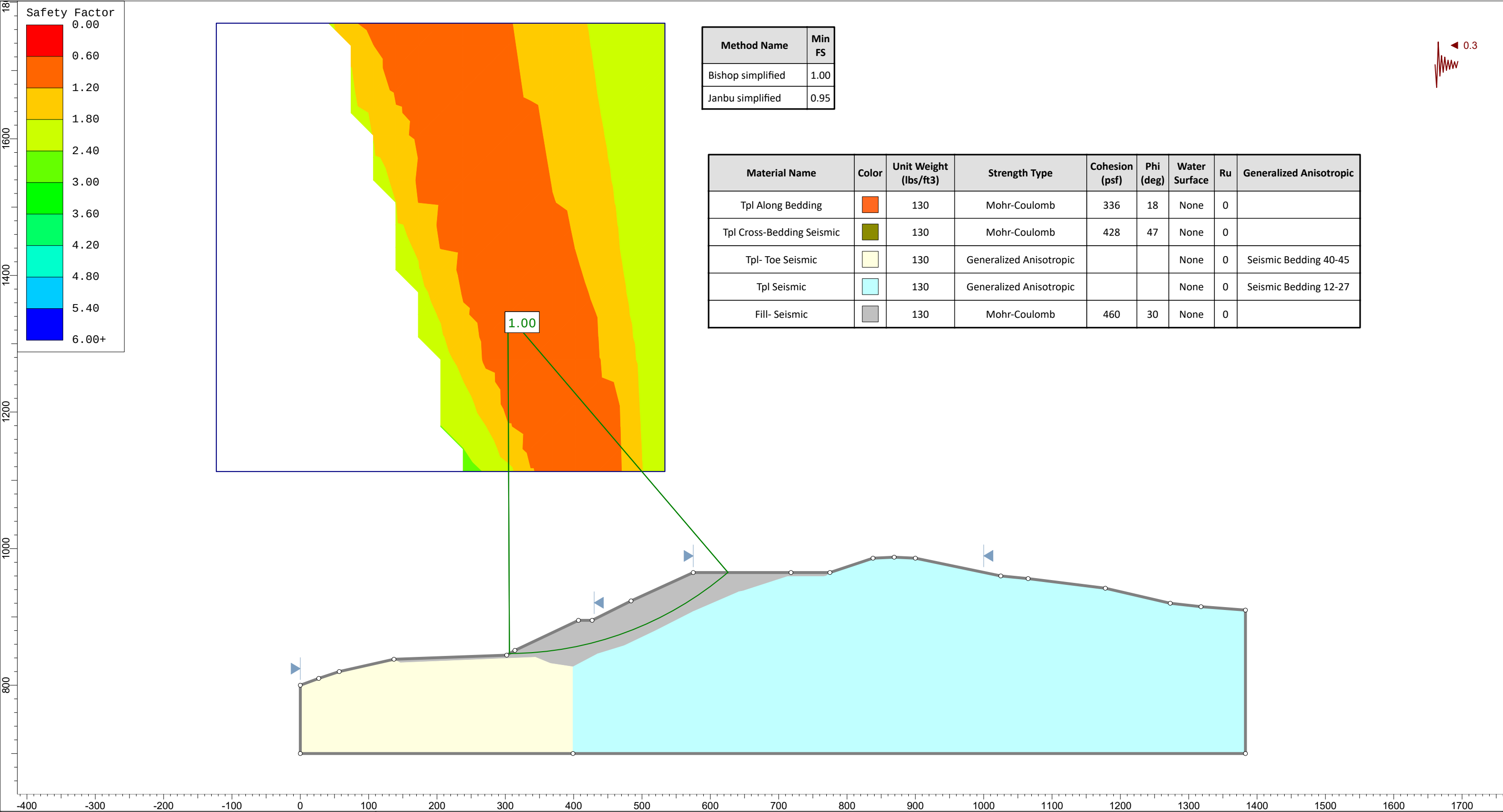
X	Y
0	700
399	700
1383	700
1383	910
1318	915
1273	920
1178	942
1065	956.2
1025	960
900	986
869	987.5
838	986
775	965
718	965
575	965
484	923.5
427	895
407	895
314	851
302	844
137	838
57	820
27	810
0	800

Material Boundary

X	Y
137	838
148	833
344	841
366	832
399	827
435	846
474	858
520	880
575	908
642	937
713	960
770	960
775	965

Material Boundary

X	Y
399	700
399	827



Method Name	Min FS
Bishop simplified	1.00
Janbu simplified	0.95

Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (deg)	Water Surface	Ru	Generalized Anisotropic
Tpl Along Bedding		130	Mohr-Coulomb	336	18	None	0	
Tpl Cross-Bedding Seismic		130	Mohr-Coulomb	428	47	None	0	
Tpl- Toe Seismic		130	Generalized Anisotropic			None	0	Seismic Bedding 40-45
Tpl Seismic		130	Generalized Anisotropic			None	0	Seismic Bedding 12-27
Fill- Seismic		130	Mohr-Coulomb	460	30	None	0	

Slide Analysis Information

HOFF Property, Yorba Linda, CA

Project Summary

File Name: Cross-section A-A' Seismic
 Slide Modeler Version: 7.038
 Project Title: HOFF Property, Yorba Linda, CA
 Analysis: Cross-section A-A', Seismic
 Company: TGR Geotechnical, Inc.
 Date Created: 4/15/20

General Settings

Units of Measurement: Imperial Units
 Time Units: days
 Permeability Units: feet/second
 Failure Direction: Right to Left
 Data Output: Standard
 Maximum Material Properties: 20
 Maximum Support Properties: 20

Analysis Options

Slices Type: Vertical

Analysis Methods Used

Bishop simplified
 Janbu simplified

Number of slices: 50
 Tolerance: 0.005
 Maximum number of iterations: 75
 Check $\alpha < 0.2$: Yes
 Create Interslice boundaries at intersections with water tables and piezos: Yes
 Initial trial value of FS: 1
 Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
 Pore Fluid Unit Weight [lbs/ft³]: 62.4
 Use negative pore pressure cutoff: Yes
 Maximum negative pore pressure [psf]: 0
 Advanced Groundwater Method: None

Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3

Surface Options

Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined
Minimum Area: Not Defined
Minimum Weight: Not Defined

Seismic

Advanced seismic analysis: No
Staged pseudostatic analysis: No

Loading

Seismic Load Coefficient (Horizontal): 0.3

Material Properties

Property	Tpl- Toe Seismic	Tpl Seismic	Fill- Seismic
Color			
Strength Type	Generalized Anisotropic	Generalized Anisotropic	Mohr-Coulomb
Unit Weight [lbs/ft3]	130	130	130
Cohesion [psf]			460
Friction Angle [deg]			30
Water Surface	None	None	None
Ru Value	0	0	0

Generalized Anisotropic Functions

Name: Seismic Bedding 40-45

Angle From	Angle To	Material
40	-90	Tpl Cross-Bedding Seismic
45	40	Tpl Along Bedding
90	45	Tpl Cross-Bedding Seismic

Name: Seismic Bedding 12-27

Angle From	Angle To	Material
-27	-90	Tpl Cross-Bedding Seismic
-12	-27	Tpl Along Bedding
90	-12	Tpl Cross-Bedding Seismic

Global Minimums

Method: bishop simplified

FS	1.000960
Center:	303.811, 1342.660
Radius:	496.336
Left Slip Surface Endpoint:	305.991, 846.328
Right Slip Surface Endpoint:	625.872, 965.000
Resisting Moment:	3.95434e+008 lb-ft
Driving Moment:	3.95056e+008 lb-ft
Total Slice Area:	9944.48 ft ²
Surface Horizontal Width:	319.88 ft
Surface Average Height:	31.0882 ft

Method: janbu simplified

FS	0.954049
Center:	369.455, 1145.730
Radius:	296.930
Left Slip Surface Endpoint:	318.617, 853.184
Right Slip Surface Endpoint:	605.048, 965.000
Resisting Horizontal Force:	751235 lb
Driving Horizontal Force:	787418 lb
Total Slice Area:	10438.4 ft ²
Surface Horizontal Width:	286.43 ft
Surface Average Height:	36.4432 ft

Valid / Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 2783
 Number of Invalid Surfaces: 2068

Error Codes:

Error Code -101 reported for 46 surfaces
 Error Code -103 reported for 141 surfaces
 Error Code -1000 reported for 1881 surfaces

Method: janbu simplified

Number of Valid Surfaces: 2783
 Number of Invalid Surfaces: 2068

Error Codes:

Error Code -101 reported for 46 surfaces
 Error Code -103 reported for 141 surfaces
 Error Code -1000 reported for 1881 surfaces

Error Codes

The following errors were encountered during the computation:

- 101 = Only one (or zero) surface / slope intersections.
- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

Slice Data

Global Minimum Query (bishop simplified) - Safety Factor: 1.00096

Slice Number	Width [ft]	Weight [lbs]	Angle of Slice Base [degrees]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]	Base Vertical Stress [psf]	Effective Vertical Stress [psf]
1	6.3976	1523.07	0.620949	Fill-Seismic	460	30	593.165	593.734	231.634	0	231.634	238.063	238.063
2	6.3976	4370.77	1.3596	Fill-Seismic	460	30	842.081	842.889	663.183	0	663.183	683.169	683.169
3	6.3976	6746.11	2.09848	Fill-Seismic	460	30	1045.66	1046.66	1016.12	0	1016.12	1054.44	1054.44
4	6.3976	9034.13	2.83771	Fill-Seismic	460	30	1238.61	1239.8	1350.66	0	1350.66	1412.05	1412.05
5	6.3976	11253.3	3.57741	Fill-Seismic	460	30	1422.78	1424.15	1669.96	0	1669.96	1758.91	1758.91
6	6.3976	13403.5	4.31771	Fill-Seismic	460	30	1598.33	1599.86	1974.29	0	1974.29	2094.97	2094.97
7	6.3976	15484.5	5.05874	Fill-Seismic	460	30	1765.39	1767.08	2263.94	0	2263.94	2420.21	2420.21
8	6.3976	17496.1	5.80061	Fill-Seismic	460	30	1924.12	1925.97	2539.14	0	2539.14	2734.6	2734.6
9	6.3976	19438.1	6.54346	Fill-Seismic	460	30	2074.67	2076.66	2800.13	0	2800.13	3038.1	3038.1
10	6.3976	21310.1	7.28741	Fill-Seismic	460	30	2217.14	2219.27	3047.15	0	3047.15	3330.68	3330.68
11	6.3976	23111.8	8.03261	Fill-Seismic	460	30	2351.67	2353.93	3280.38	0	3280.38	3612.25	3612.25
12	6.3976	24842.9	8.77917	Fill-Seismic	460	30	2478.37	2480.75	3500.04	0	3500.04	3882.79	3882.79
13	6.3976	26502.9	9.52724	Fill-Seismic	460	30	2597.34	2599.83	3706.29	0	3706.29	4142.2	4142.2
14	6.3976	28091.4	10.277	Fill-Seismic	460	30	2708.68	2711.28	3899.33	0	3899.33	4390.45	4390.45
15	6.3976	29607.9	11.0285	Fill-Seismic	460	30	2812.48	2815.18	4079.3	0	4079.3	4627.44	4627.44
16	6.3976	30995.5	11.7819	Fill-Seismic	460	30	2904.31	2907.1	4238.5	0	4238.5	4844.28	4844.28
17	6.3976	30631.6	12.5374	Fill-Seismic	460	30	2854.73	2857.47	4152.53	0	4152.53	4787.36	4787.36
18	6.3976	29411.3	13.2951	Fill-Seismic	460	30	2737.73	2740.36	3949.7	0	3949.7	4596.62	4596.62
19	6.3976	28126.3	14.0552	Fill-Seismic	460	30	2617.1	2619.61	3740.57	0	3740.57	4395.76	4395.76
20	6.3976	28304	14.8178	Fill-Seismic	460	30	2612.39	2614.9	3732.39	0	3732.39	4423.49	4423.49
21	6.3976	29518.7	15.5831	Fill-Seismic	460	30	2688.08	2690.66	3863.62	0	3863.62	4613.3	4613.3
22	6.3976	30656.6	16.3513	Fill-Seismic	460	30	2756.56	2759.21	3982.35	0	3982.35	4791.1	4791.1
23	6.3976	31716.8	17.1225	Fill-Seismic	460	30	2817.88	2820.59	4088.66	0	4088.66	4956.77	4956.77
24	6.3976	32698.5	17.897	Fill-Seismic	460	30	2872.1	2874.86	4182.66	0	4182.66	5110.16	5110.16
25	6.3976	33600.6	18.6748	Fill-Seismic	460	30	2919.27	2922.07	4264.43	0	4264.43	5251.11	5251.11
26	6.3976	34422	19.4562	Fill-Seismic	460	30	2959.4	2962.24	4334.01	0	4334.01	5379.44	5379.44
27	6.3976	35161.6	20.2414	Fill-Seismic	460	30	2992.56	2995.43	4391.49	0	4391.49	5495	5495
28	6.3976	35814.5	21.0306	Fill-Seismic	460	30	3018.49	3021.39	4436.45	0	4436.45	5596.99	5596.99
29	6.3976	36232.2	21.824	Fill-Seismic	460	30	3026.46	3029.37	4450.27	0	4450.27	5662.24	5662.24
30	6.3976	36484.7	22.6218	Fill-Seismic	460	30	3021.93	3024.83	4442.41	0	4442.41	5701.66	5701.66
31	6.3976	36650.1	23.4243	Fill-	460	30	3010.77	3013.66	4423.07	0	4423.07	5727.46	5727.46

				Seismic									
32	6.3976	36726.6	24.2316	Fill- Seismic	460	30	2993.02	2995.89	4392.28	0	4392.28	5739.39	5739.39
33	6.3976	36712.7	25.0442	Fill- Seismic	460	30	2968.66	2971.51	4350.07	0	4350.07	5737.17	5737.17
34	6.3976	36606.5	25.8621	Fill- Seismic	460	30	2937.73	2940.55	4296.45	0	4296.45	5720.53	5720.53
35	6.3976	36406.1	26.6858	Fill- Seismic	460	30	2900.23	2903.01	4231.43	0	4231.43	5689.19	5689.19
36	6.3976	36109.6	27.5154	Fill- Seismic	460	30	2856.16	2858.9	4155.01	0	4155.01	5642.81	5642.81
37	6.3976	35714.7	28.3514	Fill- Seismic	460	30	2805.51	2808.2	4067.21	0	4067.21	5581.07	5581.07
38	6.3976	35219.2	29.194	Fill- Seismic	460	30	2748.29	2750.93	3968	0	3968	5503.59	5503.59
39	6.3976	34620.6	30.0435	Fill- Seismic	460	30	2684.48	2687.06	3857.39	0	3857.39	5410	5410
40	6.3976	33916.2	30.9005	Fill- Seismic	460	30	2614.1	2616.61	3735.36	0	3735.36	5299.89	5299.89
41	6.3976	33103.2	31.7651	Fill- Seismic	460	30	2537.1	2539.54	3601.87	0	3601.87	5172.8	5172.8
42	6.3976	32178.5	32.6379	Fill- Seismic	460	30	2453.48	2455.84	3456.91	0	3456.91	5028.27	5028.27
43	6.3976	30040.2	33.5194	Fill- Seismic	460	30	2291.58	2293.78	3176.2	0	3176.2	4694.08	4694.08
44	6.3976	26458.5	34.4099	Fill- Seismic	460	30	2038.75	2040.71	2737.87	0	2737.87	4134.35	4134.35
45	6.3976	22751.9	35.31	Fill- Seismic	460	30	1782.07	1783.78	2292.85	0	2292.85	3555.09	3555.09
46	6.3976	18918.9	36.2202	Fill- Seismic	460	30	1521.75	1523.21	1841.55	0	1841.55	2956.12	2956.12
47	6.3976	14955.3	37.1412	Fill- Seismic	460	30	1257.85	1259.05	1384.01	0	1384.01	2336.73	2336.73
48	6.3976	10856.2	38.0735	Fill- Seismic	460	30	990.406	991.357	920.337	0	920.337	1696.17	1696.17
49	6.3976	6616.46	39.0178	Fill- Seismic	460	30	719.49	720.181	450.647	0	450.647	1033.65	1033.65
50	6.3976	2230.37	39.975	Fill- Seismic	460	30	445.171	445.598	-24.9445	0	-24.9445	348.267	348.267

Global Minimum Query (janbu simplified) - Safety Factor: 0.954049

Slice Number	Width [ft]	Weight [lbs]	Angle of Slice Base [degrees]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]	Base Vertical Stress [psf]	Effective Vertical Stress [psf]
1	5.72861	1358.45	-9.29815	Fill- Seismic	460	30	694.457	662.546	350.819	0	350.819	237.12	237.12
2	5.72861	4032.72	-8.17969	Fill- Seismic	460	30	994.675	948.969	846.92	0	846.92	703.945	703.945
3	5.72861	6622.1	-7.06437	Fill- Seismic	460	30	1277.49	1218.79	1314.26	0	1314.26	1155.95	1155.95
4	5.72861	9127.24	-5.95173	Fill- Seismic	460	30	1543.72	1472.78	1754.18	0	1754.18	1593.25	1593.25
5	5.72861	11548.7	-4.84135	Fill- Seismic	460	30	1794.08	1711.64	2167.91	0	2167.91	2015.96	2015.96
6	5.72861	13887	-3.73278	Fill- Seismic	460	30	2029.26	1936.01	2556.51	0	2556.51	2424.12	2424.12
7	5.72861	16142.4	-2.62561	Fill- Seismic	460	30	2249.83	2146.45	2921.01	0	2921.01	2817.84	2817.84
8	5.72861	18315.2	-1.51942	Fill- Seismic	460	30	2456.36	2343.49	3262.3	0	3262.3	3197.14	3197.14
9	5.72861	20405.6	-0.413799	Fill- Seismic	460	30	2649.34	2527.6	3581.18	0	3581.18	3562.04	3562.04
10	5.72861	22413.7	0.691669	Fill- Seismic	460	30	2829.23	2699.22	3878.44	0	3878.44	3912.6	3912.6

11	5.72861	24339.4	1.79739	Fill- Seismic	460	30	2996.42	2858.73	4154.72	0	4154.72	4248.75	4248.75
12	5.72861	26182.7	2.90379	Fill- Seismic	460	30	3151.32	3006.51	4410.69	0	4410.69	4570.54	4570.54
13	5.72861	27943.3	4.01127	Fill- Seismic	460	30	3294.24	3142.87	4646.87	0	4646.87	4877.88	4877.88
14	5.72861	29621	5.12026	Fill- Seismic	460	30	3425.53	3268.12	4863.81	0	4863.81	5170.75	5170.75
15	5.72861	31215.4	6.23117	Fill- Seismic	460	30	3545.45	3382.53	5061.97	0	5061.97	5449.08	5449.08
16	5.72861	32396.2	7.34444	Fill- Seismic	460	30	3621.94	3455.51	5188.37	0	5188.37	5655.2	5655.2
17	5.72861	31989.1	8.46051	Fill- Seismic	460	30	3542.56	3379.78	5057.21	0	5057.21	5584.15	5584.15
18	5.72861	31311.8	9.57982	Fill- Seismic	460	30	3438.69	3280.68	4885.56	0	4885.56	5465.93	5465.93
19	5.72861	30555.5	10.7028	Fill- Seismic	460	30	3329.23	3176.25	4704.69	0	4704.69	5333.93	5333.93
20	5.72861	30936.8	11.83	Fill- Seismic	460	30	3328.4	3175.46	4703.32	0	4703.32	5400.48	5400.48
21	5.72861	32132.1	12.9619	Fill- Seismic	460	30	3402.63	3246.28	4825.98	0	4825.98	5609.15	5609.15
22	5.72861	33238.5	14.0989	Fill- Seismic	460	30	3466.56	3307.27	4931.62	0	4931.62	5802.29	5802.29
23	5.72861	34254.6	15.2416	Fill- Seismic	460	30	3520.33	3358.57	5020.48	0	5020.48	5979.69	5979.69
24	5.72861	35179.1	16.3906	Fill- Seismic	460	30	3564.06	3400.29	5092.74	0	5092.74	6141.07	6141.07
25	5.72861	36010.3	17.5464	Fill- Seismic	460	30	3597.86	3432.53	5148.57	0	5148.57	6286.18	6286.18
26	5.72861	36746.5	18.7096	Fill- Seismic	460	30	3621.78	3455.36	5188.12	0	5188.12	6414.71	6414.71
27	5.72861	37385.8	19.8809	Fill- Seismic	460	30	3635.93	3468.86	5211.5	0	5211.5	6526.32	6526.32
28	5.72861	37926.1	21.0609	Fill- Seismic	460	30	3640.35	3473.07	5218.79	0	5218.79	6620.63	6620.63
29	5.72861	38363.5	22.2504	Fill- Seismic	460	30	3634.95	3467.92	5209.88	0	5209.88	6697	6697
30	5.72861	38582	23.45	Fill- Seismic	460	30	3610.28	3444.38	5169.11	0	5169.11	6735.16	6735.16
31	5.72861	38623	24.6607	Fill- Seismic	460	30	3570.34	3406.28	5103.1	0	5103.1	6742.3	6742.3
32	5.72861	38554.2	25.8832	Fill- Seismic	460	30	3521.15	3359.35	5021.81	0	5021.81	6730.31	6730.31
33	5.72861	38372.3	27.1185	Fill- Seismic	460	30	3462.68	3303.57	4925.22	0	4925.22	6698.57	6698.57
34	5.72861	38073.7	28.3676	Fill- Seismic	460	30	3394.95	3238.95	4813.29	0	4813.29	6646.45	6646.45
35	5.72861	37654.1	29.6316	Fill- Seismic	460	30	3317.9	3165.44	4685.96	0	4685.96	6573.21	6573.21
36	5.72861	37109.1	30.9117	Fill- Seismic	460	30	3231.5	3083.01	4543.19	0	4543.19	6478.09	6478.09
37	5.72861	36433.7	32.2091	Fill- Seismic	460	30	3135.68	2991.59	4384.85	0	4384.85	6360.19	6360.19
38	5.72861	35622.3	33.5254	Fill- Seismic	460	30	3030.39	2891.14	4210.85	0	4210.85	6218.55	6218.55
39	5.72861	34668.7	34.862	Fill- Seismic	460	30	2915.53	2781.56	4021.06	0	4021.06	6052.09	6052.09
40	5.72861	33565.9	36.2207	Fill- Seismic	460	30	2791.02	2662.77	3815.32	0	3815.32	5859.59	5859.59
41	5.72861	32306.2	37.6035	Fill- Seismic	460	30	2656.76	2534.68	3593.45	0	3593.45	5639.69	5639.69
42	5.72861	30880.7	39.0124	Fill- Seismic	460	30	2512.62	2397.16	3355.27	0	3355.27	5390.85	5390.85
43	5.72861	29279.6	40.4501	Fill-	460	30	2358.48	2250.11	3100.56	0	3100.56	5111.35	5111.35

				Seismic									
44	5.72861	27491.3	41.9192	Fill- Seismic	460	30	2194.21	2093.38	2829.1	0	2829.1	4799.18	4799.18
45	5.72861	25444.4	43.423	Fill- Seismic	460	30	2015.71	1923.09	2534.15	0	2534.15	4441.86	4441.86
46	5.72861	21849.3	44.9652	Fill- Seismic	460	30	1739.19	1659.27	2077.19	0	2077.19	3814.27	3814.27
47	5.72861	17467	46.5501	Fill- Seismic	460	30	1420.19	1354.93	1550.07	0	1550.07	3049.27	3049.27
48	5.72861	12831	48.1828	Fill- Seismic	460	30	1096.19	1045.82	1014.67	0	1014.67	2239.95	2239.95
49	5.72861	7916.28	49.8693	Fill- Seismic	460	30	767.509	732.241	471.536	0	471.536	1381.99	1381.99
50	5.72861	2692.95	51.617	Fill- Seismic	460	30	434.625	414.653	-78.5428	0	-78.5428	470.151	470.151

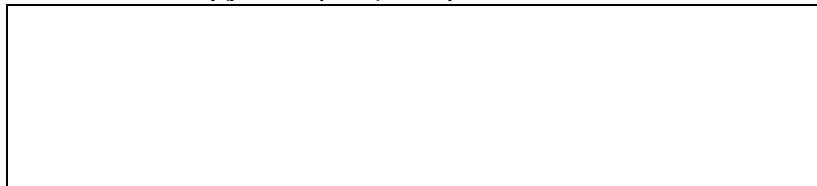
Interslice Data

Global Minimum Query (bishop simplified) - Safety Factor: 1.00096



Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	305.991	846.328	0	0	0
2	312.389	846.398	3325.49	0	0
3	318.787	846.549	7306.03	0	0
4	325.184	846.784	11740.1	0	0
5	331.582	847.101	16533.3	0	0
6	337.979	847.501	21600.5	0	0
7	344.377	847.984	26861.1	0	0
8	350.775	848.55	32238.7	0	0
9	357.172	849.2	37661.3	0	0
10	363.57	849.934	43060.7	0	0
11	369.967	850.752	48372.7	0	0
12	376.365	851.655	53537	0	0
13	382.763	852.643	58496.9	0	0
14	389.16	853.717	63199.2	0	0
15	395.558	854.877	67594.3	0	0
16	401.955	856.124	71636	0	0
17	408.353	857.458	75279.9	0	0
18	414.751	858.881	78463.6	0	0
19	421.148	860.392	81201	0	0
20	427.546	861.994	83531.3	0	0
21	433.943	863.687	85452.3	0	0
22	440.341	865.471	86917	0	0
23	446.739	867.348	87897.4	0	0
24	453.136	869.319	88369	0	0
25	459.534	871.385	88310.3	0	0
26	465.931	873.547	87703.3	0	0
27	472.329	875.807	86533	0	0
28	478.727	878.166	84788.1	0	0
29	485.124	880.626	82460.9	0	0
30	491.522	883.188	79570.5	0	0
31	497.92	885.854	76133.6	0	0
32	504.317	888.625	72159.3	0	0
33	510.715	891.505	67660.5	0	0
34	517.112	894.494	62653.8	0	0
35	523.51	897.595	57159.9	0	0
36	529.908	900.811	51203.5	0	0
37	536.305	904.144	44813.9	0	0
38	542.703	907.596	38024.6	0	0
39	549.1	911.17	30874.1	0	0
40	555.498	914.871	23405.8	0	0
41	561.896	918.7	15668.5	0	0
42	568.293	922.661	7716.45	0	0
43	574.691	926.758	-389.943	0	0
44	581.088	930.996	-8186.75	0	0
45	587.486	935.378	-15066.4	0	0
46	593.884	939.909	-20870	0	0
47	600.281	944.595	-25429.8	0	0
48	606.679	949.441	-28568	0	0
49	613.076	954.453	-30094.9	0	0
50	619.474	959.637	-29808.5	0	0
51	625.872	965	0	0	0

Global Minimum Query (janbu simplified) - Safety Factor: 0.954049



Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	318.617	853.184	0	0	0
2	324.346	852.247	3899.31	0	0
3	330.074	851.423	9084.32	0	0
4	335.803	850.713	15348.1	0	0
5	341.532	850.116	22499.9	0	0
6	347.26	849.631	30363.6	0	0
7	352.989	849.257	38776.4	0	0
8	358.717	848.994	47588	0	0
9	364.446	848.842	56659	0	0
10	370.175	848.801	65860.7	0	0
11	375.903	848.87	75074	0	0
12	381.632	849.05	84188.6	0	0
13	387.36	849.34	93102.7	0	0
14	393.089	849.742	101722	0	0
15	398.818	850.256	109961	0	0
16	404.546	850.881	117738	0	0
17	410.275	851.619	124934	0	0
18	416.003	852.471	131320	0	0
19	421.732	853.438	136899	0	0
20	427.461	854.521	141709	0	0
21	433.189	855.721	145849	0	0
22	438.918	857.039	149336	0	0
23	444.647	858.478	152125	0	0
24	450.375	860.039	154177	0	0
25	456.104	861.724	155456	0	0
26	461.832	863.536	155936	0	0
27	467.561	865.476	155592	0	0
28	473.29	867.547	154406	0	0
29	479.018	869.753	152368	0	0
30	484.747	872.097	149469	0	0
31	490.475	874.582	145729	0	0
32	496.204	877.212	141171	0	0
33	501.933	879.991	135815	0	0
34	507.661	882.925	129688	0	0
35	513.39	886.019	122823	0	0
36	519.118	889.277	115263	0	0
37	524.847	892.707	107056	0	0
38	530.576	896.316	98263.2	0	0
39	536.304	900.111	88952.8	0	0
40	542.033	904.102	79205.4	0	0
41	547.761	908.298	69113.8	0	0
42	553.49	912.71	58784.8	0	0
43	559.219	917.351	48340.9	0	0
44	564.947	922.235	37923	0	0
45	570.676	927.379	27692.5	0	0
46	576.405	932.8	17865.8	0	0
47	582.133	938.522	9388	0	0
48	587.862	944.569	2908.97	0	0
49	593.59	950.972	-1158.55	0	0
50	599.319	957.768	-2341.53	0	0
51	605.048	965	0	0	0

List Of Coordinates

External Boundary



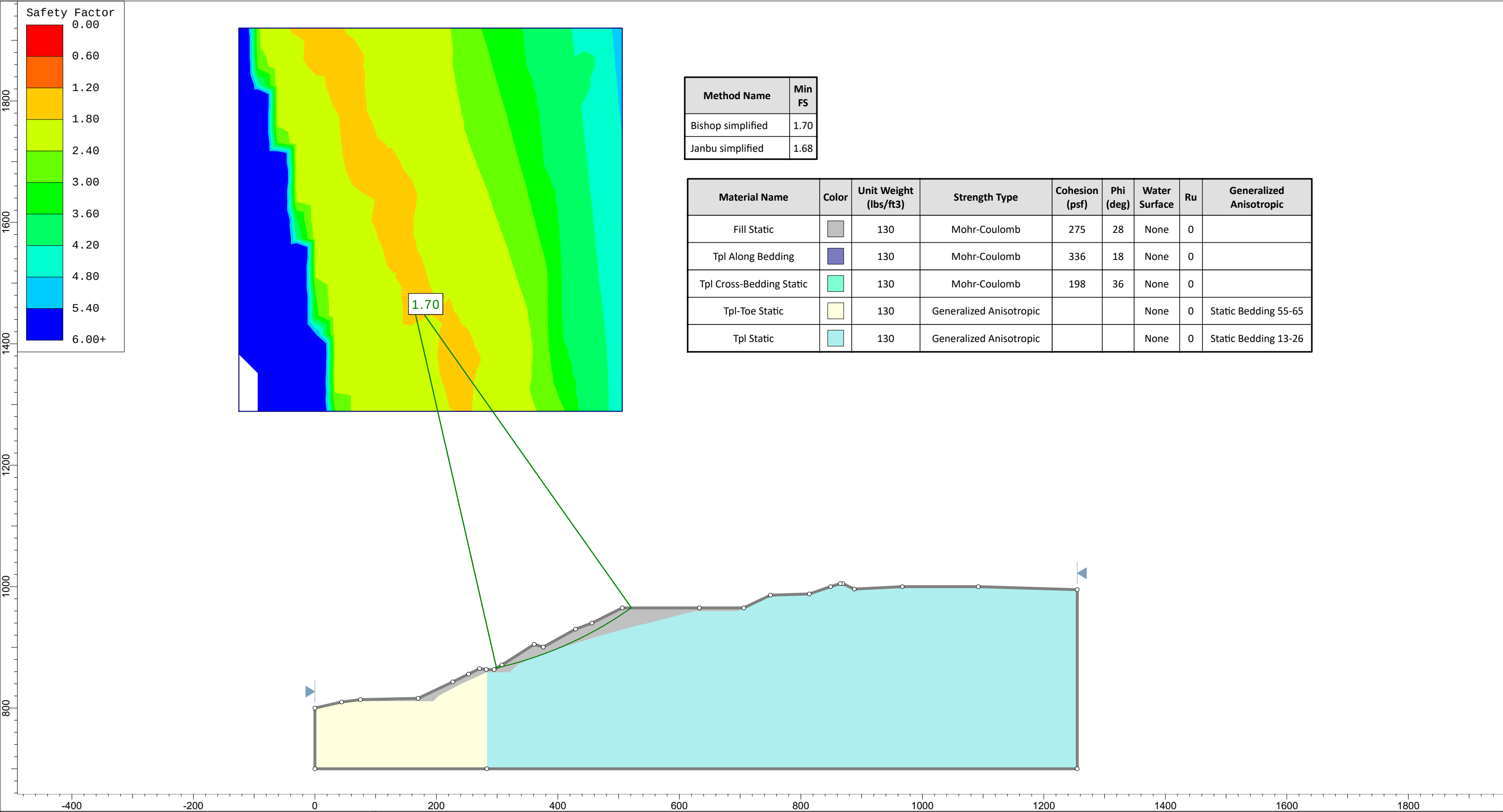
X	Y
0	700
399	700
1383	700
1383	910
1318	915
1273	920
1178	942
1065	956.2
1025	960
900	986
869	987.5
838	986
775	965
718	965
575	965
484	923.5
427	895
407	895
314	851
302	844
137	838
57	820
27	810
0	800

Material Boundary

X	Y
137	838
148	833
344	841
366	832
399	827
435	846
474	858
520	880
575	908
642	937
713	960
770	960
775	965

Material Boundary

X	Y
399	700
399	827



Method Name	Min FS
Bishop simplified	1.70
Janbu simplified	1.68

Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (deg)	Water Surface	Ru	Generalized Anisotropic
Fill Static		130	Mohr-Coulomb	275	28	None	0	
Tpl Along Bedding		130	Mohr-Coulomb	336	18	None	0	
Tpl Cross-Bedding Static		130	Mohr-Coulomb	198	36	None	0	
Tpl-Toe Static		130	Generalized Anisotropic			None	0	Static Bedding 55-65
Tpl Static		130	Generalized Anisotropic			None	0	Static Bedding 13-26

Slide Analysis Information

HOFF Property, Yorba Linda

Project Summary

File Name: Cross-section B-B' Static
 Slide Modeler Version: 7.038
 Project Title: HOFF Property, Yorba Linda
 Analysis: Cross-section B-B', Static
 Company: TGR Geotechnical, Inc.
 Date Created: 4/15/20

General Settings

Units of Measurement: Imperial Units
 Time Units: days
 Permeability Units: feet/second
 Failure Direction: Right to Left
 Data Output: Standard
 Maximum Material Properties: 20
 Maximum Support Properties: 20

Analysis Options

Slices Type: Vertical

Analysis Methods Used

Bishop simplified
 Janbu simplified

Number of slices: 50
 Tolerance: 0.005
 Maximum number of iterations: 75
 Check $m\alpha < 0.2$: Yes
 Create Interslice boundaries at intersections with water tables and piezos: Yes
 Initial trial value of FS: 1
 Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
 Pore Fluid Unit Weight [lbs/ft³]: 62.4
 Use negative pore pressure cutoff: Yes
 Maximum negative pore pressure [psf]: 0
 Advanced Groundwater Method: None

Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3

Surface Options

Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined
Minimum Area: Not Defined
Minimum Weight: Not Defined

Seismic

Advanced seismic analysis: No
Staged pseudostatic analysis: No

Material Properties

Property	Fill Static	Tpl-Toe Static	Tpl Static
Color			
Strength Type	Mohr-Coulomb	Generalized Anisotropic	Generalized Anisotropic
Unit Weight [lbs/ft3]	130	130	130
Cohesion [psf]	275		
Friction Angle [deg]	28		
Water Surface	None	None	None
Ru Value	0	0	0

Generalized Anisotropic Functions

Name: Static Bedding 13-26

Angle From	Angle To	Material
-26	-90	Tpl Cross-Bedding Static
-13	-26	Tpl Along Bedding
90	-13	Tpl Cross-Bedding Static

Name: Static Bedding 55-65

Angle From	Angle To	Material
55	-90	Tpl Cross-Bedding Static
65	55	Tpl Along Bedding
90	65	Tpl Cross-Bedding Static

Global Minimums

Method: bishop simplified

FS	1.701430
Center:	158.893, 1478.144
Radius:	628.153
Left Slip Surface Endpoint:	299.023, 865.821
Right Slip Surface Endpoint:	521.190, 965.000
Resisting Moment:	1.67715e+008 lb-ft
Driving Moment:	9.85731e+007 lb-ft
Total Slice Area:	2995.4 ft ²
Surface Horizontal Width:	222.168 ft
Surface Average Height:	13.4826 ft

Method: janbu simplified

FS	1.681820
Center:	222.008, 1383.472
Radius:	515.895
Left Slip Surface Endpoint:	316.173, 876.243
Right Slip Surface Endpoint:	523.718, 965.000
Resisting Horizontal Force:	218059 lb
Driving Horizontal Force:	129656 lb
Total Slice Area:	2755.33 ft ²
Surface Horizontal Width:	207.545 ft
Surface Average Height:	13.2758 ft

Valid / Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4696
Number of Invalid Surfaces: 155

Error Codes:

Error Code -102 reported for 11 surfaces
Error Code -103 reported for 84 surfaces
Error Code -106 reported for 37 surfaces
Error Code -108 reported for 1 surface
Error Code -1000 reported for 22 surfaces

Method: janbu simplified

Number of Valid Surfaces: 4687
Number of Invalid Surfaces: 164

Error Codes:

Error Code -102 reported for 11 surfaces
Error Code -103 reported for 84 surfaces
Error Code -106 reported for 37 surfaces
Error Code -108 reported for 10 surfaces
Error Code -1000 reported for 22 surfaces

Error Codes

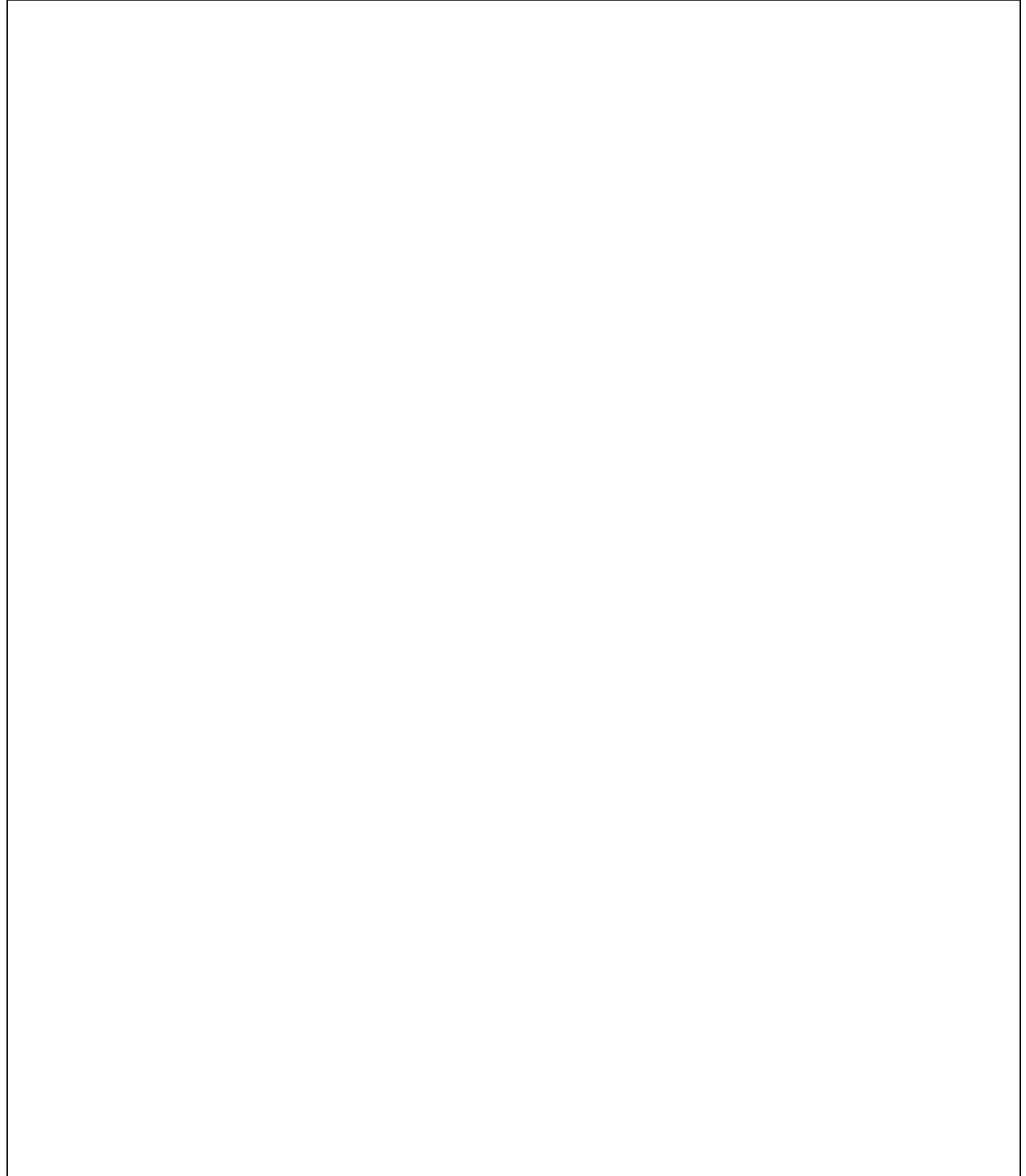
The following errors were encountered during the computation:

- 102 = Two surface / slope intersections, but resulting arc is actually outside soil region.
- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 106 = Average slice width is less than 0.0001 * (maximum horizontal extent of soil region). This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very

small (0.1 is an arbitrary number).
-1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

Slice Data

Global Minimum Query (bishop simplified) - Safety Factor: 1.70143



Slice Number	Width [ft]	Weight [lbs]	Angle of Slice Base [degrees]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]	Base Vertical Stress [psf]	Effective Vertical Stress [psf]
1	4.37257	427.894	13.0949	Fill Static	275	28	179.195	304.888	56.2118	0	56.2118	97.8949	97.8949
2	4.37257	1274.3	13.5047	Fill Static	275	28	235.075	399.963	235.022	0	235.022	291.478	291.478
3	4.37257	2173.84	13.9152	Fill Static	275	28	294.232	500.615	424.319	0	424.319	497.217	497.217
4	4.37257	3142.81	14.3265	Fill Static	275	28	357.72	608.636	627.477	0	627.477	718.835	718.835
5	4.37257	4092.98	14.7385	Fill Static	275	28	419.682	714.06	825.752	0	825.752	936.155	936.155
6	4.37257	5024.02	15.1513	Fill Static	275	28	480.104	816.863	1019.1	0	1019.1	1149.1	1149.1
7	4.37257	5935.82	15.5649	Fill Static	275	28	538.985	917.046	1207.51	0	1207.51	1357.64	1357.64
8	4.37257	6828.27	15.9794	Fill Static	275	28	596.327	1014.61	1391	0	1391	1561.76	1561.76
9	4.37257	7701.24	16.3947	Fill Static	275	28	652.129	1109.55	1569.56	0	1569.56	1761.43	1761.43
10	4.37257	8554.62	16.8109	Fill Static	275	28	706.39	1201.87	1743.2	0	1743.2	1956.61	1956.61
11	4.37257	9388.27	17.228	Fill Static	275	28	759.109	1291.57	1911.9	0	1911.9	2147.29	2147.29
12	4.37257	10202.1	17.646	Fill Static	275	28	810.289	1378.65	2075.66	0	2075.66	2333.42	2333.42
13	4.74962	11980.3	18.0832	Tpl Cross-Bedding Static	198	36	1047.53	1782.3	2180.61	0	2180.61	2522.65	2522.65
14	4.74962	12891	18.5395	Tpl Cross-Bedding Static	198	36	1115.71	1898.31	2340.27	0	2340.27	2714.44	2714.44
15	4.74962	12399.5	18.9971	Tpl Cross-Bedding Static	198	36	1073.49	1826.46	2241.38	0	2241.38	2610.94	2610.94
16	4.74962	10496.9	19.4559	Tpl Cross-Bedding Static	198	36	921.254	1567.45	1884.89	0	1884.89	2210.33	2210.33
17	4.74962	8567.84	19.916	Tpl Cross-Bedding Static	198	36	767.96	1306.63	1525.9	0	1525.9	1804.14	1804.14
18	4.74962	7499.05	20.3775	Tpl Cross-Bedding Static	198	36	682.431	1161.11	1325.6	0	1325.6	1579.09	1579.09
19	4.41247	7412.28	20.8239	Fill Static	275	28	613.711	1044.19	1446.63	0	1446.63	1680.05	1680.05
20	4.41247	7847.48	21.2551	Fill Static	275	28	639.719	1088.44	1529.85	0	1529.85	1778.69	1778.69
21	4.41247	8260.72	21.6876	Fill Static	275	28	664.205	1130.1	1608.21	0	1608.21	1872.36	1872.36
22	4.41247	8651.8	22.1214	Fill Static	275	28	687.164	1169.16	1681.67	0	1681.67	1961	1961
23	4.41247	9020.51	22.5565	Fill Static	275	28	708.593	1205.62	1750.25	0	1750.25	2044.57	2044.57
24	4.41247	9366.64	22.993	Fill Static	275	28	728.489	1239.47	1813.91	0	1813.91	2123.03	2123.03
25	4.41247	9689.99	23.431	Fill Static	275	28	746.847	1270.71	1872.65	0	1872.65	2196.32	2196.32
26	4.41247	9990.31	23.8703	Fill Static	275	28	763.664	1299.32	1926.46	0	1926.46	2264.4	2264.4
27	4.41247	10267.4	24.3112	Fill Static	275	28	778.933	1325.3	1975.32	0	1975.32	2327.2	2327.2
28	4.41247	10521	24.7536	Fill Static	275	28	792.645	1348.63	2019.21	0	2019.21	2384.68	2384.68
29	4.41247	10750.8	25.1976	Fill Static	275	28	804.805	1369.32	2058.12	0	2058.12	2436.79	2436.79
30	4.41247	10768.6	25.6433	Fill Static	275	28	803.818	1367.64	2054.96	0	2054.96	2440.83	2440.83
31	4.41247	10481.5	26.0906	Fill Static	275	28	784.082	1334.06	1991.8	0	1991.8	2375.76	2375.76
32	4.41247	10167.2	26.5396	Fill Static	275	28	762.758	1297.78	1923.56	0	1923.56	2304.52	2304.52
33	4.41247	9827.98	26.9903	Fill Static	275	28	740.002	1259.06	1850.75	0	1850.75	2227.64	2227.64
34	4.41247	9463.66	27.4429	Fill Static	275	28	715.813	1217.91	1773.35	0	1773.35	2145.07	2145.07
35	4.41247	9073.9	27.8974	Fill Static	275	28	690.185	1174.3	1691.34	0	1691.34	2056.73	2056.73
36	4.41247	8756.69	28.3538	Fill Static	275	28	669.07	1138.38	1623.77	0	1623.77	1984.84	1984.84
37	4.41247	8634.82	28.8121	Fill Static	275	28	659.854	1122.7	1594.29	0	1594.29	1957.22	1957.22
38	4.41247	8494.88	29.2725	Fill Static	275	28	649.577	1105.21	1561.4	0	1561.4	1925.51	1925.51
39	4.41247	8328.16	29.7349	Fill Static	275	28	637.722	1085.04	1523.46	0	1523.46	1887.73	1887.73
40	4.41247	8134.28	30.1995	Fill Static	275	28	624.281	1062.17	1480.45	0	1480.45	1843.79	1843.79
41	4.41247	7912.85	30.6663	Fill Static	275	28	609.247	1036.59	1432.34	0	1432.34	1793.6	1793.6
42	4.41247	7663.49	31.1354	Fill Static	275	28	592.609	1008.28	1379.1	0	1379.1	1737.09	1737.09
43	4.41247	7385.77	31.6068	Fill Static	275	28	574.358	977.23	1320.7	0	1320.7	1674.14	1674.14
44	4.41247	7079.26	32.0806	Fill Static	275	28	554.485	943.417	1257.11	0	1257.11	1604.67	1604.67
45	4.41247	6743.52	32.5568	Fill Static	275	28	532.979	906.826	1188.29	0	1188.29	1528.58	1528.58
46	4.41247	6378.09	33.0356	Fill Static	275	28	509.829	867.439	1114.22	0	1114.22	1445.75	1445.75
47	4.41247	5858.52	33.517	Fill Static	275	28	477.752	812.861	1011.57	0	1011.57	1327.99	1327.99
48	4.41247	4363.28	34.0012	Fill Static	275	28	388.771	661.466	726.836	0	726.836	989.077	989.077
49	4.41247	2640.22	34.488	Fill Static	275	28	287.047	488.391	401.331	0	401.331	598.525	598.525
50	4.41247	885.414	34.9778	Fill Static	275	28	184.117	313.261	71.9592	0	71.9592	200.773	200.773

Global Minimum Query (janbu simplified) - Safety Factor: 1.68182

	Angle	Base	Base	Shear	Shear	Base	Pore	Effective	Base	Effective
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Number	[ft]	[lbs]	of Slice Base [degrees]	Material	Cohesion [psf]	Friction Angle [degrees]	Stress [psf]	Strength [psf]	Normal Stress [psf]	Pressure [psf]	Normal Stress [psf]	Vertical Stress [psf]	Vertical Stress [psf]
1	4.1509	505.795	10.7517	Fill Static	275	28	190.61	320.572	85.7088	0	85.7088	121.903	121.903
2	4.1509	1507.86	11.2213	Fill Static	275	28	261.952	440.556	311.366	0	311.366	363.335	363.335
3	4.1509	2490.82	11.6917	Fill Static	275	28	331.564	557.631	531.553	0	531.553	600.166	600.166
4	4.1509	3454.6	12.1629	Fill Static	275	28	399.451	671.805	746.281	0	746.281	832.374	832.374
5	4.1509	4399.08	12.6349	Fill Static	275	28	465.615	783.081	955.562	0	955.562	1059.94	1059.94
6	4.1509	5324.16	13.1078	Fill Static	275	28	530.06	891.465	1159.4	0	1159.4	1282.83	1282.83
7	4.1509	6229.73	13.5816	Fill Static	275	28	592.786	996.96	1357.81	0	1357.81	1501.02	1501.02
8	4.1509	7115.68	14.0563	Fill Static	275	28	653.797	1099.57	1550.79	0	1550.79	1714.48	1714.48
9	4.1509	7981.88	14.5321	Fill Static	275	28	713.094	1199.3	1738.35	0	1738.35	1923.19	1923.19
10	4.1509	8828.22	15.0088	Fill Static	275	28	770.677	1296.14	1920.48	0	1920.48	2127.11	2127.11
11	4.1509	9612.05	15.4866	Fill Static	275	28	823.572	1385.1	2087.79	0	2087.79	2315.98	2315.98
12	4.1509	8982.93	15.9656	Fill Static	275	28	777.473	1307.57	1941.98	0	1941.98	2164.41	2164.41
13	4.1509	7659.96	16.4457	Fill Static	275	28	683.257	1149.12	1643.97	0	1643.97	1845.66	1845.66
14	4.1509	6316.56	16.9269	Fill Static	275	28	588.106	989.088	1343	0	1343	1521.99	1521.99
15	4.1509	5283.23	17.4094	Fill Static	275	28	514.934	866.026	1111.56	0	1111.56	1273.02	1273.02
16	4.1509	5653.64	17.8932	Fill Static	275	28	539.164	906.777	1188.2	0	1188.2	1362.27	1362.27
17	4.1509	6166.7	18.3783	Fill Static	275	28	573.087	963.829	1295.5	0	1295.5	1485.9	1485.9
18	4.1509	6658.67	18.8648	Fill Static	275	28	605.365	1018.12	1397.6	0	1397.6	1604.45	1604.45
19	4.1509	7129.37	19.3527	Fill Static	275	28	635.997	1069.63	1494.49	0	1494.49	1717.87	1717.87
20	4.1509	7578.61	19.8421	Fill Static	275	28	664.979	1118.38	1586.16	0	1586.16	1826.12	1826.12
21	4.1509	8006.2	20.3329	Fill Static	275	28	692.31	1164.34	1672.61	0	1672.61	1929.15	1929.15
22	4.1509	8411.93	20.8254	Fill Static	275	28	717.985	1207.52	1753.82	0	1753.82	2026.92	2026.92
23	4.1509	8795.58	21.3194	Fill Static	275	28	742.002	1247.91	1829.78	0	1829.78	2119.37	2119.37
24	4.1509	9156.93	21.8151	Fill Static	275	28	764.356	1285.51	1900.49	0	1900.49	2206.45	2206.45
25	4.1509	9495.77	22.3126	Fill Static	275	28	785.042	1320.3	1965.93	0	1965.93	2288.1	2288.1
26	4.1509	9811.84	22.8118	Fill Static	275	28	804.058	1352.28	2026.08	0	2026.08	2364.26	2364.26
27	4.1509	10104.9	23.3128	Fill Static	275	28	821.396	1381.44	2080.92	0	2080.92	2434.89	2434.89
28	4.1509	10234.9	23.8158	Fill Static	275	28	827.71	1392.06	2100.88	0	2100.88	2466.21	2466.21
29	4.1509	10070.8	24.3207	Fill Static	275	28	814.356	1369.6	2058.65	0	2058.65	2426.7	2426.7
30	4.1509	9876.09	24.8276	Fill Static	275	28	799.015	1343.8	2010.12	0	2010.12	2379.79	2379.79
31	4.1509	9657.27	25.3366	Fill Static	275	28	782.141	1315.42	1956.74	0	1956.74	2327.07	2327.07

32	4.1509	9414.03	25.8478	Fill Static	275	28	763.72	1284.44	1898.49	0	1898.49	2268.47	2268.47
33	4.1509	9146.06	26.3612	Fill Static	275	28	743.758	1250.87	1835.34	0	1835.34	2203.91	2203.91
34	4.1509	8867.36	26.8768	Fill Static	275	28	723.18	1216.26	1770.25	0	1770.25	2136.77	2136.77
35	4.1509	8771.01	27.3949	Fill Static	275	28	714.633	1201.88	1743.22	0	1743.22	2113.57	2113.57
36	4.1509	8717.19	27.9154	Fill Static	275	28	708.883	1192.21	1725.03	0	1725.03	2100.61	2100.61
37	4.1509	8637.25	28.4383	Fill Static	275	28	701.438	1179.69	1701.48	0	1701.48	2081.35	2081.35
38	4.1509	8530.8	28.9639	Fill Static	275	28	692.289	1164.31	1672.54	0	1672.54	2055.71	2055.71
39	4.1509	8397.44	29.4922	Fill Static	275	28	681.425	1146.03	1638.18	0	1638.18	2023.59	2023.59
40	4.1509	8236.74	30.0232	Fill Static	275	28	668.837	1124.86	1598.36	0	1598.36	1984.88	1984.88
41	4.1509	8048.28	30.5571	Fill Static	275	28	654.514	1100.77	1553.06	0	1553.06	1939.47	1939.47
42	4.1509	7831.59	31.094	Fill Static	275	28	638.443	1073.75	1502.22	0	1502.22	1887.27	1887.27
43	4.1509	7586.19	31.6339	Fill Static	275	28	620.615	1043.76	1445.83	0	1445.83	1828.14	1828.14
44	4.1509	7311.59	32.177	Fill Static	275	28	601.016	1010.8	1383.84	0	1383.84	1761.98	1761.98
45	4.1509	7007.26	32.7233	Fill Static	275	28	579.634	974.84	1316.21	0	1316.21	1688.66	1688.66
46	4.1509	6632.26	33.273	Fill Static	275	28	553.906	931.571	1234.83	0	1234.83	1598.31	1598.31
47	4.1509	5446.44	33.8261	Fill Static	275	28	477.354	802.823	992.69	0	992.69	1312.57	1312.57
48	4.1509	3929.61	34.3829	Fill Static	275	28	380.592	640.087	686.629	0	686.629	947.059	947.059
49	4.1509	2380.71	34.9434	Fill Static	275	28	282.518	475.144	376.416	0	376.416	573.821	573.821
50	4.1509	799.08	35.5077	Fill Static	275	28	183.125	307.984	62.0338	0	62.0338	192.693	192.693

Interslice Data

Global Minimum Query (bishop simplified) - Safety Factor: 1.70143



Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	299.023	865.821	0	0	0
2	303.395	866.838	725.694	0	0
3	307.768	867.888	1505.88	0	0
4	312.14	868.971	2331.64	0	0
5	316.513	870.088	3193.74	0	0
6	320.885	871.238	4077.41	0	0
7	325.258	872.422	4968.26	0	0
8	329.631	873.64	5852.28	0	0
9	334.003	874.892	6715.82	0	0
10	338.376	876.179	7545.64	0	0
11	342.748	877.5	8328.84	0	0
12	347.121	878.856	9052.95	0	0
13	351.493	880.247	9705.84	0	0
14	356.243	881.798	11295.1	0	0
15	360.993	883.39	12862.1	0	0
16	365.742	885.026	14291.3	0	0
17	370.492	886.703	15500.6	0	0
18	375.242	888.424	16519.2	0	0
19	379.991	890.188	17419	0	0
20	384.404	891.867	17696.8	0	0
21	388.816	893.583	17891.4	0	0
22	393.229	895.338	17997.5	0	0
23	397.641	897.132	18010.7	0	0
24	402.054	898.964	17926.7	0	0
25	406.466	900.837	17742.1	0	0
26	410.878	902.749	17453.7	0	0
27	415.291	904.702	17058.8	0	0
28	419.703	906.695	16555.4	0	0
29	424.116	908.729	15941.8	0	0
30	428.528	910.806	15217	0	0
31	432.941	912.924	14407.9	0	0
32	437.353	915.085	13560.9	0	0
33	441.766	917.288	12684.5	0	0
34	446.178	919.536	11787.7	0	0
35	450.591	921.827	10880	0	0
36	455.003	924.163	9971.77	0	0
37	459.416	926.544	9054.92	0	0
38	463.828	928.971	8094.68	0	0
39	468.241	931.445	7096.53	0	0
40	472.653	933.965	6068.3	0	0
41	477.066	936.533	5018.64	0	0
42	481.478	939.15	3956.98	0	0
43	485.891	941.815	2893.61	0	0
44	490.303	944.53	1839.67	0	0
45	494.716	947.296	807.227	0	0
46	499.128	950.113	-190.717	0	0
47	503.54	952.983	-1140.18	0	0
48	507.953	955.905	-1990.18	0	0
49	512.365	958.882	-2439.56	0	0
50	516.778	961.913	-2390.6	0	0
51	521.19	965	0	0	0

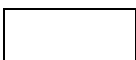
Global Minimum Query (janbu simplified) - Safety Factor: 1.68182



Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	316.173	876.243	0	0	0
2	320.324	877.032	722.525	0	0
3	324.475	877.855	1551.91	0	0
4	328.626	878.714	2469.65	0	0
5	332.777	879.609	3457.72	0	0
6	336.928	880.539	4498.56	0	0
7	341.079	881.506	5575.06	0	0
8	345.23	882.508	6670.55	0	0
9	349.381	883.548	7768.86	0	0
10	353.532	884.624	8854.22	0	0
11	357.682	885.737	9911.34	0	0
12	361.833	886.887	10923.9	0	0
13	365.984	888.074	11840.3	0	0
14	370.135	889.3	12658.1	0	0
15	374.286	890.563	13399.2	0	0
16	378.437	891.864	14086.8	0	0
17	382.588	893.205	14729.3	0	0
18	386.739	894.584	15318.2	0	0
19	390.89	896.002	15845.2	0	0
20	395.041	897.46	16302.5	0	0
21	399.191	898.958	16683	0	0
22	403.342	900.496	16979.9	0	0
23	407.493	902.075	17186.9	0	0
24	411.644	903.695	17298.2	0	0
25	415.795	905.356	17308.8	0	0
26	419.946	907.06	17213.9	0	0
27	424.097	908.806	17009.5	0	0
28	428.248	910.594	16691.9	0	0
29	432.399	912.427	16273.7	0	0
30	436.55	914.303	15787.1	0	0
31	440.7	916.223	15238.8	0	0
32	444.851	918.188	14635	0	0
33	449.002	920.199	13983	0	0
34	453.153	922.256	13290.6	0	0
35	457.304	924.36	12564	0	0
36	461.455	926.511	11776.2	0	0
37	465.606	928.71	10920.8	0	0
38	469.757	930.958	10003.4	0	0
39	473.908	933.256	9030.33	0	0
40	478.059	935.604	8008.85	0	0
41	482.209	938.002	6947.09	0	0
42	486.36	940.453	5854.06	0	0
43	490.511	942.956	4739.76	0	0
44	494.662	945.513	3615.16	0	0
45	498.813	948.125	2492.3	0	0
46	502.964	950.792	1384.28	0	0
47	507.115	953.516	316.76	0	0
48	511.266	956.298	-465.802	0	0
49	515.417	959.138	-838.52	0	0
50	519.568	962.038	-759.233	0	0
51	523.718	965	0	0	0

List Of Coordinates

External Boundary



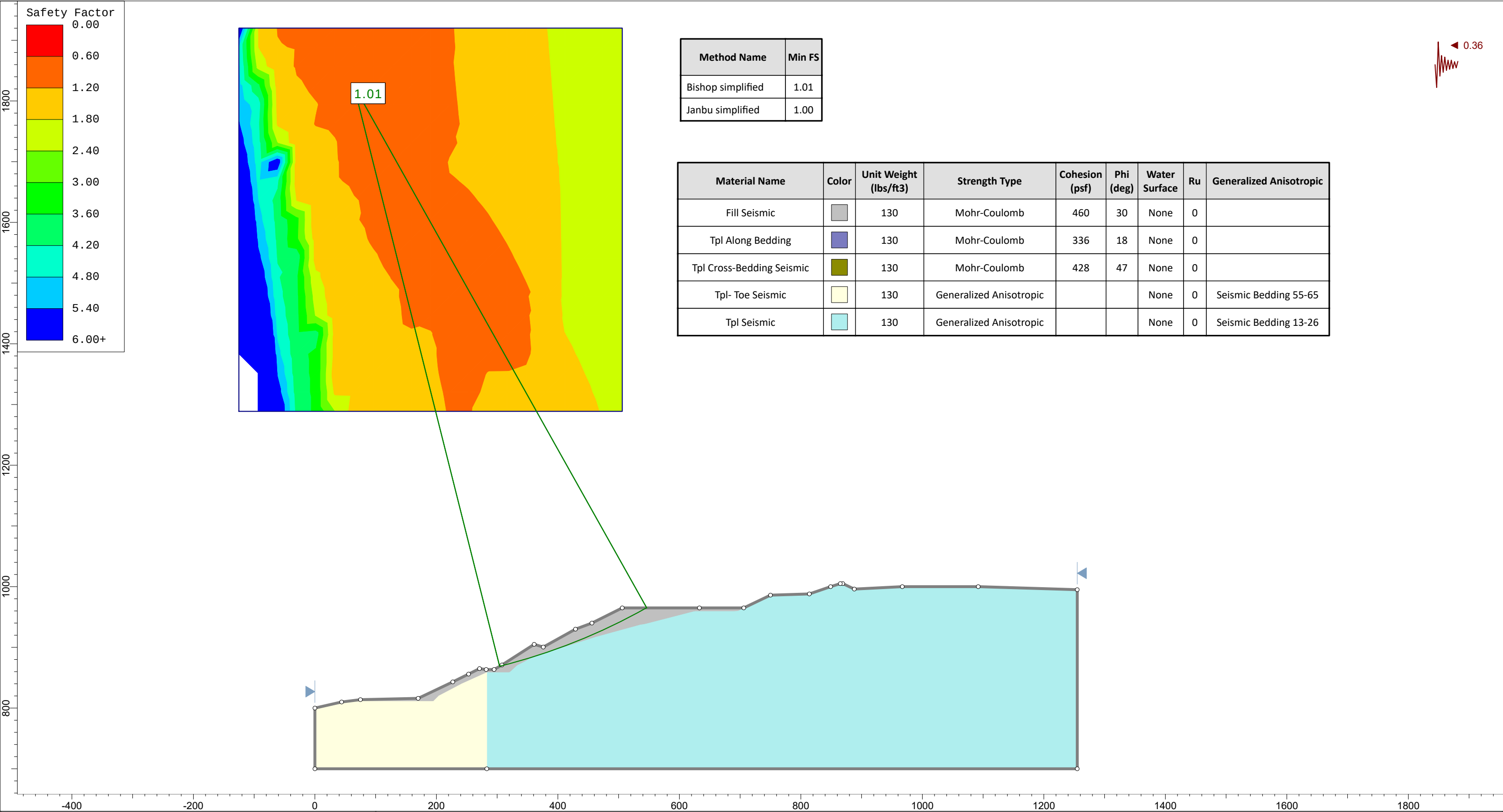
X	Y
0	700
283	700
1255	700
1255	995
1092	1000
967	1000
888	996
869	1005
865	1005
849	1000
814	988
750	986
706	965
633	965
506	965
456	940
429	930
376	900.5
361	905
308	871
295	863.5
282	863.5
271	865
253	856
227	843.1
170	816
75	813.9
44	810
0	800

Material Boundary

X	Y
75	813.9
81	811
195	811
204	820
242	840
283	858.5
320	858.5
324.266	863.03
333	871
353	881
400	897
463	917
536	937
625	960
698	960
706	965

Material Boundary

X	Y
283	700
283	858.5



Slide Analysis Information

HOFF Property, Yorba Linda

Project Summary

File Name: Cross-section B-B' Seismic
 Slide Modeler Version: 7.038
 Project Title: HOFF Property, Yorba Linda
 Analysis: Cross-section B-B', Seismic
 Company: TGR Geotechnical, Inc.
 Date Created: 4/15/20

General Settings

Units of Measurement: Imperial Units
 Time Units: days
 Permeability Units: feet/second
 Failure Direction: Right to Left
 Data Output: Standard
 Maximum Material Properties: 20
 Maximum Support Properties: 20

Analysis Options

Slices Type: Vertical

Analysis Methods Used

Bishop simplified
 Janbu simplified

Number of slices: 50
 Tolerance: 0.005
 Maximum number of iterations: 75
 Check $\alpha < 0.2$: Yes
 Create Interslice boundaries at intersections with water tables and piezos: Yes
 Initial trial value of FS: 1
 Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
 Pore Fluid Unit Weight [lbs/ft³]: 62.4
 Use negative pore pressure cutoff: Yes
 Maximum negative pore pressure [psf]: 0
 Advanced Groundwater Method: None

Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3

Surface Options

Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined
Minimum Area: Not Defined
Minimum Weight: Not Defined

Seismic

Advanced seismic analysis: No
Staged pseudostatic analysis: No

Loading

Seismic Load Coefficient (Horizontal): 0.36

Material Properties

Property	Fill Seismic	Tpl- Toe Seismic	Tpl Seismic
Color			
Strength Type	Mohr-Coulomb	Generalized Anisotropic	Generalized Anisotropic
Unit Weight [lbs/ft3]	130	130	130
Cohesion [psf]	460		
Friction Angle [deg]	30		
Water Surface	None	None	None
Ru Value	0	0	0

Generalized Anisotropic Functions

Name: Seismic Bedding 55-65

Angle From	Angle To	Material
55	-90	Tpl Cross-Bedding Seismic
65	55	Tpl Along Bedding
90	65	Tpl Cross-Bedding Seismic

Name: Seismic Bedding 13-26

Angle From	Angle To	Material
-26	-90	Tpl Cross-Bedding Seismic
-13	-26	Tpl Along Bedding
90	-13	Tpl Cross-Bedding Seismic

Global Minimums

Method: bishop simplified

FS	1.009140
Center:	64.221, 1825.277
Radius:	986.114
Left Slip Surface Endpoint:	304.203, 868.810
Right Slip Surface Endpoint:	546.242, 965.000
Resisting Moment:	3.36486e+008 lb-ft
Driving Moment:	3.3344e+008 lb-ft
Total Slice Area:	3693.71 ft ²
Surface Horizontal Width:	242.039 ft
Surface Average Height:	15.2608 ft

Method: janbu simplified

FS	0.995778
Center:	253.565, 1383.472
Radius:	507.219
Left Slip Surface Endpoint:	323.806, 881.140
Right Slip Surface Endpoint:	540.188, 965.000
Resisting Horizontal Force:	294087 lb
Driving Horizontal Force:	295334 lb
Total Slice Area:	3477.65 ft ²
Surface Horizontal Width:	216.382 ft
Surface Average Height:	16.0718 ft

Valid / Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4697
Number of Invalid Surfaces: 154

Error Codes:

Error Code -102 reported for 11 surfaces
Error Code -103 reported for 84 surfaces
Error Code -106 reported for 37 surfaces
Error Code -1000 reported for 22 surfaces

Method: janbu simplified

Number of Valid Surfaces: 4690
Number of Invalid Surfaces: 161

Error Codes:

Error Code -102 reported for 11 surfaces
Error Code -103 reported for 84 surfaces
Error Code -106 reported for 37 surfaces
Error Code -108 reported for 7 surfaces
Error Code -1000 reported for 22 surfaces

Error Codes

The following errors were encountered during the computation:

- 102 = Two surface / slope intersections, but resulting arc is actually outside soil region.
- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 106 = Average slice width is less than 0.0001 * (maximum horizontal extent of soil region). This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

Slice Data

Global Minimum Query (bishop simplified) - Safety Factor: 1.00914

Slice Number	Width [ft]	Weight [lbs]	Angle of Slice Base [degrees]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]	Base Vertical Stress [psf]	Effective Vertical Stress [psf]
1	4.84078	497.05	14.2301	Fill Seismic	460	30	449.297	453.404	-11.4254	0	-11.4254	102.516	102.516
2	4.84078	1610.01	14.5205	Fill Seismic	460	30	562.628	567.77	186.663	0	186.663	332.383	332.383
3	4.84078	2766.99	14.8112	Fill Seismic	460	30	679.86	686.074	391.572	0	391.572	571.341	571.341
4	4.84078	3907.43	15.1024	Fill Seismic	460	30	794.76	802.024	592.403	0	592.403	806.881	806.881
5	4.84078	5031.24	15.3939	Fill Seismic	460	30	907.334	915.627	789.17	0	789.17	1038.99	1038.99
6	4.84078	6138.38	15.6858	Fill Seismic	460	30	1017.59	1026.89	981.884	0	981.884	1267.64	1267.64
7	4.84078	7228.75	15.9782	Fill Seismic	460	30	1125.54	1135.82	1170.56	0	1170.56	1492.84	1492.84
8	4.84078	8302.29	16.271	Fill Seismic	460	30	1231.18	1242.43	1355.21	0	1355.21	1714.56	1714.56
9	4.84078	9358.93	16.5642	Fill Seismic	460	30	1334.53	1346.72	1535.85	0	1535.85	1932.78	1932.78
10	4.84078	10398.6	16.8578	Fill Seismic	460	30	1435.58	1448.7	1712.49	0	1712.49	2147.5	2147.5
11	4.84078	11421.2	17.152	Fill Seismic	460	30	1534.36	1548.38	1885.13	0	1885.13	2358.69	2358.69
12	4.84078	12324.4	17.4466	Fill Seismic	460	30	1620.62	1635.43	2035.9	0	2035.9	2545.22	2545.22
13	4.84078	11214.9	17.7416	Fill Seismic	460	30	1505.35	1519.11	1834.43	0	1834.43	2316.06	2316.06
14	4.84078	9317.67	18.0372	Fill Seismic	460	30	1312.24	1324.24	1496.9	0	1496.9	1924.21	1924.21
15	4.84078	7440.05	18.3332	Fill Seismic	460	30	1122.12	1132.38	1164.59	0	1164.59	1536.42	1536.42
16	4.84078	7215.08	18.6298	Fill Seismic	460	30	1096.73	1106.76	1120.22	0	1120.22	1489.95	1489.95
17	4.84078	7874.89	18.9269	Fill Seismic	460	30	1158.87	1169.47	1228.83	0	1228.83	1626.21	1626.21
18	4.84078	8517.04	19.2245	Fill Seismic	460	30	1218.91	1230.05	1333.77	0	1333.77	1758.82	1758.82
19	4.84078	9141.43	19.5226	Fill Seismic	460	30	1276.85	1288.52	1435.05	0	1435.05	1887.77	1887.77
20	4.84078	9747.95	19.8213	Fill Seismic	460	30	1332.7	1344.88	1532.66	0	1532.66	2013.02	2013.02
21	4.84078	10336.5	20.1206	Fill Seismic	460	30	1386.46	1399.14	1626.63	0	1626.63	2134.57	2134.57
22	4.84078	10907	20.4204	Fill Seismic	460	30	1438.14	1451.29	1716.96	0	1716.96	2252.38	2252.38
23	4.84078	11459.3	20.7208	Fill Seismic	460	30	1487.74	1501.34	1803.65	0	1803.65	2366.44	2366.44
24	4.84078	11993.4	21.0218	Fill Seismic	460	30	1535.27	1549.3	1886.72	0	1886.72	2476.73	2476.73
25	4.84078	12509.1	21.3235	Fill Seismic	460	30	1580.72	1595.17	1966.16	0	1966.16	2583.21	2583.21
26	4.84078	12992.5	21.6257	Fill Seismic	460	30	1622.79	1637.62	2039.71	0	2039.71	2683.06	2683.06
27	4.84078	13076.5	21.9286	Fill Seismic	460	30	1626.22	1641.08	2045.69	0	2045.69	2700.37	2700.37

28	4.84078	12969	22.2321	Fill Seismic	460	30	1611.26	1625.99	2019.56	0	2019.56	2678.16	2678.16
29	4.84078	12842.6	22.5363	Fill Seismic	460	30	1594.57	1609.14	1990.37	0	1990.37	2652.04	2652.04
30	4.84078	12697.2	22.8411	Fill Seismic	460	30	1576.13	1590.54	1958.15	0	1958.15	2622.03	2622.03
31	4.84078	12532.8	23.1467	Fill Seismic	460	30	1555.95	1570.17	1922.87	0	1922.87	2588.04	2588.04
32	4.84078	12430.5	23.4529	Fill Seismic	460	30	1541.75	1555.84	1898.04	0	1898.04	2566.91	2566.91
33	4.84078	12597.1	23.7599	Fill Seismic	460	30	1552.97	1567.16	1917.65	0	1917.65	2601.29	2601.29
34	4.84078	12769.4	24.0675	Fill Seismic	460	30	1564.63	1578.93	1938.05	0	1938.05	2636.88	2636.88
35	4.84078	12922.1	24.376	Fill Seismic	460	30	1574.35	1588.74	1955.04	0	1955.04	2668.4	2668.4
36	4.84078	13055	24.6851	Fill Seismic	460	30	1582.13	1596.59	1968.64	0	1968.64	2695.84	2695.84
37	4.84078	13168	24.9951	Fill Seismic	460	30	1587.97	1602.48	1978.83	0	1978.83	2719.14	2719.14
38	4.84078	13260.8	25.3058	Fill Seismic	460	30	1591.86	1606.41	1985.65	0	1985.65	2738.32	2738.32
39	4.84078	13333.5	25.6173	Fill Seismic	460	30	1593.83	1608.4	1989.09	0	1989.09	2753.32	2753.32
40	4.84078	13385.7	25.9296	Fill Seismic	460	30	1593.85	1608.42	1989.14	0	1989.14	2764.09	2764.09
41	4.84078	13417.4	26.2428	Fill Seismic	460	30	1591.95	1606.5	1985.8	0	1985.8	2770.62	2770.62
42	4.84078	13353.6	26.5568	Fill Seismic	460	30	1581.25	1595.7	1967.08	0	1967.08	2757.42	2757.42
43	4.84078	12179.8	26.8717	Fill Seismic	460	30	1468.89	1482.32	1770.71	0	1770.71	2515.01	2515.01
44	4.84078	10625.6	27.1874	Fill Seismic	460	30	1322.46	1334.55	1514.76	0	1514.76	2194.04	2194.04
45	4.84078	9050.19	27.5041	Fill Seismic	460	30	1174.96	1185.7	1256.94	0	1256.94	1868.69	1868.69
46	4.84078	7453.35	27.8217	Fill Seismic	460	30	1026.39	1035.77	997.269	0	997.269	1538.92	1538.92
47	4.84078	5834.88	28.1401	Fill Seismic	460	30	876.768	884.782	735.744	0	735.744	1204.68	1204.68
48	4.84078	4194.6	28.4596	Fill Seismic	460	30	726.09	732.726	472.376	0	472.376	865.948	865.948
49	4.84078	2532.32	28.78	Fill Seismic	460	30	574.361	579.611	207.171	0	207.171	522.668	522.668
50	4.84078	847.826	29.1014	Fill Seismic	460	30	421.586	425.439	-59.8606	0	-59.8606	174.805	174.805

Global Minimum Query (janbu simplified) - Safety Factor: 0.995778

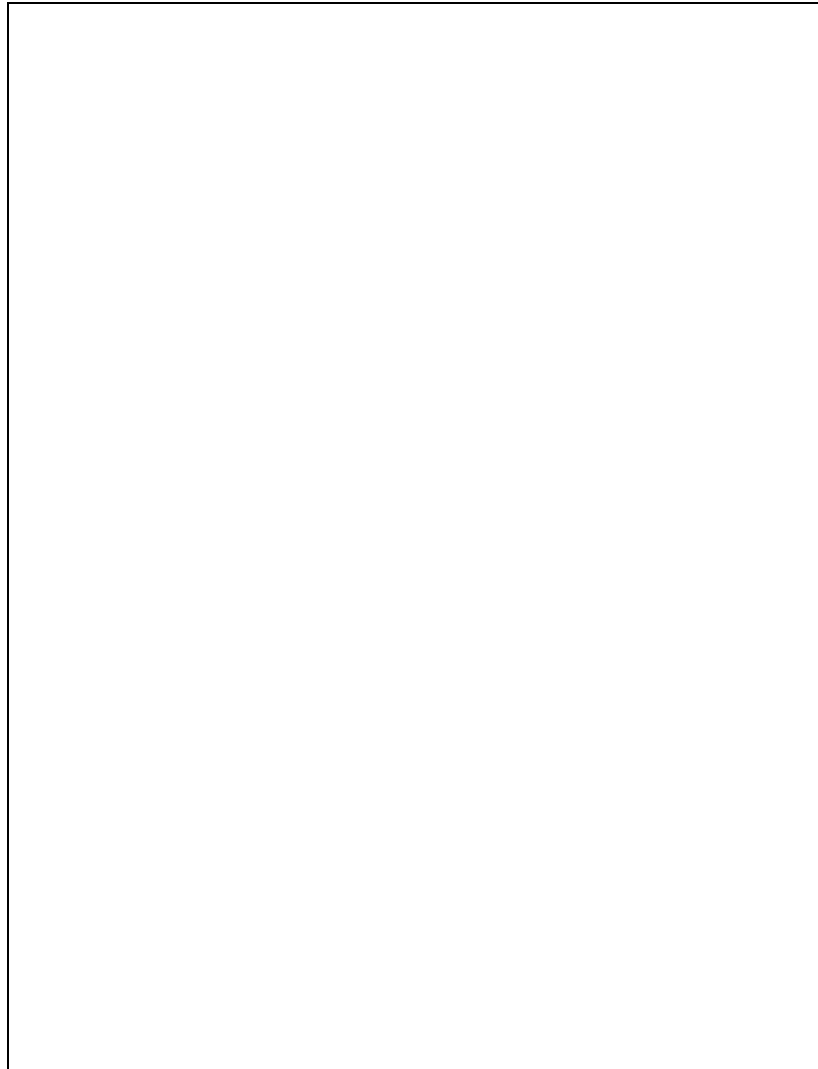
Slice Number	Width [ft]	Weight [lbs]	Angle of Slice Base [degrees]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]	Base Vertical Stress [psf]	Effective Vertical Stress [psf]
1	4.32764	605.367	8.20698	Fill Seismic	460	30	501.311	499.195	67.8869	0	67.8869	140.189	140.189
2	4.32764	1805.37	8.7012	Fill Seismic	460	30	646.685	643.955	318.618	0	318.618	417.589	417.589
3	4.32764	2983.86	9.19609	Fill Seismic	460	30	788.056	784.729	562.447	0	562.447	690.029	690.029
4	4.32764	4140.76	9.69166	Fill Seismic	460	30	925.459	921.552	799.431	0	799.431	957.484	957.484
5	4.32764	5275.97	10.188	Fill Seismic	460	30	1058.93	1054.46	1029.63	0	1029.63	1219.94	1219.94
6	4.32764	6389.39	10.6851	Fill Seismic	460	30	1188.5	1183.48	1253.11	0	1253.11	1477.36	1477.36
7	4.32764	7480.91	11.183	Fill	460	30	1314.21	1308.66	1469.92	0	1469.92	1729.73	1729.73

				Seismic									
8	4.32764	8550.43	11.6817	Fill	460	30	1436.07	1430.01	1680.11	0	1680.11	1977.03	1977.03
				Seismic									
9	4.32764	9409.36	12.1814	Fill	460	30	1531.68	1525.21	1845.01	0	1845.01	2175.64	2175.64
				Seismic									
10	4.32764	8547.29	12.682	Fill	460	30	1422.29	1416.29	1656.34	0	1656.34	1976.4	1976.4
				Seismic									
11	4.32764	7257.78	13.1835	Fill	460	30	1263.45	1258.11	1382.37	0	1382.37	1678.33	1678.33
				Seismic									
12	4.32764	5945.77	13.6862	Fill	460	30	1103.4	1098.75	1106.34	0	1106.34	1375.04	1375.04
				Seismic									
13	4.32764	5531.37	14.1898	Fill	460	30	1049.77	1045.34	1013.84	0	1013.84	1279.27	1279.27
				Seismic									
14	4.32764	6255.66	14.6947	Fill	460	30	1129.1	1124.34	1150.66	0	1150.66	1446.76	1446.76
				Seismic									
15	4.32764	6960.82	15.2006	Fill	460	30	1205.43	1200.35	1282.32	0	1282.32	1609.84	1609.84
				Seismic									
16	4.32764	7642.86	15.7078	Fill	460	30	1278.34	1272.94	1408.06	0	1408.06	1767.57	1767.57
				Seismic									
17	4.32764	8301.61	16.2163	Fill	460	30	1347.84	1342.15	1527.93	0	1527.93	1919.93	1919.93
				Seismic									
18	4.32764	8936.9	16.7261	Fill	460	30	1413.96	1407.99	1641.96	0	1641.96	2066.87	2066.87
				Seismic									
19	4.32764	9548.53	17.2372	Fill	460	30	1476.69	1470.46	1750.18	0	1750.18	2208.34	2208.34
				Seismic									
20	4.32764	10136.3	17.7498	Fill	460	30	1536.09	1529.6	1852.61	0	1852.61	2344.31	2344.31
				Seismic									
21	4.32764	10700.1	18.2638	Fill	460	30	1592.14	1585.42	1949.28	0	1949.28	2474.72	2474.72
				Seismic									
22	4.32764	11239.6	18.7794	Fill	460	30	1644.86	1637.92	2040.22	0	2040.22	2599.52	2599.52
				Seismic									
23	4.32764	11754.6	19.2965	Fill	460	30	1694.28	1687.13	2125.45	0	2125.45	2718.66	2718.66
				Seismic									
24	4.32764	12244.9	19.8153	Fill	460	30	1740.4	1733.05	2205	0	2205	2832.11	2832.11
				Seismic									
25	4.32764	12601.5	20.3358	Fill	460	30	1771.23	1763.75	2258.16	0	2258.16	2914.62	2914.62
				Seismic									
26	4.32764	12609.6	20.8581	Fill	460	30	1763.41	1755.96	2244.67	0	2244.67	2916.58	2916.58
				Seismic									
27	4.32764	12570.9	21.3821	Fill	460	30	1750.45	1743.06	2222.32	0	2222.32	2907.68	2907.68
				Seismic									
28	4.32764	12506.4	21.9081	Fill	460	30	1734.72	1727.4	2195.2	0	2195.2	2892.84	2892.84
				Seismic									
29	4.32764	12415.9	22.436	Fill	460	30	1716.25	1709	2163.33	0	2163.33	2871.98	2871.98
				Seismic									
30	4.32764	12299.1	22.9659	Fill	460	30	1695.03	1687.87	2126.73	0	2126.73	2845.03	2845.03
				Seismic									
31	4.32764	12188.2	23.4979	Fill	460	30	1674.54	1667.47	2091.4	0	2091.4	2819.44	2819.44
				Seismic									
32	4.32764	12286.4	24.032	Fill	460	30	1676.44	1669.36	2094.68	0	2094.68	2842.2	2842.2
				Seismic									
33	4.32764	12404.4	24.5684	Fill	460	30	1680.31	1673.22	2101.37	0	2101.37	2869.55	2869.55
				Seismic									
34	4.32764	12494.7	25.1071	Fill	460	30	1681.13	1674.03	2102.76	0	2102.76	2890.51	2890.51
				Seismic									
35	4.32764	12557.1	25.6482	Fill	460	30	1678.89	1671.8	2098.9	0	2098.9	2905.03	2905.03
				Seismic									
36	4.32764	12591.2	26.1917	Fill	460	30	1673.59	1666.52	2089.75	0	2089.75	2912.95	2912.95
				Seismic									
37	4.32764	12596.5	26.7378	Fill	460	30	1665.23	1658.2	2075.35	0	2075.35	2914.26	2914.26
				Seismic									
38	4.32764	12572.6	27.2865	Fill	460	30	1653.83	1646.85	2055.68	0	2055.68	2908.79	2908.79
				Seismic									
39	4.32764	12519.1	27.838	Fill	460	30	1639.37	1632.45	2030.75	0	2030.75	2896.48	2896.48
				Seismic									

40	4.32764	12435.6	28.3922	Fill Seismic	460	30	1621.88	1615.03	2000.57	0	2000.57	2877.23	2877.23
41	4.32764	12321.6	28.9494	Fill Seismic	460	30	1601.33	1594.57	1965.13	0	1965.13	2850.92	2850.92
42	4.32764	12176.5	29.5096	Fill Seismic	460	30	1577.74	1571.08	1924.44	0	1924.44	2817.43	2817.43
43	4.32764	11507	30.0729	Fill Seismic	460	30	1501.6	1495.26	1793.12	0	1793.12	2662.62	2662.62
44	4.32764	10087.1	30.6395	Fill Seismic	460	30	1351.27	1345.56	1533.84	0	1533.84	2334.24	2334.24
45	4.32764	8628.51	31.2093	Fill Seismic	460	30	1198.68	1193.62	1270.67	0	1270.67	1996.88	1996.88
46	4.32764	7136.7	31.7827	Fill Seismic	460	30	1044.46	1040.05	1004.68	0	1004.68	1651.83	1651.83
47	4.32764	5611.07	32.3596	Fill Seismic	460	30	888.618	884.866	735.89	0	735.89	1298.95	1298.95
48	4.32764	4050.97	32.9402	Fill Seismic	460	30	731.168	728.081	464.329	0	464.329	938.071	938.071
49	4.32764	2455.72	33.5246	Fill Seismic	460	30	572.123	569.707	190.018	0	190.018	569.052	569.052
50	4.32764	824.613	34.1131	Fill Seismic	460	30	411.497	409.76	-87.0188	0	-87.0188	191.723	191.723

Interslice Data

Global Minimum Query (bishop simplified) - Safety Factor: 1.00914



Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	304.203	868.81	0	0	0
2	309.044	870.037	2013.17	0	0
3	313.885	871.291	3927.01	0	0
4	318.726	872.571	5725.47	0	0
5	323.566	873.877	7397.7	0	0
6	328.407	875.21	8933.15	0	0
7	333.248	876.57	10321.6	0	0
8	338.089	877.956	11553.1	0	0
9	342.93	879.369	12617.9	0	0
10	347.77	880.808	13506.8	0	0
11	352.611	882.275	14210.7	0	0
12	357.452	883.769	14720.8	0	0
13	362.293	885.291	15043.1	0	0
14	367.133	886.839	15462.2	0	0
15	371.974	888.416	16109.7	0	0
16	376.815	890.02	17002.9	0	0
17	381.656	891.652	17894.1	0	0
18	386.497	893.311	18637.3	0	0
19	391.337	895	19228.7	0	0
20	396.178	896.716	19664.6	0	0
21	401.019	898.461	19941.7	0	0
22	405.86	900.234	20057	0	0
23	410.7	902.036	20007.9	0	0
24	415.541	903.868	19791.9	0	0
25	420.382	905.728	19407	0	0
26	425.223	907.618	18851.3	0	0
27	430.064	909.537	18126.4	0	0
28	434.904	911.485	17315.8	0	0
29	439.745	913.464	16462	0	0
30	444.586	915.473	15570.6	0	0
31	449.427	917.512	14647.7	0	0
32	454.268	919.581	13699.5	0	0
33	459.108	921.681	12712.4	0	0
34	463.949	923.812	11619.3	0	0
35	468.79	925.974	10417	0	0
36	473.631	928.168	9108.9	0	0
37	478.471	930.393	7698.68	0	0
38	483.312	932.65	6190.47	0	0
39	488.153	934.938	4588.73	0	0
40	492.994	937.26	2898.28	0	0
41	497.835	939.613	1124.32	0	0
42	502.675	942	-727.581	0	0
43	507.516	944.419	-2628.82	0	0
44	512.357	946.872	-4236.02	0	0
45	517.198	949.358	-5416.71	0	0
46	522.038	951.879	-6146.87	0	0
47	526.879	954.433	-6402	0	0
48	531.72	957.023	-6157.12	0	0
49	536.561	959.646	-5386.76	0	0
50	541.402	962.306	-4064.91	0	0
51	546.242	965	0	0	0

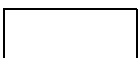
Global Minimum Query (janbu simplified) - Safety Factor: 0.995778



Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	323.806	881.14	0	0	0
2	328.134	881.764	1900.03	0	0
3	332.461	882.426	3825.87	0	0
4	336.789	883.127	5753.64	0	0
5	341.117	883.866	7660.26	0	0
6	345.444	884.644	9523.46	0	0
7	349.772	885.46	11321.7	0	0
8	354.1	886.316	13034.4	0	0
9	358.427	887.211	14641.5	0	0
10	362.755	888.145	16131.1	0	0
11	367.083	889.119	17570.3	0	0
12	371.41	890.132	19000.8	0	0
13	375.738	891.186	20449.4	0	0
14	380.065	892.28	21872.5	0	0
15	384.393	893.415	23180.3	0	0
16	388.721	894.591	24361.3	0	0
17	393.048	895.808	25405	0	0
18	397.376	897.067	26301.6	0	0
19	401.704	898.367	27042.2	0	0
20	406.031	899.71	27618.4	0	0
21	410.359	901.095	28022.6	0	0
22	414.687	902.524	28247.7	0	0
23	419.014	903.995	28287.6	0	0
24	423.342	905.51	28136.7	0	0
25	427.669	907.07	27790.2	0	0
26	431.997	908.674	27264.6	0	0
27	436.325	910.323	26623	0	0
28	440.652	912.017	25875.3	0	0
29	444.98	913.757	25028	0	0
30	449.308	915.544	24088.5	0	0
31	453.635	917.378	23065	0	0
32	457.963	919.26	21958.5	0	0
33	462.291	921.189	20717.7	0	0
34	466.618	923.168	19335.8	0	0
35	470.946	925.196	17818.2	0	0
36	475.274	927.274	16171.1	0	0
37	479.601	929.402	14402	0	0
38	483.929	931.583	12518.8	0	0
39	488.256	933.815	10530.6	0	0
40	492.584	936.1	8447.32	0	0
41	496.912	938.439	6280.06	0	0
42	501.239	940.833	4040.77	0	0
43	505.567	943.283	1742.52	0	0
44	509.895	945.789	-422.46	0	0
45	514.222	948.352	-2162.53	0	0
46	518.55	950.974	-3434.76	0	0
47	522.878	953.655	-4196.99	0	0
48	527.205	956.397	-4405.5	0	0
49	531.533	959.201	-4014.95	0	0
50	535.86	962.069	-2978.32	0	0
51	540.188	965	0	0	0

List Of Coordinates

External Boundary



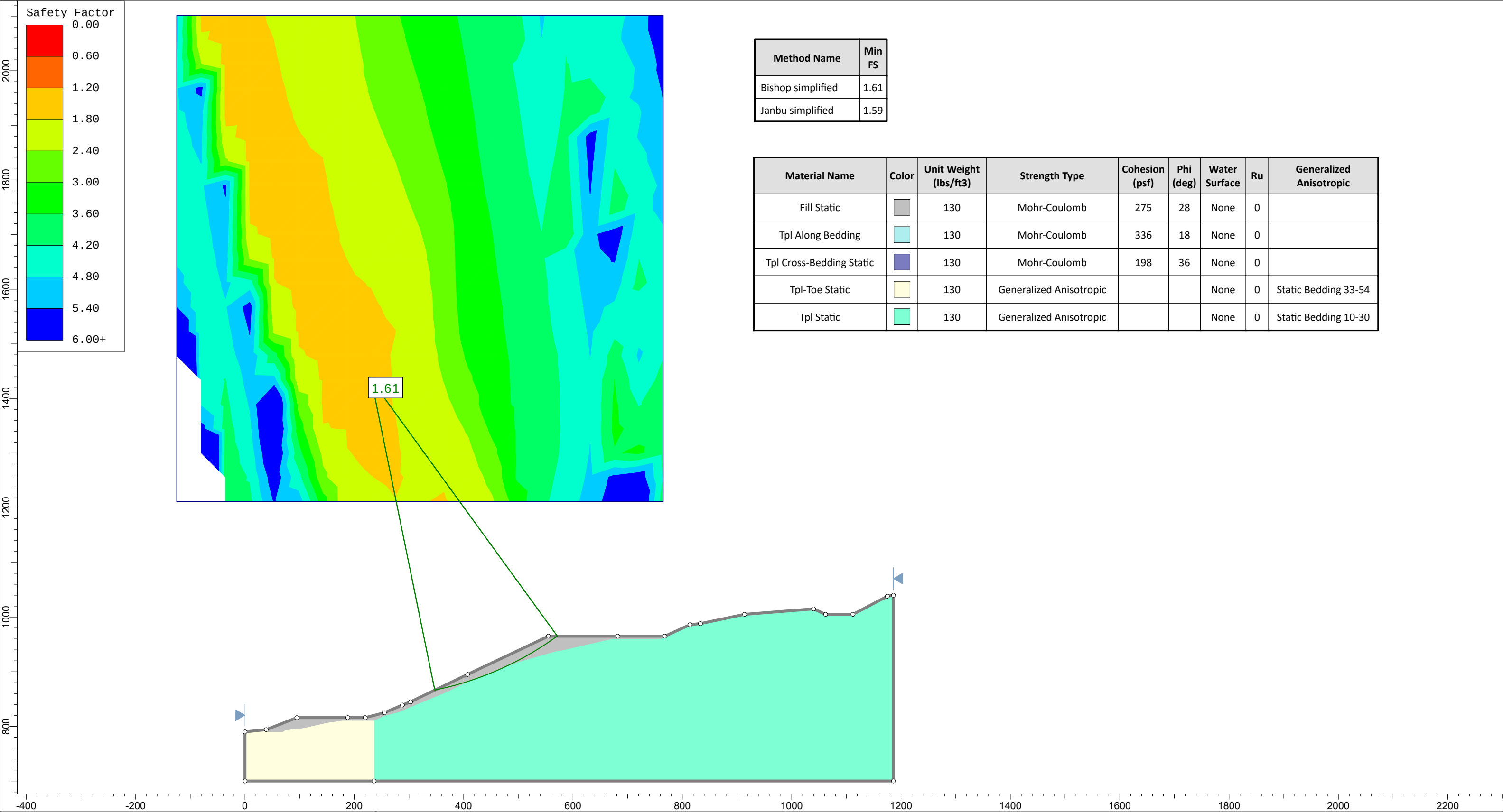
X	Y
0	700
283	700
1255	700
1255	995
1092	1000
967	1000
888	996
869	1005
865	1005
849	1000
814	988
750	986
706	965
633	965
506	965
456	940
429	930
376	900.5
361	905
308	871
295	863.5
282	863.5
271	865
253	856
227	843.1
170	816
75	813.9
44	810
0	800

Material Boundary

X	Y
75	813.9
81	811
195	811
204	820
242	840
283	858.5
320	858.5
324.266	863.03
333	871
353	881
400	897
463	917
536	937
625	960
698	960
706	965

Material Boundary

X	Y
283	700
283	858.5



Method Name	Min FS
Bishop simplified	1.61
Janbu simplified	1.59

Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (deg)	Water Surface	Ru	Generalized Anisotropic
Fill Static		130	Mohr-Coulomb	275	28	None	0	
Tpl Along Bedding		130	Mohr-Coulomb	336	18	None	0	
Tpl Cross-Bedding Static		130	Mohr-Coulomb	198	36	None	0	
Tpl-Toe Static		130	Generalized Anisotropic			None	0	Static Bedding 33-54
Tpl Static		130	Generalized Anisotropic			None	0	Static Bedding 10-30

Slide Analysis Information

HOFF Property, Yorba Linda

Project Summary

File Name: Cross-section C-C' Static
 Slide Modeler Version: 7.038
 Project Title: HOFF Property, Yorba Linda
 Analysis: Cross-section C-C' Static
 Company: TGR Geotechnical, Inc.
 Date Created: 4/15/20

General Settings

Units of Measurement: Imperial Units
 Time Units: days
 Permeability Units: feet/second
 Failure Direction: Right to Left
 Data Output: Standard
 Maximum Material Properties: 20
 Maximum Support Properties: 20

Analysis Options

Slices Type: Vertical

Analysis Methods Used

Bishop simplified
 Janbu simplified

Number of slices: 50
 Tolerance: 0.005
 Maximum number of iterations: 75
 Check $m\alpha < 0.2$: Yes
 Create Interslice boundaries at intersections with water tables and piezos: Yes
 Initial trial value of FS: 1
 Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
 Pore Fluid Unit Weight [lbs/ft³]: 62.4
 Use negative pore pressure cutoff: Yes
 Maximum negative pore pressure [psf]: 0
 Advanced Groundwater Method: None

Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3

Surface Options

Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined
Minimum Area: Not Defined
Minimum Weight: Not Defined

Seismic

Advanced seismic analysis: No
Staged pseudostatic analysis: No

Material Properties

Property	Fill Static	Tpl-Toe Static	Tpl Static
Color			
Strength Type	Mohr-Coulomb	Generalized Anisotropic	Generalized Anisotropic
Unit Weight [lbs/ft3]	130	130	130
Cohesion [psf]	275		
Friction Angle [deg]	28		
Water Surface	None	None	None
Ru Value	0	0	0

Generalized Anisotropic Functions

Name: Static Bedding 33-54

Angle From	Angle To	Material
33	-90	Tpl Cross-Bedding Static
54	33	Tpl Along Bedding
90	54	Tpl Cross-Bedding Static

Name: Static Bedding 10-30

Angle From	Angle To	Material
-30	-90	Tpl Cross-Bedding Static
-10	-30	Tpl Along Bedding
90	-10	Tpl Cross-Bedding Static

Global Minimums

Method: bishop simplified

FS	1.612850
Center:	231.114, 1433.951
Radius:	579.453
Left Slip Surface Endpoint:	347.182, 866.242
Right Slip Surface Endpoint:	571.483, 965.000
Resisting Moment:	1.49473e+008 lb-ft
Driving Moment:	9.26766e+007 lb-ft
Total Slice Area:	2993.65 ft2
Surface Horizontal Width:	224.3 ft
Surface Average Height:	13.3466 ft

Method: janbu simplified

FS	1.593630
Center:	231.114, 1433.951
Radius:	579.453
Left Slip Surface Endpoint:	347.182, 866.242
Right Slip Surface Endpoint:	571.483, 965.000
Resisting Horizontal Force:	233586 lb
Driving Horizontal Force:	146575 lb
Total Slice Area:	2993.65 ft2
Surface Horizontal Width:	224.3 ft
Surface Average Height:	13.3466 ft

Valid / Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4640
Number of Invalid Surfaces: 211

Error Codes:

Error Code -103 reported for 89 surfaces
Error Code -106 reported for 43 surfaces
Error Code -108 reported for 2 surfaces
Error Code -1000 reported for 77 surfaces

Method: janbu simplified

Number of Valid Surfaces: 4640
Number of Invalid Surfaces: 211

Error Codes:

Error Code -103 reported for 89 surfaces
Error Code -106 reported for 43 surfaces
Error Code -108 reported for 2 surfaces
Error Code -1000 reported for 77 surfaces

Error Codes

The following errors were encountered during the computation:

- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 106 = Average slice width is less than 0.0001 * (maximum horizontal extent of soil region). This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

Slice Data

Global Minimum Query (bishop simplified) - Safety Factor: 1.61285

Slice Number	Width [ft]	Weight [lbs]	Angle of Slice Base [degrees]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]	Base Vertical Stress [psf]	Effective Vertical Stress [psf]
1	4.48601	356.054	11.7814	Fill Static	275	28	184.027	296.807	41.0137	0	41.0137	79.3966	79.3966
2	4.48601	1057.34	12.2349	Fill Static	275	28	231.659	373.631	185.498	0	185.498	235.732	235.732
3	4.48601	1736.92	12.6892	Fill Static	275	28	277.561	447.664	324.734	0	324.734	387.23	387.23
4	4.48601	2394.7	13.1443	Fill Static	275	28	321.737	518.913	458.734	0	458.734	533.867	533.867
5	4.48601	3030.53	13.6003	Fill Static	275	28	364.189	587.382	587.506	0	587.506	675.615	675.615
6	4.48601	3644.31	14.0571	Fill Static	275	28	404.92	653.076	711.058	0	711.058	812.444	812.444
7	4.48601	4235.9	14.5148	Fill Static	275	28	443.933	715.998	829.395	0	829.395	944.326	944.326
8	4.48601	4805.16	14.9735	Fill Static	275	28	481.23	776.151	942.528	0	942.528	1071.23	1071.23
9	4.48601	5351.96	15.4332	Fill Static	275	28	516.811	833.539	1050.46	0	1050.46	1193.13	1193.13
10	4.48601	5876.15	15.8938	Fill Static	275	28	550.679	888.163	1153.19	0	1153.19	1309.99	1309.99
11	4.48601	6377.56	16.3556	Fill Static	275	28	582.835	940.025	1250.73	0	1250.73	1421.78	1421.78
12	4.48601	6856.05	16.8184	Fill Static	275	28	613.278	989.125	1343.07	0	1343.07	1528.45	1528.45
13	4.48601	7311.45	17.2824	Fill Static	275	28	642.009	1035.47	1430.23	0	1430.23	1629.97	1629.97
14	4.48601	7739.06	17.7475	Fill Static	275	28	668.729	1078.56	1511.27	0	1511.27	1725.3	1725.3
15	4.48601	8128.48	18.2139	Fill Static	275	28	692.76	1117.32	1584.17	0	1584.17	1812.12	1812.12
16	4.48601	8493.14	18.6815	Fill Static	275	28	715.009	1153.2	1651.66	0	1651.66	1893.42	1893.42
17	4.48601	8833.97	19.1504	Fill Static	275	28	735.553	1186.34	1713.97	0	1713.97	1969.41	1969.41
18	4.48601	9150.77	19.6206	Fill Static	275	28	754.387	1216.71	1771.1	0	1771.1	2040.04	2040.04
19	4.48601	9443.34	20.0923	Fill Static	275	28	771.511	1244.33	1823.05	0	1823.05	2105.26	2105.26
20	4.48601	9711.45	20.5653	Fill Static	275	28	786.923	1269.19	1869.8	0	1869.8	2165.04	2165.04
21	4.48601	9954.89	21.0398	Fill Static	275	28	800.62	1291.28	1911.34	0	1911.34	2219.31	2219.31
22	4.48601	10173.4	21.5158	Fill Static	275	28	812.599	1310.6	1947.68	0	1947.68	2268.03	2268.03
23	4.48601	10366.8	21.9934	Fill Static	275	28	822.86	1327.15	1978.8	0	1978.8	2311.14	2311.14
24	4.48601	10534.7	22.4726	Fill Static	275	28	831.392	1340.91	2004.68	0	2004.68	2348.59	2348.59
25	4.48601	10677	22.9535	Fill Static	275	28	838.199	1351.89	2025.33	0	2025.33	2380.32	2380.32
26	4.48601	10793.4	23.4361	Fill Static	275	28	843.271	1360.07	2040.71	0	2040.71	2406.26	2406.26
27	4.48601	10883.5	23.9204	Fill Static	275	28	846.607	1365.45	2050.83	0	2050.83	2426.36	2426.36
28	4.48601	10947.1	24.4066	Fill Static	275	28	848.194	1368.01	2055.67	0	2055.67	2440.54	2440.54

29	4.48601	10983.8	24.8947	Fill Static	275	28	848.039	1367.76	2055.19	0	2055.19	2448.74	2448.74
30	4.48601	10993.5	25.3847	Fill Static	275	28	846.136	1364.69	2049.39	0	2049.39	2450.89	2450.89
31	4.48601	10975.7	25.8767	Fill Static	275	28	842.465	1358.77	2038.28	0	2038.28	2446.94	2446.94
32	4.48601	10930	26.3707	Fill Static	275	28	837.028	1350	2021.79	0	2021.79	2436.76	2436.76
33	4.48601	10856.2	26.8669	Fill Static	275	28	829.823	1338.38	1999.91	0	1999.91	2420.3	2420.3
34	4.48601	10753.9	27.3652	Fill Static	275	28	820.833	1323.88	1972.65	0	1972.65	2397.5	2397.5
35	4.48601	10622.6	27.8659	Fill Static	275	28	810.051	1306.49	1939.96	0	1939.96	2368.24	2368.24
36	4.48601	10462	28.3688	Fill Static	275	28	797.477	1286.21	1901.81	0	1901.81	2332.45	2332.45
37	4.48601	10271.7	28.8741	Fill Static	275	28	783.097	1263.02	1858.19	0	1858.19	2290.02	2290.02
38	4.48601	10051.2	29.3819	Fill Static	275	28	766.902	1236.9	1809.07	0	1809.07	2240.88	2240.88
39	4.48601	9800.14	29.8923	Fill Static	275	28	748.884	1207.84	1754.41	0	1754.41	2184.9	2184.9
40	4.48601	9517.97	30.4052	Fill Static	275	28	729.031	1175.82	1694.19	0	1694.19	2122	2122
41	4.48601	9204.21	30.9209	Fill Static	275	28	707.334	1140.82	1628.38	0	1628.38	2052.06	2052.06
42	4.48601	8858.38	31.4394	Fill Static	275	28	683.781	1102.84	1556.93	0	1556.93	1974.96	1974.96
43	4.48601	8479.93	31.9608	Fill Static	275	28	658.362	1061.84	1479.83	0	1479.83	1890.59	1890.59
44	4.48601	8068.31	32.4851	Fill Static	275	28	631.065	1017.81	1397.03	0	1397.03	1798.83	1798.83
45	4.48601	7622.94	33.0125	Fill Static	275	28	601.877	970.737	1308.49	0	1308.49	1699.54	1699.54
46	4.48601	7143.21	33.5431	Fill Static	275	28	570.786	920.592	1214.18	0	1214.18	1592.59	1592.59
47	4.48601	6347.22	34.077	Fill Static	275	28	520.875	840.093	1062.79	0	1062.79	1415.14	1415.14
48	4.48601	4625.13	34.6142	Fill Static	275	28	415.843	670.693	744.189	0	744.189	1031.21	1031.21
49	4.48601	2801.06	35.155	Fill Static	275	28	305.479	492.691	409.418	0	409.418	624.55	624.55
50	4.48601	939.927	35.6994	Fill Static	275	28	193.721	312.444	70.4213	0	70.4213	209.621	209.621

Global Minimum Query (janbu simplified) - Safety Factor: 1.59363

Slice Number	Width [ft]	Weight [lbs]	Angle of Slice Base [degrees]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]	Base Vertical Stress [psf]	Effective Vertical Stress [psf]
1	4.48601	356.054	11.7814	Fill Static	275	28	186.107	296.586	40.5975	0	40.5975	79.4144	79.4144
2	4.48601	1057.34	12.2349	Fill Static	275	28	234.272	373.342	184.955	0	184.955	235.756	235.756
3	4.48601	1736.92	12.6892	Fill Static	275	28	280.684	447.306	324.06	0	324.06	387.259	387.259
4	4.48601	2394.7	13.1443	Fill Static	275	28	325.347	518.483	457.926	0	457.926	533.902	533.902
5	4.48601	3030.53	13.6003	Fill Static	275	28	368.266	586.879	586.559	0	586.559	675.654	675.654
6	4.48601	3644.31	14.0571	Fill Static	275	28	409.442	652.499	709.972	0	709.972	812.491	812.491
7	4.48601	4235.9	14.5148	Fill Static	275	28	448.878	715.345	828.169	0	828.169	944.381	944.381
8	4.48601	4805.16	14.9735	Fill	275	28	486.576	775.422	941.159	0	941.159	1071.29	1071.29

9	4.48601	5351.96	15.4332	Static Fill	275	28	522.538	832.733	1048.94	0	1048.94	1193.2	1193.2
10	4.48601	5876.15	15.8938	Static Fill	275	28	556.767	887.28	1151.53	0	1151.53	1310.07	1310.07
11	4.48601	6377.56	16.3556	Static Fill	275	28	589.261	939.064	1248.92	0	1248.92	1421.85	1421.85
12	4.48601	6856.05	16.8184	Static Fill	275	28	620.023	988.087	1341.12	0	1341.12	1528.53	1528.53
13	4.48601	7311.45	17.2824	Static Fill	275	28	649.052	1034.35	1428.13	0	1428.13	1630.07	1630.07
14	4.48601	7739.06	17.7475	Static Fill	275	28	676.045	1077.37	1509.03	0	1509.03	1725.4	1725.4
15	4.48601	8128.48	18.2139	Static Fill	275	28	700.32	1116.05	1581.79	0	1581.79	1812.23	1812.23
16	4.48601	8493.14	18.6815	Static Fill	275	28	722.793	1151.86	1649.14	0	1649.14	1893.53	1893.53
17	4.48601	8833.97	19.1504	Static Fill	275	28	743.538	1184.92	1711.32	0	1711.32	1969.52	1969.52
18	4.48601	9150.77	19.6206	Static Fill	275	28	762.555	1215.23	1768.32	0	1768.32	2040.16	2040.16
19	4.48601	9443.34	20.0923	Static Fill	275	28	779.843	1242.78	1820.13	0	1820.13	2105.39	2105.39
20	4.48601	9711.45	20.5653	Static Fill	275	28	795.398	1267.57	1866.75	0	1866.75	2165.17	2165.17
21	4.48601	9954.89	21.0398	Static Fill	275	28	809.222	1289.6	1908.18	0	1908.18	2219.46	2219.46
22	4.48601	10173.4	21.5158	Static Fill	275	28	821.301	1308.85	1944.4	0	1944.4	2268.18	2268.18
23	4.48601	10366.8	21.9934	Static Fill	275	28	831.649	1325.34	1975.4	0	1975.4	2311.3	2311.3
24	4.48601	10534.7	22.4726	Static Fill	275	28	840.252	1339.05	2001.18	0	2001.18	2348.75	2348.75
25	4.48601	10677	22.9535	Static Fill	275	28	847.104	1349.97	2021.72	0	2021.72	2380.48	2380.48
26	4.48601	10793.4	23.4361	Static Fill	275	28	852.205	1358.1	2037.01	0	2037.01	2406.43	2406.43
27	4.48601	10883.5	23.9204	Static Fill	275	28	855.55	1363.43	2047.04	0	2047.04	2426.54	2426.54
28	4.48601	10947.1	24.4066	Static Fill	275	28	857.131	1365.95	2051.79	0	2051.79	2440.72	2440.72
29	4.48601	10983.8	24.8947	Static Fill	275	28	856.949	1365.66	2051.25	0	2051.25	2448.93	2448.93
30	4.48601	10993.5	25.3847	Static Fill	275	28	854.998	1362.55	2045.39	0	2045.39	2451.09	2451.09
31	4.48601	10975.7	25.8767	Static Fill	275	28	851.264	1356.6	2034.19	0	2034.19	2447.11	2447.11
32	4.48601	10930	26.3707	Static Fill	275	28	845.748	1347.81	2017.66	0	2017.66	2436.95	2436.95
33	4.48601	10856.2	26.8669	Static Fill	275	28	838.438	1336.16	1995.75	0	1995.75	2420.51	2420.51
34	4.48601	10753.9	27.3652	Static Fill	275	28	829.327	1321.64	1968.45	0	1968.45	2397.69	2397.69
35	4.48601	10622.6	27.8659	Static Fill	275	28	818.415	1304.25	1935.73	0	1935.73	2368.44	2368.44
36	4.48601	10462	28.3688	Static Fill	275	28	805.683	1283.96	1897.58	0	1897.58	2332.64	2332.64
37	4.48601	10271.7	28.8741	Static Fill	275	28	791.13	1260.77	1853.96	0	1853.96	2290.22	2290.22
38	4.48601	10051.2	29.3819	Static Fill	275	28	774.745	1234.66	1804.85	0	1804.85	2241.08	2241.08
39	4.48601	9800.14	29.8923	Static Fill	275	28	756.518	1205.61	1750.22	0	1750.22	2185.1	2185.1
40	4.48601	9517.97	30.4052	Static Fill	275	28	736.438	1173.61	1690.04	0	1690.04	2122.2	2122.2

41	4.48601	9204.21	30.9209	Fill Static	275	28	714.498	1138.65	1624.28	0	1624.28	2052.25	2052.25
42	4.48601	8858.38	31.4394	Fill Static	275	28	690.684	1100.7	1552.91	0	1552.91	1975.15	1975.15
43	4.48601	8479.93	31.9608	Fill Static	275	28	664.986	1059.74	1475.89	0	1475.89	1890.78	1890.78
44	4.48601	8068.31	32.4851	Fill Static	275	28	637.393	1015.77	1393.18	0	1393.18	1799.02	1799.02
45	4.48601	7622.94	33.0125	Fill Static	275	28	607.892	968.755	1304.76	0	1304.76	1699.72	1699.72
46	4.48601	7143.21	33.5431	Fill Static	275	28	576.471	918.681	1210.59	0	1210.59	1592.77	1592.77
47	4.48601	6347.22	34.077	Fill Static	275	28	526.044	838.32	1059.45	0	1059.45	1415.3	1415.3
48	4.48601	4625.13	34.6142	Fill Static	275	28	419.956	669.254	741.483	0	741.483	1031.35	1031.35
49	4.48601	2801.06	35.155	Fill Static	275	28	308.489	491.617	407.397	0	407.397	624.649	624.649
50	4.48601	939.927	35.6994	Fill Static	275	28	195.623	311.751	69.119	0	69.119	209.685	209.685

Interslice Data

Global Minimum Query (bishop simplified) - Safety Factor: 1.61285



Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	347.182	866.242	0	0	0
2	351.668	867.177	786.595	0	0
3	356.154	868.15	1644.65	0	0
4	360.64	869.16	2560.91	0	0
5	365.126	870.208	3522.66	0	0
6	369.612	871.293	4517.65	0	0
7	374.099	872.416	5534.18	0	0
8	378.585	873.578	6561.02	0	0
9	383.071	874.777	7587.47	0	0
10	387.557	876.016	8603.34	0	0
11	392.043	877.293	9598.94	0	0
12	396.529	878.61	10565.1	0	0
13	401.015	879.966	11493.2	0	0
14	405.501	881.361	12375	0	0
15	409.987	882.797	13203	0	0
16	414.473	884.273	13970.1	0	0
17	418.959	885.79	14670.2	0	0
18	423.445	887.348	15297.5	0	0
19	427.931	888.947	15846.9	0	0
20	432.417	890.588	16314	0	0
21	436.903	892.271	16694.6	0	0
22	441.389	893.997	16985.5	0	0
23	445.875	895.765	17183.8	0	0
24	450.361	897.577	17287.2	0	0
25	454.847	899.433	17294.3	0	0
26	459.333	901.333	17203.9	0	0
27	463.819	903.277	17015.7	0	0
28	468.305	905.267	16730.1	0	0
29	472.791	907.303	16348.1	0	0
30	477.277	909.385	15871.2	0	0
31	481.763	911.513	15301.8	0	0
32	486.249	913.689	14643.2	0	0
33	490.735	915.913	13899	0	0
34	495.221	918.186	13073.9	0	0
35	499.707	920.508	12173.4	0	0
36	504.193	922.88	11203.5	0	0
37	508.679	925.302	10171.5	0	0
38	513.165	927.776	9085.35	0	0
39	517.651	930.302	7953.81	0	0
40	522.137	932.88	6786.75	0	0
41	526.623	935.513	5594.99	0	0
42	531.109	938.2	4390.37	0	0
43	535.595	940.942	3185.77	0	0
44	540.081	943.741	1995.24	0	0
45	544.567	946.598	833.949	0	0
46	549.053	949.512	-281.693	0	0
47	553.539	952.486	-1334	0	0
48	558.025	955.521	-2224.14	0	0
49	562.511	958.617	-2664.23	0	0
50	566.997	961.777	-2588.26	0	0
51	571.483	965	0	0	0

Global Minimum Query (janbu simplified) - Safety Factor: 1.59363

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	347.182	866.242	0	0	0
2	351.668	867.177	795.936	0	0
3	356.154	868.15	1665.76	0	0
4	360.64	869.16	2596.13	0	0
5	365.126	870.208	3574.26	0	0
6	369.612	871.293	4587.81	0	0
7	374.099	872.416	5625	0	0
8	378.585	873.578	6674.53	0	0
9	383.071	874.777	7725.61	0	0
10	387.557	876.016	8767.97	0	0
11	392.043	877.293	9791.86	0	0
12	396.529	878.61	10788	0	0
13	401.015	879.966	11747.7	0	0
14	405.501	881.361	12662.8	0	0
15	409.987	882.797	13525.4	0	0
16	414.473	884.273	14328.5	0	0
17	418.959	885.79	15065.8	0	0
18	423.445	887.348	15731.6	0	0
19	427.931	888.947	16320.5	0	0
20	432.417	890.588	16828.1	0	0
21	436.903	892.271	17250.3	0	0
22	441.389	893.997	17583.6	0	0
23	445.875	895.765	17825	0	0
24	450.361	897.577	17972.4	0	0
25	454.847	899.433	18023.9	0	0
26	459.333	901.333	17978.6	0	0
27	463.819	903.277	17836	0	0
28	468.305	905.267	17596.3	0	0
29	472.791	907.303	17260.5	0	0
30	477.277	909.385	16830	0	0
31	481.763	911.513	16307.3	0	0
32	486.249	913.689	15695.2	0	0
33	490.735	915.913	14997.6	0	0
34	495.221	918.186	14218.9	0	0
35	499.707	920.508	13364.5	0	0
36	504.193	922.88	12440.5	0	0
37	508.679	925.302	11454	0	0
38	513.165	927.776	10412.7	0	0
39	517.651	930.302	9325.35	0	0
40	522.137	932.88	8201.8	0	0
41	526.623	935.513	7052.69	0	0
42	531.109	938.2	5889.74	0	0
43	535.595	940.942	4725.75	0	0
44	540.081	943.741	3574.61	0	0
45	544.567	946.598	2451.38	0	0
46	549.053	949.512	1372.34	0	0
47	553.539	952.486	355.044	0	0
48	558.025	955.521	-502.862	0	0
49	562.511	958.617	-916.979	0	0
50	566.997	961.777	-821.754	0	0
51	571.483	965	0	0	0

List Of Coordinates

External Boundary



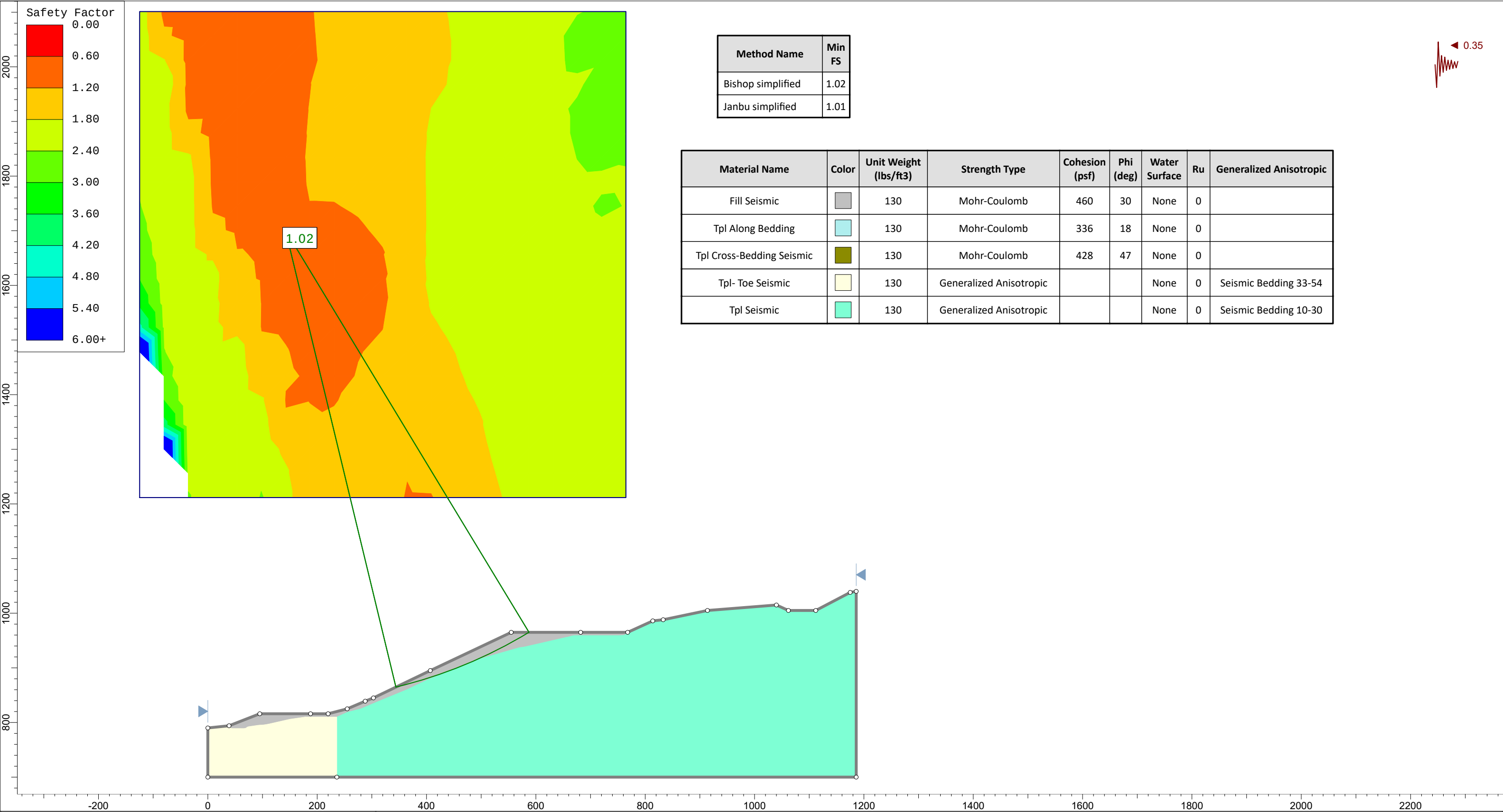
X	Y
0	700
236	700
1186	700
1186	1040
1175	1038
1112	1005
1062	1005
1040	1015
914	1005
833	988
814	986
768	965
682	965
555	965
407	895
303	845
288	839
255	825
220	816
188	816
95	816
39	794
0	790

Material Boundary

X	Y
39	794
44	789
67	789
73	793
93	796
111	797
150	806
178	810
236	810
256	819
280	825
313	839
364	860
404	879
449	897
500	917
570	937
657	957
669	960
763	960
768	965

Material Boundary

X	Y
236	700
236	810



Slide Analysis Information

HOFF Property, Yorba Linda

Project Summary

File Name: Cross-section C-C' Seismic
 Slide Modeler Version: 7.038
 Project Title: HOFF Property, Yorba Linda
 Analysis: Cross-section C-C' Seismic
 Company: TGR Geotechnical, Inc.
 Date Created: 4/15/20

General Settings

Units of Measurement: Imperial Units
 Time Units: days
 Permeability Units: feet/second
 Failure Direction: Right to Left
 Data Output: Standard
 Maximum Material Properties: 20
 Maximum Support Properties: 20

Analysis Options

Slices Type: Vertical

Analysis Methods Used

Bishop simplified
 Janbu simplified

Number of slices: 50
 Tolerance: 0.005
 Maximum number of iterations: 75
 Check $\alpha < 0.2$: Yes
 Create Interslice boundaries at intersections with water tables and piezos: Yes
 Initial trial value of FS: 1
 Steffensen Iteration: Yes

Groundwater Analysis

Groundwater Method: Water Surfaces
 Pore Fluid Unit Weight [lbs/ft³]: 62.4
 Use negative pore pressure cutoff: Yes
 Maximum negative pore pressure [psf]: 0
 Advanced Groundwater Method: None

Random Numbers

Pseudo-random Seed: 10116
 Random Number Generation Method: Park and Miller v.3

Surface Options

Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined
Minimum Area: Not Defined
Minimum Weight: Not Defined

Seismic

Advanced seismic analysis: No
Staged pseudostatic analysis: No

Loading

Seismic Load Coefficient (Horizontal): 0.35

Material Properties

Property	Fill Seismic	Tpl- Toe Seismic	Tpl Seismic
Color			
Strength Type	Mohr-Coulomb	Generalized Anisotropic	Generalized Anisotropic
Unit Weight [lbs/ft3]	130	130	130
Cohesion [psf]	460		
Friction Angle [deg]	30		
Water Surface	None	None	None
Ru Value	0	0	0

Generalized Anisotropic Functions

Name: Seismic Bedding 33-54

Angle From	Angle To	Material
33	-90	Tpl Cross-Bedding Seismic
54	33	Tpl Along Bedding
90	54	Tpl Cross-Bedding Seismic

Name: Seismic Bedding 10-30

Angle From	Angle To	Material
-30	-90	Tpl Cross-Bedding Seismic
-10	-30	Tpl Along Bedding
90	-10	Tpl Cross-Bedding Seismic

Global Minimums

Method: bishop simplified

FS	1.022610
Center:	142.164, 1700.801
Radius:	860.104
Left Slip Surface Endpoint:	344.011, 864.717
Right Slip Surface Endpoint:	587.558, 965.000
Resisting Moment:	2.80396e+008 lb-ft
Driving Moment:	2.74198e+008 lb-ft
Total Slice Area:	3451.05 ft ²
Surface Horizontal Width:	243.548 ft
Surface Average Height:	14.1699 ft

Method: janbu simplified

FS	1.008500
Center:	231.114, 1433.951
Radius:	579.453
Left Slip Surface Endpoint:	347.182, 866.242
Right Slip Surface Endpoint:	571.483, 965.000
Resisting Horizontal Force:	261244 lb
Driving Horizontal Force:	259042 lb
Total Slice Area:	2993.65 ft ²
Surface Horizontal Width:	224.3 ft
Surface Average Height:	13.3466 ft

Valid / Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4642
Number of Invalid Surfaces: 209

Error Codes:

Error Code -103 reported for 89 surfaces
Error Code -106 reported for 43 surfaces
Error Code -1000 reported for 77 surfaces

Method: janbu simplified

Number of Valid Surfaces: 4640
Number of Invalid Surfaces: 211

Error Codes:

Error Code -103 reported for 89 surfaces
Error Code -106 reported for 43 surfaces
Error Code -108 reported for 2 surfaces
Error Code -1000 reported for 77 surfaces

Error Codes

The following errors were encountered during the computation:

- 103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- 106 = Average slice width is less than 0.0001 * (maximum horizontal extent of soil region). This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.
- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 1000 = No valid slip surfaces are generated at a grid center. Unable to draw a surface.

Slice Data

Global Minimum Query (bishop simplified) - Safety Factor: 1.02261

Slice Number	Width [ft]	Weight [lbs]	Angle of Slice Base [degrees]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]	Base Vertical Stress [psf]	Effective Vertical Stress [psf]
1	4.87095	364.365	13.7396	Fill Seismic	460	30	432.184	441.956	-31.253	0	-31.253	74.4187	74.4187
2	4.87095	1083.55	14.0739	Fill Seismic	460	30	503.847	515.239	95.6772	0	95.6772	221.991	221.991
3	4.87095	1783.59	14.4086	Fill Seismic	460	30	573.127	586.085	218.385	0	218.385	365.631	365.631
4	4.87095	2464.4	14.7439	Fill Seismic	460	30	640.033	654.504	336.891	0	336.891	505.325	505.325
5	4.87095	3125.9	15.0797	Fill Seismic	460	30	704.577	720.507	451.211	0	451.211	641.053	641.053
6	4.87095	3768	15.416	Fill Seismic	460	30	766.766	784.103	561.362	0	561.362	772.796	772.796
7	4.87095	4390.6	15.7529	Fill Seismic	460	30	826.611	845.301	667.361	0	667.361	900.535	900.535
8	4.87095	4993.6	16.0903	Fill Seismic	460	30	884.122	904.112	769.225	0	769.225	1024.25	1024.25
9	4.87095	5576.92	16.4283	Fill Seismic	460	30	939.306	960.544	866.968	0	866.968	1143.92	1143.92
10	4.87095	6140.43	16.7669	Fill Seismic	460	30	992.173	1014.61	960.606	0	960.606	1259.54	1259.54
11	4.87095	6684.05	17.1061	Fill Seismic	460	30	1042.73	1066.3	1050.15	0	1050.15	1371.06	1371.06
12	4.87095	7207.66	17.4459	Fill Seismic	460	30	1090.98	1115.65	1135.62	0	1135.62	1478.47	1478.47
13	4.87095	7711.1	17.7863	Fill Seismic	460	30	1136.94	1162.65	1217.02	0	1217.02	1581.75	1581.75
14	4.87095	8180.74	18.1274	Fill Seismic	460	30	1179.28	1205.95	1292.02	0	1292.02	1678.09	1678.09
15	4.87095	8619.6	18.4692	Fill Seismic	460	30	1218.34	1245.88	1361.19	0	1361.19	1768.11	1768.11
16	4.87095	9037.99	18.8116	Fill Seismic	460	30	1255.13	1283.51	1426.36	0	1426.36	1853.93	1853.93
17	4.87095	9435.78	19.1548	Fill Seismic	460	30	1289.68	1318.84	1487.55	0	1487.55	1935.52	1935.52
18	4.87095	9812.86	19.4986	Fill Seismic	460	30	1321.98	1351.87	1544.76	0	1544.76	2012.86	2012.86
19	4.87095	10169.1	19.8432	Fill Seismic	460	30	1352.04	1382.61	1598	0	1598	2085.92	2085.92
20	4.87095	10504.3	20.1886	Fill Seismic	460	30	1379.87	1411.06	1647.29	0	1647.29	2154.67	2154.67
21	4.87095	10818.4	20.5347	Fill Seismic	460	30	1405.47	1437.24	1692.63	0	1692.63	2219.09	2219.09
22	4.87095	11111.2	20.8816	Fill Seismic	460	30	1428.84	1461.15	1734.04	0	1734.04	2279.14	2279.14
23	4.87095	11382.6	21.2293	Fill Seismic	460	30	1450.01	1482.79	1771.52	0	1771.52	2334.79	2334.79
24	4.87095	11632.5	21.5778	Fill Seismic	460	30	1468.95	1502.16	1805.08	0	1805.08	2386.02	2386.02
25	4.87095	11860.6	21.9271	Fill Seismic	460	30	1485.69	1519.28	1834.73	0	1834.73	2432.79	2432.79
26	4.87095	12066.8	22.2774	Fill Seismic	460	30	1500.22	1534.14	1860.47	0	1860.47	2475.06	2475.06
27	4.87095	12251	22.6285	Fill Seismic	460	30	1512.56	1546.76	1882.32	0	1882.32	2512.82	2512.82
28	4.87095	12413	22.9804	Fill Seismic	460	30	1522.7	1557.13	1900.28	0	1900.28	2546.02	2546.02
29	4.87095	12552.6	23.3334	Fill	460	30	1530.65	1565.26	1914.36	0	1914.36	2574.62	2574.62

30	4.87095	12669.6	23.6872	Seismic Fill	460	30	1536.41	1571.15	1924.57	0	1924.57	2598.59	2598.59
31	4.87095	12763.9	24.042	Seismic Fill	460	30	1539.99	1574.81	1930.91	0	1930.91	2617.91	2617.91
32	4.87095	12835.3	24.3978	Seismic Fill	460	30	1541.39	1576.24	1933.39	0	1933.39	2632.53	2632.53
33	4.87095	12883.5	24.7546	Seismic Fill	460	30	1540.61	1575.44	1931.99	0	1931.99	2642.37	2642.37
34	4.87095	12908.4	25.1125	Seismic Fill	460	30	1537.64	1572.41	1926.75	0	1926.75	2647.44	2647.44
35	4.87095	12909.7	25.4714	Seismic Fill	460	30	1532.51	1567.16	1917.66	0	1917.66	2647.69	2647.69
36	4.87095	12887.4	25.8313	Seismic Fill	460	30	1525.21	1559.69	1904.73	0	1904.73	2643.07	2643.07
37	4.87095	12841	26.1924	Seismic Fill	460	30	1515.74	1550.01	1887.94	0	1887.94	2633.53	2633.53
38	4.87095	12770.5	26.5545	Seismic Fill	460	30	1504.09	1538.1	1867.32	0	1867.32	2619.02	2619.02
39	4.87095	12675.6	26.9179	Seismic Fill	460	30	1490.29	1523.99	1842.88	0	1842.88	2599.53	2599.53
40	4.87095	12556	27.2824	Seismic Fill	460	30	1474.32	1507.65	1814.59	0	1814.59	2574.97	2574.97
41	4.87095	12411.6	27.6481	Seismic Fill	460	30	1456.19	1489.11	1782.47	0	1782.47	2545.31	2545.31
42	4.87095	12242	28.015	Seismic Fill	460	30	1435.89	1468.36	1746.53	0	1746.53	2510.49	2510.49
43	4.87095	12047.1	28.3832	Seismic Fill	460	30	1413.44	1445.4	1706.76	0	1706.76	2470.47	2470.47
44	4.87095	11485	28.7526	Seismic Fill	460	30	1358.64	1389.36	1609.69	0	1609.69	2355.14	2355.14
45	4.87095	9852.41	29.1234	Seismic Fill	460	30	1209.86	1237.22	1346.18	0	1346.18	2020.23	2020.23
46	4.87095	8120.83	29.4955	Seismic Fill	460	30	1053.44	1077.26	1069.13	0	1069.13	1665.02	1665.02
47	4.87095	6362.76	29.869	Seismic Fill	460	30	895.809	916.063	789.923	0	789.923	1304.39	1304.39
48	4.87095	4577.9	30.2439	Seismic Fill	460	30	736.97	753.633	508.588	0	508.588	938.271	938.271
49	4.87095	2765.94	30.6202	Seismic Fill	460	30	576.934	589.978	225.128	0	225.128	566.601	566.601
50	4.87095	926.575	30.998	Seismic Fill	460	30	415.703	425.102	-60.4451	0	-60.4451	189.315	189.315

Global Minimum Query (janbu simplified) - Safety Factor: 1.0085

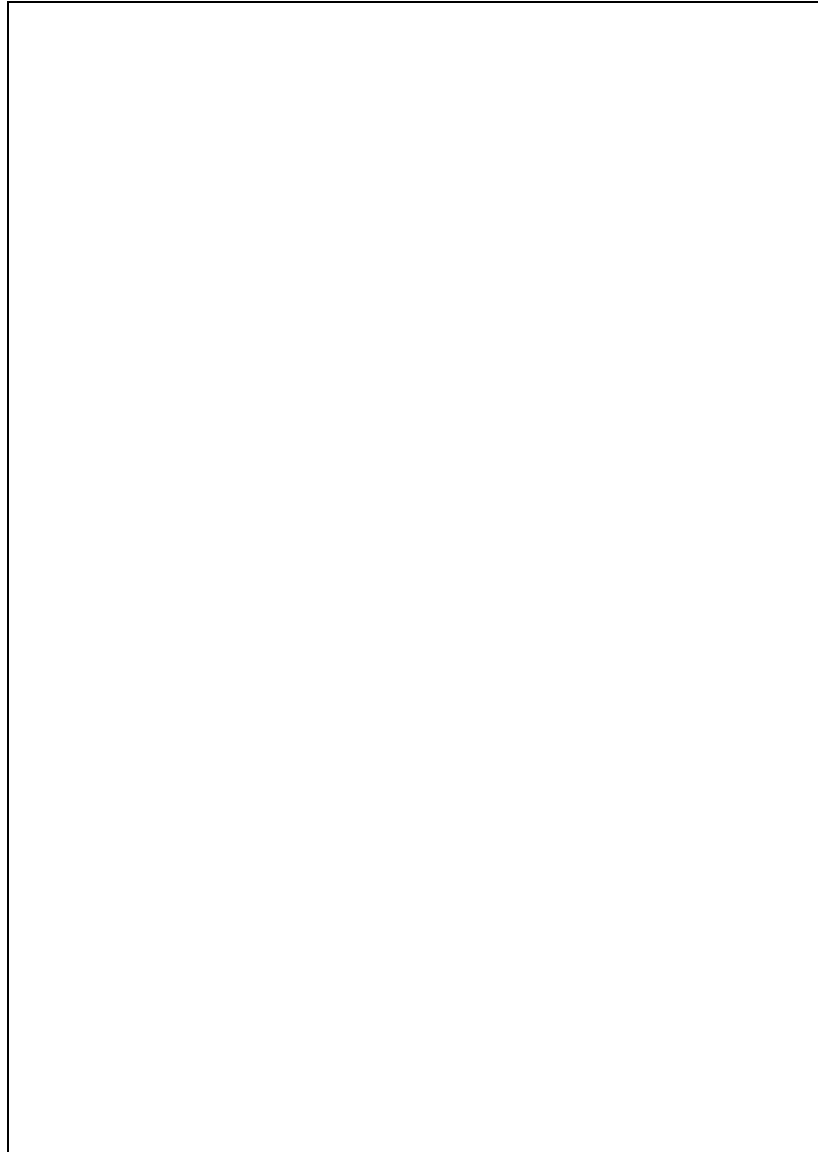
Slice Number	Width [ft]	Weight [lbs]	Angle of Slice Base [degrees]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]	Base Vertical Stress [psf]	Effective Vertical Stress [psf]
1	4.48601	356.054	11.7814	Seismic Fill	460	30	448.047	451.856	-14.1066	0	-14.1066	79.3439	79.3439
2	4.48601	1057.34	12.2349	Seismic Fill	460	30	525.768	530.237	121.655	0	121.655	235.665	235.665
3	4.48601	1736.92	12.6892	Seismic Fill	460	30	600.371	605.474	251.968	0	251.968	387.149	387.149
4	4.48601	2394.7	13.1443	Seismic Fill	460	30	671.876	677.587	376.873	0	376.873	533.771	533.771
5	4.48601	3030.53	13.6003	Seismic Fill	460	30	740.304	746.597	496.401	0	496.401	675.503	675.503
6	4.48601	3644.31	14.0571	Seismic Fill	460	30	805.675	812.523	610.586	0	610.586	812.316	812.316
7	4.48601	4235.9	14.5148	Seismic Fill	460	30	868.005	875.383	719.464	0	719.464	944.185	944.185
8	4.48601	4805.16	14.9735	Seismic Fill	460	30	927.314	935.196	823.063	0	823.063	1071.08	1071.08

9	4.48601	5351.96	15.4332	Fill Seismic	460	30	983.617	991.978	921.414	0	921.414	1192.96	1192.96
10	4.48601	5876.15	15.8938	Fill Seismic	460	30	1036.93	1045.75	1014.54	0	1014.54	1309.8	1309.8
11	4.48601	6377.56	16.3556	Fill Seismic	460	30	1087.28	1096.52	1102.48	0	1102.48	1421.57	1421.57
12	4.48601	6856.05	16.8184	Fill Seismic	460	30	1134.66	1144.3	1185.25	0	1185.25	1528.22	1528.22
13	4.48601	7311.45	17.2824	Fill Seismic	460	30	1179.1	1189.12	1262.88	0	1262.88	1629.73	1629.73
14	4.48601	7739.06	17.7475	Fill Seismic	460	30	1220.13	1230.5	1334.54	0	1334.54	1725.05	1725.05
15	4.48601	8128.48	18.2139	Fill Seismic	460	30	1256.65	1267.34	1398.34	0	1398.34	1811.85	1811.85
16	4.48601	8493.14	18.6815	Fill Seismic	460	30	1290.17	1301.14	1456.9	0	1456.9	1893.13	1893.13
17	4.48601	8833.97	19.1504	Fill Seismic	460	30	1320.82	1332.04	1510.43	0	1510.43	1969.1	1969.1
18	4.48601	9150.77	19.6206	Fill Seismic	460	30	1348.6	1360.06	1558.95	0	1558.95	2039.71	2039.71
19	4.48601	9443.34	20.0923	Fill Seismic	460	30	1373.53	1385.2	1602.5	0	1602.5	2104.93	2104.93
20	4.48601	9711.45	20.5653	Fill Seismic	460	30	1395.61	1407.47	1641.07	0	1641.07	2164.69	2164.69
21	4.48601	9954.89	21.0398	Fill Seismic	460	30	1414.86	1426.89	1674.7	0	1674.7	2218.95	2218.95
22	4.48601	10173.4	21.5158	Fill Seismic	460	30	1431.29	1443.46	1703.39	0	1703.39	2267.65	2267.65
23	4.48601	10366.8	21.9934	Fill Seismic	460	30	1444.9	1457.18	1727.17	0	1727.17	2310.75	2310.75
24	4.48601	10534.7	22.4726	Fill Seismic	460	30	1455.7	1468.07	1746.03	0	1746.03	2348.18	2348.18
25	4.48601	10677	22.9535	Fill Seismic	460	30	1463.7	1476.14	1760	0	1760	2379.9	2379.9
26	4.48601	10793.4	23.4361	Fill Seismic	460	30	1468.89	1481.38	1769.08	0	1769.08	2405.83	2405.83
27	4.48601	10883.5	23.9204	Fill Seismic	460	30	1471.3	1483.81	1773.29	0	1773.29	2425.91	2425.91
28	4.48601	10947.1	24.4066	Fill Seismic	460	30	1470.93	1483.43	1772.64	0	1772.64	2440.08	2440.08
29	4.48601	10983.8	24.8947	Fill Seismic	460	30	1467.77	1480.25	1767.13	0	1767.13	2448.28	2448.28
30	4.48601	10993.5	25.3847	Fill Seismic	460	30	1461.84	1474.27	1756.77	0	1756.77	2450.42	2450.42
31	4.48601	10975.7	25.8767	Fill Seismic	460	30	1453.15	1465.5	1741.57	0	1741.57	2446.45	2446.45
32	4.48601	10930	26.3707	Fill Seismic	460	30	1441.67	1453.93	1721.53	0	1721.53	2436.27	2436.27
33	4.48601	10856.2	26.8669	Fill Seismic	460	30	1427.44	1439.57	1696.67	0	1696.67	2419.81	2419.81
34	4.48601	10753.9	27.3652	Fill Seismic	460	30	1410.44	1422.43	1666.98	0	1666.98	2397	2397
35	4.48601	10622.6	27.8659	Fill Seismic	460	30	1390.69	1402.51	1632.47	0	1632.47	2367.74	2367.74
36	4.48601	10462	28.3688	Fill Seismic	460	30	1368.17	1379.8	1593.14	0	1593.14	2331.94	2331.94
37	4.48601	10271.7	28.8741	Fill Seismic	460	30	1342.9	1354.31	1548.99	0	1548.99	2289.52	2289.52
38	4.48601	10051.2	29.3819	Fill Seismic	460	30	1314.87	1326.04	1500.03	0	1500.03	2240.37	2240.37
39	4.48601	9800.14	29.8923	Fill Seismic	460	30	1284.08	1294.99	1446.25	0	1446.25	2184.4	2184.4
40	4.48601	9517.97	30.4052	Fill Seismic	460	30	1250.54	1261.17	1387.66	0	1387.66	2121.5	2121.5
41	4.48601	9204.21	30.9209	Fill	460	30	1214.24	1224.56	1324.25	0	1324.25	2051.56	2051.56

42	4.48601	8858.38	31.4394	Seismic Fill	460	30	1175.18	1185.17	1256.03	0	1256.03	1974.47	1974.47
43	4.48601	8479.93	31.9608	Seismic Fill	460	30	1133.36	1143	1182.98	0	1182.98	1890.11	1890.11
44	4.48601	8068.31	32.4851	Seismic Fill	460	30	1088.79	1098.04	1105.12	0	1105.12	1798.36	1798.36
45	4.48601	7622.94	33.0125	Seismic Fill	460	30	1041.45	1050.3	1022.43	0	1022.43	1699.08	1699.08
46	4.48601	7143.21	33.5431	Seismic Fill	460	30	991.348	999.774	934.918	0	934.918	1592.15	1592.15
47	4.48601	6347.22	34.077	Seismic Fill	460	30	912.607	920.364	797.374	0	797.374	1414.72	1414.72
48	4.48601	4625.13	34.6142	Seismic Fill	460	30	749.945	756.32	513.241	0	513.241	1030.87	1030.87
49	4.48601	2801.06	35.155	Seismic Fill	460	30	579.771	584.699	215.985	0	215.985	624.287	624.287
50	4.48601	939.927	35.6994	Seismic Fill	460	30	408.135	411.604	-83.8249	0	-83.8249	209.443	209.443

Interslice Data

Global Minimum Query (bishop simplified) - Safety Factor: 1.02261



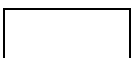
Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	344.011	864.717	0	0	0
2	348.882	865.908	2022.51	0	0
3	353.753	867.129	3989.59	0	0
4	358.624	868.38	5893.88	0	0
5	363.495	869.662	7728.42	0	0
6	368.365	870.975	9486.63	0	0
7	373.236	872.318	11162.3	0	0
8	378.107	873.692	12749.7	0	0
9	382.978	875.097	14243.4	0	0
10	387.849	876.533	15638.3	0	0
11	392.72	878.001	16929.8	0	0
12	397.591	879.5	18113.7	0	0
13	402.462	881.03	19186.2	0	0
14	407.333	882.593	20143.7	0	0
15	412.204	884.188	20985.3	0	0
16	417.075	885.814	21710	0	0
17	421.946	887.474	22315.9	0	0
18	426.817	889.166	22801.4	0	0
19	431.688	890.89	23165.3	0	0
20	436.559	892.648	23406.8	0	0
21	441.43	894.439	23525.6	0	0
22	446.301	896.264	23521.8	0	0
23	451.172	898.122	23395.8	0	0
24	456.043	900.014	23148.5	0	0
25	460.914	901.941	22781.1	0	0
26	465.784	903.901	22295.5	0	0
27	470.655	905.897	21693.7	0	0
28	475.526	907.927	20978.4	0	0
29	480.397	909.993	20152.5	0	0
30	485.268	912.094	19219.7	0	0
31	490.139	914.231	18183.8	0	0
32	495.01	916.404	17049.1	0	0
33	499.881	918.613	15820.7	0	0
34	504.752	920.859	14503.7	0	0
35	509.623	923.142	13104.1	0	0
36	514.494	925.463	11628	0	0
37	519.365	927.821	10082.4	0	0
38	524.236	930.217	8474.48	0	0
39	529.107	932.651	6812.12	0	0
40	533.978	935.124	5103.64	0	0
41	538.849	937.636	3357.93	0	0
42	543.72	940.188	1584.43	0	0
43	548.591	942.78	-206.882	0	0
44	553.462	945.411	-2005.44	0	0
45	558.333	948.084	-3685.27	0	0
46	563.204	950.798	-4872.16	0	0
47	568.074	953.553	-5510.31	0	0
48	572.945	956.351	-5567.68	0	0
49	577.816	959.19	-5011.49	0	0
50	582.687	962.073	-3808.16	0	0
51	587.558	965	0	0	0

Global Minimum Query (janbu simplified) - Safety Factor: 1.0085

Slice Number	X coordinate [ft]	Y coordinate - Bottom [ft]	Interslice Normal Force [lbs]	Interslice Shear Force [lbs]	Interslice Force Angle [degrees]
1	347.182	866.242	0	0	0
2	351.668	867.177	1899.08	0	0
3	356.154	868.15	3769.92	0	0
4	360.64	869.16	5601.51	0	0
5	365.126	870.208	7383.44	0	0
6	369.612	871.293	9105.94	0	0
7	374.099	872.416	10759.9	0	0
8	378.585	873.578	12336.7	0	0
9	383.071	874.777	13828.4	0	0
10	387.557	876.016	15227.9	0	0
11	392.043	877.293	16528.3	0	0
12	396.529	878.61	17723.6	0	0
13	401.015	879.966	18808.3	0	0
14	405.501	881.361	19777.6	0	0
15	409.987	882.797	20627.8	0	0
16	414.473	884.273	21357.6	0	0
17	418.959	885.79	21964.5	0	0
18	423.445	887.348	22446.5	0	0
19	427.931	888.947	22802.1	0	0
20	432.417	890.588	23030.7	0	0
21	436.903	892.271	23132.1	0	0
22	441.389	893.997	23106.9	0	0
23	445.875	895.765	22956.2	0	0
24	450.361	897.577	22682.1	0	0
25	454.847	899.433	22287	0	0
26	459.333	901.333	21774.2	0	0
27	463.819	903.277	21147.6	0	0
28	468.305	905.267	20411.9	0	0
29	472.791	907.303	19572.6	0	0
30	477.277	909.385	18635.6	0	0
31	481.763	911.513	17608.1	0	0
32	486.249	913.689	16497.5	0	0
33	490.735	915.913	15312.4	0	0
34	495.221	918.186	14062.1	0	0
35	499.707	920.508	12756.8	0	0
36	504.193	922.88	11407.3	0	0
37	508.679	925.302	10025.7	0	0
38	513.165	927.776	8624.68	0	0
39	517.651	930.302	7218.01	0	0
40	522.137	932.88	5820.4	0	0
41	526.623	935.513	4447.6	0	0
42	531.109	938.2	3116.39	0	0
43	535.595	940.942	1844.61	0	0
44	540.081	943.741	651.268	0	0
45	544.567	946.598	-443.493	0	0
46	549.053	949.512	-1418.3	0	0
47	553.539	952.486	-2250.51	0	0
48	558.025	955.521	-2796.68	0	0
49	562.511	958.617	-2639.44	0	0
50	566.997	961.777	-1700.59	0	0
51	571.483	965	0	0	0

List Of Coordinates

External Boundary



X	Y
0	700
236	700
1186	700
1186	1040
1175	1038
1112	1005
1062	1005
1040	1015
914	1005
833	988
814	986
768	965
682	965
555	965
407	895
303	845
288	839
255	825
220	816
188	816
95	816
39	794
0	790

Material Boundary

X	Y
39	794
44	789
67	789
73	793
93	796
111	797
150	806
178	810
236	810
256	819
280	825
313	839
364	860
404	879
449	897
500	917
570	937
657	957
669	960
763	960
768	965

Material Boundary

X	Y
236	700
236	810

APPENDIX E EARTHWORK AND GRADING GUIDELINES

STANDARD GRADING SPECIFICATIONS

These specifications present the usual and minimum requirements for grading operations performed under the observation and testing of TGR Geotechnical, Inc.

No deviation from these specifications will be allowed, except where specifically superseded in the Preliminary Geotechnical Investigation report, or in other written communication signed by the Soils Engineer or Engineering Geologist.

1.0 GENERAL

- The Soils Engineer and Engineering Geologist are the Owner's or Builder's representatives on the project. For the purpose of these specifications, observation and testing by the Soils Engineer includes that observation and testing performed by any person or persons employed by, and responsible to, the licensed Geotechnical Engineer or Geologist signing the grading report.
- All clearing, site preparation or earthwork performed on the project shall be conducted by the Contractor under the observation of the Geotechnical Engineer.
- It is the Contractor's responsibility to prepare the ground surface to receive the fills to the satisfaction of the Geotechnical Engineer and to place, spread, mix, water and compact the fill in accordance with the specifications of the Geotechnical Engineer. The Contractor shall also remove all material considered unsatisfactory by the Geotechnical Engineer.
- It is also the Contractor's responsibility to have suitable and sufficient compaction equipment on the job site to handle the amount of fill being placed. If necessary, excavation equipment will be shut down to permit completion of Compaction. Sufficient watering apparatus will also be provided by the Contractor, with due consideration for the fill material, rate of placement and time of year.
- A final report will be issued by the Geotechnical Engineer and Engineering Geologist attesting to the Contractor's conformance with these specifications.

2.0 SITE PREPARATION

- All vegetation and deleterious material such as rubbish shall be disposed of off-site. The removal must be concluded prior to placing fill.
- The Civil Engineer shall locate all houses, sheds, sewage disposal systems, large trees or structures on the site, or on the grading plan to the best of his knowledge prior to preparing the ground surface.
- Soil, alluvium or rock materials determined by the Geotechnical Engineer as being unsuitable for placement in compacted fills shall be removed and wasted from the site. Any material incorporated as part of a compacted fill must be approved by the Geotechnical Engineer.
- After the ground surface to receive fill has been cleared, it shall be scarified, disced or bladed by the Contractor until it is uniform and free from ruts, hollows, hummocks or other uneven features which may prevent uniform compaction.

The scarified ground surface shall then be brought to optimum moisture content, mixed as required, and compacted as specified. If the scarified zone is greater than twelve inches in depth, the excess shall be removed and placed in lifts restricted to six inches. Prior to placing fill, the ground surface to receive fill shall be inspected, tested and approved by the Geotechnical Engineer.

- Any underground structures such as cesspools, cisterns, mining shafts, tunnels, septic tanks, wells, pipe lines or others not located prior to grading are to be removed or treated in a manner prescribed by the Geotechnical Engineer.

3.0 COMPACTED FILLS

- Any material imported or excavated on the property may be utilized in the fill, provided each material has been determined to be suitable by the Geotechnical Engineer. Roots, tree branches and other matter missed during clearing shall be removed from the fill as directed by the Geotechnical Engineer.
- Rock fragments less than six inches in diameter may be utilized in the fill, provided:

- They are not placed in concentrated pockets.
 - There is a sufficient percentage of fine-grained material to surround the rocks.
 - The distribution of the rocks is observed by the Geotechnical Engineer.
- Rocks greater than six inches in diameter shall be taken off-site, or placed in accordance with the recommendations of the Geotechnical Engineer in areas designated as suitable for rock disposal. Details for rock disposal such as location, moisture control, percentage of the rock placed, etc., will be referred to in the “Conclusions and Recommendations” section of the Geotechnical Report, if applicable.

If rocks greater than six inches in diameter were not anticipated in the Preliminary Geotechnical report, rock disposal recommendations may not have been made in the “Conclusions and Recommendations” section. In this case, the Contractor shall notify the Geotechnical Engineer if rocks greater than six inches in diameter are encountered. The Geotechnical Engineer will then prepare a rock disposal recommendation or request that such rocks be taken off-site.

- Material that is spongy, subject to decay, or otherwise considered unsuitable shall not be used in the compacted fill.
- Representative samples of materials to be utilized as compacted fill shall be analyzed in the laboratory by the Geotechnical Engineer to determine their physical properties. If any material other than that previously tested is encountered during grading, the appropriate analysis of this material shall be conducted by the Geotechnical Engineer as soon as possible.
- Material used in the compacting process shall be evenly spread, watered or dried, processed and compacted in thin lifts not to exceed six inches in thickness to obtain a uniformly dense layer. The fill shall be placed and compacted on a horizontal plane, unless otherwise approved by the Geotechnical Engineer.

- If the moisture content or relative compaction varies from that required by the Geotechnical Engineer, the Contractor shall rework the fill until it is approved by the Geotechnical Engineer.
- Each layer shall be compacted to 90 percent of the maximum dry density in compliance with the testing method specified by the controlling governmental agency; (in general, ASTM D1557 will be used.)

If compaction to a lesser percentage is authorized by the controlling governmental agency because of a specific land use or expansive soil conditions, the area to receive fill compacted to less than 90 percent shall either be delineated on the grading plan or appropriate reference made to the area in the grading report.

- All fill shall be keyed and benched through all topsoil, colluvium, alluvium or creep material, into sound bedrock or firm material where the slope receiving fill exceeds a ratio of five horizontal to one vertical, in accordance with the recommendations of the Geotechnical Engineer.
- The key for side hill fills shall be a minimum of 15 feet within bedrock or firm materials, unless otherwise specified in the Preliminary report. (See details)
- Drainage terraces and subdrainage devices shall be constructed in compliance with the ordinances of the controlling governmental agency, or with the recommendation of the Geotechnical Engineer and Engineer Geologist.
- The Contractor will be required to obtain a minimum relative compaction of 90 percent out to the finish slope face of fill slopes, buttresses and stabilization fills. This may be achieved by either overbuilding the slope and cutting back to the compacted core, or by direct compaction of the slope face with suitable equipment, or by any other procedure which produces the required compaction.

The Contractor shall prepare a written detailed description of the method or methods he will employ to obtain the required slope compaction. Such documents shall be submitted to the Geotechnical Engineer for review and comments prior to the start of grading.

If a method other than overbuilding and cutting back to the compacted core is to be employed, slope tests will be made by the Geotechnical Engineer during construction of the slopes to determine if the required compaction is being achieved. Where failing tests occur or other field problems arise, the contractor will be notified by the Geotechnical Engineer.

If the method of achieving the required slope compaction selected by the Contractor fails to produce the necessary results, the Contractor shall rework or rebuild such slopes until the required degree of compaction is obtained, at no additional cost to the Owner or Geotechnical Engineer.

- All fill slopes should be planted or protected from erosion by methods specified in the preliminary report or by means approved by the governing authorities.
- Fill-over-cut slopes shall be properly keyed through topsoil, colluvium or creep material into rock or firm materials; and the transition shall be stripped of all soil prior to placing fill. (See detail)

4.0 CUT SLOPES

- The Engineering Geologist shall inspect all cut slopes excavated in rock, lithified or formation material at vertical intervals not exceeding ten feet.
- If any conditions not anticipated in the preliminary report such as perched water, seepage, lenticular or confined strata of a potentially adverse nature, unfavorably inclined bedding, joints or fault planes are encountered during grading, these

conditions shall be analyzed by the Engineering Geologist and Geotechnical Engineer; and recommendations shall be made to treat these problems.

- Cut slopes that face in the same direction as the prevailing drainage shall be protected from slope wash by a non-erosive interceptor swale placed at the top of the slope.
- Unless otherwise specified in the soils and geological report, no cut slopes shall be excavated higher or steeper than that allowed by the ordinances of controlling governmental agencies.
- Drainage terraces shall be constructed in compliance with the ordinances of controlling governmental agencies, or with the recommendations of the Geotechnical Engineer or Engineering Geologist.

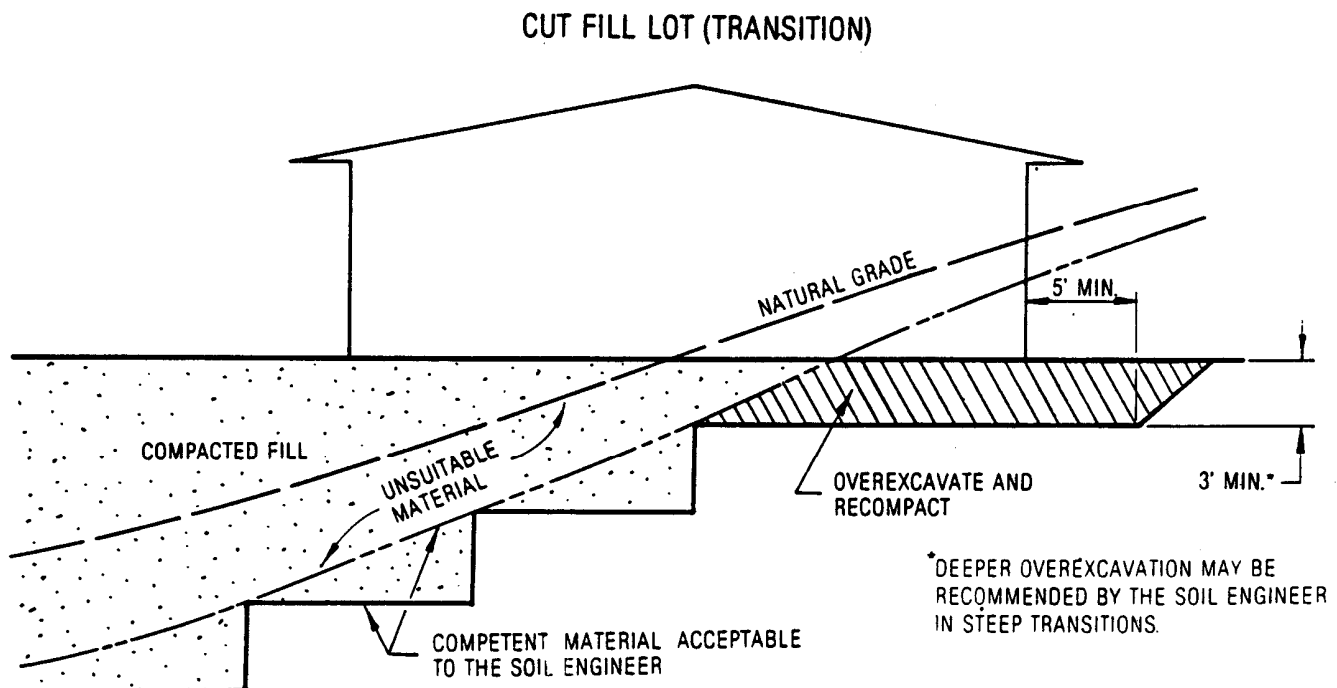
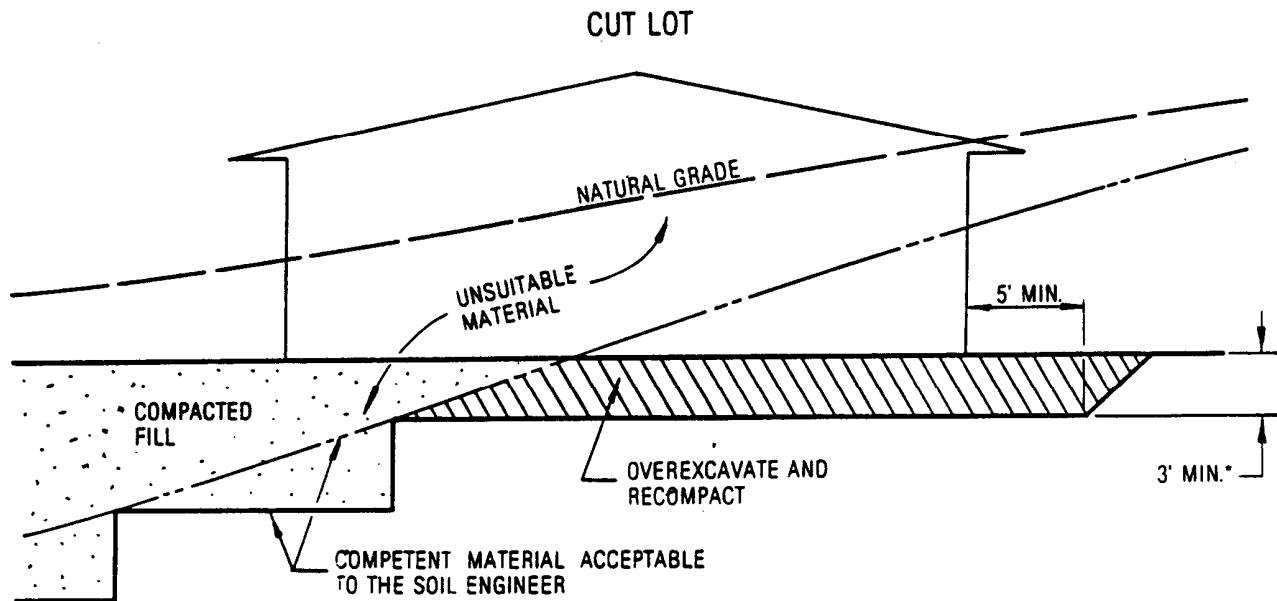
5.0 GRADING CONTROL

- Inspection of the fill placement shall be provided by the Geotechnical Engineer during the progress of grading.
- In general, density tests should be made at intervals not exceeding two feet of fill height or every 500 cubic yards of fill placed. This criteria will vary depending on soil conditions and the size of the job. In any event, an adequate number of field density tests shall be made to verify that the required compaction of being achieved.
- Density tests should be made on the surface material to receive fill as required by the Geotechnical Engineer.
- All cleanout, processed ground to receive fill, key excavations, subdrains and rock disposal must be inspected and approved by the Geotechnical Engineer (and often by the governing authorities) prior to placing any fill. It shall be the Contractor's responsibility to notify the Geotechnical Engineer and governing authorities when such areas are ready for inspection.

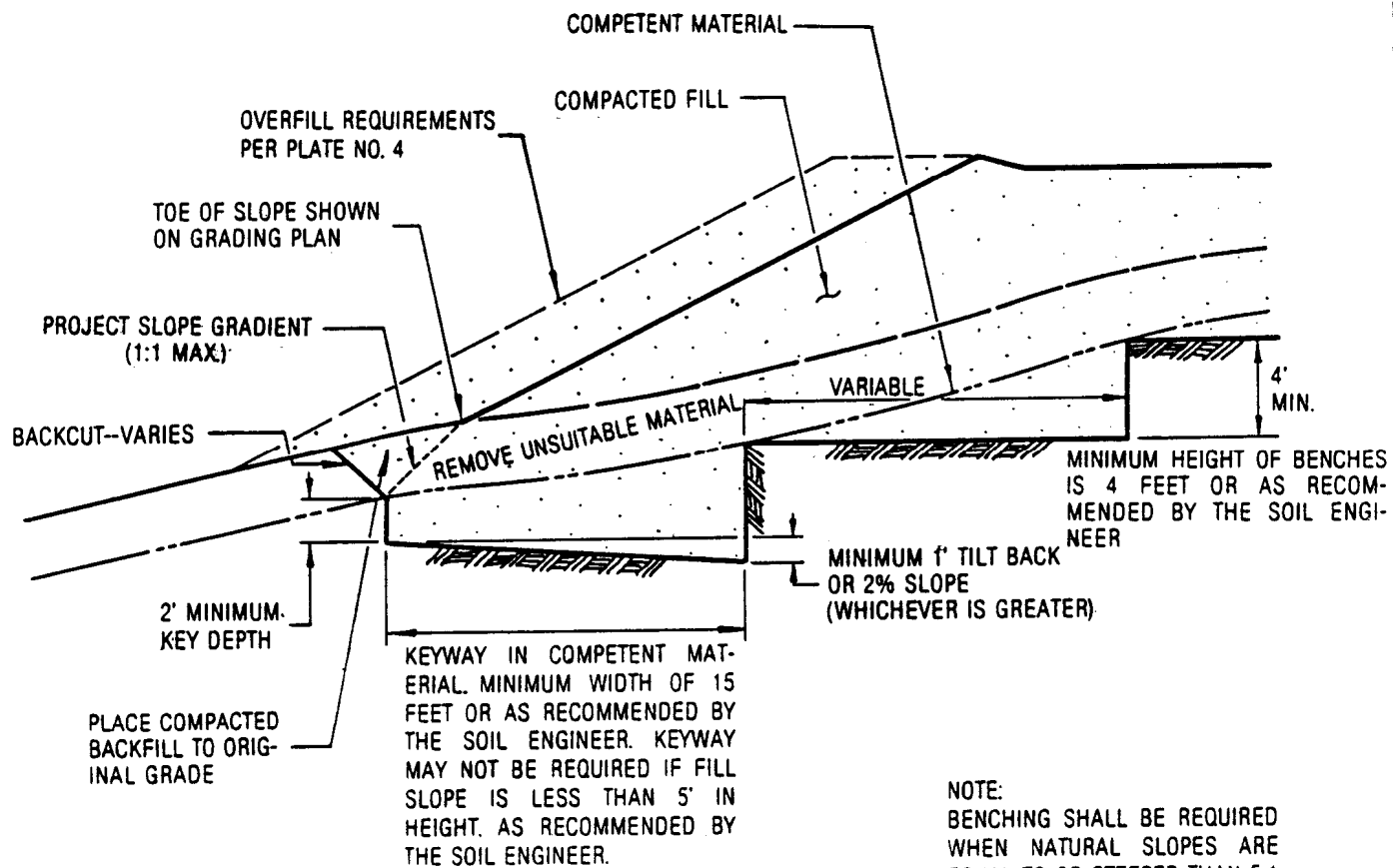
6.0 CONSTRUCTION CONSIDERATIONS

- Erosion control measures, when necessary, shall be provided by the Contractor during grading and prior to the completion and construction of permanent drainage controls.
- Upon completion of grading and termination of observations by the Geotechnical Engineer, no further filling or excavating, including that necessary for footings, foundations, large tree wells, retaining walls, or other features shall be performed without the approval of the Geotechnical Engineer or Engineering Geologist.
- Care shall be taken by the Contractor during final grading to preserve any berms, drainage terraces, interceptor swales, or other devices of a permanent nature on or adjacent to the property.

TYPICAL OVEREXCAVATION OF DAYLIGHT LINE

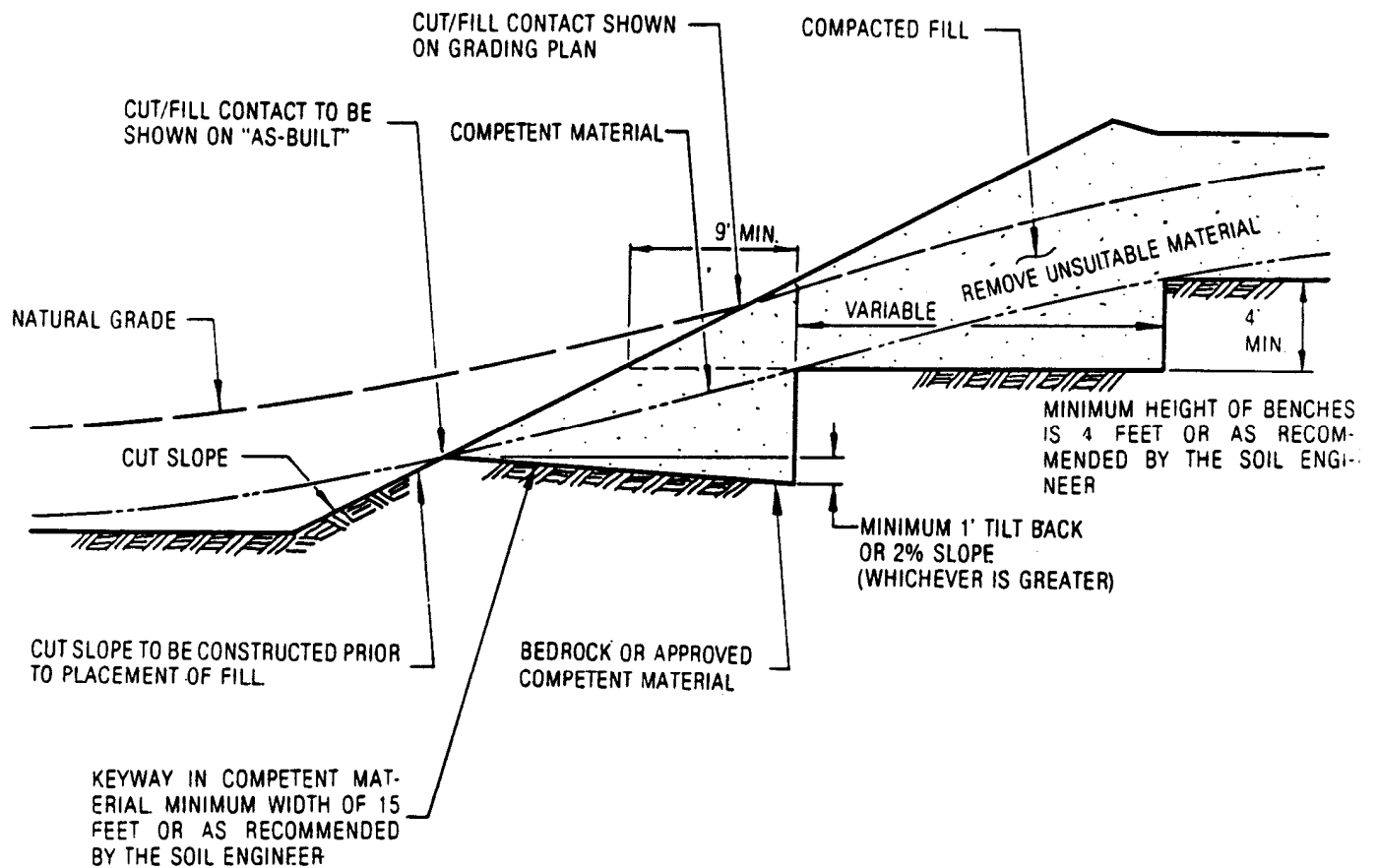


TYPICAL FILL OVER NATURAL SLOPE

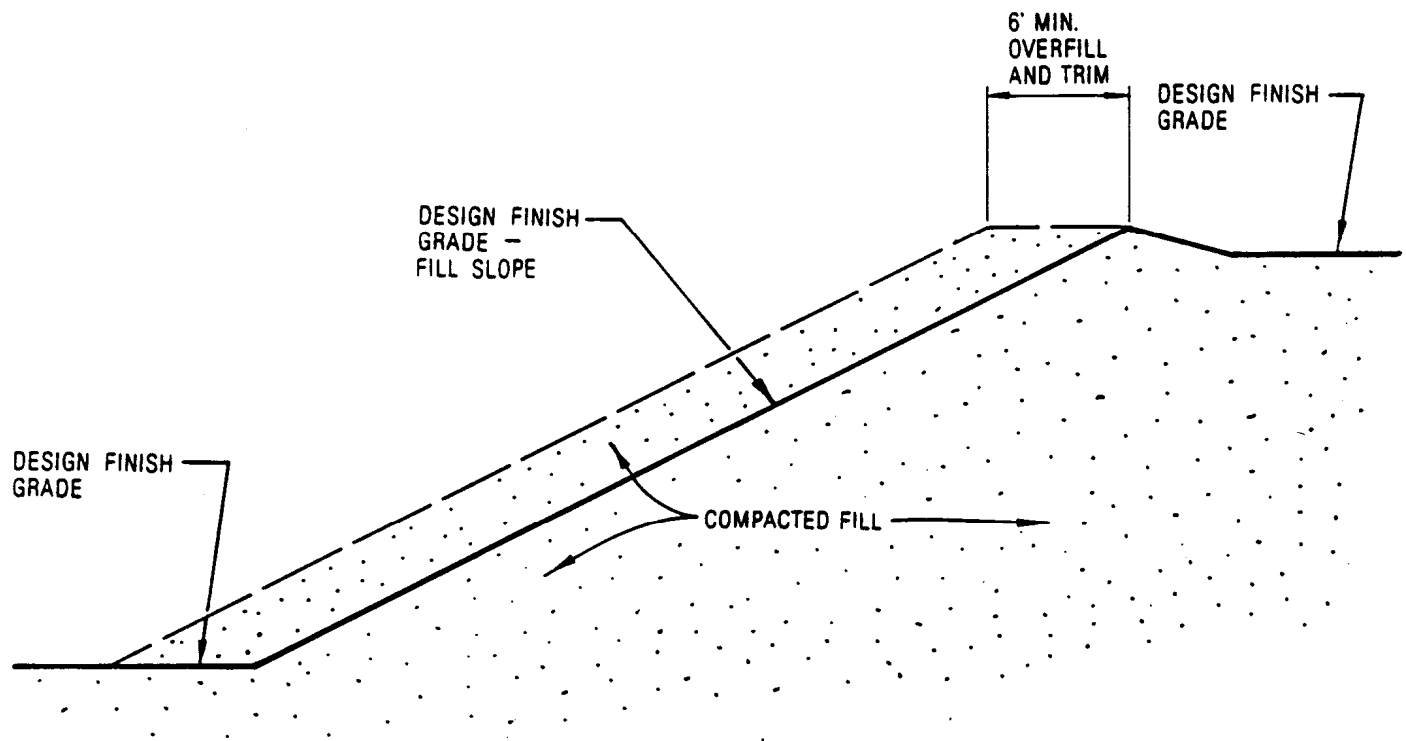


NOTE:
BENCHING SHALL BE REQUIRED
WHEN NATURAL SLOPES ARE
EQUAL TO OR STEEPER THAN 5:1
OR WHEN RECOMMENDED BY
THE SOIL ENGINEER.

TYPICAL FILL-OVER-CUT SLOPE



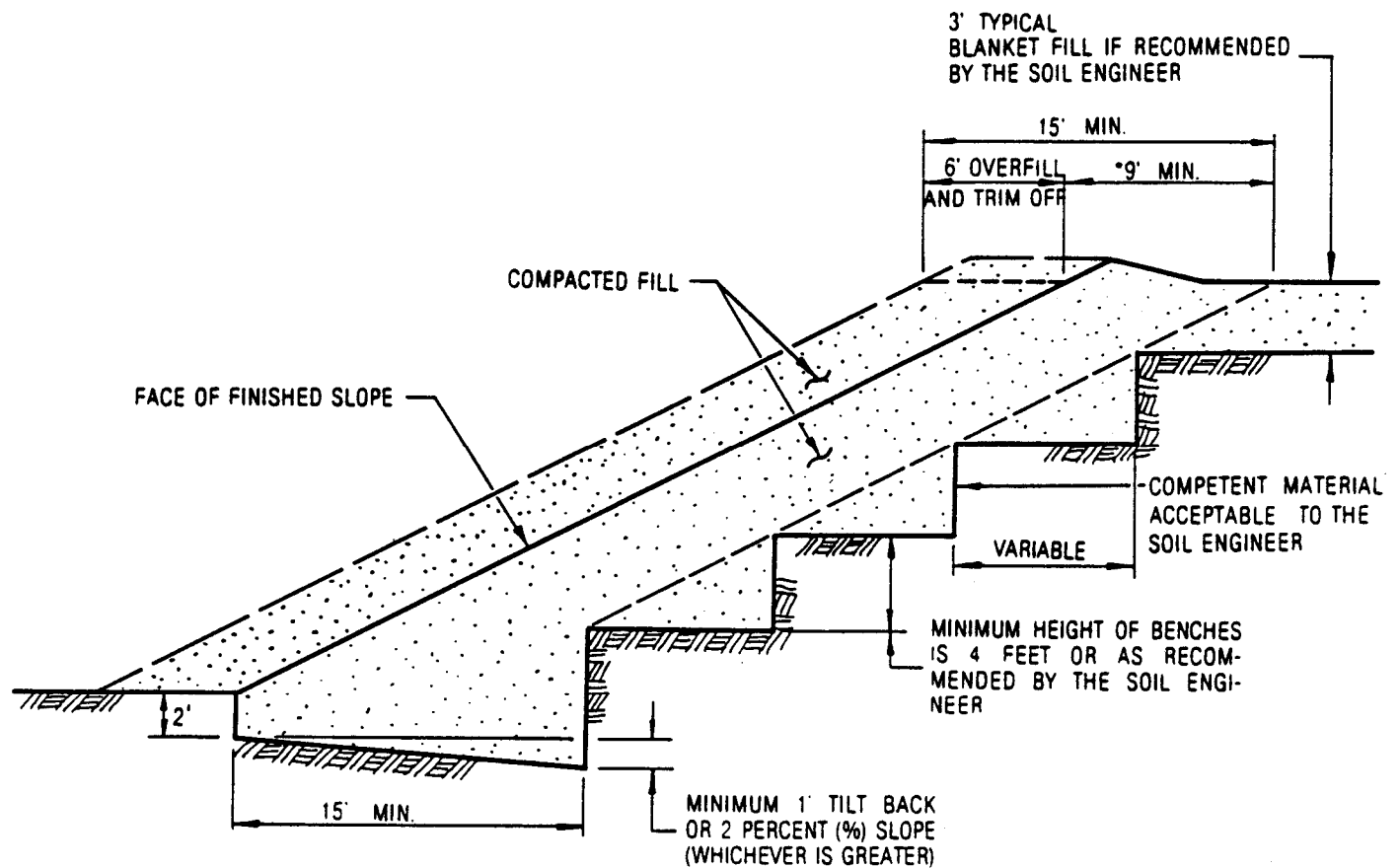
TYPICAL FILL SLOPE CONSTRUCTION



NOTES:

1. ALL FILL SLOPES, INCLUDING BUTTRESS AND STABILIZATION FILLS, SHALL BE OVERFILLED A MINIMUM OF SIX FEET HORIZONTALLY WITH COMPACTED FILL AND TRIMMED TO THE DESIGN FINISH GRADE.
EXCEPTIONS:
A. FILL SLOPE OVER CUT SLOPE.
B. FILL SLOPE ADJACENT TO EXISTING IMPROVEMENTS.
2. THE EXCEPTIONS ABOVE WHICH DO NOT HAVE THE 6 FOOT SLOPE OVERFILL AND TRIM SHALL BE COMPACTED AS STATED IN THE PROJECT SPECIFICATIONS.

TYPICAL STABILIZATION FILL

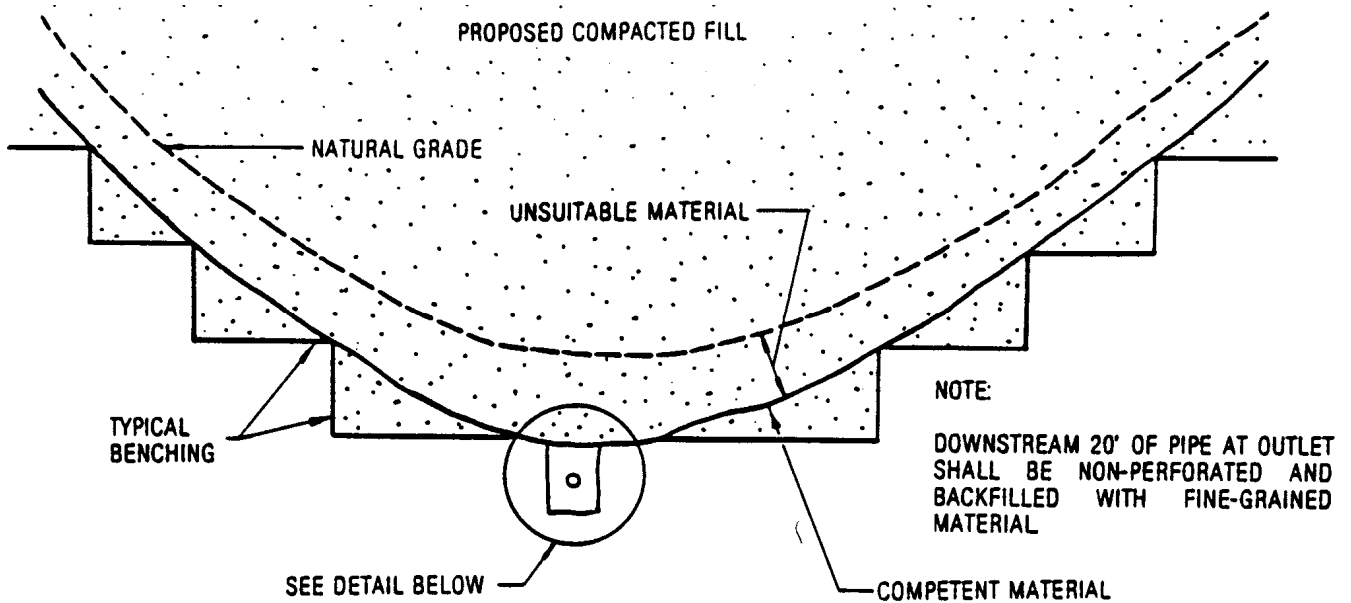


NOTE:

SEE PLATE 6 FOR TYPICAL SUBDRAIN DETAILS FOR STABILIZATION FILLS. IF RECOMMENDED BY THE SOIL ENGINEER.

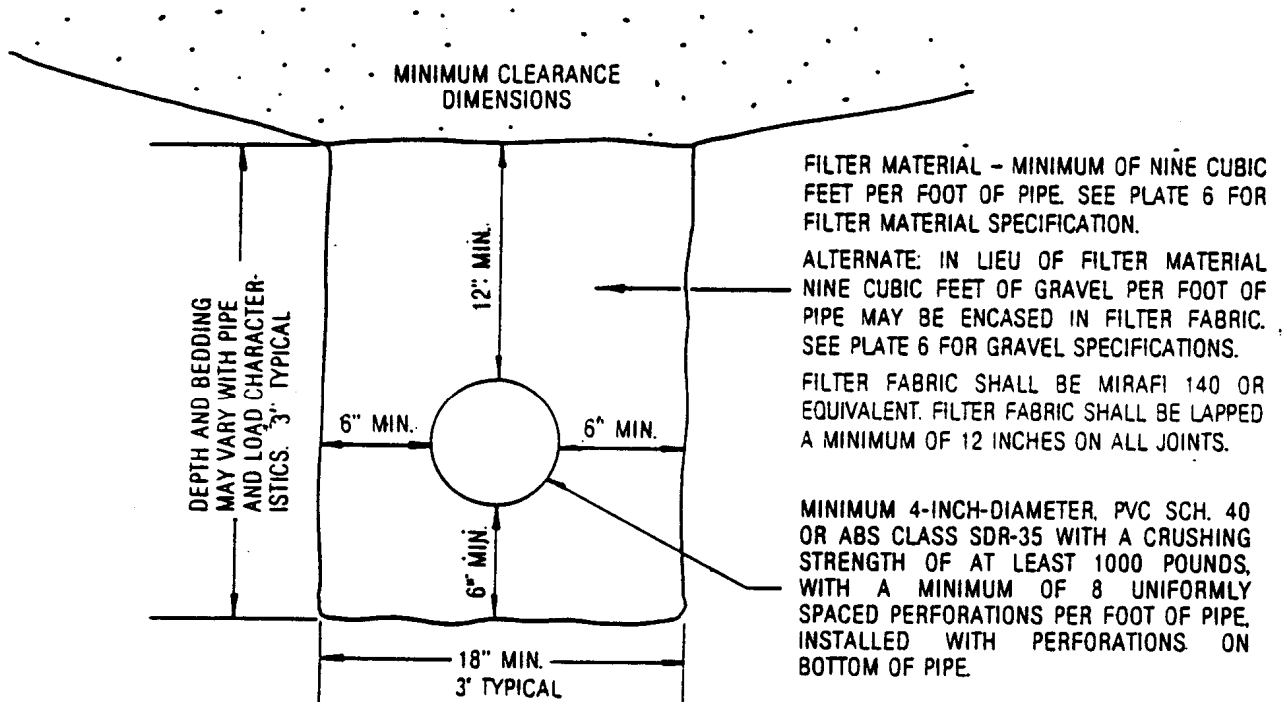
*GREATER THAN 9' IF RECOMMENDED BY THE SOIL ENGINEER.
15' WHERE NO 6' OVERFILL

TYPICAL CANYON SUBDRAIN

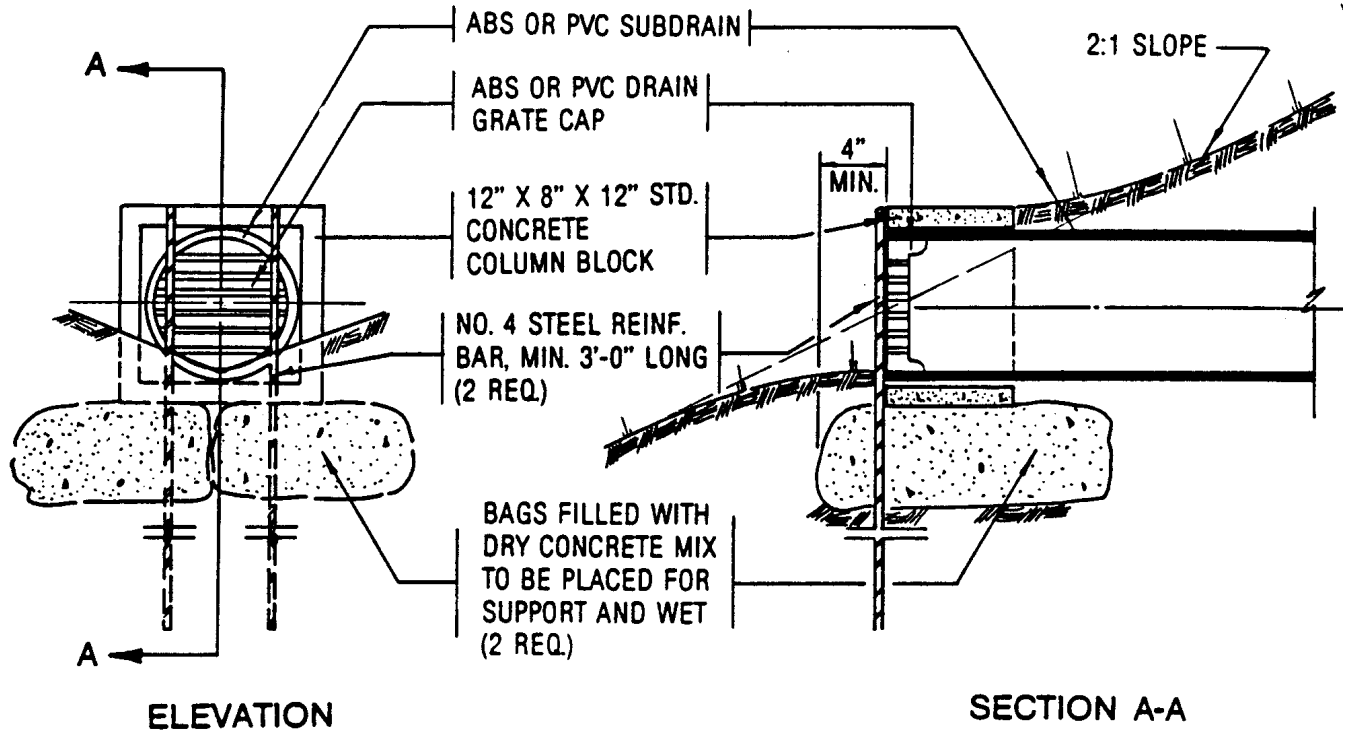


NOTES:

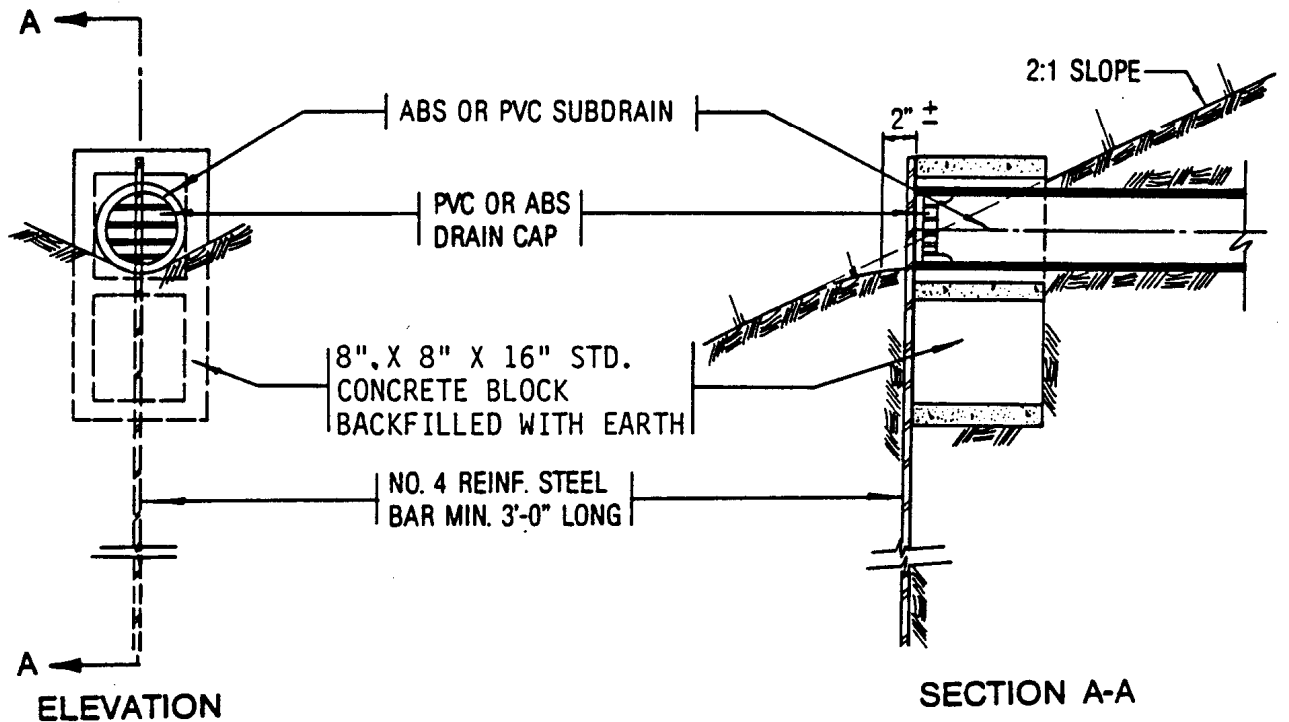
PIPE SHALL BE A MINIMUM OF 4 INCHES DIAMETER AND RUNS OF 500 FEET OR MORE USE 6-INCH DIAMETER PIPE, OR AS RECOMMENDED BY THE SOIL ENGINEER



SUBDRAIN OUTLET MARKER

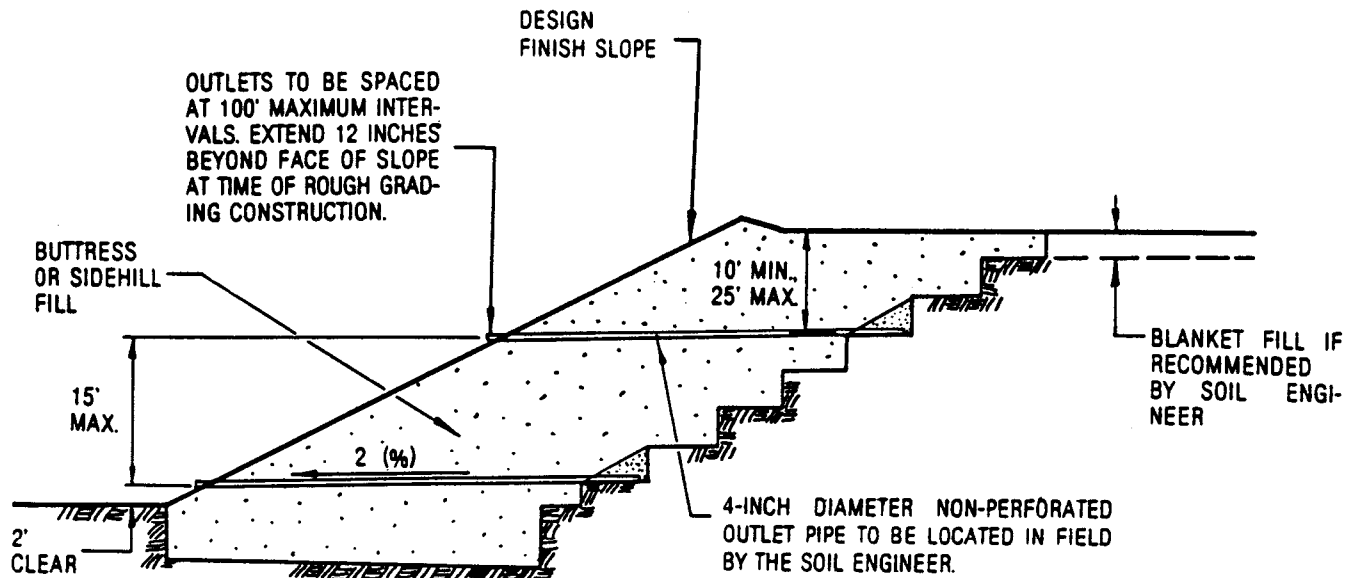


SUBDRAIN OUTLET MARKER FOR 6" AND 8" PIPES



SUBDRAIN OUTLET MARKER - 4" PIPE

TYPICAL STABILIZATION AND BUTTRESS FILL SUBDRAIN



FILTER MATERIAL TO MEET FOLLOWING SPECIFICATION OR APPROVED EQUIVALENT: (CONFORMS TO MA STD. PLAN 323)

SIEVE SIZE	PERCENTAGE PASSING
1"	100
3/4"	90-100
3/8"	40-100
NO. 4	25-40
NO. 8	18-33
NO. 30	5-15
NO. 50	0-7
NO. 200	0-3

"GRAVEL" TO MEET FOLLOWING SPECIFICATION OR APPROVED EQUIVALENT:

SIEVE SIZE	MAXIMUM PERCENTAGE PASSING
1 1/2"	100
NO. 4	50
NO. 200	8

SAND EQUIVALENT = MINIMUM OF 50

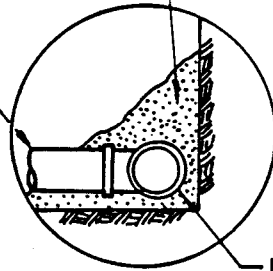
FILTER MATERIAL - MINIMUM OF FIVE CUBIC FEET PER FOOT OF PIPE. SEE ABOVE FOR FILTER MATERIAL SPECIFICATION.

ALTERNATIVE: IN LIEU OF FILTER MATERIAL, FIVE CUBIC FEET OF GRAVEL PER FOOT OF PIPE MAY BE ENCASED IN FILTER FABRIC. SEE ABOVE FOR GRAVEL SPECIFICATION.

FILTER FABRIC SHALL BE MIRAFI 140 OR EQUIVALENT. FILTER FABRIC SHALL BE LAPPED A MINIMUM OF 12 INCHES ON ALL JOINTS.

MINIMUM 4-INCH DIAMETER PVC SCH 40 OR ABS CLASS SDR 35 WITH A CRUSHING STRENGTH OF AT LEAST 1,000 POUNDS, WITH A MINIMUM OF 8 UNIFORMLY SPACED PERFORATIONS PER FOOT OF PIPE INSTALLED WITH PERFORATIONS ON BOTTOM OF PIPE. PROVIDE CAP AT UPSTREAM END OF PIPE. SLOPE AT 2 PERCENT TO OUTLET PIPE.

OUTLET PIPE TO BE CONNECTED TO SUBDRAIN PIPE WITH TEE OR ELBOW

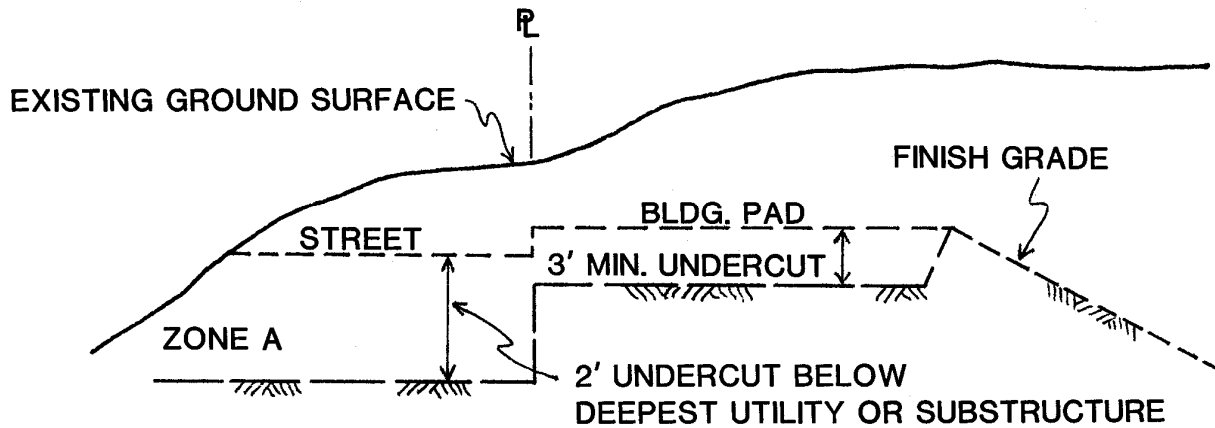


NOTES:

1. TRENCH FOR OUTLET PIPES TO BE BACKFILLED WITH ON-SITE SOIL.

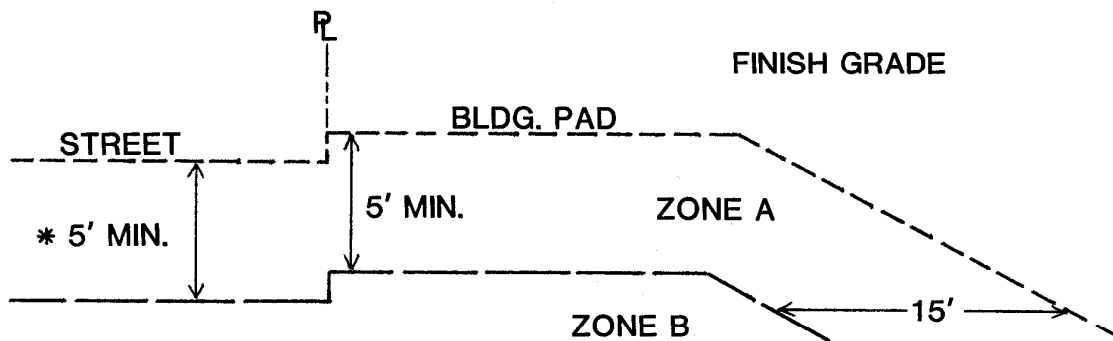
TYPICAL CUT AND FILL GRADING DETAILS

TYPICAL GRADING WITHIN PROPOSED DEEP BEDROCK CUT AREAS



NO SCALE

TYPICAL GRADING WITHIN PROPOSED FILL AREAS



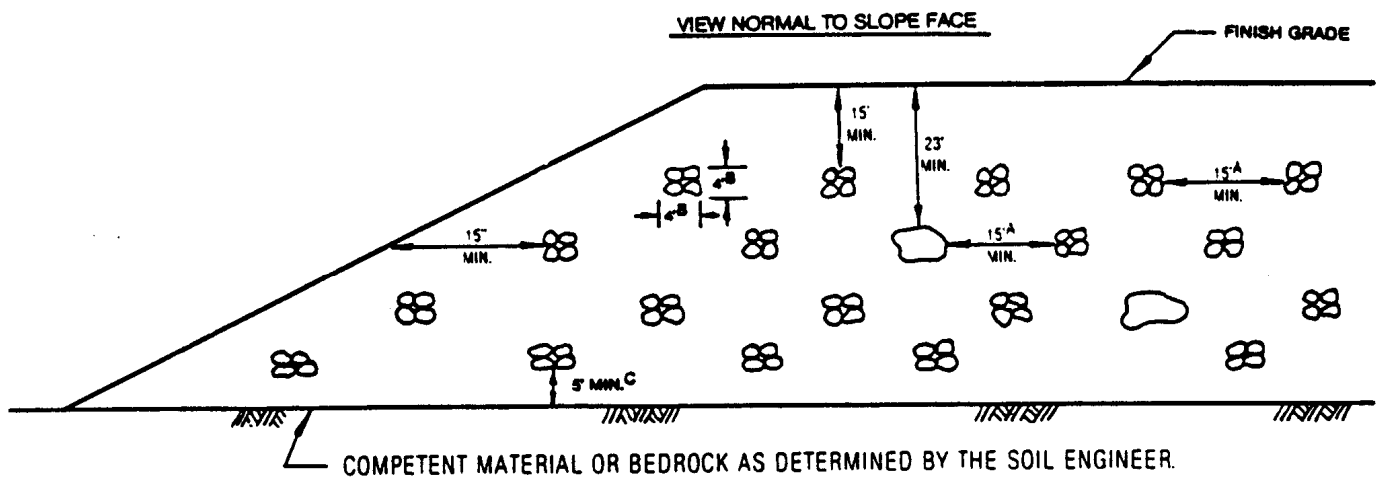
LEGEND

ZONE A "SOIL" FILL PLACED IN ACCORDANCE WITH THE RECOMMENDATIONS PRESENTED IN SECTION 11.2.3 OF THIS REPORT

ZONE B "SOIL-ROCK" AND/OR "ROCK" FILL PLACED IN ACCORDANCE WITH THE RECOMMENDATIONS PRESENTED IN SECTION 11.2.3 OF THIS REPORT

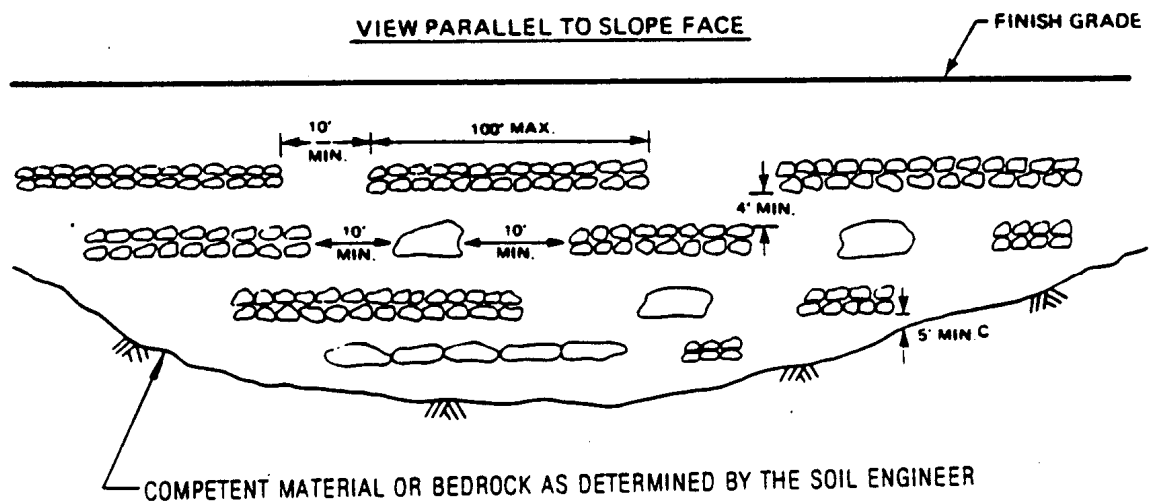
* 5' OR 1' BELOW DEEPEST UTILITY, WHICHEVER IS GREATER

TYPICAL OVERSIZE ROCK DISPOSAL – “SOIL-ROCK” FILL



NOTE:

ORIENTATION OF WINDROWS MAY VARY BUT SHALL BE AS RECOMMENDED BY SOIL ENGINEER.



NOTES:

- ONE EQUIPMENT WIDTH OR A MINIMUM OF 15 FEET.
- HEIGHT AND WIDTH MAY VARY DEPENDING ON ROCK SIZE AND TYPE OF EQUIPMENT.
- IF APPROVED BY THE SOIL ENGINEER, WINDROWS MAY BE PLACED DIRECTLY ON COMPETENT MATERIALS OR BEDROCK PROVIDING ADEQUATE SPACE IS AVAILABLE FOR COMPACTION.
- VOIDS IN WINDROW TO BE FILLED BY FLOODING GRANULAR SOIL INTO PLACE. GRANULAR SOIL SHALL MEAN ANY SOIL WHICH HAS A UNIFIED SOIL CLASSIFICATION SYSTEM (UBC 29-1) DESIGNATION OF SM, SP, SW, GM, GP, OR GW.
- AFTER FILL BETWEEN WINDROWS IS PLACED AND COMPACTED WITH THE LIFT OF FILL COVERING WINDROW, WINDROW SHALL BE PROOF-ROLLED WITH D-9 DOZER OR EQUIVALENT.
- OVERSIZED ROCK IS DEFINED AS LARGER THAN 12" IN SIZE.