

PDP SWQMP

PRIORITY DEVELOPMENT PROJECT (PDP) STORM WATER QUALITY MANAGEMENT PLAN

FOR

EASTLAKE BEHAVIORAL HEALTH HOSPITAL

595-710-11,12

DR 19-0012, CUP 19-0010, PER 19-0006

Insert Drawing Number

ENGINEER OF WORK:

8/2/2019 Kamal S. Sweis P.E. 48592

PREPARED FOR: Acadia Health Care 6100 Tower Circle, Suite 1000 Franklin, TN 91914 370067 Insert Telephone Number



PREPARED BY: K &S ENGINEERING, INC. Planning Engineering Surveying

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(619)296-5565

DATE: 08/01/2019

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ACRONYMS

APN	Assessor's Parcel Number
BMP	Best Management Practice
HMP	Hydromodification Management Plan
HSG	Hydrologic Soil Group
MS4	Municipal Separate Storm Sewer System
N/A	Not Applicable
NRCS	Natural Resources Conservation Service
PDP	Priority Development Project
PE	Professional Engineer
SC	Source Control
SD	Site Design
SDRWQCB	San Diego Regional Water Quality Control Board
SIC	Standard Industrial Classification
SWQMP	Storm Water Quality Management Plan

CERTIFICATION PAGE Project Name: EASTLAKE BEHAVIORAL HEALTH HOSPITAL Permit Application Number: DR 19-0012, CUP 19-0010, PER 19-0006

I hereby declare that I am the Engineer in Responsible Charge of design of storm water best management practices (BMPs) for this project, and that I have exercised responsible charge over the design of the BMPs as defined in Section 6703 of the Business and Professions Code, and that the design is consistent with the PDP requirements of the City of Chula Vista BMP Design Manual, which is based on the requirements of the San Diego Regional Water Quality Control Board Order No. R9-2013-0001 as amended by R9-2015-0001 and R9-2015-0100 (MS4 Permit).

I have read and understand that the City Engineer has adopted minimum requirements for managing urban runoff, including storm water, from land development activities, as described in the BMP Design Manual. I certify that this PDP SWQMP has been completed to the best of my ability and accurately reflects the project being proposed and the applicable BMPs proposed to minimize the potentially negative impacts of this project's land development activities on water quality. I understand and acknowledge that the plan check review of this PDP SWQMP by the City Engineer is confined to a review and does not relieve me, as the Engineer in Responsible Charge of design of storm water BMPs for this project, of my responsibilities for project design.

Engineer of Work's Signature, PE Number & Expiration Date

<u>KASENGINZERINS IKC.</u> Company <u>2/26/19</u>



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SUBMITTAL RECORD

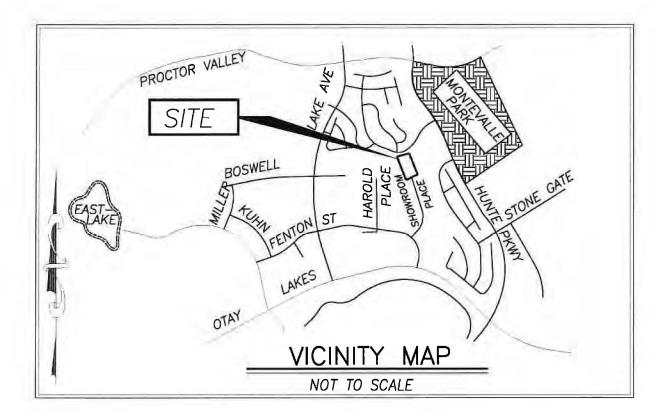
Use this Table to keep a record of submittals of this PDP SWQMP. Each time the PDP SWQMP is re-submitted, provide the date and status of the project. In column 4 summarize the changes that have been made or indicate if response to plancheck comments is included. When applicable, insert response to plancheck comments behind this page.

Submittal Number	Date	Project Status	Summary of Changes
1	02/28/2019	 Preliminary Design/ Planning/ CEQA Final Design 	Click here to enter text.
2	08/05/2019	 Preliminary Design/ Planning/ CEQA Final Design 	Resubmittal
3	Click here to enter a date.	 Preliminary Design/ Planning/ CEQA Final Design 	Click here to enter text.
4	Click here to enter a date.	□Preliminary Design/ Planning/ CEQA □Final Design	Click here to enter text.

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PROJECT VICINITY MAP

Project Name: EASTLAKE BEHAVIORAL HEALTH HOSPITAL Permit Application Number: DR 19-0012, CUP 19-0010, PER 19-0006



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Complete and attach Storm Water Requirements Applicability Checklist (Intake Form) included in Appendix A.1

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Storm Water Requirements Applicability Checklist (Intake Form) for All Permit Applications

Public Works Department - Storm Water Management Section	and the states	April 2016				
Project Information						
Project Address: 830, 831 Showroom Place Chula Vista	Project Applic DR 19-0012, CU	ation Number: P 19-0010, PER 19-0006				
Project Name: EASTLAKE BEHAVIORAL HEALTH HOSPITAL	APN(s) 561-7	710-11,12				
Brief Description of Work Proposed: Grading and Drainag	je					
Owner/Contact I	nformation					
Name of Person Completing this Form: Kamal Sweis						
Role: Property Owner Contractor Architect	X Engineer	Other				
Email: kss@ks-engr.com	Phone:	(619)296-5565				
Signature:	Date Corr	npleted: Feb 28, 2019				
information for determining the requirements is found in t	Answer each section below, starting with Section 1 and progressing through each section. Additional information for determining the requirements is found in the Chula Vista BMP Design Manual available on the City's website at http://www.chulavistaca.gov/departments/public-works/services/storm-water-pollution-prevention/documents-and-reports .					
SECTION 1: Storm Water BMP Requirements						
 Does the project consist of one or both of the following: Repair or improvements to an existing building or structure that donot alter the size such as: tenant improvements, interior remodeling, electrical work, fire alarm, fire sprinkler system, HVAC work, Gas, plumbing, etc. Routine maintenance activities such as: roof or exterior structure surface replacement; resurfacing existing roadways and parking lots 						
including digouts, slurry seal, overlay and restriping; repair damaged sidewalks or pedestrian ramps on existing roads without expanding the impervious footprint; routine replacement of damaged pavement, trenching and resurfacing associated with utility work (i.e. sewer, water, gas or electrical laterals, etc.) and pot holing or geotechnical investigation borings.	⊠No	Continue to Section 2, page3.				

City of Chula Vista

Construction Storm Water BMP Certification Statement

The following stormwater quality protection measures are required by City Chula Vista Municipal Code Chapter 14.20 and the City's Jurisdictional Runoff Management Program.

- 1. All applicable construction BMPs and non-stormwater discharge BMPs shall be installed and maintained for the duration of the project in accordance with the Appendix K "Construction BMP Standards" of the Chula Vista BMP Design Manual.
- 2. Erosion control BMPs shall be implemented for all portions of the project area in which no work has been done or is planned to be done over a period of 14 or more days. All onsite drainage pathways that convey concentrated flows shall be stabilized to prevent erosion.
- 3. Run-on from areas outside the project area shall be diverted around work areas to the extent feasible. Run-on that cannot be diverted shall be managed using appropriate erosion and sediment control BMPs.
- 4. Sediment control BMPs shall be implemented, including providing fiber rolls, gravel bags, or other equally effective BMPs around the perimeter of the project to prevent transport of soil and sediment offsite. Any sediment tracked onto offsite paved areas shall be removed via sweeping at least daily.
- 5. Trash and other construction wastes shall be placed in a designated area at least daily and shall be disposed of in accordance with applicable requirements.
- 6. Materials shall be stored to avoid being transported in storm water runoff and non-storm water discharges. Concrete washout shall be directed to a washout area and shall not be washed out to the ground.
- 7. Stockpiles and other sources of pollutants shall be covered when the chance of rain within the next 48 hours is at least 50%.

I certify that the stormwater quality protection measures listed above will be implemented at the project described on Intake Form. I understand that failure to implement these measures may result in monetary penalties or other enforcement actions. This certification is signed under penalty of periury and does not require notarization.

WEIS

Name:

Title:

Signature:

tle: <u>PRESIDENT</u> Date: <u>8/2/2019</u>

÷	City of Chula Vista Storm Water Applicability Checklist (Intake Form)	* Pag (Ap	e 3 of ril 201
Se	ection 2: Determine if Project is a Standard Projec	t or Priority Developm	ent Pr	oiec
1.				
	X New Development			
	 Redevelopment (is the creation and/or replacement of imp 	envious surface on an alread	v devel	hear
	site)		ly deven	opeu
2.	. Is the project in any of the following categories, (a) through (j)?			
a.	a. New development that creates 10,000 square feet or more (collectively over the entire project site). This includes comme mixed-use, and public development projects on public or privat	rcial, industrial, residential,	XYes	
b.	b. Redevelopment project that creates and/or replaces 5,000 impervious surface (collectively over the entire project site on square feet or more of impervious surfaces). This include residential, mixed-use, and public development projects on public	an existing site of 10,000 es commercial, industrial,	Yes	×N
C.	New development or redevelopment of a restaurant that creat square feet or more of impervious surface (collectively over the category is defined as a facility that sells prepared foods are including stationary lunch counters and refreshment stands as drinks for immediate consumption (Standard Industrial Classific)	e entire project site). This ad drinks for consumption, selling prepared foods and	Yes	×
d.	 New development or redevelopment of hillside that creates and feet or more of impervious surface (collectively over the category includes development on any natural slope that greater. 	entire project site). This	Yes	×
e.	New development or redevelopment of parking lot that creat square feet or more of impervious surface (collectively over the category is defined as a land area or facility for the temporary proventices used personally, for business, or for commerce.	ne entire project site). This	XYes	
f.	New development or redevelopment of Streets, roads, driveways that creates and/or replaces 5,000 square feet or n (collectively over the entire project site). This category is impervious surface used for the transportation of automobiles other vehicles	nore of impervious surface s defined as any paved	⊠ Yes	N
g.	New development or redevelopment project that creates and feet or more of impervious surface (collectively over the entire directly to an Environmentally Sensitive Area (ESA). "Discha flow that is conveyed overland a distance of 200 feet or less fr or conveyed in a pipe or open channel any distance as an iso to the ESA (i.e. not commingled with flows from adjacent lands).	e project site), discharging arging directly to" includes om the project to the ESA, lated flow from the project	Yes	×N
h.	. New development or redevelopment project of automotive and/or replaces 5,000 square feet or more of impervious s defined as a facility that is categorized in any one of the fol Classification (SIC) codes: 5013, 5014, 5541, 7532-7534, or 75	surface. This category is lowing Standard Industrial	Yes	×N
i.	New development or redevelopment projects of retail gasoline replaces 5,000 square feet or more of impervious surface or in Traffic (ADT) of 100 or more vehicles per day.		Yes	×n
j.	New development or redevelopment that result in the disturban	ice of one or more acres of	XYes	

City of Chula Vista

The project is (select one):

If "No" is checked for every category in Section 2, project is "Standard Development Project". Site design and source control BMP requirements apply. Complete and submit Standard SWQMP (refer to Chapter 4 & Appendix E of the BMP Design Manual for guidance). Continue to Section 4.					
project is "Priority Development Project (PDP)". ection 3.					
LY:					
project site is:ft² (A)					
us area is 270494 ft ² (B)					
100: 100 %					
(select one based on the above calculation):					
w impervious areas are considered a PDP					
ct site is considered a PDP					
pt					
k, bicycle lane or trails that:					
ter runoff to adjacent vegetated areas, or other					
isconnected from paved streets or roads? Or;					
vements or surfaces in accordance with USEPA					
⊠ No. Next question					
opment of existing paved alleys, streets or roads een Streets standards?					
No. Project is PDP. Site design, source control and structural pollutant control BMPs apply. Complete and submit PDP SWQMP (refer to Chapters 4, 5 & 6 of the BMP Design Manual for guidance). Continue to Section 4.					

* City of Chula Vista

SECTION 4: Construction Storm Water BMP Requirements:

All construction sites are required to implement construction BMPs in accordance with the performance standards in the BMP Design Manual. Some sites are additionally required to obtain coverage under the State Construction General Permit (CGP), which is administered by the State Water Resource Control Board.

- 1. Does the project include Building/Grading/Construction permits proposing less than 5,000 square feet of ground disturbance and has less than 5-foot elevation change over the entire project area?
 - Yes; review & sign Construction Storm Water Certification X No; next question Statement, skip questions 2-4
- 2. Does the project propose construction or demolition activity, including but not limited to, clearing grading, grubbing, excavation, or other activity that results in ground disturbance of less than one acre and more than 5,000 square feet?

Yes. complete & submit Construction Storm Water Pollution Control Plan (CSWPCP), skip questions 3-4 No; next question

- Does the project results in disturbance of an acre or more of total land area and are considered regular maintenance projects performed to maintain original line and grade, hydraulic capacity, or original purpose of the facility? (Projects such as sewer/storm drain/utility replacement)
 - ☐ Yes. complete & submit Construction Storm Water Pollution X No; next question Control Plan (CSWPCP), skip question 4
- 4. Is the project proposing land disturbance greater than or equal to one acre OR the project is part of a larger common plan of development disturbing 1 acre or more?
 - Yes; Storm Water Pollution Prevention Plan (SWPPP) is required. Refer to online CASQA or Caltrans Template. Visit the SWRCB web site at http://www.waterboards.ca.gov/water-issues/programs/stormwater/construction.shtml.

Note: for Projects that result in disturbance of one to five acres of total land area and can demonstrate that there will be no adverse water quality impacts by applying for a Construction Rainfall Erosivity Waiver, may be allowed to submit a CSWPCP in lieu of a SWPPP.

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Site Informat	Form I-3B	
	For PDPs	(for PDPs)
Project Sum	imary Information	
Project Name	EASTLAKE BEHAVIORAL	HEALTH HOSPITAL
Project Address	830, 831 SHOWROOM P	LACE CHULA VISTA, CA
Assessor's Parcel Number(s) (APN(s))	595-710-11, 12	
Permit Application Number	Click here to enter text.	
Project Hydrologic Unit	Select One: ☐ Pueblo San Diego (903 ⊠ Sweetwater (909) ☐ Otay (910) ☐ Tijuana (911)	8)
Project Watershed		
(Complete Hydrologic Unit, Area, and Subarea Name with Numeric Identifier)	909.110 Lower Sweetwa	ter, Telegraph HSA
Parcel Area		
(total area of Assessor's Parcel(s) associated with the project)	10.43 Acres (454,330 Squ	uare Feet)
Area to be Disturbed by the Project (Project Area)	9.82 Acres (427,759 Squa	are Feet)
Project Proposed Impervious Area (subset of Project Area)	6.2 Acres (270,072 Squar	re Feet)
Project Proposed Pervious Area (subset of Project Area)	4.18 Acres (182,080 Squa	are Feet)
Note: Proposed Impervious Area + Proposed Perv This may be less than the Parcel Area.	vious Area = Area to be Dis	turbed by the Project.
The proposed increase or decrease in impervious area in the proposed condition as compared to the pre-project condition.	60 %	

Form I-3B Page 2 of 10	
Description of Existing Site Condition and Drainage Patterns	
Current Status of the Site (select all that apply):	
Existing development	
🖾 Previously graded but not built out	
Demolition completed without new construction	
□ Agricultural or other non-impervious use	
Vacant, undeveloped/natural	
Description / Additional Information:	
Click here to enter text	
Existing Land Cover Includes (select all that apply):	_
☑ Vegetative Cover	
Non-Vegetated Pervious Areas	
Impervious Areas	
Description / Additional Information:	
Click here to enter text.	
Underlying Soil belongs to Hydrologic Soil Group (select all that apply):	_
NRCS Type A	
NRCS Type B	
NRCS Type C	
🖾 NRCS Type D	
Approximate Depth to Groundwater (GW):	
GW Depth < 5 feet	
□ 5 feet < GW Depth < 10 feet	
□ 10 feet < GW Depth < 20 feet	
⊠ GW Depth > 20 feet	
Existing Natural Hydrologic Features (select all that apply):	
Watercourses	
□ Seeps	
Springs	
Wetlands	
X None	
Description / Additional Information:	
Click here to enter text	

Form I-3B Page 3 of 10

Description of Existing Site Topography and Drainage

How is storm water runoff conveyed from the site? At a minimum, this description should answer:

- (1) whether existing drainage conveyance is natural or urban;
- (2) Is runoff from offsite conveyed through the site? if yes, quantify all offsite drainage areas, design flows, and locations where offsite flows enter the project site, and summarize how such flows are conveyed through the site;
- (3) Provide details regarding existing project site drainage conveyance network, including any existing storm drains, concrete channels, swales, detention facilities, storm water treatment facilities, natural or constructed channels; and
- (4) Identify all discharge locations from the existing project site along with a summary of conveyance system size and capacity for each of the discharge locations. Provide summary of the pre-project drainage areas and design flows to each of the existing runoff discharge locations.

Description / Additional Information:

The site is mass graded and runoff sheetflows mildly into two existing desilting basins located at the south side of the site. One drains to an existing 24" storm drain pipe located within Showroom Place. The second desilting basin drains east to another existing 18" storm drain. The existing site generates Q50= 24.2 CFS. The calculated flows for existing condition are based on flat barren slopes runoff coefficient of C=0.65

Form I-3B Page 4 of 10

Description of Proposed Site Development and Drainage Patterns

Project Description / Proposed Land Use and/or Activities:

The development consists of construction of one medical building with attached parking and landscaped areas including three biofiltration with partial retention facilities for water quality and flow control purposes. In general runoff from project will be directed into the proposed biofiltration basins underground storm drain pipes. After stormwater is treated and mitigated it will be conveyed to the two existing storm drain pipes mentioned before. Developed condition generates 42.1 CFS of runoff (undetained). The calculated flows for developed condition are based on Commercial site runoff coefficient of C=0.85

List/describe proposed impervious features of the project (e.g., buildings, roadways, parking lots, courtyards, athletic courts, other impervious features):

The project proposes impervious features such as: sidewalks, parking lot, drive aisles and one building which are approximately 70% of the site.

List/describe proposed pervious features of the project (e.g., landscape areas):

The project proposes previous features such as: landscaped areas and slopes and bioretention basins for approximately 30% of the site.

Does the project include grading and changes to site topography? ⊠ Yes

🗆 No

Description / Additional Information:

The propject proposes the precise grading for buildings, parking and landscape areas.

Does the project include changes to site drainage (e.g., installation of new storm water conveyance systems)?

🛛 Yes

🗆 No

If yes, provide details regarding the proposed project site drainage conveyance network, including storm drains, concrete channels, swales, detention facilities, storm water treatment facilities, natural or constructed channels, and the method for conveying offsite flows through or around the proposed project site. Identify all discharge locations from the proposed project site along with a summary of the conveyance system size and capacity for each of the discharge locations. Provide a summary of pre- and post-project drainage areas and design flows to each of the runoff discharge locations. Reference the drainage study for detailed calculations.

Describe proposed site drainage patterns::

The project has two drainage basins. The westerly one drains to the south via an underground storm drain system a proposed biofiltration basin for water quality and HMP purposes and then conveyed to Showroom Place via an existing public 24" RCP storm drain. The easterly basin also drains to the south to a second biofiltration basin which drainage is intercepted and conveyed easterly by an existing private 18" RCP storm drain to Yosemitie Drive at the Eastlake III Woods. Developed condition generates cfs of runoff (undetained). The calculated flows for developed condition are based on Commercial site runoff coefficient of C=0.85

BASIN 1- PEAK FLOW TABLE (CFS) AT DETENTION BASIN

STORM	EXISTING CONDITION	PROPOSED CONDITION	PROPOSED CONDITION
EVENT		BEFORE DETENTION	AFTER DETENTION
50-YEAR	13.4	24.5	10.7

BASIN 2- PEAK FLOW TABLE (CFS) AT DETENTION BASIN

STORM	EXISTING CONDITION	PROPOSED CONDITION	PROPOSED CONDITION
EVENT		BEFORE DETENTION	AFTER DETENTION
50-YEAR	10.8	17.6	7.8

	Form I-3B Page 6 of 10
	Identify whether any of the following features, activities, and/or pollutant source areas will be present
	(select all that apply):
	🛛 On-site storm drain inlets
1	\Box Interior floor drains and elevator shaft sump <code>pumps</code>
	Interior parking garages
	\Box Need for future indoor & structural pest control
	⊠ Landscape/Outdoor Pesticide Use
	\Box Pools, spas, ponds, decorative fountains, and other water features
	Food service
	⊠ Refuse areas
	Industrial processes
[Outdoor storage of equipment or materials
	Vehicle and Equipment Cleaning
1	Vehicle/Equipment Repair and Maintenance
	Fuel Dispensing Areas
1	⊠ Loading Docks
1	⊠ Fire Sprinkler Test Water
1	🛛 Miscellaneous Drain or Wash Water
1	oxtimes Plazas, sidewalks, and parking lots
	Description / Additional Information:
(Click here to enter text

Form I-3B Page 7 of 10

Identification and Narrative of Receiving Water and Pollutants of Concern

Describe flow path of storm water from the project site discharge location(s), through urban storm conveyance systems as applicable, to receiving creeks, rivers, and lagoons as applicable, and ultimate discharge to the Pacific Ocean (or bay, lagoon, lake or reservoir, as applicable): The project discharges into the MS4 located at Showroom Place, which inturn drains south into Fenton

Street and into Telegraph Canyon Creek, then into the San Diego Bay and eventually into the Pacific Ocean.

List any 303(d) impaired water bodies within the path of storm water from the project site to the Pacific Ocean (or bay, lagoon, lake or reservoir, as applicable), identify the pollutant(s)/stressor(s) causing impairment, and identify any TMDLs and/or Highest Priority Pollutants from the WQIP for the impaired water bodies:

303(d) Impaired Water Body	Pollutant(s)/Stressor(s)	TMDLs/ WQIP Highest Priority Pollutant
San Diego Bay	PCBs	TMDL required
Telegraph Canyon Creek	Selenium	TMDL required
Click here to enter text	Click here to enter text.	Click here to enter text.

Identification of Project Site Pollutants*

*Identification of project site pollutants is only required if flow-thru treatment BMPs are implemented onsite in lieu of retention or biofiltration BMPs (note the project must also participate in an alternative compliance program unless prior lawful approval to meet earlier PDP requirements is demonstrated)

Identify pollutants anticipated from the project site based on all proposed use(s) of the site (see BMP Design Manual Appendix B.6):

Pollutant	Not Applicable to the Project Site	Anticipated from the Project Site	Also a Receiving Water Pollutant of Concern
Sediment			
Nutrients			
Heavy Metals			
Organic Compounds			
Trash & Debris			
Oxygen Demanding Substances			
Oil & Grease			
Bacteria & Viruses			
Pesticides			

Form I-3B Page 8 of 10	
Hydromodification Management Requirements	
Do hydromodification management requirements apply (see Section 1.6 of the BMP Design Manu	ial)?
Yes, hydromodification management flow control structural BMPs required.	
□ No, the project will discharge runoff directly to existing underground storm drains discharging	
directly to water storage reservoirs, lakes, enclosed embayments, or the Pacific Ocean.	
🗆 No, the project will discharge runoff directly to conveyance channels whose bed and bank are	
concrete-lined all the way from the point of discharge to water storage reservoirs, lakes	5,
enclosed embayments, or the Pacific Ocean.	
\square No, the project will discharge runoff directly to an area identified as appropriate for an exempt	tion by
the WMAA for the watershed in which the project resides.	
Description / Additional Information (to be provided if a 'No' answer has been selected above):	
Click here to enter text	
Critical Coarse Sediment Yield Areas*	
*This Section only required if hydromodification management requirements apply	
Based on the maps provided within the WMAA, do potential critical coarse sediment yield areas e	exist
within the project drainage boundaries?	
Yes	
No, No critical coarse sediment yield areas to be protected based on WMAA maps	
If yes, have any of the optional analyses presented in Section 6.2 of the BMP Design Manual been	n -
performed?	
6.2.1 Verification of Geomorphic Landscape Units (GLUs) Onsite	
6.2.2 Downstream Systems Sensitivity to Coarse Sediment	
6.2.3 Optional Additional Analysis of Potential Critical Coarse Sediment Yield Areas Onsite	
No optional analyses performed, the project will avoid critical coarse sediment yield areas iden based on WMAA maps	ntified
If optional analyses were performed, what is the final result?	
No critical coarse sediment yield areas to be protected based on verification of GLUs onsite	
□ Critical coarse sediment yield areas exist but additional analysis has determined that protection	n is no
required. Documentation attached in Attachment 2.b of the SWQMP.	11 13 110
Critical coarse sediment yield areas exist and require protection. The project will implement	
management measures described in Sections 6.2.4 and 6.2.5 as applicable, and the area	as are
identified on the SWQMP Exhibit.	as are
Discussion / Additional Information:	
Click here to enter text	
Form I-3B Page 9 of 10	

Flow Control for Post-Project Runoff*

*This Section only required if hydromodification management requirements apply

List and describe point(s) of compliance (POCs) for flow control for hydromodification management (see Section 6.3.1). For each POC, provide a POC identification name or number correlating to the project's HMP Exhibit and a receiving channel identification name or number correlating to the project's HMP Exhibit.

The project consists of two POC's located at the project boundary which discharge to two different storm drain systems. POC#1 discharges to an existing storm drain pipe on Showroom Place while POC#2 discharges to second existing storm drain pipe which drains southwestery to the easterly property. No diversion of flow is proposed for these POC's.

Has a geomorphic assessment been performed for the receiving channel(s)?

No, the low flow threshold is 0.1Q2 (default low flow threshold)

 \Box Yes, the result is the low flow threshold is 0.1Q2

 \Box Yes, the result is the low flow threshold is 0.3Q2

 \square Yes, the result is the low flow threshold is 0.5Q2

If a geomorphic assessment has been performed, provide title, date, and preparer: Click here to enter text

Discussion / Additional Information: (optional) Click here to enter text

Form I-3B Page 10 of 10

Other Site Requirements and Constraints

When applicable, list other site requirements or constraints that will influence storm water management design, such as zoning requirements including setbacks and open space, or local codes governing minimum street width, sidewalk construction, allowable pavement types, and drainage requirements.

None

Optional Additional Information or Continuation of Previous Sections As Needed

This space provided for additional information or continuation of information from previous sections as needed.

None

Source Control BMP Checklist for All Development Projects

Form I-4

(Standard Projects and PDPs)

Project Identification

Project Name: EASTLAKE BEHAVIORAL HEALTH HOSPITAL

Permit Application Number: DR 19-0012, CUP 19-0010, PER 19-0006

Source Control BMPs

All development projects must implement source control BMPs SC-1 through SC-6 where applicable and feasible. See Chapter 4 and Appendix E of the manual for information to implement source control BMPs shown in this checklist.

Answer each category below pursuant to the following.

- "Yes" means the project will implement the source control BMP as described in Chapter 4 and/or Appendix E of the manual. Discussion / justification is not required.
- "No" means the BMP is applicable to the project but it is not feasible to implement. Discussion / justification must be provided.
- "N/A" means the BMP is not applicable at the project site because the project does not include the feature that is addressed by the BMP (e.g., the project has no outdoor materials storage areas). Discussion / justification may be provided.

Source Control Requirement		Applied?	
SC-1 Prevention of Illicit Discharges into the MS4	⊠Yes	□No	□n/a
Discussion / justification if SC-1 not implemented:		- 11-	
Click here to enter text			
SC-2 Storm Drain Stenciling or Signage	⊠Yes	□No	□N/A
Discussion / justification if SC-2 not implemented:			
Click here to enter text			
SC-3 Protect Outdoor Materials Storage Areas from Rainfall, Run-On,	□Yes	□No	⊠N/A
Runoff, and Wind Dispersal			
Discussion / justification if SC-3 not implemented:			
The is no outdoor storage proposed.			
SC-4 Protect Materials Stored in Outdoor Work Areas from Rainfall,	□Yes	□No	⊠N/A
Run-On, Runoff, and Wind Dispersal			
Discussion / justification if SC-4 not implemented:			
There are no materials stored outdoor			
Form I-4 Page 2 of 2			

Source Control Requirement		Applied?	
SC-5 Protect Trash Storage Areas from Rainfall, Run-On, Runoff, and Wind Dispersal	⊠Yes	□No	□n/A
Discussion / justification if SC-5 not implemented:			
Click here to enter text			
	[
SC-6 Additional BMPs Based on Potential Sources of Runoff Pollutants			
(must answer for each source listed below)		□No	
	⊠Yes		□N/A
Interior floor drains and elevator shaft sump pumps	□Yes	□No	□N/A
□ Interior parking garages	□Yes	□No	□N/A
□ Need for future indoor & structural pest control	□Yes	□No	□N/A
Landscape/outdoor pesticide use	⊠Yes	□No	□N/A
Pools, spas, ponds, decorative fountains, and other water features	□Yes	□No	□n/a
Food service	⊠Yes	□No	□n/a
	□Yes	□No	□N/A
Industrial processes	□Yes	□No	□n/a
Outdoor storage of equipment or materials	□Yes	□No	□n/a
Vehicle and equipment cleaning	□Yes	□No	□N/A
□ Vehicle/equipment repair and maintenance	□Yes	□No	□N/A
Fuel dispensing areas	□Yes	□No	□n/a
Loading docks	⊠Yes	□No	□n/a
Fire sprinkler test water	⊠Yes	□No	□n/a
Miscellaneous drain or wash water	□Yes	□No	□n/a
Plazas, sidewalks, and parking lots	⊠Yes	□No	□n/a
Discussion / justification if SC-6 not implemented. Clearly identify which sources of runoff pollutants are			lutants are
discussed. Justification must be provided for <u>all</u> "No" answers shown al			f
Site does not propose: interior floor drains, interior parking garage, wa			orage ot
equipment or materials, vehicle/equipment cleaning nor maintenance,	tuel disper	ising areas.	

Site Design BMP Checklist for All Development Projects

Form I-5

(Standard Projects and PDPs)

Project Identification

Project Name: EASTLAKE BEHAVIORAL HEALTH HOSPITAL

Permit Application Number: DR 19-0012, CUP 19-0010, PER 19-0006

Site Design BMPs

All development projects must implement site design BMPs SD-1 through SD-8 where applicable and feasible. See Chapter 4 and Appendix E of the manual for information to implement site design BMPs shown in this checklist.

Answer each category below pursuant to the following.

- "Yes" means the project will implement the site design BMP as described in Chapter 4 and/or Appendix E of the manual. Discussion / justification is not required.
- "No" means the BMP is applicable to the project but it is not feasible to implement. Discussion / justification must be provided.
- "N/A" means the BMP is not applicable at the project site because the project does not include the feature that is addressed by the BMP (e.g., the project site has no existing natural areas to conserve). Discussion / justification may be provided.

Site Design Requirement		Applied?	
SD-1 Maintain Natural Drainage Pathways and Hydrologic Features	□Yes	No	⊠N/A
Discussion / justification if SD-1 not implemented:			
There is no existing natural drainage pathway nor hydrologic features			
	_		
SD-2 Conserve Natural Areas, Soils, and Vegetation	⊠Yes	□No	□n/a
Discussion / justification if SD-2 not implemented:			
Click here to enter text			
SD-3 Minimize Impervious Area	⊠Yes	□No	□N/A
Discussion / justification if SD-3 not implemented:			
32% of the site will be landscaped			
SD-4 Minimize Soil Compaction	⊠Yes	□No	□N/A
Discussion / justification if SD-4 not implemented:			
PR-1 BASINS BOTTOMS PROVIDE MINIMIZED COMPACTION			

Form I-5 Page 2 of 2		1000	the second second
Site Design Requirement		Applied?	
SD-5 Impervious Area Dispersion	⊠Yes	□No	$\Box N/A$
Discussion / justification if SD-5 not implemented:			
SD-6 Runoff Collection	□Yes	□No	⊠N/A
Discussion / justification if SD-6 not implemented: Per Form B.3-1 Harvest and Use is infeasible			
SD-7 Landscaping with Native or Drought Tolerant Species Discussion / justification if SD-7 not implemented: Click here to enter text.	⊠Yes	□No	□n/A
SD-8 Harvesting and Using Precipitation	□Yes	□No	×N/A
Discussion / justification if SD-8 not implemented: Per Form B.3-1 Harvest and Use is infeasible			

Summary of PDP Structural BMPs

Form I-6 (For PDPs)

Project Identification

Project Name: EASTLAKE BEHAVIORAL HEALTH HOSPITAL

Permit Application Number: DR 19-0012, CUP 19-0010, PER 19-0006

PDP Structural BMPs

All PDPs must implement structural BMPs for storm water pollutant control (see Chapter 5 of the manual). Selection of PDP structural BMPs for storm water pollutant control must be based on the selection process described in Chapter 5. PDPs subject to hydromodification management requirements must also implement structural BMPs for flow control for hydromodification management (see Chapter 6 of the manual). Both storm water pollutant control and flow control for hydromodification management can be achieved within the same structural BMP(s).

PDP structural BMPs must be verified by the local jurisdiction at the completion of construction. This may include requiring the project owner or project owner's representative to certify construction of the structural BMPs (see Section 1.12 of the manual). PDP structural BMPs must be maintained into perpetuity, and the local jurisdiction must confirm the maintenance (see Section 7 of the manual).

Use this form to provide narrative description of the general strategy for structural BMP implementation at the project site in the box below. Then complete the PDP structural BMP summary information sheet (page 3 of this form) for each structural BMP within the project (copy the BMP summary information page as many times as needed to provide summary information for each individual structural BMP).

Describe the general strategy for structural BMP implementation at the site. This information must describe how the steps for selecting and designing storm water pollutant control BMPs presented in Section 5.1 of the manual were followed, and the results (type of BMPs selected). For projects requiring hydromodification flow control BMPs, indicate whether pollutant control and flow control BMPs are integrated or separate.

One PR-1 biofiltration basin is being proposed for this project on the southwest corner of the site for HMP and pollutant control of DMA 1. A second PR-1 biofiltration basin is being proposed on the southeast corner of the site for HMP and pollutant control of DMA 2.

(Continue on page 2 as necessary.)

Form I-6 Page 3 of 6 (Copy as many as needed)

Structural BMP Summary Information

(Copy this page as needed to provide information for each individual proposed structural BMP)

Structural BMP ID No.: 1

Construction Plan Sheet No.: C-5

Type of structural BMP:

□ Retention by harvest and use (HU-1)

□ Retention by infiltration basin (INF-1)

□ Retention by bioretention (INF-2)

□ Retention by permeable pavement (INF-3)

Partial retention by biofiltration with partial retention (PR-1)

□ Biofiltration (BF-1)

□ Flow-thru treatment control with prior lawful approval to meet earlier PDP requirements (provide BMP type/description in discussion section below)

Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)

□ Flow-thru treatment control with alternative compliance (provide BMP type/description in discussion section below)

Detention pond or vault for hydromodification management

□ Other (describe in discussion section below)

Purpose:

Pollutant control only

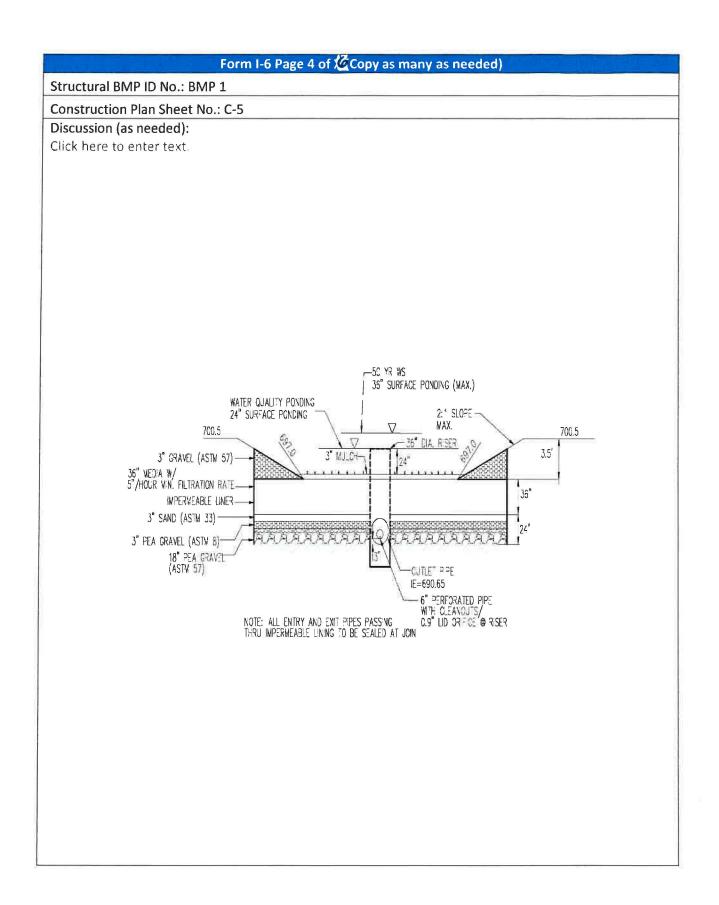
□ Hydromodification control only

Combined pollutant control and hydromodification control

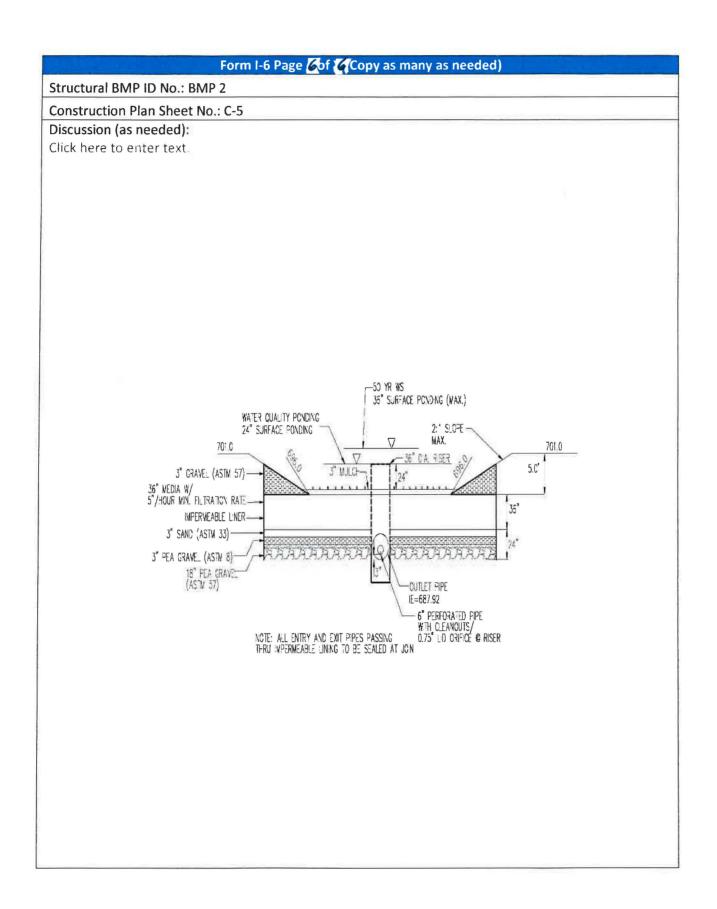
Pre-treatment/forebay for another structural BMP

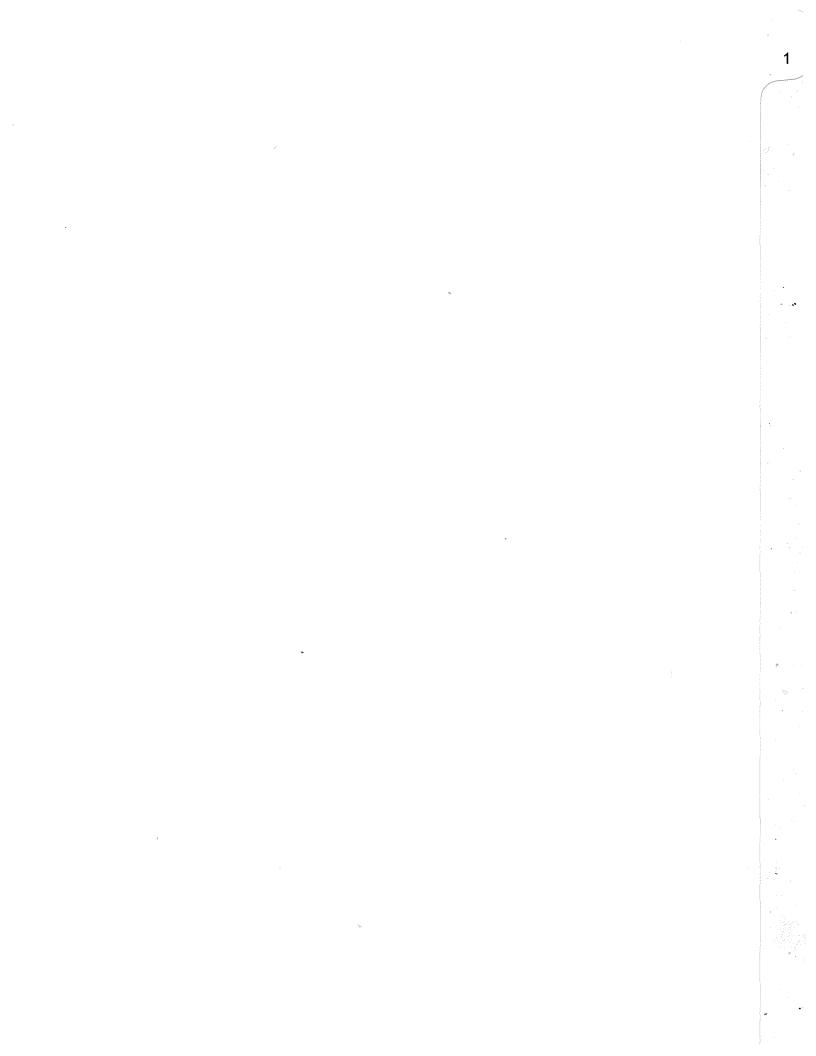
□ Other (describe in discussion section below)

Who will certify construction of this BMP? Provide name and contact information for the party responsible to sign BMP verification forms if required by the City Engineer (See Section 1.12 of the manual)	Kamal S. Swies, P.E. 48592 7801 Mission Center Court, Suite 100 San Diego, CA 92108 (619) 296-5565 kss@ks-engr.com
Who will be the final owner of this BMP?	Eastlake Behavioral Center
Who will maintain this BMP into perpetuity?	Eastlake Behavioral Center
What is the funding mechanism for maintenance?	Private
Discussion (as needed): Click here to enter text.	



	opy as many as needed) Immary Information
	ion for each individual proposed structural BMP)
Structural BMP ID No.: 2	
Construction Plan Sheet No.: C-5	
Type of structural BMP:	
Retention by harvest and use (HU-1)	
Retention by infiltration basin (INF-1)	
Retention by bioretention (INF-2)	
\Box Retention by permeable pavement (INF-3)	
oxtimes Partial retention by biofiltration with partial rete	ntion (PR-1)
□ Biofiltration (BF-1)	
Flow-thru treatment control with prior lawful app type/description in discussion section below	proval to meet earlier PDP requirements (provide BMI ow)
	tment/forebay for an onsite retention or biofiltration ndicate which onsite retention or biofiltration BMP i
Flow-thru treatment control with alternative cor section below)	npliance (provide BMP type/description in discussion
Detention pond or vault for hydromodification m	nanagement
Other (describe in discussion section below)	
Purpose:	
Pollutant control only	
Hydromodification control only	
Combined pollutant control and hydromodification	
Pre-treatment/forebay for another structural BN	1P.
Other (describe in discussion section below)	
Who will certify construction of this BMP?	Kamal S. Swies, P.E. 48592
Provide name and contact information for the	7801 Mission Center Court, Suite 100
party responsible to sign BMP verification forms if	San Diego, CA 92108
required by the City Engineer (See Section 1.12 of	(619) 296-5565
the manual)	kss@ks-engr.com
Who will be the final owner of this BMP?	Eastlake Behavioral Center
Who will maintain this BMP into perpetuity?	Eastlake Behavioral Center
What is the funding mechanism for maintenance?	Private
Discussion (as needed):	
Click here to enter text.	





ATTACHMENT 1 BACKUP FOR PDP POLLUTANT CONTROL BMPS

This is the cover sheet for Attachment 1.

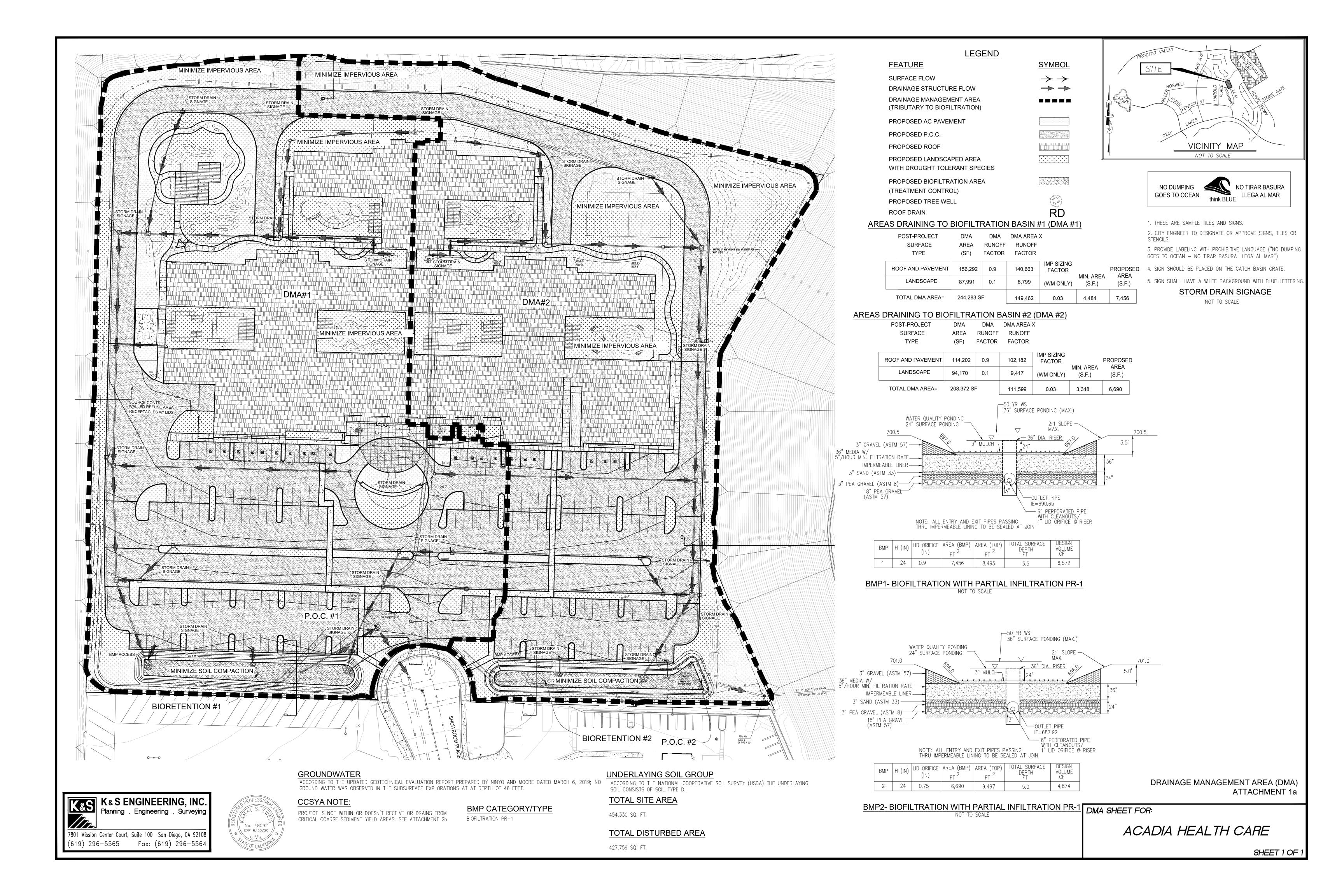
Indicate which items are included behind this cover sheet:

Attachment Sequence	Contents	Checklist
Attachment 1a	DMA Exhibit (Required) See DMA Exhibit Checklist on the back of this Attachment cover sheet.	⊠ Included
Attachment 1b	Tabular Summary of DMAs Showing DMA ID matching DMA Exhibit, DMA Area, and DMA Type (Required)* *Provide table in this Attachment OR on DMA Exhibit in Attachment 1a	 ☑ Included on DMA Exhibit in Attachment 1a ☑ Included as Attachment 1b, separate from DMA Exhibit
Attachment 1c	Form I-7, Harvest and Use Feasibility Screening Checklist (Required unless the entire project will use infiltration BMPs) Refer to Appendix B.3-1 of the BMP Design Manual to complete Form I-7.	⊠ Included □ Not included because the entire project will use infiltration BMPs
Attachment 1d	Form I-8, Categorization of Infiltration Feasibility Condition (Required unless the project will use harvest and use BMPs) Refer to Appendices C and D of the BMP Design Manual to complete Form I-8.	 ☑ Included □ Not included because the entire project will use harvest and use BMPs
Attachment 1e	Pollutant Control BMP Design Worksheets / Calculations (Required) Refer to Appendices B and E of the BMP Design Manual for structural pollutant control BMP design guidelines	⊠ Included

Use this checklist to ensure the required information has been included on the DMA Exhibit:

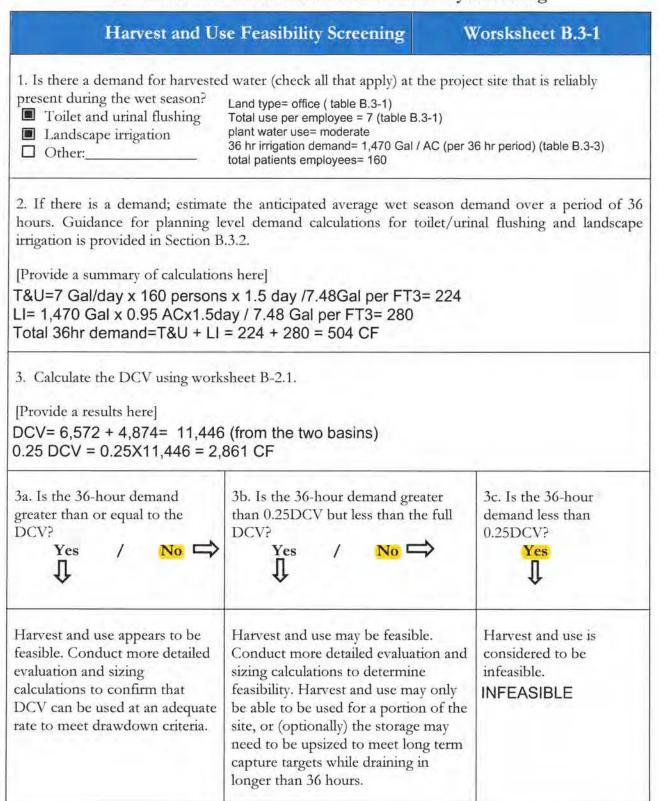
The DMA Exhibit must identify:

- ☑ Underlying hydrologic soil group
- $oxed{intermatter}$ Approximate depth to groundwater
- Existing natural hydrologic features (watercourses, seeps, springs, wetlands)
- \Box Critical coarse sediment yield areas to be protected
- \boxtimes Existing topography and impervious areas
- \boxtimes Existing and proposed site drainage network and connections to drainage offsite
- □ Proposed demolition
- \boxtimes Proposed grading
- \boxtimes Proposed impervious features
- Proposed design features and surface treatments used to minimize imperviousness
- ☑ Drainage management area (DMA) boundaries, DMA ID numbers, and DMA areas (square footage or acreage), and DMA type (i.e., drains to BMP, self-retaining, or self-mitigating)
- ☑ Potential pollutant source areas and corresponding required source controls (see Chapter 4, Appendix E.1, and Form I-3B)
- Structural BMPs (identify location, type of BMP, and size/detail)



Storm Water Pollutant Control Hydrologic Calculations and Sizing Methods

Worksheet B.3-1. Harvest and Use Feasibility Screening





Appendix C: Geotechnical and Groundwater Investigation Requirements

Worksheet C.4-1: Categorization of Infiltration Feasibility Condition

Categ	tegorization of Infiltration Feasibility Condition Workshe		et C.4-1	
Would	Full Infiltration Feasibility Screening Criteria infiltration of the full design volume be feasible from a physi able consequences that cannot be reasonably mitigated?	ical perspecti	ve without an	
Criteria	Screening Question	Yes	No	

Provide basis:

As presented in the project geotechnical evaluation report (Ninyo & Moore, 2019), in-situ infiltration rates at the site were measured between 0.02 inches per hour and 0.47 inches per hour. Any infiltration system utilizing these results should apply the appropriate factor of safety to determine applicable site infiltration rates prior to design. For this project, a Suitability Assessment Safety Factor of 2.25 should be used. The design safety factor shall be determined by the design engineer.

Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability.

2	Can infiltration greater than 0.5 inches per hour be allowed without increasing risk of geotechnical hazards (slope stability, groundwater mounding, utilities, or other factors) that cannot be mitigated to an acceptable level? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.2.		\checkmark
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Provide basis:

As discussed in the project geotechnical evaluation report (Ninyo & Moore, 2019), laboratory testing of the subsurface soils indicated the presence of soils with high expansion potential. Infiltration of storm water into expansive soils is not recommended. Additionally, fill slopes up to 90 feet in height are present on the west, east, and north portions of the site. Infiltration within 50 feet of the top of a slope is anticipated to induce seepage on the slope face and increase the risk of slope failures in these areas. As stated in the project geotechnical evaluation report, Ninyo & Moore recommends that the bottom and sides of stormwater control devices be lined with an impermeable liner.

Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability.



Appendix C: Geotechnical and Groundwater Investigation Requirements

	Worksheet C.4-1 Page 2 of 4		
Criteria	Screening Question	Yes	No
3	Can infiltration greater than 0.5 inches per hour be allowed without increasing risk of groundwater contamination (shallow water table, storm water pollutants or other factors) that cannot be mitigated to an acceptable level? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.	\checkmark	
Provide l	Dasis:		
not en Based	cussed in the project geotechnical evaluation report (Ninyo & Moo countered during our subsurface exploration and is anticipated at I on the measured infiltration rates and the anticipated groundwate not likely to have a significant impact on groundwater contaminati	depths in exc er depth, infiltr	ess of 60 feet.
	ze findings of studies; provide reference to studies, calculations, maps, d n of study/data source applicability. Can infiltration greater than 0.5 inches per hour be allowed without causing potential water balance issues such as change of	ata sources, etc	:. Provide narrativo
	seasonality of ephemeral streams or increased discharge of contaminated groundwater to surface waters? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C 3	\checkmark	
Provide b	contaminated groundwater to surface waters? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.		
neare	contaminated groundwater to surface waters? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3. Masis: d on the measured infiltration rates and the distance between the est surface water (Salt Creek), infiltration at the site is not likely to vater balance of the creek or increased discharge of contaminated	have a signifi	cant impact to
Base nearc the w wate	contaminated groundwater to surface waters? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3. Masis: d on the measured infiltration rates and the distance between the est surface water (Salt Creek), infiltration at the site is not likely to vater balance of the creek or increased discharge of contaminated	have a signifi I groundwater	cant impact to to surface

*To be completed using gathered site information and best professional judgment considering the definition of MEP in the MS4 Permit. Additional testing and/or studies may be required by City Engineer to substantiate findings.



Worksheet C.4-1 Page 3 of 4

Part 2 - Partial Infiltration vs. No Infiltration Feasibility Screening Criteria

Would infiltration of water in any appreciable amount be physically feasible without any negative consequences that cannot be reasonably mitigated?

Criteria	Screening Question	Yes	No
5	Do soil and geologic conditions allow for infiltration in any appreciable rate or volume? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.2 and Appendix D.	\checkmark	

Provide basis:

As presented in the project geotechnical evaluation report (Ninyo & Moore, 2019), in-situ infiltration rates at the site were measured between 0.02 inches per hour and 0.47 inches per hour. Any infiltration system utilizing these results should apply the appropriate factor of safety to determine applicable site infiltration rates prior to design. For this project, a Suitability Assessment Safety Factor of 2.25 should be used. The design safety factor shall be determined by the design engineer.

Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.

6	Can Infiltration in any appreciable quantity be allowed without increasing risk of geotechnical hazards (slope stability, groundwater mounding, utilities, or other factors) that cannot be mitigated to an acceptable level? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.2.	~
---	---	---

Provide basis:

As discussed in the project geotechnical evaluation report (Ninyo & Moore, 2019), laboratory testing of the subsurface soils indicated the presence of soils with high expansion potential. Infiltration of storm water into expansive soils is not recommended. Additionally, fill slopes up to 90 feet in height are present on the west, east, and north portions of the site. Infiltration within 50 feet of the top of a slope is anticipated to induce seepage on the slope face and increase the risk of slope failures in these areas. As stated in the project geotechnical evaluation report, Ninyo & Moore recommends that the bottom and sides of stormwater control devices be lined with an impermeable liner.

Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.



Appendix C: Geotechnical and Groundwater Investigation Requirements

	Worksheet C.4-1 Page 4 of 4		
Criteria	Screening Question	Yes	No
7	Can Infiltration in any appreciable quantity be allowed without posing significant risk for groundwater related concerns (shallow water table, storm water pollutants or other factors)? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.	~	
Provide b	asis:		
not en Based site is Summariz	cussed in the project geotechnical evaluation report (Ninyo & Mod countered during our subsurface exploration and is anticipated at on the measured infiltration rates and the anticipated groundwate not likely to have a significant impact on groundwater contaminat e findings of studies; provide reference to studies, calculations, maps, d of study/data source applicability and why it was not feasible to mitigate 2	depths in excess er depth, infiltratio ion. ata sources, etc. Pr	s of 60 feet. on at the rovide narrative
	Can infiltration be allowed without violating downstream water		5.
8	rights? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.	v	
neare	isis: d on the measured infiltration rates and the distance between the st surface water (Salt Creek), infiltration at the site is not likely to ater balance of the creek or increased discharge of contaminated	have a significar	nt impact to
Base neare the w water	isis: d on the measured infiltration rates and the distance between the st surface water (Salt Creek), infiltration at the site is not likely to ater balance of the creek or increased discharge of contaminated	have a significar I groundwater to ata sources, etc. Pr	nt impact to surface rovide narrativ

*To be completed using gathered site information and best professional judgment considering the definition of MEP in the MS4 Permit. Additional testing and/or studies may be required by Agency/Jurisdictions to substantiate findings



BMP 1

Appendix D: Approved Infiltration Rate Assessment Methods

Factor of Safety and Design Infiltration Rate Worksheet			Worksheet D.5-1		
Fac	tor Category	Factor Description	Assigned Weight (w)	Factor Value (v)	$\begin{array}{ c c } Product (p) \\ p = w x v \end{array}$
			0.25	2	0.5
		Predominant soil texture	0.25	3	0.75
А	Suitability	Site soil variability	0.25	3	0.75
	Assessment	Depth to groundwater / impervious layer	0.25	1	0.25
		Suitability Assessment Safety Factor,	0.0		
		Level of pretreatment/ expected sediment loads	0.5	1	0.5
В	Design	Redundancy/resiliency	0.25	1	0.25
	0	Compaction during construction	0.25	1	0.25
		Design Safety Factor, $S_B = \Sigma_p$			2
Con	nbined Safety Fa	ctor, $S_{total} = S_A x S_B$			2.25
	erved Infiltration rected for test-sp	n Rate, inch/hr, K _{observed} becific bias)			0.29
Design Infiltration Rate, in/hr,		ate, in/hr, K _{design} = K _{observed} / S _{total}			0.13
Sup	porting Data				

Worksheet D.5-1: Factor of Safety and Design Infiltration Rate Worksheet

Briefly describe infiltration test and provide reference to test forms:

Two infiltration tests were performed at each of the four proposed basin areas for a total of eight tests. Tests were performed in boreholes drilled to depths of 5 feet below existing grades. In-situ infiltration rates at the site were measured between 0.02 and 0.47 inches per hour. Test locations and infiltration tests results are presented in the project geotechnical evaluation report (Ninyo and Moore, 2019)

BMP 2

Appendix D: Approved Infiltration Rate Assessment Methods

Factor of Safety and Design Infiltration Rate Worksheet			AVABLABAAT I A-I			
Factor Category		Factor Description	Assigned Weight (w)	Factor Value (v)	$\begin{array}{c} Product (p) \\ p = w x v \end{array}$	
Ĩ.		Soil assessment methods 0	0.25	2	0.5	
		Predominant soil texture	0.25	3	0.75	
	Suitability	Site soil variability	0.25	3	0.75	
11	Assessment	Depth to groundwater / impervious layer	0.25	1	0.25	
		Suitability Assessment Safety Factor, $S_A = \Sigma p$				
	1	Level of pretreatment/ expected sediment loads	0.5	1	0.5	
В	Design	Redundancy/resiliency	0.25	1	0.25	
		Compaction during construction	0.25	1	0.25	
		Design Safety Factor, $S_B = \Sigma_P$				
Con	nbined Safety Fa	ctor, $S_{total} = S_A x S_B$			2.25	
	erved Infiltration rected for test-sp	n Rate, inch/hr, K _{observed} becific bias)			0.27	
Des	ign Infiltration R	ate, in/hr, K _{design} = K _{observed} / S _{total}			0.14	
Sup	porting Data	A THE REAL PROPERTY.		-		

Worksheet D.5-1: Factor of Safety and Design Infiltration Rate Worksheet

Briefly describe infiltration test and provide reference to test forms:

Two infiltration tests were performed at each of the four proposed basin areas for a total of eight tests. Tests were performed in boreholes drilled to depths of 5 feet below existing grades. In-situ infiltration rates at the site were measured between 0.02 and 0.47 inches per hour. Test locations and infiltration tests results are presented in the project geotechnical evaluation report (Ninyo and Moore, 2019)

DMA #1

Appendix B:

Storm Water Pollutant Control Hydrologic Calculations and Sizing Methods

Worksheet B.2-1. DCV

	Design Capture Volume		Design Capture Volume Worksheet B-2		B-2.1
1	85 th percentile 24-hr storm depth from Figure B.1-1	d=	0.53	inches	
2	Area tributary to BMP (s)	A=	5.6	acres	
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.61	unitless	
4	Street trees volume reduction	TCV=		cubic-feet	
5	Rain barrels volume reduction	RCV=	-	cubic-feet	
6	Calculate DCV =(3630 x C x d x A) – TCV - RCV	DCV=	6,572	cubic-feet	

WEIGHTED RUNOFF FACTOR EQUATION;

Wc=(C*)(AREA imp)+(C*)(AREA perv)/TOTAL AREA

Where:

Aimp=Tributary Area 156,292 sf Aperv=Tributary Area 87,991 sf

Wc=(0.90)(156,292sf)+(0.10)(87,991 sf)/244,283sf Wc= 0.61





Simple Sizing Method for Biofiltration BMPs BMP1 Worksheet B	.5-1	
1 Remaining DCV after implementing retention BMPs	6572	cubic-feet
Partial Retention		
2 Infiltration rate from Worksheet D.5-1 if partial infiltration is feasible	0.15	in/hr.
3 Allowable drawdown time for aggregate storage below the underdrain	36	hours
4 Depth of runoff that can be infiltrated [Line 2 x Line 3]	5.4	inches
5 Aggregate pore space	0.40	in/in
6 Required depth of gravel below the underdrain [Line 4/ Line 5]	13.5	inches
7 Assumed surface area of the biofiltration BMP	7456	sq-ft
8 Media retained pore storage	0.1	in/in
9 Volume retained by BMP [[Line 4 + (Line 12 x Line 8)]/12] x Line 7	3345	cubic-feet
10 DCV that requires biofiltration [Line 1 – Line 9]	3227	cubic-feet
BMP Parameters		
11 Surface Ponding [6 inch minimum, 12 inch maximum]	24	inches
12 Media Thickness [18 inches minimum], also add mulch layer thickness to this line for	r 36	inches
sizing calculations	24	
Aggregate Storage above underdrain invert (12 inches typical) – use 0	24	inches
13 inches for sizing if the aggregate is not over the entire bottom surface area		
14 Media available pore space	0.2	in/in
Media filtration rate to be used for sizing (5 in/hr. with no outlet	0.66	in/hr.
15 control; if the filtration rate is controlled by the outlet use the outlet controlled rate)		
	1	
Baseline Calculations		
16 Allowable Routing Time for sizing	6	hours
17 Depth filtered during storm [Line 15 x Line 16]	4	inches
18 Depth of Detention Storage [Line 11 + (Line 12 x Line 14)+ (Line 13 x Line 5)]	40.8	inches
19 Total Depth Treated [Line 17 + Line 18]	44.8	inches
Simple Sizing Method for Biofiltration BMPs Worksheet B.	5-1 (Pag	ge 2 of 2)
Option 1 – Biofilter 1.5 times the DCV		
20 Required biofiltered volume [1.5 x Line 10]	484	0.5 cubic-feet
21 Required Footprint [Line 20/ Line 19] x 12	129	7.7 sq-ft
Option 2 - Store 0.75 of remaining DCV in pores and ponding		
22 Required Storage (surface + pores) Volume [0.75 x Line 10]	1	cubic-feet
 Required Footprint [Line 22/ Line 18] x 12 		
		sq-f
Footprint of the BMP	laur	202 1 -
24 Area draining to the BMP		283 sq-f
Adjusted Runoff Factor for drainage area (Refer to Appendix B.1 and B.2)	0.61	1
26 Minimum BMP Footprint [Line 24 x Line 25 x 0.03]	447	0 sq-f
25 Footprint of the BMP = Maximum(Minimum(Line 21, Line 23), Line 26)	373	-
26 BMP Footprint Sizing Factor (Default 0.03 or an alternative minimum footprint s	zing 0.03	3 unitless
factor from Worksheet B.5-2, Line 11)		
	447	0 sq-f
7 Minimum BMP Footprint [Line 24 x Line 25 x Line 26] 4		

Check for Volume Reduction [Not applicable for No Infiltration Condition]			
29	Calculate the fraction of the DCV retained by the BMP [Line 9/ Line 1]	0.5	unitless
30	Minimum required fraction of DCV retained for partial infiltration condition	0.375	unitless
31	Is the retained DCV > 0.375 ? If the answer is no increase the footprint sizing factor in Line 26 until the answer is yes for this criterion.	🕱 Yes	□ No

DMA #2

Worksheet B.2-1. DCV

Design Capture Volume		Worksheet B-2.1		
1	85 th percentile 24-hr storm depth from Figure B.1-1	d=	0.53	inches
2	Area tributary to BMP (s)	A=	4.78	acres
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.53	unitless
4	Street trees volume reduction	TCV=		cubic-feet
5	Rain barrels volume reduction	RCV=	-	cubic-feet
6	Calculate DCV = $(3630 \times C \times d \times A) - TCV - RCV$	DCV=	4,874	cubic-feet

WEIGHTED RUNOFF FACTOR EQUATION;

Wc=(C*)(AREA imp)+(C*)(AREA perv)/TOTAL AREA

Where:

Aimp=Tributary Area 114,202 sf Aperv=Tributary Area 94,170 sf

Wc=(0.90)(114,202sf)+(0.10)(94,170 sf)/208,372sf Wc= 0.53



	nple Sizing Method for Biofiltration BMPs BMP2 Worksheet B.		
	Remaining DCV after implementing retention BMPs	4874	cubic-feet
	tial Retention		
	Infiltration rate from Worksheet D.5-1 if partial infiltration is feasible	0.14	in/hr.
	Allowable drawdown time for aggregate storage below the underdrain	36	hours
	Depth of runoff that can be infiltrated [Line 2 x Line 3]	5	inches
_	Aggregate pore space	0.40	in/in
_	Required depth of gravel below the underdrain [Line 4/ Line 5]	12.6	inches
_	Assumed surface area of the biofiltration BMP	6690	sq-ft
	Media retained pore storage	0.1	in/in
-	Volume retained by BMP [[Line 4 + (Line 12 x Line 8)]/12] x Line 7	2784	cubic-feet
_	DCV that requires biofiltration [Line 1 – Line 9]	2090	cubic-feet
	P Parameters		1
_	Surface Ponding [6 inch minimum, 12 inch maximum]	24	inches
	Media Thickness [18 inches minimum], also add mulch layer thickness to this line for a calculations	36	inches
_	regate Storage above underdrain invert (12 inches typical) – use 0	24	inches
	inches for sizing if the aggregate is not over the entire bottom surface area		
14	Media available pore space	0.2	in/in
	lia filtration rate to be used for sizing (5 in/hr. with no outlet	0.58	in/hr.
	control; if the filtration rate is controlled by the outlet use the outlet controlled rate)		
	eline Calculations		
_	Allowable Routing Time for sizing	6	hours
17	Depth filtered during storm [Line 15 x Line 16]	3.5	inches
18	Depth of Detention Storage [Line 11 + (Line 12 x Line 14)+ (Line 13 x Line 5)]	40.8	inches
19	Total Depth Treated [Line 17 + Line 18]	44.3	inches
Sin	pple Sizing Method for Biofiltration BMPs Worksheet B.5-	1 (Pag	e 2 of 2)
Opt	ion 1 – Biofilter 1.5 times the DCV		
20	Required biofiltered volume [1.5 x Line 10]	3135	cubic-feet
21	Required Footprint [Line 20/ Line 19] x 12	850	sq-ft
	ion 2 - Store 0.75 of remaining DCV in pores and ponding	-	1 1
22	Required Storage (surface + pores) Volume [0.75 x Line 10]	T	cubic-feet
23	Required Footprint [Line 22/ Line 18] x 12		
-vorea			sq-ft
-	tprint of the BMP	lease	-
24	Area draining to the BMP	2083	1
25	Adjusted Runoff Factor for drainage area (Refer to Appendix B.1 and B.2)	Refer to Appendix B.1 and B.2) 0.53	
26	Minimum BMP Footprint [Line 24 x Line 25 x 0.03]		3 sq-ft
25	Footprint of the BMP = Maximum(Minimum(Line 21, Line 23), Line 26)		
26	BMP Footprint Sizing Factor (Default 0.03 or an alternative minimum footprint sizi factor from Worksheet B.5-2, Line 11)	0.03 or an alternative minimum footprint sizing 0.03	
		12212	
27	Minimum BMP Footprint [Line 24 x Line 25 x Line 26]	3313	3 sq-ft

Che	ck for Volume Reduction [Not applicable for No Infiltration Condition]		
29	Calculate the fraction of the DCV retained by the BMP [Line 9/ Line 1]	0.57	unitless
30	Minimum required fraction of DCV retained for partial infiltration condition	0.375	unitless
31	Is the retained DCV > 0.375 ? If the answer is no increase the footprint sizing factor in Line 26 until the answer is yes for this criterion.	🛛 Yes 🗆 No	



United States Department of Agriculture

NKCS Natural

Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for San Diego County Area, California

Acadia Healthcare Chula Vista



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

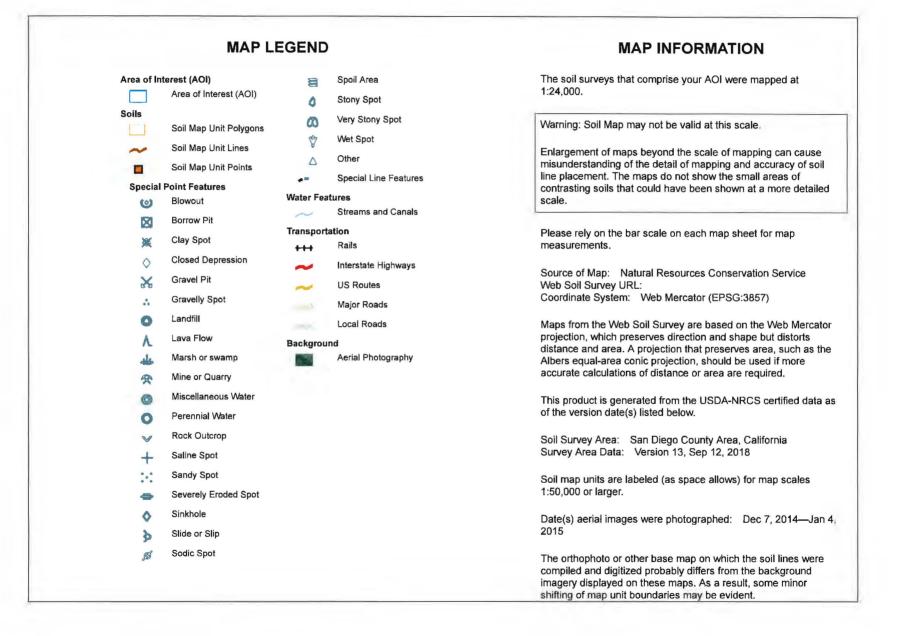
After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.





Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
DaC	Diablo clay, 2 to 9 percent slopes	6.5	62.1%
DaE	Diablo clay, 15 to 30 percent slopes	4.0	37.9%
Totals for Area of Interest		10.5	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

San Diego County Area, California

DaC-Diablo clay, 2 to 9 percent slopes

Map Unit Setting

National map unit symbol: hbb8 Elevation: 30 to 3,000 feet Mean annual precipitation: 12 to 35 inches Mean annual air temperature: 57 to 61 degrees F Frost-free period: 200 to 320 days Farmland classification: Farmland of statewide importance

Map Unit Composition

Diablo and similar soils: 85 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Diablo

Setting

Landform: Hillslopes Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Convex Parent material: Calcareous sandstone and shale

Typical profile

H1 - 0 to 15 inches: clay

H2 - 15 to 32 inches: silty clay loam, clay

H2 - 15 to 32 inches: weathered bedrock

H3 - 32 to 36 inches:

Properties and qualities

Slope: 2 to 9 percent
Depth to restrictive feature: 24 to 40 inches to paralithic bedrock
Natural drainage class: Well drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 10 percent
Available water storage in profile: Moderate (about 7.7 inches)

Interpretive groups

Land capability classification (irrigated): 3e Land capability classification (nonirrigated): 3e Hydrologic Soil Group: D Hydric soil rating: No

Minor Components

Altamont

Percent of map unit: 10 percent

Hydric soil rating: No

Linne

Percent of map unit: 3 percent Hydric soil rating: No

Olivenhain

Percent of map unit: 2 percent Hydric soil rating: No

DaE—Diablo clay, 15 to 30 percent slopes

Map Unit Setting

National map unit symbol: hbbb Elevation: 200 to 3,250 feet Mean annual precipitation: 9 to 25 inches Mean annual air temperature: 59 to 63 degrees F Frost-free period: 200 to 310 days Farmland classification: Not prime farmland

Map Unit Composition

Diablo and similar soils: 85 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Diablo

Setting

Landform: Hillslopes Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Convex Parent material: Calcareous sandstone and shale

Typical profile

H1 - 0 to 15 inches: clay H2 - 15 to 32 inches: silty clay loam, clay H2 - 15 to 32 inches: weathered bedrock H3 - 32 to 36 inches:

Properties and qualities

Slope: 15 to 30 percent
Depth to restrictive feature: 24 to 40 inches to paralithic bedrock
Natural drainage class: Well drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None

Calcium carbonate, maximum in profile: 10 percent Available water storage in profile: Moderate (about 7.7 inches)

Interpretive groups

Land capability classification (irrigated): 4e Land capability classification (nonirrigated): 4e Hydrologic Soil Group: D Ecological site: CLAYEY (1975) (R019XD001CA) Hydric soil rating: No

Minor Components

Altamont

Percent of map unit: 10 percent Hydric soil rating: No

Linne

Percent of map unit: 3 percent Hydric soil rating: No

Oliventain

Percent of map unit: 2 percent Hydric soil rating: No

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ATTACHMENT 2 BACKUP FOR PDP HYDROMODIFICATION CONTROL MEASURES

This is the cover sheet for Attachment 2.

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 \Box Mark this box if this attachment is empty because the project is exempt from PDP hydromodification management requirements.

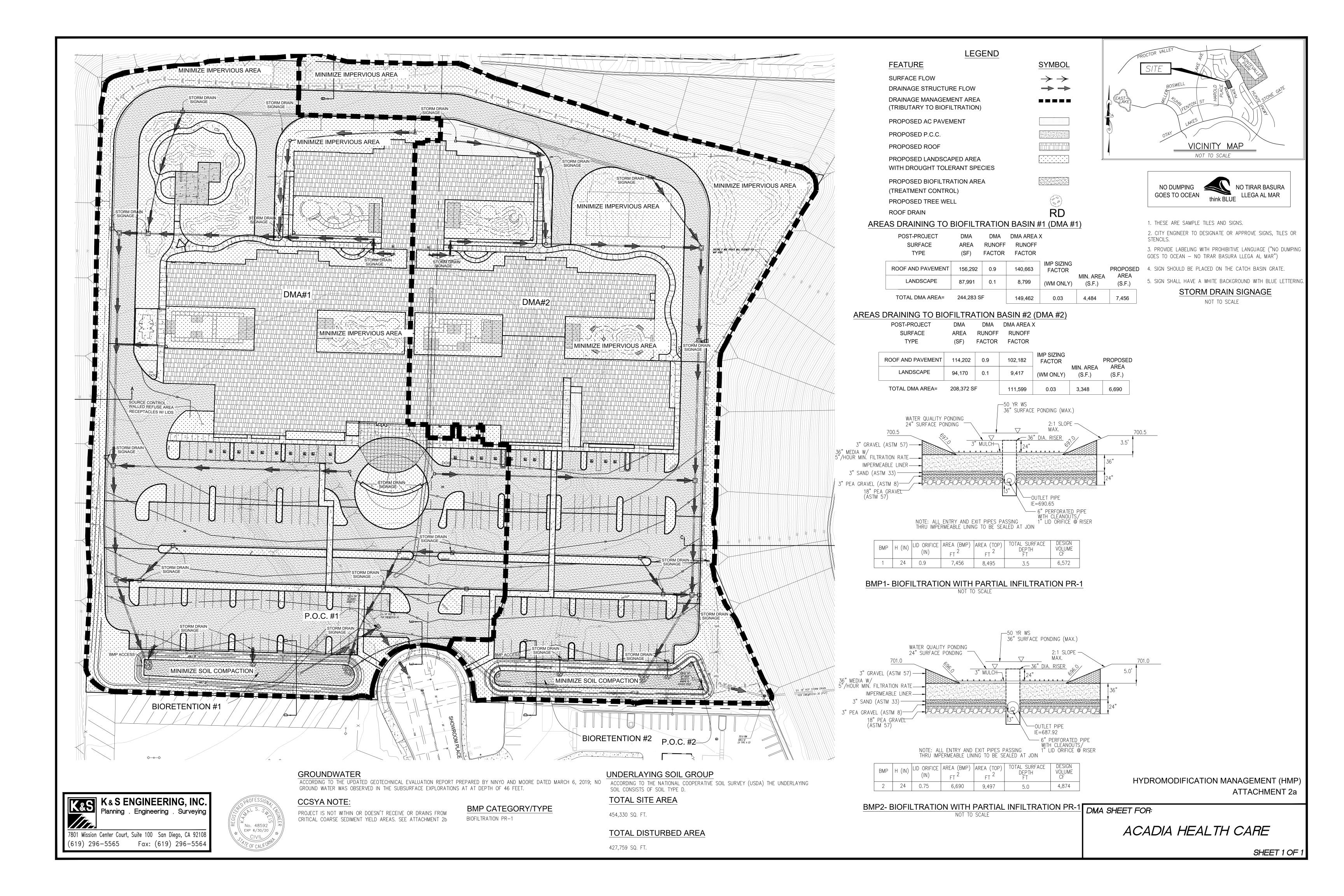
Attachment Sequence	Contents	Checklist
Attachment 2a	Hydromodification Management Exhibit (Required)	☑ Included See Hydromodification Management Exhibit Checklist on the back of this Attachment cover sheet.
	Management of Critical Coarse Sediment Yield Areas (WMAA Exhibit is required, additional analyses are optional)	 Exhibit showing project drainage boundaries marked on WMAA Critical Coarse Sediment Yield Area Map (Required)
Attachment 2b	See Section 6.2 of the BMP Design Manual.	 Optional analyses for Critical Coarse Sediment Yield Area Determination 6.2.1 Verification of Geomorphic Landscape Units Onsite 6.2.2 Downstream Systems Sensitivity to Coarse Sediment 6.2.3 Optional Additional Analysis of Potential Critical Coarse Sediment Yield Areas Onsite
Attachment 2c	Geomorphic Assessment of Receiving Channels (Optional) See Section 6.3.4 of the BMP Design Manual.	 Not performed Included Submitted as separate stand-alone document
Attachment 2d	Flow Control Facility Design, including Structural BMP Drawdown Calculations and Overflow Design Summary (Required) See Chapter 6 and Appendix G of the BMP Design Manual	 ☑ Included □ Submitted as separate stand-alone document
Attachment 2e	Vector Control Plan (Required when structural BMPs will not drain in 96 hours)	 □ Included ⊠ Not required because BMPs will drain in less than 96 hours

Indicate which items are included behind this cover sheet:

Use this checklist to ensure the required information has been included on the Hydromodification Management Exhibit:

The Hydromodification Management Exhibit must identify:

- Underlying hydrologic soil group
- Approximate depth to groundwater
- Existing natural hydrologic features (watercourses, seeps, springs, wetlands)
- Critical coarse sediment yield areas to be protected
- ⊠ Existing topography
- oxtimes Existing and proposed site drainage network and connections to drainage offsite
- \boxtimes Proposed grading
- ☑ Proposed impervious features
- \boxtimes Proposed design features and surface treatments used to minimize imperviousness
- ☑ Point(s) of Compliance (POC) for Hydromodification Management
- Existing and proposed drainage boundary and drainage area to each POC (when necessary, create separate exhibits for pre-development and post-project conditions)
- Structural BMPs for hydromodification management (identify location, type of BMP, and size/detail)



CRITICAL COARSE SEDIMENT YIELD AREAS MAP



Source: SWRCB



K&S ENGINEERING, INC. Planning Engineering Surveying

HMP Modeling for Hydromodification Compliance

DR 19-0012, CUP 19-0010, PER 19-0006

Prepared for:

EASTLAKE BEHAVIORAL HEALTH HOSPITAL

6100 Tower Circle, Suite 1000 Franklin, TN 91914 370067

Prepared by:

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> August 1, 2019 K&S JN 16-054



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8/1/2019 Date

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SECTION 2 - INTRODUCTION

SECTION 3 – PURPOSE OF THIS MODEL

SECTION 4 - HMP MODELING

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- 4.2 Summary of Proposed Condition

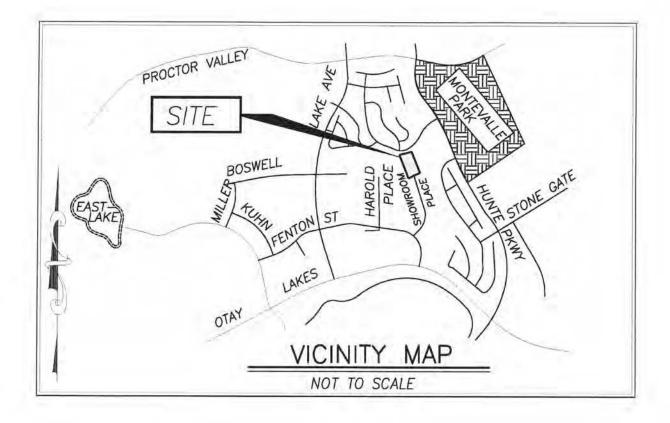
SECTION 5- REFERENCES

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APPENDIX A SDHM 3.1 Project Report Calculations and Charts

APPENDIX B HMP Exhibit

1 VICINITY MAP



2 INTRODUCTION

The project consists of two existing mass graded lots, i.e., Lots 7 & 8 of Map No. 14395

The project consists of grading and drainage improvements for one medical building with loading docks, parking, landscaping, storm drains and biofiltration BMP's for pollutant control and hydromodification purposes.

The subject report reflects the proposed precise grading and drainage as shown on precise grading plan Drawing No. C-1 through C-5 of DRC 19-0012.

3 PURPOSE OF THIS MODEL

Continuous simulation hydrologic modeling was conducted on this project to demonstrate compliance with the performance standards for hydromodification management in San Diego.

The San Diego Hydrology Model (SDHM) distributed by Clear Creek Solutions, Inc. was used for hydromodification management on the subject driveway.

The inputs required to develop SDHM models include rainfall, watershed characteristics, and BMP configurations. The Bonita gauge from the Project Clean Water website was used for this study. Default SDHM 3.1 pervious and impervious soil parameters used are found in Appendix A of this report.

Per the NRCS web soil survey, the project site is situated upon Class D soils. Soils have been assumed to be compacted to represent the current existing developed condition of the site, while fully compacted in the post developed conditions

4 HMP MODELING

In current existing conditions, the existing site is a mass graded development that includes vegetated areas areas.

Table 4.1 below illustrates the pre-developed area and impervious percentage accordingly.

POC	DMA ID	Tributary Area, A (Ac)	Impervious Percentage, Ip
POC-1	DMA-1	5.60	0%
POC-2	DMA-2	4.78	0%

4.1 Summary of Predeveloped Condition

Runoff from the improved areas of the project site are drained to two one onsite receiving LID BMPs. Once flows are routed via the proposed BMPs, onsite flows are then discharged to the existing discharge locations. Table 4.2 summarizes the post-developed area and impervious percentage accordingly.

POC	DMA ID	Tributary Area, A (Ac)	Impervious Percentage, Ip
POC-1	DMA-1	5.60	64%
POC-2	DMA-2	4.78	56%

4.2 Summary of Proposed Condition

Two HMP Biofiltration basins with Partial Retention are proposed within the project site and are responsible for performing hydromodification and water quality requirements for the project site. Runoff is discharged to these dual purpose water quality and HMP biofiltration basins prior to draining to the receiving POC's.

In developed conditions, the Biofiltration basin 1 will have surface depth of 24-inches. This Biofiltration basin will include a riser spillway structure set to an elevation of 24-inches from the surface bottom of the basin.

Underneath the basin invert is located the proposed LID biofiltration portion of the drainage facility. This portion of the basin is comprised of a 3-inch layer of mulch, an 36-inch layer of amended soil (a highly sandy, organic rich composite with an infiltration capacity of at least 5 inches/hr) and an 24-inch layer of gravel which includes the 6 inches of filter layer and the 3 inches of dead storage below the LID orifice. The BMP will be un lined to allow infiltration into the underlying soil.

Similarly, the Biofiltration basin 2 will have surface depth of 24-inches. This Biofiltration basin will include a riser spillway structure set to an elevation of 24-inches from the surface bottom of the basin.

Underneath the basin invert is located the proposed LID biofiltration portion of the drainage facility. This portion of the basin is comprised of a 3-inch layer of mulch, an 36-inch layer of amended soil (a highly sandy, organic rich composite with an infiltration capacity of at least 5 inches/hr) and an 24-inch layer of gravel which includes the 6 inches of filter layer and the 3 inches of dead storage below the LID orifice. The BMP will be un lined to allow infiltration into the underlying soil. The BMP will be un lined to allow infiltration into the underlying soil.

		DIMENSIONS					
BMP	Tributary Area (Ac)	BMP Area, (ft ²)	LID Orifice (in)	Gravel Depth (in)	Depth to Top of Riser(ft)	Weir Perimeter Length (ft)	Total Surface Depth (ft)
1	5.60	7,456	1.0"	24"	2.0'	9.42'	3.5'
2	4.78	6,690	1.0"	24"	2.0'	4.42'	5.0"

TABLE 3 – SUMMARY OF DEVELOPED DUAL PURPOSE BMP

Water Quality BMP Sizing

The BMPs have been designed in accordance with City of Chula Vista 2015 BMP Design Manual sizing criteria.

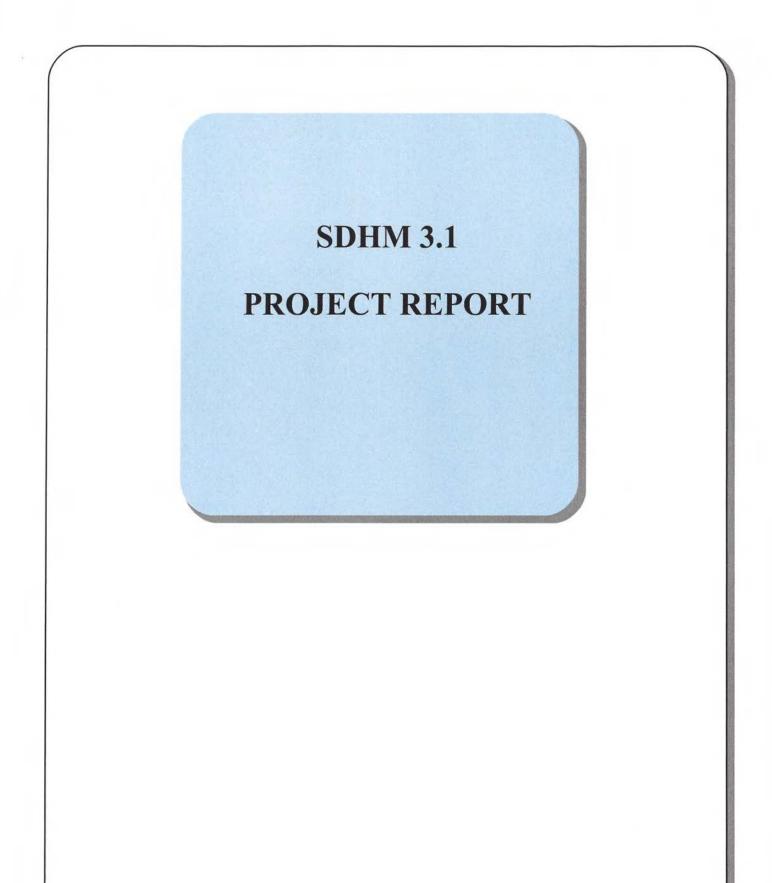
5. REFERENCES

1 – "Final Hydromodification Management Plan (HMP) prepared for the County of San Diego", March 2011, Brown and Caldwell.

2 - San Diego Regional Water Quality Control Board Order No. R9-2013-0001 as amended by R9-2015-0001 and R9-2015-0100 (MS4 Permit).
3 - "City of Chula Vista BMP Design Manual",.

4 - San Diego Hydrology Model 3.1 User Manual - April 2017

APPENDIX A – SDHM 3.1 PROJECT REPORT CALCULATIONS AND CHARTS



General Model Information

Project Name:	18-062
Site Name:	Acadia
Site Address:	Shwiroom Place
City:	Chula Vista
Report Date:	8/2/2019
Gage:	BONITA
Data Start:	10/01/1971
Data End:	09/30/2004
Timestep:	Hourly
Precip Scale:	1.000
Version Date:	2019/04/19

POC Thresholds

Low Flow Threshold for POC1:	10 Percent of the 2 Year	
High Flow Threshold for POC1:	10 Year	
Low Flow Threshold for POC2:	10 Percent of the 2 Year	
High Flow Threshold for POC2:	10 Year	

Landuse Basin Data Predeveloped Land Use

Basin 1

Bypass:	No
GroundWater:	No
Pervious Land Use D,NatVeg,Flat	acre 5.61
Pervious Total	5.61
Impervious Land Use	acre
Impervious Total	0
Basin Total	5.61

Element Flows To: Surface Interflow

G

Groundwater

Basin 2	
Bypass:	No
GroundWater:	No
Pervious Land Use D,NatVeg,Flat	acre 4.78
Pervious Total	4.78
Impervious Land Use	acre
Impervious Total	0
Basin Total	4.78

Element Flows To: Surface Interflow

Groundwater

Mitigated Land Use

		2.3	• • • • •	
B	2	C	In	1
		0		
-	-	-		

Bypass:	No
GroundWater:	No
Pervious Land Use D,NatVeg,Moderate	acre 2.02
Pervious Total	2.02
Impervious Land Use IMPERVIOUS-MOD	acre 3.59
Impervious Total	3.59
Basin Total	5.61

Element Flows To:		
Surface	Interflow	Groundwater
Surface Biofilter 1	Surface Biofilter 1	

Basin 2 Bypass:	No	
GroundWater:	No	
Pervious Land Use D,NatVeg,Moderate	acre 2.16	
Pervious Total	2.16	
Impervious Land Use IMPERVIOUS-MOD	acre 2.62	
Impervious Total	2.62	
Basin Total	4.78	
Element Flows To: Surface Surface Biofilter 2	Interflow Surface Biofilter 2	Groundwater

Routing Elements Predeveloped Routing

Mitigated Routing

Biofilter 1

Bottom Length: Bottom Width: Material thickness of f Material type for first I Material thickness of s Material type for seco Material thickness of t	ayer: second layer: nd layer: hird layer:	223.00 ft. 20.00 ft. 0.25 Mulch 3 ESM 2 GRAVEL
Material type for third Infiltration On	layer.	GRAVEL
Infiltration rate: Infiltration safety facto Total Volume Infiltrate Total Volume Through Total Volume Through Percent Infiltrated: Total Precip Applied to Total Evap From Facil Underdrain used	ed (ac-ft.): n Riser (ac-ft.): n Facility (ac-ft.): n Facility:	0.29 0.13 19.339 3.381 71.696 26.97 5.61 5.875
Underdrain Diameter Orifice Diameter (in.):	(feet):	0.5 0.9
Offset (in.): Flow Through Underd Total Outflow (ac-ft.): Percent Through Under Discharge Structure		3 48.976 71.696 68.31
Riser Height: Riser Diameter:	2 ft. 36 in.	
Element Flows To:		
Outlet 1	Outlet 2	

Biofilter Hydraulic Table

Stage(feet) 697.00	Area(ac.) 0.3009	Volume(ac-ft.) 0.0000	Discharge(cfs) 0.0000	Infilt(cfs) 0.0000
697.09	0.2974	0.0028	0.0000	0.0000
697.18	0.2936	0.0057	0.0000	0.0000
697.27	0.2898	0.0087	0.0000	0.0000
697.36	0.2861	0.0118	0.0000	0.0000
697.45	0.2823	0.0150	0.0000	0.0024
697.54	0.2785	0.0182	0.0000	0.0039
697.63	0.2748	0.0215	0.0000	0.0039
697.73	0.2711	0.0250	0.0000	0.0039
697.82	0.2674	0.0285	0.0000	0.0039
697.91	0.2637	0.0320	0.0000	0.0039
698.00	0.2600	0.0357	0.0000	0.0039
698.09	0.2563	0.0395	0.0000	0.0039
698.18	0.2527	0.0433	0.0000	0.0039
698.27	0.2491	0.0472	0.0000	0.0039
698.36	0.2454	0.0513	0.0018	0.0039
698.45	0.2418	0.0554	0.0027	0.0039
698.54	0.2382	0.0596	0.0044	0.0039
698.63	0.2346	0.0639	0.0052	0.0039
698.72	0.2311	0.0682	0.0064	0.0039

698.81 698.90 699.09 699.09 699.18 699.27 699.36 699.45 699.45 699.54 699.63 699.72 699.81 699.90 699.99 700.08 700.17 700.26 700.35 700.45 700.54 700.54 700.54 700.54 700.54 700.63 700.72 700.81 700.90 700.99 701.08 701.17 701.26 701.35 701.44 701.53 701.62 701.71 701.80 701.99 702.08 702.17 702.25	0.227 0.224 0.220 0.216 0.213 0.209 0.206 0.203 0.199 0.199 0.199 0.199 0.199 0.192 0.189 0.189 0.189 0.189 0.189 0.185 0.189 0.179 0.175 0.175 0.169 0.169 0.169 0.169 0.169 0.159 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.159 0.159 0.159 0.159 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.159 0.159 0.159 0.159 0.159 0.159 0.159 0.159 0.159 0.159 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.159 0.159 0.120	04949405173951841852974208642108766544	0.0727 0.0772 0.0819 0.0866 0.0915 0.0964 0.1014 0.1065 0.1117 0.1223 0.1278 0.1334 0.1390 0.1448 0.1507 0.1389 0.1672 0.1757 0.1589 0.1672 0.1757 0.1844 0.1931 0.2020 0.2110 0.2202 0.2295 0.2390 0.2485 0.2582 0.2390 0.2485 0.2582 0.2390 0.2485 0.2582 0.2390 0.2485 0.2582 0.2681 0.2781 0.2882 0.2882 0.2985 0.3089 0.3194 0.3301 0.3410 0.3519 0.3631 0.3733	0.0070 0.0080 0.0085 0.0093 0.0097 0.0104 0.0108 0.0114 0.0118 0.0124 0.0127 0.0132 0.0135 0.0140 0.0143 0.0143 0.0143 0.0151 0.0155 0.0158 0.0162 0.0165 0.0162 0.0165 0.0165 0.0165 0.0165 0.0162 0.0171 0.0175 0.0175 0.0178 0.0181 0.0181 0.0185 0.0192 0.0201 0.0210 0.0230 0.0239 0.0248 0.0257 0.0265 0.0274 0.0492	0.0039 0.00
Stage(fe 5.2500 5.3407 5.4313 5.5220 5.6126 5.7033 5.7940 5.8846 5.9753 6.0659 6.1566 6.2473 6.3379 6.4286 6.5192 6.6099	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	/olume(a .3733 .4008 .4286 .4567 .4852 .5140 .5433 .5728 .6027 .6330 .6636 .6946 .7259 .7576 .7897 .8221	c-ft.)Discharg 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	e(cfs)To Amen 0.0530 0.0530 0.0530 0.0530 0.0530 0.0530 0.0530 0.0530 0.0530 0.0530 0.0530 0.0530 0.0530 0.0530 0.0530 0.0530 0.0530 0.0530 0.0530	ded(cfs)Infilt(cfs) 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000

					1.000
6.7005	0.3638	0.8549	0.0000	0.0530	0.0000
6.7912	0.3678	0.8881	0.0000	0.0530	0.0000
6.8819	0.3719	0.9216	0.0000	0.0530	0.0000
6.9725	0.3759	0.9555	0.0000	0.0530	0.0000
7.0632	0.3800	0.9898	0.0000	0.0530	0.0000
7.1538	0.3841	1.0244	0.0000	0.0530	0.0000
7.2445	0.3882	1.0594	0.0000	0.0530	0.0000
7.3352	0.3924	1.0948	0.7909	0.0530	0.0000
7.4258	0.3965	1,1306	2.3428	0.0530	0.0000
7.5165	0.4007	1.1667	4.3596	0.0530	0.0000
7.6071	0.4048	1,2032	6,7280	0.0530	0.0000
7.6978	0.4090	1.2401	9.3612	0.0530	0.0000
7.7885	0.4132	1.2774	12.177	0.0530	0.0000
7.8791	0.4174	1.3151	15.091	0.0530	0.0000
7.9698	0.4216	1.3531	18.017	0.0530	0.0000
8.0604	0.4259	1.3915	20.869	0.0530	0.0000
8.1511	0.4301	1.4303	23,565	0.0530	0.0000
8.2418	0.4344	1.4695	26.032	0.0530	0.0000
8.2500	0.4348	1.4731	28.212	0.0530	0.0000
0.2000					2.0000

Surface Biofilter 1 Element Flows To: Outlet 1 Outl

Outlet 2 Biofilter 1

Biofilter 2	
Bottom Length:	184.00 ft.
Bottom Width:	20.00 ft.
Material thickness of first layer:	0.25
Material type for first layer:	Mulch
Material thickness of second layer:	3
Material type for second layer:	ESM
Material thickness of third layer:	2
Material type for third layer:	GRAVEL
Infiltration On	
Infiltration rate:	0.27
Infiltration safety factor:	0.14
Total Volume Infiltrated (ac-ft.):	16.029
Total Volume Through Riser (ac-ft.):	2,711
Total Volume Through Facility (ac-ft.):	53.91
Percent Infiltrated:	29.73
Total Precip Applied to Facility:	4.763
Total Evap From Facility:	4.952
Underdrain used	
Underdrain Diameter (feet):	0.5
Orifice Diameter (in.):	0.75
Offset (in.):	3
Flow Through Underdrain (ac-ft.):	35.17
Total Outflow (ac-ft.):	53.91
Percent Through Underdrain:	65.24
Discharge Structure	
Riser Height: 2 ft.	
Riser Diameter: 36 in. Element Flows To:	
Outlet 1 Outlet 2	
Oulet 1 Oulet 2	

Biofilter Hydraulic Table

Stage(feet) 696.00	Area(ac.) 0.2548	Volume(ac-ft.) 0.0000	Discharge(cfs) 0.0000	Infilt(cfs) 0.0000
696.09	0.2518	0.0023	0.0000	0.0000
696.18	0.2484	0.0047	0.0000	0.0000
696.27	0.2451	0.0072	0.0000	0.0000
696.36	0.2419	0.0097	0.0000	0.0000
696.45	0.2386	0.0124	0.0000	0.0020
696.54	0.2353	0.0150	0.0000	0.0032
696.63	0.2321	0.0178	0.0000	0.0032
696.73	0.2288	0.0206	0.0000	0.0032
696.82	0.2256	0.0235	0.0000	0.0032
696.91	0.2224	0.0265	0.0000	0.0032
697.00	0.2192	0.0295	0.0000	0.0032
697.09	0.2160	0.0235	0.0000	0.0032
697.18	0.2129	0.0359	0.0000	0.0032
697.27	0.2097	0.0391	0.0000	0.0032
			이 전에 이 것 같아. 것이 같아. 그 아이들이 있는 것이 같아. 것이 않아. ???????????????????????????????????	
697.36	0.2066	0.0425	0.0013	0.0032
697.45	0.2035	0.0459	0.0019	0.0032
697.54	0.2003	0.0494	0.0030	0.0032
697.63	0.1972	0.0529	0.0036	0.0032
697.72	0.1942	0.0566	0.0044	0.0032
697.81	0.1911	0.0603	0.0049	0.0032
697.90	0.1880	0.0641	0.0055	0.0032

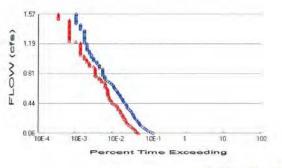
697.99	0.1	850	0.0680	0.0059	0.0032
698.09	0.1	820	0.0719	0.0064	0.0032
698.18 698.27		789 759	0.0760 0.0801	0.0067 0.0072	0.0032 0.0032
698.36	0.1	730	0.0842	0.0075	0.0032
698.45		700	0.0885	0.0079	0.0032
698.54 698.63		670 641	0.0928 0.0973	0.0082 0.0086	0.0032 0.0032
698.72		611	0.1018	0.0088	0.0032
698.81		582	0.1064	0.0092	0.0032
698.90 698.99		553 524	0.1110 0.1158	0.0094 0.0098	0.0032 0.0032
699.08		495	0.1206	0.0099	0.0032
699.17		467	0.1255	0.0103	0.0032
699.26 699.35		438 410	0.1324 0.1394	0.0105 0.0108	0.0032 0.0032
699.45		381	0.1466	0.0110	0.0032
699.54		353	0.1538	0.0113	0.0032
699.63 699.72		325 298	0.1612 0.1686	0.0114 0.0117	0.0032 0.0032
699.81		270	0.1762	0.0119	0.0032
699.90		242	0.1840	0.0122	0.0032
699.99 700.08		215 187	0.1918 0.1997	0.0123 0.0125	0.0032 0.0032
700.17		160	0.2078	0.0126	0.0032
700.26		133	0.2160	0.0128	0.0032
700.35 700.44		106 080	0.2243 0.2327	0.0133 0.0139	0.0032 0.0032
700.53	0.1	053	0.2413	0.0146	0.0032
700.62		026	0.2499	0.0153	0.0032
700.71 700.80		000 974	0.2587 0.2677	0.0159 0.0166	0.0032 0.0032
700.90	0.0	948	0.2767	0.0172	0.0032
700.99 701.08	and the second se	922 896	0.2858 0.2951	0.0178 0.0184	0.0032 0.0032
701.17		870	0.3045	0.0190	0.0032
701.25		845	0.3132	0.0341	0.0032
		draulic Tab		Carlo Star	
Stage(fe 5.2500	et)Area(ac 0.2548	.)Volume(0.3132	ac-ft.)Discharg 0.0000	e(cfs)To Amen 0.0374	ded(cfs)Infilt(cfs) 0.0000
5.3407	0.2581	0.3365	0.0000	0.0374	0.0000
5.4313	0.2615	0.3600	0.0000	0.0374	0.0000 0.0000
5.5220 5.6126	0.2648 0.2682	0.3839 0.4080	0.0000 0.0000	0.0374 0.0374	0.0000
5.7033	0.2716	0.4325	0.0000	0.0374	0.0000
5.7940 5.8846	0.2750 0.2785	0.4573	0.0000 0.0000	0.0374	0.0000
5.9753	0.2785	0.4824 0.5078	0.0000	0.0374 0.0374	0.0000 0.0000
6.0659	0.2853	0.5335	0.0000	0.0374	0.0000
6.1566 6.2473	0.2888 0.2923	0.5595 0.5858	0.0000	0.0374 0.0374	0.0000 0.0000
6.3379	0.2923	0.6125	0.0000	0.0374	0.0000
6.4286	0.2993	0.6395	0.0000	0.0374	0.0000
6.5192 6.6099	0.3028 0.3063	0.6668 0.6944	0.0000 0.0000	0.0374 0.0374	0.0000 0.0000
6.7005	0.3099	0.7223	0.0000	0.0374	0.0000
6.7912	0.3134	0.7506	0.0000	0.0374	0.0000

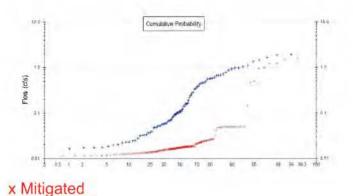
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6.8819	0.3170	0.7791	0.0000	0.0374	0.0000
6.9725	0.3206	0.8080	0.0000	0.0374	0.0000
7.0632	0.3242	0.8373	0.0000	0.0374	0.0000
7,1538	0.3278	0.8668	0.0000	0.0374	0.0000
7.2445	0.3314	0.8967	0.0000	0.0374	0.0000
7.3352	0.3351	0.9269	0.7909	0.0374	0.0000
7.4258	0.3387	0.9575	2.3428	0.0374	0.0000
7.5165	0.3424	0.9883	4.3596	0.0374	0.0000
7.6071	0.3461	1.0195	6.7280	0.0374	0.0000
7.6978	0.3498	1.0511	9.3612	0.0374	0.0000
7.7885	0.3535	1.0830	12.177	0.0374	0.0000
7.8791	0.3572	1.1152	15.091	0.0374	0.0000
7.9698	0.3609	1.1477	18.017	0.0374	0.0000
8.0604	0.3647	1.1806	20.869	0.0374	0.0000
8.1511	0.3684	1.2138	23.565	0.0374	0.0000
8.2418	0.3722	1.2474	26.032	0.0374	0.0000
8.2500	0.3725	1.2505	28.212	0.0374	0.0000

Surface Biofilter 2 Element Flows To: Outlet 1 Outlet 2 Biofilter 2

Analysis Results





+ Predeveloped

Predeveloped Landuse Totals for POC #1 Total Pervious Area: 5.61 Total Impervious Area: 0

Mitigated Landuse Totals for POC #1 Total Pervious Area: 2.02 Total Impervious Area: 3.59

Flow Frequency Method: Weibull

Flow Frequency Return Periods for Predeveloped. POC #1Return PeriodFlow(cfs)2 year0.6059585 year1.066418

1.539163

 10 year
 1.567726

 25 year
 1.875636

 Flow Frequency Return Periods for Mitigated. POC #1

 Return Period
 Flow(cfs)

 2 year
 0.048954

 5 year
 0.580303

 10 year
 1.216318

25 year

8/2/2019 7:48:34 AM

Duration Flows The Facility PASSED

Flow(cfs) 0.0606 0.0758 0.0910 0.1063 0.1215 0.1367 0.1519 0.1672 0.1824 0.1976 0.2128 0.2281 0.2281 0.2433 0.2585 0.2737 0.2889	Predev 357 306 262 229 209 192 173 159 152 143 134 128 114 128 114 108 105	Mit 143 131 124 114 107 98 90 80 73 70 68 66 62 60 56 56	Percentage 40 42 47 49 51 51 52 50 48 48 48 50 51 52 52 52 52 51 53	Pass/Fail Pass Pass Pass Pass Pass Pass Pass Pas
0.3042 0.3194 0.3346 0.3498 0.3651 0.3803 0.3955 0.4107 0.4260 0.4412 0.4564 0.4716 0.4869 0.5021 0.5021 0.5173 0.5325 0.5477 0.5630	101 96 93 86 81 73 71 69 66 63 59 56 54 51 48 47 44 42	52 49 48 47 42 39 38 33 30 29 27 25 24 24 23 22 22	51 51 54 51 57 54 55 50 47 49 48 46 47 50 48 50 52	Pass Pass Pass Pass Pass Pass Pass Pass
0.5030 0.5782 0.5934 0.6086 0.6239 0.6391 0.6543 0.6695 0.6848 0.7000 0.7152 0.7304 0.7457 0.7609 0.7761 0.7913 0.8065 0.8218 0.8370 0.8522	42 40 39 35 34 32 31 30 27 25 22 20 18 18 18 16 16 16	22 22 22 21 20 19 19 19 19 19 19 19 19 19 19 19 19 19	55 56 62 61 62 61 63 70 72 68 75 72 68 75 72 72 61 62 62 62 62 62 62	Pass Pass Pass Pass Pass Pass Pass Pass

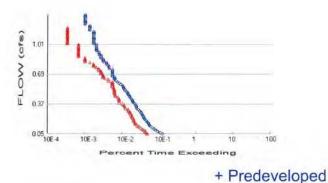
0.8674	16	10	62	Pass
0.8827	16 13	10 9 7 7 7 6 6 6 5 5 5 4 4 4 4 4 4 4 4 4 4 4 4 3 2 2 2 2 2 2 2	76 69 53	Pass
0.8979	13	9	69	Pass
0.9131	13	17	53	Pass
0.9283	12	7	00 63	Pass Pass
0.9588	11	6	54	Pass
0.9740	9	õ	66	Pass
0.9892	9	6	66	Pass
1.0045	9	5	55	Pass
1.0197	8	5	62	Pass
1.0349	8	5	62	Pass
1.0653	7	4	57	Pass Pass
1.0806	7	4	57	Pass
1.0958	7	4	57	Pass
1.1110	6	4	66	Pass
1.1262	6	4	66	Pass
1.1415	6	4	66	Pass Pass Pass
0.9436 0.9588 0.9740 0.9892 1.0045 1.0197 1.0349 1.0501 1.0653 1.0806 1.0958 1.1110 1.1262 1.1415 1.1567 1.1719 1.1871 1.2024 1.2176 1.2328 1.2480 1.2480 1.2633 1.2785 1.2937 1.3089 1.3241 1.3546 1.3698 1.3850 1.4003	13 12 11 1 9 9 9 8 8 8 7 7 7 6 6 6 6 6 6 5 5 5 5 5 5 5 5 5 5 5	4	58 63 54 66 65 52 62 57 57 57 66 66 66 66 66 66 66 60 40 40 40 40 40 40 40 40 50 50 50	Pass
1.1871	õ	4	66	Fass
1.2024	5	4	80	Pass
1.2176	5	3	60	Pass Pass Pass Pass Pass Pass Pass Pass
1.2328	5	2	40	Pass
1.2480	5	22	40	Pass
1.2785	5	2	40	Pass
1.2937	5	2	40	Pass
1.3089	5	2	40	Pass
1.3241	5	2	40	Pass
1.3394	5	2	40	Pass
1.3540	5	2	40	Pass
1.3850	4	2	50	Pass
1.4003	4	2	50	Pass Pass Pass Pass
1 4 1 5 5	4	2	50	Pass
1.4307 1.4459 1.4612 1.4764	4	2	50	Pass
1.4459	3	2	66	Pass
1 4764	3	2	66	Pass Pass
1 1016	3	2	66	Pass
1.5068	3	1	33	Pass
1.5221	3	1	33	Pass
1.5373	3	1	33	Pass
1.5068 1.5221 1.5373 1.5525 1.5677	4 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	1	50 50 66 66 66 33 33 33 33 33 33 33	Pass Pass
1.3077	3	1	00	F 455

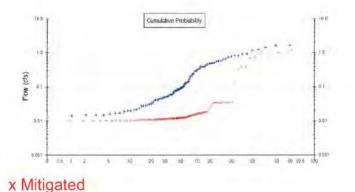
Water Quality Drawdown Time Results

Days	Stage(feet)	Percent of Total Run Time
1	N/A	100.00
2	N/A	100.00
3	N/A	100.00
4	N/A	100.00
5	N/A	100.00
Maximum Stage:	2.000 Drawdown Tir	ne: Less than 1 day

18-062

POC 2





Predeveloped Landuse Totals for POC #2 Total Pervious Area: 4.78 Total Impervious Area: 0

Mitigated Landuse Totals for POC #2 Total Pervious Area: 2.16 Total Impervious Area: 2.62

Flow Frequency Method: Weibull

Flow Frequency Return Periods for Predeveloped. POC #2

Return Period	Flow(cfs)		
2 year	0.516306		
5 year	0.908641		
10 year	1.33578		
25 year	1.598135		

Flow Frequency Return Periods for Mitigated. POC #2Return PeriodFlow(cfs)2 year0.0335785 year0.44588810 year0.81480725 year1.103909

Duration Flows

The Facility PASSED

Flow(cfs) 0.0516 0.0646 0.0776 0.0905 0.1035 0.1165 0.1295 0.1424 0.1554 0.1684 0.1813 0.1943 0.2073 0.2203 0.2332 0.2462 0.2592 0.2721 0.2851 0.2981 0.3111 0.3240 0.3370 0.3500 0.3629 0.3759 0.3889 0.4019 0.31554 0.4019 0.4148 0.4278 0.4019 0.4148 0.4278 0.4019 0.4148 0.4278 0.4667 0.4797 0.5056 0.5186 0.5275 0.5835 0.5964 0.6094 0.6224 0.6224 0.6353 0.6483 0.6483 0.6483 0.6742 0.6872 0.7002	Predev 357 306 264 230 192 173 160 152 143 134 128 113 105 101 96 93 86 173 169 66 63 95 64 143 105 101 96 85 56 45 143 105 105 105 105 105 105 105 105 105 105	Mit 137 129 1212 100 92 147 65 63 63 65 55 55 55 55 55 55 55 55 55 55 55 55	Percentage 38 42 46 48 47 47 46 46 44 45 47 47 50 48 49 50 49 49 50 49 48 50 49 49 50 49 49 50 49 49 50 49 49 50 49 49 50 49 49 50 49 49 50 49 49 50 47 42 42 44 47 42 43 45 43 45 43 45 43 45 43 45 53 50 51 52 53 54 53 59 66 72 65 66 55 50 56 50 50 56 50 70 70 70 70 70 70 70 70 70 70	Pass/Fail Pass Pass Pass Pass Pass Pass Pass Pas
0.6872	16	9	56	Pass
0.7002	16	8	50	Pass
0.7132	16	8	50	Pass
0.7261	16	8	50	Pass

0.7391 0.7521 0.7650 0.7780 0.7910 0.8040 0.8169 0.8299 0.8429 0.8558 0.8688 0.8818 0.9077 0.9207 0.9207 0.9337 0.9466 0.9596 0.9726 0.9726 0.9856 0.9856 0.9985 1.0115 1.0245 1.0374 1.0504 1.0634 1.0764 1.0764 1.0764 1.0764 1.0893 1.1153 1.1282 1.1412 1.1532 1.1412 1.1532 1.1282 1.1412 1.1542 1.1672 1.1801 1.2900 1.2290 1.2450 1.2580	1633321119999888777766666666555555555555444443333	776555433332222222222222222222222222222222	43 53 46 38 41 45 36 33 33 37 25 25 28 28 28 28 33 33 33 33 33 33 33 33 33 33 33 33 33	Pass Pass Pass Pass Pass Pass Pass Pass
1.2190 1.2320 1.2450	4 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3			Pass

Water Quality Drawdown Time Results

Days	Stage(feet)	Percent of Total Run Time
1	N/A	100.00
2	N/A	100.00
3	N/A	100.00
4	N/A	100.00
5	N/A	100.00

Maximum Stage: 2.000 Drawdown Time: Less than 1 day

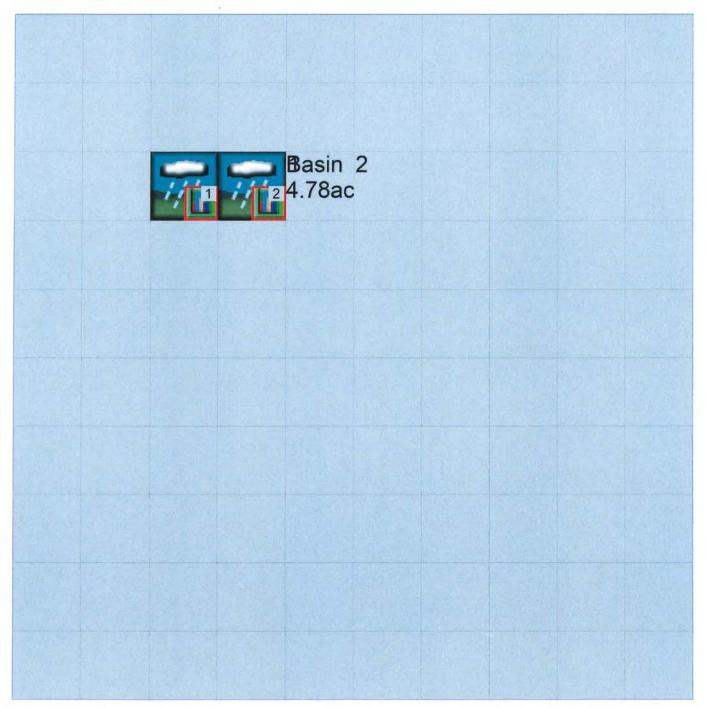
Model Default Modifications

Total of 0 changes have been made.

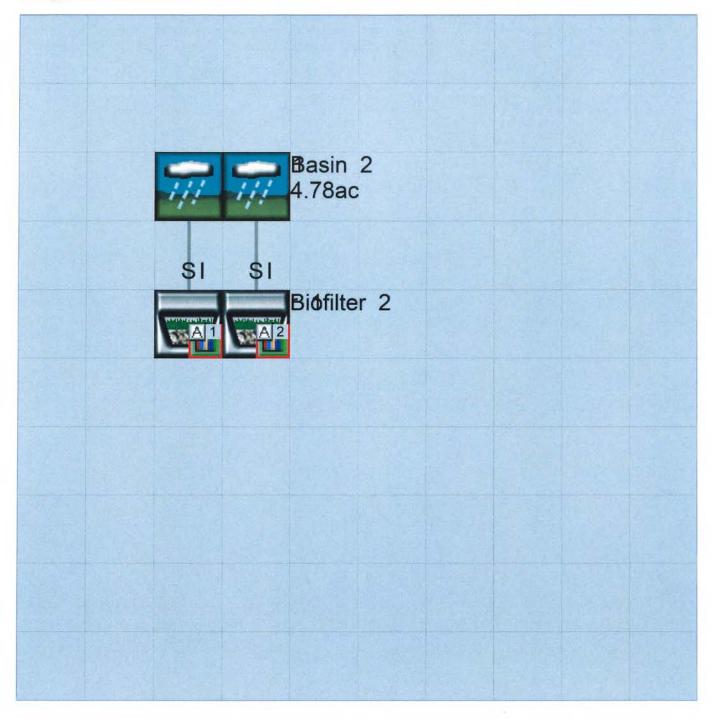
PERLND Changes No PERLND changes have been made.

IMPLND Changes No IMPLND changes have been made.

Appendix Predeveloped Schematic



Mitigated Schematic



Predeveloped UCI File

Mitigated UCI File RUN GLOBAL WWHM4 model simulation END 2004 09 30 START 1971 10 01 RUN INTERP OUTPUT LEVEL 3 0 RESUME 0 RUN 1 UNIT SYSTEM 1 END GLOBAL FILES <File> <Un#> <-----File Name------>*** *** <-ID-> 26 18-062.wdm WDM MESSU 25 Mit18-062.MES 27 Mit18-062.L61 28 Mit18-062.L62 30 POC18-0621.dat 31 POC18-0622.dat END FILES OPN SEQUENCE EQUENCE NGRP PERLND 29 IMPLND 2 INDELT 00:60 INGRP RCHRES 1 RCHRES 2 3 RCHRES RCHRES COPY 1 501 COPY COPY 2 502 COPY DISPLY 1 DISPLY 2 END INGRP END OPN SEQUENCE DISPLY DISPLY-INFO1 # - #<-----Title---->***TRAN PIVL DIG1 FIL1 PYR DIG2 FIL2 YRND Surface Biofilter 1 MAX Surface Biofilter 2 MAX 1 2 30 9 1 2 31 9 1 2 END DISPLY-INFO1 END DISPLY COPY TIMESERIES # - # NPT NMN *** 1 1 1 501 1 1 1 1 1 2 502 1 1 END TIMESERIES END COPY GENER OPCODE # # OPCD *** END OPCODE PARM K *** # # END PARM END GENER PERLND GEN-INFO <PLS ><-----Name---->NBLKS Unit-systems Printer *** User t-series Engl Metr *** # - # in out *** 29 D,NatVeg,Moderate 1 1 1 1 27 0 END GEN-INFO *** Section PWATER***

ACTIVITY # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC *** 29 0 0 1 0 0 0 0 0 0 0 0 END ACTIVITY PRINT-INFO END PRINT-INFO PWAT-PARM1 <PLS > PWATER variable monthly parameter value flags ***
 # # CSNO RTOP UZFG
 VCS
 VUZ
 VNN VIFW
 VIRC
 VLE INFC
 HWT

 29
 0
 1
 1
 0
 0
 0
 1
 1
 0
 END PWAT-PARM1 PWAT-PARM2 <PLS > PWATER input info: Part 2 ***
- # ***FOREST LZSN INFILT LSUR SLSUR KVARY AGWRC
29 0 3 0.025 80 0.1 2.5 0.915 <PLS > 29 END PWAT-PARM2 PWAT - PARM3 PWA1-PARMS<PLS >PWATER input info: Part 3***# - # ***PETMAXPETMININFEXPINFILDDEEPFRBASETPAGWETP29002200.050.05DD DMATDNDATDNDATDNDATDNDATDNDAT END PWAT-PARM3 PWAT-PARM4 <PLS > PWATER input info: Part 4 ***
- # CEPSC UZSN NSUR INTFW IRC LZETP ***
29 0 0.6 0.04 1 0.3 0
END DWAT_DARM4 *** END PWAT-PARM4 MON-LZETPARM <PLS > PWATER input info: Part 3 ***
- # JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC ***
29 0.4 0.4 0.4 0.4 0.6 0.6 0.6 0.6 0.6 0.4 0.4 0.4 END MON-LZETPARM MON-INTERCEP <PLS > PWATER input info: Part 3 *** 29 END MON-INTERCEP PWAT-STATE1 <PLS > *** Initial conditions at start of simulation ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 *** # *** CEPS SURS UZS IFWS LZS AGWS 0 0 0.01 0 0.4 0.01 GWVS # 29 0 END PWAT-STATE1 END PERLND IMPLND GEN-INFO <PLS ><-----Name----> Unit-systems Printer *** # - # User t-series Engl Metr *** in out ** IMPERVIOUS-MOD 1 1 1 27 0 2 END GEN-INFO *** Section IWATER*** ACTIVITY # - # ATMP SNOW IWAT SLD IWG IQAL *** 2 0 0 1 0 0 0 END ACTIVITY

PRINT-INFO <ILS > ******* Print-flags ******* PIVL PYR # - # ATMP SNOW IWAT SLD IWG IQAL ********* 2 0 0 4 0 0 0 1 9 2 END PRINT-INFO IWAT-PARM1 <PLS > IWATER variable monthly parameter value flags *** # - # CSNO RTOP VRS VNN RTL1 *** 2 0 0 0 0 1 END IWAT-PARM1 IWAT-PARM2
 <PLS >
 IWATER input info: Part 2

 # - # *** LSUR
 SLSUR
 NSUR
 RETSC

 2
 100
 0.1
 0.011
 0.08
 *** END IWAT-PARM2 IWAT-PARM3 <PLS > IWATER input info: Part 3 *** # - # ***PETMAX PETMIN 2 0 END IWAT-PARM3 0 IWAT-STATE1 <PLS > *** Initial conditions at start of simulation # - # *** RETS SURS 2 0 0 2 END IWAT-STATE1 END IMPLND SCHEMATIC <--Area--> <-Target-> MBLK *** <-factor-> <Name> # Tbl# *** <-Source-> *** <Name> # Basin 1*** 2.02 RCHRES 1 2.02 RCHRES 1 3.59 RCHRES 1 PERLND 29 2 3 PERLND 29 IMPLND 2 5 Basin 2*** 2.16 RCHRES 3 2 2.16 RCHRES 3 3 2.62 RCHRES 3 5 PERLND 29 PERLND 29 IMPLND 2 *****Routing***** 2.02 COPY 1 12 3.59 COPY 1 15 2.02 COPY 1 13 1 RCHRES 2 8 2.16 COPY 2 12 2.62 COPY 2 15 2.16 COPY 2 15 2.16 COPY 2 13 1 RCHRES 4 8 1 COPY 501 17 1 COPY 501 17 1 COPY 502 17 1 COPY 502 17 PERLND 29 IMPLND 2 PERLND 29 RCHRES 1 PERLND 29 IMPLND 2 PERLND 29 32 RCHRES RCHRES RCHRES 1 RCHRES 4 RCHRES 3 END SCHEMATIC NETWORK <-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> *** <Name> # <Name> # #<-factor->strg <Name> # # <Name> # # <Name> # # ***
COPY 501 OUTPUT MEAN 1 1 12.1 DISPLY 1 INPUT TIMSER 1
COPY 502 OUTPUT MEAN 1 1 12.1 DISPLY 2 INPUT TIMSER 1 <-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> *** <Name> # <Name> # #<-factor->strg <Name> # # <Name> # # *** END NETWORK

RCHRES GEN-INFO								
# - #.	<	me N	<> U	lser T-se	ries E	ngl Metr	LKFG	***
	Comfrage D	iofilte-004 1 iofilte-008 2	2	ir	out	20 0		***
2 1	Biofilter	10111Ce-004	3	1 1	1	28 0	1	
3 1	Surface B	iofilte-008	3	1 1	ī	28 0	î	
4 1	Biofilter	2	2	1 1	ĩ	28 0	ī	
END GEN-	INFO ion RCHRE							
ACTIVITY		***** Activ	a Conti	076 ****	******	*******	********	
# - #	HYEG ADE	G CNEG HTEG	SDFG G	OFG OXEG	NITEG PI	KEG PHEG	***	
1	1		0	0 0	0	0 0		
2	1	0 0 0	0	0 0	0	0 0		
3	1	0 0 0	0	0 0	Ō	0 0		
4	1	0 0 0	0	0 0	0	0 0		
END ACTIV	VITY							
PRINT-INI		******** P	dat 51		*******		DIU DVD	
<pu2></pu2>		A CONS HEAT	CED /	COL OVEN	ום סידיווא	INV DUCE	PIVL PIR	*******
	a supervise states a supervise of			and the second sec	Contraction and the second	a second de la construction de la c	and the second	
2	4		0	0 0	0	0 0	1 9	
3	4	0 0 0	õ	0 0	õ	0 0	1 9	
4	4	0 0 0	0	0 0	0	0 0	1 9	
END PRINT	-INFO							
HYDR - PARM	For the second sec			1. N.				* * *
RCHRES	Flags I	or each HYDI	K Section	on	onomed .	Few seab	TIMOT	
	FC FC FC	2 A3 ODFVF0 3 FG possil * * * 0 0 4 9 0 0 4 9 0 0 4 9 0 0 4 9	vo alc	1+ ***	noccible	a avit	nogeih	le ovit
1	0 1 (0 0 4 4	5 6 0	0	0 0	0 0 0	2 2	2 2 2
2	0 1 (0 0 4 !	500	0	0 0	0 0 0	2 2	2 2 2
3	0 1 (0 0 4	5 6 0	0	0 0	0 0 0	2 2	2 2 2
4 END HYDR-	0 1 (- PARM1	0 0 4	5 0 0	0	0 0	0 0 0	2 2	2 2 2
HYDR - PARM	40							
# - #	FTABNO	D LEN	DE:	LTH	STCOR	KS	DB50	***
		><>						***
1 2		1 0.01 2 0.04			697.0 697.0	0.0	0.0	
3		3 0.01			696.0	0.0	0.0	
4		4 0.03			696.0	0.0	0.0	
END HYDR- HYDR-INIT	-PARM2							
RCHRES	Initial	conditions					-	***
	*** VOI *** ac-ft	for eac	ch poss	ue of C ible exi	t	for eac	l value h possible	exit
<><	<						><><	
1	0	4.0				0.0	0.0 0.0	0.0 0.0
2	0	4.0			0.0		0.0 0.0	
3	0			6.0 0.0 0.0 0.0			0.0 0.0	
END HYDR-		4.0	5.0	0.0 0.0	0.0	0.0	0.0 0.0	0.0 0.0
END RCHRES	INII							
SPEC-ACTION END SPEC-AC								
FTABLES	0							
FTABLE 59 5	2							
59 5 Depth	7.200	a Volume	Outfle	OWI Out	flow2 V	Velocity	Travel Ti	mo***
(ft)	Area	(acre-ft)					(Minute	
0.000000		0.000000			00000	110/060/	Turnace	

0.090659	0.297421	0.002826	0.000000	0.000000			
0.181319		0.005735		0.000000			
0.271978	0.289831	0.008727	0.000000	0.000000			
0.362637	0.286056	0.011803	0.000000	0.000000			
0.453297	0.282295	0.014963	0.000000	0.002410			
0.543956	0.278547	0.018207	0.000000	0.003892			
0.634615	0.274813	0.021536	0.000000	0.003892			
0.725275	0.271092	0.024950	0.000000	0.003892			
0.815934	0.267385	0.028450		0.003892			
			0.00000				
0.906593	0.263692	0.032036	0.000000	0.003892			
0.997253	0.260012	0.035707	0.000000	0.003892			
1.087912	0.256346	0.039466	0.000000	0.003892			
1.178571	0.252694	0.043311	0.000000	0.003892			
1.269231	0.249055		0.000000	0.003892			
1.359890	0.245429	0.051264	0.001826	0.003892			
1.450549	0.241817		0.002739	0.003892			
1.541209	0.238219	0.059569	0.004351	0.003892			
1.631868	0.234634	0.063855	0.005158	0.003892			
1.722527	0.231063	0.068229	0.006379	0.003892			
1.813187	0.227506	0.072694	0.006989	0.003892			
1.903846	0.223962	0.077248	0.007965	0.003892			
1.994505	0.220432	0.081893	0.008452	0.003892			
2.085165							
	0.216915	0.086628	0.009278	0.003892			
2.175824	0.213412	0.091454	0.009690	0.003892			
2,266484	0.209922	0.096372	0.010417	0.003892			
2.357143	0.206446	0.101381	0.010781	0.003892			
2.447802	0.202984	0.106483	0.011438	0.003892			
2.538462	0.199535	0.111677	0.011767	0.003892			
2.629121	0.196100	0.116964	0.012372	0.003892			
2.719780	0.192678	0.122344	0.012674	0.003892			
2.810440	0.189270	0.127818	0.013238	0.003892			
2,901099	0.185875	0.133386	0.013520	0.003892			
2.991758	0.182495	0.139048	0.014050	0.003892			
3.082418	0.179127	0.144805	0.014315	0.003892			
3.173077	0.175774	0.150657	0.014816	0.003892			
3.263736	0.172433	0.158884	0.015067	0.003892			
3.354396	0.169107	0.167244	0.015545	0.003892			
3.445055	0.165794	0.175737	0.015783	0.003892			
3.535714	0.162495	0.184363	0.016240	0.003892			
3.626374	0.159209	0.193124	0.016468	0.003892			
3.717033	0.155937	0.202019	0.016906	0.003892			
3.807692	0.152678	0.211049	0.017125	0.003892			
3.898352	0.149433	0.220215	0.017547	0.003892			
3.989011	0.146201	0.229517	0.017758	0.003892			
4.079670	0.142984	0.238956	0.018011	0.003892			
4.170330	0.139779	0.248532	0.018138	0.003892			
4.260989	0.136589	0.258246	0.018498	0.003892			
4.351648	0.133411	0.268098	0.019195	0.003892			
4.442308	0.130248	0.278088	0.020084	0.003892			
4.532967	0.127098	0.288218	0.021041	0.003892			
4.623626	0.123962	0.298487	0.022010	0.003892			
4.714286	0.120839	0.308897	0.022965	0.003892			
4.804945	0.117730	0.319448	0.023896	0.003892			
4.895604	0.114634	0.330139	0.024798	0.003892			
4.986264	0.111552	0.340973	0.025673	0.003892			
5.076923	0.108484	0.351949	0.026523	0.003892			
5.167582	0.105429	0.363067	0.027350	0.003892			
5.250000	0.102388	0.377683	0.049151	0.003892			
END FTABL	E 2						
FTABLE	1						
35 6	÷						
		11. 1.	0.1.57	0.1.51	0.1.51	**-*	
Depth	Area	Volume	Outflow1	Outflow2	Outflow3	Velocity	Travel
Time***		100 Mar 100 2	in the second		6 351-2	1000 mail	
(ft)	(acres)	(acre-ft)	(cfs)	(cfs)	(cfs)	(ft/sec)	
(Minutes) **							
0.000000	0.102388	0.000000	0.000000	0.000000	0.000000		
0.090659	0.304718		0.000000	0.053043	0.000000		
0.181319	0.308559	0.055252	0.000000	0.053043	0.000000		
0.271978	0.312414	0.083400	0.000000	0.053043	0.000000		
0.362637	0.316283	0.111899	0.00000	0.053043	0.000000		

0.453297	0.320165	0.140749	0.000000	0.053043	0.000000	
0.543956	0.324061		0.00000	0.053043	0.000000	
0.634615	0.327971		0.000000	0.053043	0.000000	
0.725275	0.331894	0.229419	0.000000	0.053043	0.000000	
0.815934	0.335830	0.259687	0.000000	0.053043	0.000000	
0.906593	0.339781		0.000000	0.053043	0.000000	
14.5 PF 75 () () () () () () () () () (
0.997253	0.343744	0.321296	0.000000	0.053043	0.000000	
1.087912	0.347722	0.352640	0.000000	0.053043	0.000000	
1.178571	0.351713	0.384345	0.000000	0.053043	0.000000	
1.269231	0.355717	0.416413	0.000000	0.053043	0.000000	
1.359890	0.359736	0.448844	0.000000	0.053043	0.000000	
1.450549	0.363767		0.000000	0.053043	0.000000	
1.541209	0.367813	0.514802	0.000000	0.053043	0.00000	
1.631868	0.371872	0.548332	0.000000	0.053043	0.000000	
1.722527	0.375944	0.582230	0.000000	0.053043	0.000000	
1.813187	0.380030	0.616498	0.000000	0.053043	0.000000	
1.903846	0.384130	0.651137	0.000000	0.053043	0.000000	
1.994505	0.388243	0.686149	0.000000	0.053043	0.000000	
2.085165	0.392370	0.721534	0.790872	0.053043	0.000000	
2.175824	0.396510	0.757294	2.342816	0.053043	0.000000	
		0.793429				
2.266484	0.400664	a find find the	4.359603	0.053043	0.000000	
2.357143	0.404832	0.829942	6.728010	0.053043	0.000000	
2.447802	0.409013	0.866833	9.361249	0.053043	0.000000	
2.538462	0.413208	0.904104	12.17685	0.053043	0.000000	
2.629121	0.417416	0.941756	15,09073	0.053043	0.000000	
			18.01672	0.053043	0.000000	
2.719780	0.421638	0.979790				
2.810440	0.425874	1.018208	20.86880	0.053043	0.000000	
2.901099	0.430123	1.057010	23.56490	0.053043	0.000000	
2.991758	0.434386	1.096198	26.03191	0.053043	0.000000	
3.000000	0.434774	1.099780	28.21160	0.053043	0.000000	
END FTABL		1.055700	20.21100	0.000010	0.000000	
FTABLE	4					
59 5						
Depth	Area	Volume	Outflow1	Outflow2	Velocity	Travel Time***
(ft)	(acres)	(acre-ft)	(cfs)	(cfs)	(ft/sec)	(Minutes) ***
0.000000	0.254781	0 000000	0 000000	0 000000		
	U. 274/01	0.000000	0.000000	0.000000		
		0.000000	0.000000	0.000000		
0.090659	0.251755	0.002332	0.000000	0.000000		
0.090659 0.181319	0.251755	0.002332	0.000000	0.000000		
0.090659 0.181319 0.271978	0.251755 0.248440 0.245139	0.002332 0.004735 0.007207	0.000000 0.000000 0.000000	0.000000 0.000000 0.000000		
0.090659 0.181319	0.251755	0.002332	0.000000	0.000000		
0.090659 0.181319 0.271978	0.251755 0.248440 0.245139 0.241851	0.002332 0.004735 0.007207 0.009749	0.000000 0.000000 0.000000 0.000000	0.000000 0.000000 0.000000 0.000000		
0.090659 0.181319 0.271978 0.362637 0.453297	0.251755 0.248440 0.245139 0.241851 0.238577	0.002332 0.004735 0.007207 0.009749 0.012362	$\begin{array}{c} 0.00000\\ 0.000000\\ 0.000000\\ 0.000000\\ 0.000000\\ 0.000000\\ \end{array}$	0.000000 0.000000 0.000000 0.000000 0.001989		
0.090659 0.181319 0.271978 0.362637 0.453297 0.543956	0.251755 0.248440 0.245139 0.241851 0.238577 0.235316	0.002332 0.004735 0.007207 0.009749 0.012362 0.015047	0.000000 0.000000 0.000000 0.000000 0.000000	0.000000 0.000000 0.000000 0.000000 0.001989 0.003220		
0.090659 0.181319 0.271978 0.362637 0.453297 0.543956 0.634615	0.251755 0.248440 0.245139 0.241851 0.238577 0.235316 0.232069	0.002332 0.004735 0.007207 0.009749 0.012362 0.015047 0.017803	0.000000 0.000000 0.000000 0.000000 0.000000	0.000000 0.000000 0.000000 0.000000 0.001989 0.003220 0.003220		
0.090659 0.181319 0.271978 0.362637 0.453297 0.543956 0.634615 0.725275	0.251755 0.248440 0.245139 0.241851 0.238577 0.235316 0.232069 0.228835	0.002332 0.004735 0.007207 0.009749 0.012362 0.015047 0.017803 0.020631	$\begin{array}{c} 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ \end{array}$	0.000000 0.000000 0.000000 0.001989 0.003220 0.003220 0.003220		
0.090659 0.181319 0.271978 0.362637 0.453297 0.543956 0.634615 0.725275 0.815934	0.251755 0.248440 0.245139 0.241851 0.238577 0.235316 0.232069 0.228835 0.225616	0.002332 0.004735 0.007207 0.009749 0.012362 0.015047 0.017803 0.020631 0.023531	$\begin{array}{c} 0.00000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.000\\ 0$	0.000000 0.000000 0.000000 0.000000 0.001989 0.003220 0.003220		
0.090659 0.181319 0.271978 0.362637 0.453297 0.543956 0.634615 0.725275	0.251755 0.248440 0.245139 0.241851 0.238577 0.235316 0.232069 0.228835	0.002332 0.004735 0.007207 0.009749 0.012362 0.015047 0.017803 0.020631	$\begin{array}{c} 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ \end{array}$	0.000000 0.000000 0.000000 0.001989 0.003220 0.003220 0.003220		
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0.090659 0.181319 0.271978 0.362637 0.453297 0.543956 0.634615 0.725275 0.815934 0.906593 0.997253	0.251755 0.248440 0.245139 0.241851 0.238577 0.235316 0.232069 0.228835 0.225616 0.222409 0.219216	0.002332 0.004735 0.007207 0.009749 0.012362 0.015047 0.017803 0.020631 0.023531 0.026503 0.029549	$\begin{array}{c} 0.00000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.000\\ $	0.000000 0.000000 0.000000 0.001989 0.003220 0.003220 0.003220 0.003220 0.003220 0.003220 0.003220		
0.090659 0.181319 0.271978 0.362637 0.453297 0.543956 0.634615 0.725275 0.815934 0.906593 0.997253 1.087912	0.251755 0.248440 0.245139 0.241851 0.238577 0.235316 0.232069 0.228835 0.225616 0.222409 0.219216 0.216037	0.002332 0.004735 0.007207 0.009749 0.012362 0.015047 0.017803 0.020631 0.023531 0.026503 0.029549 0.032668	$\begin{array}{c} 0.000000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.00000\\ 0.000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.000\\ 0$	0.000000 0.000000 0.000000 0.001989 0.003220 0.003220 0.003220 0.003220 0.003220 0.003220 0.003220 0.003220		
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3.173077	0.146666	0.125498	0.010289	0.003220			
3.263736	0.143813	0.132401	0.010463	0.003220			
3.354396	0.140973	0.139419	0.010795	0.003220			
3.445055	0.138147	0.146551	0.010961	0.003220			
3.535714	0.135335	0.153798	0.011278	0.003220			
3.626374	0.132536	0.161161	0.011436	0.003220			
3.717033	0.129751	0.168641	0.011741	0.003220			
3.807692	0.126979	0.176237	0.011893	0.003220			
3.898352	0.124221	0.183950	0.012186	0.003220			
3.989011	0.121477	0.191782	0.012332	0.003220			
4.079670	0.118746	0.199732	0.012508	0.003220			
4.170330		0.207800					
	0.116029		0.012596	0.003220			
4.260989	0.113325	0.215988	0.012846	0.003220			
4.351648	0,110635	0.224296	0.013330	0.003220			
4.442308	0.107958	0.232724	0.013947	0.003220			
4.532967	0.105296	0.241273	0.014612	0.003220			
4.623626	0.102646	0.249943	0.015285	0.003220			
4.714286	0.100010	0.258736	0.015948	0.003220			
4.804945	0.097388	0.267651	0.016594	0.003220			
4.895604	0.094780	0.276688	0.017221	0.003220			
4.986264	0.092185	0.285849	0.017829	0.003220			
5.076923	0.089603	0.295135	0.018419	0.003220			
5.167582	0.087035	0.304544	0.018993	0.003220			
5.250000	0.084481	0.316294	0.034132	0.003220			
END FTABL	E 4						
FTABLE	3						
35 6							
Depth	Area	Volume	Outflow1	Outflow2	Outflow3	Velocity	Travel
Time***	Area	vorume	OUCTIONI	OUCTIOW2	OUCTIONS	verocity	ILAVEL
	(aguage)	loove ELV	1-6-1	(- E -)	1-5-1	15-1	
(ft)		(acre-ft)	(cfs)	(cfs)	(cfs)	(ft/sec)	
(Minutes) **		5 353594	Var and and	AND AND AR	9 935 59 21		
0.00000	0.084481	0.00000	0.000000	0.000000	0.000000		
0.090659	0.258122	0.023250	0.000000	0.037352	0.000000		
0.181319	0.261476	0.046803	0.000000	0.037352	0.000000		
0.271978	0.264844	0.070661	0.000000	0.037352	0.000000		
0.362637	0.268226	0.094825	0.000000	0.037352	0.000000		
0.453297	0.271621	0.119296	0.000000	0.037352	0.000000		
0.543956	0.275030	0.144075	0.000000	0.037352	0.000000		
0.634615	0.278453	0.169165	0.000000	0.037352			
					0.000000		
0.725275	0.281889	0.194565	0.000000	0.037352	0.000000		
0.815934	0.285338	0.220277	0.00000	0.037352	0.00000		
0.906593	0.288802	0.246303	0.000000	0.037352	0.00000		
0.997253	0.292278	0.272643	0.000000	0.037352	0.000000		
1.087912	0.295769	0.299299	0.000000	0.037352	0.000000		
1.178571	0.299273	0.326272	0.000000	0.037352	0.000000		
1.269231	0.302790	0.353563	0.000000	0.037352	0.000000		
1.359890	0.306322	0.381174	0.000000	0.037352	0.000000		
1.450549	0.309866	0.409106	0.000000	0.037352	0.000000		
1.541209	0.313425				0.000000		
		0.437359	0.000000	0.037352			
1.631868	0.316997	0.465936	0.000000	0.037352	0.000000		
1.722527	0.320582	0.494837	0.000000	0.037352	0.000000		
1.813187	0.324181	0.524064	0.000000	0.037352	0.000000		
1.903846	0.327794	0.553618	0.000000	0.037352	0.000000		
1.994505	0.331420	0.583500	0.000000	0.037352	0.000000		
2.085165	0.335060	0.613711	0.790872	0.037352	0.000000		
2.175824	0.338713	0.644253	2.342816	0.037352	0.000000		
2.266484	0.342380		4.359603				
		0.675127		0.037352	0.000000		
2.357143	0.346061	0.706334	6.728010	0.037352	0.000000		
2.447802	0.349755	0.737875	9.361249	0.037352	0.000000		
2.538462	0.353463	0.769751	12.17685	0.037352	0.000000		
2.629121	0.357184	0.801965	15.09073	0.037352	0.000000		
2.719780	0.360919	0.834516	18.01672	0.037352	0.000000		
2.810440	0.364668	0.867407	20.86880	0.037352	0.000000		
2.901099	0.368430	0.900638	23.56490	0.037352	0.000000		
2.991758	0.372205	0.934211	26.03191	0.037352	0.000000		
3.000000	0.372549	0.937280	28.21160	0.037352	0.000000		
END FTABL		0.00/200	20.21100	0.001002	0.000000		
LIND L'IMDE							

END FTABLE END FTABLES

ALC: NOTE: N

EXT SOURCES

		1 1 1 1 1 P 1 1 I 1 1 R 1 1 R 1 0.5 R 0.7 R 0.5 R	Name> # PERLND 1 MPLND 1	ols> <-Grp> # 999 EXTNL 999 EXTNL 999 EXTNL 999 EXTNL EXTNL EXTNL EXTNL EXTNL EXTNL EXTNL EXTNL	<-Member-> *** <name> # # *** PREC PREC PETINP PETINP PREC PREC POTEV POTEV POTEV POTEV</name>
END EXT SOURCES					
EXT TARGETS <-Volume-> <-Grp> <name> # RCHRES 2 HYDR RCHRES 2 HYDR RCHRES 2 HYDR RCHRES 2 HYDR RCHRES 1 HYDR RCHRES 1 HYDR COPY 1 OUTPUT COPY 501 OUTPUT RCHRES 4 HYDR RCHRES 4 HYDR RCHRES 4 HYDR RCHRES 4 HYDR RCHRES 4 HYDR RCHRES 4 HYDR RCHRES 3 HYDR RCHRES 4 HYDR RCHRES 3 HYDR RCHRES 4 HYDR RCHRES 3 HYDR RCHRES 3 HYDR RCHRES 3 HYDR RCHRES 4 HYDR RCHRES 3 HYDR RCHRES 4 HYDR RCHRES 3 HYDR RCHRES 4 HYDR RCHRES 4 HYDR RCHRES 4 HYDR RCHRES 4 HYDR RCHRES 4 HYDR RCHRES 4 HYDR RCHRES 7 HYDR RCHRES</name>	<name> RO O STAGE STAGE O MEAN RO O STAGE STAGE STAGE O MEAN</name>	1 1 1 W 2 1 1 W 1 1 1 W 1 1 1 W 1 1 1 W 1 1 1 W 1 1 12.1 W 1 1 1.1 W 1 1 1.2.1 W	Name> # DM 1000 DM 1001 DM 1002 DM 1003 DM 1004 DM 1005 DM 701 DM 801 DM 1006 DM 1010 DM 1010 DM 1007 DM 1008 DM 1009 DM 702	<name> t FLOW EI FLOW EI FLOW EI STAG EI STAG EI FLOW EI FLOW EI FLOW EI FLOW EI FLOW EI STAG EI STAG EI STAG EI FLOW EI</name>	sys Tgap Amd *** tem strg strg*** NGL REPL NGL REPL
MASS-LINK <volume> <-Grp> <name> MASS-LINK PERLND PWATER</name></volume>	<name> 2</name>	# #<-factor-> <	Target> Name> CHRES	<-Grp>	<-Member->*** <name> # #***</name>
END MASS-LINK	2	0.003333 10	CITICED	INTHOW	TYOH
MASS-LINK PERLND PWATER END MASS-LINK		0.083333 R	CHRES	INFLOW	IVOL
MASS-LINK IMPLND IWATER END MASS-LINK	SURO	0.083333 R	CHRES	INFLOW	IVOL
MASS-LINK RCHRES OFLOW END MASS-LINK	OVOL	2 R	CHRES	INFLOW	IVOL
MASS-LINK PERLND PWATER END MASS-LINK	SURO	0.083333 C	OPY	INPUT	MEAN
MASS-LINK PERLND PWATER END MASS-LINK	IFWO	0.083333 C	OPY	INPUT	MEAN
MASS-LINK IMPLND IWATER END MASS-LINK	SURO		OPY	INPUT	MEAN
MASS-LINK RCHRES OFLOW END MASS-LINK	OVOL	1 C	OPY	INPUT	MEAN

END MASS-LINK

END RUN

Predeveloped HSPF Message File

Mitigated HSPF Message File

APPENDIX B – HMP EXHIBIT

