APPENDIX G STORMWATER INTAKE FORM AND STORMWATER QUALITY MANAGEMENT PLAN

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CITY OF ENCINITAS STORMWATER INTAKE FORM AND PRIORITY DEVELOPMENT PROJECT STORMWATER QUALITY MANAGEMENT PLAN (SWQMP)

FOR: LEUCADIA 101 MIXED USE

1900 and 1950 N COAST HIGHWAY 101 ENCINITAS, CA 92024 APN 216-041-06, 20 and 21

PREPARED BY:

WILLIAM J. SUITER, RCE 68964 PASCO LARET SUITER & ASSOCIATES 535 NORTH HIGHWAY 101, SUITE A SOLANA BEACH, CA 92075 858-259-8212

PREPARED FOR:

ENCINITAS BEACH LAND VENTURE I, LLC 674 VIA DE LA VALLE , SUITE 310 SOLANA BEACH 92075 858-436-3600

DATE OF SWQMP:

MAY 2020 REVISED: SEPTEMBER 2020 REVISED: DECEMBER 2020 REVISED: MAY 2021 REVISED: JULY 2021

GRADING PLAN PREPARED BY:

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PREPARER'S CERTIFICATION

I hereby declare that I am the Engineer in Responsible Charge of design of storm water best management practices (BMPs) for this project, and that I have exercised responsible charge over the design of the BMPs as defined in Section 6703 of the Business and Professions Code, and that the design is consistent with the Priority Development Project (PDP) requirements of the City of Encinitas BMP Design Manual, which is a design manual for compliance with local City of Encinitas and regional MS4 Permit (California Regional Water Quality Control Board San Diego Region Order No. R9-2015-0100) requirements for storm water management.

I have read and understand that the City Engineer has adopted minimum requirements for managing urban runoff, including storm water, from land development activities, as described in the BMP Design Manual. I certify that this PDP SWQMP has been completed to the best of my ability and accurately reflects the project being proposed and the applicable BMPs proposed to minimize the potentially negative impacts of this project's land development activities on water quality. I understand and acknowledge that the plan check review of this PDP Storm Water Quality Management Plan (SWQMP) by the City Engineer is confined to a review and does not relieve me, as the Engineer in Responsible Charge of design of storm water BMPs for this project, of my responsibilities for project design.



Engineer's Seal

Engineer of Work's Signature, PE Number

William J. Suiter, RCE 68964 Print Name

Pasco, Laret, Suiter & Associates Company

<u>7/21/2021</u> Date

PROJECT OWNER'S CERTIFICATION

This PDP SWQMP has been prepared for <u>Encinitas Beach Land Venture I, LLC</u> by <u>Pasco Laret Suiter &</u> <u>Associates</u>. The PDP SWQMP is intended to comply with the PDP requirements of the City of Encinitas BMP Design Manual, which is a design manual for compliance with local City of Encinitas and regional MS4 Permit (California Regional Water Quality Control Board San Diego Region Order No. R9-2015-0100) requirements for storm water management.

The undersigned, while it owns the subject property, is responsible for the implementation of the provisions of this plan. Once the undersigned transfers its interests in the property, its successor-in-interest shall bear the aforementioned responsibility to implement the best management practices (BMPs) described within this plan, including ensuring on-going operation and maintenance of structural BMPs. A signed copy of this document shall be available on the subject property into perpetuity.

Project Owner's Signature J The a

Print Name

Encinitas Beach Land Venture I, LI	.C 0.
Company	

	39	21		
Date	1			-

SUBMITTAL RECORD

Use this table to keep a record of submittals of this PDP SWQMP. Each time the PDP SWQMP is resubmitted, provide the date and status of the project. In the fourth column, summarize the changes that have been made or indicate if response to plancheck comments is included. When applicable, insert response to plancheck comments behind this page.

Submittal Number	Date	Project Status	Summary of Changes
1	May 2020	 ☑ Preliminary Design / □ Planning/ CEQA □ Final Design 	
2	Sept 2020	☑ Preliminary Design / Planning/ CEQA Final Design	
3	Dec 2020	☑ Preliminary Design / Planning/ CEQA Final Design	
4	May 2021	☑ Preliminary Design / Planning/ CEQA Final Design	
5	July 2021	☑ Preliminary Design / Planning/ CEQA Final Design	

PROJECT IDENTIFICATION

Project/Applicant Name: Encinitas Beach Land Venture I, LLC			
Permit/Application Number:	Date: July 2021		
Site Address: 1900 and 1950 N Coast Hwy 101, Encinitas CA 92024	APN: 216-041-06, 20 and 21		
Scope of work/project description:			

Scope of work/project description:

The project proposes to demolish the existing development and construct a new apartment complex with approximately 94 units, boutique hotel, retail units, structured parking, access drive, hardscape and landscape, and associated improvements including storm water biofiltration basins to meet the requirements for pollutant control and an underground storage vault to comply with hydromodification management flow control and to mitigate the 100-year storm event peak discharge rate.

Due to the timing of construction, the right-of-way improvements directly in front of the project site, per the plans prepared by Michael Baker International, will be built by Encinitas Beach Land Venture I, LLC. Plans prepared by Michael Baker International Inc. at this time do not indicate a clear strategy on how to treat all the street runoff for the portion of right-of-way improvements this project proposes to build. Other areas of the City of Encintas' capital improvement project, 101 Streetscape, proposes to utilize a Green Streets design strategy. Due to conflicts with existing utilities, the project proposes to install two Modular Wetlands System units to treat the street runoff, in lieu of Green Streets improvements.

DETERMINATION OF PROJECT STATUS AND REQUIREMENTS

This form will identify permanent, post construction BMP requirements. Refer to City of Encinitas Stormwater BMP Design Manual for guidance. Step 1: Is the project a "development project"? ⊡Yes Go to Step 2. Development projects are defined as "construction, rehabilitation, redevelopment, or reconstruction of any public or private projects". Stop. See Section 1.3 and Table 1-2 of the manual for Permanent BMP requirements do □ No guidance. For example, interior remodels, roof not apply. No SWQMP will be replacements, and electrical and plumbing work required. Provide discussion below. are not development projects. If "No", provide discussion / justification explaining why the project is not a "development project": Step 2: Complete questions below for Project Type Determination. □ New Development Redevelopment The project is (select one): The total proposed, newly created and/or replaced impervious area is: 169,082 ft²

Yes	No	(a)	New development projects or redevelopment projects that create and/or replaced
Ø		()	10,000 square feet or more of impervious surfaces (collectively over the entire project site). This includes commercial, industrial, residential, mixed-use, and public development projects.
Yes	No	(b)	Redevelopment projects that create and/or replace 5,000 square feet or more of
V			impervious surface (collectively over the entire project site on an existing site of 10,000 square feet or more of impervious surfaces). This includes commercial, industrial, residential, mixed-use, and public development projects.
Yes	No	(C)	New and redevelopment projects that create and/or replace 5,000 square feet or
Ŋ			 more of impervious surface (collectively over the entire project site), and support one or more of the following uses: (i) Restaurants. This category is defined as a facility that sells prepared foods and drinks for consumption, including stationary lunch counters and refreshment stands selling prepared foods and drinks for immediate consumption (SIC code 5812). (ii) Hillside development projects. This category includes development on any natural slope that is twenty-five percent or greater.
			 (iii) Parking lots. This category is defined as a land area or facility for the temporary parking or storage of motor vehicles used personally, for business, or for commerce. (iv) Streets, roads, highways, freeways, and driveways. This category is defined as any paved impervious surface used for the transportation of automobiles, trucks, motorcycles, and other vehicles.
Yes □	No 12	(d)	New or redevelopment projects that create and/or replace 2,500 square feet or more of impervious surface (collectively over the entire project site), and discharge directly to an Environmentally Sensitive Area (ESA). "Discharge directly to" includes flow that is conveyed overland a distance of 200 feet or less from the project to the ESA, or conveyed in a pipe or open channel any distance as an isolated flow from the project to the ESA (i.e. not commingled with flows from adjacent lands). <u>Note: ESAs are areas that include but are not limited to all Clean Water Act Section 303(d) impaired water bodies; areas designated as Areas of Special Biological Significance by the State Water Board and SDRWQCB; State Water Quality Protected Areas; water bodies designated with the RARE beneficial use by the State Water Board and SDRWQCB; and any other equivalent environmentally sensitive areas which have been identified by the Copermittees. See manual Section 1.4.2 for additional guidance.</u>
Yes	No 12	(e)	 New development projects, or redevelopment projects that create and/or replace 5,000 square feet or more of impervious surface, that support one or more of the following uses: (i) Automotive repair shops. This category is defined as a facility that is categorized in any one of the following SIC codes: 5013, 5014, 5541, 7532-7534, or 7536-7539. (ii) Retail gasoline outlets. This category includes retail gasoline outlets that meet the following criteria: (a) 5,000 square feet or more or (b) a projected
Yes	No	(f)	Average Daily Traffic of 100 or more vehicles per day.

Does the project meet the definition of one or more of the PDP categories (a) through (f) listed above?

 \square Yes – The project is a <u>Priority Development Project</u>, the applicant shall provide PDP Post Construction BMPs and *continue to Step 3*.

□No – The project is a <u>Standard or Basic Project</u>. Stop here and complete the "City of Encinitas Stormwater Intake Form for All Developments and Standard Projects SWQMP".

The following is for *redevelopment PDPs* only:

The area of existing (pre-project) impervious area at the project site is: $\underline{76,819}$ ft² (A) The total proposed newly created or replaced impervious area is: $\underline{143,659}$ ft² (B) Percent impervious surface created or replaced (B/A)*100: $\underline{187\%}$ The percent impervious surface created or replaced is (select one based on the above calculation):

□ Less than or equal to fifty percent (50%) – only new and/or replaced impervious areas are considered PDP subject to treatment and HMP criteria OR

 \square Greater than fifty percent (50%) – the entire site is a PDP; treatment and HMP criteria apply to entire site regardless of whether it is replaced

Step 3 (PDPs only): Do hydromodification control requirements apply? See Section 1.6 of the BMP Design	⊠ Yes	PDP structural BMPs required for pollutant control (Chapter 5) and hydromodification control (Chapter 6). <i>Go to Step 4.</i>
Manual for guidance.	□ No	PDP structural BMPs required for pollutant control (Chapter 5) only. Provide brief discussion of exemption to hydromodification control below. Go to "Site Information Checklist"
Discussion / justification if hydromodifica	ation control requ	uirements do <u>not</u> apply:
	Γ	
Step 4 (PDPs subject to treatment and hydromodification controls): Does protection of critical coarse sediment yield areas apply based on	□ Yes	Management measures required for protection of critical coarse sediment yield areas (Chapter 6.2). <i>Go to "Site Information Checklist"</i>
review of City of Encinitas Potential Critical Coarse Sediment Yield Area Map? See Section 6.2 of the BMP Design Manual for guidance.	☑ No	Management measures not required for protection of critical coarse sediment yield areas. Provide brief discussion below. Go to "Site Information Checklist"
Discussion / justification if management yield areas:	measures <u>not</u> re	equired for protection of critical coarse sediment

Pursuant to the City of Encinitas Potential Critical Coarse Sediment Yield Area GIS layer, there are no potential critical coarse sediment yield areas on or upstream of the project site. Refer to the exhibit located in Attachment 2b.

SITE INFORMATION CHECKLIST

	Carlsbad HU, San Marcos HA, Batquitos HSA, 904.51		
Project's Watershed	Cansbau 110, San Marcos 11A, Balquilos 115A, 304.51		
(Complete Hydrologic Unit, Area, and Subarea Name with Numeric Identifier)			
Parcel Area	2 70 Acres (165 107 Square Feet)		
(Total area of Assessor's Parcel(s) associated with the project)	3.79 Acres (165,107 Square Feet)		
Area to be Disturbed by the Project	4.56 Acres (198,534 Square Feet)		
(Project Area)	4.50 Acres (190,554 Square Feel)		
Project Proposed Impervious Area	3.66 Acres (159,288 Square Feet)		
(Subset of Project Area)			
Project Proposed Pervious Area	0.90 Acres (39,246 Square Feet)		
(Subset of Project Area)			
Note: Proposed Impervious Area + Proposed Per This may be less than the Parcel Area.	vious Area = Area to be Disturbed by the Project.		
Description of E	Existing Site Condition		
Current status of the site (select all that apply):			
☑ Existing development			
Previously graded but not built out			
 Demolition completed without new construction 			
☐ Agricultural or other non-impervious use			
□ Vacant, undeveloped/natural			
Description / Additional Information:			
The existing site is currently developed and consists of commercial buildings, paved access drives and parking, open space, hardscape and landscape.			
Existing Land Cover includes (select all that apply):			
☑ Vegetative Cover			
 ✓ Vegetated Pervious Areas 			
 ✓ Impervious Areas 			
Description / Additional Information:			
Underlying soil belongs to Hydrologic Soil Group (select all that apply):			
□ NRCS Type A			
☑ NRCS Type B			
□ NRCS Type C			
□ NRCS Type D			

Approximate Depth to Groundwater (GW):

□ GW Depth < 5 feet</p>

 \Box 5 feet < GW Depth < 10 feet

□ 10 feet < GW Depth < 20 feet

☑ GW Depth > 20 feet

Existing Natural Hydrologic Features (select all that apply):

□ Watercourses

 $\Box \, \text{Seeps}$

Springs

□ Wetlands

☑ None

Description / Additional Information:

Description of Existing Site Drainage Patterns

How is storm water runoff conveyed from the site? At a minimum, this description should answer:

- 1) Is existing drainage conveyance natural or urban?
- Is runoff from offsite conveyed through the site? If yes, quantify all offsite drainage areas, design flows, and locations where offsite flows enter the project site, and summarize how such flows are conveyed through the site.
- 3) Provide details regarding existing project site drainage conveyance network, including any existing storm drains, concrete channels, swales, detention facilities, storm water treatment facilities, natural or constructed channels. And
- 4) Identify all discharge locations from the existing project site along with a summary of conveyance system size and capacity for each of the discharge locations. Provide summary of the pre-project drainage areas and design flows to each of the existing runoff discharge locations.

Describe existing site drainage patterns:

In the existing condition, storm water runoff from the site generally flows overland and in onsite storm drain easterly to North Coast Highway 101. There is offsite run-on from the hillside along the westerly and southerly boundaries.

An existing high point in North Coast Highway 101 is located approximately at the midpoint of the property's easterly boundary. Surface runoff from the property that enters the right-of-way north of the high point will continue to surface flow northerly and enters the public storm drain system within the La Costa Avenue and North Coast Highway 101 intersection. The storm drain system then conveys captured flows to the east side Carlsbad Boulevard into the Batiquitos Lagoon and ultimately the Pacific Ocean. Surface runoff from the property that enters the right-of-way south of the high point will surface flow southerly and enters a separate public storm drain system that conveys captured flow northerly to an extended detention basin located on the west side of Carlsbad Boulevard which discharges to Batiquitos Lagoon and ultimately the Pacific Ocean. The onsite storm drain connects to the public storm drain located on the west side of Carlsbad Boulevard which discharges to Batiquitos Lagoon and ultimately the Pacific Ocean.

Drainage Basin	Area (ac)	Q100 (cfs)
POC-1	4.13	14.65
Refer to the "Preliminary Hydrology Stu & Associates dated July 2021.	udy for Leucadia 101 Mixed Us	se" prepared by Pasco Laret Suiter
	tion of Proposed Site Develo	opment
Project Description / Proposed Land Us	se and/or Activities:	
The project proposes to demolish the e with approximately 94 units, boutique landscape, and associated improvemen requirements for pollutant control and management flow control and to mitig	hotel, retail units, structured p nts including storm water biofi an underground storage vaul	parking, access drive, hardscape and iltration basins to meet the t to comply with hydromodification
List/describe proposed impervious featu courtyards, athletic courts, other impervious		ngs, roadways, parking lots,
Proposed impervious features include	the buildings, parking areas, a	ccess drive and hardscape.
List/describe proposed pervious feature	es of the project (e.g., landscap	pe areas):
Proposed pervious features include lan	idscape areas and biofiltration	basins.
Does the project include grading and ch	nanges to site topography?	
☑ Yes		
□ No		
Description / Additional Information:		
The project site will be graded to create and associated underground utilities.	e pads suitable for the constru	uction of structures, improvements
Description	n of Proposed Site Drainage	Patterns
Does the project include changes to site systems)?	e drainage (e.g., installation of	new storm water conveyance
☑ Yes		

If yes, provide details regarding the proposed project site drainage conveyance network, including storm drains, concrete channels, swales, detention facilities, storm water treatment facilities, natural or constructed channels, and the method for conveying offsite flows through or around the proposed project site. Identify all discharge locations from the proposed project site along with a summary of the conveyance system size and capacity for each of the discharge locations. Provide a summary of pre- and post-project drainage areas and design flows to each of the runoff discharge locations. Reference the drainage study for detailed calculations.

Describe proposed site drainage patterns:

In the proposed condition, storm water runoff will be collected by proposed storm drain and conveyed to biofiltration basins located throughout the site. Discharge from the biofiltration basins will flow to the proposed underground storage vault located in the northeastern corner of the project site. The vault will discharge to a proposed 18" RCP which will connect to the back of the existing curb inlet located north of the project along North Coast Highway 101. The existing curb outlets to an 18" RCP then transitions to a 24" RCP that conveys flows northerly and into the Batiquitos Lagoon on the east side of Carlsbad Boulevard. Offsite storm water that runs onto the site along the westerly boundary will be intercepted via a new concrete ditch and routed to proposed storm drain which runs along the northern boundary of the site and connects to the underground vault outlet pipe and continues as described above. Offsite run-on along the southern boundary will be captured in a new concrete ditch and discharged to North Coast Highway 101 via sidewalk underdrain which will enter the public storm drain system and be conveyed to the extended detention basin on the west side of Carlsbad Boulevard as in the existing condition.

The biofiltration basins and underground storage vault are designed to provide pollutant control treatment and hydromodification management flow control to meet the requirements of the California Regional Water Quality Control Board San Diego Region municipal storm water permit (Order No. R9-2013-0001, referred to as the MS4 Permit). The underground storage vault will also provide mitigation for the 100-year storm event peak discharge rate.

Drainago Basin	Existing	Condition	Proposed Detai	ined Condition
Drainage Basin	Area (ac)	Q100 (cfs)	Area (ac)	Q100 (cfs)
POC-1	4.13	14.65	4.13	1.17

Refer to the "Preliminary Hydrology Study for the Leucadia 101 Mixed Use" prepared by Pasco Laret Suiter & Associates dated July 2021.

In the proposed condition, storm water runoff in the right-of-way will be collected by proposed curb inlet type Modular Wetlands System units. Discharge from the southerly Modular Wetlands System unit will flow to an existing curb inlet which conveys flow northerly towards the existing extended basin on the west side of Carlsbad Boulevard. The northerly Modular Wetlands System unit will flow to the proposed 18" RCP serving as the outlet for the proposed on-site underground storage vault and towards the east side of Carlsbad Boulevard (as described below).

Identification and Narrative of Receiving Water and Pollutants of Concern

Describe flow path of storm water from the project site discharge location(s), through urban storm conveyance systems as applicable, to receiving creeks, rivers, and lagoons as applicable, and ultimate discharge to the Pacific Ocean (or bay, lagoon, lake or reservoir, as applicable):

On-site storm water runoff confluences with off-site run-on collected from the westerly boundary and discharges through a single point at the northeast corner of the project site to N Coast Highway 101. The discharge will flow through a new 18" storm drain and connects to the back of an existing concrete curb inlet (constructed per Drawing No. 6169-I) located at the southwest corner of the La Costa Avenue and N. Coast Highway 101 intersection. Discharge entering the curb inlet flows to the east through an 18" storm drain into a second curb inlet in the center median of N. Coast Highway 101. Discharge then flows to the north through an 18" storm drain which upsizes to a 24" before it discharges on the east side of Carlsbad Boulevard to Batiquitos Lagoon and ultimately the Pacific Ocean.

Offsite run-on along the southern boundary will be captured in a new concrete ditch and discharged to North Coast Highway via sidewalk underdrain which will enter the public storm drain system and be conveyed to the existing extended basin on the west side of Carlsbad Boulevard which discharges to the Batiquitos Lagoon and ultimately the Pacific Ocean.

List any 303(d) impaired water bodies within the path of storm water from the project site to the Pacific
Ocean (or bay, lagoon, lake or reservoir, as applicable), identify the pollutant(s)/stressor(s) causing
impairment, and identify any TMDLs and/or Highest Priority Pollutants from the WQIP for the impaired
water bodies:

303(d) Impaired Water Body	Pollutant(s)/Stressor(s)	TMDLs / WQIP Highest Priority Pollutant
Batiquitos Lagoon	Toxicity	TMDL Estimated 2027

Identification of Project Site Pollutants*

*Identification of project site pollutants is only required if flow-thru treatment BMPs are implemented onsite in lieu of retention or biofiltration BMPs (note the project must also participate in an alternative compliance program unless prior lawful approval to meet earlier PDP requirements is demonstrated)

Identify pollutants expected from the project site based on all proposed use(s) of the site (see BMP Design Manual Appendix B.6):

Pollutant	Not Applicable to the Project Site	Expected from the Project Site	Also a Receiving Water Pollutant of Concern
Sediment			
Nutrients			
Heavy Metals			
Organic Compounds			
Trash & Debris			

Oxygen Demanding Substances			
Oil & Grease			
Bacteria & Viruses			
Pesticides			
Hydromodification Management Requirements			
Do hydromodification management requirements apply (see Section 1.6 of the BMP Design Manual)?			
☑ Yes, hydromodification management flow control structural BMPs required.			
No, the project will discharge runoff directly to existing underground storm drains discharging directly to water storage reservoirs, lakes, enclosed embayments, or the Pacific Ocean.			
No, the project will discharge runoff directly to conveyance channels whose bed and bank are concrete-lined all the way from the point of discharge to water storage reservoirs, lakes, enclosed embayments, or the Pacific Ocean.			

□ No, the project will discharge runoff directly to an area identified as appropriate for an exemption by the WMAA for the watershed in which the project resides.

Description / Additional Information (to be provided if a 'No' answer has been selected above):

Critical Coarse Sediment Yield Areas*

*This section only required if hydromodification management requirements apply

Based on the maps provided within the City of Encinitas Engineering Design Manual dated January 2016, do potential critical coarse sediment yield areas exist within the project drainage boundaries?

 \Box Yes

☑ No, no critical coarse sediment yield areas to be protected based on WMAA maps

If yes, have any of the optional analyses presented in Section 6.2 of the BMP Design Manual been performed?

© 6.2.1 Verification of Geomorphic Landscape Units (GLUs) Onsite

□ 6.2.2 Downstream Systems Sensitivity to Coarse Sediment

- © 6.2.3 Optional Additional Analysis of Potential Critical Coarse Sediment Yield Areas Onsite
- □ No optional analyses performed, the project will avoid critical coarse sediment yield areas identified based on WMAA maps

If optional analyses were performed, what was the final result?

□ No critical coarse sediment yield areas to be protected based on verification of GLUs onsite

- □ Critical coarse sediment yield areas exist but additional analysis has determined that protection is not required. Documentation attached in Attachment 2.b of the SWQMP.
- □ Critical coarse sediment yield areas exist and require protection. The project will implement management measures described in Sections 6.2.4 and 6.2.5 as applicable, and the areas are identified on the SWQMP Exhibit.

Discussion / Additional Information:

Flow Control for Post-Project Runoff*

*This section only required if hydromodification management requirements apply

List and describe point(s) of compliance (POCs) for flow control for hydromodification management (see Section 6.3.1). For each POC, provide a POC identification name or number correlating to the project's HMP Exhibit and a receiving channel identification name or number correlating to the project's HMP Exhibit.

There is one (1) POC for the project, POC-1. POC-1 is located near the northeastern corner of the project site. Refer the exhibit located in Attachment 2a for the POC location.

Has a geomorphic assessment been performed for the receiving channel(s)?

☑ No, the low flow threshold is 0.1Q2 (default low flow threshold)

 \Box Yes, the result is low flow threshold 0.1Q2

 \Box Yes, the result is low flow threshold 0.3Q2

 \Box Yes, the result is low flow threshold 0.5Q2

If a geomorphic assessment has been performed, provide title, date, and preparer:

Discussion / Additional Information: (optional)

Other Site Requirements and Constraints

When applicable, list other site requirements or constraints that will influence storm water management design, such as zoning requirements including setbacks and open space, or local codes governing minimum street width, sidewalk construction, allowable pavement types, and drainage requirements.

SOURCE CONTROL BMP CHECKLIST

All development projects must implement source control BMPs SC-1 through SC-6 where applicable and feasible. See Chapter 4 and Appendix E of the manual for information to implement source control BMPs shown in this checklist.

Answer each category below pursuant to the following.

- "Yes" means the project will implement the source control BMP as described in Chapter 4 and/or Appendix E of the manual. Discussion / justification is not required.
- "No" means the BMP is applicable to the project but it is not feasible to implement. Discussion / justification must be provided.
- "N/A" means the BMP is not applicable at the project site because the project does not include the feature that is addressed by the BMP (e.g., the project has no outdoor materials storage areas). Discussion / justification may be provided.

Source Control Requirement		Applied?	
SC-1 Prevention of Illicit Discharges into the MS4	☑ Yes	□ No	□ N/A
SC-2 Storm Drain Stenciling or Signage	⊠ Yes	□ No	□ N/A
SC-3 Protect Outdoor Materials Storage Areas from Rainfall, Run-On, Runoff, and Wind Dispersal	□ Yes	□ No	⊠ N/A
SC-4 Protect Materials Stored in Outdoor Work Areas from Rainfall, Run-On, Runoff, and Wind Dispersal	□ Yes	□ No	⊠ N/A
SC-5 Protect Trash Storage Areas from Rainfall, Run-On, Runoff, and Wind Dispersal	⊠ Yes	□ No	□ N/A
SC-6 Additional BMPs Based on Potential Sources of Runoff Pollutants (must answer for each source listed below)			
☑ Onsite storm drain inlets	☑ Yes	🗆 No	□ N/A
☑ Interior floor drains and elevator shaft sump pumps drain to sewer	I Yes	🗆 No	□ N/A
☑ Interior parking garages drain to sewer	I Yes	🗆 No	N/A
Need for future indoor & structural pest control	🗌 Yes	🗆 No	⊠ N/A
☑ Landscape/outdoor pesticide use	☑ Yes	🗆 No	□ N/A
☑ Pools, spas, ponds, decorative fountains, and other water features	⊠ Yes	🗆 No	□ N/A
☑ Food service	☑ Yes	🗆 No	□ N/A
☑ Refuse/Trash areas must be covered	☑ Yes		□ N/A
Industrial processes	🗌 Yes	🗆 No	⊠ N/A
Outdoor storage of equipment or materials must be covered			⊠ N/A
□ Vehicle and equipment cleaning			⊠ N/A
□ Vehicle/equipment repair and maintenance			⊠ N/A
□ Fuel dispensing areas			⊠ N/A
			⊠ N/A
☑ Fire sprinkler test water	I Yes		□ N/A
☐ Miscellaneous drain or wash water	🗌 Yes		☑ N/A
☑ Plazas, sidewalks, and parking lots	⊠ Yes		□ N/A
Discussion / justification if SC-1 through SC-6 not implemented Justific	ation must		

Discussion / justification if <u>SC-1 through SC-6</u> not implemented. Justification must be provided for <u>ALL</u> "No" answers shown above.

SITE DESIGN BMP CHECKLIST

All development projects must implement site design BMPs SD-1 through SD-8 where applicable and feasible. See Chapter 4 and Appendix E of the manual for information to implement site design BMPs shown in this checklist.

Answer each category below pursuant to the following.

- "Yes" means the project will implement the site design BMP as described in Chapter 4 and/or Appendix E of the manual. Discussion / justification is not required.
- "No" means the BMP is applicable to the project but it is not feasible to implement. Discussion / justification must be provided.
- "N/A" means the BMP is not applicable at the project site because the project does not include the feature that is addressed by the BMP (e.g., the project site has no existing natural areas to conserve). Discussion / justification may be provided.

Source Control Requirement		Applied?	
SD-1 Maintain Natural Drainage Pathways and Hydrologic Features	□ Yes	□ No	⊠ N/A
SD-2 Conserve Natural Areas, Soils, and Vegetation	□ Yes	⊠ No	□ N/A
SD-3 Minimize Impervious Area	⊠ Yes	□ No	□ N/A
SD-4 Minimize Soil Compaction	⊠ Yes	□ No	□ N/A
SD-5 Impervious Area Dispersion - Directly Connected Impervious Areas (e.g. roof downspouts connected to street) are not allowed	⊠ Yes	□ No	□ N/A
SD-6 Runoff Collection	⊠ Yes	□ No	□ N/A
SD-7 Landscaping with Native or Drought Tolerant Species	⊠ Yes	□ No	□ N/A
SD-8 Harvesting and Using Precipitation	□ Yes	⊠ No	□ N/A

Discussion / justification if <u>SD-1 through SD-8</u> not implemented. Justification must be provided for <u>ALL</u> "No" answers shown above.

SD-2: The open space area in the southwestern portion of the site is a steep slope that will be graded to accommodate construction of buildings and improvements.

SD-8: Harvesting and using precipitation is not a feasible BMP for this project. Refer to Attachment 1c.

PDP STRUCTURAL BMPS

All PDPs must implement structural BMPs for storm water pollutant control (see Chapter 5 of the BMP Design Manual). Selection of PDP structural BMPs for storm water pollutant control must be based on the selection process described in Chapter 5. PDPs subject to hydromodification management requirements must also implement structural BMPs for flow control for hydromodification management (see Chapter 6 of the BMP Design Manual). Both storm water pollutant control and flow control for hydromodification management can be achieved within the same structural BMP(s).

PDP structural BMPs must be verified by the local jurisdiction at the completion of construction. This may include requiring the project owner or project owner's representative and engineer of record to certify construction of the structural BMPs (see Section 1.12 of the BMP Design Manual). PDP structural BMPs must be maintained into perpetuity (see Section 7 of the BMP Design Manual). The local jurisdiction will confirm the maintenance annually.

Use this section to provide narrative description of the general strategy for structural BMP implementation at the project site in the box below. Then complete the PDP structural BMP summary information sheet (page 3 of this form) for each structural BMP within the project (copy the BMP summary information page as many times as needed to provide summary information for each individual structural BMP).

Describe the general strategy for structural BMP implementation at the site. This information must describe how the steps for selecting and designing storm water pollutant control BMPs presented in Section 5.1 of the BMP Design Manual were followed, and the results (type of BMPs selected). For projects requiring hydromodification flow control BMPs, indicate whether pollutant control and flow control BMPs are integrated or separate.

DMAs 1-10 / BMPs 1-10 and 14

Step 1A: The DMA is not self-mitigating, de minimis, or self-retaining.

Step 1B: There are no site design BMPs proposed for the project for which the runoff factor can be adjusted.

Step 2: Harvest and use is not feasible. Refer to Attachment 1c.

Step 3: Pursuant to the geotechnical report and I-8 form, infiltration is feasible, however due to its potential to result in an increased risk of slope failure of existing slopes and nearby coastal bluff zones, infiltration is not suitable. Refer to Attachment 1d for the I-8 form.

Step 4: Biofiltration BMPs (BF-1) have been selected and sized per the design criteria to meet pollutant control requirements and an underground storage vault has been selected and sized per the design criteria to meet hydromodification management flow control requirements.

Right-Of-Way Improvements

Due to the timing of construction, the right-of-way improvements directly in front of the project site, per the plans prepared by Michael Baker International, will be built by Encinitas Beach Land Venture I, LLC. Plans prepared by Michael Baker International Inc. at this time do not indicate a clear strategy on how to treat all the street runoff for the portion of right-of-way improvements this project proposes to build. Other areas of the City of Encintas' capital improvement project, 101 Streetscape, proposes to utilize a Green Streets design strategy. Due to conflicts with existing utilities, the project proposes to install two Modular Wetlands System units to treat the street runoff, in lieu of Green Streets improvements.

Structural BMP ID No: BMPs 1	DMA Nos: 1	
Construction Plan Sheet No:		
Type of structural BMP:		
□ Retention by harvest and use (HU-1)		
□ Retention by infiltration basin (INF-1)		
□ Retention by bioretention (INF-2)		
□ Retention by permeable pavement (INF-3)		
□ Partial retention by biofiltration with partial retentior	n (PR-1)	
☑ Biofiltration (BF-1)		
□ Biofiltration with Nutrient Sensitive Media Design (E	3F-2)	
□ Proprietary Biofiltration (BF-3) meeting all requirem	,	
Flow-thru treatment control with prior lawful approv type/description in discussion section below)	al to meet earlier PDP requirements (provide BMP	
□ Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)		
□ Flow-thru treatment control with alternative compliance (provide BMP type/description in discussion section below)		
Detention pond or vault for hydromodification mana	agement	
□ Other (describe in discussion section below)		
Durneses		
Purpose: ☑ Pollutant control only		
Hydromodification control only		
Combined pollutant control and hydromodification control		
□ Pre-treatment/forebay for another structural BMP		
□ Other (describe in discussion section below)		
Who will inspect and certify construction of this	William J. Suiter, RCE 68964	
BMP? Provide name and contact information for	Pasco Laret Suiter & Associates	
the party responsible to sign BMP verification forms	535 North Highway 101, Suite A	
required by the City Engineer (See Section 1.12 of	Solana Beach, CA 92075	
the BMP Design Manual) Who will be the final owner of this BMP?	858-259-8212 Encinitas Beach Land Venture I, LLC	
Who will maintain this BMP into perpetuity?	Encinitas Beach Land Venture I, LLC	
What is the funding mechanism for maintenance?	Encinitas Beach Land Venture I, LLC	

Structural BMP ID No: BMPs 2	DMA Nos: 2	
Construction Plan Sheet No:		
Type of structural BMP:		
□ Retention by harvest and use (HU-1)		
□ Retention by infiltration basin (INF-1)		
□ Retention by bioretention (INF-2)		
□ Retention by permeable pavement (INF-3)		
□ Partial retention by biofiltration with partial retentior) (PR-1)	
☑ Biofiltration (BF-1)		
□ Biofiltration with Nutrient Sensitive Media Design (B	3F-2)	
□ Proprietary Biofiltration (BF-3) meeting all requirem	ents of Appendix F	
 Flow-thru treatment control with prior lawful approval to meet earlier PDP requirements (provide BMP type/description in discussion section below) 		
Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)		
 Flow-thru treatment control with alternative complia section below) 	nce (provide BMP type/description in discussion	
Detention pond or vault for hydromodification mana	agement	
□ Other (describe in discussion section below)		
Durneses		
Purpose: ☑ Pollutant control only		
□ Hydromodification control only		
□ Combined pollutant control and hydromodification control		
□ Pre-treatment/forebay for another structural BMP		
□ Other (describe in discussion section below)		
Who will inspect and certify construction of this	William J. Suiter, RCE 68964	
BMP? Provide name and contact information for	Pasco Laret Suiter & Associates	
the party responsible to sign BMP verification forms	535 North Highway 101, Suite A	
required by the City Engineer (See Section 1.12 of	Solana Beach, CA 92075	
the BMP Design Manual)	858-259-8212	
Who will be the final owner of this BMP? Who will maintain this BMP into perpetuity?	Encinitas Beach Land Venture I, LLC Encinitas Beach Land Venture I, LLC	
Who will maintain this BMP into perpetuity? What is the funding mechanism for maintenance?	Encinitas Beach Land Venture I, LLC Encinitas Beach Land Venture I, LLC	
	LININGS DEANT LANU VEIKUIE I, LLO	

Structural BMP ID No: BMPs 3	DMA Nos: 3	
Construction Plan Sheet No:		
Type of structural BMP:		
□ Retention by harvest and use (HU-1)		
□ Retention by infiltration basin (INF-1)		
□ Retention by bioretention (INF-2)		
□ Retention by permeable pavement (INF-3)		
□ Partial retention by biofiltration with partial retention	ı (PR-1)	
☑ Biofiltration (BF-1)		
□ Biofiltration with Nutrient Sensitive Media Design (E	3F-2)	
□ Proprietary Biofiltration (BF-3) meeting all requirem	ents of Appendix F	
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Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)		
 Flow-thru treatment control with alternative compliansection below) 	nce (provide BMP type/description in discussion	
Detention pond or vault for hydromodification mana	agement	
Other (describe in discussion section below)		
Purpose:		
☑ Pollutant control only		
□ Hydromodification control only		
□ Combined pollutant control and hydromodification control		
□ Pre-treatment/forebay for another structural BMP		
□ Other (describe in discussion section below)		
Who will inspect and certify construction of this	William J. Suiter, RCE 68964	
BMP? Provide name and contact information for	Pasco Laret Suiter & Associates	
the party responsible to sign BMP verification forms required by the City Engineer (See Section 1.12 of	535 North Highway 101, Suite A Solana Beach, CA 92075	
the BMP Design Manual)	858-259-8212	
Who will be the final owner of this BMP?	Encinitas Beach Land Venture I, LLC	
Who will maintain this BMP into perpetuity?	Encinitas Beach Land Venture I, LLC	
What is the funding mechanism for maintenance?	Encinitas Beach Land Venture I, LLC	

Structural BMP ID No: BMPs 4	DMA Nos: 4	
Construction Plan Sheet No:		
Type of structural BMP:		
□ Retention by harvest and use (HU-1)		
□ Retention by infiltration basin (INF-1)		
□ Retention by bioretention (INF-2)		
□ Retention by permeable pavement (INF-3)		
□ Partial retention by biofiltration with partial retention	ı (PR-1)	
☑ Biofiltration (BF-1)		
□ Biofiltration with Nutrient Sensitive Media Design (B	3F-2)	
□ Proprietary Biofiltration (BF-3) meeting all requirem	ents of Appendix F	
 Flow-thru treatment control with prior lawful approv type/description in discussion section below) 	al to meet earlier PDP requirements (provide BMP	
Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)		
 Flow-thru treatment control with alternative complianes section below) 	nce (provide BMP type/description in discussion	
Detention pond or vault for hydromodification mana	agement	
□ Other (describe in discussion section below)		
Purpose: ☑ Pollutant control only		
Hydromodification control only		
Combined pollutant control and hydromodification control		
□ Pre-treatment/forebay for another structural BMP		
□ Other (describe in discussion section below)		
Who will inspect and certify construction of this	William J. Suiter, RCE 68964	
BMP? Provide name and contact information for	Pasco Laret Suiter & Associates	
the party responsible to sign BMP verification forms	535 North Highway 101, Suite A	
required by the City Engineer (See Section 1.12 of the BMP Design Manual)	Solana Beach, CA 92075 858-259-8212	
Who will be the final owner of this BMP?	Encinitas Beach Land Venture I, LLC	
Who will maintain this BMP into perpetuity?	Encinitas Beach Land Venture I, LLC	
What is the funding mechanism for maintenance?	Encinitas Beach Land Venture I, LLC	

Structural BMP ID No: BMPs 5	DMA Nos: 5	
Construction Plan Sheet No:		
Type of structural BMP:		
□ Retention by harvest and use (HU-1)		
□ Retention by infiltration basin (INF-1)		
Retention by bioretention (INF-2)		
□ Retention by permeable pavement (INF-3)		
□ Partial retention by biofiltration with partial retention	ו (PR-1)	
☑ Biofiltration (BF-1)		
□ Biofiltration with Nutrient Sensitive Media Design (B	3F-2)	
□ Proprietary Biofiltration (BF-3) meeting all requirem	ents of Appendix F	
 Flow-thru treatment control with prior lawful approv type/description in discussion section below) 	al to meet earlier PDP requirements (provide BMP	
Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)		
 Flow-thru treatment control with alternative compliansection below) 	nce (provide BMP type/description in discussion	
Detention pond or vault for hydromodification mana	agement	
Other (describe in discussion section below)		
Purpose:		
☑ Pollutant control only		
□ Hydromodification control only		
Combined pollutant control and hydromodification control		
□ Pre-treatment/forebay for another structural BMP		
□ Other (describe in discussion section below)		
Who will inspect and certify construction of this	William J. Suiter, RCE 68964	
BMP? Provide name and contact information for	Pasco Laret Suiter & Associates	
the party responsible to sign BMP verification forms required by the City Engineer (See Section 1.12 of	535 North Highway 101, Suite A Solana Beach, CA 92075	
the BMP Design Manual)	858-259-8212	
Who will be the final owner of this BMP?	Encinitas Beach Land Venture I, LLC	
Who will maintain this BMP into perpetuity?	Encinitas Beach Land Venture I, LLC	
What is the funding mechanism for maintenance?	Encinitas Beach Land Venture I, LLC	

Structural BMP ID No: BMPs 6	DMA Nos: 6	
Construction Plan Sheet No:		
Type of structural BMP:		
□ Retention by harvest and use (HU-1)		
□ Retention by infiltration basin (INF-1)		
Retention by bioretention (INF-2)		
□ Retention by permeable pavement (INF-3)		
□ Partial retention by biofiltration with partial retention	ı (PR-1)	
☑ Biofiltration (BF-1)		
□ Biofiltration with Nutrient Sensitive Media Design (B	3F-2)	
□ Proprietary Biofiltration (BF-3) meeting all requirem	ents of Appendix F	
□ Flow-thru treatment control with prior lawful approval to meet earlier PDP requirements (provide BMP type/description in discussion section below)		
Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)		
 Flow-thru treatment control with alternative compliansection below) 	nce (provide BMP type/description in discussion	
Detention pond or vault for hydromodification mana	agement	
□ Other (describe in discussion section below)		
Purpose: ☑ Pollutant control only		
□ Hydromodification control only		
□ Combined pollutant control and hydromodification control		
□ Pre-treatment/forebay for another structural BMP		
□ Other (describe in discussion section below)		
Who will inspect and certify construction of this	William J. Suiter, RCE 68964	
BMP? Provide name and contact information for	Pasco Laret Suiter & Associates	
the party responsible to sign BMP verification forms	535 North Highway 101, Suite A	
required by the City Engineer (See Section 1.12 of the BMP Design Manual)	Solana Beach, CA 92075 858-259-8212	
Who will be the final owner of this BMP?	Encinitas Beach Land Venture I, LLC	
Who will maintain this BMP into perpetuity?	Encinitas Beach Land Venture I, LLC	
What is the funding mechanism for maintenance?	Encinitas Beach Land Venture I, LLC	

Structural BMP ID No: BMPs 7	DMA Nos: 7	
Construction Plan Sheet No:		
Type of structural BMP:		
□ Retention by harvest and use (HU-1)		
□ Retention by infiltration basin (INF-1)		
Retention by bioretention (INF-2)		
□ Retention by permeable pavement (INF-3)		
□ Partial retention by biofiltration with partial retention	ı (PR-1)	
☑ Biofiltration (BF-1)		
□ Biofiltration with Nutrient Sensitive Media Design (B	3F-2)	
□ Proprietary Biofiltration (BF-3) meeting all requirem	ents of Appendix F	
□ Flow-thru treatment control with prior lawful approval to meet earlier PDP requirements (provide BMP type/description in discussion section below)		
Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)		
 Flow-thru treatment control with alternative complianes section below) 	nce (provide BMP type/description in discussion	
Detention pond or vault for hydromodification mana	agement	
□ Other (describe in discussion section below)		
Purpose: ☑ Pollutant control only		
□ Hydromodification control only		
□ Combined pollutant control and hydromodification control		
□ Pre-treatment/forebay for another structural BMP		
□ Other (describe in discussion section below)		
Who will inspect and certify construction of this	William J. Suiter, RCE 68964	
BMP? Provide name and contact information for	Pasco Laret Suiter & Associates	
the party responsible to sign BMP verification forms required by the City Engineer (See Section 1.12 of	535 North Highway 101, Suite A Solana Beach, CA 92075	
the BMP Design Manual)	858-259-8212	
Who will be the final owner of this BMP?	Encinitas Beach Land Venture I, LLC	
Who will maintain this BMP into perpetuity?	Encinitas Beach Land Venture I, LLC	
What is the funding mechanism for maintenance?	Encinitas Beach Land Venture I, LLC	

Structural BMP ID No: BMPs 8	DMA Nos: 8	
Construction Plan Sheet No:		
Type of structural BMP:		
□ Retention by harvest and use (HU-1)		
□ Retention by infiltration basin (INF-1)		
□ Retention by bioretention (INF-2)		
□ Retention by permeable pavement (INF-3)		
□ Partial retention by biofiltration with partial retention	ו (PR-1)	
☑ Biofiltration (BF-1)		
□ Biofiltration with Nutrient Sensitive Media Design (E	3F-2)	
□ Proprietary Biofiltration (BF-3) meeting all requirem	ents of Appendix F	
 Flow-thru treatment control with prior lawful approv type/description in discussion section below) 	al to meet earlier PDP requirements (provide BMP	
Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)		
Flow-thru treatment control with alternative compliance (provide BMP type/description in discussion section below)		
Detention pond or vault for hydromodification mana	agement	
□ Other (describe in discussion section below)		
Purpose:		
Pollutant control only		
Hydromodification control only		
Combined pollutant control and hydromodification control		
□ Pre-treatment/forebay for another structural BMP		
□ Other (describe in discussion section below)		
Who will inspect and certify construction of this BMP? Provide name and contact information for	William J. Suiter, RCE 68964 Pasco Laret Suiter & Associates	
the party responsible to sign BMP verification forms	535 North Highway 101, Suite A	
required by the City Engineer (See Section 1.12 of	Solana Beach, CA 92075	
the BMP Design Manual)	858-259-8212	
Who will be the final owner of this BMP?	Encinitas Beach Land Venture I, LLC	
Who will maintain this BMP into perpetuity?	Encinitas Beach Land Venture I, LLC	
What is the funding mechanism for maintenance?	Encinitas Beach Land Venture I, LLC	

Structural BMP ID No: BMPs 9	DMA Nos: 9				
Construction Plan Sheet No:					
Type of structural BMP:					
□ Retention by harvest and use (HU-1)					
□ Retention by infiltration basin (INF-1)					
Retention by bioretention (INF-2)	□ Retention by bioretention (INF-2)				
□ Retention by permeable pavement (INF-3)					
□ Partial retention by biofiltration with partial retention	ı (PR-1)				
☑ Biofiltration (BF-1)					
□ Biofiltration with Nutrient Sensitive Media Design (B	3F-2)				
□ Proprietary Biofiltration (BF-3) meeting all requirements of Appendix F					
 Flow-thru treatment control with prior lawful approval to meet earlier PDP requirements (provide BMP type/description in discussion section below) 					
 Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below) 					
Flow-thru treatment control with alternative compliance (provide BMP type/description in discussion section below)					
Detention pond or vault for hydromodification management					
□ Other (describe in discussion section below)					
Purpose:					
Pollutant control only					
Hydromodification control only					
Combined pollutant control and hydromodification control					
□ Pre-treatment/forebay for another structural BMP					
□ Other (describe in discussion section below)					
Who will inspect and certify construction of this	William J. Suiter, RCE 68964				
BMP? Provide name and contact information for	Pasco Laret Suiter & Associates				
the party responsible to sign BMP verification forms required by the City Engineer (See Section 1.12 of	535 North Highway 101, Suite A Solana Beach, CA 92075				
the BMP Design Manual)	858-259-8212				
Who will be the final owner of this BMP?	Encinitas Beach Land Venture I, LLC				
Who will maintain this BMP into perpetuity?	Encinitas Beach Land Venture I, LLC				
What is the funding mechanism for maintenance? Encinitas Beach Land Venture I, LLC					

Structural BMP ID No: BMPs 10	DMA Nos: 10			
Construction Plan Sheet No:				
Type of structural BMP:				
□ Retention by harvest and use (HU-1)				
□ Retention by infiltration basin (INF-1)				
□ Retention by bioretention (INF-2)				
□ Retention by permeable pavement (INF-3)				
□ Partial retention by biofiltration with partial retention	n (PR-1)			
☑ Biofiltration (BF-1)				
□ Biofiltration with Nutrient Sensitive Media Design (E	3F-2)			
□ Proprietary Biofiltration (BF-3) meeting all requirem	ents of Appendix F			
Flow-thru treatment control with prior lawful approval to meet earlier PDP requirements (provide BMP type/description in discussion section below)				
 Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below) 				
Flow-thru treatment control with alternative compliance (provide BMP type/description in discussion section below)				
Detention pond or vault for hydromodification management				
Other (describe in discussion section below)				
Purpose:				
Pollutant control only				
Hydromodification control only				
Combined pollutant control and hydromodification control				
□ Pre-treatment/forebay for another structural BMP				
Other (describe in discussion section below)				
Who will inspect and certify construction of this	William J. Suiter, RCE 68964			
BMP? Provide name and contact information for the party responsible to sign BMP verification forms	Pasco Laret Suiter & Associates 535 North Highway 101, Suite A			
required by the City Engineer (See Section 1.12 of	Solana Beach, CA 92075			
the BMP Design Manual)	858-259-8212			
Who will be the final owner of this BMP?	Encinitas Beach Land Venture I, LLC			
Who will maintain this BMP into perpetuity? Encinitas Beach Land Venture I, LLC				
What is the funding mechanism for maintenance? Encinitas Beach Land Venture I, LLC				

	[]			
Structural BMP ID No: BMP-14	DMA No: 1-10			
Construction Plan Sheet No:				
Type of structural BMP:				
□ Retention by harvest and use (HU-1)				
Retention by infiltration basin (INF-1)				
□ Retention by bioretention (INF-2)				
□ Retention by permeable pavement (INF-3)				
□ Partial retention by biofiltration with partial retention (PR-1)				
□ Biofiltration (BF-1)				
 Biofiltration with Nutrient Sensitive Media Design (BF-2) Proprietary Biofiltration (BF-3) meeting all requirements of Appendix F 				
Flow-thru treatment control with prior lawful approval to meet earlier PDP requirements (provide BMP type/description in discussion section below)				
Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)				
 Flow-thru treatment control with alternative compliance (provide BMP type/description in discussion section below) 				
☑ Detention pond or vault for hydromodification management				
□ Other (describe in discussion section below)				
Purpose:				
□ Pollutant control only				
☑Hydromodification control only				
Combined pollutant control and hydromodification control				
□ Pre-treatment/forebay for another structural BMP				
□ Other (describe in discussion section below)				
Who will inspect and certify construction of this	William J. Suiter, RCE 68964			
BMP? Provide name and contact information for	Pasco Laret Suiter & Associates			
the party responsible to sign BMP verification forms	535 North Highway 101, Suite A			
required by the City Engineer (See Section 1.12 of	Solana Beach, CA 92075			
the BMP Design Manual) Who will be the final owner of this BMP?	858-259-8212			
Who will be the final owner of this BMP?Encinitas Beach Land Venture I, LLCWho will maintain this BMP into perpetuity?Encinitas Beach Land Venture I, LLC				
What is the funding mechanism for maintenance? Encinitas Beach Land Venture I, LLC				

ATTACHMENT 1 - BACKUP FOR PDP POLLUTANT CONTROL BMPS

This is the cover sheet for Attachment 1.

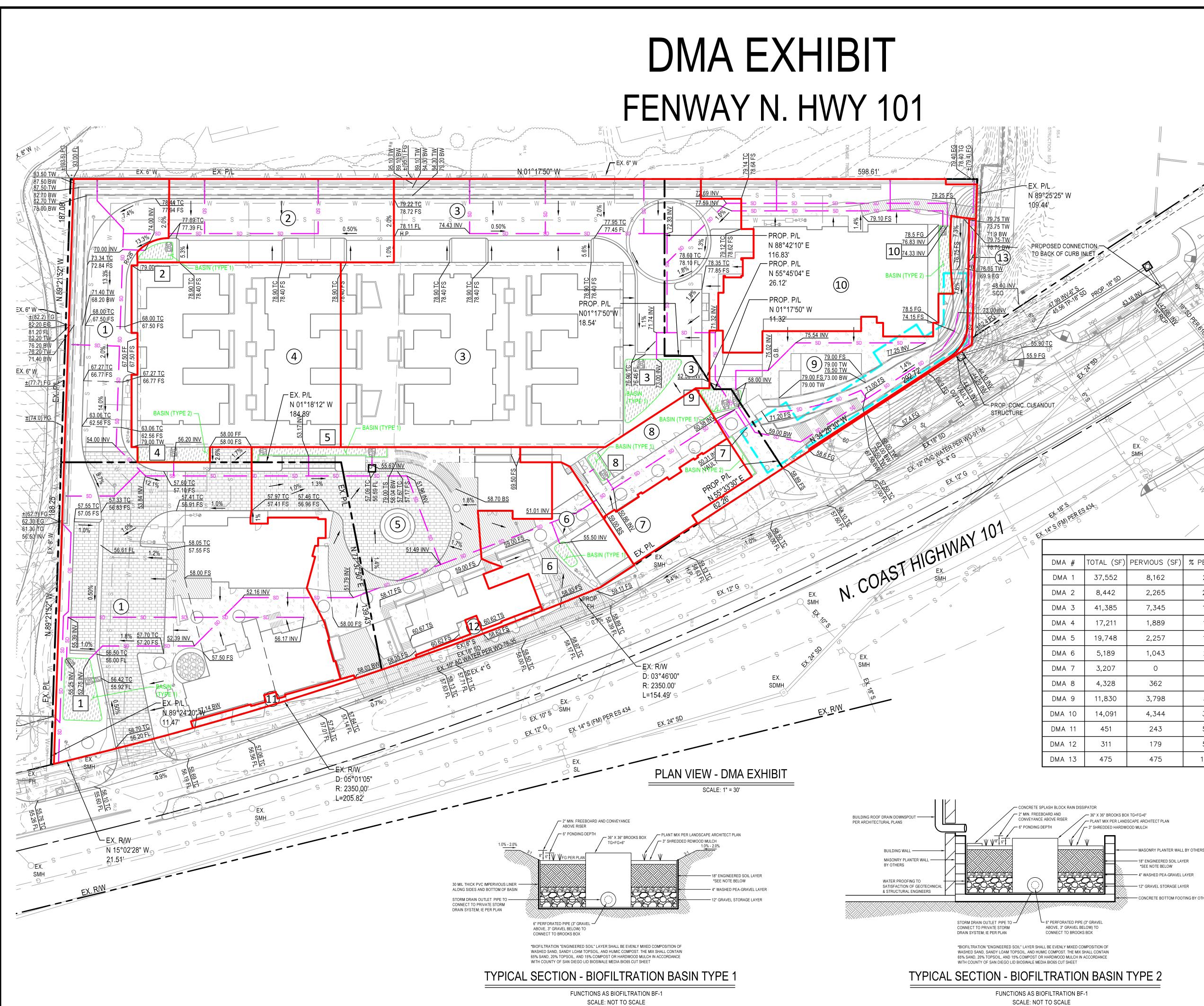
Indicate which items are included behind this cover sheet:

Attachment	Contents	Checklist		
Attachment 1a	DMA Exhibit (Required) See DMA Exhibit Checklist on the back of this Attachment cover sheet.	☑ Included		
Attachment 1b	Tabular Summary of DMAs Showing DMA ID matching DMA Exhibit, DMA Area, and DMA Type (Required)* *Provide table in this Attachment OR on DMA Exhibit in Attachment 1a	 ☑ Included on DMA Exhibit in Attachment 1a □ Included as Attachment 1b, separate from DMA Exhibit 		
Attachment 1c	Form I-7, Harvest and Use Feasibility Screening Checklist (Required unless the entire project will use infiltration BMPs) Refer to Appendix B.3-1 of the BMP Design Manual to complete Form I-7.	 ☑ Included □ Not included because the entire project will use infiltration BMPs 		
Attachment 1d	Form I-8, Categorization of Infiltration Feasibility Condition (Required unless the project will use harvest and use BMPs) Refer to Appendices C and D of the BMP Design Manual to complete Form I- 8.	 ☑ Included □ Not included because the entire project will use harvest and use BMPs 		
Attachment 1e	Pollutant Control BMP Design Worksheets / Calculations (Required) Refer to Appendices B and E of the BMP Design Manual for structural pollutant control BMP design guidelines	☑ Included		

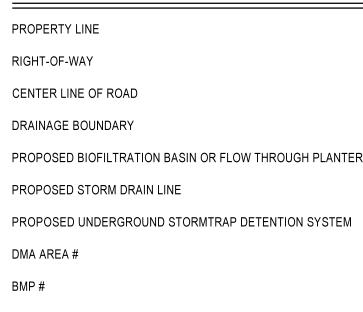
Use this checklist to ensure the required information has been included on the DMA Exhibit:

The DMA Exhibit must identify:

- Underlying hydrologic soil group
- □ Approximate depth to groundwater
- □ Existing natural hydrologic features (watercourses, seeps, springs, wetlands)
- □ Critical coarse sediment yield areas to be protected
- □ Existing topography and impervious areas
- □ Existing and proposed site drainage network and connections to drainage offsite
- Proposed demolition
- □ Proposed grading
- □ Proposed impervious features
- □ Proposed design features and surface treatments used to minimize imperviousness
- □ Drainage management area (DMA) boundaries, DMA ID numbers, and DMA areas (square footage or acreage), and DMA type (i.e., drains to BMP, self-retaining, or self-mitigating)
- □ Potential pollutant source areas and corresponding required source controls (see Chapter 4, Appendix E.1, and Form I-3B)
- □ Structural BMPs (identify location, type of BMP, and size/detail)



LEGEND



PROPOSED BMP BIOFILTRATION BASIN (TYPE 1 OR TYPE 2) PLANTER PER DETAILS HEREON.

AREA CALCULATIONS

TOTAL SITE AREA: AREA DISTURBED BY PROJECT: 165,107 SF (3.790 AC)

165,107 SF (3.790 AC)

EXISTING IMPERVIOUS AREA: 76,820 SF (1.764 AC) PROPOSED IMPERVIOUS AREA: 143,659 SF (3.298 AC) INCREASE IMPERVIOUS AREA: 66,839 SF (1.534 AC)

SOIL TYPE INFORMATION

SOIL: TYPE B HYDROLOGIC SOILS PER OBSERVED ONSITE / FIELD INFILTRATION RATES PROVIDED IN "GEOTECHNICAL INVESTIGATION -LEUCADIA MIXED-USE " PREPARED BY NOVA SERVICES, INC. AND ONSITE SOIL CLASSIFICATION PROVIDED BY NOVA SERVICES, INC.

 \mathbf{X}

GROUNDWATER INFORMATION

GROUNDWATER NOT ENCOUNTERED IN ANY OF THE BORINGS CONDUCTED BY NOVA SERVICES, INC., ACCORDING TO SECTION 4.2.3 OF THE PRELIMINARY GEOTECHNICAL STUDY PREPARED BY NOVA SERVICES, INC. CONCLUSION IN REPORT STATES THAT GROUNDWATER "THUS OCCURS BELOW ABOUT EI + 10 MSL, AT LEAST 48 FEET BELOW THE FINISHED FLOOR OF THE LOWEST PARKING LEVEL (SET +58 MSL)."

TREATMENT CONTROL BMPS

BIOFILTRATION BF-1

POTENTIAL POLLUTANT SOURCE AREAS

TRASH DUMPESTERS, ROOFS, PARKING LOTS, LANDSCAPING, RESTAURANT(S), WATER FEATURES, AND OUTDOOR STORAGE AREAS.

SOURCE CONTROLS

SC-1: PREVENTION OF ILLICIT DISCHARGES INTO THE MS4 SC-2: STORM DRAIN STENCILING OR SIGNAGE

SC-5: PROTECT TRASH STORAGE AREAS FROM RAINFALL, RUN-ON, RUNOFF, AND WIND DISPERSAL SC-6: ADDITIONAL BMPS BASED ON POTENTIAL SOURCES ON RUNOFF POLLUTANTS

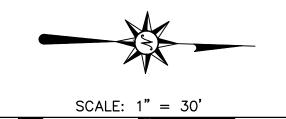
NOTE

PROJECT SITE WILL MAINTAIN AND FOLLOW SAME WATER QUALITY DESIGN PROPOSED WITHIN THE CITY'S HIGHWAY 101 STREETSCAPE PROJECT FOR RIGHT-OF-WAY IN FRONT OF PROJECT SITE.

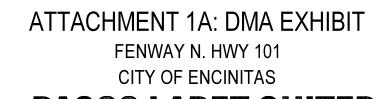
DMA AREAS						
OTAL (SF)	PERVIOUS (SF)	% PERVIOUS	% IMPERVIOUS	MIN. BASIN AREA (SF)	BASIN PROVIDED (SF)	TREATMENT BMP(S)
37,552	8,162	21.7	78.3	867	870	1
8,442	2,265	26.8	73.2	187	196	2
41,385	7,345	17.7	82.3	981	983	3
17,211	1,889	11.0	89.0	429	434	4
19,748	2,257	11.4	88.6	492	500	5
5,189	1,043	20.1	79.9	121	128	6
3,207	0	0	100	87	93	7
4,328	362	8.4	91.6	110	132	8
11,830	3,798	32.1	67.9	252	301	9
14,091	4,344	30.8	69.2	304	305	10
451	243	53.9	46.1	_	-	SELF MITIGATING
311	179	57.6	42.4	_	-	SELF MITIGATING
475	475	100.0	0	_	_	SELF MITIGATING

GRAVEL STORAGE LAYE

CONCRETE BOTTOM FOOTING BY OTHERS



15



PASCO LARET SUITER & ASSOCIATES San Diego I Solana Beach I Orange County

Phone 949.661.6695 | www.plsaengineering.com

ATTACHMENT 1c

Worksheet B.3-1. Harvest and Use Feasibility Screening

Worksheet B.3-1. Harvest and Use Feasib Harvest and Use Feasibility Scree	· · ·	/orsksheet B.3-1
	ter (check all that apply) at the proje	ect site that is reliably present during the
wet season? ✓Toilet and urinal flushing		
 ✓ Landscape irrigation 		
Other:		
		n demand over a period of 36 hours. ning and landscape irrigation is provided
Toilet/Urinal Flushing Residences and (9.3 gal/person-day) x (0.13368 cuft/ga Assume (400 people) x (1.86 cuft/pers	al) x (1.5 days) = 1.86 cuft/person-30	5hr
<u>Toilet/Urinal Flushing Retail</u> (7x1.4 gal/person-day) x (0.13368 cuft Assume (18 people) x (1.97 cuft/perso		-36hr
Landscape Irrigation (0.76 ac irrigated) x (390 gal/ac-36hr) >	(0.13368 cuft/gal) = 40 cuft/36hr	
Total = 744 cuft + 36 cuft + 40 cuft = 8	320 cuft	
3. Calculate the DCV using worksheet		
DCV = 5,977		
3a. Is the 36-hour demand greater than or equal to the DCV? Yes / ✓ No	3b. Is the 36-hour demand greater 0.25DCV but less than the full DCV Yes / ✓ No	
Harvest and use appears to be feasible. Conduct more detailed evaluation and sizing calculations to confirm that DCV can be used at an adequate rate to meet drawdown criteria.	Harvest and use may be feasible. O more detailed evaluation and sizin calculations to determine feasibilit Harvest and use may only be able used for a portion of the site, or (optionally) the storage may need upsized to meet long term capture while draining in longer than 36 ho	to be targets

ATTACHMENT 1d

Categ	orization of Infiltration Feasibility Condition	Form	n I-8			
Would i	Full Infiltration Feasibility Screening Criteria nfiltration of the full design volume be feasible from a physical per lences that cannot be reasonably mitigated?	spective withou	it any undesirable			
Criteria Screening Question Yes No						
1	Is the estimated reliable infiltration rate below proposed facility locations greater than 0.5 inches per hour? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.2 and Appendix D.	X				
study w hour fo	iltration rate of the existing soils for locations P-1 and P-2, based bas calculated to be greater than 0.5 inches per hour (1.51 inches or P-1 and P-2, respectively) after applying a minimum factor of so ize findings of studies; provide reference to studies, calculations, maps, on of study/data source applicability.	per hour and 1 afety (F) of F=	.74 inches per 2.			
2	Can infiltration greater than 0.5 inches per hour be allowed without increasing risk of geotechnical hazards (slope stability, groundwater mounding, utilities, or other factors) that cannot be mitigated to an acceptable level? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.2.		X			
	tion of water has the potential to result in an increased risk of slop coastal bluff zones. As such, BMPs are not suitable for any location		sting slopes and			

Appendix I: Forms and Checklists

	Form I-8 Page 2 of 4							
Criteria	Screening Question	Yes	No					
3	Can infiltration greater than 0.5 inches per hour be allowed without increasing risk of groundwater contamination (shallow water table, stormwater pollutants or other factors) that cannot be mitigated to an acceptable level? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.							
Provide	pasis:							
Water c	ontamination was not evaluated by NOVA Services.							
	ze findings of studies; provide reference to studies, calculations, maps, on of study/data source applicability.	lata sources, etc	:. Provide narrative					
4	Can infiltration greater than 0.5 inches per hour be allowed without causing potential water balance issues such as change of seasonality of ephemeral streams or increased discharge of contaminated groundwater to surface waters? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.							
Provide	pasis:							
The pot	ential for water balance was not evaluated by NOVA Services.							
	ze findings of studies; provide reference to studies, calculations, maps, on of study/data source applicability.	lata sources, etc	. Provide narrative					
Part 1 Result *	Part 1 If all answers to rows 1 - 4 are "Yes" a full infiltration design is potentially feasible. The feasibility screening category is Full Infiltration If any answer from row 1.4 is "No" infiltration may be possible to some extent but							

*To be completed using gathered site information and best professional judgment considering the definition of MEP in the MS4 Permit. Additional testing and/or studies may be required by Agency/Jurisdictions to substantiate findings

Form I-8 Page 3 of 4

Part 2 - Partial Infiltration vs. No Infiltration Feasibility Screening Criteria

Would infiltration of water in any appreciable amount be physically feasible without any negative consequences that cannot be reasonably mitigated?

Criteria	Screening Question	Yes	No
5	Do soil and geologic conditions allow for infiltration in any appreciable rate or volume? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.2 and Appendix D.	X	

Provide basis:

The infiltration rate of the existing soils for locations P-1 and P-2, based on the on-site infiltration study was calculated to be greater than 0.5 inches per hour (1.51 inches per hour and 1.74 inches per hour for P-1 and P-2, respectively) after applying a minimum factor of safety (F) of F=2.

The soil and geologic conditions allow for infiltration but not without increasing the risk of geotechnical hazards.

Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.

	Can Infiltration in any appreciable quantity be allowed without increasing risk of geotechnical hazards (slope stability,	
6	groundwater mounding, utilities, or other factors) that cannot	v
0	be mitigated to an acceptable level? The response to this Screening	Λ
	Question shall be based on a comprehensive evaluation of the factors	
	presented in Appendix C.2.	

Provide basis:

Infiltration of water has the potential to result in an increased risk of slope failure of existing slopes and nearby coastal bluff zones. As such, BMPs are not suitable for any location on site.

Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.

Appendix I: Forms and Checklists

CriteriaScreening QuestionYesNo7Can Infiltration in any appreciable quantity be allowed without posing significant risk for groundwater related concerns (shallow water table, stormwater pollutanits or other factors)? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.Image: Can Infiltration any appreciable quantity be allowed without posing significant risk for groundwater pollutanits or other factors)? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.Image: Can Infiltration appendix C.3.Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrad discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.8Can infiltration be allowed without violating downstream water rights? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.Image: Can Infiltration appendix C.3.Provide basis: The pot=mtial for water balance was not evaluated by NOVA Services.Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrad discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrad discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.Summarize findings of studies; provide reference to studies, calculations, maps, data		Form I-8 Page 4 of 4							
7 posing significant risk for groundwater related concerns (shallow water table, stormwater pollutants or other factors)? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3. Provide basis: Water contamination was not evaluated by NOVA Services. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrad discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. 8 Can infiltration be allowed without violating downstream water rights? The response to this Sercening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3. Provide basis: The potential for water balance was not evaluated by NOVA Services. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrad discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. Provide basis: The potential for water balance was not evaluated by NOVA Services. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrad discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrad discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. <tr< th=""><th>Criteria</th><th>Screening Question</th><th>Yes</th><th>No</th></tr<>	Criteria	Screening Question	Yes	No					
Water contamination was not evaluated by NOVA Services. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrat discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. 8 Can infiltration be allowed without violating downstream water rights? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3. Provide basis: The potential for water balance was not evaluated by NOVA Services. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrat discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. Provide basis: The potential for water balance was not evaluated by NOVA Services. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrat discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. Part 2 If all answers from row 1-4 are yes then partial infiltration design is potentially feasible. The feasibility screening category is Partial Infiltration. If any answer from row 5-8 is no, then infiltration of any volume is considered to be No Infiltration be and the provide reference to be and the provide reference is a provide reference to studies, calculations, maps, data sources, etc. Provide narrated iscussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.	7	 posing significant risk for groundwater related concerns (shallow water table, stormwater pollutants or other factors)? The response to this Screening Question shall be based on a 							
Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrat discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. 8 Can infiltration be allowed without violating downstream water rights? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3. Provide basis: The potential for water balance was not evaluated by NOVA Services. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrat discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. Part 2 If all answers from row 1-4 are yes then partial infiltration. The feasibility screening category is Partial Infiltration. If any answer from row 5-8 is no, then infiltration of any volume is considered to be No Infiltration	Provide b	asis:							
discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. 8 Can infiltration be allowed without violating downstream water rights? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3. Provide basis: The potential for water balance was not evaluated by NOVA Services. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrat discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. Part 2 If all answers from row 1-4 are yes then partial infiltration. The feasibility screening category is Partial Infiltration. If any answer from row 5-8 is no, then infiltration of any volume is considered to be No Infiltration	Water co	ontamination was not evaluated by NOVA Services.							
discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. 8 Can infiltration be allowed without violating downstream water rights? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3. Provide basis: The potential for water balance was not evaluated by NOVA Services. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrat discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. Part 2 If all answers from row 1-4 are yes then partial infiltration. The feasibility screening category is Partial Infiltration. If any answer from row 5-8 is no, then infiltration of any volume is considered to be No Infiltration									
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discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. 8 Can infiltration be allowed without violating downstream water rights? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3. Provide basis: The potential for water balance was not evaluated by NOVA Services. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrat discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. Part 2 If all answers from row 1-4 are yes then partial infiltration. The feasibility screening category is Partial Infiltration. If any answer from row 5-8 is no, then infiltration of any volume is considered to be No Infiltration									
8 rights? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3. Provide basis: The potential for water balance was not evaluated by NOVA Services. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrat discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. Part 2 If all answers from row 1-4 are yes then partial infiltration. The feasibility screening category is Partial Infiltration. If any answer from row 5-8 is no, then infiltration of any volume is considered to be No Infiltration									
The potential for water balance was not evaluated by NOVA Services. Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrat discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. Part 2 Result* If all answers from row 1-4 are yes then partial infiltration. The feasibility screening category is Partial Infiltration. If any answer from row 5-8 is no, then infiltration of any volume is considered to be No Infiltration	8	rights? The response to this Screening Question shall be based on a							
Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrat discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. Provide narrat from rates. Part 2 If all answers from row 1-4 are yes then partial infiltration design is potentially feasible. The feasibility screening category is Partial Infiltration. No Infiltration If any answer from row 5-8 is no, then infiltration of any volume is considered to be No Infiltration	Provide b	isis:							
discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. Part 2 Result* If all answer from row 1-4 are yes then partial infiltration. No Infiltration N	The pote	ntial for water balance was not evaluated by NOVA Services.							
discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. Part 2 Result* If all answer from row 1-4 are yes then partial infiltration. No Infiltration N									
discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates. Part 2 Result* If all answer from row 1-4 are yes then partial infiltration. No Infiltration N									
Part 2 Result*The feasibility screening category is Partial Infiltration.No InfiltrationIf any answer from row 5-8 is no, then infiltration of any volume is considered to beNo Infiltration									
Result* If any answer from row 5-8 is no, then infiltration of any volume is considered to be	Part 2		ootentially feasible.	No Infiltration					
		If any answer from row 5-8 is no, then infiltration of any volume is considered to be							

*To be completed using gathered site information and best professional judgment considering the definition of MEP in the MS4 Permit. Additional testing and/or studies may be required by Agency/Jurisdictions to substantiate findings

ATTACHMENT 1e

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Appendix B: Stormwater Pollutant Control Hydrologic Calculations and Sizing Methods

Worksheet B.2-1. DCV

	DMA 1				
De	esign Capture Volume	Worksheet	B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches	
2	Area tributary to BMP (s)	A=	0.86	acres	
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1) * See calculation below	C=	0.77	unitless	
4	Street trees volume reduction	TCV=	0	cubic-feet	
5	5 Rain barrels volume reduction (1 cubic foot=7.48 gallons)		0	cubic-feet	
6	Calculate DCV = (3630 x C x d x A) – TCV - RCV	DCV=	1346	cubic-feet	

	Area (sq ft)	Runoff Factor	A x RF	Weighted RF
Impervious	29390	0.9	26451	
Pervious Pavers		0.1	0	
Landscape	8162	0.3	2448.6	
Total	37552		28900	0.77

	DMA 2			
De	esign Capture Volume	Worksheet	B-2.1	
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches
2	Area tributary to BMP (s)	A=	0.19	acres
2	Area weighted runoff factor (estimate using Appendix B.1.1 and	C=	0.74	unitless
5	B.2.1) * See calculation below	C-	0.74	unitiess
4	Street trees volume reduction	TCV=	0	cubic-feet
5	Rain barrels volume reduction (1 cubic foot=7.48 gallons)	RCV=	0	cubic-feet
6	Calculate DCV =	DCV=	286	cubic-feet
0	(3630 x C x d x A) – TCV - RCV	DCV-	280	cubic-leet

	Area (sq ft)	Runoff Factor	A x RF	Weighted RF
Impervious	6177	0.9	5559.3	
Pervious Pavers		0.1	0	
Landscape	2265	0.3	679.5	
Total	8442		6239	0.74

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Appendix B: Stormwater Pollutant Control Hydrologic Calculations and Sizing Methods

Worksheet B.2-1. DCV

	DMA 3				
De	esign Capture Volume	Worksheet B-2.1			
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches	
2	Area tributary to BMP (s)	A=	0.95	acres	
3	Area weighted runoff factor (estimate using Appendix B.1.1 and	C=	0.79	unitless	
5	B.2.1) * See calculation below	C-	0.79	unitiess	
4	Street trees volume reduction	TCV=	0	cubic-feet	
5	Rain barrels volume reduction (1 cubic foot=7.48 gallons)	RCV=	0	cubic-feet	
6	Calculate DCV =	DCV=	1526	cubic-feet	
0	(3630 x C x d x A) – TCV - RCV	DCV-	1520	cubic-leet	

	Area (sq ft)	Runoff Factor	A x RF	Weighted RF
Impervious	34040	0.9	30636	
Pervious Pavers		0.1	0	
Landscape	7345	0.3	2203.5	
Total	41385		32840	0.79

	DMA 4			
De	esign Capture Volume	Worksheet	B-2.1	
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches
2	Area tributary to BMP (s)	A=	0.395	acres
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1) * See calculation below	C=	0.83	unitless
4	Street trees volume reduction	TCV=	0	cubic-feet
5	Rain barrels volume reduction (1 cubic foot=7.48 gallons)	RCV=	0	cubic-feet
6	Calculate DCV = (3630 x C x d x A) – TCV - RCV	DCV=	666	cubic-feet

	Area (sq ft)	Runoff Factor	A x RF	Weighted RF
Impervious	15322	0.9	13789.8	
Pervious Pavers		0.1	0	
Landscape	1889	0.3	566.7	
Total	17211		14357	0.83

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Appendix B: Stormwater Pollutant Control Hydrologic Calculations and Sizing Methods

Worksheet B.2-1. DCV

	DMA 5				
De	esign Capture Volume	Worksheet	B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches	
2	Area tributary to BMP (s)	A=	0.45	acres	
3	Area weighted runoff factor (estimate using Appendix B.1.1 and	C=	0.83	unitless	
Ū	B.2.1) * See calculation below	Ũ	0.00	unnenees	
4	Street trees volume reduction	TCV=	0	cubic-feet	
5	Rain barrels volume reduction (1 cubic foot=7.48 gallons)	RCV=	0	cubic-feet	
6	Calculate DCV =	DCV=	759	cubic-feet	
0	(3630 x C x d x A) – TCV - RCV		759	cubic-leet	

	Area (sq ft)	Runoff Factor	A x RF	Weighted RF
Impervious	17491	0.9	15741.9	
Pervious Pavers		0.1	0	
Landscape	2257	0.3	677.1	
Total	19748		16419	0.83

	DMA 6			
De	esign Capture Volume	Worksheet	B-2.1	
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches
2	Area tributary to BMP (s)	A=	0.12	acres
2	Area weighted runoff factor (estimate using Appendix B.1.1 and	C=	0.78	unitless
5	B.2.1) * See calculation below	C=	0.78	unitiess
4	Street trees volume reduction	TCV=	0	cubic-feet
5	Rain barrels volume reduction (1 cubic foot=7.48 gallons)	RCV=	0	cubic-feet
6	Calculate DCV =	DCV=	190	cubic-feet
0	(3630 x C x d x A) – TCV - RCV	DCv=	190	CUDIC-IEEL

	Area (sq ft)	Runoff Factor	A x RF	Weighted RF
Impervious	4146	0.9	3731.4	
Pervious Pavers		0.1	0	
Landscape	1043	0.3	312.9	
Total	5189		4044	0.78

Appendix B: Stormwater Pollutant Control Hydrologic Calculations and Sizing Methods Worksheet B.2-1. DCV

	DMA 7			
De	esign Capture Volume	Worksheet	B-2.1	
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches
2	Area tributary to BMP (s)	A=	0.07	acres
3	Area weighted runoff factor (estimate using Appendix B.1.1 and	C=	0.90	unitless
5	B.2.1) * See calculation below	C-	0.90	unitiess
4	Street trees volume reduction	TCV=	0	cubic-feet
5	Rain barrels volume reduction (1 cubic foot=7.48 gallons)	RCV=	0	cubic-feet
6	Calculate DCV =	DCV=	128	cubic-feet
0	(3630 x C x d x A) – TCV - RCV	DCV-	120	cubic-leet

	Area (sq ft)	Runoff Factor	A x RF	Weighted RF
Impervious	3207	0.9	2886.3	
Pervious Pavers		0.1	0	
Landscape	0	0.3	0	
Total	3207		2886	0.90

	DMA 8				
De	esign Capture Volume	Worksheet	B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches	
2	Area tributary to BMP (s)	A=	0.1	acres	
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1) * See calculation below	C=	0.85	unitless	
4	Street trees volume reduction	TCV=	0	cubic-feet	
5	Rain barrels volume reduction (1 cubic foot=7.48 gallons)	RCV=	0	cubic-feet	
6	Calculate DCV = (3630 x C x d x A) – TCV - RCV	DCV=	173	cubic-feet	

	Area (sq ft)	Runoff Factor	A x RF	Weighted RF
Impervious	3966	0.9	3569.4	
Pervious Pavers		0.1	0	
Landscape	362	0.3	108.6	
Total	4328		3678	0.85

Appendix B: Stormwater Pollutant Control Hydrologic Calculations and Sizing Methods Worksheet B.2-1. DCV

	DMA 9			
De	esign Capture Volume	Worksheet	B-2.1	
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches
2	Area tributary to BMP (s)	A=	0.27	acres
3	Area weighted runoff factor (estimate using Appendix B.1.1 and	C=	0.71	unitless
5	B.2.1) * See calculation below	C-	0.71	unitiess
4	Street trees volume reduction	TCV=	0	cubic-feet
5	Rain barrels volume reduction (1 cubic foot=7.48 gallons)	RCV=	0	cubic-feet
6	Calculate DCV =	DCV=	390	cubic-feet
0	(3630 x C x d x A) – TCV - RCV	DCV-	590	cubic-leet

	Area (sq ft)	Runoff Factor	A x RF	Weighted RF
Impervious	8032	0.9	7228.8	
Pervious Pavers		0.1	0	
Landscape	3798	0.3	1139.4	
Total	11830		8368	0.71

	DMA 10				
De	esign Capture Volume	Worksheet	B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches	
2	Area tributary to BMP (s)	A=	0.32	acres	
	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1) * See calculation below	C=	0.72	unitless	
4	Street trees volume reduction	TCV=	0	cubic-feet	
5	Rain barrels volume reduction (1 cubic foot=7.48 gallons)	RCV=	0	cubic-feet	
6	Calculate DCV = (3630 x C x d x A) – TCV - RCV	DCV=	468	cubic-feet	

	Area (sq ft)	Runoff Factor	A x RF	Weighted RF
Impervious	9747	0.9	8772.3	
Pervious Pavers		0.1	0	
Landscape	4344	0.3	1303.2	
Total	14091		10076	0.72

Appendix B: Stormwater Pollutant Control Hydrologic Calculations and Sizing Methods Worksheet B.2-1. DCV

	DMA 11				
Design Capture Volume		Worksheet B-2.1			
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches	
2	Area tributary to BMP (s)	A=	0.01	acres	
3	Area weighted runoff factor (estimate using Appendix B.1.1 and	C=	0.58	unitless	
	B.2.1) * See calculation below				
4	Street trees volume reduction	TCV=	0	cubic-feet	
5	Rain barrels volume reduction (1 cubic foot=7.48 gallons)	RCV=	0	cubic-feet	
6	Calculate DCV =	DCV=	12	cubic-feet	
Ľ	(3630 x C x d x A) – TCV - RCV	501-	12		

	Area (sq ft)	Runoff Factor	A x RF	Weighted RF
Impervious	208	0.9	187.2	
Pervious Pavers		0.1	0	
Landscape	243	0.3	72.9	
Total	451		260	0.58

	DMA 12				
De	esign Capture Volume	Worksheet	B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches	
2	Area tributary to BMP (s)	A=	0.007	acres	
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1) * See calculation below	C=	0.55	unitless	
4	Street trees volume reduction	TCV=	0	cubic-feet	
5	Rain barrels volume reduction (1 cubic foot=7.48 gallons)	RCV=	0	cubic-feet	
6	Calculate DCV = (3630 x C x d x A) – TCV - RCV	DCV=	8	cubic-feet	

	Area (sq ft)	Runoff Factor	A x RF	Weighted RF
Impervious	132	0.9	118.8	
Pervious Pavers		0.1	0	
Landscape	179	0.3	53.7	
Total	311		173	0.55

	Simple Sizing Method for Biofiltration BMPs	Worksh	eet B.5-1		
1	Remaining DCV after implementing retention BMPs	1346.0	cu-ft		
Parti	al Retention				
2	Infiltration rate from Worksheet D.5-1 if partial infiltration is feasible	0.00	in/hr		
3	Allowable drawdown time for aggregate storage below the underdrain	36	hours		
4	Depth of runoff that can be infiltrated [Line 2 x Line 3]	0.00	inches		
5	Aggregate pore space	0.40	in/in		
6	Required depth of gravel below the underdrain [Line 4 / Line 5]	0.00	inches		
7	Assumed surface area of the biofiltration BMP	867.0	sq-ft		
8	Media retained pore space	0.1	in/in		
9	Volume retained by BMP [[Line 4 + (Line 12 x Line 8)]/12] x Line 7	130.05	cu-ft		
10	DCV that requires biofiltration [Line 1 - Line 9]	1216.0	cu-ft		
BMP	Parameters				
11	Surface Ponding [6 inch minimum, 12 inch maximum]	6	inches		
12	Media Thickness [18 in minimum]	18	inches		
10	Aggregate Storage above underdrain invert (12 inches typical) - Use 0 inches for	0	inches		
13	sizing if the aggregate is not over the entire bottom surface area	9	inches		
14	Media available pore space	0.2	in/in		
15	Media filtration rate to be used for sizing	5	in/hr		
Base	line Calculations				
16	Allowable Routing Time for sizing	6	hours		
17	Depth filtered during storm [Line 15 x Line 16]	30	inches		
	Depth of Detention Storage	13.20	inches		
18	[Line 11 + (Line 12 x Line 14) + (Line 13 x Line 5)]	13.20	literes		
19	Total Depth Treated [Line 17 + Line 18]	43.20	inches		
Optio	on 1 - Biofilter 1.5 times the DCV				
20	Required biofiltered volume [1.5 x Line 10]	1824	cu-ft		
21	Required Footprint [Line 20 / Line 19] x 12	506.6	sq-ft		
Optio	on 2 - Store 0.75 of remaining DCV in pores and ponding				
22	Required Storage (surface + pores) Volume [0.75 x Line 10]	912	cu-ft		
23	Required Footprint [Line 22 / Line 18] x 12	829	sq-ft		
Foot	print of the BMP				
24	Area draining to the BMP	37552	sq-ft		
25	Adjusted Runoff Factor for drainage area (Refer to Appendix B.1 and B.2)	0.77	•		
26	Minimum BMP Footprint [Line 24 x Line 25 x 0.03]	867	sq-ft		
27	Footprint of the BMP = Maximum (Minimum(Line 21, Line 23), Line 27)	867	sq-ft		

	Simple Sizing Method for Biofiltration BMPs Worksheet B.5-1					
1	Remaining DCV after implementing retention BMPs	286.0	cu-ft			
Partia	al Retention					
2	Infiltration rate from Worksheet D.5-1 if partial infiltration is feasible	0.00	in/hr			
3	Allowable drawdown time for aggregate storage below the underdrain	36	hours			
4	Depth of runoff that can be infiltrated [Line 2 x Line 3]	0.00	inches			
5	Aggregate pore space	0.40	in/in			
6	Required depth of gravel below the underdrain [Line 4 / Line 5]	0.00	inches			
7	Assumed surface area of the biofiltration BMP	192.0	sq-ft			
8	Media retained pore space	0.1	in/in			
9	Volume retained by BMP [[Line 4 + (Line 12 x Line 8)]/12] x Line 7	28.80	cu-ft			
10	DCV that requires biofiltration [Line 1 - Line 9]	257.2	cu-ft			
BMP	Parameters					
11	Surface Ponding [6 inch minimum, 12 inch maximum]	6	inches			
12	Media Thickness [18 in minimum]	18	inches			
13	Aggregate Storage above underdrain invert (12 inches typical) - Use 0 inches for	0	inches			
13	sizing if the aggregate is not over the entire bottom surface area	9	Inches			
14	Media available pore space	0.2	in/in			
15	Media filtration rate to be used for sizing	5	in/hr			
Base	line Calculations					
16	Allowable Routing Time for sizing	6	hours			
17	Depth filtered during storm [Line 15 x Line 16]	30	inches			
	Depth of Detention Storage	13.20	inches			
18	[Line 11 + (Line 12 x Line 14) + (Line 13 x Line 5)]	15.20	inches			
19	Total Depth Treated [Line 17 + Line 18]	43.20	inches			
Optic	on 1 - Biofilter 1.5 times the DCV					
20	Required biofiltered volume [1.5 x Line 10]	386	cu-ft			
21	Required Footprint [Line 20 / Line 19] x 12	107.2	sq-ft			
Optic	on 2 - Store 0.75 of remaining DCV in pores and ponding					
22	Required Storage (surface + pores) Volume [0.75 x Line 10]	193	cu-ft			
23	Required Footprint [Line 22 / Line 18] x 12	175	sq-ft			
Foot	print of the BMP					
24	Area draining to the BMP	8442	sq-ft			
25	Adjusted Runoff Factor for drainage area (Refer to Appendix B.1 and B.2)	0.74				
26	Minimum BMP Footprint [Line 24 x Line 25 x 0.03]	187	sq-ft			
27	Footprint of the BMP = Maximum (Minimum(Line 21, Line 23), Line 27)	187	sq-ft			

	Simple Sizing Method for Biofiltration BMPs Worksheet B.5-1					
1	Remaining DCV after implementing retention BMPs	1526.0	cu-ft			
Parti	al Retention					
2	Infiltration rate from Worksheet D.5-1 if partial infiltration is feasible	0.00	in/hr			
3	Allowable drawdown time for aggregate storage below the underdrain	36	hours			
4	Depth of runoff that can be infiltrated [Line 2 x Line 3]	0.00	inches			
5	Aggregate pore space	0.40	in/in			
6	Required depth of gravel below the underdrain [Line 4 / Line 5]	0.00	inches			
7	Assumed surface area of the biofiltration BMP	981.0	sq-ft			
8	Media retained pore space	0.1	in/in			
9	Volume retained by BMP [[Line 4 + (Line 12 x Line 8)]/12] x Line 7	147.15	cu-ft			
10	DCV that requires biofiltration [Line 1 - Line 9]	1378.9	cu-ft			
BMP	Parameters					
11	Surface Ponding [6 inch minimum, 12 inch maximum]	6	inches			
12	Media Thickness [18 in minimum]	18	inches			
10	Aggregate Storage above underdrain invert (12 inches typical) - Use 0 inches for	0				
13	sizing if the aggregate is not over the entire bottom surface area	9	inches			
14	Media available pore space	0.2	in/in			
15	Media filtration rate to be used for sizing	5	in/hr			
Base	ine Calculations					
16	Allowable Routing Time for sizing	6	hours			
17	Depth filtered during storm [Line 15 x Line 16]	30	inches			
	Depth of Detention Storage	13.20	inches			
18	[Line 11 + (Line 12 x Line 14) + (Line 13 x Line 5)]	15.20	Inches			
19	Total Depth Treated [Line 17 + Line 18]	43.20	inches			
Optio	on 1 - Biofilter 1.5 times the DCV					
20	Required biofiltered volume [1.5 x Line 10]	2068	cu-ft			
21	Required Footprint [Line 20 / Line 19] x 12	574.5	sq-ft			
Optio	on 2 - Store 0.75 of remaining DCV in pores and ponding					
22	Required Storage (surface + pores) Volume [0.75 x Line 10]	1034	cu-ft			
23	Required Footprint [Line 22 / Line 18] x 12	940	sq-ft			
Foot	print of the BMP	· 1				
24	Area draining to the BMP	41385	sq-ft			
25	Adjusted Runoff Factor for drainage area (Refer to Appendix B.1 and B.2)	0.79	•			
26	Minimum BMP Footprint [Line 24 x Line 25 x 0.03]	981	sq-ft			
27	Footprint of the BMP = Maximum (Minimum(Line 21, Line 23), Line 27)	981	sq-ft			

	Simple Sizing Method for Biofiltration BMPs Worksheet B.5-1				
1	Remaining DCV after implementing retention BMPs	666.0	cu-ft		
Parti	al Retention				
2	Infiltration rate from Worksheet D.5-1 if partial infiltration is feasible	0.00	in/hr		
3	Allowable drawdown time for aggregate storage below the underdrain	36	hours		
4	Depth of runoff that can be infiltrated [Line 2 x Line 3]	0.00	inches		
5	Aggregate pore space	0.40	in/in		
6	Required depth of gravel below the underdrain [Line 4 / Line 5]	0.00	inches		
7	Assumed surface area of the biofiltration BMP	429.0	sq-ft		
8	Media retained pore space	0.1	in/in		
9	Volume retained by BMP [[Line 4 + (Line 12 x Line 8)]/12] x Line 7	64.35	cu-ft		
10	DCV that requires biofiltration [Line 1 - Line 9]	601.7	cu-ft		
BMP	Parameters				
11	Surface Ponding [6 inch minimum, 12 inch maximum]	6	inches		
12	Media Thickness [18 in minimum]	18	inches		
13	Aggregate Storage above underdrain invert (12 inches typical) - Use 0 inches for	9	inches		
15	sizing if the aggregate is not over the entire bottom surface area	9	inches		
14	Media available pore space	0.2	in/in		
15	Media filtration rate to be used for sizing	5	in/hr		
Base	line Calculations				
16	Allowable Routing Time for sizing	6	hours		
17	Depth filtered during storm [Line 15 x Line 16]	30	inches		
	Depth of Detention Storage	13.20	inches		
18	[Line 11 + (Line 12 x Line 14) + (Line 13 x Line 5)]	15.20	literes		
19	Total Depth Treated [Line 17 + Line 18]	43.20	inches		
Optic	on 1 - Biofilter 1.5 times the DCV				
20	Required biofiltered volume [1.5 x Line 10]	902	cu-ft		
21	Required Footprint [Line 20 / Line 19] x 12	250.7	sq-ft		
Optio	on 2 - Store 0.75 of remaining DCV in pores and ponding				
22	Required Storage (surface + pores) Volume [0.75 x Line 10]	451	cu-ft		
23	Required Footprint [Line 22 / Line 18] x 12	410	sq-ft		
Foot	print of the BMP				
24	Area draining to the BMP	17211	sq-ft		
25	Adjusted Runoff Factor for drainage area (Refer to Appendix B.1 and B.2)	0.83	-		
26	Minimum BMP Footprint [Line 24 x Line 25 x 0.03]	429	sq-ft		
27	Footprint of the BMP = Maximum (Minimum(Line 21, Line 23), Line 27)	429	sq-ft		

	Simple Sizing Method for Biofiltration BMPs Worksheet B.5-1				
1	Remaining DCV after implementing retention BMPs	759.0	cu-ft		
Parti	al Retention				
2	Infiltration rate from Worksheet D.5-1 if partial infiltration is feasible	0.00	in/hr		
3	Allowable drawdown time for aggregate storage below the underdrain	36	hours		
4	Depth of runoff that can be infiltrated [Line 2 x Line 3]	0.00	inches		
5	Aggregate pore space	0.40	in/in		
6	Required depth of gravel below the underdrain [Line 4 / Line 5]	0.00	inches		
7	Assumed surface area of the biofiltration BMP	492.0	sq-ft		
8	Media retained pore space	0.1	in/in		
9	Volume retained by BMP [[Line 4 + (Line 12 x Line 8)]/12] x Line 7	73.80	cu-ft		
10	DCV that requires biofiltration [Line 1 - Line 9]	685.2	cu-ft		
BMP	Parameters				
11	Surface Ponding [6 inch minimum, 12 inch maximum]	6	inches		
12	Media Thickness [18 in minimum]	18	inches		
10	Aggregate Storage above underdrain invert (12 inches typical) - Use 0 inches for	0	inchoo		
13	sizing if the aggregate is not over the entire bottom surface area	9	inches		
14	Media available pore space	0.2	in/in		
15	Media filtration rate to be used for sizing	5	in/hr		
Base	ine Calculations				
16	Allowable Routing Time for sizing	6	hours		
17	Depth filtered during storm [Line 15 x Line 16]	30	inches		
	Depth of Detention Storage	13.20	inches		
18	[Line 11 + (Line 12 x Line 14) + (Line 13 x Line 5)]	13.20	linches		
19	Total Depth Treated [Line 17 + Line 18]	43.20	inches		
Optio	on 1 - Biofilter 1.5 times the DCV				
20	Required biofiltered volume [1.5 x Line 10]	1028	cu-ft		
21	Required Footprint [Line 20 / Line 19] x 12	285.5	sq-ft		
Optio	on 2 - Store 0.75 of remaining DCV in pores and ponding	· · · · · · · · · · · · · · · · · · ·			
22	Required Storage (surface + pores) Volume [0.75 x Line 10]	514	cu-ft		
23	Required Footprint [Line 22 / Line 18] x 12	467	sq-ft		
Foot	print of the BMP				
24	Area draining to the BMP	19748	sq-ft		
25	Adjusted Runoff Factor for drainage area (Refer to Appendix B.1 and B.2)	0.83			
26	Minimum BMP Footprint [Line 24 x Line 25 x 0.03]	492	sq-ft		
27	Footprint of the BMP = Maximum (Minimum(Line 21, Line 23), Line 27)	492	sq-ft		

	DIVIA	_				
	Simple Sizing Method for Biofiltration BMPs Worksheet B.5-1					
1	Remaining DCV after implementing retention BMPs	190.0	cu-ft			
Parti	al Retention					
2	Infiltration rate from Worksheet D.5-1 if partial infiltration is feasible	0.00	in/hr			
3	Allowable drawdown time for aggregate storage below the underdrain	36	hours			
4	Depth of runoff that can be infiltrated [Line 2 x Line 3]	0.00	inches			
5	Aggregate pore space	0.40	in/in			
6	Required depth of gravel below the underdrain [Line 4 / Line 5]	0.00	inches			
7	Assumed surface area of the biofiltration BMP	121.0	sq-ft			
8	Media retained pore space	0.1	in/in			
9	Volume retained by BMP [[Line 4 + (Line 12 x Line 8)]/12] x Line 7	18.15	cu-ft			
10	DCV that requires biofiltration [Line 1 - Line 9]	171.9	cu-ft			
BMP	Parameters					
11	Surface Ponding [6 inch minimum, 12 inch maximum]	6	inches			
12	Media Thickness [18 in minimum]	18	inches			
13	Aggregate Storage above underdrain invert (12 inches typical) - Use 0 inches for	9	inches			
13	sizing if the aggregate is not over the entire bottom surface area	9	inches			
14	Media available pore space	0.2	in/in			
15	Media filtration rate to be used for sizing	5	in/hr			
Base	line Calculations					
16	Allowable Routing Time for sizing	6	hours			
17	Depth filtered during storm [Line 15 x Line 16]	30	inches			
	Depth of Detention Storage	13.20	inches			
18	[Line 11 + (Line 12 x Line 14) + (Line 13 x Line 5)]					
19	Total Depth Treated [Line 17 + Line 18]	43.20	inches			
Optio	on 1 - Biofilter 1.5 times the DCV					
20	Required biofiltered volume [1.5 x Line 10]	258	cu-ft			
21	Required Footprint [Line 20 / Line 19] x 12	71.6	sq-ft			
Optio	on 2 - Store 0.75 of remaining DCV in pores and ponding					
22	Required Storage (surface + pores) Volume [0.75 x Line 10]	129	cu-ft			
23	Required Footprint [Line 22 / Line 18] x 12	117	sq-ft			
Foot	print of the BMP					
24	Area draining to the BMP	5189	sq-ft			
25	Adjusted Runoff Factor for drainage area (Refer to Appendix B.1 and B.2)	0.78	•			
26	Minimum BMP Footprint [Line 24 x Line 25 x 0.03]	121	sq-ft			
27	Footprint of the BMP = Maximum (Minimum(Line 21, Line 23), Line 27)	121	sq-ft			

		-	
	Simple Sizing Method for Biofiltration BMPs	Worksh	eet B.5-1
1	Remaining DCV after implementing retention BMPs	128.0	cu-ft
Parti	al Retention		
2	Infiltration rate from Worksheet D.5-1 if partial infiltration is feasible	0.00	in/hr
3	Allowable drawdown time for aggregate storage below the underdrain	36	hours
4	Depth of runoff that can be infiltrated [Line 2 x Line 3]	0.00	inches
5	Aggregate pore space	0.40	in/in
6	Required depth of gravel below the underdrain [Line 4 / Line 5]	0.00	inches
7	Assumed surface area of the biofiltration BMP	87.0	sq-ft
8	Media retained pore space	0.1	in/in
9	Volume retained by BMP [[Line 4 + (Line 12 x Line 8)]/12] x Line 7	13.05	cu-ft
10	DCV that requires biofiltration [Line 1 - Line 9]	115.0	cu-ft
BMP	Parameters		
11	Surface Ponding [6 inch minimum, 12 inch maximum]	6	inches
12	Media Thickness [18 in minimum]	18	inches
13	Aggregate Storage above underdrain invert (12 inches typical) - Use 0 inches for	9	inches
15	sizing if the aggregate is not over the entire bottom surface area	9	inches
14	Media available pore space	0.2	in/in
15	Media filtration rate to be used for sizing	5	in/hr
Base	line Calculations		
16	Allowable Routing Time for sizing	6	hours
17	Depth filtered during storm [Line 15 x Line 16]	30	inches
	Depth of Detention Storage	13.20	inches
18	[Line 11 + (Line 12 x Line 14) + (Line 13 x Line 5)]	15.20	inches
19	Total Depth Treated [Line 17 + Line 18]	43.20	inches
Optic	on 1 - Biofilter 1.5 times the DCV		
20	Required biofiltered volume [1.5 x Line 10]	172	cu-ft
21	Required Footprint [Line 20 / Line 19] x 12	47.9	sq-ft
Optic	on 2 - Store 0.75 of remaining DCV in pores and ponding		
22	Required Storage (surface + pores) Volume [0.75 x Line 10]	86	cu-ft
23	Required Footprint [Line 22 / Line 18] x 12	78	sq-ft
Foot	print of the BMP		
24	Area draining to the BMP	3207	sq-ft
25	Adjusted Runoff Factor for drainage area (Refer to Appendix B.1 and B.2)	0.90	•
26	Minimum BMP Footprint [Line 24 x Line 25 x 0.03]	87	sq-ft
27	Footprint of the BMP = Maximum (Minimum(Line 21, Line 23), Line 27)	87	sq-ft

	Simple Sizing Method for Biofiltration BMPs		eet B.5-1
1	Remaining DCV after implementing retention BMPs	173.0	cu-ft
Partia	al Retention		
2	Infiltration rate from Worksheet D.5-1 if partial infiltration is feasible	0.00	in/hr
3	Allowable drawdown time for aggregate storage below the underdrain	36	hours
4	Depth of runoff that can be infiltrated [Line 2 x Line 3]	0.00	inches
5	Aggregate pore space	0.40	in/in
6	Required depth of gravel below the underdrain [Line 4 / Line 5]	0.00	inches
7	Assumed surface area of the biofiltration BMP	110.0	sq-ft
8	Media retained pore space	0.1	in/in
9	Volume retained by BMP [[Line 4 + (Line 12 x Line 8)]/12] x Line 7	16.50	cu-ft
10	DCV that requires biofiltration [Line 1 - Line 9]	156.5	cu-ft
BMP	Parameters		
11	Surface Ponding [6 inch minimum, 12 inch maximum]	6	inches
12	Media Thickness [18 in minimum]	18	inches
13	Aggregate Storage above underdrain invert (12 inches typical) - Use 0 inches for	0	inches
15	sizing if the aggregate is not over the entire bottom surface area	9	inches
14	Media available pore space	0.2	in/in
15	Media filtration rate to be used for sizing	5	in/hr
Base	line Calculations		
16	Allowable Routing Time for sizing	6	hours
17	Depth filtered during storm [Line 15 x Line 16]	30	inches
	Depth of Detention Storage	13.20	inches
18	[Line 11 + (Line 12 x Line 14) + (Line 13 x Line 5)]	13.20	inches
19	Total Depth Treated [Line 17 + Line 18]	43.20	inches
Optic	on 1 - Biofilter 1.5 times the DCV		
20	Required biofiltered volume [1.5 x Line 10]	235	cu-ft
21	Required Footprint [Line 20 / Line 19] x 12	65.2	sq-ft
Optic	on 2 - Store 0.75 of remaining DCV in pores and ponding		
22	Required Storage (surface + pores) Volume [0.75 x Line 10]	117	cu-ft
23	Required Footprint [Line 22 / Line 18] x 12	107	sq-ft
Foot	print of the BMP	· ·	
24	Area draining to the BMP	4328	sq-ft
25	Adjusted Runoff Factor for drainage area (Refer to Appendix B.1 and B.2)	0.85	
26	Minimum BMP Footprint [Line 24 x Line 25 x 0.03]	110	sq-ft
27	Footprint of the BMP = Maximum (Minimum(Line 21, Line 23), Line 27)	110	sq-ft

	Simple Sizing Method for Biofiltration BMPs	Worksh	eet B.5-1
1	Remaining DCV after implementing retention BMPs	390.0	cu-ft
Parti	al Retention		
2	Infiltration rate from Worksheet D.5-1 if partial infiltration is feasible	0.00	in/hr
3	Allowable drawdown time for aggregate storage below the underdrain	36	hours
4	Depth of runoff that can be infiltrated [Line 2 x Line 3]	0.00	inches
5	Aggregate pore space	0.40	in/in
6	Required depth of gravel below the underdrain [Line 4 / Line 5]	0.00	inches
7	Assumed surface area of the biofiltration BMP	252.0	sq-ft
8	Media retained pore space	0.1	in/in
9	Volume retained by BMP [[Line 4 + (Line 12 x Line 8)]/12] x Line 7	37.80	cu-ft
10	DCV that requires biofiltration [Line 1 - Line 9]	352.2	cu-ft
BMP	Parameters		
11	Surface Ponding [6 inch minimum, 12 inch maximum]	6	inches
12	Media Thickness [18 in minimum]	18	inches
10	Aggregate Storage above underdrain invert (12 inches typical) - Use 0 inches for	0	in choo
13	sizing if the aggregate is not over the entire bottom surface area	9	inches
14	Media available pore space	0.2	in/in
15	Media filtration rate to be used for sizing	5	in/hr
Base	line Calculations		
16	Allowable Routing Time for sizing	6	hours
17	Depth filtered during storm [Line 15 x Line 16]	30	inches
	Depth of Detention Storage	13.20	inches
18	[Line 11 + (Line 12 x Line 14) + (Line 13 x Line 5)]	15.20	inches
19	Total Depth Treated [Line 17 + Line 18]	43.20	inches
Optio	on 1 - Biofilter 1.5 times the DCV		
20	Required biofiltered volume [1.5 x Line 10]	528	cu-ft
21	Required Footprint [Line 20 / Line 19] x 12	146.8	sq-ft
Optio	on 2 - Store 0.75 of remaining DCV in pores and ponding		
22	Required Storage (surface + pores) Volume [0.75 x Line 10]	264	cu-ft
23	Required Footprint [Line 22 / Line 18] x 12	240	sq-ft
Foot	print of the BMP		
24	Area draining to the BMP	11830	sq-ft
25	Adjusted Runoff Factor for drainage area (Refer to Appendix B.1 and B.2)	0.71	
26	Minimum BMP Footprint [Line 24 x Line 25 x 0.03]	252	sq-ft
27	Footprint of the BMP = Maximum (Minimum(Line 21, Line 23), Line 27)	252	sq-ft

S	Simple Sizing Method for Biofiltration BMPs	Worksh	eet B.5-1	
1	Remaining DCV after implementing retention BMPs	468.0	cu-ft	
Parti	al Retention			
2	Infiltration rate from Worksheet D.5-1 if partial infiltration is feasible	0.00	in/hr	
3	Allowable drawdown time for aggregate storage below the underdrain	36	hours	
4	Depth of runoff that can be infiltrated [Line 2 x Line 3]	0.00	inches	
5	Aggregate pore space	0.40	in/in	
6	Required depth of gravel below the underdrain [Line 4 / Line 5]	0.00	inches	
7	Assumed surface area of the biofiltration BMP	304.0	sq-ft	
8	Media retained pore space	0.1	in/in	
9	Volume retained by BMP [[Line 4 + (Line 12 x Line 8)]/12] x Line 7	45.60	cu-ft	
10	DCV that requires biofiltration [Line 1 - Line 9]	422.4	cu-ft	
BMP	Parameters			
11	Surface Ponding [6 inch minimum, 12 inch maximum]	6	inches	
12	Media Thickness [18 in minimum]	18	inches	
10	Aggregate Storage above underdrain invert (12 inches typical) - Use 0	0	in choc	
13	inches for sizing if the aggregate is not over the entire bottom surface	9	inches	
14	Media available pore space	0.2	in/in	
15	Media filtration rate to be used for sizing	5	in/hr	
Base	line Calculations			
16	Allowable Routing Time for sizing	6	hours	
17	Depth filtered during storm [Line 15 x Line 16]	30	inches	
	Depth of Detention Storage	13.20	inches	
18	[Line 11 + (Line 12 x Line 14) + (Line 13 x Line 5)]	13.20	inches	
19	Total Depth Treated [Line 17 + Line 18]	43.20	inches	
Opti	on 1 - Biofilter 1.5 times the DCV			
20	Required biofiltered volume [1.5 x Line 10]	634	cu-ft	
21	Required Footprint [Line 20 / Line 19] x 12	176.0	sq-ft	
Opti	on 2 - Store 0.75 of remaining DCV in pores and ponding			
22	Required Storage (surface + pores) Volume [0.75 x Line 10]	317	cu-ft	
23	Required Footprint [Line 22 / Line 18] x 12	288	sq-ft	
Foot	print of the BMP		· ·	
24	Area draining to the BMP	14091	sq-ft	
25	Adjusted Runoff Factor for drainage area (Refer to Appendix B.1 and B.2)			
26	Minimum BMP Footprint [Line 24 x Line 25 x 0.03]	304	sq-ft	
27	Footprint of the BMP = Maximum (Minimum(Line 21, Line 23), Line 27)	304	sq-ft	

ATTACHMENT 2 - BACKUP FOR PDP HYDROMODIFICATION CONTROL MEASURES

This is the cover sheet for Attachment 2.

□ Mark this box if this attachment is not included because the project is exempt from PDP hydromodification management requirements.

Attachment	Contents	Checklist
Attachment 2a	Hydromodification Management Exhibit (Required)	☑ Included
		See Hydromodification Management Exhibit Checklist on the back of this Attachment cover sheet.
Attachment 2b	Management of Critical Coarse Sediment Yield Areas (WMAA Exhibit is required, additional analyses are optional) See Section 6.2 of the BMP Design Manual.	 Exhibit showing project drainage boundaries marked on City of Encinitas Potential Critical Coarse Sediment Yield Area Map (Required) Optional analyses for Critical Coarse Sediment Yield Area Determination 6.2.1 Verification of Geomorphic Landscape Units Onsite 6.2.2 Downstream Systems Sensitivity to Coarse Sediment 6.2.3 Optional Additional Analysis of Potential Critical Coarse Sediment Yield Areas Onsite
Attachment 2c	Geomorphic Assessment of Receiving Channels (Optional) See Section 6.3.4 of the BMP Design Manual.	 Not performed Included Submitted as separate stand-alone document
Attachment 2d	Flow Control Facility Design, including Structural BMP Drawdown Calculations and Overflow Design Summary (Required) See Chapter 6 and Appendix G of the BMP Design Manual	 ☑ Included □ Submitted as separate stand-alone document
Attachment 2e	Vector Control Plan (Required when structural BMPs will not drain in 96 hours)	 □ Included ☑ Not required because BMPs will drain in less than 96 hours

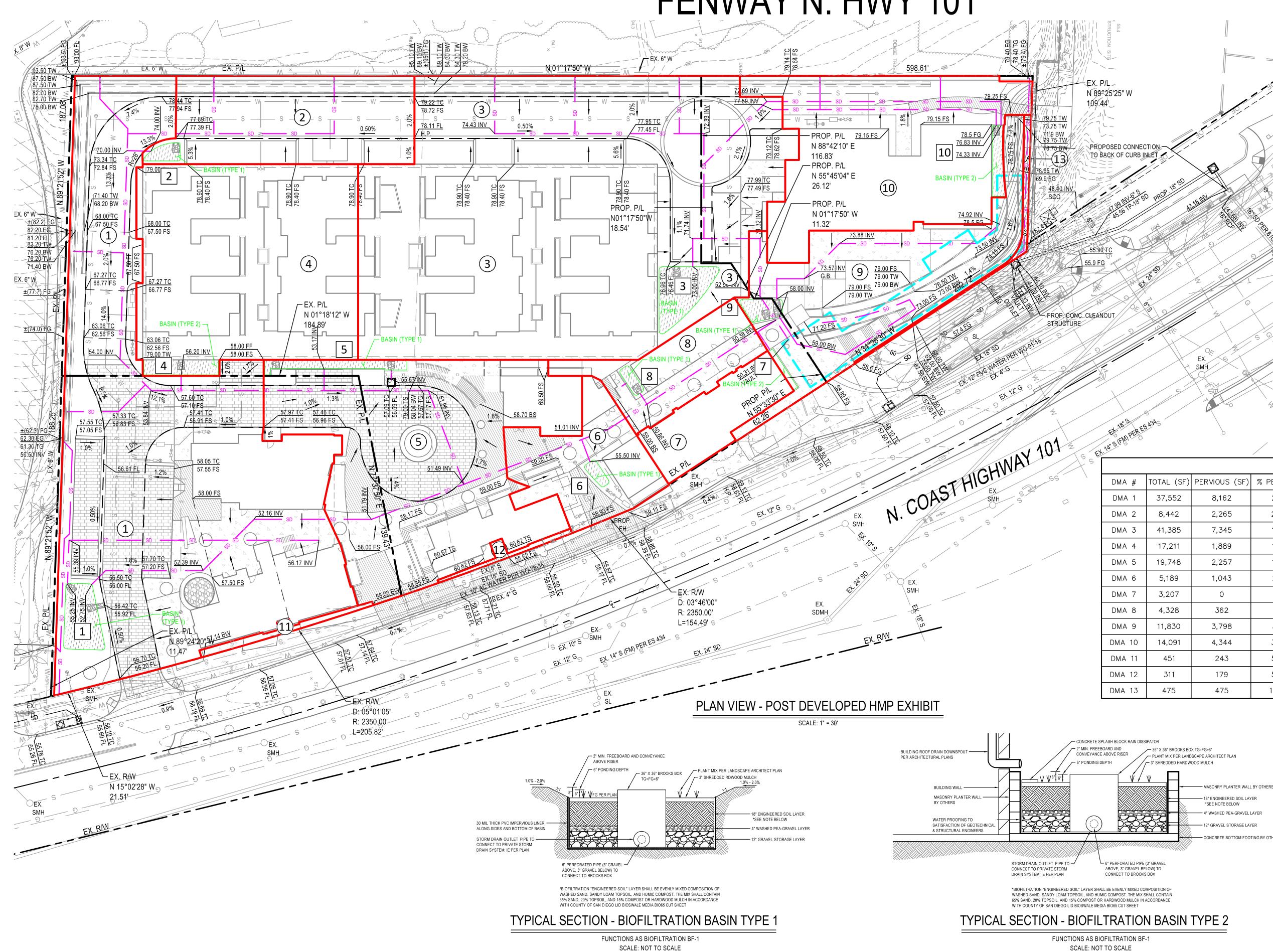
Indicate which items are included behind this cover sheet:

Use this checklist to ensure the required information has been included on the Hydromodification Management Exhibit:

The Hydromodification Management Exhibit must identify:

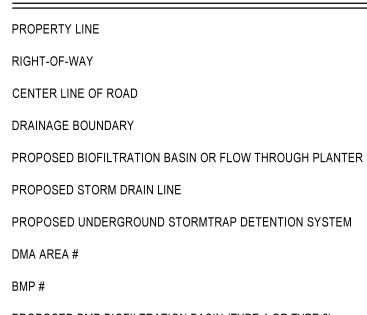
- □ Underlying hydrologic soil group
- □ Approximate depth to groundwater
- □ Existing natural hydrologic features (watercourses, seeps, springs, wetlands)
- □ Critical coarse sediment yield areas to be protected
- □ Existing topography
- □ Existing and proposed site drainage network and connections to drainage offsite
- □ Proposed grading
- □ Proposed impervious features
- □ Proposed design features and surface treatments used to minimize imperviousness
- □ Point(s) of Compliance (POC) for Hydromodification Management
- □ Existing and proposed drainage boundary and drainage area to each POC (when necessary, create separate exhibits for pre-development and post-project conditions)
- □ Structural BMPs for hydromodification management (identify location, type of BMP, and size/detail)

ATTACHMENT 2a



POST DEVELOPED HMP EXHIBIT FENWAY N. HWY 101

LEGEND



PROPOSED BMP BIOFILTRATION BASIN (TYPE 1 OR TYPE 2) PLANTER PER DETAILS HEREON.

AREA CALCULATIONS

TOTAL SITE AREA: AREA DISTURBED BY PROJECT: 165,107 SF (3.790 AC)

165,107 SF (3.790 AC)

EXISTING IMPERVIOUS AREA: 76,820 SF (1.764 AC) PROPOSED IMPERVIOUS AREA: 143,659 SF (3.298 AC) INCREASE IMPERVIOUS AREA: 66,839 SF (1.534 AC)

SOIL TYPE INFORMATION

SOIL: TYPE B HYDROLOGIC SOILS PER OBSERVED ONSITE / FIELD INFILTRATION RATES PROVIDED IN "GEOTECHNICAL INVESTIGATION LEUCADIA MIXED-USE " PREPARED BY NOVA SERVICES, INC. AND ONSITE SOIL CLASSIFICATION PROVIDED BY NOVA SERVICES, INC.

 \mathbf{X}

GROUNDWATER INFORMATION

GROUNDWATER NOT ENCOUNTERED IN ANY OF THE BORINGS CONDUCTED BY NOVA SERVICES, INC., ACCORDING TO SECTION 4.2.3 OF THE PRELIMINARY GEOTECHNICAL STUDY PREPARED BY NOVA SERVICES, INC. CONCLUSION IN REPORT STATES THAT GROUNDWATER "THUS OCCURS BELOW ABOUT EI + 10 MSL. AT LEAST 48 FEET BELOW THE FINISHED FLOOR OF THE LOWEST PARKING LEVEL (SET +58 MSL)."

TREATMENT CONTROL BMPS

BIOFILTRATION BF-1

POTENTIAL POLLUTANT SOURCE AREAS

TRASH DUMPESTERS, ROOFS, PARKING LOTS, LANDSCAPING, RESTAURANT(S), WATER FEATURES, AND OUTDOOR STORAGE AREAS.

SOURCE CONTROLS

SC-1: PREVENTION OF ILLICIT DISCHARGES INTO THE MS4 SC-2: STORM DRAIN STENCILING OR SIGNAGE

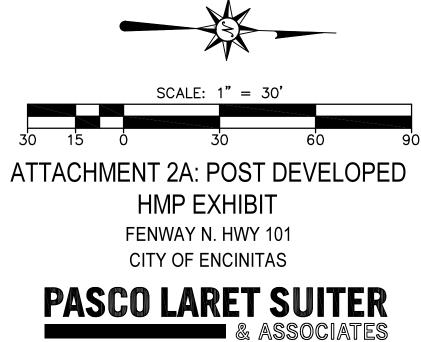
SC-5: PROTECT TRASH STORAGE AREAS FROM RAINFALL, RUN-ON, RUNOFF, AND WIND DISPERSAL SC-6: ADDITIONAL BMPS BASED ON POTENTIAL SOURCES ON RUNOFF POLLUTANTS

NOTE

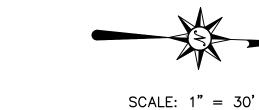
PROJECT SITE WILL MAINTAIN AND FOLLOW SAME WATER QUALITY DESIGN PROPOSED WITHIN THE CITY'S HIGHWAY 101 STREETSCAPE PROJECT FOR RIGHT-OF-WAY IN FRONT OF PROJECT SITE.

DMA AREAS								
OTAL (SF)	DTAL (SF) PERVIOUS (SF) % PERVIOUS % IMPERVIOUS MIN. BASIN AREA (SF) BASIN PROVIDED (SF) TREATMENT							
37,552	8,162	21.7	78.3	867	870	1		
8,442	2,265	26.8	73.2	187	196	2		
41,385	7,345	17.7	82.3	981	983	3		
17,211	1,889	11.0	89.0	429	434	4		
19,748	2,257	11.4	88.6	492	500	5		
5,189	1,043	20.1	79.9	121	128	6		
3,207	0	0	100	87	93	7		
4,328	362	8.4	91.6	110	132	8		
11,830	3,798	32.1	67.9	252	301	9		
14,091	4,344	30.8	69.2	304	305	10		
451	243	53.9	46.1	_	-	SELF MITIGATING		
311	179	57.6	42.4	_	-	SELF MITIGATING		
475	475	100.0	0	_	_	SELF MITIGATING		

ONCRETE BOTTOM FOOTING BY OTHER



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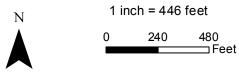


ATTACHMENT 2b

City of Encinitas Web Map - PCCSY



March 27, 2020



Every reasonable effort has been made to assure the accuracy of the data provided; nevertheless, some information may not be accurate. The City of Encinitas assumes no liability or responsibility arising from the use of or reliance upon this information.

ATTACHMENT 2d

BMP Sizing Spreadsheet V3.0					
Project Name:	101 Leucadia				
Project Applicant:	Fenway Capital Advisors				
Jurisdiction:	City of Encinitas				
Parcel (APN):	216-041-06, 20 and 21				
Hydrologic Unit:	904.51				
Rain Gauge:	Oceanside				
Total Project Area (sf):	166,988				
Channel Susceptibility:	High				

BMP Sizing Spreadsheet V3.0

	BMP Sizing Spreadsheet V3.0						
Project Name:	101 Leucadia	Hydrologic Unit:	904.51				
Project Applicant:	Fenway Capital Advisors	Rain Gauge:	Oceanside				
Jurisdiction:	City of Encinitas	Total Project Area:	166,988				
Parcel (APN):	216-041-06, 20 and 21	Low Flow Threshold:	0.1Q2				
BMP Name:	Vault	BMP Type:	Cistern				
BMP Native Soil Type:	В	BMP Infiltration Rate (in/hr):	NA				

			BMP Sizin	g Spreadsheet V3.0				
Project Name:	101 L	eucadia	Hydrologic Unit:		904.	51		
Project Applicant:	Fenway Ca	pital Advisors	Rain Gauge:	Oceanside				
urisdiction:	City of	Encinitas	Total Project Area:	166,988				
Parcel (APN):	216-041-0	6, 20 and 21	Low Flow Threshold:		0.10	22		
3MP Name:	V	ault	BMP Type:	Cistern				
3MP Native Soil Type:		В	BMP Infiltration Rate (in/hr):		NA	A		
			Areas Draining to BMP			HMP Sizing Factors	Minimum BMP Size	
					Area Weighted Runoff	č		
DMA		Pre Project Soil		Post Project	Factor	Volume	Volume (CF)	
Name	Area (sf)	Туре	Pre-Project Slope	Surface Type	(Table G.2-1) ¹	Volume	volume (er)	
1a-10a	133,865	В	Steep	Concrete	1.00	0.16	21418	
1b-10b	33,123	В	Steep	Landscape	0.1	0.16	530	
						0	0	
						0	0	
						0	0	
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BMP Tributary Area	166,988					Minimum BMP Size	21948	
		_				Proposed BMP Size*	23600	
				Standard Cistern	Depth (Overflow Elevation)	3.5	ft	
					Depth (Overflow Elevation)		ft	
					Required Cistern Footprint)			

Standard Cistern Depth (Overflow Elevation)	3.5	ft
Provided Cistern Depth (Overflow Elevation)	7.5	ft
Minimum Required Cistern Footprint)	6271	CF

1. Runoff factors which are used for hydromodification management flow control (Table G.2-1) are different from the runoff factors used for pollutant control BMP sizing (Table B.1-1). Table references are taken from the San Diego Region Model BMP Design Manu

Describe the BMP's in sufficient detail in your PDP SWQMP to demonstrate the area, volume, and other criteria can be met within the constraints of the site.

BMP's must be adapted and applied to the conditions specific to the development project such as unstable slopes or the lack of available head. Designated Staff have final review and approval authority over the project design.

This BMP Sizing Spreadsheet has been updated in conformance with the San Diego Region Model BMP Design Manual, April 2018. For questions or concerns please contact the jurisdiction in which your project is located.

	BMP Sizing Spreadsheet V3.0						
Project Name:	101 Leucadia	Hydrologic Unit:	904.51				
Project Applicant:	Fenway Capital Advisors	Rain Gauge:	Oceanside				
Jurisdiction:	City of Encinitas	Total Project Area:	166,988				
Parcel (APN):	216-041-06, 20 and 21	Low Flow Threshold:	0.1Q2				
BMP Name	Vault	BMP Type:	Cistern				

DMA Name	Rain Gauge	Pre-deve Soil Type	loped Condition Slope	Unit Runoff Ratio (cfs/ac)	DMA Area (ac)	Orifice Flow - %Q ₂ (cfs)	Orifice Area (in ²)
1a-10a	Oceanside	В	Steep	0.395	3.073	0.121	1.79
1b-10b	Oceanside	В	Steep	0.395	0.760	0.030	0.44

3.50	0.151	2.23	1.69
Max Orifice Head	Max Tot. Allowable Orifice Flow	Max Tot. Allowable Orifice Area	Max Orifice Diameter
(feet)	(cfs)	(in ²)	(in)

Provide Hand Calc.	0.112	1.65	1.450
Average outflow during	Max Orifice Outflow	Actual Orifice Area	Selected
surface drawdown	Max Office Outflow	Actual Office Area	Orifice Diameter
(cfs)	(cfs)	(in ²)	(in)

Drawdown (Hrs)	Provide Hand
Drawdown (Hrs)	Calculation

Vault Drawdown Calculation

Vault Drawdown	89.0	hrs	
Project No	3161 Date 7/16/202		7/16/2021
Project Name	101 Leucadia		

Note: Drawdown time is calculated assuming an initial water surface depth equal

to the invert of the lowest surface discharge opening in the vault outlet structure.

Underdrain Orifice Diameter:	1.45	in		
C:	0.6			
Surface Depth (ft)	Volume (cf)	Qorifice (cfs)	ΔT (hr)	Total Time (hr)
7.5	23601	0.150	0.000	0.0
7	22027	0.145	2.960	3.0
6	18880	0.134	6.258	9.2
5	15734	0.122	6.811	16.0
4	12587	0.109	7.549	23.6
3	9440	0.094	8.593	32.2
2	6293	0.076	10.245	42.4
1	3147	0.053	13.523	55.9
0	0	0.000	33.089	89.0

8'-0" STORMTRAP STAGE STORAGE BREAKDOWN		
Elevation	Storage (cubic-feet)	
44.31	0	
44.56	786.68	
44.81	1,573.37	
45.06	2,360.05	
45.31	3,146.74	
45.56	3,933.42	
45.81	4,720.11	
46.06	5,506.79	
46.31	6,293.48	
46.56	7,080.16	
46.81	7,866.85	
47.06	8,653.53	
47.31	9,440.22	
47.56	10,226.90	
47.81	11,013.59	
48.06	11,800.27	
48.31	12,586.96	
48.56	13,373.64	
48.81	14,160.33	
49.06	14,947.01	
49.31	15,733.70	
49.56	16,520.38	
49.81	17,307.07	
49.99	17,866.49	
50.06	18,093.75	
50.31	18,880.44	
50.56	19,667.12	
50.81	20,453.81	
51.06	21,240.49	
51.31	22,027.18	
51.56	22,813.86	
51.81	23,600.55	
52.06	24,387.22	
52.31	25,173.92	





MODULAR CONCRETE **STORMWATER MANAGEMENT**

PAGE	
0.0	COVER
1.0	SINGLET
2.0	SINGLET
2.1	SINGLET
3.0	SINGLET
3.1	SINGLET
4.0	SINGLET
5.0	RECOMME
6.0	SINGLET

STORMTRAP SU CONTACT CELL SALES

ENCINITAS BEACH HOTEL LEUCADIA, CA

SHEET INDEX

DESCRIPTION

- SHEET TRAP DESIGN CRITERIA TRAP SYSTEM LAYOUT TRAP FOUNDATION LAYOUT TRAP INSTALLATION SPECIFICATIONS
- TRAP INSTALLATION SPECIFICATIONS TRAP BACKFILL SPECIFICATIONS
- ENDED PIPE/ACCESS OPENING SPECIFICATIONS
- TRAP MODULE TYPES

STORMTRAP CONTACT INFORMATION



PATENTS LISTED AT: [HTTP://STORMTRAP.COM/PATENT]

1287 WINDHAM PARKWAY ROMEOVILLE, IL 60446 P:815-941-4549 / F:331-318-5347

ENGINEER INFORMATION:

PASCO LARET SUITER & ASSOCIATES 535 NORTH COAST HIGHWAY SOLANA BEACH, CA 858-259-8212

PROJECT INFORMATION:

ENCINITAS BEACH HOTEL

LEUCADIA, CA

CURRENT ISSUE DATE:

7/19/2021

ISSUED FOR:

PRELIMINARY

REV.	DATE:	ISSUED FOR:	DWN BY:
\triangle	7/9/2021	PRELIMINARY	RJL

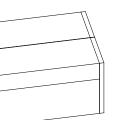
SCALE:

NTS

SHEET TITLE:

COVER SHEET

SHEET NUMBER:



9

STRUCTURAL DESIGN LOADING CRITERIA

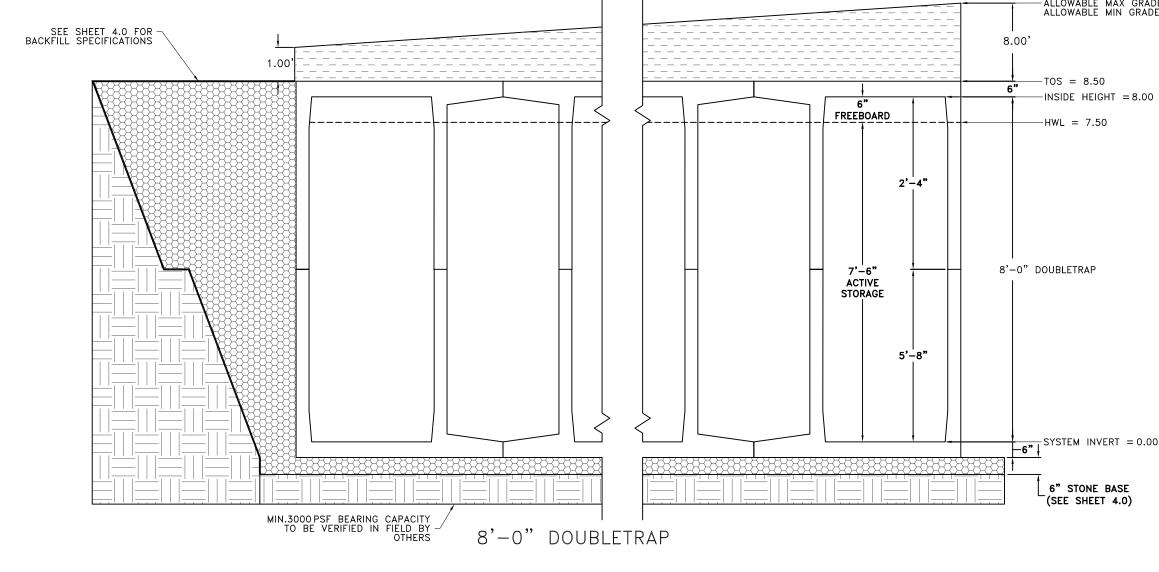
LIVE LOADING: AASHTO HS-20 HIGHWAY LOADING

GROUND WATER TABLE: BELOW INVERT OF SYSTEM SOIL BEARING PRESSURE: 3000PSF SOIL DENSITY: 120 PCF EQUIVALENT UNSATURATED LATERAL ACTIVE EARTH PRESSURE: 35 PSF / FT. EQUIVALENT SATURATED LATERAL ACTIVE EARTH PRESSURE: 80 PSF/FT. (IF WATER TABLE PRESENT) APPLICABLE CODES: ASTM C857 ACI-318 BACKFILL TYPE: SEE SHEET 4.0 FOR BACKFILL OPTIONS

STORMTRAP SYSTEM INFORMATION TOTAL WATER STORAGE PROV: 25,033.16 CUBIC FEET WATER STORAGE PROV: 23,468.58 CUBIC FEET ELEV 0.00 - 7.50 UNIT HEADROOM: 8'-0" DOUBLETRAP UNIT QUANTITY: 84 TOTAL PIECES

SITE SPECIFIC DESIGN CRITERIA

- 1. STORMTRAP UNITS SHALL BE MANUFACTURED AND INSTALLED ACCORDING TO SHOP DRAWINGS APPROVED BY THE INSTALLING CONTRACTOR AND ENGINEER OF RECORD. THE SHOP DRAWINGS SHALL INDICATE SIZE AND LOCATION OF ROOF OPENINGS AND INLET/OUTLET PIPE TYPES, SIZES, INVERT ELEVATIONS AND SIZE OF OPENINGS.
- 2. COVER RANGE: MIN. 1.00' MAX. 8.00' CONSULT STORMTRAP FOR ADDITIONAL COVER OPTIONS.
- 3. ALL DIMENSIONS AND SOIL CONDITIONS, INCLUDING BUT NOT LIMITED TO GROUNDWATER AND SOIL BEARING CAPACITY ARE REQUIRED TO BE VERIFIED IN THE FIELD BY OTHERS PRIOR TO STORMTRAP INSTALLATION.
- 4. FOR STRUCTURAL CALCULATIONS THE GROUND WATER TABLE IS ASSUMED TO BE BELOW INVERT OF SYSTEM IF WATER TABLE IS DIFFERENT THAN ASSUMED, CONTACT STORMTRAP.
- 5. SYSTEM DESIGN MAY ALLOW FOR INCIDENTAL LEAKAGE AND WILL NOT BE SUBJECT TO LEAKAGE TESTING.



ALLOWABLE MAX GRADE = 16.50 ALLOWABLE MIN GRADE = 9.50



PATENTS LISTED AT: [HTTP://STORMTRAP.COM/PATENT]

1287 WINDHAM PARKWAY ROMEOVILLE, IL 60446 P:815-941-4549 / F:331-318-5347

ENGINEER INFORMATION:

PASCO LARET SUITER & ASSOCIATES 535 NORTH COAST HIGHWAY SOLANA BEACH, CA 858-259-8212

PROJECT INFORMATION:

ENCINITAS BEACH HOTEL

LEUCADIA, CA

CURRENT ISSUE DATE:

7/19/2021

ISSUED FOR:

PRELIMINARY

REV.	DATE:	ISSUED FOR:	DWN BY:
	7/9/2021	PRELIMINARY	RJL

SCALE:

NTS

SHEET TITLE:

DOUBLETRAP DESIGN CRITERIA

SHEET NUMBER:

BILL OF MATERIALS				
QTY.	UNIT TYPE	DESCRIPTION	TOP WEIGHT	BASE WEIGHT
0	I	8'-0" DOUBLETRAP	0	0
2	П	8'-0" DOUBLETRAP	14286	18560
0	Ш	8'-0" DOUBLETRAP	0	0
14	IV	8'-0" DOUBLETRAP	12249	17584
0	VII	8'-0" DOUBLETRAP	0	0
11	VII-2	8'-0" DOUBLETRAP	9822	14096
9	VII-4	8'-0" DOUBLETRAP	10017	15352
5	SPIV	8'-0" DOUBLETRAP	VARIES	VARIES
1	SPVII-4	8'-0" DOUBLETRAP	VARIES	VARIES
0	T2 PANEL	8" THICK PANEL	()
9	T4 PANEL	8" THICK PANEL	59	63
5	T7 PANEL	8" THICK PANEL	43	48
6	JOINT WRAP	150' PER ROLL		
80	JOINT TAPE	14.5' PER ROLL		

LOADING DISCLAIMER:

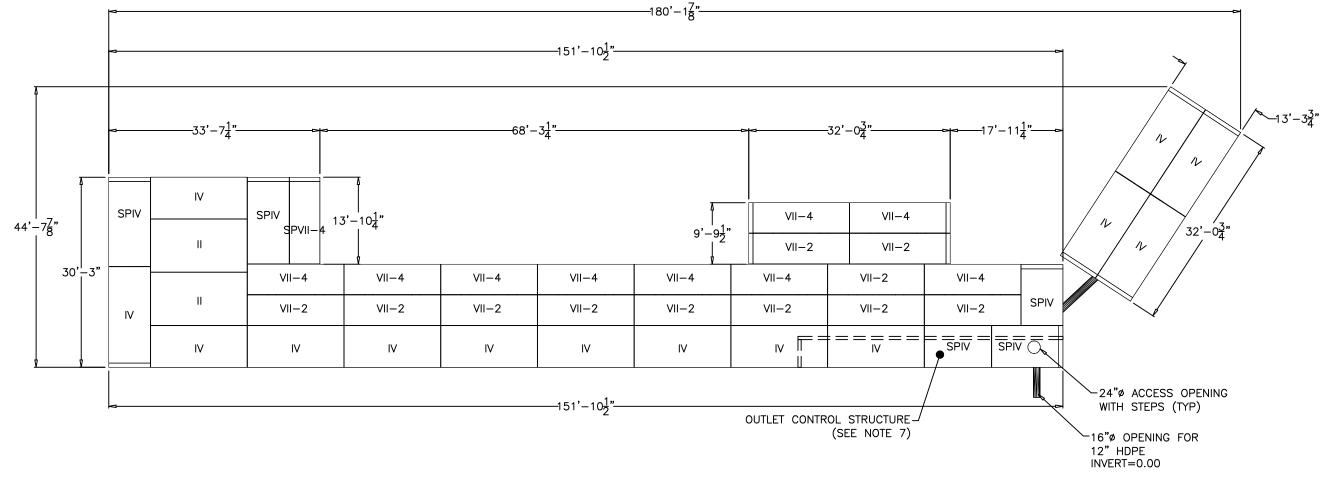
STORMTRAP IS NOT DESIGNED TO ACCEPT ANY ADDITIONAL LOADINGS FROM NEARBY STRUCTURES NEXT TO OR OVER THE TOP OF STORMTRAP. IF ADDITIONAL LOADING CONSIDERATIONS ARE REQUIRED FOR STRUCTURAL DESIGN OF STORMTRAP, PLEASE CONTACT STORMTRAP IMMEDIATELY.

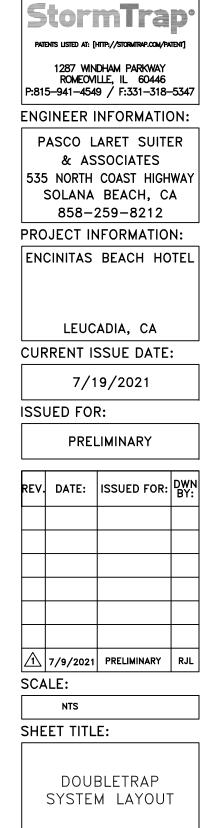
THE STORMTRAP SYSTEM HAS NOT BEEN DESIGNED TO SUPPORT THE ADDITIONAL WEIGHT OF ANY TREES. FURTHERMORE, THE ROOTS OF THE TREES MUST BE CONTAINED TO PREVENT FUTURE DAMAGE TO THE STORMTRAP SYSTEM. STORMTRAP ACCEPTS NO LIABILITY FOR DAMAGES CAUSED BY TREES OR OTHER VEGETATION PLACE AROUND OR ON TOP OF THE SYSTEM.

DESIGN CRITERIA ALLOWABLE MAX GRADE = 16.50 ALLOWABLE MIN GRADE = 9.50 INSIDE HEIGHT ELEVATION = 8.00 SYSTEM INVERT = 0.00

NOTES:

- 1. DIMENSIONING OF STORMTRAP SYSTEM SHOWN BELOW ALLOW FOR A 3/4" GAP BETWEEN EACH MODULE.
- 2. ALL DIMENSIONS TO BE VERIFIED IN THE FIELD BY OTHERS.
- 3. SEE SHEET 3.0 FOR INSTALLATION SPECIFICATIONS.
- 4. SP INDICATES A MODULE WITH MODIFICATIONS.
- 5. P INDICATES A MODULE WITH A PANEL ATTACHMENT.
- CONTRACTORS RESPONSIBILITY TO ENSURE CONSISTENCY/ACCURACY TO 6. FINAL ENGINEER OF RECORD PLAN SET.
- 7. IF A WATERTIGHT SOLUTION IS REQUIRED FOR THIS OUTLET CONTROL STRUCTURE, ALL EXTERIOR COLD JOINTS, INCLUDING JOINT BETWEEN TOP AND BASE MODULES, BETWEEN TOP AND BASE OF ADJOINING SYMONS WALLS, AND JOINTS BETWEEN MODULE AND ADJACENT END PANELS WILL BE THE SOLE RESPONSIBILITY OF THE INSTALLING CONTRACTOR TO PROVIDE AND INSTALL THE WATERTIGHT APPLICATION PER THE EOR'S SPECIFICATION.

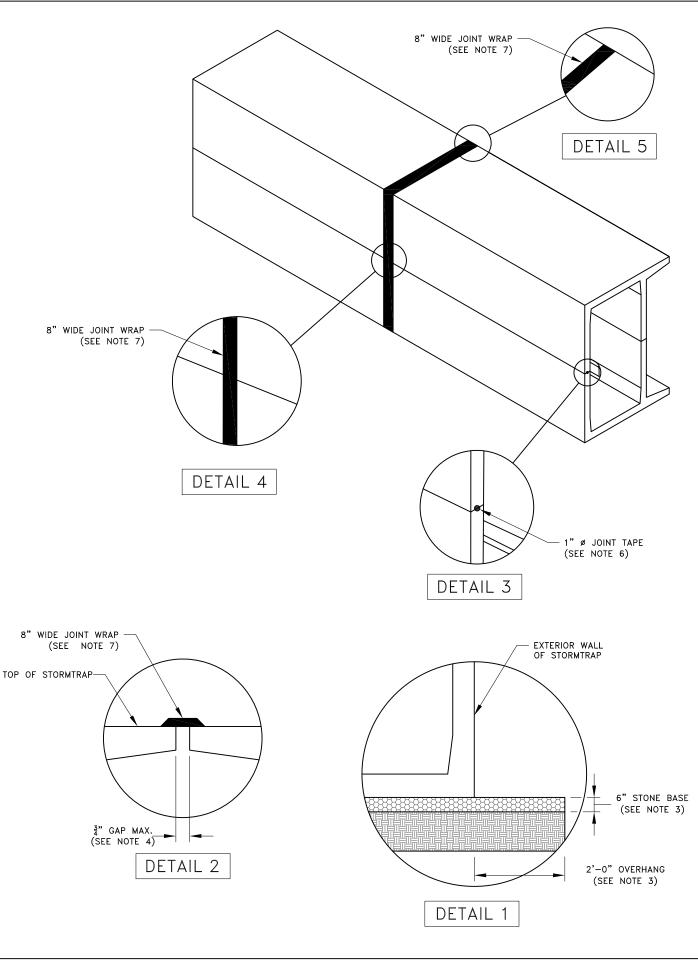


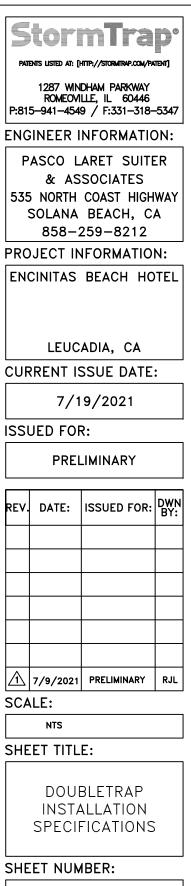


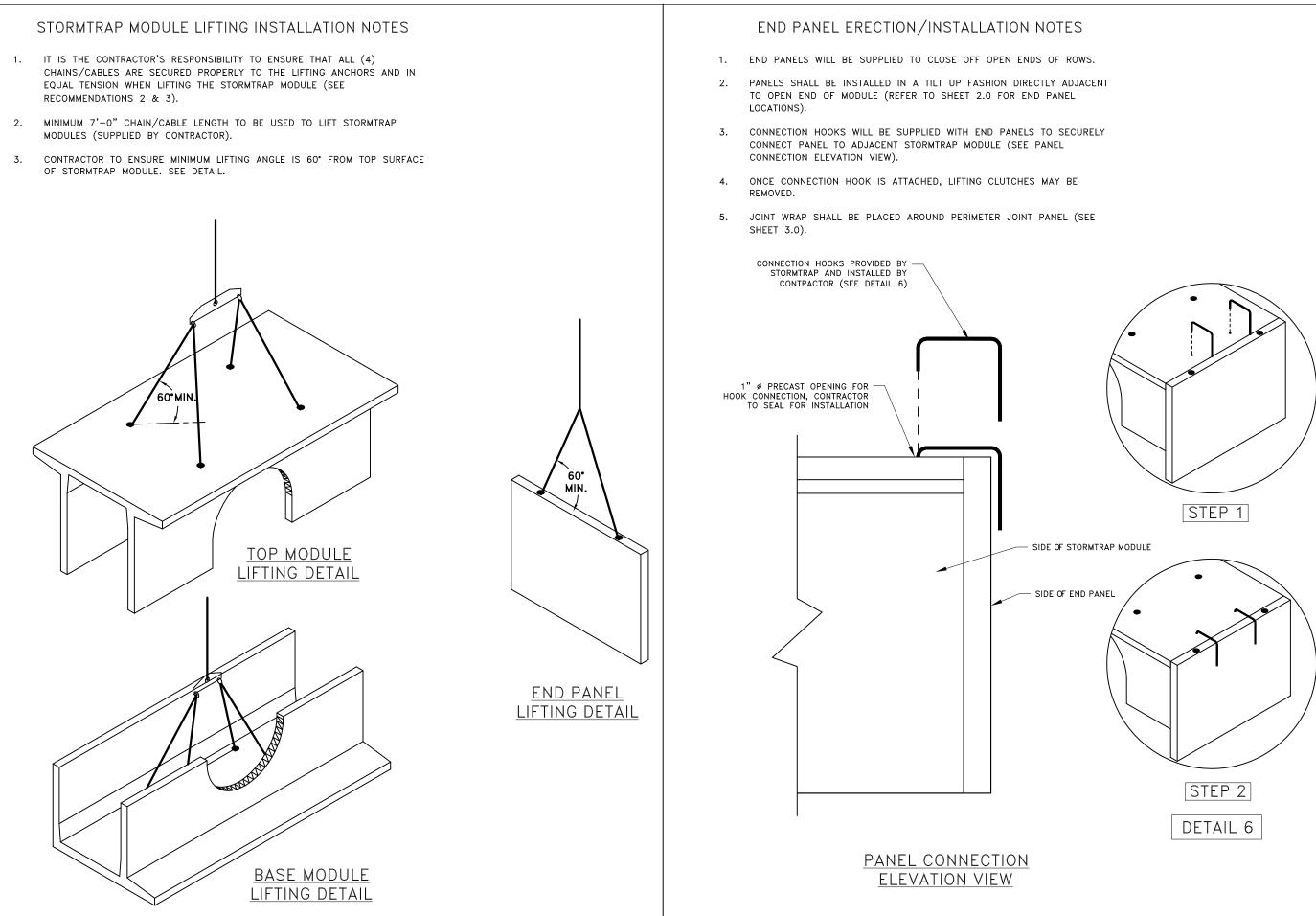
SHEET NUMBER:

STORMTRAP INSTALLATION SPECIFICATIONS

- 1. STORMTRAP SHALL BE INSTALLED IN ACCORDANCE WITH ASTM C891, STANDARD FOR INSTALLATION OF UNDERGROUND PRECAST CONCRETE UTILITY STRUCTURES, THE FOLLOWING ADDITIONS AND/OR EXCEPTIONS SHALL APPLY:
- 2. IT IS THE RESPONSIBILITY OF THE INSTALLING CONTRACTOR TO ENSURE THAT PROPER/ADEQUATE EQUIPMENT IS USED TO SET/INSTALL THE MODULES.
- 3. STORMTRAP MODULES CAN BE PLACED ON A LEVEL, 6" FOUNDATION OF $\frac{3}{4}$ " AGGREGATE EXTENDING 2'-0" PAST THE OUTSIDE OF THE SYSTEM (SEE DETAIL 1) AND SHALL BE PLACED ON PROPERLY COMPACTED SOILS (SEE SHEET 1.0 FOR SOIL BEARING CAPACITY REQUIREMENTS), AND IN ACCORDANCE WITH ASTM C891 STANDARD PRACTICE FOR INSTALLATION OF UNDERGROUND PRECAST UTILITY STRUCTURES.
- 4. THE STORMTRAP MODULES SHALL BE PLACED SUCH THAT THE MAXIMUM SPACE BETWEEN ADJACENT MODULES DOES NOT EXCEED $\frac{3}{4}$ " (SEE DETAIL 2). IF THE SPACE EXCEEDS $\frac{3}{4}$ ", THE MODULES SHALL BE RESET WITH APPROPRIATE ADJUSTMENT MADE TO LINE AND GRADE TO BRING THE SPACE INTO SPECIFICATION.
- 5. STORMTRAP MODULES ARE NOT WATERTIGHT. IF A WATERTIGHT SOLUTION IS REQUIRED, CONTACT STORMTRAP FOR RECOMMENDATIONS. THE WATERTIGHT APPLICATION IS TO BE PROVIDED AND IMPLEMENTED BY THE CONTRACTOR. THE CONTRACTOR IS RESPONSIBLE TO ENSURE THAT THE SELECTED WATERTIGHT SOLUTION PERFORMS AS SPECIFIED BY THE MANUFACTURER.
- 6. THE PERIMETER HORIZONTAL JOINT BETWEEN THE TOP AND BASE LEG CONNECTION OF THE STORMTRAP MODULES SHALL BE SEALED WITH PREFORMED MASTIC JOINT TAPE ACCORDING TO ASTM C891, 8.8 AND 8.12. (SEE DETAIL 3). THE MASTIC JOINT TAPE DOES NOT PROVIDE A WATERTIGHT SEAL.
- 7. ALL EXTERIOR JOINTS BETWEEN ADJACENT STORMTRAP MODULES SHALL BE SEALED WITH 8" WIDE PRE-FORMED, COLD-APPLIED, SELF-ADHERING ELASTOMERIC RESIN, BONDED TO A WOVEN, HIGHLY PUNCTURE RESISTANT POLYMER WRAP, CONFORMING TO ASTM C891 AND SHALL BE INTEGRATED WITH PRIMER SEALANT AS APPROVED BY STORMTRAP (SEE DETAILS 4 & 5). THE JOINT WRAP DOES NOT PROVIDE A WATERTIGHT SEAL. THE SOLE PURPOSE OF THE JOINT WRAP IS TO PROVIDE A SILT AND SOIL TIGHT SYSTEM. THE ADHESIVE EXTERIOR JOINT WRAP SHALL BE INSTALLED ACCORDING TO THE FOLLOWING INSTALLATION INSTRUCTIONS:
- 7.1. USE A BRUSH OR WET CLOTH TO THOROUGHLY CLEAN THE OUTSIDE SURFACE AT THE POINT WHERE JOINT WRAP IS TO BE APPLIED.
- 7.2. A RELEASE PAPER PROTECTS THE ADHESIVE SIDE OF THE JOINT WRAP. PLACE THE ADHESIVE TAPE (ADHESIVE SIDE DOWN) AROUND THE STRUCTURE, REMOVING THE RELEASE PAPER AS YOU GO. PRESS THE JOINT WRAP FIRMLY AGAINST THE STORMTRAP MODULE SURFACE WHEN APPLYING.
- 8. IF THE CONTRACTOR NEEDS TO CANCEL ANY SHIPMENTS, THEY MUST DO SO 48 HOURS PRIOR TO THEIR SCHEDULED ARRIVAL AT THE JOB SITE. IF CANCELED AFTER THAT TIME, PLEASE CONTACT THE PROJECT MANAGER.
- 9. IF THE STORMTRAP MODULE(S) IS DAMAGED IN ANY WAY PRIOR, DURING, OR AFTER INSTALL, STORMTRAP MUST BE CONTACTED IMMEDIATELY TO ASSESS THE DAMAGE AND TO DETERMINE WHETHER OR NOT THE MODULE(S) WILL NEED TO BE REPLACED. IF ANY MODULE ARRIVES AT THE JOBSITE DAMAGED DO NOT UNLOAD IT; CONTACT STORMTRAP IMMEDIATELY. ANY DAMAGE NOT REPORTED BEFORE THE TRUCK IS UNLOADED WILL BE THE CONTRACTOR'S RESPONSIBILITY.
- 10. STORMTRAP MODULES CANNOT BE ALTERED IN ANY WAY AFTER MANUFACTURING WITHOUT WRITTEN CONSENT FROM STORMTRAP.







ENGINEER INFORMATION: PASCO LARET SUITER & ASSOCIATES 535 NORTH COAST HIGHWAY SOLANA BEACH, CA 858-259-8212 **PROJECT INFORMATION:**

StormTrap[•]

PATENTS LISTED AT: [HTTP://STORMTRAP.COM/PATENT]

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ENCINITAS BEACH HOTEL

LEUCADIA, CA

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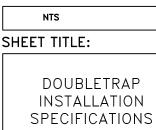
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	ZONE CHART			
ZONES	ZONE DESCRIPTIONS	REMARKS		
ZONE 1	FOUNDATION AGGREGATE	#5 $(\frac{3}{4}^{"})$ STONE AGGREGATE (SEE NOTE 4 FOR DESCRIPTION)		
ZONE 2	BACKFILL	UNIFIED SOILS CLASSIFICATION (GW, GP, SW, SP) OR SEE BELOW FOR APPROVED BACKFILL OPTIONS		
ZONE 3	FINAL COVER OVERTOP	MATERIALS NOT TO EXCEED 120 PCF		

FILL DEPTH	TRACK WIDTH	MAX VEHICLE WEIGHT (KIPS)	MAX GROUND PRESSURE
	12"	51.8	1690 psf
	18"	56.1	1219 psf
12"	24"	68.1	1111 psf
	30"	76.7	1000 psf
	36"	85.0	924 psf
NOTE			

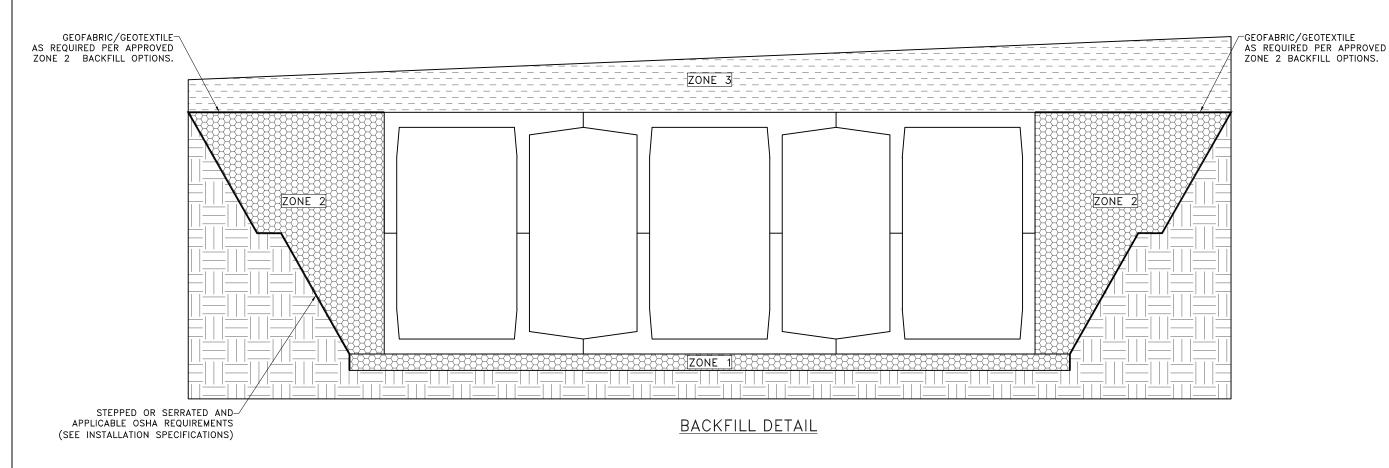
NOTE: TRACK LENGTH NOT TO EXCEED 15'-4". ONLY TWO TRACKS PER VEHICLE.

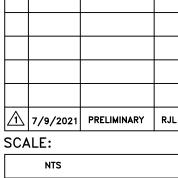
	APPROVED ZONE 2 BACKFILL OPTIONS
OPTION	REMARKS
³ ∰ STONE AGGREGATE	THE STONE AGGREGATE SHALL CONSIST OF CLEAN AND FREE DRAINING ANGULAR MATERIAL. THE SIZE OF THIS MATERIAL SHALL HAVE 100% PASSING THE 1" SIEVE WITH 0% TO 5% PASSING THE #8 SIEVE. THIS MATERIAL SHALL BE SEPARATED FROM NATIVE MATERIAL USING GEOFABRIC AROUND THE PERIMETER OF THE BACKFILL (ASTM SIZE #57) AS DETERMINED BY THE GEOTECHNICAL ENGINEER.
SAND	IMPORTED PURE SAND IS PERMITTED TO BE USED AS BACKFILL IF IT IS CLEAN AND FREE DRAINING. THE SAND USED FOR BACKFILLING SHALL HAVE LESS THAN 40% PASSING #40 SIEVE AND LESS THAN 5% PASSING #200 SIEVE. THIS MATERIAL SHALL BE SEPARATED FROM NATIVE MATERIAL USING GEOFABRIC AROUND THE PERIMETER OF THE SAND BACKFILL.
CRUSHED CONCRETE AGGREGATE	CLEAN, FREE DRAINING CRUSHED CONCRETE AGGREGATE MATERIAL CAN BE USED AS BACKFILL FOR STORMTRAP'S MODULES. THE SIZE OF THIS MATERIAL SHALL HAVE 100% PASSING THE 1" SIEVE WITH 0% TO 5% PASSING THE #8 SIEVE. THIS MATERIAL SHALL BE SEPARATED FROM NATIVE MATERIAL USING GEOFABRIC AROUND THE PERIMETER OF THE BACKFILL.
ROAD PACK	STONE AGGREGATE 100% PASSING THE $1-1/2$ " SIEVE WITH LESS THAN 12% PASSING THE #200 SIEVE (ASTM SIZE #467). GEOFABRIC AS PER GEOTECHNICAL ENGINEER

RECOMMENDATION.

STORMTRAP ZONE INSTALLATION SPECIFICATIONS/PROCEDURES

- 1. THE FILL PLACED AROUND THE STORMTRAP MODULES MUST DEPOSITED ON BOTH SIDES AT THE SAME TIME AND TO APPROXIMATELY THE SAME ELEVATION. AT NO TIME SHALL THE FILL BEHIND ONE SIDE WALL BE MORE THAN 2'-O" HIGHER THAN THE FILL ON THE OPPOSITE SIDE. BACKFILL SHALL EITHER BE COMPACTED AND/OR VIBRATED TO ENSURE THAT BACKFILL AGGREGATE/STONE MATERIAL IS WELL SEATED AND PROPERLY INTER LOCKED. CARE SHALL BE TAKEN TO PREVENT ANY WEDGING ACTION AGAINST THE STRUCTURE, AND ALL SLOPES WITHIN THE AREA TO BE BACKFILLED MUST BE STEPPED OR SERRATED TO PREVENT WEDGING ACTION. CARE SHALL ALSO BE TAKEN AS NOT TO DISRUPT THE JOINT WRAP FROM THE JOINT DURING THE BACKFILL PROCESS. BACKFILL MUST BE FREE-DRAINING MATERIAL. SEE ZONE 2 BACKFILL CHART ON THIS PAGE FOR APPROVED BACKFILL OPTIONS. IF NATIVE EARTH IS SUSCEPTIBLE TO MIGRATION, CONFIRM WITH GEOTECHNICAL ENGINEER AND PROVIDE PROTECTION AS REQUIRED (PROVIDED BY OTHERS).
- DURING PLACEMENT OF MATERIAL OVERTOP THE SYSTEM, AT NO TIME SHALL MACHINERY BE USED 2. OVERTOP THAT EXCEEDS THE DESIGN LIMITATIONS OF THE SYSTEM. WHEN PLACEMENT OF MATERIAL OVERTOP, MATERIAL SHALL BE PLACED SUCH THAT THE DIRECTION OF PLACEMENT IS PARALLEL WITH THE OVERALL LONGITUDINAL DIRECTION OF THE SYSTEM WHENEVER POSSIBLE.
- 3. THE FILL PLACED OVERTOP THE SYSTEM SHALL BE PLACED AT A MINIMUM OF 6" LIFTS. AT NO TIME SHALL MACHINERY OR VEHICLES GREATER THAN THE DESIGN HS-20 LOADING CRITERIA TRAVEL OVERTOP THE SYSTEM WITHOUT THE MINIMUM DESIGN COVERAGE. IF TRAVEL IS NECESSARY OVERTOP THE SYSTEM PRIOR TO ACHIEVING THE MINIMUM DESIGN COVER, IT MAY BE NECESSARY TO REDUCE THE ULTIMATE LOAD/BURDEN OF THE OPERATING MACHINERY SO AS TO NOT EXCEED THE DESIGN CAPACITY OF THE SYSTEM. IN SOME CASES, IN ORDER TO ACHIEVE REQUIRED COMPACTION, HAND COMPACTION MAY BE NECESSARY IN ORDER NOT TO EXCEED THE ALLOTTED DESIGN LOADING. SEE CHART FOR TRACKED VEHICLE WIDTH AND ALLOWABLE MAXIMUM PRESSURE PER TRACK.
- STONE AGGREGATE FOUNDATION IN ZONE 1 IS RECOMMENDED FOR LEVELING PURPOSES ONLY 4. (OPTIONAL).





DOUBLETRAP BACKFILL SPECIFICATIONS

StormTrap[•]

PATENTS LISTED AT: [HTTP://STORMIRAP.COM/PATENT

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SOLANA BEACH, CA 858-259-8212

PROJECT INFORMATION:

ENCINITAS BEACH HOTEL

LEUCADIA, CA

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RECOMMENDED ACCESS OPENING SPECIFICATION

- A TYPICAL ACCESS OPENING FOR THE STORMTRAP SYSTEM ARE 2'-0" IN DIAMETER. ACCESS OPENINGS LARGER THAN 3'-0" IN DIAMETER NEED TO BE APPROVED BY STORMTRAP. ALL OPENINGS MUST RETAIN AT LEAST 1'-0" OF CLEARANCE FROM THE END OF THE STORMTRAP MODULE UNLESS NOTED OTHERWISE. ALL ACCESS OPENINGS TO BE LOCATED ON INSIDE LEG UNLESS OTHERWISE SPECIFIED.
- 2. PLASTIC COATED STEEL STEPS PRODUCED BY M.A. INDUSTRIES PART #PS3-PFC OR APPROVED EQUAL (SEE STEP DETAIL) ARE PROVIDED INSIDE ANY MODULE WHERE DEEMED NECESSARY. THE HIGHEST STEP IN THE MODULE IS TO BE PLACED A DISTANCE OF 1'-O" FROM THE INSIDE EDGE OF THE STORMTRAP MODULES. ALL ENSUING STEPS SHALL BE PLACED AT A DISTANCE BETWEEN 10" MIN AND 14" MAX BETWEEN THEM. STEPS MAY BE MOVED OR ALTERED TO AVOID OPENINGS OR OTHER IRREGULARITIES IN THE MODULE.
- 3. STORMTRAP LIFTING INSERTS MAY BE RELOCATED TO AVOID INTERFERENCE WITH ACCESS OPENINGS OR THE CENTER OF GRAVITY OF THE MODULE AS NEEDED.
- 4. STORMTRAP ACCESS OPENINGS MAY BE RELOCATED TO AVOID INTERFERENCE WITH INLET AND/OR OUTLET PIPE OPENINGS SO PLACEMENT OF STEPS IS ATTAINABLE.
- 5. ACCESS OPENINGS SHOULD BE LOCATED IN ORDER TO MEET THE APPROPRIATE MUNICIPAL REQUIREMENTS. STORMTRAP RECOMMENDS AT LEAST TWO ACCESS OPENINGS PER SYSTEM FOR ACCESS AND INSPECTION.
- USE PRECAST ADJUSTING RINGS AS NEEDED TO MEET GRADE. STORMTRAP RECOMMENDS FOR COVER OVER 2' TO USE PRECAST BARREL OR CONE SECTIONS. (PROVIDED BY OTHERS)

RECOMMENDED PIPE OPENING SPECIFICATION

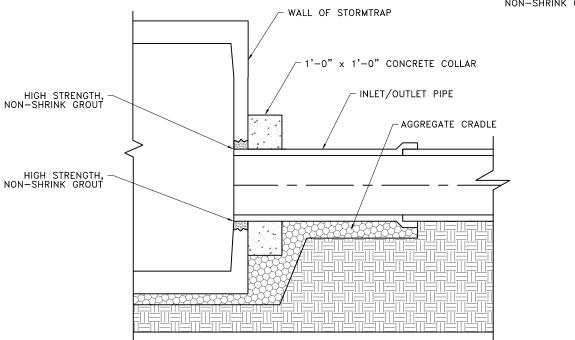
- 1. MINIMUM EDGE DISTANCE FOR AN OPENING ON THE OUTSIDE WALL SHALL BE NO LESS THAN 1'-0".
- 2. MAXIMUM OPENING SIZE TO BE DETERMINED BY THE MODULE HEIGHT. PREFERRED OPENING SIZE Ø 36" OR LESS. ANY OPENING NEEDED THAT DOES NOT FIT THIS CRITERIA SHALL BE BROUGHT TO THE ATTENTION OF STORMTRAP FOR REVIEW.
- 3. CONNECTING PIPES SHALL BE INSTALLED WITH A 1'-0" CONCRETE COLLAR, AND AN AGGREGATE CRADLE FOR AT LEAST ONE PIPE LENGTH (SEE PIPE CONNECTION DETAIL). A STRUCTURAL GRADE CONCRETE OR HIGH STRENGTH, NON-SHRINK GROUT WITH A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI SHALL BE USED.
- 4. THE ANNULAR SPACE BETWEEN THE PIPE AND THE HOLE SHALL BE FILLED WITH HIGH STRENGTH NON-SHRINK GROUT.

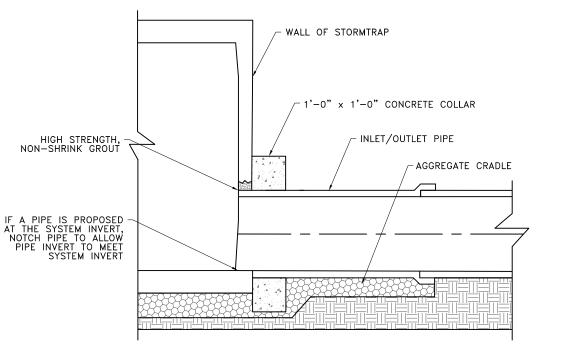
RECOMMENDED PIPE INSTALLATION INSTRUCTIONS

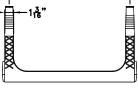
- 1. CLEAN AND LIGHTLY LUBRICATE ALL OF THE PIPE TO BE INSERTED INTO STORMTRAP.
- 2. IF PIPE IS CUT, CARE SHOULD BE TAKEN TO ALLOW NO SHARP EDGES. BEVEL AND LUBRICATE LEAD END OF PIPE.
- 3. ALIGN CENTER OF PIPE TO CORRECT ELEVATION AND INSERT INTO OPENING.

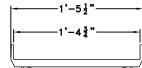
NOTE: ALL ANCILLARY PRODUCTS/SPECIFICATIONS RECOMMENDED AND SHOWN ON THIS SHEET ARE RECOMMENDATIONS ONLY AND SUBJECT TO CHANGE PER THE INSTALLING CONTRACTOR AND/OR PER LOCAL MUNICIPAL CODE/REQUIREMENTS.

PRECAST CONCRETE ADJUSTING RINGS, BARREL OR CONE SECTIONS AS NEEDED SEE RECOMMENDED ACCESS OPENING SPECIFICATION NOTE 6. (SUPPLIED BY OTHERS) NON-SHRINK GROUT

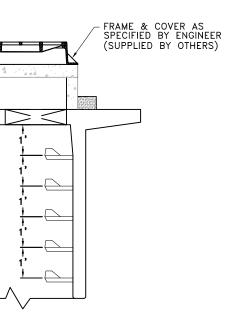








PIPE CONNECTION DETAIL

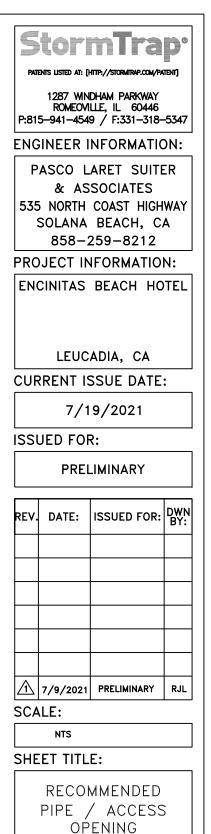


RISER / STAIR DETAIL

MEETS: OPSS 1351.08.02 BNQ ASTM C-478.95a ASTM D4-101.95b

AASHTO M-199 ASTM 4A-15

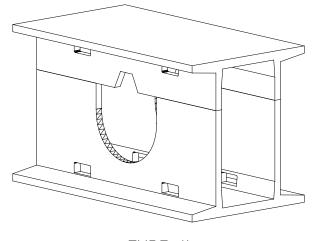
STEP DETAIL

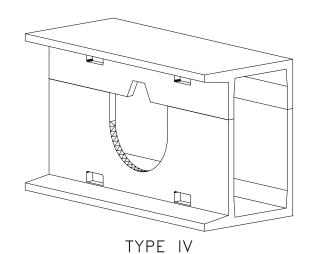


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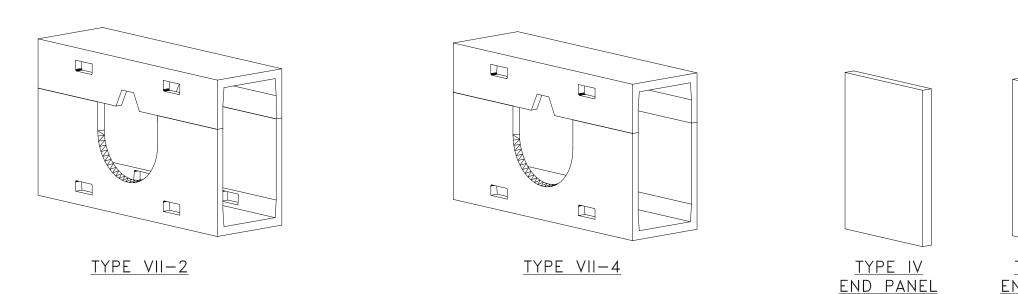
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SPECIFICATIONS



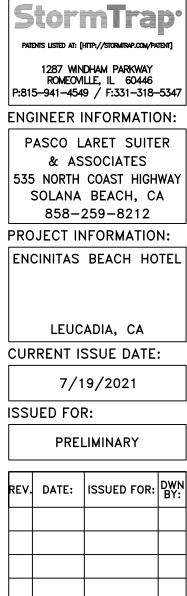


<u>TYPE II</u>



NOTES:

- 1. OPENING LOCATIONS AND SHAPES MAY VARY.
- 2. SP INDICATES A MODULE WITH MODIFICATIONS.
- 3. P INDICATES A MODULE WITH A PANEL ATTACHMENT.
- 4. POCKET WINDOW OPENINGS ARE OPTIONAL.



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<u> </u>	7/9/2021	PRELIMINARY	RJL

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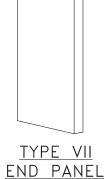
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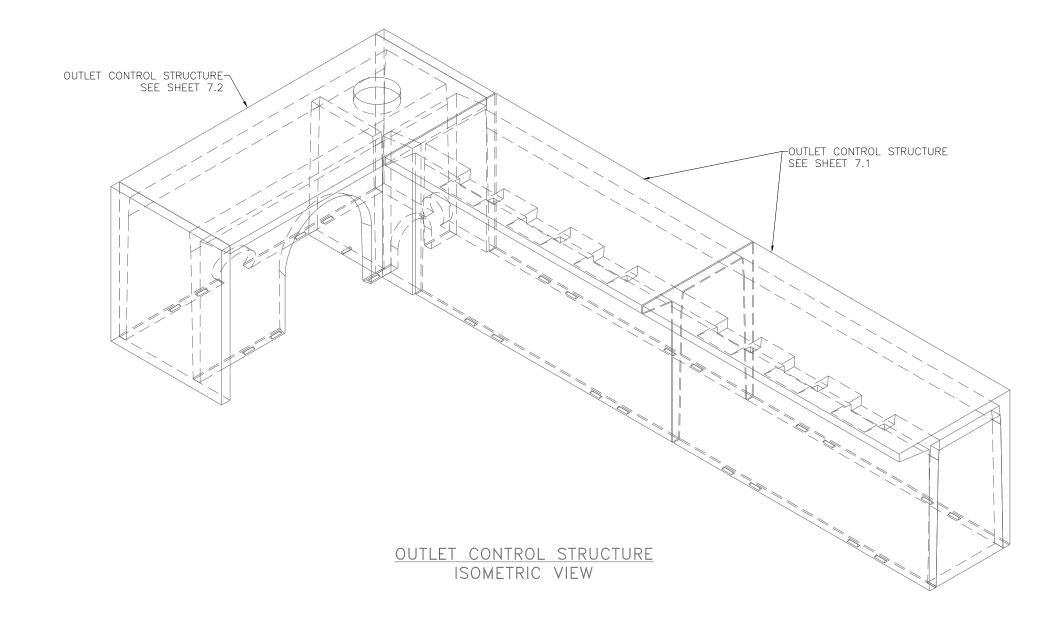
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DOUBLETRAP MODULE TYPES

SHEET NUMBER:

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OUTLET CONTROL

SHEET TITLE:

NTS

1 8/26/2020 PRELIMINARY SCALE:

3/9/2021 PRELIMINARY 2 12/11/2020 PRELIMINARY

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PRELIMINARY

ISSUED FOR: DWN BY:

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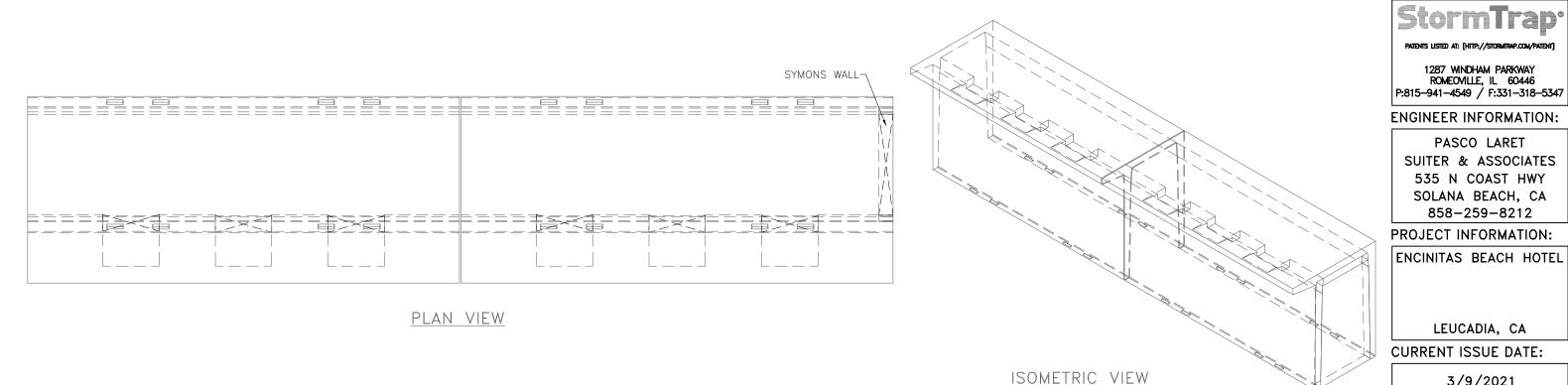
PROJECT INFORMATION: ENCINITAS BEACH HOTEL

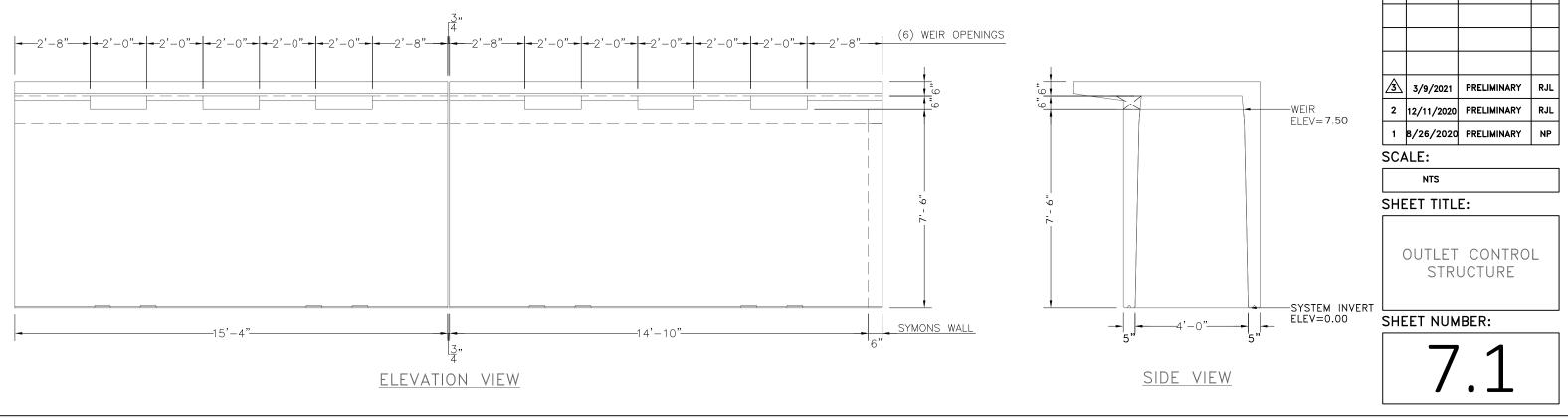
LEUCADIA, CA



ENGINEER INFORMATION:

PASCO LARET SUITER & ASSOCIATES 535 N COAST HWY SOLANA BEACH, CA 858-259-8212





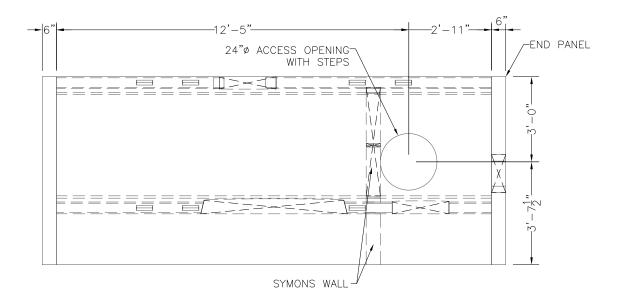
ENCINITAS BEACH HOTEL

3/9/2021

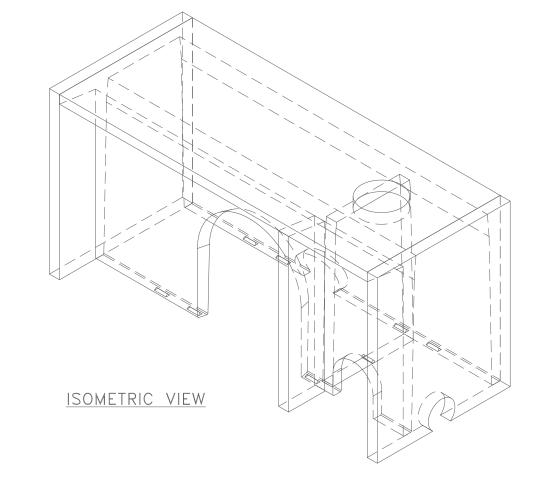
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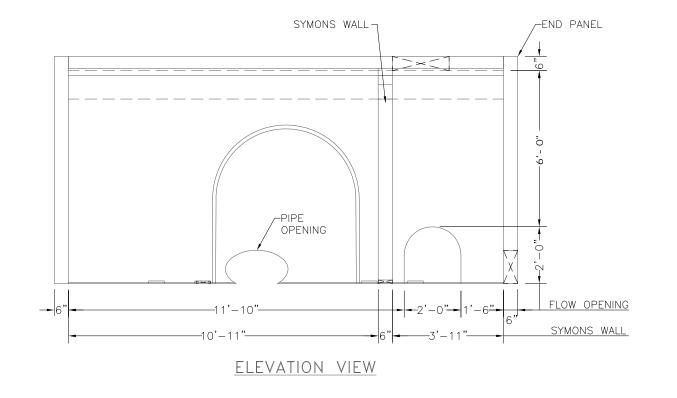
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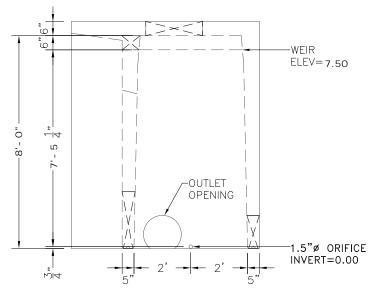
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2	12/11/2020	PRELIMINARY	RJL
1	8/26/2020	PRELIMINARY	NP











SIDE VIEW



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1	8/26/2020	PRELIMINARY	NP

SCALE:

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SHEET TITLE:

OUTLET CONTROL STRUCTURE

7.2

SHEET NUMBER:

ATTACHMENT 3 - STRUCTURAL BMP MAINTENANCE INFORMATION

This is the cover sheet for Attachment 3.

Indicate which items are included behind this cover sheet:

Attachment	Contents	Checklist
Attachment 3a	Structural BMP Maintenance Thresholds and Actions (Required)	☑ Included
		See Structural BMP Maintenance Information Checklist on the back of this Attachment cover sheet.
Attachment 3b	Draft Maintenance Agreement (when applicable)	□ Included □ Not Applicable

Use this checklist to ensure the required information has been included in the Structural BMP Maintenance Information Attachment:

□ Preliminary Design / Planning / CEQA level submittal:

Attachment 3a must identify:

Typical maintenance indicators and actions for proposed structural BMP(s) based on Section 7.7 of the BMP Design Manual

Attachment 3b is not required for preliminary design / planning / CEQA level submittal.

□ Final Design level submittal:

Attachment 3a must identify:

- Specific maintenance indicators and actions for proposed structural BMP(s). This shall be based on Section 7.7 of the BMP Design Manual and enhanced to reflect actual proposed components of the structural BMP(s)
- □ How to access the structural BMP(s) to inspect and perform maintenance
- Features that are provided to facilitate inspection (e.g., observation ports, cleanouts, silt posts, or other features that allow the inspector to view necessary components of the structural BMP and compare to maintenance thresholds)
- □ Manufacturer and part number for proprietary parts of structural BMP(s) when applicable
- □ Maintenance thresholds specific to the structural BMP(s), with a location-specific frame of reference (e.g., level of accumulated materials that triggers removal of the materials, to be identified based on viewing marks on silt posts or measured with a survey rod with respect to a fixed benchmark within the BMP)
- □ Recommended equipment to perform maintenance
- □ When applicable, necessary special training or certification requirements for inspection and maintenance personnel such as confined space entry or hazardous waste management

Attachment 3b: For private entity operation and maintenance, Attachment 3b shall include a draft maintenance agreement in the local jurisdiction's standard format (PDP applicant to contact the City Engineer to obtain the current maintenance agreement forms).

BMP MAINTENANCE FACT SHEET FOR STRUCTURAL BMP BF-1 BIOFILTRATION

Biofiltration facilities are vegetated surface water systems that filter water through vegetation, and soil or engineered media prior to discharge via underdrain or overflow to the downstream conveyance system. Biofiltration facilities have limited or no infiltration. They are typically designed to provide enough hydraulic head to move flows through the underdrain connection to the storm drain system. Typical biofiltration components include:

- Inflow distribution mechanisms (e.g., perimeter flow spreader or filter strips)
- Energy dissipation mechanism for concentrated inflows (e.g., splash blocks or riprap)
- Shallow surface ponding for captured flows
- Side slope and basin bottom vegetation selected based on climate and ponding depth
- Non-floating mulch layer
- Media layer (planting mix or engineered media) capable of supporting vegetation growth
- Filter course layer consisting of aggregate to prevent the migration of fines into uncompacted native soils or the aggregate storage layer
- Aggregate storage layer with underdrain(s)
- Impermeable liner or uncompacted native soils at the bottom of the facility
- Overflow structure

Normal Expected Maintenance

Biofiltration requires routine maintenance to: remove accumulated materials such as sediment, trash or debris; maintain vegetation health; maintain infiltration capacity of the media layer; replenish mulch; and maintain integrity of side slopes, inlets, energy dissipators, and outlets. A summary table of standard inspection and maintenance indicators is provided within this Fact Sheet.

Non-Standard Maintenance or BMP Failure

If any of the following scenarios are observed, the BMP is not performing as intended to protect downstream waterways from pollution and/or erosion. Corrective maintenance, increased inspection and maintenance, BMP replacement, or a different BMP type will be required.

- The BMP is not drained between storm events. Surface ponding longer than approximately 24 hours following a storm event may be detrimental to vegetation health, and surface ponding longer than approximately 96 hours following a storm event poses a risk of vector (mosquito) breeding. Poor drainage can result from clogging of the media layer, filter course, aggregate storage layer, underdrain, or outlet structure. The specific cause of the drainage issue must be determined and corrected.
- Sediment, trash, or debris accumulation greater than 25% of the surface ponding volume within one month. This means the load from the tributary drainage area is too high, reducing BMP function or clogging the BMP. This would require pretreatment measures within the tributary area draining to the BMP to intercept the materials. Pretreatment components, especially for sediment, will extend the life of components that are more expensive to replace such as media, filter course, and aggregate layers.
- Erosion due to concentrated storm water runoff flow that is not readily corrected by adding erosion control blankets, adding stone at flow entry points, or minor re-grading to restore proper drainage according to the original plan. If the issue is not corrected by restoring the BMP to the original plan and grade, the [City Engineer] shall be contacted prior to any additional repairs or reconstruction.

Other Special Considerations

Biofiltration is a vegetated structural BMP. Vegetated structural BMPs that are constructed in the vicinity of, or connected to, an existing jurisdictional water or wetland could inadvertently result in creation of expanded waters or wetlands. As such, vegetated structural BMPs have the potential to come under the jurisdiction of the United States Army Corps of Engineers, SDRWQCB, California Department of Fish and Wildlife, or the United States Fish and Wildlife Service. This could result in the need for specific resource agency permits and costly mitigation to perform maintenance of the structural BMP. Along with proper placement of a structural BMP, <u>routine maintenance is key to preventing this scenario</u>.

SUMMARY OF STANDARD INSPECTION AND MAINTENANCE FOR BF-1 BIOFILTRATION

The property owner is responsible to ensure inspection, operation and maintenance of permanent BMPs on their property unless responsibility has been formally transferred to an agency, community facilities district, homeowners association, property owners association, or other special district.

Maintenance frequencies listed in this table are average/typical frequencies. Actual maintenance needs are site-specific, and maintenance may be required more frequently. Maintenance must be performed whenever needed, based on maintenance indicators presented in this table. The BMP owner is responsible for conducting regular inspections to see when maintenance is needed based on the maintenance indicators. During the first year of operation of a structural BMP, inspection is recommended at least once prior to August 31 and then monthly from September through May. Inspection during a storm event is also recommended. After the initial period of frequent inspections, the minimum inspection and maintenance frequency can be determined based on the results of the first year inspections.

Threshold/Indicator	Maintenance Action	Typical Maintenance Frequency
Accumulation of sediment, litter, or debris	Remove and properly dispose of accumulated materials, without damage to the vegetation or compaction of the media layer.	 Inspect monthly. If the BMP is 25% full* or more in one month, increase inspection frequency to monthly plus after every 0.1-inch or larger storm event. Remove any accumulated materials found at each inspection.
Obstructed inlet or outlet structure	Clear blockage.	 Inspect monthly and after every 0.5-inch or larger storm event. Remove any accumulated materials found at each inspection.
Damage to structural components such as weirs, inlet or outlet structures	Repair or replace as applicable	Inspect annually.Maintenance when needed.
Poor vegetation establishment	Re-seed, re-plant, or re-establish vegetation per original plans.	Inspect monthly.Maintenance when needed.
Dead or diseased vegetation	Remove dead or diseased vegetation, re-seed, re-plant, or re-establish vegetation per original plans.	Inspect monthly.Maintenance when needed.
Overgrown vegetation	Mow or trim as appropriate.	Inspect monthly.Maintenance when needed.
2/3 of mulch has decomposed, or mulch has been removed	Remove decomposed fraction and top off with fresh mulch to a total depth of 3 inches.	 Inspect monthly. Replenish mulch annually, or more frequently when needed based on inspection.

*"25% full" is defined as ¼ of the depth from the design bottom elevation to the crest of the outflow structure (e.g., if the height to the outflow opening is 12 inches from the bottom elevation, then the materials must be removed when there is 3 inches of accumulation – this should be marked on the outflow structure).

SUMMARY OF STANDARD IN	SUMMARY OF STANDARD INSPECTION AND MAINTENANCE FOR BF-1 BIOFILTRATION (Continued from previous page)			
Threshold/Indicator	Maintenance Action	Typical Maintenance Frequency		
Erosion due to concentrated irrigation flow	Repair/re-seed/re-plant eroded areas and adjust the irrigation system.	Inspect monthly.Maintenance when needed.		
Erosion due to concentrated storm water runoff flow	Repair/re-seed/re-plant eroded areas, and make appropriate corrective measures such as adding erosion control blankets, adding stone at flow entry points, or minor re-grading to restore proper drainage according to the original plan. If the issue is not corrected by restoring the BMP to the original plan and grade, the [City Engineer] shall be contacted prior to any additional repairs or reconstruction.	 Inspect after every 0.5-inch or larger storm event. If erosion due to storm water flow has been observed, increase inspection frequency to after every 0.1-inch or larger storm event. Maintenance when needed. If the issue is not corrected by restoring the BMP to the original plan and grade, the [City Engineer] shall be contacted prior to any additional repairs or reconstruction. 		
Standing water in BMP for longer than 24 hours following a storm event Surface ponding longer than approximately 24 hours following a storm event may be detrimental to vegetation health	Make appropriate corrective measures such as adjusting irrigation system, removing obstructions of debris or invasive vegetation, clearing underdrains, or repairing/replacing clogged or compacted soils.	 Inspect monthly and after every 0.5-inch or larger storm event. If standing water is observed, increase inspection frequency to after every 0.1-inch or larger storm event. Maintenance when needed. 		
Presence of mosquitos/larvae For images of egg rafts, larva, pupa, and adult mosquitos, see <u>http://www.mosquito.org/biology</u>	If mosquitos/larvae are observed: first, immediately remove any standing water by dispersing to nearby landscaping; second, make corrective measures as applicable to restore BMP drainage to prevent standing water.	 Inspect monthly and after every 0.5-inch or larger storm event. If mosquitos are observed, increase inspection frequency to after every 0.1-inch or larger storm event. Maintenance when needed. 		
	If mosquitos persist following corrective measures to remove standing water, or if the BMP design does not meet the 96-hour drawdown criteria due to release rates controlled by an orifice installed on the underdrain, the [City Engineer] shall be contacted to determine a solution. A different BMP type, or a Vector Management Plan prepared with concurrence from the County of San Diego Department of Environmental Health, may be required.			
Underdrain clogged	Clear blockage.	 Inspect if standing water is observed for longer than 24-96 hours following a storm event. Maintenance when needed. 		

References

American Mosquito Control Association. <u>http://www.mosquito.org/</u> California Storm Water Quality Association (CASQA). 2003. Municipal BMP Handbook. <u>https://www.casqa.org/resources/bmp-handbooks/municipal-bmp-handbook</u> County of San Diego. 2014. Low Impact Development Handbook. <u>http://www.sandiegocounty.gov/content/sdc/dpw/watersheds/susmp/lid.html</u> San Diego County Copermittees. 2016. Model BMP Design Manual, Appendix E, Fact Sheet BF-1. <u>http://www.projectcleanwater.org/index.php?option=com_content&view=article&id=250&Itemid=220</u>

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Date:	Inspector: BMP ID No.:		BMP ID No.:
Permit No.:	APN(s):		
Property / Development Name:		Responsible Party Name and	l Phone Number:
Property Address of BMP:		Responsible Party Address:	

INSPECTION AND MAINTENANCE CHECKLIST FOR BF-1 BIOFILTRATION PAGE 1 of 5			
Threshold/Indicator	Maintenance Recommendation	Date	Description of Maintenance Conducted
Accumulation of sediment, litter, or debris Maintenance Needed? YES NO N/A	 Remove and properly dispose of accumulated materials, without damage to the vegetation If sediment, litter, or debris accumulation exceeds 25% of the surface ponding volume within one month (25% full*), add a forebay or other pre-treatment measures within the tributary area draining to the BMP to intercept the materials. Other / Comments: 		
Poor vegetation establishment Maintenance Needed? YES NO N/A	 Re-seed, re-plant, or re-establish vegetation per original plans Other / Comments: 		

*"25% full" is defined as ¼ of the depth from the design bottom elevation to the crest of the outflow structure (e.g., if the height to the outflow opening is 12 inches from the bottom elevation, then the materials must be removed when there is 3 inches of accumulation – this should be marked on the outflow structure).

Date:	Inspector:	BMP ID No.:
Permit No.:	APN(s):	

INSPECTION AND MAINTENANCE CHECKLIST FOR BF-1 BIOFILTRATION PAGE 2 of 5			
Threshold/Indicator	Maintenance Recommendation	Date	Description of Maintenance Conducted
Dead or diseased vegetation Maintenance Needed? YES NO N/A	 Remove dead or diseased vegetation, reseed, re-plant, or re-establish vegetation per original plans Other / Comments: 		
Overgrown vegetation	□ Mow or trim as appropriate		
Maintenance Needed?	Other / Comments:		
□ YES □ NO □ N/A			
 2/3 of mulch has decomposed, or mulch has been removed Maintenance Needed? YES NO N/A 	 Remove decomposed fraction and top off with fresh mulch to a total depth of 3 inches Other / Comments: 		

Date:	Inspector:	BMP ID No.:
Permit No.:	APN(s):	

INSPECTION AND MAINTENANCE CHECKLIST FOR BF-1 BIOFILTRATION PAGE 3 of 5			
Threshold/Indicator	Maintenance Recommendation	Date	Description of Maintenance Conducted
Erosion due to concentrated irrigation flow Maintenance Needed? VES NO N/A	 Repair/re-seed/re-plant eroded areas and adjust the irrigation system Other / Comments: 	Date	
Erosion due to concentrated storm water runoff flow Maintenance Needed? YES NO N/A	 Repair/re-seed/re-plant eroded areas, and make appropriate corrective measures such as adding erosion control blankets, adding stone at flow entry points, or minor re-grading to restore proper drainage according to the original plan If the issue is not corrected by restoring the BMP to the original plan and grade, the [City Engineer] shall be contacted prior to any additional repairs or reconstruction Other / Comments: 		

Date:	Inspector:	BMP ID No.:
Permit No.:	APN(s):	

INSPECTION AND MAINTENANCE CHECKLIST FOR BF-1 BIOFILTRATION PAGE 4 of 5			
Threshold/Indicator	Maintenance Recommendation	Date	Description of Maintenance Conducted
Obstructed inlet or outlet structure	Clear blockage		
Maintenance Needed?	Other / Comments:		
□ YES			
□ N/A			
Underdrain clogged (inspect underdrain if	Clear blockage		
standing water is observed for longer than 24-96 hours following a storm event)	Other / Comments:		
Maintenance Needed?			
□ YES			
□ N/A			
Damage to structural components such as weirs,	Repair or replace as applicable		
inlet or outlet structures	□ Other / Comments:		
Maintenance Needed?			
□ YES			

Date:	Inspector:	BMP ID No.:
Permit No.:	APN(s):	

INS	INSPECTION AND MAINTENANCE CHECKLIST FOR BF-1 BIOFILTRATION PAGE 5 of 5			
Threshold/Indicator	Maintenance Recommendation	Date	Description of Maintenance Conducted	
Standing water in BMP for longer than 24-96 hours following a storm event* Surface ponding longer than approximately 24 hours following a storm event may be detrimental to vegetation health Maintenance Needed? YES NO N/A	 Make appropriate corrective measures such as adjusting irrigation system, removing obstructions of debris or invasive vegetation, clearing underdrains, or repairing/replacing clogged or compacted soils Other / Comments: 			
Presence of mosquitos/larvae For images of egg rafts, larva, pupa, and adult mosquitos, see <u>http://www.mosquito.org/biology</u> Maintenance Needed? YES NO N/A	 Apply corrective measures to remove standing water in BMP when standing water occurs for longer than 24-96 hours following a storm event.** Other / Comments: 			

*Surface ponding longer than approximately 24 hours following a storm event may be detrimental to vegetation health, and surface ponding longer than approximately 96 hours following a storm event poses a risk of vector (mosquito) breeding. Poor drainage can result from clogging of the media layer, filter course, aggregate storage layer, underdrain, or outlet structure. The specific cause of the drainage issue must be determined and corrected.

**If mosquitos persist following corrective measures to remove standing water, or if the BMP design does not meet the 96-hour drawdown criteria due to release rates controlled by an orifice installed on the underdrain, the [City Engineer] shall be contacted to determine a solution. A different BMP type, or a Vector Management Plan prepared with concurrence from the County of San Diego Department of Environmental Health, may be required.



StormTrap Maintenance Manual

1. Introduction

Regular inspections are recommended to ensure that the system is functioning as designed. Please call your Authorized StormTrap Representative if you have questions in regards to the inspection and maintenance of the StormTrap system. Prior to entry into any underground storm sewer or underground detention systems, appropriate OSHA and local safety regulations and guidelines should be followed.

2. Inspection Schedules for Municipalities

StormTrap Stormwater Management Systems are recommended for inspection whenever the upstream and downstream catch basins and stormwater pipes of the stormwater collection system are inspected or maintained. This will economize the cost of the inspection if it is done at the same time the Municipal crews are visiting the area.

3. Inspection Schedules for Private Development

StormTrap Stormwater Management Systems, for a private development, are recommended for inspection after each major storm water event. At a minimum, until a cleaning schedule can be established, an annual inspection is recommended. If inspected on an annual basis, the inspection should be conducted before the stormwater season begins to be sure that everything is functioning properly for the upcoming storm season.

4. Inspection Process

Inspections should be done such that at least 2-3 days has lapsed since the most recent rain event to allow for draining. Visually inspect the system at all manhole locations. Utilizing a sediment pole, measure and document the amount of silt at each manhole location. Inspect each pipe opening to ensure that the silt level or any foreign objects are not blocking the pipes. Be sure to inspect the outlet pipe(s) because this is typically the smallest pipe in the system. It is common that most of the larger materials will be collected upstream of the system in catch basins, and it is therefore important at time of inspections to check these structures for large trash or blockages.

Remove any blockages if you can during the inspection process only if you can do so safely from the top of the system without entering into the system. **Do not go into the system under any circumstances** without proper ventilation equipment and training. Pass any information requiring action onto the appropriate maintenance personnel if you cannot remove the blockages from above during the inspection process. Be sure to describe the location of each manhole and the type of material that needs to be removed.

The sediment level of the system should also be measured and recorded during the inspection process. Recording the sediment level at each manhole is very important in order get a history of sediment that can be graphed over time (i.e. years) in order to estimate when the system will

need to be maintained next. It is also important to keep these records to verify that the inspection process was actually performed if anyone asks for your records in the future.

The sediment level in the underground detention system can be determined from the outside of the system by opening up all the manholes and using a sediment pole to measure the amount of sediment at each location. Force the stick to the bottom of the system and then remove it and measure the amount of sediment at that location. Again, do not go into the system under any circumstances without proper ventilation equipment and training.

5. When to Clean the System

Any blockages should be safely removed as soon as practical so that the Stormwater detention system will fill and drain properly before the next stormwater event.

The Dry Detention System should be completely cleaned whenever the sediment occupies more than 10% to 15% of the originally designed system's volume. The Wet Detention System should be cleaned when the sediment occupies more than 30% or 1/3rd of the originally designed system's volume. NOTE: Check with your municipality in regards to cleaning criteria, as the allowable sediment before cleaning may be more or less then described above.

6. How to Clean the StormTrap

The system should be completely cleaned back to 100% of the originally designed storage volume whenever the above sediment levels have been reached. Be sure to wait at least 3 days after a stormwater event to be sure that the system is completely drained (if it is a Dry Detention System), and all of the sediments have settled to the bottom of the system (if it is a Wet Detention System).

Do not enter the System unless you are properly trained, equipped, and qualified to enter a confined space as identified by local occupational safety and health regulations.

There are many maintenance companies that are in business to help you clean your underground stormwater detention systems and water quality units. Please call your StormTrap representative for referrals in your area.

A. Dry Detention System Cleaning

Maintenance is typically performed using a vacuum truck. Sediment should be flushed towards a vacuum hose for thorough removal. For a Dry Detention System, remove the manhole cover at the top of the system and lower a vacuum hose into one of the rows of the StormTrap system. Open up the manhole at the opposite end of the StormTrap and use sewer jetting equipment to force water in the same row from one end of the StormTrap row to the opposite side. The rows of the StormTrap are completely open in one contiguous channel from one end to the other for easy cleaning.

Place the vacuum hose and the sewer jetting equipment in the next row and repeat the process until all of the rows have been cleaned.

When finished, replace all covers that were removed and dispose of the collected material properly.

B. Wet Detention System Cleaning

If the system was designed to maintain a permanent pool of water, floatables and any oil should be removed in a separate procedure prior to the removal of all sediment.

The floatable trash is removed first by using a bucket strainer to capture and remove any floating debris.

The floatable oils are then removed off the top of the water by using the vacuum truck to suck off any floatable fluids and liquids.

The next step is to use the vacuum truck to gently remove the clarified water above the sediment layer.

The final step is to clean the sediment for each row as described above in the paragraph "A. Dry Detention System Cleaning". For smaller systems, the vacuum truck can remove all of the sediment in the basin without using the sewer jetting equipment because of the smaller space.

8. Proof of these inspections is the responsibility of the property owner. All inspection reports and data should be kept on site or at a location where they will be accessible for years in the future. Some municipalities require these inspection and cleaning reports to be forwarded to the proper governmental permitting agency on an annual basis.

Refer to your local and national regulations for any additional maintenance requirements and schedules not contained herein. Inspections should be a part of your standard operating procedure.

SAMPLE INSPECTION AND MAINTENANCE LOG

Date	Depth of Sediment	Accumulated Trash	Maintenance Performed	Maintenance Personnel	Comments
	3"	None	Sediment Removal/Vac	B. Johnson	

ATTACHMENT 4 - COPY OF PLAN SHEETS SHOWING PERMANENT STORM WATER BMPS

This is the cover sheet for Attachment 4.

Use this checklist to ensure the required information has been included on the plans:

The plans must identify:

- □ Structural BMP(s) with ID numbers matching Form I-6 Summary of PDP Structural BMPs
- □ The grading and drainage design shown on the plans must be consistent with the delineation of DMAs shown on the DMA exhibit
- Details and specifications for construction of structural BMP(s)
- □ Signage indicating the location and boundary of structural BMP(s) as required by the [City Engineer]
- □ How to access the structural BMP(s) to inspect and perform maintenance
- Features that are provided to facilitate inspection (e.g., observation ports, cleanouts, silt posts, or other features that allow the inspector to view necessary components of the structural BMP and compare to maintenance thresholds)
- □ Manufacturer and part number for proprietary parts of structural BMP(s) when applicable
- □ Maintenance thresholds specific to the structural BMP(s), with a location-specific frame of reference (e.g., level of accumulated materials that triggers removal of the materials, to be identified based on viewing marks on silt posts or measured with a survey rod with respect to a fixed benchmark within the BMP)
- □ Recommended equipment to perform maintenance
- □ When applicable, necessary special training or certification requirements for inspection and maintenance personnel such as confined space entry or hazardous waste management
- □ Include landscaping plan sheets showing vegetation requirements for vegetated structural BMP(s)
- □ All BMPs must be fully dimensioned on the plans
- □ When proprietary BMPs are used, site-specific cross section with outflow, inflow, and model number shall be provided. Photocopies of general brochures are not acceptable.

ļ	EGEND	
ę	UBJECT PROPERTY / SUBDIVISION BOUNDARY	
F	RIGHT-OF-WAY / PROPOSED FUTURE LOT LINE	
(ENTERLINE OF ROAD	
E	XISTING EASEMENTS	
F	ROPOSED EASEMENTS	
ļ	DJACENT PROPERTY LINE	
l	EXISTING EASEMENT INFOR	MATION
1	AN EASEMENT GRANTED TO THE STATE OF CALIFO OFFICE OF THE COUNTY RECORDER OF SAN DIEG 751, PAGE 377, AND BOOK 1073, PAGE 395, AS SHOW	O COUNTY IN BOOK 751, PAGE 375, AND IN BOOK
2	AN EASEMENT GRANTED TO THE STATE OF CALIFOR IN THE OFFICE OF THE COUNTY RECORDER OF S OFFICIAL RECORDS, DATED JUNE 11, 1934, AS SHOW	SAN DIEGO COUNTY AS BOOK 297, PAGE 324 OF
3	AN EASEMENT GRANTED TO SAN DIEGO GAS A INGRESS AND EGRESS, RECORDED IN THE OFFIC COUNTY AS BOOK 1867, PAGE 333, OF OFFICIAL REC	CE OF THE COUNTY RECORDER OF SAN DIEGO
ı ١	AN EASEMENT FOR PUBLIC UTILITIES, RECORDED IN DIEGO COUNTY AS BOOK 1935, PAGE 418, OF OFFICI THE EXACT LOCATION OF SAID EASEMENT IS NO HEREON.	AL RECORDS, DATED OCTOBER 4, 1945.
5	AN EASEMENT GRANTED TO SAN DIEGO GAS A INGRESS AND EGRESS, RECORDED IN THE OFFIC COUNTY AS BOOK 2198, PAGE 227, DATED FEBRUAR	CE OF THE COUNTY RECORDER OF SAN DIEGO
3	AN EASEMENT GRANTED TO CITY OF ENCINITAS F THE OFFICE OF THE COUNTY RECORDER OF SAN I DATED JANUARY 10, 2019, OF OFFICIAL RECORDS.	
7	AN EASEMENT GRANTED TO CITY OF ENCINITAS F THE OFFICE OF THE COUNTY RECORDER OF SAN DATED JANUARY 10, 2019, OF OFFICIAL RECORDS.	
	PROPOSED EASEMENT INFO	ORMATION
1	PROPOSED EMERGENCY ACCESS EASEMENT FOR PRI	VATE ROADS TO THE CITY OF ENCINITAS
2	PROPOSED SEWER EASEMENT TO THE ENCINITAS SAM	NITARY DISTRICT

- 3 PROPOSED WATER EASEMENT TO THE SAN DIEGUITO WATER DISTRICT
- 4 PROPOSED PRIVATE DRAINAGE EASEMENT

UTILITIES

WATER FIRE SEWER ELEMENTARY SCHOOL HIGH SCHOOL

SAN DIEGUITO WATER DISTRICT ENCINITAS FIRE PROTECTION DISTRICT LEUCADIA WASTEWATER DISTRICT ENCINITAS UNION SCHOOL DISTRICT SAN DIEGUITO UNION HIGH SCHOOL DISTRICT

165,107 SF (3.790 AC)

MH

PA

P/L

PP

PROP

PVC

R/W

SCO

SDCO

SDMH

TC

ΤG

TS

ΤW

TYP

WAR

WM

WV

SL SMH

AREA CALCULATIONS

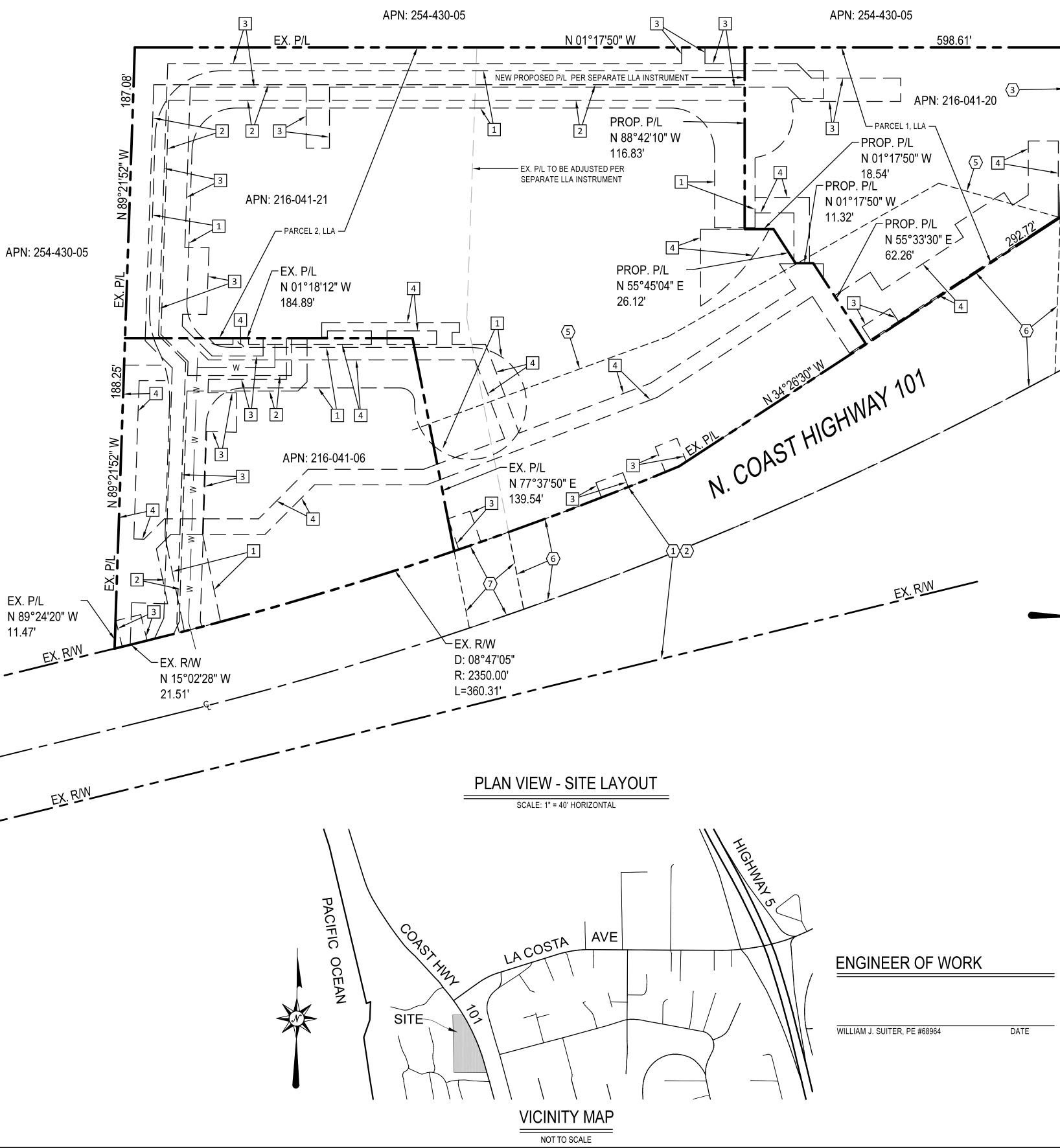
TOTAL SITE AREA: AREA DISTURBED BY PROJECT: 166,988 SF (3.834 AC)

EXISTING IMPERVIOUS AREA: 76,820 SF (1.764 AC) PROPOSED IMPERVIOUS AREA: 143,659 SF (3.298 AC) INCREASE IMPERVIOUS AREA: 66,839 SF (1.534 AC)

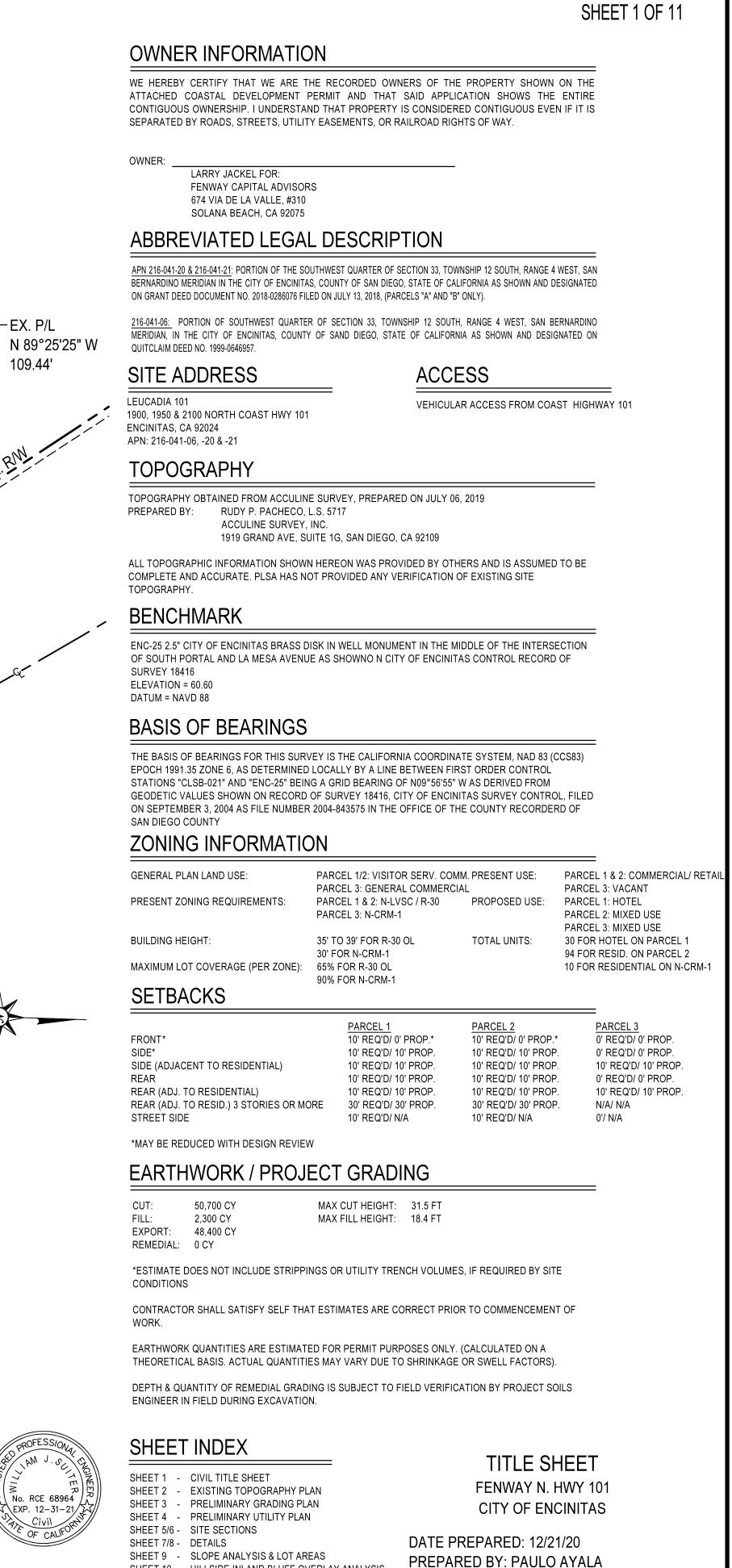
ABBREVIATIONS

٩C	ASPHALT CONCRETE
BFD	BACKFLOW DEVICE
3S	BOTTOM OF STAIRS
3W	BOTTOM OF WALL
СВ	CATCH BASIN
CF	CURB FACE
00	CLEANOUT
CONC	CONCRETE
ĒG	EDGE OF GUTTER
ELEC	ELECTRICAL
ELVT	ELEVATOR
ΞX	EXISTING
F	FINISH FLOOR
G	FINISHED GRADE
L	FLOW LINE
M	FORCE MAIN
₹S	FINISHED SURFACE
GA	GUY ANCHOR
GB	GRADE BREAK
GF	GARAGE FLOOR
GP	GUY POLE
GV	GAS VALVE
NV	INVERT ELEVATION

MANHOLE PLANTER AREA PROPERTY LINE POWER POLE PROPOSED POLYVINYL CHLORIIDE RIGHT-OF-WAY SEWER CLEANOUT STORM DRAIN CLEANOUT STREET LIGHT SEWER MANHOLE STORM DRAIN MANHOLE TOP OF CURB TOP OF GRATE TOP OF STAIRS TOP OF WALL TYPICAL WATER AIR RELEASE WATER METER WATER VALVE



MULTI-003780-2020/ BADJ-003787-2020/ CDP-003788-2020/ DR-003786-2020 FENWAY N. HWY 101

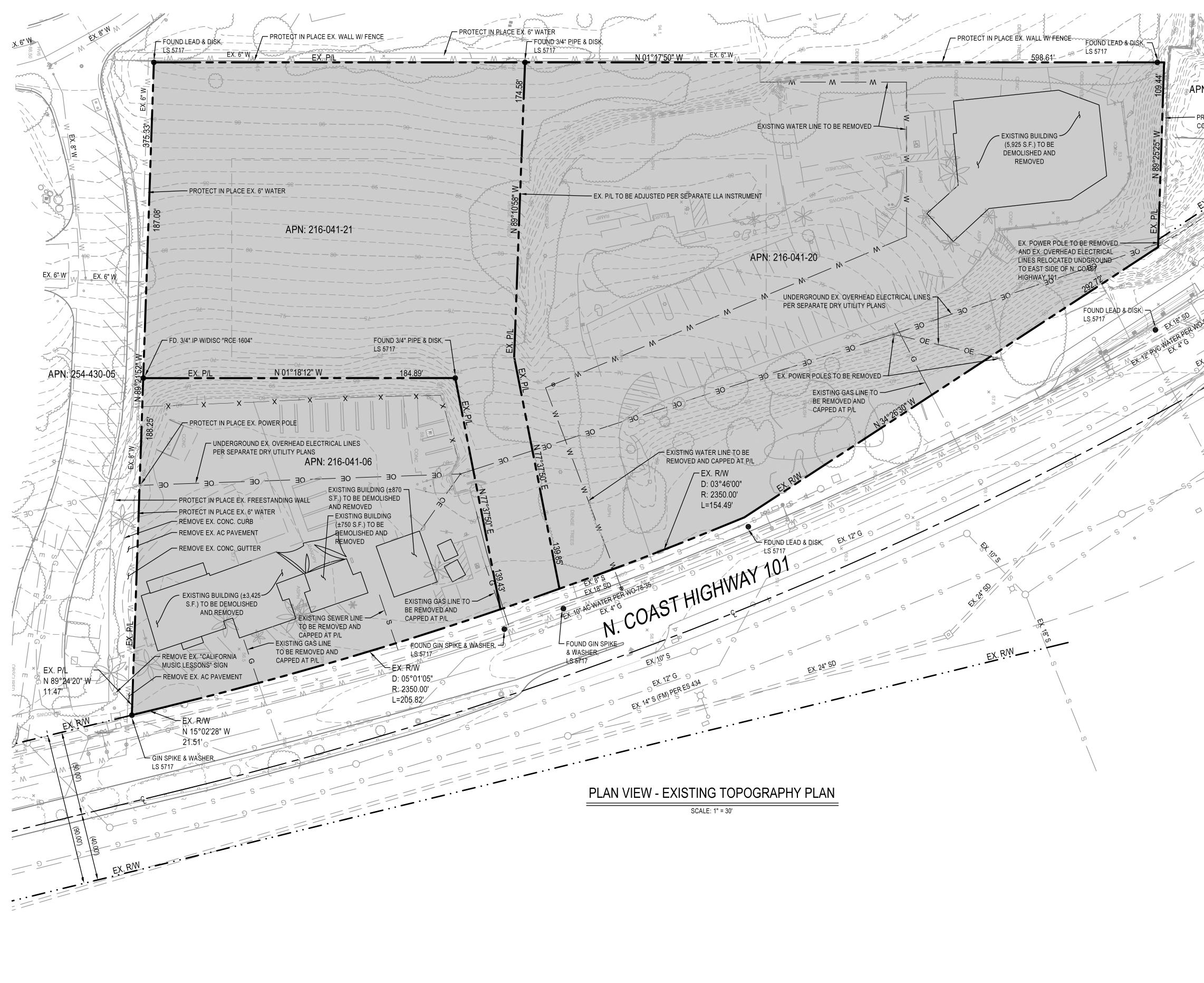




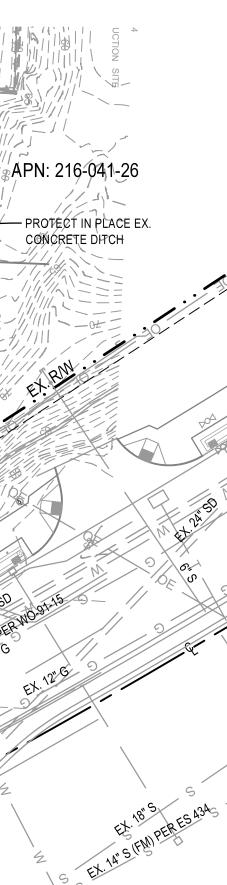
SHEET 9 - SLOPE ANALYSIS & LOT AREAS SHEET 10 - HILLSIDE INLAND BLUFF OVERLAY ANALYSIS

SHEET 11 - PRELIMINARY WALL & FENCE PLAN

PASCO LARET SUITER 🔲 & ASSOCIATES San Diego I Solana Beach I Orange County Phone 949.661.6695 | www.plsaengineering.com



EXISTING TOPOGRAPHY PLAN FENWAY N. HWY 101



LEGEND

PROPERTY BOUNDARY	 	
CENTERLINE OF ROAD	 <u> </u>	
ADJACENT PROPERTY LINE / RIGHT-OF-WAY	 · · —	••
EXISTING CONTOUR LINE	64	
EXISTING WATER MAIN (SIZE PER PLAN)	 — W —	- VV
EXISTING SEWER MAIN (SIZE PER PLAN)	 — S ———	- S ———
EXISTING STORM DRAIN (SIZE PER PLAN)	 SD	- SD
EXISTING GAS MAIN	 — G ———	- G
EXISTING TELECOM CONDUIT	 — т ——	- T
LIMITS OF ON-SITE REMOVAL		

SITE NOTES

- 1. EXISTING SURVEY MONUMENTS TO BE PROTECTED IN PLNCE. IF MONUMENT IS DISTURBED OR DESTROYED, IT SHALL BE REPLACED BY A LICENSED LAND SURVEYOR AND A CORNER RECORD OF SURVEY SHALL BE FILED WITH THE COUNTY.
- 2. ALL EXISTING STRUCTURES AND WALLS WITHIN THE PROPOSED DISTURBED AREA TO BE DEMOLISHED UNLESS OTHERWISE NOTED
- 3. ALL EXISTING TREES WITHIN THE PROPOSED DISTURBED AREA TO BE REMOVED AND THE AREA CLEARED / GRUBBED UNLESS OTHERWISE NOTED.
- 4. ALL UTILITIES SHOWN HEREON PER BEST AVAILABLE RECORDS. CONTRACTOR SHALL VERIFY EXACT HORIZONTAL AND VERTICAL LOCATION PRIOR TO CONSTRUCTION. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD OF DISCREPANCIES UPON DISCOVERY.

PP

PROP

PVC

R/W

SCO

SL

TC

ΤG

TS

тw

TYP

WM

WV

SMH

SDMH

SDCO

ABBREVIATIONS

EASEMENT NOTES

**SEE SHEET 1 FOR ALL EXISTING EASEMENTS PLOTTED AND LABELED ONSITE

PROPERTY LINE POWER POLE PROPOSED POLYVINYL CHLORIIDE RIGHT-OF-WAY SEWER CLEANOUT STORM DRAIN CLEANOUT STREET LIGHT SEWER MANHOLE STORM DRAIN MANHOLE TOP OF CURB TOP OF GRATE TOP OF STAIRS TOP OF WALL TYPICAL WATER METER WATER VALVE

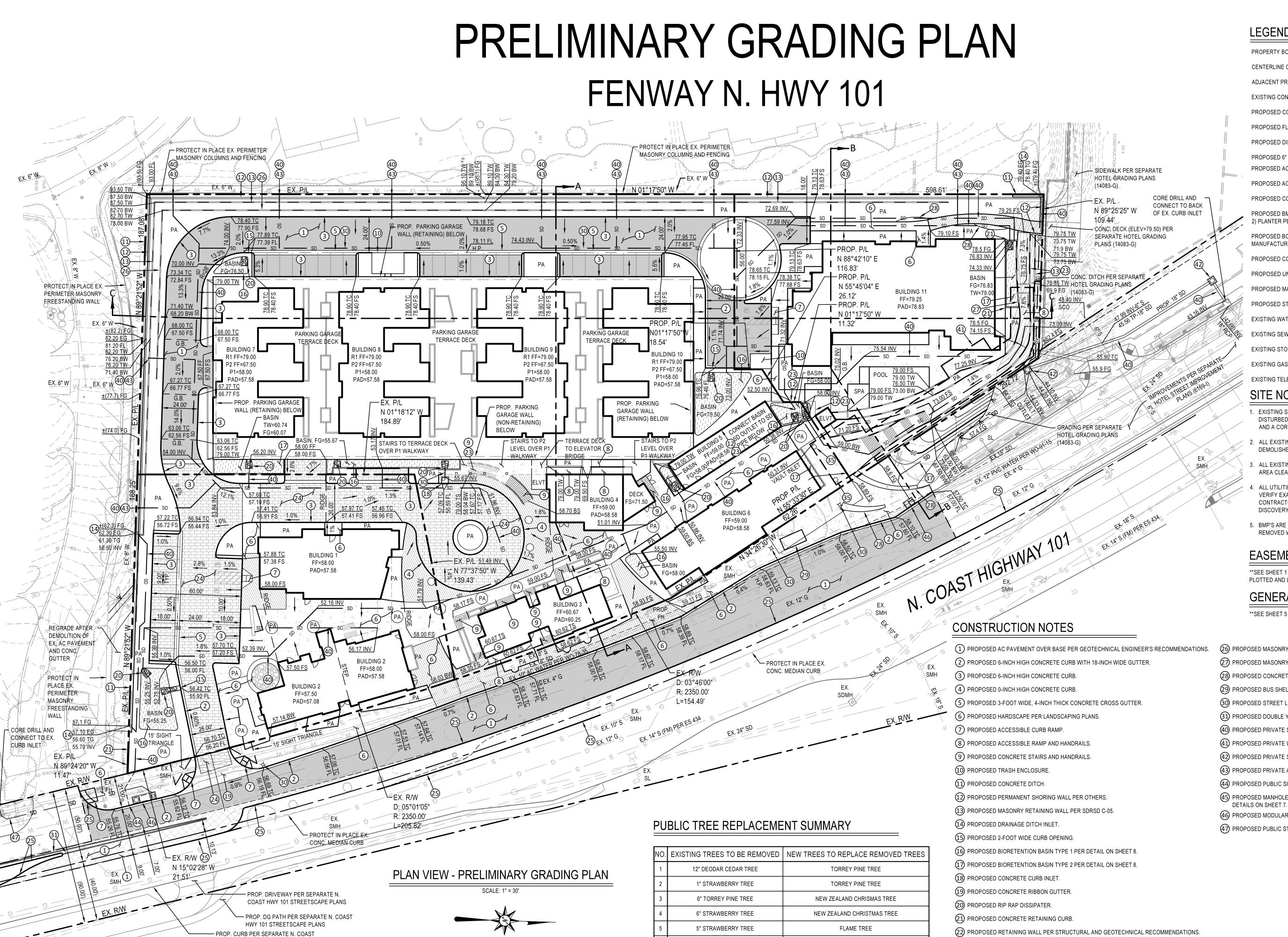


SCALE: 1'' = 30'

30 15 Ó EXISTING TOPOGRAPHY PLAN FENWAY N. HWY 101 CITY OF ENCINITAS

PASCO LARET SUITER & ASSOCIATES

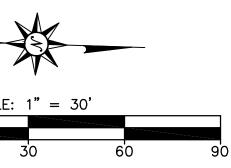
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15

HWY 101 STREETSCAPE PLANS

FLAME TREE 6" STRAWBERRY TREE **4" TORREY PINE TREE** FLAME TREE



SHEET 3 OF 11

PROPERTY BOUNDARY		
CENTERLINE OF ROAD		
ADJACENT PROPERTY LINE / RIGHT-OF-WAY		
EXISTING CONTOUR LINE		64 — — — —
PROPOSED CONTOUR LINE		- 64
PROPOSED FLOWLINE	· · · · · · · · · · · · · · · · · · · 	•••••••••
PROPOSED DIRECTION OF FLOW		
PROPOSED 6" PCC CURB & GUTTER PER SDRSD G-2		
PROPOSED AC PAVEMENT		
PROPOSED AC PAVEMENT TO REPLACE EX. MEDIAN		
PROPOSED CONCRETE PAVEMENT	_ ~ ~ ~ ~	·
PROPOSED BMP BIOFILTRATION BASIN (TYPE 1 OR TYPE 2) PLANTER PER DETAILS ON SHEET 7		
PROPOSED BOARDWALK PAVING (LINEAR PAVERS OR MANUFACTURED WOOD DECKING)		
PROPOSED CONCRETE PAVERS		
PROPOSED UNDERGROUND PARKING GARAGE WALL		77777
PROPOSED MASONRY RETAINING WALL		
PROPOSED STORM DRAIN		SD
EXISTING WATER MAIN (SIZE PER PLAN)	W	VV
EXISTING SEWER MAIN (SIZE PER PLAN)	S	S
EXISTING STORM DRAIN (SIZE PER PLAN)	SD	SD
EXISTING GAS MAIN (SIZE PER PLAN)	G	G
EXISTING TELECOM CONDUIT	т —	— T ——
SITE NOTES		

- 1. EXISTING SURVEY MONUMENTS TO BE PROTECTED IN PLACE. IF MONUMENT IS DISTURBED OR DESTROYED, IT SHALL BE REPLACED BY A LICENSED LAND SURVEYOR AND A CORNER RECORD OF SURVEY SHALL BE FILED WITH THE COUNTY
- ALL EXISTING STRUCTURES AND WALLS WITHIN THE PROPOSED DISTURBED AREA TO B DEMOLISHED UNLESS OTHERWISE NOTED.
- ALL EXISTING TREES WITHIN THE PROPOSED DISTURBED AREA TO BE REMOVED AND TH AREA CLEARED / GRUBBED UNLESS OTHERWISE NOTED
- ALL UTILITIES SHOWN HEREON PER BEST AVAILABLE RECORDS. CONTRACTOR SHAL VERIFY EXACT HORIZONTAL AND VERTICAL LOCATION PRIOR TO CONSTRUCTION. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD OF DISCREPANCIES UPON
- 5. BMP'S ARE TO BE PRIVATELY MAINTAINED AND THE FACILITIES NOT MODIFIED OR REMOVED WITHOUT A PERMIT FROM THE CITY.

EASEMENT NOTES

**SEE SHEET 1 FOR ALL EXISTING EASEMENTS

PLOTTED AND LABELED ONSITE **GENERAL NOTES**

**SEE SHEET 5 & 6 FOR SITE SECTIONS

(25) PROPOSED AC BERM.

23 PROPOSED SAFETY GUARDRAIL.

(24) PROPOSED VEHICULAR CONCRETE PAVERS PER GEOTECHNICAL RECOMMENDATIONS.

- (26) PROPOSED MASONRY RETAINING WALL PER SDRSD C-01.
- (27) PROPOSED MASONRY RETAINING WALL PER SDRSD C-03.
- (28) PROPOSED CONCRETE SEAT WALL.

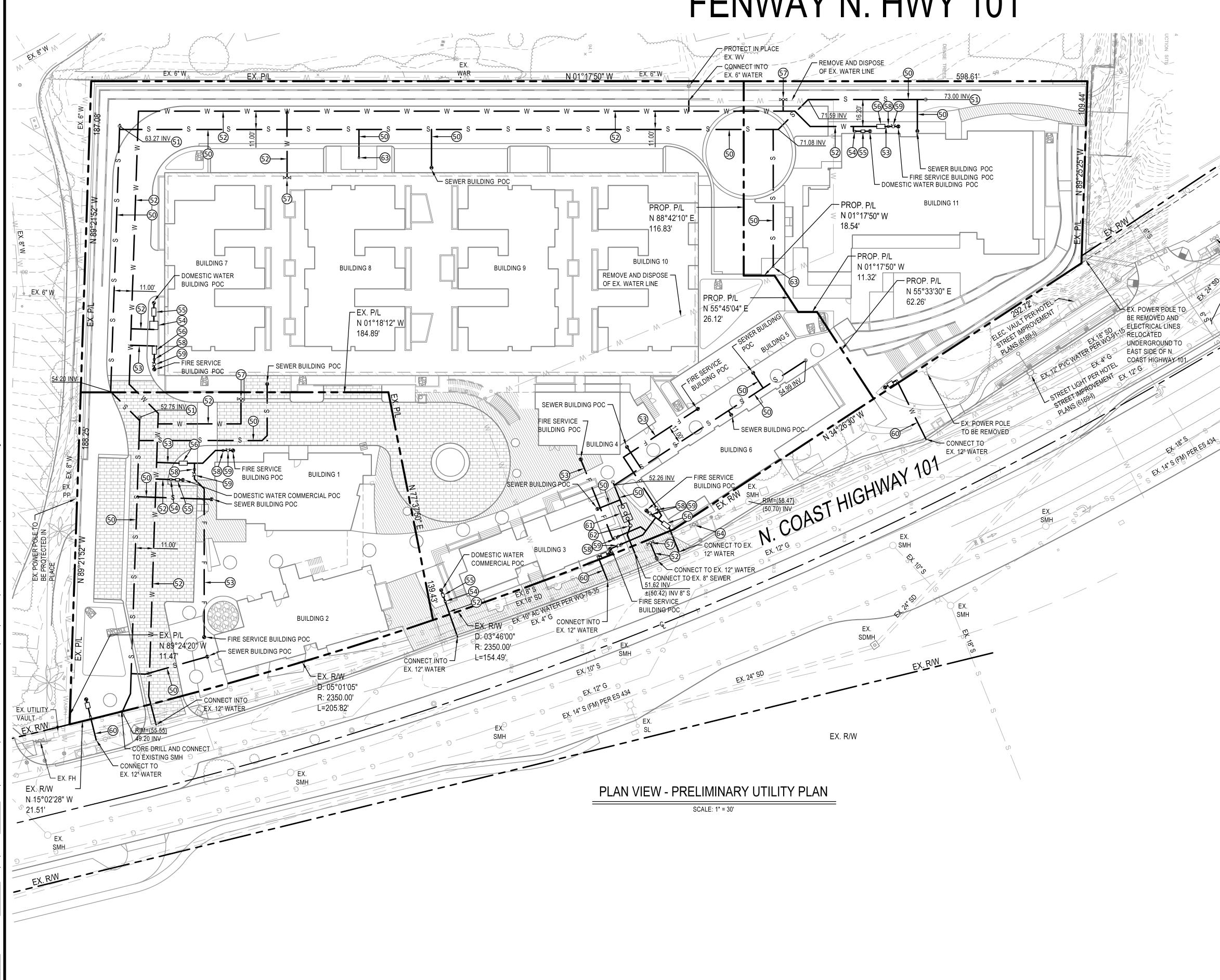
LEGEND

- (29) PROPOSED BUS SHELTER STRUCTURE.
- (30) PROPOSED STREET LIGHT.
- (31) PROPOSED DOUBLE YELLOW STRIPING.
- (40) PROPOSED PRIVATE STORM DRAINAGE PIPE.
- (41) PROPOSED PRIVATE UNDERGROUND STORMTRAP DETENTION SYSTEM PER DETAILS ON SHEET 7.
- (42) PROPOSED PRIVATE STORM DRAIN CLEANOUT PER SDRSD D-09.
- (43) PROPOSED PRIVATE AREA DRAIN.
- (44) PROPOSED PUBLIC SIDEWALK UNDERDRAIN.
- (45) PROPOSED MANHOLE FRAME AND COVER FOR ACCESS INTO STORMTRAP DETENTION SYSTEM PER
- (46) PROPOSED MODULAR WETLANDS SYSTEM (CURB INLET TYPE).
- (47) PROPOSED PUBLIC STORM DRAINAGE PIPE.

PRELIMINARY GRADING PLAN FENWAY N. HWY 101 CITY OF ENCINITAS

PASCO LARET SUITER & ASSOCIATES

San Diego I Solana Beach I Orange County Phone 949.661.6695 | www.plsaengineering.com



PRELIMINARY UTILITY PLAN FENWAY N. HWY 101

SHEET 4 OF 11

LEGEND

PROPERTY BOUNDARY/RIGHT-OF-WAY		
CENTERLINE OF ROAD		
SETBACK LINE		
EXISTING CONTOUR LINE		64
PROPOSED CONTOUR LINE		64
PROPOSED DIRECTION OF FLOW		>
PROPOSED CURB & GUTTER		
PROPOSED MASONRY RETAINING WALL		
PROPOSED WATER MAIN	— w —	— w —
PROPOSED FIRE SERVICE	——— F —	— F —
PROPOSED SEWER MAIN	\$	s
EXISTING WATER MAIN (SIZE PER PLAN)	W	VV
EXISTING SEWER MAIN (SIZE PER PLAN)	S	S
EXISTING STORM DRAIN (SIZE PER PLAN)	SD	SD
EXISTING GAS MAIN	G	G
EXISTING TELECOM CONDUIT	— т —	— т —

CONSTRUCTION NOTES

50 PROPOSED PRIVATE SEWER PIPE.

(51) PROPOSED PRIVATE SEWER CLEANOUT.

(52) PROPOSED PUBLIC WATER PIPE.

(53) PROPOSED PRIVATE FIRE SERVICE WATER PIPE.

(54) PROPOSED PUBLIC WATER METER.

(55) PROPOSED PRIVATE DOMESTIC WATER BACKFLOW.

(56) PROPOSED PUBLIC FIRE SERVICE REDUCED PRESSURE BACKFLOW PREVENTION DEVICE.

(57) PROPOSED PUBLIC FIRE HYDRANT

(58) PROPOSED PRIVATE POST INDICATOR VALVE.

(59) PROPOSED PRIVATE FIRE DEPARTMENT CONNECTION.

60 PROPOSED PUBLIC IRRIGATION METER, SERVICE LATERAL AND PRIVATE BACKFLOW.

(61) PROPOSED PRIVATE SEWER GREASE INTERCEPTOR.

(62) PROPOSED PRIVATE SEWER GREASE SAMPLE BOX.

(63) PROPOSED PRIVATE DRAINAGE INLET FOR TRASH ENCLOSURE.

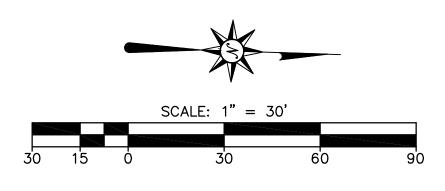
64 REMOVE EX. PUBLIC FIRE HYDRANT.

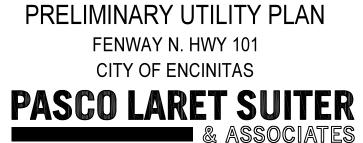
SITE NOTES

- 1. EXISTING SURVEY MONUMENTS TO BE PROTECTED IN PLNCE. IF MONUMENT IS DISTURBED OR DESTROYED, IT SHALL BE REPLACED BY A LICENSED LAND SURVEYOR AND A CORNER RECORD OF SURVEY SHALL BE FILED WITH THE COUNTY.
- 2. ALL EXISTING STRUCTURES AND WALLS WITHIN THE PROPOSED DISTURBED AREA TO BE DEMOLISHED UNLESS OTHERWISE NOTED.
- 3. ALL EXISTING TREES WITHIN THE PROPOSED DISTURBED AREA TO BE REMOVED AND THE AREA CLEARED / GRUBBED UNLESS OTHERWISE NOTED.
- 4. ALL UTILITIES SHOWN HEREON PER BEST AVAILABLE RECORDS. CONTRACTOR SHALL VERIFY EXACT HORIZONTAL AND VERTICAL LOCATION PRIOR TO CONSTRUCTION. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD OF DISCREPANCIES UPON DISCOVERY.

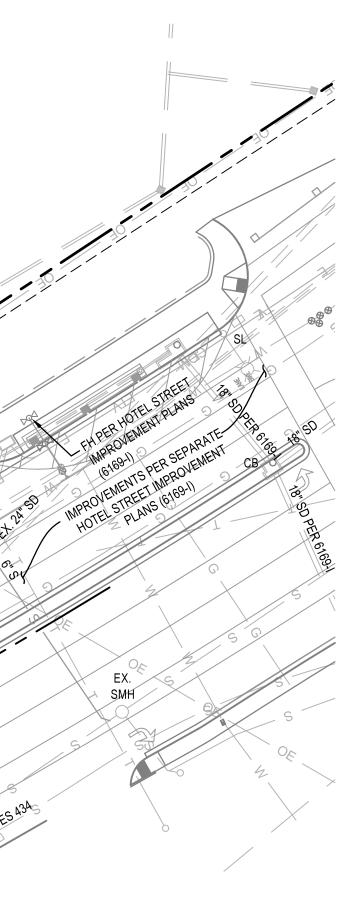
EASEMENT NOTES

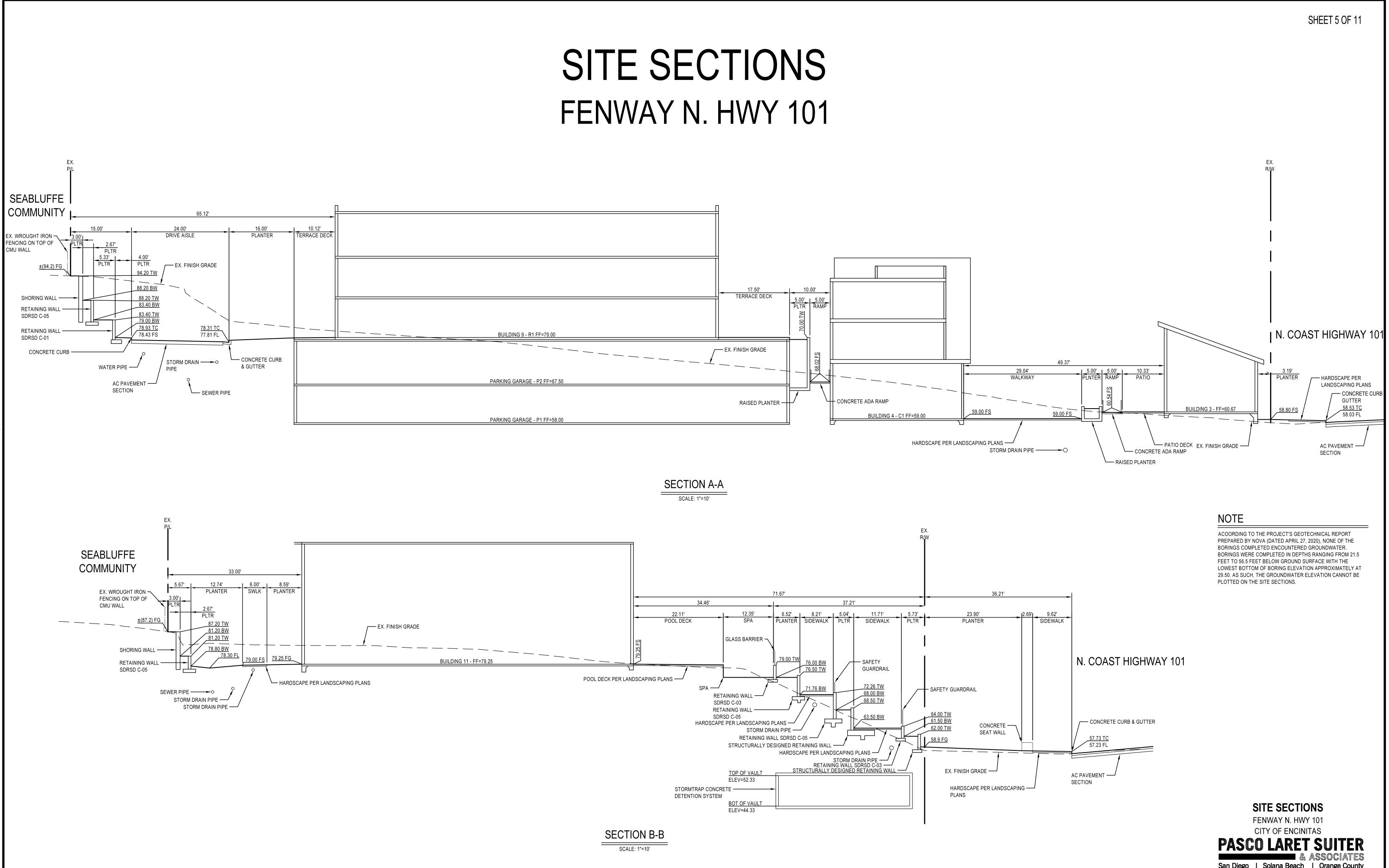
**SEE SHEET 1 FOR ALL EXISTING AND PROPOSED EASEMENTS PLOTTED AND LABELED ONSITE



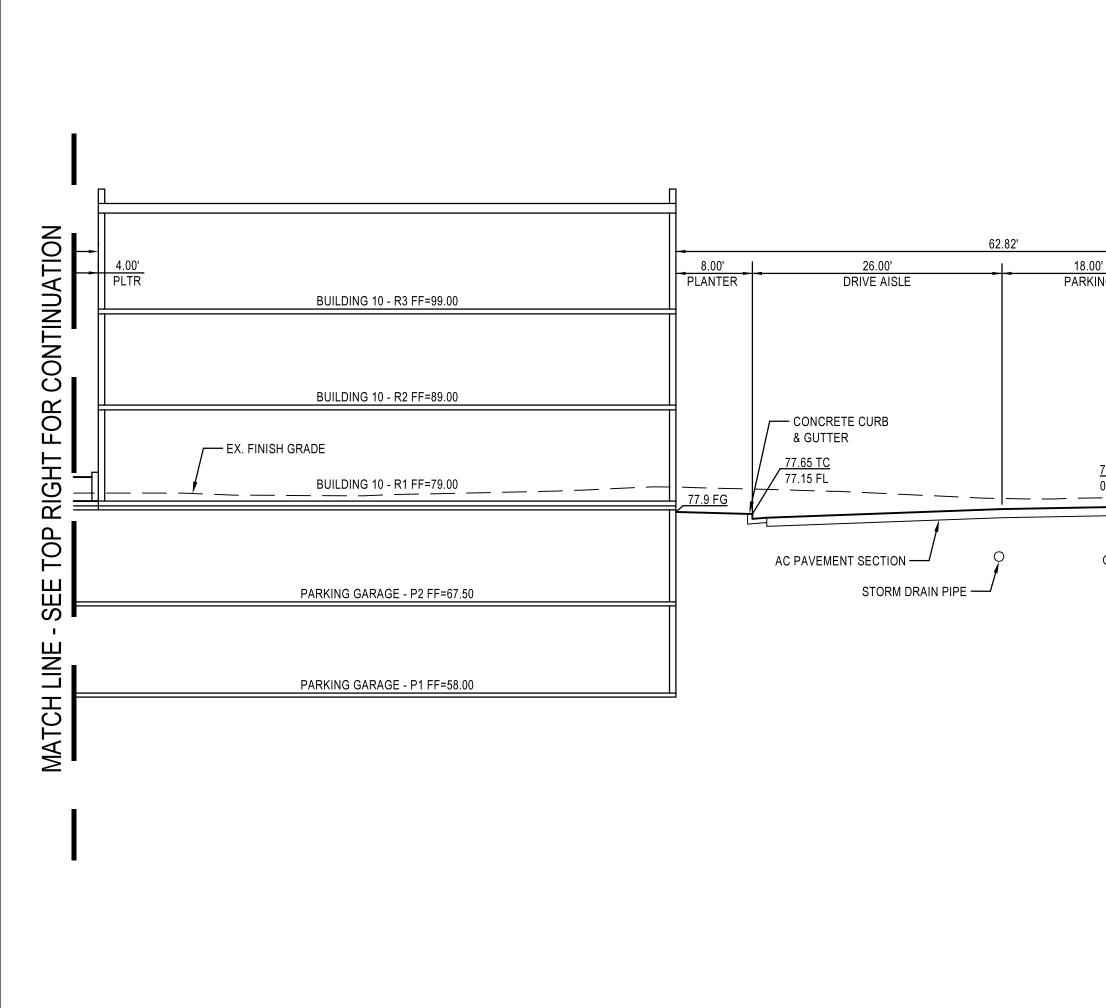


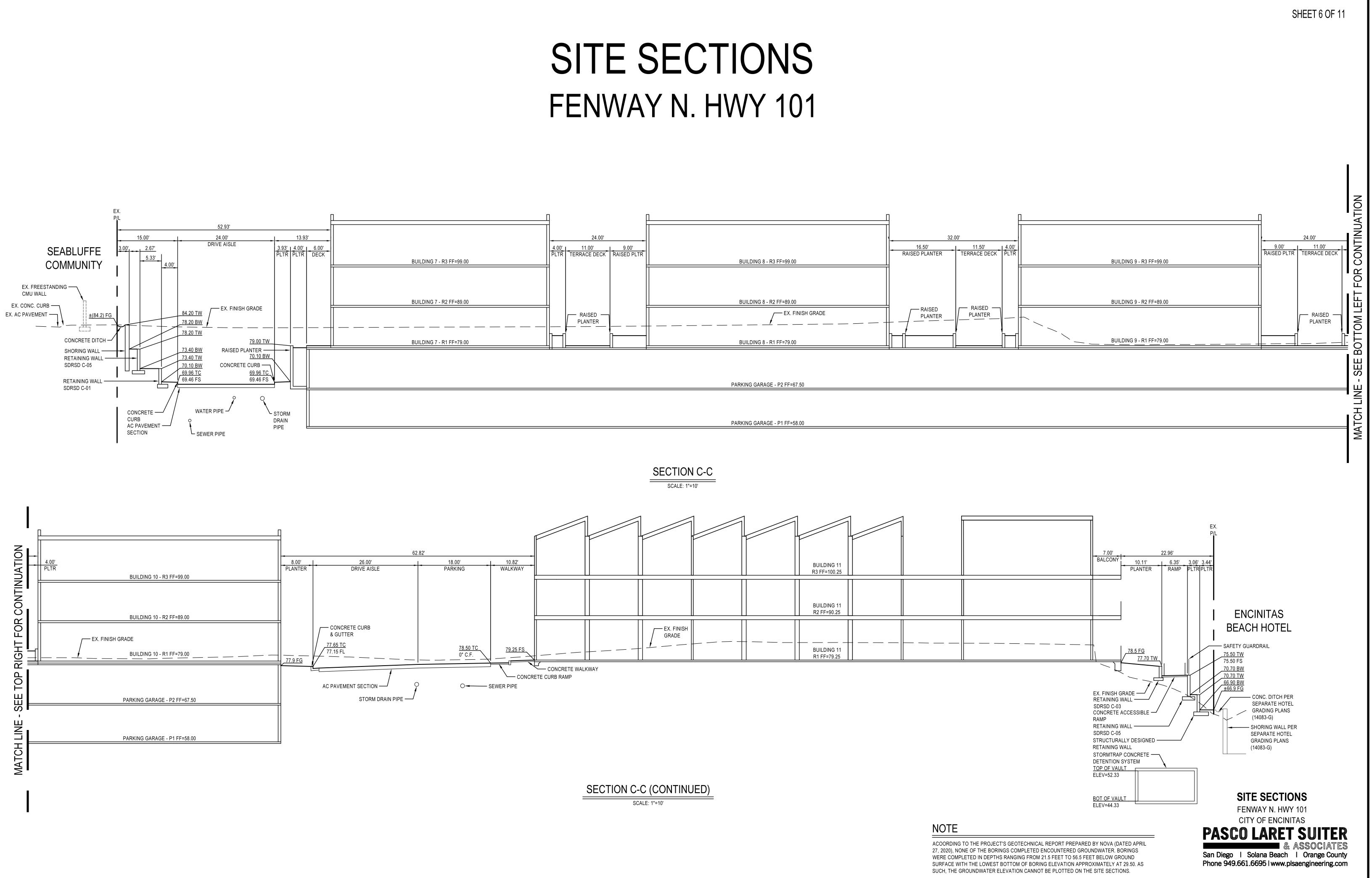
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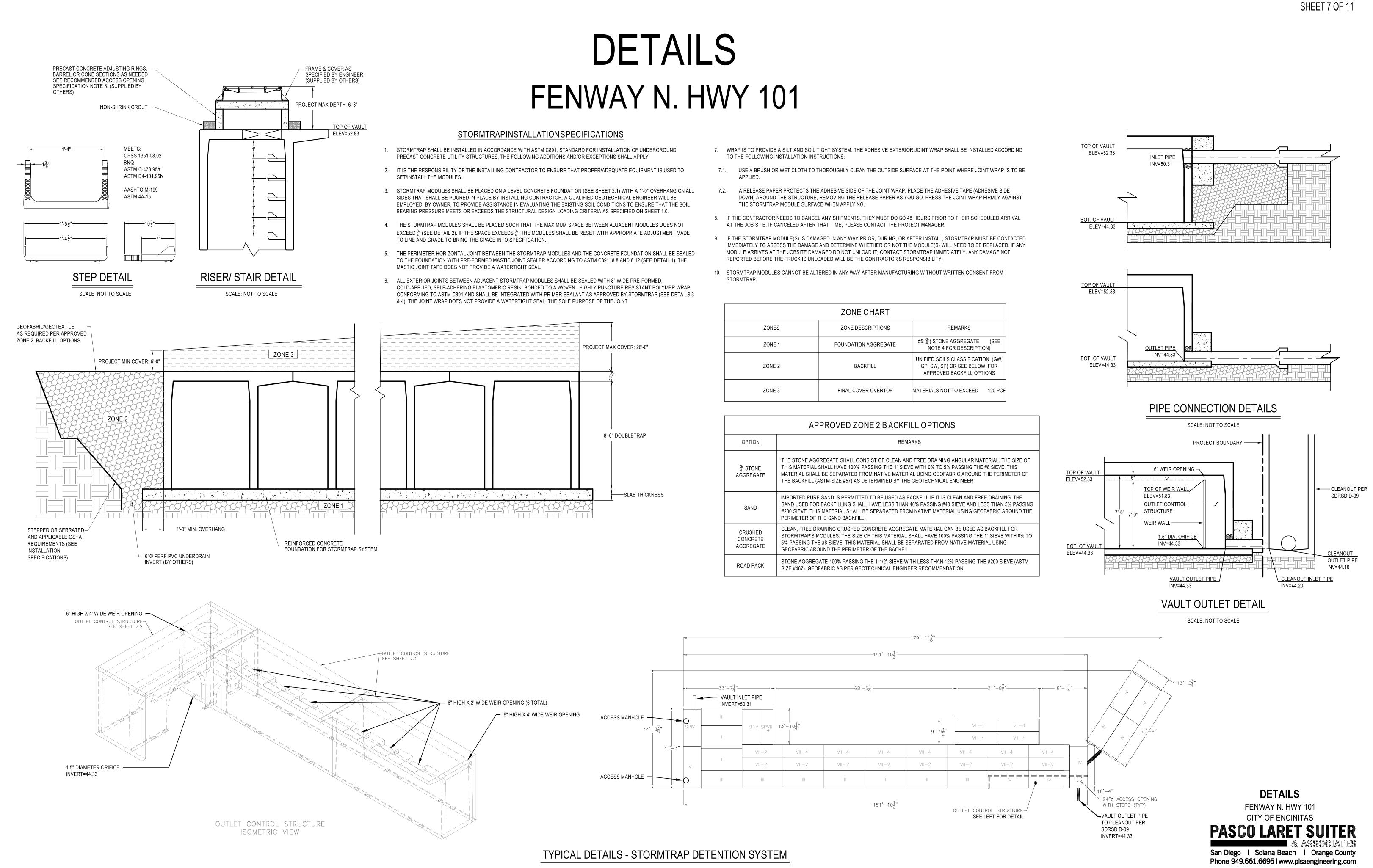




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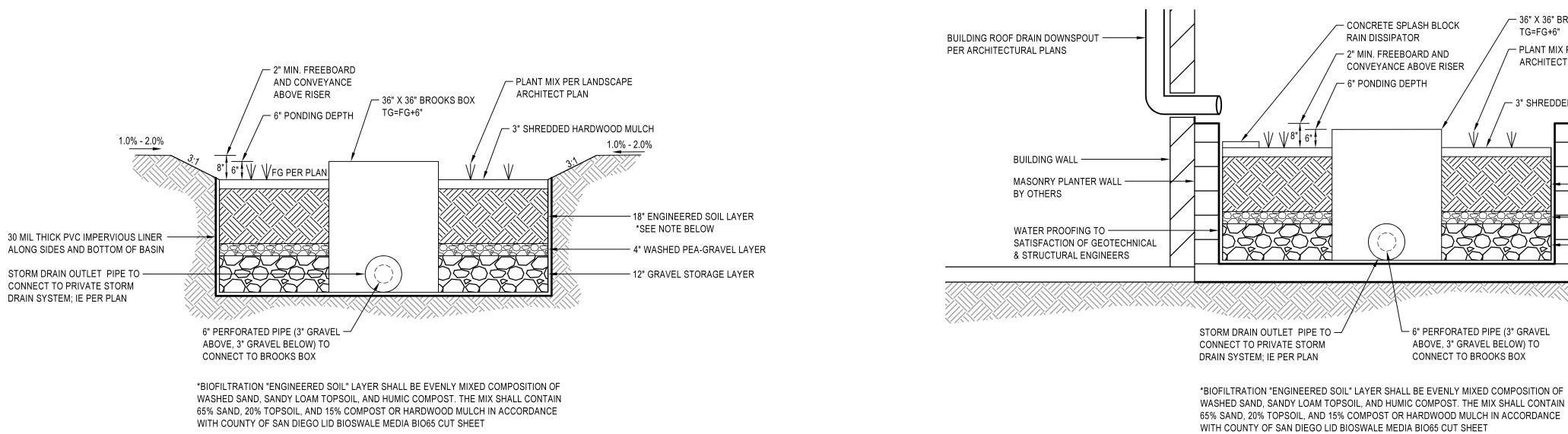




	ZONE CHART	
ZONES	ZONE DESCRIPTIONS	REMARKS
ZONE 1	FOUNDATION AGGREGATE	#5 (³ ") STONE AGGREG/ NOTE 4 FOR DESCF
ZONE 2	BACKFILL	UNIFIED SOILS CLASSIFI GP, SW, SP) OR SEE B APPROVED BACKFILL
ZONE 3	FINAL COVER OVERTOP	MATERIALS NOT TO EXCE

APPROVED ZONE 2 BACKFILL OPTIONS			
OPTION	REMARKS		
³ " STONE AGGREGATE	THE STONE AGGREGATE SHALL CONSIST OF CLEAN AND FREE DRAINING ANGULAF THIS MATERIAL SHALL HAVE 100% PASSING THE 1" SIEVE WITH 0% TO 5% PASSING MATERIAL SHALL BE SEPARATED FROM NATIVE MATERIAL USING GEOFABRIC ARO THE BACKFILL (ASTM SIZE #57) AS DETERMINED BY THE GEOTECHNICAL ENGINEEF		
SAND	IMPORTED PURE SAND IS PERMITTED TO BE USED AS BACKFILL IF IT IS CLEAN AND SAND USED FOR BACKFILLING SHALL HAVE LESS THAN 40% PASSING #40 SIEVE AN #200 SIEVE. THIS MATERIAL SHALL BE SEPARATED FROM NATIVE MATERIAL USING PERIMETER OF THE SAND BACKFILL.		
CRUSHED CONCRETE AGGREGATE	CLEAN, FREE DRAINING CRUSHED CONCRETE AGGREGATE MATERIAL CAN BE USE STORMTRAP'S MODULES. THE SIZE OF THIS MATERIAL SHALL HAVE 100% PASSING 5% PASSING THE #8 SIEVE. THIS MATERIAL SHALL BE SEPARATED FROM NATIVE M. GEOFABRIC AROUND THE PERIMETER OF THE BACKFILL.		
ROAD PACK	STONE AGGREGATE 100% PASSING THE 1-1/2" SIEVE WITH LESS THAN 12% PASSIN SIZE #467). GEOFABRIC AS PER GEOTECHNICAL ENGINEER RECOMMENDATION.		

SCALE: NOT TO SCALE



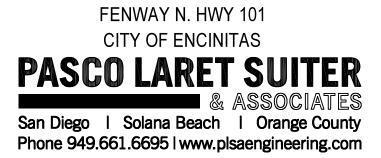
TYPICAL SECTION - BIOFILTRATION BASIN TYPE 1

SCALE: NOT TO SCALE

DETAILS FENWAY N. HWY 101

SCALE: NOT TO SCALE

TYPICAL SECTION - BIOFILTRATION BASIN TYPE 2



DETAILS

- MASONRY PLANTER WALL BY OTHERS

- CONCRETE BOTTOM FOOTING BY OTHERS

- 18" ENGINEERED SOIL LAYER

- 12" GRAVEL STORAGE LAYER

– 4" WASHED PEA-GRAVEL LAYER

*SEE NOTE BELOW

└── 6" PERFORATED PIPE (3" GRAVEL ABOVE, 3" GRAVEL BELOW) TO CONNECT TO BROOKS BOX

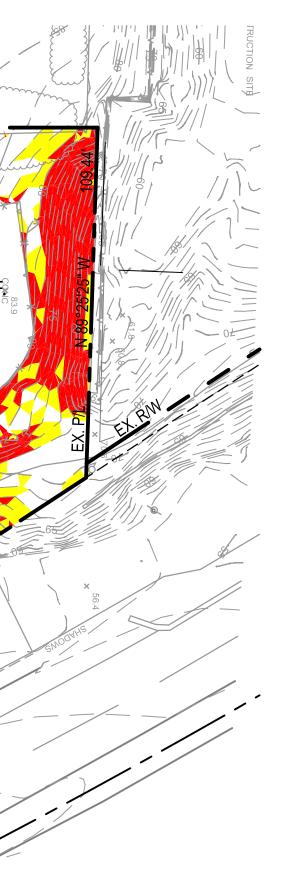
/-- 3" SHREDDED HARDWOOD MULCH

— 36" X 36" BROOKS BOX TG=FG+6" - PLANT MIX PER LANDSCAPE ARCHITECT PLAN

SHEET 8 OF 11



SLOPE ANALYSIS & LOT AREAS FENWAY N. HWY 101



SITE ADDRESS:

LEUCADIA 101 1950 NORTH COAST HWY 101 ENCINITAS, CA 92024 APN: 216-041-06, -20 & -21

SOURCE OF TOPOGRAPHY:

TOPOGRAPHY OBTAINED BY AERIAL SURVEY METHODS PERFORMED BY ACCULINE SURVEY, INC. DATED JULY 6, 2019

NATURAL SLOPE AREA REDUCTIONS:

TOTAL GROSS	s lot are	EA (R30 &	N-CRM-1) = 3.790 AC
SLOPES	0 - 25%	3.200 AC	(84.43% OF PROJECT SITE)
SLOPES	25 - 40%	0.208 AC	(5.48% OF PROJECT SITE)
SLOPES	40% +	0.382 AC	(10.09% OF PROJECT SITE)
ΤΟΤΑ	L SLOPES =	3.790 AC	

PER GEOTECHNICAL INVESTIGATION REPORT (DATED APRIL 27, 2020) BY NOVA SERVICES, INC., REVIEW OF HISTORICAL PHOTOS OF THE AREA AND ANALYSIS OF RECENT GRADING ACTIVITIES CONCLUDES THAT THE SLOPES STEEPER THAN 25% ON THE PROPERTY ARE THE DIRECT RESULT OF GRADING ACTIVITIES; THEREFORE ALL ONSITE SLOPES STEEPER THAN 25% ARE MANUFACTURED AND PARCEL NET ACREAGES WILL NOT BE REDUCED PURSUANT TO CITY MUNICIPAL CODE SECTION 30.16.10.B.2.

LOT AREA CALCULATIONS

PARCEL 1

GROSS LOT AREA (PARCEL 1)	= 30,096 SF
LESS PRIVATE ROAD EASEMENT*	= (0) SF
NET LOT AREA (PARCEL 1)	= 30,096 SF

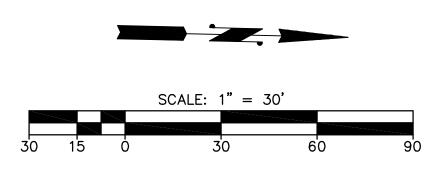
PARCEL 2

GROSS LOT AREA (PARCEL 2)	= 100,357 SF
GROSS LOT AREA (PARCEL 2) LESS PRIVATE ROAD EASEMENT*	= (0) SF
NET LOT AREA (PARCEL 2)	= 100,357 SF

PARCEL 3

	GROSS LOT AREA (PARCEL 3)	= 34,654 SF
$\backslash \backslash \rangle$	LESS PRIVATE ROAD EASEMENT*	= (0) SF
	NET LOT AREA (PARCEL 3)	= 34,654 SF

*PRIVATE ROAD EASEMENT FOR EMERGENCY VEHICLE ACCESS PURPOSES DOES NOT SUBTRACT FROM GROSS LOT AREA



SLOPE ANALYSIS & LOT AREAS FENWAY N. HWY 101 CITY OF ENCINITAS

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HILLSIDE INLAND BLUFF OVERLAY ANALYSIS FENWAY N. HWY 101

SHEET 10 OF 11

1950 NORTH COAST HWY 101 ENCINITAS, CA 92024 APN: 216-041-06, -20 & -21

SOURCE OF TOPOGRAPHY:

TOPOGRAPHY OBTAINED BY AERIAL SURVEY METHODS PERFORMED BY ACCULINE SURVEY, INC. DATED JULY 6, 2019

HILLSIDE INLAND BLUFF OVERLAY:

PURSUANT TO THE CITY OF ENCINITAS HILLSIDE / INLAND BLUFF OVERLAY ZONE (H/IBO GIS MAP (DATED FEBRUARY 2008), A PORTION OF PARCEL 1 IS LOCATED WITHIN THE CITY'S SPECIAL STUDY OVERLAY ZONE. THEREFORE PARCEL 1 HAS BEEN EVALUATED FOR HILLSIDE / INLAND BLUFF OVERLAY ZONE APPLICABILITY

PER CITY MUNICIPAL CODE SECTION 30.34.030, IF MORE THAN 10% OF PARCEL SLOPE EXCEED 25%, PARCEL IS SUBJECT TO HILLSIDE / INLAND BLUFF OVERLAY REGULATIONS AND SLOPES GREATER THAN 25% GRADE SHALL BE PRESERVED IN THEIR NATURAL STATE.

TOTAL GROSS LOT AREA (PARCEL 1) = 0.691 AC

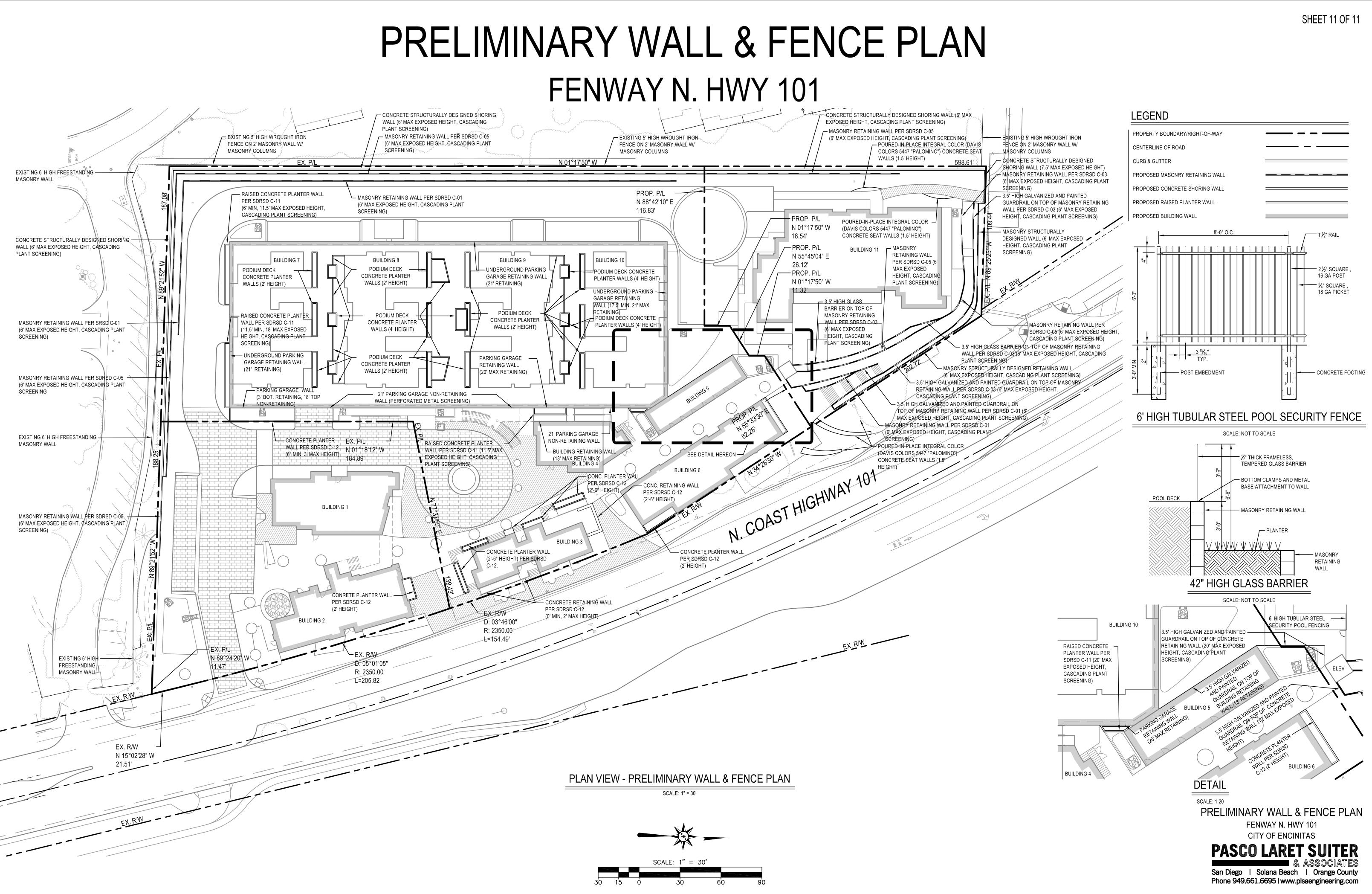
SLOPES	0 - 25%	0.475 AC	(68.79% OF PROJECT SITE)
SLOPES	25 - 40%	0.054 AC	(7.82% OF PROJECT SITE)
SLOPES	40% +	0.162 AC	(23.39% OF PROJECT SITE)
TOT	AL SLOPES =	0.691 AC	

PER GEOTECHNICAL INVESTIGATION REPORT (DATED APRIL 27, 2020) BY NOVA SERVICES, INC., REVIEW OF HISTORICAL PHOTOS OF THE AREA AND ANALYSIS OF RECENT GRADING ACTIVITIES CONCLUDES THAT THE SLOPES STEEPER THAN 25% ON PARCEL 1 ARE THE DIRECT RESULT OF GRADING ACTIVITIES; THEREFORE ALL ONSITE SLOPES ON PARCEL 1 STEEPER THAN 25% ARE MANUFACTURED AND THE HILLSIDE / INLAND BLUFF OVERLAY REGULATIONS DO NOT APPLY TO PARCEL 1.

> HILLSIDE INLAND BLUFF **OVERLAY ANALYSIS** FENWAY N. HWY 101 CITY OF ENCINITAS

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