

Geotechnical Exploration, Inc.

SOIL AND FOUNDATION ENGINEERING • GROUNDWATER • ENGINEERING GFOLOGY

28 March 2018

All Right Storage, Inc. 11300 Sorrento Valley Road, Suite 250

San Diego, CA 92121 Attn: Mr. Olivier Andreu Job No. 14-10558.1

Subject:

Update Geotechnical Investigation and Infiltration Testing

Cottonwood Self Storage Project

Northwest of State Route 52 and Cottonwood Ave

Santee, California

APN Nos. 383-112-05-00 and 28-00

Dear Mr. Andreu:

In accordance with your request, *Geotechnical Exploration, Inc.* has performed an update geotechnical investigation and infiltration testing for the subject project (see Vicinity Map, Figure No. I). We previously performed a preliminary geotechnical investigation for a previously planned project at the site, the results of which were presented in our report dated July 10, 2014. It is our understanding, based on preliminary plans provided to us, that the presently proposed development of the currently vacant 6-acre site will include 3 single-story and 2 three-story self storage buildings with pavements and other associated improvements (see Figure Nos. II and III). The objectives of this update investigation were to evaluate the depth of existing undocumented fill soils along the western and northern property boundaries and to provide supplemental grading and foundation recommendations as needed. The objective of the infiltration testing was to evaluate the subsurface soil infiltration rates in an area that may be used for an infiltration basin.

SCOPE OF SERVICES

Based on the above information, the update geotechnical investigation and infiltration testing consisted of the following:

- 1. A field subsurface exploration program which consisted of the excavation of six exploratory test pits along the western and northern property boundaries and near the northeast corner of the property to evaluate the depth of existing undocumented fills that require removal and recompaction. The work was performed under the direction of our geologist who supervised, logged and sampled the excavations. In addition, two exploratory excavations were made to perform infiltration testing in a potential area for a storm water infiltration basin. The proposed infiltration testing was conducted in accordance with the City of Santee BMP Design Manual, Appendix C (Geotechnical and Groundwater Investigation Requirements) and Appendix D (Approved Infiltration Rate Assessment Methods).
- 2. Laboratory testing of samples obtained from the excavations to assist in classification of the materials and to help evaluate the index, strength, compressibility, and expansion properties of the soils encountered.
- 3. Geotechnical engineering analysis and evaluation of the resulting field and laboratory test data.
- 4. Preparation of this update geotechnical investigation report presenting the results of our study along with updated design and construction recommendations for the site grading, building foundations, and slab on-



grade construction as warranted. In addition, our infiltration rate findings are provided.

FIELD INVESTIGATION

The field investigation consisted of a surface reconnaissance and a subsurface exploration program using a rubber tired backhoe to investigate and sample the subsurface soils. On March 7, 2018, five exploratory test pits were excavated along the western and northern property boundaries where single-story buildings are planned close to those boundaries, and one exploratory test pit was excavated near the northeast corner of the property where a boring for the previous investigation (Boring B-2) had encountered debris-laden fill to a depth of about 8 feet. The test pits were excavated to a maximum depth of 5 feet. The soils encountered in the exploratory excavations were continuously logged in the field by our geologist and described in accordance with the Unified Soil Classification System (refer to Appendix A). The approximate locations of the exploratory excavations are shown on the Plot Plan, Figure No. IV.

In addition, two exploratory excavations were also made on March 7, 2018, to a maximum depth of $6\frac{1}{2}$ feet in order to perform infiltration testing.

Representative samples were obtained from the exploratory excavations at selected depths appropriate to the investigation. All samples were returned to our laboratory for evaluation and testing.

Exploratory excavation logs have been prepared on the basis of our observations and laboratory test results. Logs of the exploratory test pits and infiltration testing excavations are attached as Figure Nos. Va-h.



SOIL DESCRIPTION

Existing fill soils consisting of loose to medium dense clayey sands were encountered in exploratory Test Pits 1 through 5 to depths of 1 to 3 feet. In Test Pit 6, the fill soils extended to a depth of 4 feet. Existing fill soils, consisting of loose to medium dense clayey sands were also encountered in the infiltration excavations to a depth of 2 feet. The fill soils encountered in Test Pits 1 through 3 appear to be retaining wall backfill along the western property boundary and it appears that the retaining wall is founded on older alluvial soils. The materials encountered beneath the fill soils in all of the exploratory excavations consisted of older alluvial materials, comprised of dense silty and clayey sands, to the maximum depth explored of $6\frac{1}{2}$ feet.

The exploratory test pit and infiltration excavation logs and related information depict subsurface conditions only at the specific locations shown on the site plan and on the particular date designated on the logs. Subsurface conditions at other locations may differ from conditions occurring at these locations. Also, the passage of time may result in changes in the subsurface conditions due to environmental changes.

CONCLUSIONS AND RECOMMENDATIONS

Based on review of our previous investigation at the site, as well as the presently proposed development plans, it is our opinion that the conclusions and recommendations presented in our previous preliminary geotechnical investigation report dated July 10, 2014, remain applicable for the proposed site development with the following exceptions.



- Building Pad Grading: We recommend that the grading for the building pads consist of removal and recompaction of the existing fill soils or to a depth of 3 feet below the pad subgrade levels, whichever is deeper.
- 2. <u>Building Foundations:</u> It is our understanding that the proposed buildings will be supported on mat foundations rather than shallow footing foundations. We recommend that a subgrade modulus (K_{v1}) of 160 tons per cubic foot be used for the mat designs.
- 3. <u>Seismic Design:</u> The seismic design parameters presented in our 2014 report cited the 2013 CBC and the presently proposed project will be constructed in accordance with the 2016 CBC. In that the 2013 and 2016 CBC both utilize ASCE 7-10 for the determination of seismic design parameters, the previously presented parameters remain applicable.

INFILTRATION TESTING

We performed simple open pit falling head testing at two locations on the property at depths of 78 inches at INF-1, and 54 inches at INF-2. Falling head measurements were collected at regular time intervals for a period of 3 hours. The tests were performed per the requirements of the City of Santee Storm Water Standards, BMP Design Manual, in accordance with Appendix D. Both tests were performed in the older alluvial materials underlying the site at shallow depths. Laboratory test results at infiltration test locations INF-1 and INF-2, indicate 42 percent and 56 percent of the soils passed the #200 sieve, respectively.

Testing at location INF-1 revealed a falling head rate of 480 minutes/inch. The testing at INF-2 showed no measurable head drop in the last hour. The simple



open pit falling head test rate results for INF-1 and INF-2 have been converted to infiltration rates using the Porchet Method and indicate infiltration rates of 0.062-and 0.000-inch/hour, respectively, without a factor of safety applied. Refer to Appendix A for the simple open pit test rate results and simple open pit to infiltration rate calculations. Review of the USDA Web Soil Survey Map indicates the site has been assigned to hydrologic soil group (HSG) D. Refer to Appendix B for USDA Web Soil Survey Map.

Based on the results of our simple open pit falling head testing and evaluation of the infiltration rates, it is our professional opinion that the site is not suitable for infiltration BMPs. We also recommend that any bio remediation features be lined with an impermeable liner and drained to the storm drain system.

Our services consist of professional opinions and recommendations made in accordance with generally accepted geotechnical engineering principles and practices. This warranty is in lieu of all other warranties either expressed or implied.

This opportunity to be of service is sincerely appreciated. If you have any questions concerning this matter, please contact our office. Reference to our **Job No. 14-10558.1** will help to expedite a response to your inquiries.

Respectfully submitted,

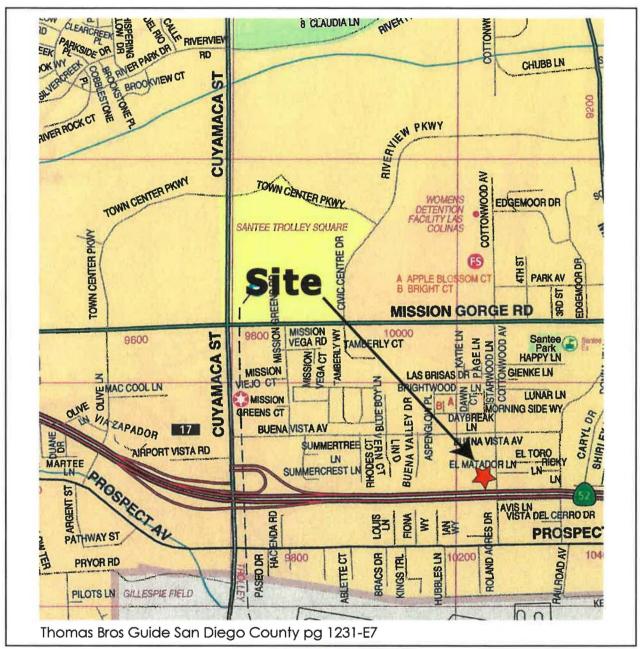
GEOTECHNICAL EXPLORATION, INC.

Wm. D. Hespeler, G.E. 396 Senior Geotechnical Engineer





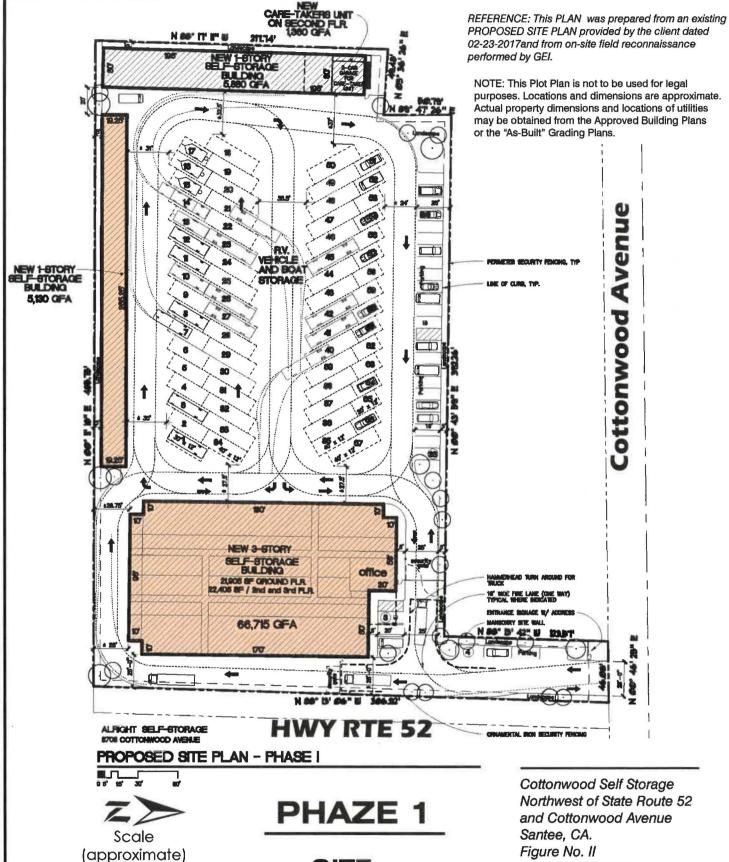
VICINITY MAP



Cottonwood Self Storage Northwest of State Route 52 and Cottonwood Avenue Santee, CA.

> Figure No. I Job No. 14-10558.1





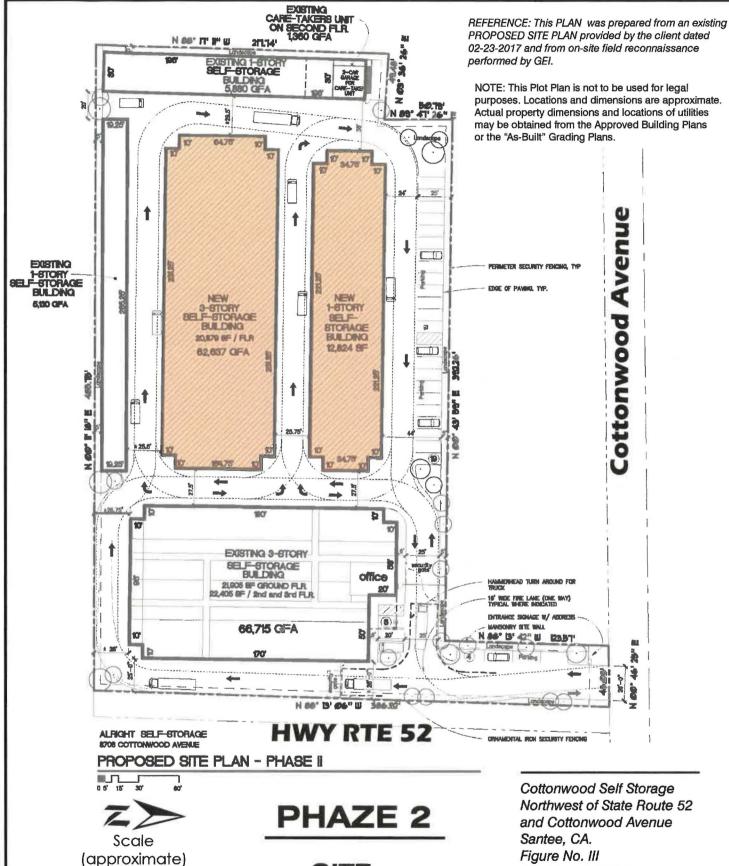
SITE

DEVELOPMENT

PLAN

Figure No. II Job No. 14-10558.1 Geotechnical Exploration, Inc.





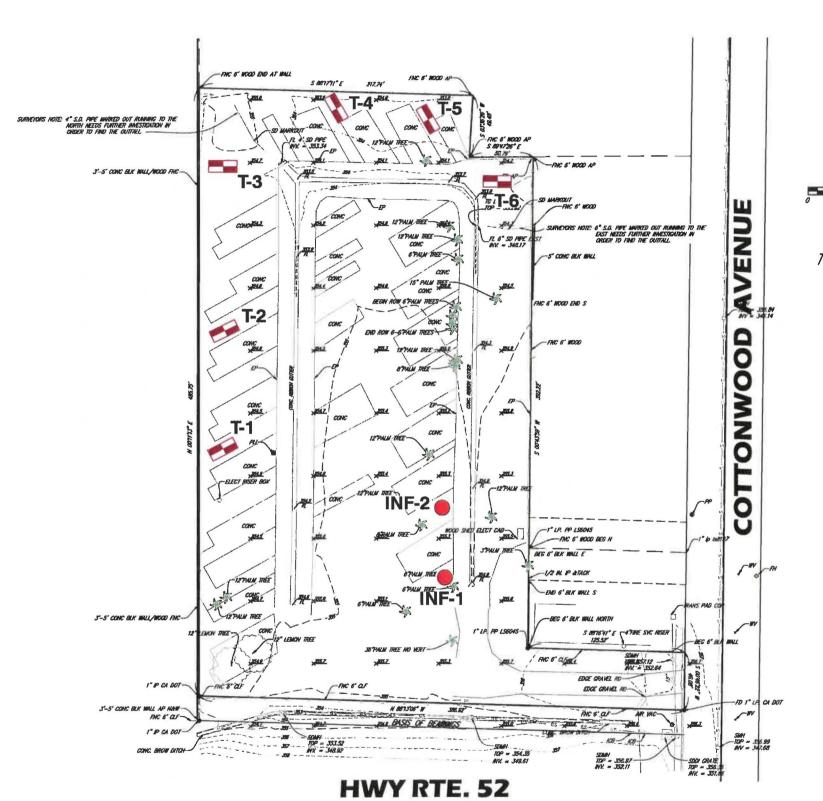
SITE DEVELOPMENT PLAN

Figure No. III Job No. 14-10558.1



Geotechnical Exploration, Inc.

March 2018



Scale: 1" = 80'
(approximate)

TOPOGRAPHIC SURVEY



LEGEND



Approximate Location of Exploratory Test Pit



Approximate Location of Infiltration Test Excavation

SITE PLAN

Cottonwood Self Storage Northwest of State Route 22 and Cottonwood Avenue Santee, CA. Figure No. IV Job No. 14-10558.1



Geotechnical Exploration, Inc.

(March 2018)

REFERENCE: This Plot Plan was prepared from an existing TOPOGRAPHIC SURVEY provided by EXCEL ENGINEERING dated December 22, 2017 and from on-site field reconnaissance performed by GEI.

NOTE: This Plot Plan is not to be used for legal

or the "As-Built" Grading Plans.

purposes. Locations and dimensions are approximate.

Actual property dimensions and locations of utilities

may be obtained from the Approved Building Plans

± 354.5' Mean Sea Level	Not Encountered	JAB			
SURFACE ELEVATION	GROUNDWATER/ SEEPAGE DEPTH	LOGGED BY			
Rubber-tire Backhoe	12' X 2' X 4' Trench	3-7-18			
EQUIPMENT	DIMENSION & TYPE OF EXCAVATION	DATE LOGGED			

	DEPTH (feet)	SYMBOL	SAMPLE	FIELD DESCRIP AND CLASSIFICATI DESCRIPTION AND REMARKS (Grain size, Density, Moisture, Color)		U.S.C.S.	IN-PLACE MOISTURE (%)	IN-PLACE DRY DENSITY (pcf)	OPTIMUM MOISTURE (%)	MAXIMUM DRY DENSITY (pcf)	DENSITY (% of M.D.D.)	EXPAN. + (%)	BLOW COUNTS/FT.	SAMPLE O.D. (INCHES)
	1-			CLAYEY SAND , fine- to medium glass and concrete debris. Loos dense. Moist. Dark red-brown. FILL (Qaf)	-grained, some e to medium	SC								
	2 -	1		SILTY SAND , fine- to medium-gradense. Slightly moist. Light brow	n.	SM								
	3 -			CLAYEY SAND , fine- to medium Dense. Moist. Dark red-brown. OLDER ALLUVIUM		SC								
5.GPJ GEO_EXPL.GDT 3/27/18	5 -			Bottom @ 4'										
TONWOOL		Y	PE	RCHED WATER TABLE	JOB NAME Cottonwood Sel	f Stor	age P	roject						
558.1 CO		_		LK BAG SAMPLE	SITE LOCATION NW of SR 52 and				San	tee. CA				
06 105				PLACE SAMPLE	JOB NUMBER		-	EWED BY		3/WDH	LOG	No.		
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± 354.5' Mean Sea Level	Not Encountered	JAB	
SURFACE ELEVATION	GROUNDWATER/ SEEPAGE DEPTH	LOGGED BY	
Rubber-tire Backhoe	12' X 2' X 4' Trench	3-7-18	
EQUIPMENT	DIMENSION & TYPE OF EXCAVATION	DATE LOGGED	

	DEPTH (feet)	SYMBOL	SAMPLE	FIELD DESCRIP AND CLASSIFICATI DESCRIPTION AND REMARKS (Grain size, Density, Moisture, Color)		U.S.C.S.	IN-PLACE MOISTURE (%)	IN-PLACE DRY DENSITY (pcf)	OPTIMUM MOISTURE (%)	MAXIMUM DRY DENSITY (pcf)	DENSITY (% of M.D.D.)	EXPAN. + (%)	BLOW COUNTS/FT.	SAMPLE O.D. (INCHES)
	1.			CLAYEY SAND , fine- to medium glass, brick, tile and concrete de medium dense. Moist. Dark red-	bris. Loose to	SC								
	2-			@2'- electric, water and sewer trench. CLAYEY SAND , fine- to medium Dense. Slightly moist to moist. D OLDER ALLUVIUM	-grained. ark red-brown.	SC								
EXPLORATION LOG 10558.1 COTTONWOOD.GPJ GEO_EXPL.GDT 3/27/18	5 -			Bottom @ 4'										
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EXPLORATION LOG 10		s	MC NU	PLACE SAMPLE DDIFIED CALIFORNIA SAMPLE ICLEAR FIELD DENSITY TEST ANDARD PENETRATION TEST	JOB NUMBER 14-10558.1 FIGURE NUMBER Vb			EWED BY	JAE	3/WDH nical ion, inc.	LOG	No.	-2	

▼ PERCHED WATER TABLE	JOB NAME Cottonwood Self Store	age Project	
☑ BULK BAG SAMPLE	SITE LOCATION		
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MODIFIED CALIFORNIA SAMPLE	JOB NUMBER	REVIEWED BY JAB/WDH	LOG No.
S NUCLEAR FIELD DENSITY TEST	14-10558.1 FIGURE NUMBER	Geotechnical Exploration, Inc.	T-2
STANDARD PENETRATION TEST			

± 354.5' Mean Sea Level	Not Encountered	JAB			
SURFACE ELEVATION	GROUNDWATER/ SEEPAGE DEPTH	LOGGED BY			
Rubber-tire Backhoe	12' X 2' X 4' Trench	3-7-18			
EQUIPMENT	DIMENSION & TYPE OF EXCAVATION	DATE LOGGED			

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	2 -	No.VA		 electrical conduit on west end CLAYEY SAND , fine- to medium 		SC								
				caliche. Dense. Slightly moist to red-brown.	moist. Dark	30								
	-			OLDER ALLUVIUM	(Qoa)									
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EQUIPMENT	DIMENSION & TYPE OF EXCAVATION	DATE LOGGED	
Rubber-tire Backhoe	10' X 2' X 4' Trench	3-7-18	
SURFACE ELEVATION	GROUNDWATER/ SEEPAGE DEPTH	LOGGED BY	
± 354' Mean Sea Level	Not Encountered	JAB	

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	-	(A) 4/4/4		trash debris. Loose to medium or red-brown.	lense. Moist. Dark									
	-	Wary												
	-	(IN)		FILL (Qaf)										
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	3 -	10)4	-	CLAYEY SAND , fine- to medium	-arained some	SC								
	-			caliche. Dense. Moist. Dark red-	brown.	30								
	_			OLDER ALLUVIUM	(Qoa)									
	_			gas/water line @ 3.5' on north	end of trench.									
	4 -	9/3/	-											
	_													
	_			Bottom @ 4'										
m	_													
3/27/18	5 -													
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EQUIPMENT	DIMENSION & TYPE OF EXCAVATION	DATE LOGGED	
Rubber-tire Backhoe	11' X 2' X 3' Trench	3-7-18	
SURFACE ELEVATION	GROUNDWATER/ SEEPAGE DEPTH	LOGGED BY	
± 354' Mean Sea Level	Not Encountered	JAB	

	DEPTH (feet)	SYMBOL	SAMPLE	FIELD DESCRIP AND CLASSIFICATI DESCRIPTION AND REMARKS (Grain size, Density, Moisture, Color)		U.S.C.S.	IN-PLACE MOISTURE (%)	IN-PLACE DRY DENSITY (pcf)	OPTIMUM MOISTURE (%)	MAXIMUM DRY DENSITY (pcf)	DENSITY (% of M.D.D.)	EXPAN. + (%)	BLOW COUNTS/FT.	SAMPLE O.D. (INCHES)
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00 1)	1			CLAYEY SAND , fine- to medium pinhole voids, some caliche. De to moist. Dark red-brown. OLDER ALLUVIUM	nse. Slightly moist	SC								
	3 -			electric line @ 2.5' on south er	nd of trench.									
	4 -			Bottom @ 3'										
D.GPJ GEO EXPL.GDT 3/27/18	5 -													
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558.1 CO				LK BAG SAMPLE	SITE LOCATION NW of SR 52 and				San	tee. CA				
EXPLORATION LOG 10558.1 COTTONWOOD.GPJ			MC	PLACE SAMPLE DDIFIED CALIFORNIA SAMPLE	JOB NUMBER 14-10558.1		7	EWED BY	JAE	s/WDH	LOG	_	_	
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± 354' Mean Sea Level	Not Encountered	JAB
SURFACE ELEVATION	GROUNDWATER/ SEEPAGE DEPTH	LOGGED BY
Rubber-tire Backhoe	17' X 2' X 5' Trench	3-7-18
EQUIPMENT	DIMENSION & TYPE OF EXCAVATION	DATE LOGGED

DEDTU (foot)	SYMBOL	SAMPLE	FIELD DESCRIF AND CLASSIFICAT DESCRIPTION AND REMARKS (Grain size, Density, Moisture, Color)		U.S.C.S.	IN-PLACE MOISTURE (%)	IN-PLACE DRY DENSITY (pcf)	OPTIMUM MOISTURE (%)	MAXIMUM DRY DENSITY (pcf)	DENSITY (% of M.D.D.)	EXPAN. + (%)	BLOW COUNTS/FT.	SAMPLE O.D. (INCHES)
3/27/1/8			CLAYEY SAND , fine- to medium gravel, some brick and metal pip dense. Moist. Dark red-brown. FILL (Qaf) CLAYEY SAND , fine- to medium Dense. Slightly moist to moist. Find the composition of the composi	n-grained, trace be debris. Medium n-grained. Red-brown.	SC								
EXPLORATION LOG 10558.1 COTTONWOOD, GPJ GEO_EXPL.GDT		B IN M	ERCHED WATER TABLE JLK BAG SAMPLE I-PLACE SAMPLE ODIFIED CALIFORNIA SAMPLE UCLEAR FIELD DENSITY TEST TANDARD PENETRATION TEST	JOB NAME Cottonwood Self SITE LOCATION NW of SR 52 and JOB NUMBER 14-10558.1 FIGURE NUMBER Vf		onwo	od Ave	JAE	tee, CA B/WDH nical lon, Inc.	LOG	No.	-6	

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1	IN-PLACE SAMPLE	
	MODIFIED CALIFORNIA SAMPLE	
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	STANDARD PENETRATION TEST	

	JOB NAME Cottonwood Self St	torage Project	
	SITE LOCATION NW of SR 52 and Co	ottonwood Ave., Santee, CA	
	JOB NUMBER	REVIEWED BY JAB/WDH	LOG No.
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EQUIPMENT	DIMENSION & TYPE OF EXCAVATION	DATE LOGGED	
Rubber-tire Backhoe	12' X 4' X 6.5' Trench	3-7-18	
SURFACE ELEVATION	GROUNDWATER/ SEEPAGE DEPTH	LOGGED BY	
± 355' Mean Sea Level	Not Encountered	JAB	

	DEPTH (feet)	SYMBOL	SAMPLE	FIELD DESCRIP AND CLASSIFICATI DESCRIPTION AND REMARKS (Grain size, Density, Moisture, Color)		U.S.C.S.	IN-PLACE MOISTURE (%)	IN-PLACE DRY DENSITY (pcf)	OPTIMUM MOISTURE (%)	MAXIMUM DRY DENSITY (pcf)	DENSITY (% of M.D.D.)	EXPAN. + (%)	BLOW COUNTS/FT.	SAMPLE O.D. (INCHES)
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	1=			FILL (Qaf)										
	2 -	000		electric lines exposed in northwaternch sewer line exposed in southweternch. CLAYEY SAND, fine- to medium	est corner of	SC								
	3 -			Dense. Slightly moist to moist. R OLDER ALLUVIUM										
	4 -													
	5 -													
	6 -			Infiltration test conducted from 6	' to 6.5'.									
∞	-		1	42% passing #200 sieve.										
EXPLORATION LOG 10558.1 COTTONWOOD.GPJ GEO EXPL.GDT 3/27/18	7 -			Bottom @ 6.5'										
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0558.1		_		LK BAG SAMPLE PLACE SAMPLE	NW of SR 52 and	Cott	onwo	od Ave	., San	tee, CA	4			
100 1		<u></u>		DDIFIED CALIFORNIA SAMPLE	JOB NUMBER			EWED BY	JAE	3/WDH	LOG	No.		
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EQUIPMENT	DIMENSION & TYPE OF EXCAVATION	DATE LOGGED
Rubber-tire Backhoe	12' X 4' X 4' Trench	3-7-18
SURFACE ELEVATION	GROUNDWATER/ SEEPAGE DEPTH	LOGGED BY
± 355' Mean Sea Level	Not Encountered	JAB

DEPTH (feet)	SYMBOL	SAMPLE	FIELD DESCRIP AND CLASSIFICATI DESCRIPTION AND REMARKS (Grain size, Density, Moisture, Color)	ON	U.S.C.S.	IN-PLACE MOISTURE (%)	IN-PLACE DRY DENSITY (pcf)	OPTIMUM MOISTURE (%)	MAXIMUM DRY DENSITY (pcf)	DENSITY (% of M.D.D.)	EXPAN. + (%) CONSOL	BLOW COUNTS/FT.	SAMPLE O.D. (INCHES)
1 -		-	SILTY SAND , fine- to medium-g Slightly moist. Gray. FILL (Qaf) CLAYEY SAND , fine- to medium asphalt and concrete debris. Me Slightly moist to moist. Red-brow FILL (Qaf)	n-grained, trace edium dense.	SM								
3 -		X	CLAYEY SAND/ SANDY CLAY , the medium-grained, some mangand some pinhole voids, some calich stiff. Slightly moist to moist. Dark OLDER ALLUVIUM Infiltration test conducted from 3 56% passing #200 sieve.	ese staining, ne. Dense/very red-brown. (Qoa)	SC/ CL								
EXPLORATION LOG 10558.1 COTTONWOOD.GPJ GEO EXPL.GDT 3/27/18	-		Bottom @ 4'										
10558.1 COTTONWOC	_	ВU	RCHED WATER TABLE LK BAG SAMPLE PLACE SAMPLE	JOB NAME Cottonwood Self SITE LOCATION NW of SR 52 and	onwo	od Ave	., San	Santee, CA		\ \			
EXPLORATION LOG	s	NU	DDIFIED CALIFORNIA SAMPLE ICLEAR FIELD DENSITY TEST ANDARD PENETRATION TEST	JOB NUMBER 14-10558.1 FIGURE NUMBER Vh		REVI	EWED BY		B/WDH nical lon, Inc.	LOG	No.	2	2

Ā	PERCHED WATER TABLE	JOB NAME Cottonwood Self Stora	age Project	
	BULK BAG SAMPLE	SITE LOCATION		
1	IN-PLACE SAMPLE	NW of SR 52 and Cotte	onwood Ave., Santee, CA	
	MODIFIED CALIFORNIA SAMPLE	JOB NUMBER	REVIEWED BY JAB/WDH	LOG No.
s	NUCLEAR FIELD DENSITY TEST	14-10558.1 FIGURE NUMBER	Geotechnical Exploration, Inc.	INF-2
	STANDARD PENETRATION TEST			

APPENDIX A



Simple Open Pit Falling Head Test Sheet

Project Name: Cottonwood Self Storage

Project No. 14-10558.1 Date Excavated: 3/7/18 Test Hole No: INF-1 **Tested By: JAB**

Soil Classification: SC/CL Depth of Test Hole: 78"

Test Hole Dia: 24"

Initial Time (Minutes)	Final Time (Minutes)	Time interval (minutes)	Initial Water Level (inches)	Final Water Level (inches)	Change in water (inches)	Falling Head Rate (min/inches)
925	1025	60	71.500	71.750	0.250	240.000
1025	1125	60	71.750	71.875	0.125	480.000
1125	1225	60	71.875	72.000	0.125	480.000

Simple Open Pit Falling Head Test Sheet

Project Name: Cottonwood Self Storage

Project No. 14-10558.1 Date Excavated: 3/7/18 Test Hole No: INF-2 **Tested By: JAB**

Soil Classification: SC/CL Depth of Test Hole: 54" Test Hole Dia: 24"

Initial Time (Minutes)	Final Time (Minutes)	Time interval (minutes)	Initial Water Level (inches)	Final Water Level (inches)	Change in water (inches)	Falling Head Rate (min/inches)
942	1042	60	47.125	47.375	0.250	240.000
1042	1142	60	47.375	47.500	0.125	480.000
1142	1242	60	47.500	47.500	0.000	#DIV/0!

Simple Open Pit Rate to Infiltration Rate Conversion (Porchet Method)

Project Name: Cottonwood Self Storage

Calculated By: JAB

Date: 3/19/2018

Project No. 14-10558.1

Checked By:

Date:

Test Hole No: INF-1

Test Hole Dia: 24"

Depth of Test Hole: 78"

Porchet Corrections

Infiltration rate=((delta h*60r)/(delta t*(r+2 h avg))

	EB Depth	Delta T	Water Depth	Water Depth	h 1	h 2	delta h	h avg	r (radius)	<u>delta</u>	delta t*(r+2 h	Infiltration
No.	(inches)	(min)	1 (inches)	2 (inches)	(inches)	(inches)	(inches)	(inches)	(inches)	<u>h*60r</u>	avg)	rate (in/hr)
1	78	60	71.500	71.750	6.500	6.250	0.250	6.375	12	180	1485	0.121
2	78	60	71.750	71.875	6.250	6.125	0.125	6.188	12	90	1462.5	0.062
3	78	60	71.875	72.000	6.125	6.000	0.125	6.063	12	90	1447.5	0.062
4												
5												
6												
7												
8												
9												

Simple Open Pit Rate to Infiltration Rate Conversion (Porchet Method)

Project Name: Cottonwood Self Storage

Calculated By: JAB

Date: 3/19/2018

Project No. 14-10558.1

Checked By:

Date:

Test Hole No: INF-2

Test Hole Dia: 24"

Depth of Test Hole: 54"

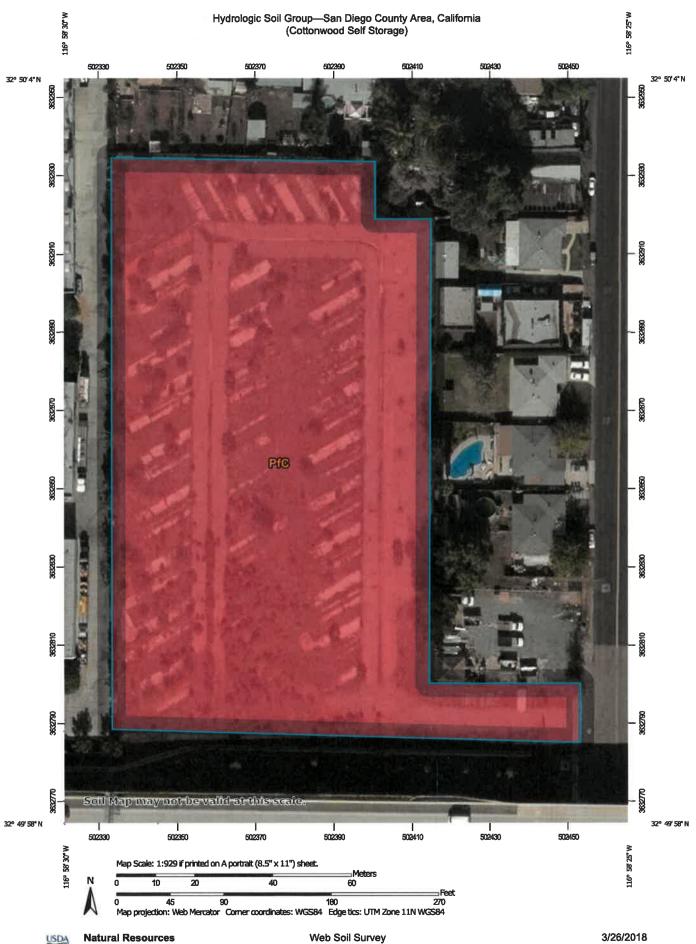
Porchet Corrections

Infiltration rate=((delta h*60r)/(delta t*(r+2 h avg))

Test	EB Depth	Delta T	Water Depth	Water Depth	h 1	h 2	delta h	h avg	r (radius)	<u>delta</u>	delta t*(r+2	Infiltration rate
No.	(inches)	(min)	1 (inches)	2 (inches)	(inches)	(inches)	(inches)	(inches)	(inches)	<u>h*60r</u>	h avg)	(in/hr)
1	54	60	47.125	47.375	6.875	6.625	0.250	6.750	12	180	1530	0.118
2	54	60	47.375	47.500	6.625	6.500	0.125	6.563	12	90	1507.5	0.060
3	54	60	47.500	47.500	6.500	6.500	0.000	6.500	12	0	1500	0.000
4												
5												
6												
7												
8												
9												

APPENDIX B





MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of interest (AOI) С 1:24,000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D **Soil Rating Polygons** Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil **Water Features** line placement. The maps do not show the small areas of A/D Streams and Canals contrasting soils that could have been shown at a more detailed В scale. **Transportation** B/D Rails Please rely on the bar scale on each map sheet for map C measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service **US Routes** Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available Local Roads Maps from the Web Soil Survey are based on the Web Mercator Soil Rating Lines projection, which preserves direction and shape but distorts Background distance and area. A projection that preserves area, such as the **Aerial Photography** Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. B/D Soil Survey Area: San Diego County Area, California C Survey Area Data: Version 12, Sep 13, 2017 Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. Not rated or not available Date(s) aerial images were photographed: Dec 7, 2014—Jan 4, 2015 **Soll Rating Points** The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. В B/D

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
PfC	Placentia sandy loam, thick surface, 2 to 9 percent slo pes	D	3.0	100.0%
Totals for Area of Interest			3.0	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher