PRELIMINARY STORMWATER QUALITY REPORT ORTEGA PARK ON-SITE

SANTA BARBARA, CA

February 27, 2020



PREPARED FOR: City of Santa Barbara, Parks and Recreation

PREPARED BY: Michael C. Hamilton, P.E.



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PURPOSE OF REPORT

The purpose of this report is to assess the project site, identify pre-development and post-development drainage conditions, and identify storm water facilities to meet the requirements of the City of Santa Barbara's Storm Water BMP Guidance Manual. The project consists of the demolition of three existing structures, hardscape and a pool at the site and the construction of two new buildings, a pool, a skate park, a basketball court, play areas and a synthetic turf sports field.

LOCATION

The 4.68 acre park is located on the city block bound by North Quarantina Street, East Cota Street, North Salsipuedes Street, and East Ortega Street. See the project vicinity map in Figure 1 below.



Figure 1. Project Vicinity Map

BACKGROUND

Slopes on the majority of the site are approximately 1-4% percent. Runoff from the site flows from the northwest to the southeast, where it enters the City storm drain system through inlets on North Quarantina Street and East Cota Street. There are no offsite flows directly impacting the project site, so the drainage boundary for this analysis will follow the property line as depicted in the Existing Hydrology Map, Exhibit 1. A separate stormwater quality report addresses the proposed improvements within the public right-of-way as these improvements are part of a separate phase of construction.

METHOD OF ANALYSIS

The approach to analyze the runoff from the project site follows the City of Santa Barbara's Storm Water BMP Guidance Manual. The analysis is a comparison of the pre-project condition to the post-project condition for both hydrologic analysis and storm water quality.

The proposed project will increase the impervious area at the site from 15.9% to approximately 29.5%. See the Proposed Hydrology Map, Exhibit B and the Drainage Area Summary, Attachment I. Table I provides a summary of the proposed changes in impervious area. This increase in impervious area will cause an increase in the amount of storm water peak runoff from the site, requiring BMP's to be designed to both retain and detain storm water as outlined in the City's BMP Guidance Manual.

Proposed Area	Definition	Area (SF)
New Impervious	Area where new impervious area (hardscape, roof, etc.) is proposed where there is existing pervious area (landscaping, etc.)	39,103
Replaced Impervious	Area where new impervious area (hardscape, roof, etc.) is proposed where there is currently existing impervious area (hardscape, roof, etc.)	21,011
Removed Impervious	Area where new pervious area (landscaping, etc.) is proposed where there is currently existing pervious area (landscaping, etc.)	11,559

Table 1. Changes in Impervious Area

Proposed Drainage Management Areas

The proposed project site has been divided into two drainage management areas (DMA): 'A', and 'B'. See Proposed Hydrology Map, Exhibit 2.

- DMA 'A' consists of the northeastern half of the project site where a synthetic turf field is
 proposed. The turf field will be designed with a drainage layer to retain and infiltrate the oneinch storm. The DMA is 99.8% pervious, with two small concrete walkways that will drain to
 adjacent turf. Therefore, no treatment is required for this portion of the project site. Runoff will
 flow to the east and be collected by a series of existing inlets lining the existing bike path on
 Quarantina Street.
- DMA 'B' consists of the southwestern half of the site where the majority of impervious development will occur. Proposed storm drain inlets throughout the DMA will collect runoff and outlet into a series of Cultec stormwater retention chambers below the proposed basketball court. The Cultec chambers are designed to retain and infiltrate the volume of runoff from the one-inch storm and provide detention for the two- through 25-year storms. The

Cultec chambers will outlet to the municipal storm drain system through a connection to the existing curb inlet at the northern corner of the Cota Street and Salsipuedes Street intersection.

Infiltration Testing

Infiltration testing for the site was performed in February of 2019 by Earth Systems Pacific. Four infiltration borings were hand-excavated throughout the site to depths varying from 2.5 to 3.5 feet. Two of the four test borings were not tested for infiltration rates due to encountered shallow groundwater. The remaining two borings yielded infiltration rates of 0.6 and 1.4 inches per hour. See Infiltration Testing Report, Attachment 4.

PEAK FLOW DETENTION

The proposed Cultec stormwater chambers were designed to decrease the post-project peak flow of runoff to that of the pre-project for the 2-year through 25-year storm events. The program HydroCAD was used to determine the volume of runoff and the peak flow of runoff from the project site for various storm events for both pre- and post-project conditions, see HydroCAD output in Attachment 2. The results are summarized in Table 2.

Table 2. Peak Flow Summary

Storm	Peak Flows (CFS)				
Event	Existing	Proposed			
2-year	3.83	3.79			
5-year	6.90	6.52			
10-year	9.06	8.42			
25-year	11.78	10.92			

The curve number (CN) used for the synthetic turf was calculated using the SCS equation for potential maximum retention (S, in inches). A gravel section of 8.5" with 32% voids gives a maximum retention depth of

$$8.5" \times 0.32 = 2.72"$$

Thus, the curve number is

$$CN = \frac{1000}{S+10} = \frac{1000}{2.72+10} = 78.6 \cong 79$$

VOLUME RETENTION

Per the City's BMP Guidance Manual, the project is required to retain on-site the volume difference between pre- and post-development conditions for the 25-year storm or the one-inch storm, whichever is larger. For this project the one-inch storm event volume difference of 4,170ft³ is larger as seen in Table 3 below. Retention is provided by the proposed Cultec stormwater chambers and the proposed synthetic turf field, see Table 4. Calculations for the synthetic turf retention and a stage-storage chart for the Cultec chambers are provided in Attachment 3. Exhibit 3 provides BMP Cross-Section Details.

Storm	Runoff Volume (CF) (Before Retention)				
Event	Existing	Proposed			
l-inch	2,826	496			
2-year	24,669	21,943			
5-year	42,940	39,543			
10-year	55,873	52,022			
25-year	72,260	67,901			

Table 3. Runoff Volume Summary

Table 4. Volume Retention Summary

Retention BMP	Depth of Gravel Storage (in)	Area of Storage (SF)	Provided Retention Volume (CF)	
Synthetic Turf	4	93,452	13,706	
Cultec Chambers	-	2,884	3,597	
Total			10,450	

STORM WATER QUALITY

The City of Santa Barbara Storm Water BMP Guidance Manual was used to design storm water quality features to treat the one-inch 24-hour storm. The proposed Cultec stormwater chambers will provide treatment by infiltrating the full retention volume described above. A stage-storage chart for the Cultec chambers is provided in Attachment 3. Exhibit 3 provides BMP Cross-Section Details.

CONCLUSIONS

Based on the findings of this report, the proposed drainage design for this project meets the applicable standards and requirements for the City of Santa Barbara. The proposed drainage plan is consistent with the City's Storm Water Management Program (SWMP) design criteria for development. In summary, the proposed design:

- Reduces the post-development peak flow of runoff to below the pre-development rate for the 2- through 25-year storm events
- Reduces the post-development volume of runoff to below the pre-development rate for the 2through 25-year storm events
- Treats the runoff from the site for the 1-inch 24-hour storm event.

EXHIBITS

EXHIBIT 1 EXISTING HYDROLOGY MAP



EXISTING DRAINAGE AREA MAP

LEGEND



DRAINAGE AREA BOUNDARY

SURFACE FLOW DIRECTION

DRAINAGE AREA NAME

HATCH LEGEND



FACILITY	<u>AREA (SF)</u>
HARDSCAPE	29,832
LANDSCAPE	171,435
BUILDING	2,738
POOL	1,011
TOTAL AREA	205,016
TOTAL IMPERVIOUS AREA	32,570 (15.

435 5,016

32,570 (15.9%)



2/27/2020 SCALE: 1''=30'

EXHIBIT 2

PROPOSED HYDROLOGY MAP



EXHIBIT 3

BMP CROSS-SECTION DETAILS



DATE 02/24/2020



ATTACHMENTS

ATTACHMENT 1

DRAINAGE AREA SUMMARY

EXISTING DRAINAGE AREA SUMMARY

ORTEGA PARK - ON-SITE EXISTING CONDITION

DMA	Total Area (SF)	Roof (SF)	Hardscape (SF)	Pool (SF)	Landscape (SF)	Total Impervious (SF)	Total Pervious (SF)	Percent Impervious
X1	205,016	2,738	29,832	1,011	171,435	32,570	172,446	15.9%
Total	205,016	2,738	29,832	1,011	171,435	32,570	172,446	15. 9 %

PROPOSED DRAINAGE AREA SUMMARY

ORTEGA PARK - ON-SITE PROPOSED IMPROVEMENTS

DMA	Total Area (SF)	Roof (SF)	Hardscape (SF)	DG (SF)	Pool (SF)	Synthetic Turf (SF)	Landscape (SF)	Total Impervious (SF)	Total Pervious (SF)	Percent Impervious	1" 24-Hour Runoff Volume (CF)
А	112,909	0	231	0	0	93,452	19,226	231	112,678	0.2%	602
В	92,107	4,930	54,953	2,532	5,481	0	24,211	59,883	32,224	65.0%	3,545
Total	205,016	4,930	55,184	2,532	5,481	93,452	43,437	60,114	144,902	29.3%	4,147

Project Statistics

New Impervious Area	39,103
Replaced Impervious Area	21,011
New Pervious Area	11,559

*Proposed New Impervious Area – area where new impervious area (hardscape, roof, etc.) is proposed where there is existing pervious area (landscaping, etc.)

**Proposed Replaced Impervious Area – area where new impervious area (hardscape, roof, etc.) is proposed where there is currently existing impervious area (hardscape, roof, etc.)

*** Proposed New Pervious Area - area where new pervious area (landscaping, etc.) is proposed where there is currently existing pervious area (landscaping, etc.)

ATTACHMENT 2

HYDROCAD OUTPUT



	Ortega Park On-Site Improvements
Existing	Type I 24-hr 1-inch Rainfall=1.00"
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Runoff	=	0.32 cfs	s@ 10.02	hrs, Volum	e= 2,826 cf, Depth>	0.17"			
Runoff Type I 2	Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00-20.00 hrs, dt= 0.10 hrs Type I 24-hr 1-inch Rainfall=1.00''								
A	Area (sf)	CN Des	scription						
*	32,570 171,435 1,011	98 Pav 80 >75 1 Poo	ved parkir 5% Grass c ol	ng, HSG D cover, Good	HSG D				
	205,016 172,446 32,570	82 We 80 84. 98 15.8	ighted Av 11% Pervic 89% Impe	verage bus Area rvious Area					
Tc (min)	: Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
1.7	120	0.0120	1.16		Sheet Flow, Smooth surfaces n= 0.011	P2= 3.20"			
1.7	120	Total, In	creased t	o minimum	c = 12.0 min				

	Ortega Park On-Site Improvements
Existing	Type I 24-hr 2-year Rainfall=3.20"
Prepared by RRM Design Group	Printed 2/27/2020
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	Runoff	=	3.83 cfs @	10.03 hrs, Volume=	24,669 cf, Depth>	1.44"
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Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00-20.00 hrs, dt= 0.10 hrs Type I 24-hr 2-year Rainfall=3.20"

	Area (:	sf)	CN	De	escription								
	32,57	70	98	Pa	ved park	king, HSC	ΞD						
	171,43	35	80	>7	5% Grass	cover, (Good	d, HSG D					
*	1,0	11	1	Po	ol								
	205,0	16	82	We	eighted A	Average	;						
	172,44	46	80	84	.11% Perv	vious Are	ea						
	32,57	70	98	15	.89% Imp	ervious /	Area						
-	Tc Len	gth	Slop	с	Velocity	' Capo	acity	Description					
(mi	<u>n) (fe</u>	eet)	(ft/	′ft)	(ft/sec)		(cfs)						
1	.7	120	0.01	20	1.16)		Sheet Flow,					
								Smooth surfaces	n= 0.011	P2= 3.2	20''		
1	.7	120	Tota	ıl, İr	ncreased	I to minii	mum	Tc = 12.0 min					

	Ortega Park On-Site Improvements
Existing	Type I 24-hr 5-Year Rainfall=4.61"
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Runoff	=	6.90 cfs @	10.03 hrs, Volume=	42,940 cf, Depth> 2.51"
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Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00-20.00 hrs, dt= 0.10 hrs Type I 24-hr 5-Year Rainfall=4.61"

	Are	a (sf)	CN	De	escription		
	32	2,570	98	Pa	ved parki	ng, HSG D	
	17	1,435	80	>7	5% Grass (cover, Good	d, HSG D
*		1,011	1	Ро	ol		
	20	5,016	82	We	eighted A	verage	
	172	2,446	80	84.	.11% Pervi	ous Area	
	32	2,570	98	15	.89% Impe	ervious Area	a
	Tc L	ength	Slop	be	Velocity	Capacity	/ Description
(mi	in)	(feet)	(ft/	ft)	(ft/sec)	(cfs)	
1	.7	120	0.01	20	1.16		Sheet Flow,
							Smooth surfaces n= 0.011 P2= 3.20"
1	.7	120	Tota	l, Ir	ncreased	to minimum	n Tc = 12.0 min

	Ortega Park	On-Site Improvements
Existing	Type I 24-hr	10-Year Rainfall=5.55"
Prepared by RRM Design Group		Printed 2/27/2020
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Runoff =	9.06 cfs @	10.02 hrs, Volume=	55,873 cf, Depth> 3.27"
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Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00-20.00 hrs, dt= 0.10 hrs Type I 24-hr 10-Year Rainfall=5.55"

	Area (sf)	CN	Des	scription		
	32,570	98	Pav	/ed parkiı	ng, HSG D	
	171,435	80	>75	5% Grass o	cover, Good	d, HSG D
*	1,011	1	Poc	bl		
	205,016	82	We	ighted Av	verage	
	172,446	80	84.	11% Pervi	ous Area	
	32,570	98	15.8	89% Impe	rvious Area	X
٦	Ic Length	Slo	be	Velocity	Capacity	Description
(mi	n) (feet)	(ft,	′ft)	(ft/sec)	(cfs)	
1	.7 120	0.01	20	1.16		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 3.20"
1	.7 120	Toto	al, In	creased	to minimum	n Tc = 12.0 min

	Ortega Park On-Site Improvements
Existing	Type I 24-hr 25-Year Rainfall=6.71"
Prepared by RRM Design Group	Printed 2/27/2020
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Runoff	=	11.78	3 cfs @	10.02	hrs, Volum	ne= 72,260 cf, Depth> 4.23"			
Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00-20.00 hrs, dt= 0.10 hrs Type I 24-hr 25-Year Rainfall=6.71"									
A	rea (sf)	CN	Descrip	otion					
1	32,570 71,435 1,011	98 80 1	Paved >75% C Pool	parking Grass co	g, HSG D over, Good	d, HSG D			
1	205,016 72,446 32,570	82 80 98	Weight 84.11% 15.89%	ted Ave Pervio Imperv	erage us Area vious Area	X			
Tc (min)	Length (feet)	Slop (ft/t	e Velo ft) (ft/	ocity /sec)	Capacity (cfs)	Description			
1.7	120	0.012	20	1.16		Sheet Flow, Smooth surfaces n= 0.011 P2= 3.20"			
1.7	120	Tota	, Increa	ased to	o minimum	n Tc = 12.0 min			



Proposed Prepared by RRM Design Group HydroCAD® 10.00-24 s/n 10829 © 2018 HydroCAD Software Solutions	Ortega Park On-Site Improvements Type I 24-hr 1-inch Rainfall=1.00" Printed 2/27/2020 LLC Page 2							
Summary for Subcatchment 1S: A								
Runoff = 0.02 cfs @ 12.96 hrs, Volume= 496	cf, Depth> 0.05"							
Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00- Type I 24-hr 1-inch Rainfall=1.00"	20.00 hrs, dt= 0.10 hrs							
Area (sf) CN Description								
231 98 Paved parking, HSG D 19,226 80 >75% Grass cover, Good, HSG D * 93,452 79 Synthetic Turf								
 112,909 79 Weighted Average 112,678 79 99.80% Pervious Area 231 98 0.20% Impervious Area 								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
1.71200.01201.16Sheet Flow, Smooth surfaces	n= 0.011 P2= 3.20"							
1.7 120 Total, Increased to minimum Tc = 12.0 min								
Summary for Subcatchment	Summary for Subcatchment 2S: B							
Runoff = 0.58 cfs @ 10.02 hrs, Volume= 3,545	cf, Depth> 0.46"							
Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00- Type I 24-hr 1-inch Rainfall=1.00"	20.00 hrs, dt= 0.10 hrs							
Area (sf) CN Description								
54,953 98 Paved parking, HSG D								
24,211 80 >75% Grass cover, Good, HSG D								
* 5 481 1 Pool								
* 2,532 80 Decomposed Granite								
92,107 87 Weighted Average								
32,224 67 34.99% Pervious Area								
59,883 98 65.01% Impervious Area								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
1.7 120 0.0120 1.16 Sheet Flow ,								
Smooth surfaces	n= 0.011 P2= 3.20"							
1.7 120 Total, Increased to minimum Tc = 12.0 min								

Propos Prepare HydroC/	s ed ed by RR AD® 10.00	M Desi -24 s/n 1	gn Group 10829 © 2013	Ortega Park On-Site Improvements Type I 24-hr 2-year Rainfall=3.20'' Printed 2/27/2020 Page 2		
			Sur	nmary for	Subcatchment 1S:	Α
Runoff	=	1.66 (cfs @ 10.04	thrs, Volum	ne= 10,995 cf,	Depth> 1.17"
Runoff I Type I 2	by SBUH n 24-hr 2-ye	nethod, ar Rainf	Split Pervic all=3.20''	us/Imperv.,	Time Span= 5.00-20.00) hrs, dt= 0.10 hrs
А	rea (sf)	CN D	escription			
*	231 19,226 93,452	98 P 80 > 79 S	aved parki 75% Grass o ynthetic Tur	ng, HSG D cover, Good f	d, HSG D	
1	112,909 112,678 231	79 V 79 9 98 0	Veighted Av 9.80% Pervi .20% Imper	verage ous Area vious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
1.7	120	0.0120	1.16		Sheet Flow , Smooth surfaces n=	0.011 P2= 3.20"
1.7	120	Total,	Increased :	to minimum	Tc = 12.0 min	
			Sui	mmary for	Subcatchment 2S:	В
Runoff	=	2.24 (cfs @ 10.02	2 hrs, Volum	ne= 14,478 cf,	Depth> 1.89"
Runoff I Type I 2	by SBUH n 24-hr 2-ye	nethod, ar Rainf	Split Pervic all=3.20''	us/Imperv.,	Time Span= 5.00-20.00) hrs, dt= 0.10 hrs
A	rea (sf)	CN D	escription			
	54,953	98 P	aved parki	ng, HSG D		
	24,211	80 >	75% Grass (cover, Good	d, HSG D	
*	4,930 5 491	98 K 1 D	oois, hsg l)		
*	2.532	80 D)ecompose	d Granite		
	92.107	87 V	Veighted Av	veraae		
	32,224	67 3	4.99% Pervi	ous Area		
	59,883	98 6	5.01% Impe	rvious Area		
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(teet)	(tt/tt)	(tt/sec)	(Cts)	AL 1 -	
1.7	120	0.0120	1.16		Sneet Flow, Smooth surfaces n=	0.011 P2= 3.20"
1.7	120	Total,	Increased :	to minimum	Tc = 12.0 min	

ProposedOrtega Park On-Site Improvements
Type I 24-hrPrepared by RRM Design GroupPrinted 2/27/2020HydroCAD® 10.00-24 s/n 10829 © 2018 HydroCAD Software Solutions LLCPage 3

Summary for Reach 3R: Total

Inflow A	vrea =	205,016 sf,	29.32% In	npervious,	Inflow Depth >	1.28"	for 2	-year event
Inflow	=	3.79 cfs @	10.04 hrs,	Volume=	21,936 cf			
Outflow	/ =	3.79 cfs @	10.04 hrs,	Volume=	21,936 cf,	Atten	= 0%,	Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.10 hrs

Summary for Pond 4P: Cultec Chambers

Inflow Are	a =	92,107 sf,	65.01% lm	pervious,	Inflow Depth >	1.89"	for 2	-year even [.]	t
Inflow	=	2.24 cfs @	10.02 hrs, \	Volume=	14,478 cf				
Outflow	=	2.11 cfs @	10.06 hrs, \	Volume=	10,940 cf,	Atten	= 6%,	Lag= 2.1 m	nin
Primary	=	2.11 cfs @	10.06 hrs, \	Volume=	10,940 cf			-	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.10 hrs / 3 Peak Elev= 8.36' @ 10.06 hrs Surf.Area= 2,894 sf Storage= 3,706 cf

Plug-Flow detention time= 162.4 min calculated for 10,935 cf (76% of inflow) Center-of-Mass det. time= 66.8 min (768.0 - 701.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	6.29'	2,067 cf	27.50'W x 105.25'L x 2.54'H Field A
			7,357 cf Overall - 2,188 cf Embedded = 5,168 cf x 40.0% Voids
#2A	6.79'	2,188 cf	Cultec R-150XLHD x 80 Inside #1
			Effective Size= 29.8"W x 18.0"H => 2.65 sf x 10.25'L = 27.2 cf
			Overall Size= 33.0"W x 18.5"H x 11.00'L with 0.75' Overlap
			Row Length Adjustment= +0.75' x 2.65 sf x 8 rows
		4,255 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices		
#1	Primary	7.10'	15.0" Vert. Orifice/Grate	C= 0.600	

Primary OutFlow Max=2.06 cfs @ 10.06 hrs HW=8.35' TW=8.23' (Fixed TW Elev= 8.23') **1=Orifice/Grate** (Orifice Controls 2.06 cfs @ 1.68 fps)

Proposed Prepared by RRM Design Group HydroCAD® 10.00-24 s/n 10829 © 2018 HydroCAD Software Solutions L	Ortega Park On-Site Improvements Type I 24-hr 5-Year Rainfall=4.61" Printed 2/27/2020 LC Page 4
Summary for Subcatchment	1S: A
Runoff = 3.30 cfs @ 10.03 hrs, Volume= 20,641	cf, Depth> 2.19"
Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00-2 Type I 24-hr 5-Year Rainfall=4.61"	0.00 hrs, dt= 0.10 hrs
Area (sf) CN Description	
231 98 Paved parking, HSG D 19,226 80 >75% Grass cover, Good, HSG D * 93,452 79 Synthetic Turf	
 112,909 79 Weighted Average 112,678 79 99.80% Pervious Area 231 98 0.20% Impervious Area 	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
1.7 120 0.0120 1.16 Sheet Flow,	n= 0.011 P2= 3.20"
1.7 120 Total, Increased to minimum Tc = 12.0 min	
Summary for Subcatchment	2S: B
Runoff = 3.53 cfs @ 10.02 hrs, Volume= 22,446	cf, Depth> 2.92"
Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00-2 Type I 24-hr 5-Year Rainfall=4.61"	0.00 hrs, dt= 0.10 hrs
Area (sf) CN Description	
54,953 98 Paved parking, HSG D 24,211 80 >75% Grass cover, Good, HSG D 4,930 98 Roofs, HSG D * 5,481 1 * 2,532 80 Decomposed Granite	
92,107 87 Weighted Average 32,224 67 34.99% Pervious Area 59,883 98 65.01% Impervious Area	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
1.7 120 0.0120 1.16 Sheet Flow ,	n= 0.011 P2= 3.20"
1.7 120 Total, Increased to minimum Tc = 12.0 min	11-0.011 12-0.20

ProposedOrtega Park On-Site Improvements
Type I 24-hrPrepared by RRM Design GroupPrinted 2/27/2020HydroCAD® 10.00-24 s/n 10829 © 2018 HydroCAD Software Solutions LLCPage 5

Summary for Reach 3R: Total

Inflow A	Area =	205,016 sf,	29.32% In	npervious,	Inflow Depth >	2.32"	for 5	-Year event
Inflow	=	6.52 cfs @	10.05 hrs,	Volume=	39,565 cf			
Outflow	v =	6.52 cfs @	10.05 hrs,	Volume=	39,565 cf,	Atten	i= 0%,	Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.10 hrs

Summary for Pond 4P: Cultec Chambers

Inflow Arec) =	92,107 sf,	65.01% In	npervious,	Inflow Depth >	2.92"	for 5	-Year ev	rent
Inflow	=	3.53 cfs @	10.02 hrs,	Volume=	22,446 cf				
Outflow	=	3.27 cfs @	10.08 hrs,	Volume=	18,924 cf,	Atten	= 7%,	Lag= 3.	3 min
Primary	=	3.27 cfs @	10.08 hrs,	Volume=	18,924 cf			-	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.10 hrs / 3 Peak Elev= 8.54' @ 10.08 hrs Surf.Area= 2,894 sf Storage= 3,913 cf

Plug-Flow detention time= 113.1 min calculated for 18,791 cf (84% of inflow) Center-of-Mass det. time= 46.7 min (747.7 - 701.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	6.29'	2,067 cf	27.50'W x 105.25'L x 2.54'H Field A
			7,357 cf Overall - 2,188 cf Embedded = 5,168 cf x 40.0% Voids
#2A	6.79'	2,188 cf	Cultec R-150XLHD x 80 Inside #1
			Effective Size= 29.8"W x 18.0"H => 2.65 sf x 10.25'L = 27.2 cf
			Overall Size= 33.0"W x 18.5"H x 11.00'L with 0.75' Overlap
			Row Length Adjustment= +0.75' x 2.65 sf x 8 rows
		4,255 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Primary	7.10'	15.0" Vert. Orifice/Grate	C= 0.600

Primary OutFlow Max=3.21 cfs @ 10.08 hrs HW=8.53' TW=8.23' (Fixed TW Elev= 8.23') **1=Orifice/Grate** (Orifice Controls 3.21 cfs @ 2.62 fps)

Propos Prepar HydroC/	sed ed by RR AD® 10.00	M Desig -24 s/n 1	gn Group 10829 © 201	8 HydroCAD	Ortega Park On-Site Improvement Type I 24-hr 10-Year Rainfall=5.55 Printed 2/27/2020 Software Solutions LLC Page			
			Su	mmary for	Subcatchment 1S: A			
Runoff	=	4.48 (cfs @ 10.03	3 hrs, Volum	me= 27,602 cf, Depth> 2.93"			
Runoff I Type I 2	by SBUH n 24-hr 10-Y	nethod, ear Rair	Split Pervic nfall=5.55''	ous/Imperv.,	, Time Span= 5.00-20.00 hrs, dt= 0.10 hrs			
А	rea (sf)	CN D	escription					
*	231 19,226 93,452	98 P 80 > 79 S	aved parki 75% Grass o ynthetic Tu	ng, HSG D cover, Good f	d, HSG D			
	112,909 112,678 231	79 W 79 9 98 0	Veighted A 9.80% Pervi .20% Imper	verage ous Area vious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
1.7	120	0.0120	1.16		Sheet Flow, Smooth surfaces n= 0.011 P2= 3.20"			
1.7	120	Total,	Increased	to minimum	n Tc = 12.0 min			
			Su	mmary for	r Subcatchment 2S: B			
Runoff	=	4.43 (cfs @ 10.02	2 hrs, Volum	ne= 27,981 cf, Depth> 3.65"			
Runoff I Type I 2	by SBUH n 24-hr 10-Y	nethod, ear Rair	Split Pervic nfall=5.55''	ous/Imperv.,	, Time Span= 5.00-20.00 hrs, dt= 0.10 hrs			
A	rea (sf)	CN D	escription					
	54,953	98 P	aved parki	ng, HSG D				
	24,211	80 >	75% Grass	cover, Good	d, HSG D			
*	4,730 5 /81	70 K 1 P	ools, hsg l)				
*	2.532	80 D	ecompose	d Granite				
	92,107	87 V	Veiahted A	veraae				
	32,224	67 3	4.99% Pervi	ous Área				
	59,883	98 6	5.01% Impe	ervious Area				
Tc	Length		Velocity	Capacity	Description			
<u>(rnin)</u>				(CIS)	Sheet Flow			
I./	120	0.0120	1.16		Smooth surfaces n= 0.011 P2= 3.20"			
1.7	120	Total,	Increased	to minimum	n Tc = 12.0 min			

ProposedOrtega Park On-Site ImprovementsPrepared by RRM Design GroupType I 24-hr10-Year Rainfall=5.55"Prepared by RRM Design GroupPrinted 2/27/2020HydroCAD® 10.00-24 s/n 10829 © 2018 HydroCAD Software Solutions LLCPage 7

Summary for Reach 3R: Total

Inflow A	Area =	205,016 sf,	29.32% lm	pervious,	Inflow Depth >	3.05" f	or 10)-Year event
Inflow	=	8.42 cfs @	10.05 hrs, "	Volume=	52,036 cf			
Outflow	v =	8.42 cfs @	10.05 hrs, '	Volume=	52,036 cf,	Atten=	0%,	Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.10 hrs

Summary for Pond 4P: Cultec Chambers

Inflow Are	a =	92,107 sf,	65.01% lm	pervious,	Inflow Depth >	3.65" fc	or 10-Year event
Inflow	=	4.43 cfs @	10.02 hrs, \	√olume=	27,981 cf		
Outflow	=	4.05 cfs @	10.09 hrs, \	√olume=	24,434 cf,	Atten=	3%, Lag= 3.9 min
Primary	=	4.05 cfs @	10.09 hrs, \	√olume=	24,434 cf		-

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.10 hrs / 3 Peak Elev= 8.70' @ 10.09 hrs Surf.Area= 2,894 sf Storage= 4,103 cf

Plug-Flow detention time= 94.9 min calculated for 24,262 cf (87% of inflow) Center-of-Mass det. time= 39.5 min (740.0 - 700.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	6.29'	2,067 cf	27.50'W x 105.25'L x 2.54'H Field A
			7,357 cf Overall - 2,188 cf Embedded = 5,168 cf x 40.0% Voids
#2A	6.79'	2,188 cf	Cultec R-150XLHD x 80 Inside #1
			Effective Size= 29.8"W x 18.0"H => 2.65 sf x 10.25'L = 27.2 cf
			Overall Size= 33.0"W x 18.5"H x 11.00'L with 0.75' Overlap
			Row Length Adjustment= +0.75' x 2.65 sf x 8 rows
		4,255 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Primary	7.10'	15.0" Vert. Orifice/Grate	C= 0.600

Primary OutFlow Max=4.00 cfs @ 10.09 hrs HW=8.69' TW=8.23' (Fixed TW Elev= 8.23') **1=Orifice/Grate** (Orifice Controls 4.00 cfs @ 3.26 fps)

Propos Prepare HydroC/	ed ed by RR AD® 10.00	M Desi -24 s/n	gn Group 10829 © 2018	8 HydroCAD	Ortega Type 1 24 Software Solutions LLC	Park On-Site Improvements 4-hr 25-Year Rainfall=6.71'' Printed 2/27/2020 Page 8
			Sur	nmary for	Subcatchment 1S: A	
Runoff	=	5.97	cfs @ 10.02	2 hrs, Volum	e= 36,520 cf, Depth>	3.88"
Runoff I Type I 2	by SBUH n 24-hr 25-Y	nethod, ear Raii	, Split Pervio nfall=6.71''	us/Imperv.,	Time Span= 5.00-20.00 hrs, dt=	= 0.10 hrs
А	rea (sf)	CN E	Description			
*	231 19,226 93,452	98 P 80 > 79 S	aved parkin 75% Grass of ynthetic Tur	ng, HSG D cover, Good f	I, HSG D	
1	112,909 112,678 231	79 V 79 9 98 0	Veighted Av 9.80% Pervie .20% Imperv	verage ous Area vious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
1.7	120	0.0120	1.16		Sheet Flow,	P0- 3 00"
1.7	120	Total,	Increased	to minimum	Tc = 12.0 min	1 2- 5.20
			Sui	mmary for	Subcatchment 2S: B	
Runoff	=	5.57	cfs @ 10.02	2 hrs, Volum	e= 34,982 cf, Depth>	4.56"
Runoff I Type I 2	by SBUH n 24-hr 25-Y	nethod, ear Raii	, Split Pervio nfall=6.71''	us/Imperv.,	Time Span= 5.00-20.00 hrs, dt=	= 0.10 hrs
A	rea (sf)	CN E	Description			
	54,953	98 P	aved parki	ng, HSG D		
	24,211	80 >	75% Grass o	cover, Good	I, HSG D	
*	4,930	98 K	COOTS, HSG L)		
*	0,401 0,530		001	d Granite		
	92 107	87 V	Veighted Av			
	32 224	67 3	4 99% Pervi	ous Area		
	59,883	98 6	5.01% Impe	rvious Area		
, Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(teet)	(††/††)	(tt/sec)	(Cts)	<u></u>	
1.7	120	0.0120	1.16		Sheet Flow, Smooth surfaces n= 0.011	P2= 3.20"
1.7	120	Total,	Increased	to minimum	Tc = 12.0 min	

ProposedOrtega Park On-Site ImprovementsProposedType I 24-hr25-Year Rainfall=6.71"Prepared by RRM Design GroupPrinted 2/27/2020HydroCAD® 10.00-24 s/n 10829 © 2018 HydroCAD Software Solutions LLCPage 9

Summary for Reach 3R: Total

Inflow /	Area	=	205,016 sf,	29.32% In	npervious,	Inflow Depth >	3.97"	for 2	5-Year event
Inflow	=	=	10.92 cfs @	10.06 hrs,	Volume=	67,865 cf			
Outflow	~ -	=	10.92 cfs @	10.06 hrs,	Volume=	67,865 cf,	Atten	= 0%,	Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.10 hrs

Summary for Pond 4P: Cultec Chambers

Inflow Arec	z =	92,107 sf,	65.01% lm	pervious,	Inflow Depth >	4.56"	for 2	5-Year event
Inflow	=	5.57 cfs @	10.02 hrs, \	√olume=	34,982 cf			
Outflow	=	5.31 cfs @	10.09 hrs, \	√olume=	31,345 cf,	Atten	= 5%,	Lag= 4.3 min
Primary	=	5.31 cfs @	10.09 hrs, \	√olume=	31,345 cf			-

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.10 hrs / 3 Peak Elev= 9.04' @ 10.09 hrs Surf.Area= 2,894 sf Storage= 4,255 cf

Plug-Flow detention time= 81.0 min calculated for 31,333 cf (90% of inflow) Center-of-Mass det. time= 33.8 min (733.5 - 699.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	6.29'	2,067 cf	27.50'W x 105.25'L x 2.54'H Field A
			7,357 cf Overall - 2,188 cf Embedded = 5,168 cf x 40.0% Voids
#2A	6.79'	2,188 cf	Cultec R-150XLHD x 80 Inside #1
			Effective Size= 29.8"W x 18.0"H => 2.65 sf x 10.25'L = 27.2 cf
			Overall Size= 33.0"W x 18.5"H x 11.00'L with 0.75' Overlap
			Row Length Adjustment= +0.75' x 2.65 sf x 8 rows
		4,255 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices		
#1	Primary	7.10'	15.0" Vert. Orifice/Grate	C= 0.600	

Primary OutFlow Max=5.25 cfs @ 10.09 hrs HW=9.02' TW=8.23' (Fixed TW Elev= 8.23') **1=Orifice/Grate** (Orifice Controls 5.25 cfs @ 4.28 fps)

ATTACHMENT 3

RETENTION CALCULATIONS



CULTEC Stormwater Design Calculator

001			
Date:	February 27, 2020		
	Project Info	rmation:	
rtega F	Park		
	Recharger 1 Chamber Spe	50XLHD	
	Height	18.5	inches
	Width	33.0 11.00	incnes
I	nstalled Length	10.25	feet
Bare	e Chamber Volume	27.19	cu. feet
			6h

Materials List

Recharger 1			
Total Number of Chambers Required	80	pieces	
Starter Chambers	8	pieces	
Intermediate Chambers	64	pieces	
End Chambers	8	pieces	
HVLV FC-24 Feed Connectors	7	pieces	Based on 1 Internal Manifold
CULTEC No. 410 Non-Woven Geotextile	898	sq. yards	
CULTEC No. 4800 Woven Geotextile	28	feet	
Stone	191	cu. yards	

Bed Detail

-	BED LE	ENGTH	
	CHAMBER R	OW LENGTH -	
			HC
			BER ROWWI
			CHAN

Bed Layout Information							
Number of Rows Wide	8	pieces					
Number of Chambers Long	10	pieces					
Chamber Row Width	25.50	feet					
Chamber Row Length	103.25	feet					
Bed Width	27.50	feet					
Bed Length	105.25	feet					
Bed Area Required	2894.38	sq. feet					
Length of Separator Row	N/A	feet					

Bed detail for reference only. Not project specific. Not to scale.



Conceptual graphic only. Not job specific.

	Cross Section Table Reference		
Α	Depth of Stone Base	6.0	inches
в	Chamber Height	18.5	inches
с	Depth of Stone Above Units	6.0	inches
D	Depth of 95% Compacted Fill	8.0	inches
E	Max. Depth Allowed Above the Chamber	12.00	feet
F	Chamber Width	33.0	inches
G	Center to Center Spacing	3.25	feet
н	Effective Depth	2.54	feet
I	Bed Depth	3.21	feet

Proposed

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Elevation Storage Elevation Storage Elevation Storage (feet) (cubic-feet) (feet) (cubic-feet) (feet) (cubic-feet) 6.29 7.33 1,828 8.37 3,721 0 6.31 23 7.35 1,873 8.39 3,744 6.33 46 7.37 1,918 8.41 3,767 69 3,790 6.35 7.39 1,962 8.43 3,814 6.37 93 7.41 2,007 8.45 6.39 116 7.43 8.47 3,837 2,051 139 7.45 8.49 6.41 2,095 3,860 6.43 162 7.47 2,139 8.51 3,883 185 7.49 2,183 8.53 6.45 3,906 6.47 208 7.51 2,227 8.55 3,929 6.49 232 7.53 2,270 8.57 3,953 6.51 255 7.55 2,313 8.59 3,976 6.53 278 7.57 2,356 8.61 3,999 6.55 301 7.59 2,399 8.63 4,022 6.57 324 7.61 2,442 8.65 4,045 6.59 347 7.63 2,484 8.67 4,068 2,526 6.61 370 7.65 8.69 4,091 394 2,568 7.67 8.71 4,115 6.63 417 7.69 4,138 6.65 2,609 8.73 8.75 6.67 440 7.71 2,650 4,161 6.69 463 7.73 2,691 8.77 4,184 8.79 6.71 486 7.75 2,732 4,207 509 4,230 6.73 7.77 2,772 8.81 7.79 6.75 533 2,812 8.83 4,254 6.77 556 7.81 2,852 6.79 579 7.83 2,891 6.81 627 7.85 2,930 6.83 674 7.87 2,968 6.85 722 7.89 3,006 6.87 769 7.91 3,044 6.89 816 7.93 3,081 6.91 863 7.95 3,118 6.93 7.97 910 3,154 6.95 956 7.99 3,189 8.01 3,224 6.97 1,003 6.99 8.03 3,259 1,049 7.01 1,096 8.05 3,293 7.03 1,142 8.07 3,325 7.05 1,188 8.09 3,357 1,234 7.07 8.11 3,388 7.09 1,280 8.13 3,418 7.11 1,326 8.15 3,447 7.13 1,372 8.17 3,475 1,418 8.19 3,502 7.15 8.21 3,528 7.17 1,464 7.19 1,510 8.23 3,554 7.21 1,556 8.25 3,579 7.23 1,601 8.27 3,604 7.25 1,647 8.29 3,628 7.27 1,692 8.31 3,651 7.29 1,738 8.33 3,675 7.31 1,783 8.35 3,698

Stage-Area-Storage for Pond 4P: Cultec Chambers