

Traffic Impact Analysis

**Trans Truck System Truck Facility
San Joaquin County, California**

January 14, 2020

Prepared for:

San Joaquin County
Department of Public Works

Prepared by:

Kimley»Horn

555 Capitol Mall, Suite 300
Sacramento, California 95814

Phone: (916) 858-5800



EXECUTIVE SUMMARY

This report documents the results of a traffic impact analysis completed for the Trans Truck System Truck Facility project proposed to be located at 707 E. Roth Road in French Camp, San Joaquin County, California (the “proposed project” or “project”). The purpose of this impact analysis is to identify potential environmental impacts to transportation facilities as required by the California Environmental Quality Act (CEQA). This study was performed in accordance with the San Joaquin County Public Works Transportation Engineering Division’s *Traffic Impact Study Guidelines*.

The project site is proposed to be constructed on 4.5-acres of vacant land on an 8.85-acre parcel. The project site is anticipated to accommodate a maximum of 171 trailers. One driveway is proposed along Roth Road at the southern border of the site and is anticipated to be controlled by a gate. The following intersections are included in this evaluation:

1. Roth Road @ Manthey Road
2. Roth Road @ I-5 SB Ramps
3. Roth Road @ I-5 NB Ramps
4. Roth Road @ Harlan Road
5. Roth Road @ S. McKinley Avenue
6. Roth Road @ S. Airport Way
7. Roth Road @ Project Driveway (Scenarios C and E only)

Per San Joaquin County requirements, a weekday AM and PM peak-hour Level of Service (LOS) analysis was conducted for the following scenarios:

- A. Existing (2019) Conditions
- B. Existing (2019) plus Approved and Pending Projects Conditions⁺
- C. Existing (2019) plus Approved and Pending Projects plus Proposed Project Conditions
- D. Cumulative (2040) Conditions⁺⁺
- E. Cumulative (2040) plus Proposed Project Conditions

⁺ Includes projects identified by the County as well as projects anticipated by the City of Lathrop.

⁺⁺ Year 2040 is the forecast year associated with the San Joaquin Council of Governments’ (SJCOC) current travel demand model. As a result, although the County guidelines specify Cumulative Year 2025, this analysis contemplates Cumulative Year 2040 conditions.

Significant findings of this study include:

- The proposed Project is estimated to generate 370 total new daily trips, with 33 and 29 trips occurring during the AM and PM peak-hours, respectively.
- As defined by the County, the addition of the proposed Project does not result in significant impacts to any of the study intersections under Existing (2019) plus Approved and Pending Projects plus Proposed Project Conditions and Cumulative (2040) plus Proposed Project Conditions.
- Based on this analysis, the 95th percentile vehicles queues are not anticipated to exceed available storage for the eastbound movement at the intersection of Roth Road and Harlan Road (Intersection #4).

TABLE OF CONTENTS

INTRODUCTION.....	1
PROJECT DESCRIPTION	1
PROJECT AREA ROADWAYS.....	1
ASSESSMENT OF PROPOSED PROJECT	5
Proposed Project Trip Generation	5
Proposed Project Trip Distribution and Assignment	5
LEVEL OF SERVICE METHODOLOGY	8
Intersection Analysis.....	8
Roadway Segment Analysis	8
Freeway Facility Analysis	9
EXISTING (2019) CONDITIONS.....	9
EXISTING (2019) PLUS APPROVED AND PENDING PROJECTS	12
EXISTING (2019) PLUS APPROVED AND PENDING PROJECTS PLUS PROPOSED PROJECT CONDITIONS	15
CUMULATIVE (2040) CONDITIONS	18
CUMULATIVE (2040) PLUS PROPOSED PROJECT CONDITIONS.....	20
IMPACTS AND MITIGATION	22
Standards of Significance	22
Impacts and Mitigation	23
OTHER CONSIDERATIONS	24
95 th Percentile Queues	24
Access	24
CONCLUSIONS	25
APPENDICES	
<i>Traffic Count Data Sheets</i>	<i>Appendix A</i>
<i>Analysis Worksheets for Existing (2019) Conditions</i>	<i>Appendix B</i>
<i>Analysis Worksheets for Existing (2019) plus Approved and Pending Projects Conditions</i>	<i>Appendix C</i>
<i>Analysis Worksheets for Existing (2019) plus Approved and Pending Projects plus Proposed Project Conditions</i>	<i>Appendix D</i>
<i>Analysis Worksheets for Cumulative (2040) Conditions.....</i>	<i>Appendix E</i>
<i>Analysis Worksheets for Cumulative (2040) plus Proposed Project Conditions</i>	<i>Appendix F</i>
<i>Analysis Worksheets for Mitigated Conditions.....</i>	<i>Appendix G</i>

LIST OF TABLES

Table 1 – Proposed Project Trip Generation.....	5
Table 2 – Intersection Level of Service Criteria	8
Table 3 – Two-Lane Roadway Level of Service Criteria	8
Table 4 – Freeway Facility Level of Service Criteria.....	9
Table 5 – Existing (2019) Intersection Levels of Service.....	11
Table 6 – Existing (2019) Roadway Levels of Service.....	11
Table 7 – Existing (2019) Freeway Levels of Service.....	12
Table 8 – Existing (2019) plus Approved and Pending Projects Intersection Levels of Service	14
Table 9 – Existing (2019) plus Approved and Pending Projects Roadway Levels of Service.....	14
Table 10 – Existing (2019) plus Approved and Pending Projects Freeway Levels of Service.....	15
Table 11 – Existing (2019) plus Approved and Pending Projects plus Proposed Project Intersection Levels of Service	15
Table 12 – Existing (2019) plus Approved and Pending Projects plus Proposed Project Roadway Levels of Service	17
Table 13 – Existing (2019) plus Approved and Pending Projects plus Proposed Project Freeway Levels of Service	17
Table 14 – Cumulative (2040) Intersection Levels of Service	18
Table 15 – Cumulative (2040) Roadway Level of Service	18
Table 16 – Cumulative (2040) Freeway Level of Service	20
Table 17 – Cumulative (2040) plus Proposed Project Intersection Levels of Service	20
Table 18 – Cumulative (2040) plus Proposed Project Roadway Level of Service.....	22
Table 19 – Cumulative (2040) plus Proposed Project Freeway Level of Service.....	22
Table 20 – Significance Criteria	23
Table 21 – Intersection Levels of Service – Existing plus Approved and Pending Projects.....	24
Table 22 – 95 th Percentile Queues	25

LIST OF FIGURES

Figure 1 – Project Vicinity Map.....	2
Figure 2 – Proposed Project Site Plan	3
Figure 3 – Study Intersections, Traffic Control, and Lane Geometries	4
Figure 4 – Proposed Project Trip Distribution.....	6
Figure 5 – Proposed Project Trip Assignment.....	7
Figure 6 – Existing (2019) Peak-Hour Traffic Volumes	10
Figure 7 – Existing (2019) plus Approved Projects Peak-Hour Traffic Volumes	13
Figure 8 – Existing (2019) plus Approved Projects plus Proposed Project Volumes.....	16
Figure 9 – Cumulative (2040) Peak-Hour Traffic Volumes	19
Figure 10 – Cumulative (2040) plus Proposed Project Peak-Hour Traffic Volumes	21

INTRODUCTION

This report documents the results of a traffic impact analysis completed for the Trans Truck System Truck Facility project proposed to be located at 707 Roth Road in French Camp, San Joaquin County, California (the “proposed Project” or “project”). The purpose of this impact analysis is to identify potential environmental impacts to transportation facilities as required by the California Environmental Quality Act (CEQA). This study was performed in accordance with the San Joaquin County Public Works Transportation Engineering Division’s *Traffic Impact Study Guidelines*¹. The remaining sections of this report document the proposed Project, analysis methodologies, impacts and mitigation, and general study conclusions.

PROJECT DESCRIPTION

The project site is proposed to be constructed on 4.5-acres of vacant land on an 8.85-acre parcel. The project site is anticipated to accommodate a maximum of 171 trailers. One driveway is proposed along Roth Road at the southern border of the site and is anticipated to be controlled by a gate. The project location is shown in **Figure 1**, and the proposed Project site plan is shown in **Figure 2**.

The following intersections are included in this evaluation:

1. Roth Road @ Manthey Road
2. Roth Road @ I-5 SB Ramps
3. Roth Road @ I-5 NB Ramps
4. Roth Road @ Harlan Road
5. Roth Road @ S. McKinley Avenue
6. Roth Road @ S. Airport Way
7. Roth Road @ Project Driveway (Scenarios C and E only)

The following roadway segments are included in this evaluation:

1. Roth Road west of McKinley Avenue
2. Roth Road east of McKinley Avenue

The following freeway facilities are included in this evaluation:

1. I-5 Northbound diverge to Roth Road
2. I-5 Northbound merge from Roth Road
3. I-5 Southbound diverge to Roth Road
4. I-5 Southbound merge from Roth Road

Figure 3 illustrates the study facilities, existing traffic control, and existing lane configurations.

PROJECT AREA ROADWAYS

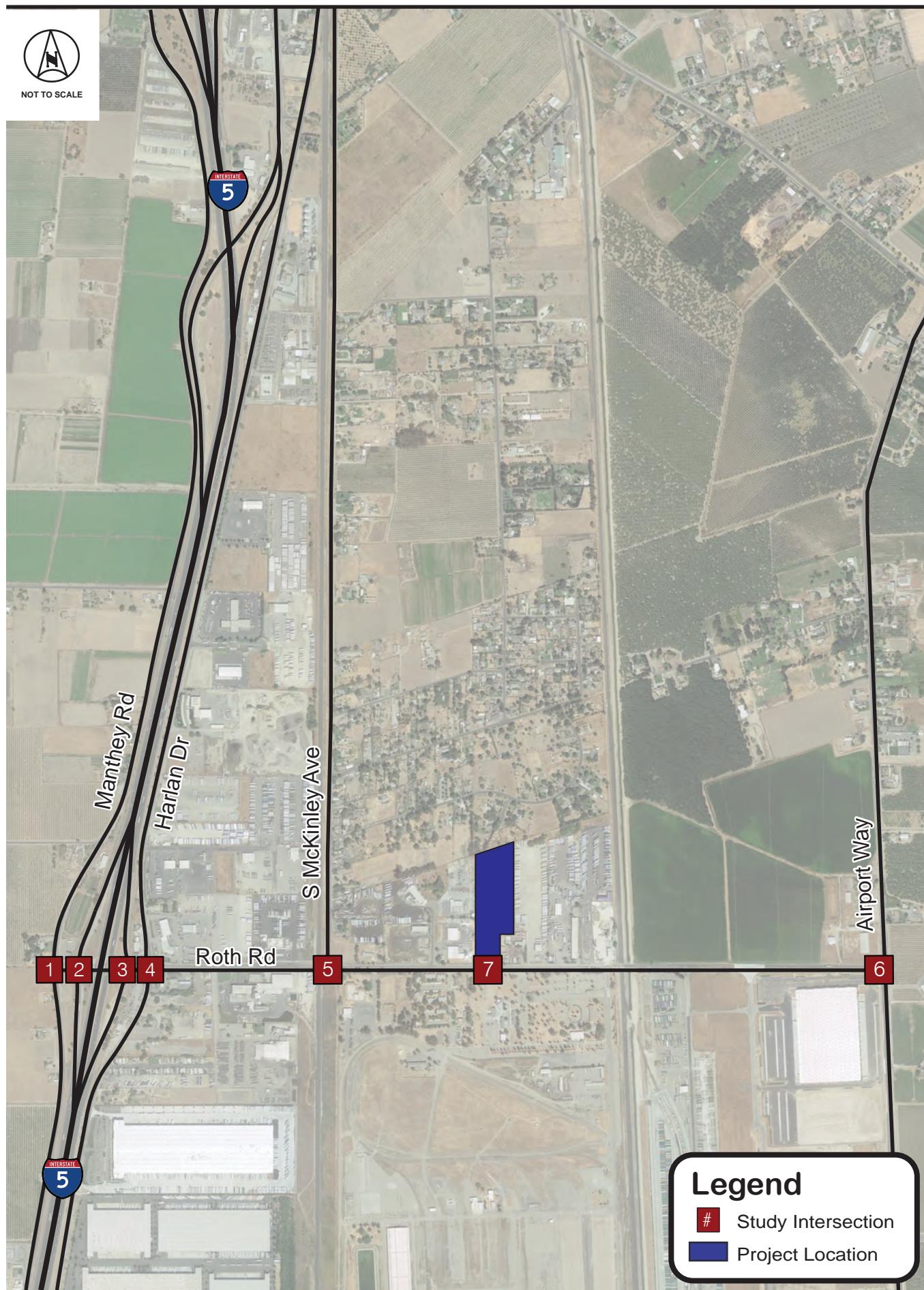
The following are descriptions of the primary roadways in the vicinity of the project:

Interstate 5 (I-5) is a north-south freeway, with ramps located within a mile from the project site.

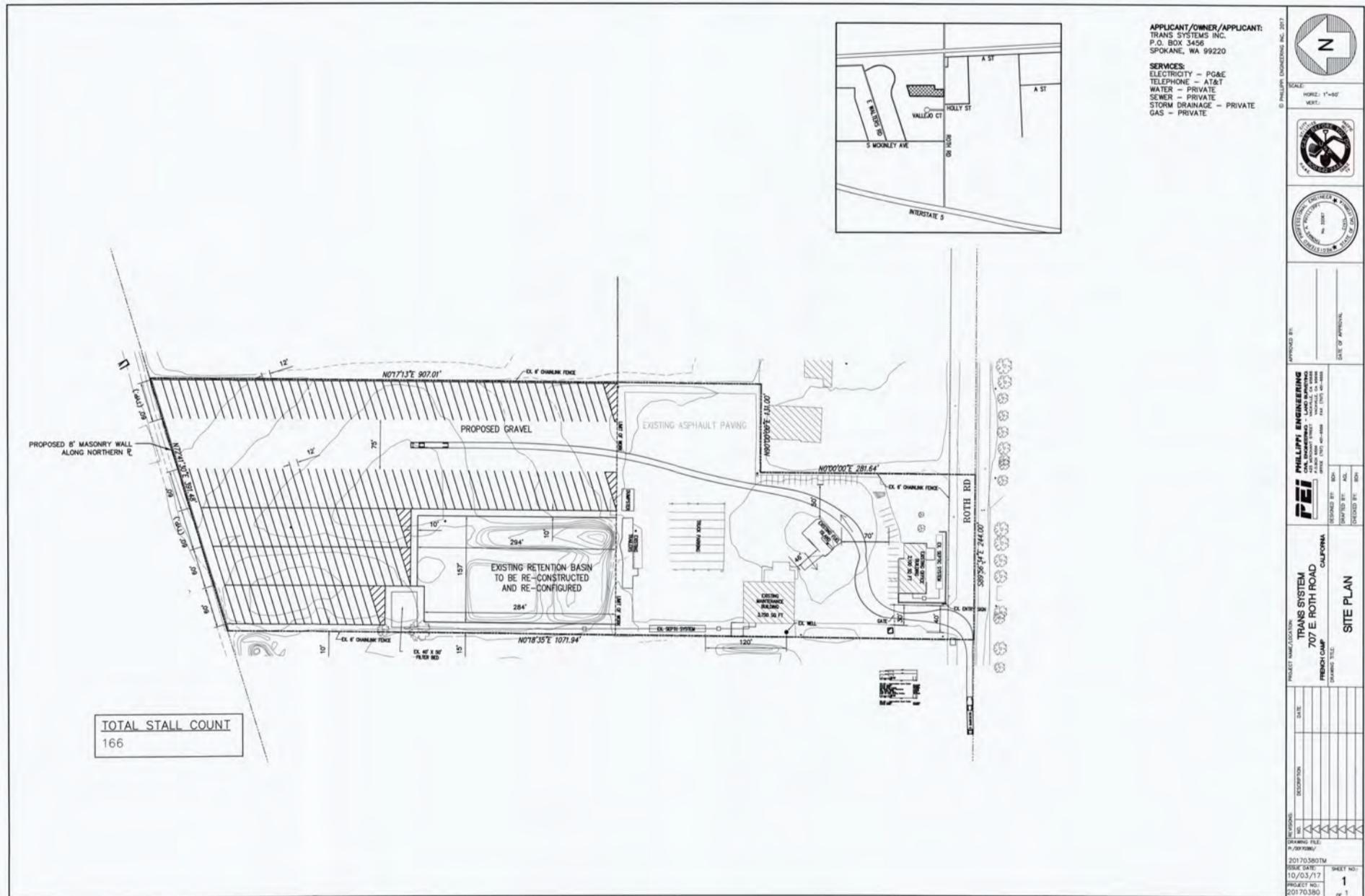
Roth Road is an east-west arterial roadway that provides access to Harlan Road via I-5. This roadway connects with both the northbound and southbound on- and off-ramps of I-5 and will act as the main route for travel between the freeway and the project site.

¹San Joaquin County *Traffic Impact Study Guidelines*, November 2008.

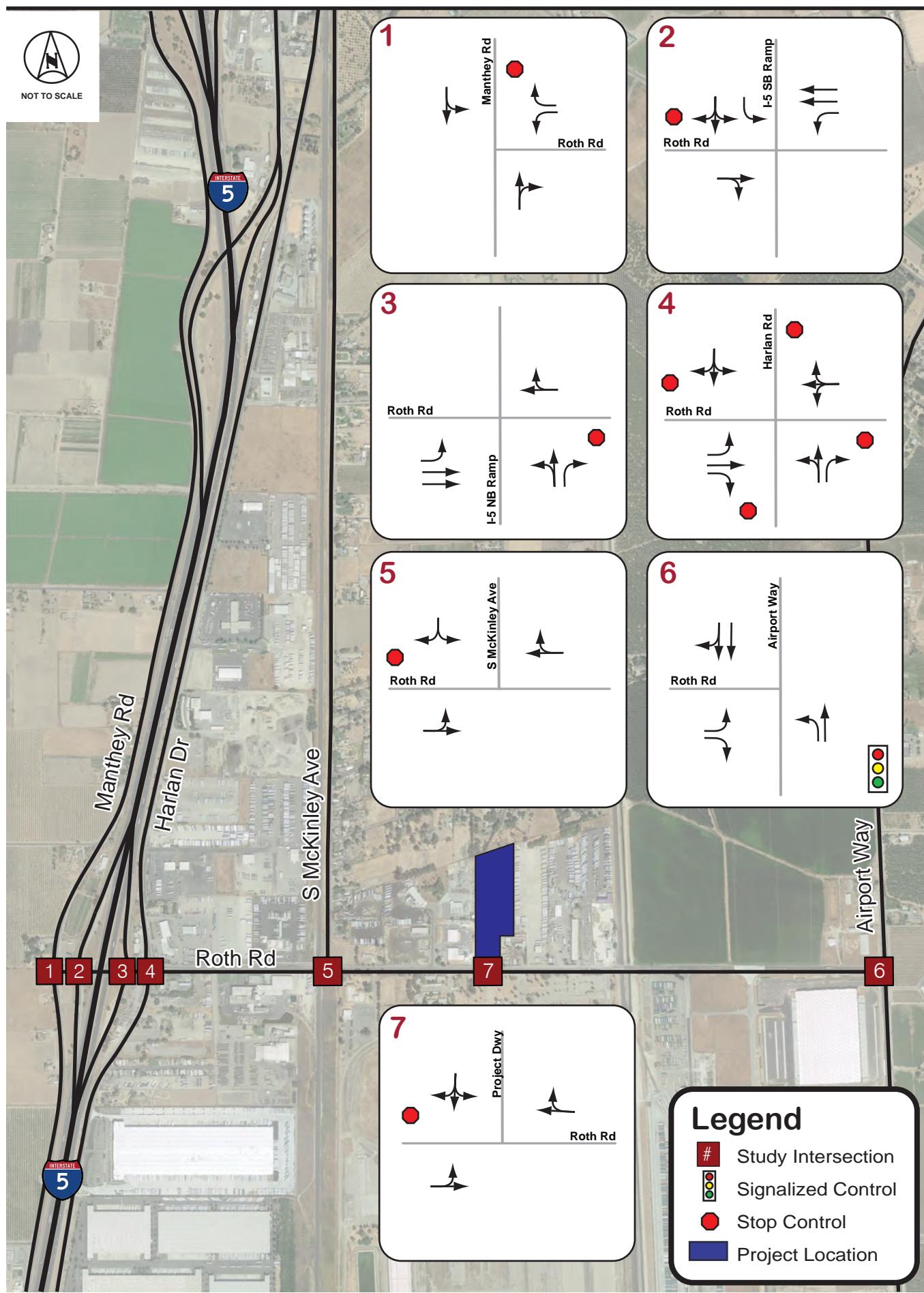
Trans Truck System Truck Facility - Traffic Impact Analysis



Trans Truck System Truck Facility - Traffic Impact Analysis



Trans Truck System Truck Facility - Traffic Impact Analysis



ASSESSMENT OF PROPOSED PROJECT

Proposed Project Trip Generation

The number of trips anticipated to be generated by proposed projects are typically approximated using data included in the *Trip Generation Manual, 9th Edition*, published by the Institute of Transportation Engineers (ITE). This resource provides trip generation information for a Truck Terminal (ITE land use code 30) for AM and PM peak-hours. The trip generation rates for the proposed Project were based on the total number of acres within the project site. The anticipated trip generation for this project is shown in **Table 1**.

Table 1 – Proposed Project Trip Generation

Land Use (ITE Code)	Size (ksf/ acres)	Daily Trips	AM Peak-Hour					PM Peak-Hour					
			Total Trips	In		Out		Total Trips	In		Out		
				%	Trips	%	Trips		%	Trips	%	Trips	
Truck Terminal (30)	4.5 acres	370	33	42%	14	58%	19	29	41%	12	59%	17	
New Project Trips			370	33		14		19	29		12		17

Source: *Trip Generation Manual, 9th Edition*

As shown in **Table 1**, the proposed Project is estimated to generate 370 new external daily trips, 33 new external trips occurring during the AM peak-hour, and 29 new external trips during the PM peak-hour.

Proposed Project Trip Distribution and Assignment

Project traffic was distributed and assigned to the local roadway network based on local understanding of vehicular patterns in the study area, existing traffic volumes, and engineering judgement. The project trip distribution percentages are illustrated in **Figure 4**. Based on the trip distribution, the net new external trips generated by the project were assigned to the street network as shown in **Figure 5**.

Analysis Scenarios

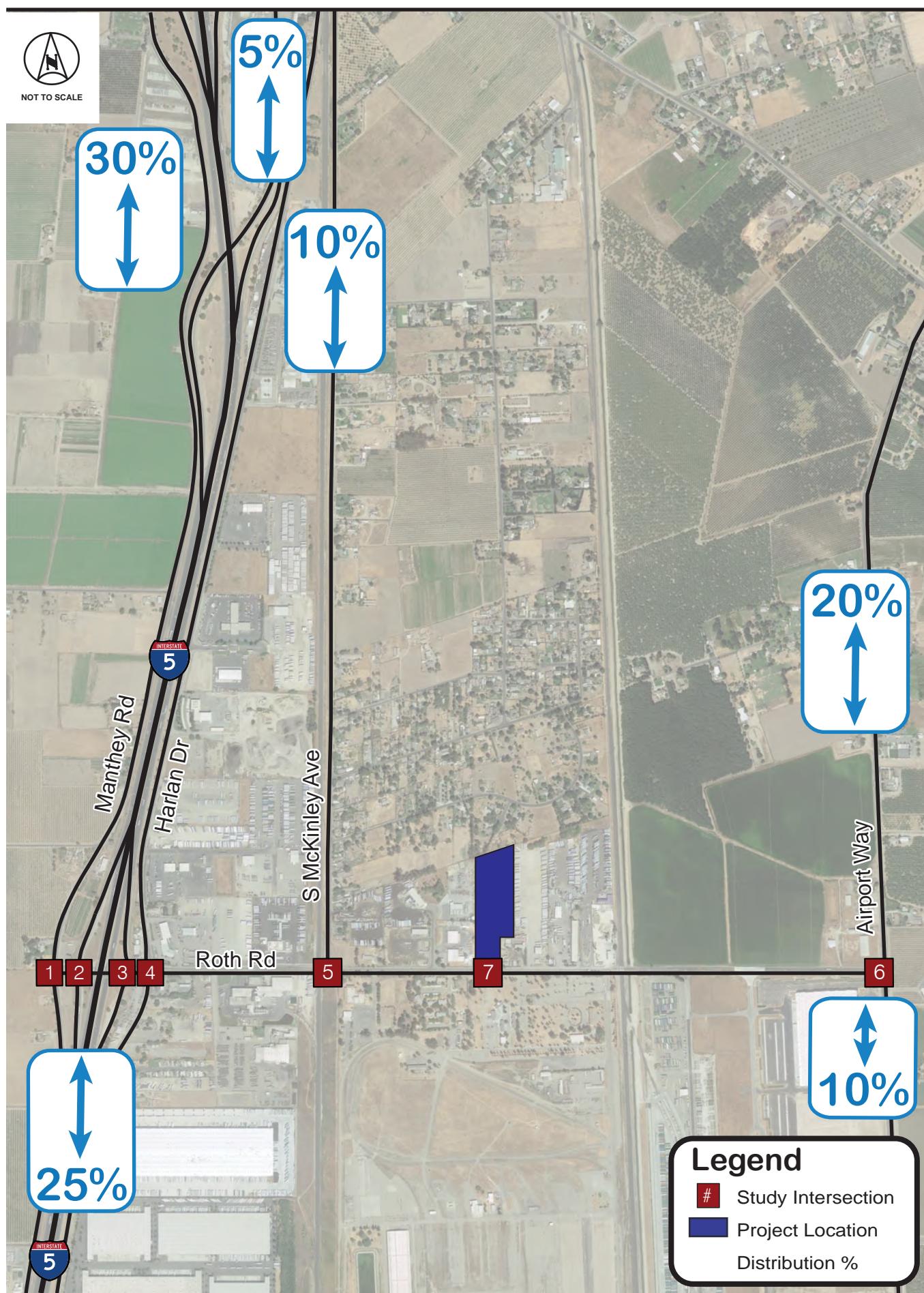
Per San Joaquin County requirements, weekday AM and PM peak-hour Level of Service (LOS) analysis was conducted for the following scenarios:

- A. Existing (2019) Conditions
- B. Existing (2019) plus Approved and Pending Projects Conditions⁺
- C. Existing (2019) plus Approved and Pending Projects plus Proposed Project Conditions
- D. Cumulative (2040) Conditions⁺⁺
- E. Cumulative (2040) plus Proposed Project Conditions

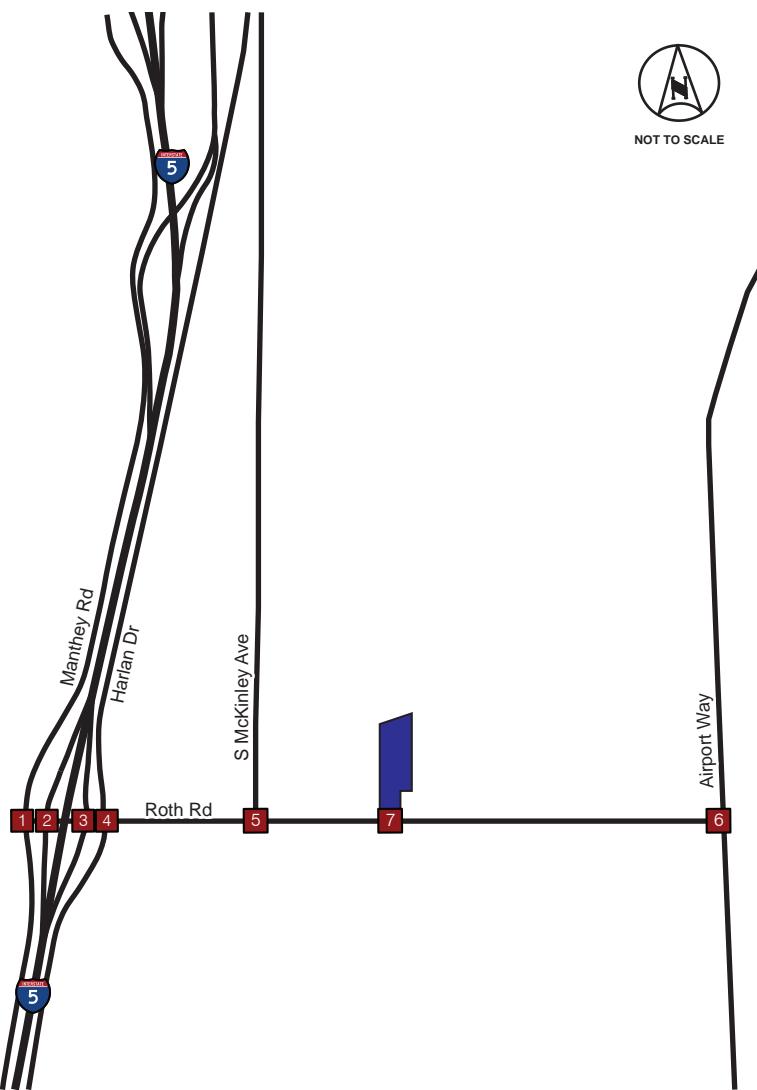
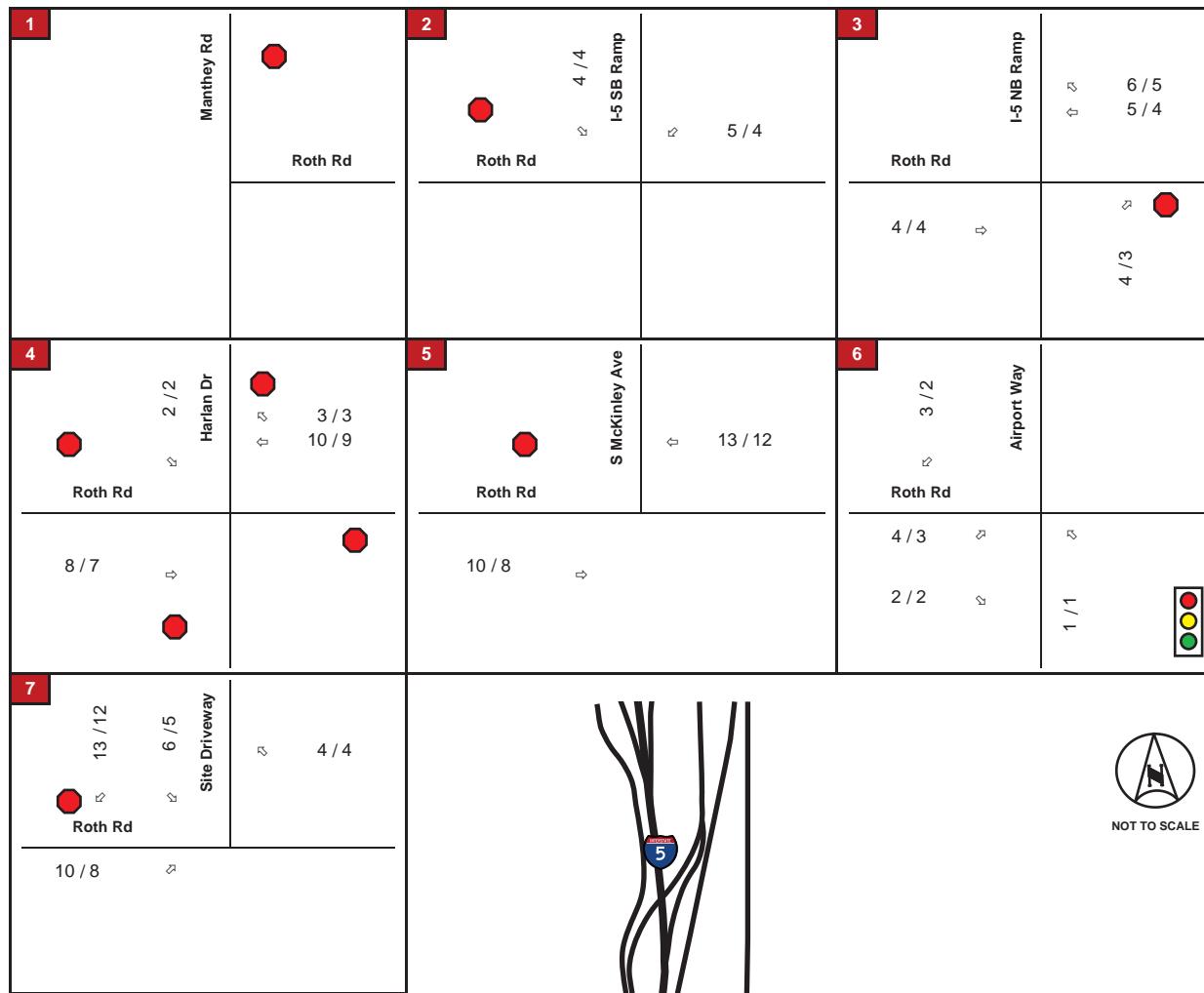
⁺ Includes projects identified by the County as well as projects anticipated by the City of Lathrop.

⁺⁺ Year 2040 is the forecast year associated with the San Joaquin Council of Governments' (SJCOC) current travel demand model. As a result, although the County guidelines¹ specify Cumulative Year 2025, this analysis contemplates Cumulative Year 2040 conditions.

Truck Trans System Truck Facility - Traffic Impact Analysis



Trans Truck System Truck Facility - Traffic Impact Analysis



LEGEND

- | | |
|---|-----------------------------|
| XX/YY | AM/PM Volumes |
|  | Study Intersection |
|  | Signalized
Intersection |
|  | Stop Controlled
Approach |
|  | Project Location |

Kimley »» Horn

*Figure 5
Proposed Project Trip Assignment*

LEVEL OF SERVICE METHODOLOGY

Analysis of transportation facility significant environmental impacts is based on the concept of Level of Service (LOS). The LOS of a facility is a qualitative measure used to describe operational conditions. LOS ranges from A (best), which represents minimal delay, to F (worst), which represents heavy delay and a facility that is operating at or near its functional capacity. Levels of Service for this study were determined using methods defined in the *Highway Capacity Manual, 6th Edition (HCM)*.

Intersection Analysis

The *HCM* includes procedures for analyzing side-street stop controlled (SSSC), all-way stop controlled (AWSC), and signalized intersections. The SSSC procedure defines LOS as a function of average control delay for each minor street approach movement. Conversely, the AWSC and signalized intersection procedures define LOS as a function of average control delay for the intersection as a whole. **Table 2** presents intersection LOS definitions as defined in the *HCM*.

Table 2 – Intersection Level of Service Criteria

Level of Service (LOS)	Un-Signalized	Signalized
	Average Control Delay* (sec/veh)	Average Control Delay (sec/veh)
A	≤ 10	≤ 10
B	> 10 – 15	> 10 – 20
C	> 15 – 25	> 20 – 35
D	> 25 – 35	> 35 – 55
E	> 35 – 50	> 55 – 80
F	> 50	> 80

Source: *Highway Capacity Manual, 6th Edition*

* Applied to the worst lane/lane group(s) for SSSC

Peak-hour LOS will be determined for the facilities, time periods and analysis scenarios identified above using *HCM* methodology with the Synchro 10 analysis software.

Roadway Segment Analysis

Roadway segments under Existing (2019) and Existing (2019) plus Approved and Pending Projects with and without the proposed Project were evaluated using the HCM methodology for analyzing two-lane roadway segments, as shown in **Table 3**. Cumulative (2040) conditions with and without the proposed Project were evaluated by comparing daily traffic volumes to the County's LOS thresholds provided in the *San Joaquin County General Plan*².

Table 3 – Two-Lane Roadway Level of Service Criteria

Level of Service (LOS)	Percent Free-Flow Speed (%)
A	> 91.7
B	> 83.3 – 91.7
C	> 75.0 – 83.3
D	> 66.7 – 75.0
E	≤ 66.7

Source: *Highway Capacity Manual, 6th Edition*

² San Joaquin County General Plan, Table 4.4, December 2016.

Freeway Facility Analysis

Caltrans' traffic study guidelines³ specify the use of vehicle density (passenger cars/mile/lane) as the appropriate measure of effectiveness for freeway facilities. The LOS criteria for basic freeway segments and freeway merge/diverge segments are summarized in Table 4.

Table 4 – Freeway Facility Level of Service Criteria

Level of Service (LOS)	<u>Basic Segments</u> Density (pc/mi/ln)	<u>Merge/Diverge Segments</u> Density (pc/mi/ln)
A	≤ 11	≤ 10
B	> 11 – 18	> 10 – 20
C	> 18 – 26	> 20 – 28
D	> 26 – 35	> 28 – 35
E	> 35 – 45	> 35
F*	> 45*	*

Source: Highway Capacity Manual, 6th Edition

* Demand exceeds capacity

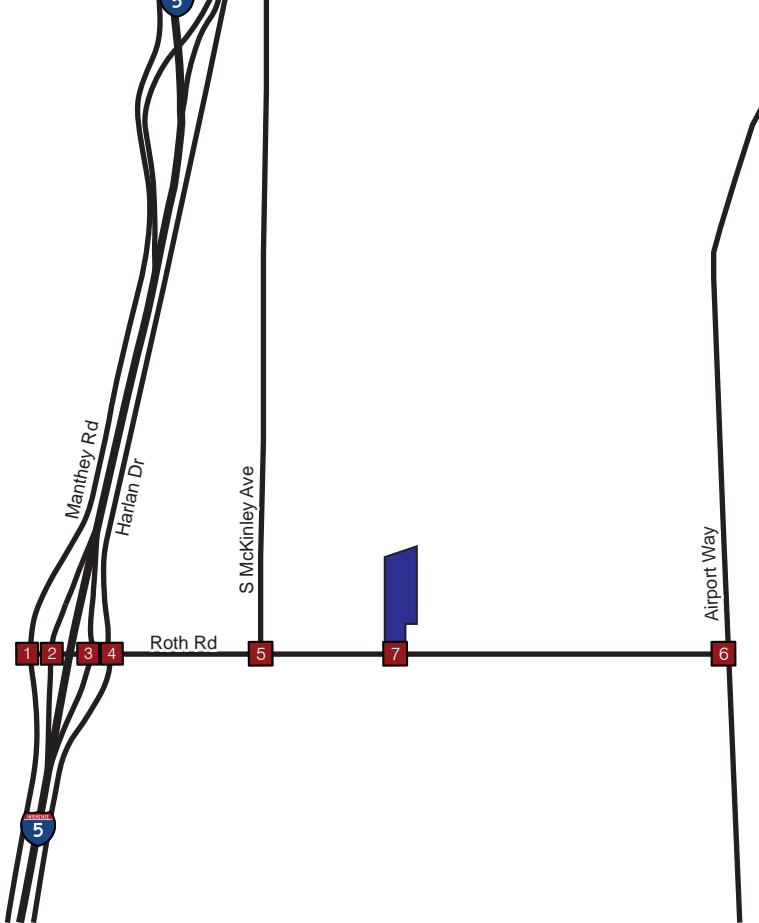
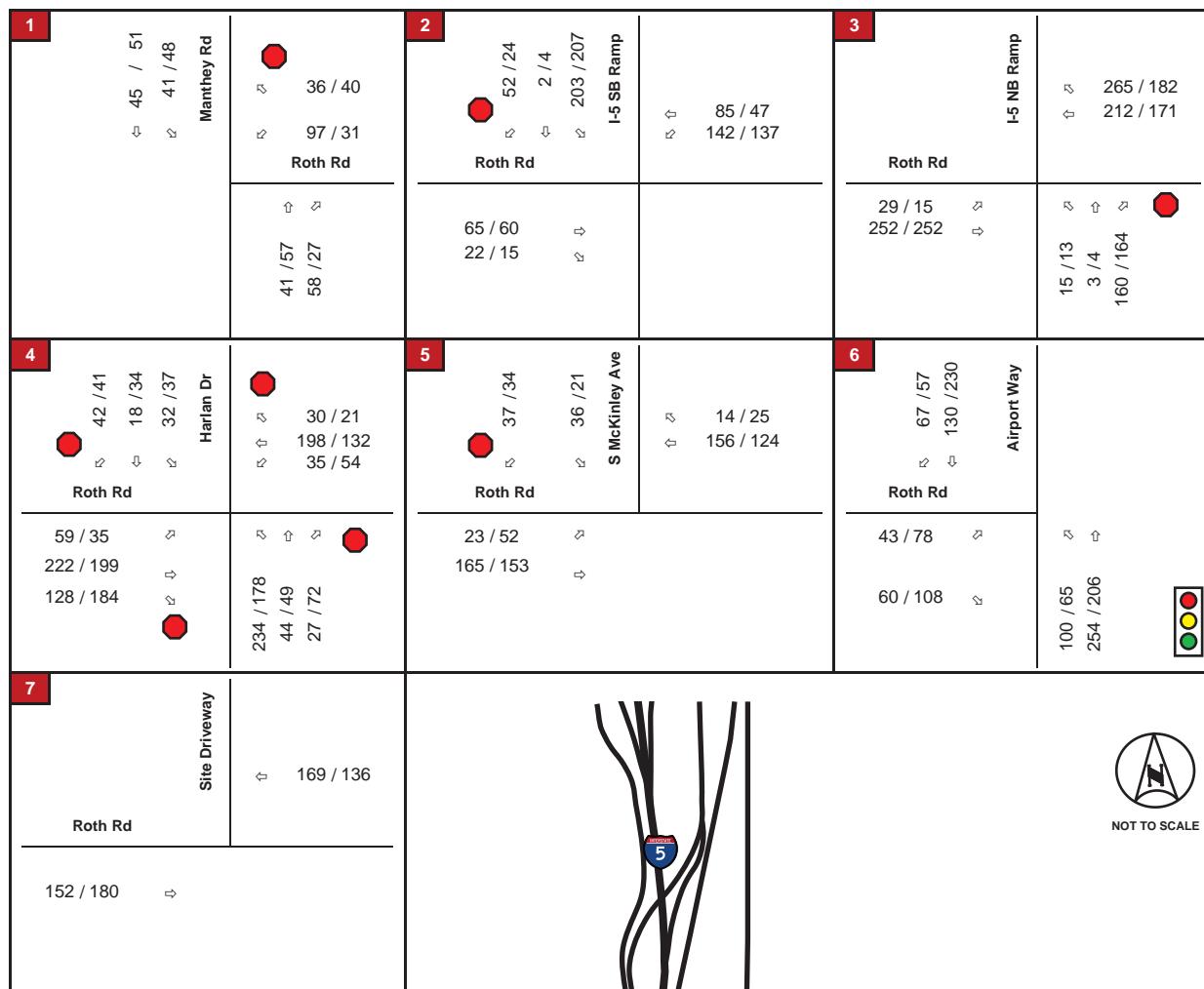
EXISTING (2019) CONDITIONS

New weekday AM and PM peak-period intersection turning movement traffic counts were conducted on Tuesday, April 30, 2019, from 6:00-8:00 AM and 5:00-7:00 PM, respectively. New roadway segment daily counts were conducted on Tuesday, April 30 and Wednesday, May 1, 2019. The project driveway along Frontage Road (Intersection #4) does not exist today and, therefore, existing counts were not required.

Existing (2019) peak-hour turning movement volumes are presented in **Figure 6**, and the traffic count data sheets are provided in **Appendix A**. Analysis worksheets for this scenario are provided in **Appendix B**. **Table 5** presents the intersection operating conditions, **Table 6** presents the roadway operating conditions, and **Table 7** presents the freeway operating conditions for this analysis scenario. As shown, the study facilities operate between LOS B and LOS D during the AM and PM peak-hours.

³ Guide for the Preparation of Traffic Impact Studies, Caltrans, December 2002.

Trans Truck System Truck Facility - Traffic Impact Analysis



NOT TO SCALE

LEGEND	
XX/YY	AM/PM Volumes
#	Study Intersection
	Signalized Intersection
	Stop Controlled Approach
	Project Location

Table 5 – Existing (2019) Intersection Levels of Service

ID	Intersection	Control	Threshold	Peak Hour	Existing (2019)	
					Delay (sec)	LOS
1	Roth Road @ Manthey Rd	SSSC	E	AM	12.2 (WBL)	B
				PM	10.5 (WBL)	B
2	Roth Road @ I-5 SB Ramp	SSSC	D	AM	23.2 (SBL)	C
				PM	17.6 (SBL)	C
3	Roth Road @ I-5 NB Ramp	SSSC	D	AM	19.2 (NBL)	C
				PM	15.4 (NBL)	C
4	Roth Road @ Harlan Road	AWSC	D	AM	34.5	D
				PM	20.6	C
5	Roth Road @ McKinley Ave	SSSC	D	AM	11.4 (SBLR)	B
				PM	11 (SBLR)	B
6	Roth Road @ Airport Way	Signal	D	AM	10.8	B
				PM	11.2	B

Side Street Stop Controlled (SSSC) intersection LOS corresponds to the worst approach.

Bolded represents unacceptable conditions.

Table 6 – Existing (2019) Roadway Levels of Service

Location	AM Peak-Hour					
	NB/EB			SB/WB		
	PFFS	v/c	LOS	PFFS	v/c	LOS
Roth Rd, west of McKinley Ave	86.4	0.17	B	86.6	0.15	B
Roth Road, east of McKinley Ave	85.7	0.15	B	85.6	0.15	B

Location	PM Peak-Hour					
	NB/EB			SB/WB		
	PFFS	v/c	LOS	PFFS	v/c	LOS
Roth Rd, west of McKinley Ave	85.8	0.19	B	86.3	0.15	B
Roth Road, east of McKinley Ave	85.3	0.17	B	85.3	0.16	B

Table 7 – Existing (2019) Freeway Levels of Service

I-5				Existing (2019)	
Direction	Segment	Type	Peak Hour	Density ^a	LOS
NB	South of Roth Road	Diverge	AM	20.1	C
			PM	23.5	C
	North of Roth Road	Merge	AM	24.5	C
			PM	27.7	C
SB	North of Roth Road	Diverge	AM	21.1	C
			PM	21.0	C
	South of Roth Road	Merge	AM	24.4	C
			PM	24.5	C

Notes:

a- Density measured in passenger cars/lane/mile (pc/ln/mi)

b- **Bold** represents unacceptable operations

EXISTING (2019) PLUS APPROVED AND PENDING PROJECTS

Per County guidance², the Existing (2019) plus Approved and Pending Project Conditions included the vehicle trips associated with the JDL Truck Facility project proposed to be located at 75 East Equipment Street in San Joaquin County. The number of trips expected to be generated by the proposed warehouse at the truck parking facility was derived using the ITE Land Use Code 30 (Truck Terminal) and were distributed and assigned to the local roadway network based on existing traffic volumes, knowledge of local traffic patterns, and engineering judgement. In addition, the City of Lathrop's 2018 Traffic Monitoring Program⁴ identifies vehicle trips associated with projects anticipated by Year 2022. These projects include approximately 2,100 new single-family units, 350 new apartment units, 7,300,597 square feet of new warehouse/distribution facilities, 388,195 square feet of new commercial, and 196 new hotel rooms. Since these projects are anticipated by the City of Lathrop within the next three years, the Existing (2019) plus Approved and Pending Projects conditions includes the vehicle trips associated with these projects. As directed by the County, the additional trips associated with the approved projected were added to the collected counts.

The peak-hour traffic volumes for Existing (2019) plus Approved and Pending Projects Conditions are shown in **Figure 7**. The analysis worksheets for this scenario are provided in **Appendix C**. **Table 8** presents the intersection operating conditions, **Table 9** presents the roadway operating conditions, and **Table 10** presents the freeway operating conditions for this analysis scenario. As shown, the study facilities operate between LOS B and LOS F during the AM and PM peak-hours.

⁴ City of Lathrop 'Traffic Monitoring Program', Crane Transportation Group, September 13, 2018.

Trans Truck System Truck Facility - Traffic Impact Analysis

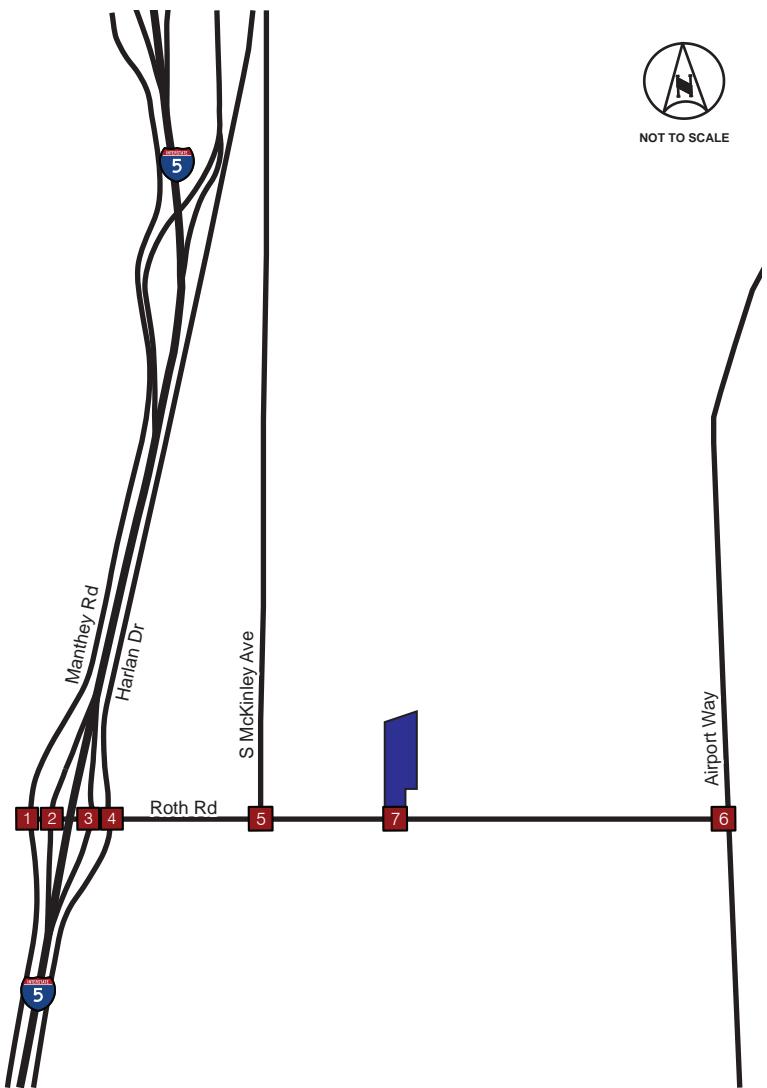
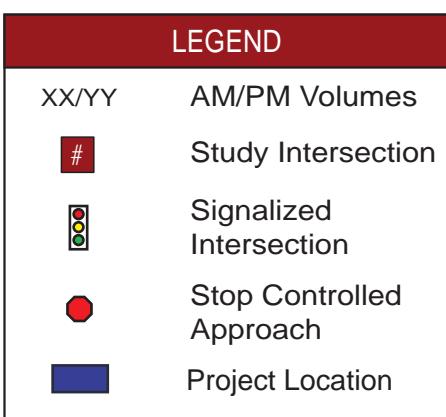
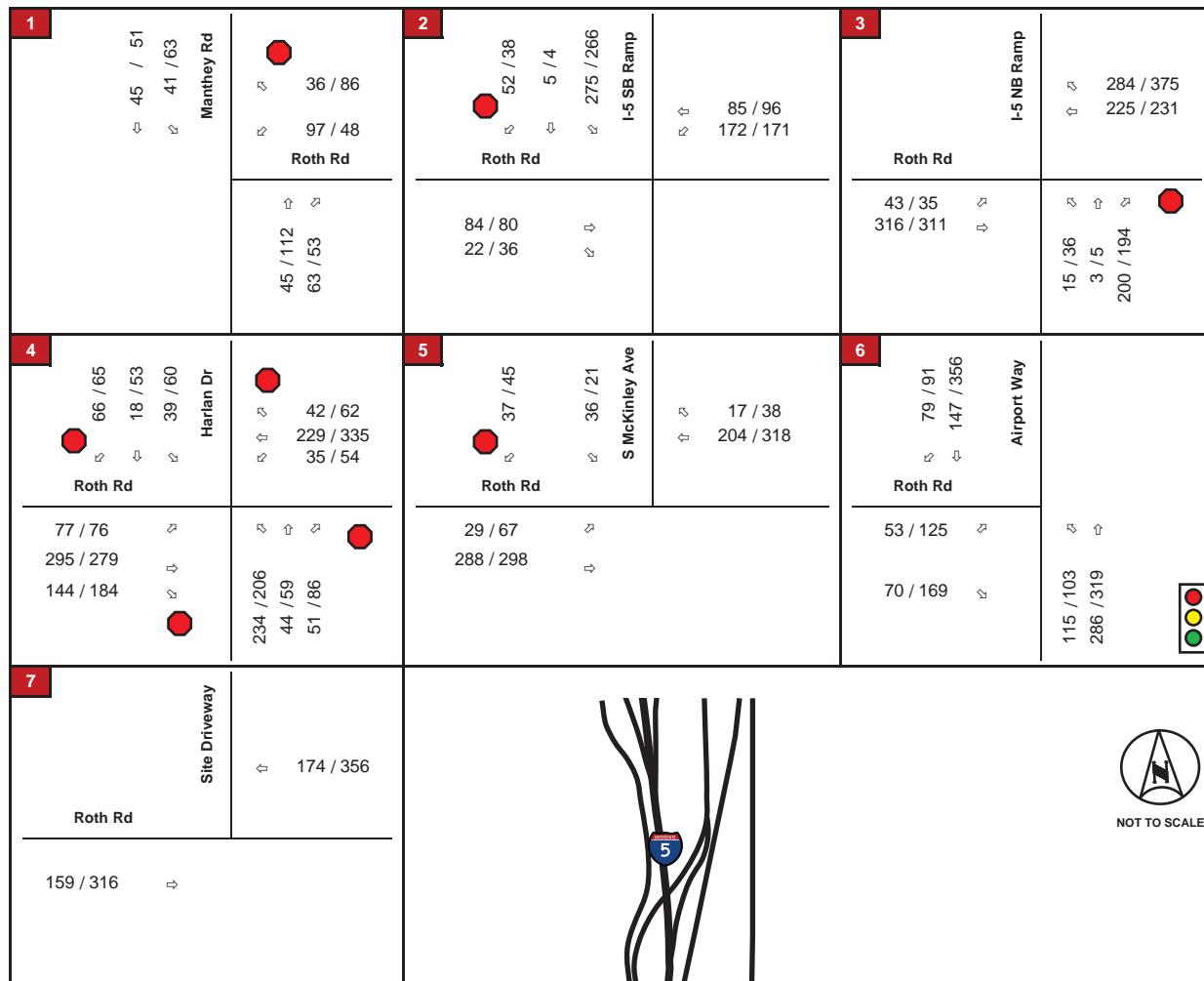


Table 8 – Existing (2019) plus Approved and Pending Projects Intersection Levels of Service

ID	Intersection	Control	Threshold	Peak Hour	Existing (2019) plus Approved Projects	
					Delay (sec)	LOS
1	Roth Road @ Manthey Rd	SSSC	E	AM	12.4 (WBL)	B
				PM	11.9 (WBL)	B
2	Roth Road @ I-5 SB Ramp	SSSC	D	AM	44.8 (SBL)	E
				PM	32.7 (SBL)	D
3	Roth Road @ I-5 NB Ramp	SSSC	D	AM	23.1 (NBL)	C
				PM	25.7 (NBL)	D
4	Roth Road @ Harlan Road	AWSC	D	AM	57.9	F
				PM	141.9	F
5	Roth Road @ McKinley Ave	SSSC	D	AM	13.4 (SBLR)	B
				PM	15.9 (SBLR)	C
6	Roth Road @ Airport Way	Signal	D	AM	12.1	B
				PM	14.1	B

Side Street Stop Controlled (SSSC) intersection LOS corresponds to the worst approach.

Bolded represents unacceptable conditions.

Table 9 – Existing (2019) plus Approved and Pending Projects Roadway Levels of Service

Location	AM Peak-Hour					
	NB/EB			SB/WB		
	PFFS	v/c	LOS	PFFS	v/c	LOS
Roth Rd, west of McKinley Ave	82.6	0.29	C	82.8	0.28	C
Roth Road, east of McKinley Ave	83.4	0.24	B	83.2	0.24	C

Location	PM Peak-Hour					
	NB/EB			SB/WB		
	PFFS	v/c	LOS	PFFS	v/c	LOS
Roth Rd, west of McKinley Ave	81.6	0.33	C	82.3	0.28	C
Roth Road, east of McKinley Ave	82.8	0.26	C	82.9	0.25	C

Table 10 – Existing (2019) plus Approved and Pending Projects Freeway Levels of Service

I-5				Existing (2019) plus Approved Projects	
Direction	Segment	Type	Peak Hour	Density ^a	LOS
NB	South of Roth Road	Diverge	AM	20.3	C
			PM	23.8	C
	North of Roth Road	Merge	AM	24.5	C
			PM	29.1	D
SB	North of Roth Road	Diverge	AM	21.5	C
			PM	21.4	C
	South of Roth Road	Merge	AM	24.3	C
			PM	24.6	C

Notes:

a- Density measured in passenger cars/lane/mile (pc/ln/mi)

b- **Bold** represents unacceptable operations

EXISTING (2019) PLUS APPROVED AND PENDING PROJECTS PLUS PROPOSED PROJECT CONDITIONS

Peak-hour traffic associated with the proposed Project was added to the Existing (2019) plus Approved and Pending Projects traffic volumes previously noted, and levels of service were determined at the study intersections. Existing (2019) plus Approved and Pending Projects plus Proposed Project peak-hour traffic volumes are presented in **Figure 8**. The analysis worksheets for this scenario are provided in **Appendix D**.

Table 11 provides the intersection operating conditions, **Table 12** provides the roadway operating conditions, and **Table 13** provides the freeway operating conditions for this analysis scenario. As shown, the study facilities operate from LOS A to LOS F during the AM and PM peak-hours.

Table 11 – Existing (2019) plus Approved and Pending Projects plus Proposed Project Intersection Levels of Service

ID	Intersection	Control	Threshold	Peak Hour	Existing (2019) plus Approved Projects		Existing (2019) plus Approved Projects plus Project	
					Delay (sec)	LOS	Delay (sec)	LOS
1	Roth Road @ Manthey Rd	SSSC	E	AM	12.4 (WBL)	B	12.4 (WBL)	B
				PM	11.9 (WBL)	B	11.9 (WBL)	B
2	Roth Road @ I-5 SB Ramp	SSSC	D	AM	44.8 (SBL)	E	49.7 (SBL)	E
				PM	32.7 (SBL)	D	34.8 (SBL)	D
3	Roth Road @ I-5 NB Ramp	SSSC	D	AM	23.1 (NBL)	C	23.7 (NBL)	C
				PM	25.7 (NBL)	D	26.2 (NBL)	D
4	Roth Road @ Harlan Road	AWSC	D	AM	57.9	F	64.6	F
				PM	141.9	F	152.1	F
5	Roth Road @ McKinley Ave	SSSC	D	AM	13.4 (SBLR)	B	13.7 (SBLR)	B
				PM	15.9 (SBLR)	C	16.3 (SBLR)	C
6	Roth Road @ Airport Way	Signal	D	AM	12.1	B	12.2	B
				PM	14.1	B	14.3	B
7	Roth Road @ Site Driveway	SSSC	D	AM	NOT ANALYZED IN THIS SCENARIO		9.9 (SBLR)	A
				PM	THIS SCENARIO		11.9 (SBLR)	B

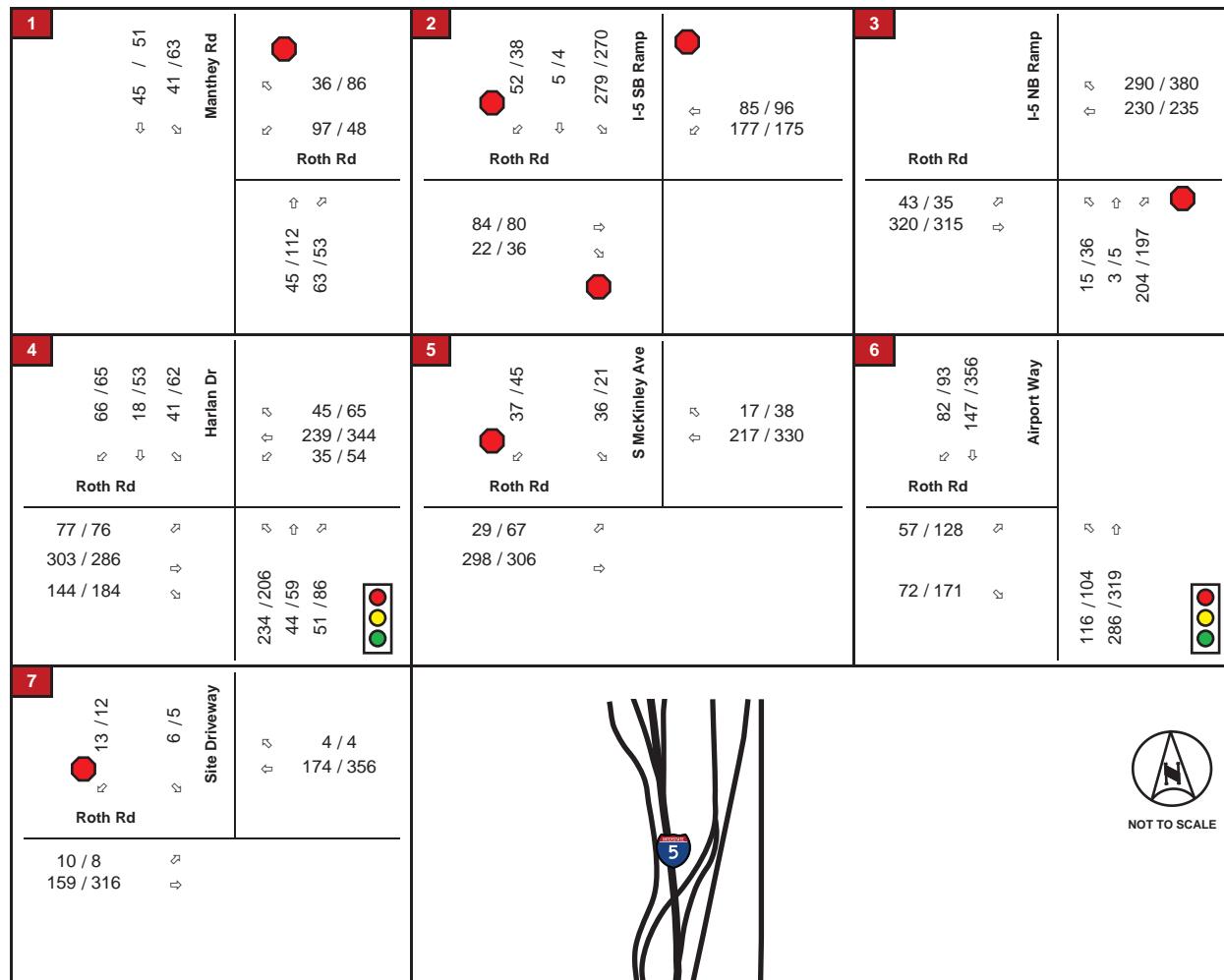
Notes:

Side Street Stop Controlled (SSSC) intersection LOS corresponds to the worst approach.

Bolded represents unacceptable conditions.

Shaded represents a significant impact.

Trans Truck System Truck Facility - Traffic Impact Analysis



NOT TO SCALE

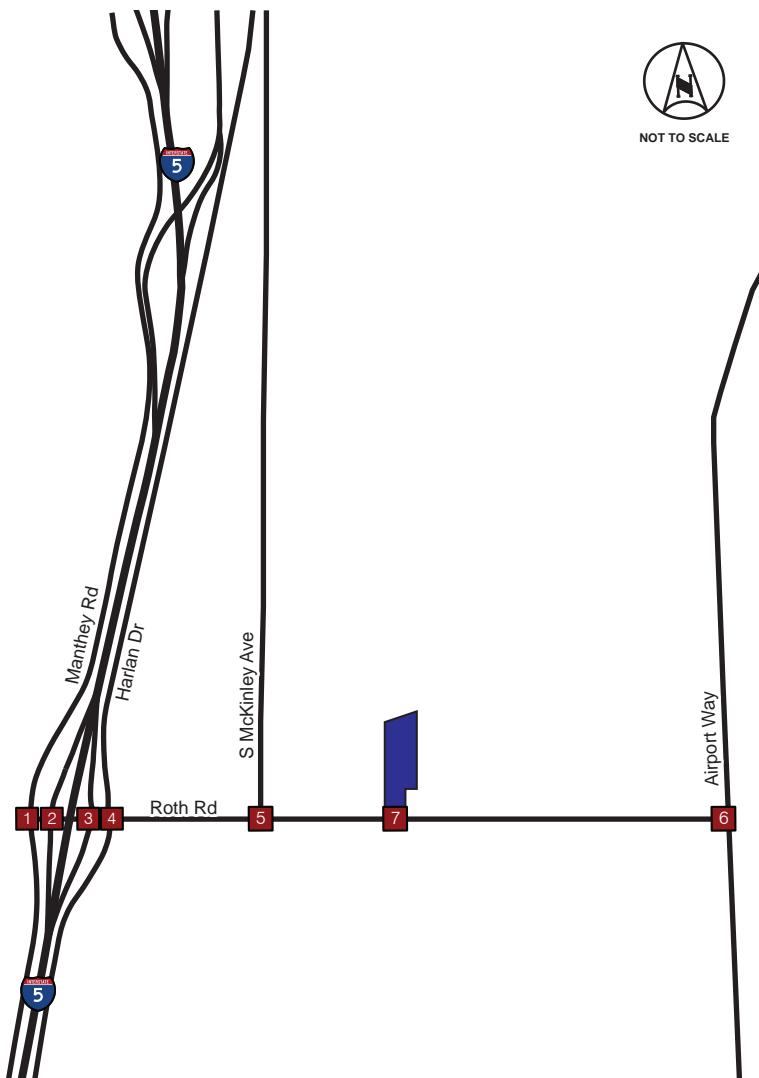
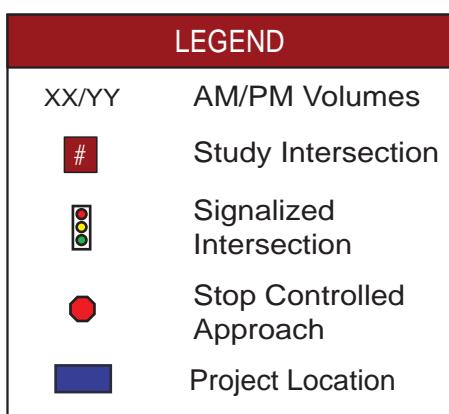


Table 12 – Existing (2019) plus Approved and Pending Projects plus Proposed Project Roadway Levels of Service

Location	AM Peak-Hour					
	NB/EB			SB/WB		
	PFFS	v/c	LOS	PFFS	v/c	LOS
Roth Rd, west of McKinley Ave	82.3	0.30	C	82.5	0.29	C
Roth Road, east of McKinley Ave	83.2	0.24	C	82.5	0.25	C
Location	PM Peak-Hour					
	NB/EB			SB/WB		
	PFFS	v/c	LOS	PFFS	v/c	LOS
Roth Rd, west of McKinley Ave	81.2	0.34	C	81.9	0.29	C
Roth Road, east of McKinley Ave	82.6	0.27	C	82.6	0.26	C

Table 13 – Existing (2019) plus Approved and Pending Projects plus Proposed Project Freeway Levels of Service

I-5				Existing (2019) plus Approved Projects		Existing (2019) plus Approved Projects plus Proposed Project	
Direction	Segment	Type	Peak Hour	Density ^a	LOS	Density ^a	LOS
NB	South of Roth Road	Diverge	AM	20.3	C	20.4	C
			PM	23.8	C	23.8	C
	North of Roth Road	Merge	AM	24.5	C	28.4	D
			PM	29.1	D	32.5	D
SB	North of Roth Road	Diverge	AM	21.5	C	21.5	C
			PM	21.4	C	21.4	C
	South of Roth Road	Merge	AM	24.3	C	29.4	D
			PM	24.6	C	27.7	C

Notes:

a- Density measured in passenger cars/lane/mile (pc/ln/mi)

b- **Bold** represents unacceptable operations

CUMULATIVE (2040) CONDITIONS

For this study, consistent with the *Guidelines*, Cumulative year traffic volumes were obtained from the regional traffic model maintained by the San Joaquin Council of Governments (SJCOC). The current version of the SJCOC travel demand model is based on year 2040 Cumulative Conditions. To obtain turning movement volumes for Cumulative (2040) Conditions, the forecasted growth from the traffic model, calculated by taking the difference between the base year and the forecast year, was added to the Existing (2019) volumes to develop Cumulative (2040) volumes. The growth between the two model years was calculated using the approach volumes obtained from the AM and PM weekday peak-hour model network. Existing (2019) plus Approved and Pending Project volumes were maintained for any approach where negative growth was calculated. Cumulative (2040) peak-hour turning movement volumes are presented in **Figure 9**. The analysis worksheets for this scenario are provided in **Appendix E**.

Table 14 provides the intersection operating conditions, **Table 15** provides the roadway operating conditions, and **Table 16** provides the freeway operating conditions for this analysis scenario. As shown, the study facilities operate from LOS B to LOS D during the AM and PM peak-hours.

Table 14 – Cumulative (2040) Intersection Levels of Service

ID	Intersection	Control	Threshold	Peak Hour	Cumulative (2040)	
					Delay (sec)	LOS
1	Roth Road @ Manthey Rd	SSSC	E	AM	22.7 (WBL)	C
				PM	15.3 (WBL)	C
2	Roth Road @ I-5 SB Ramp	AWSC	D	AM	16.2	C
				PM	14.1	B
3	Roth Road @ I-5 NB Ramp	AWSC	D	AM	27.4	D
				PM	18.9	C
4	Roth Road @ Harlan Road	Signal	D	AM	46.1	D
				PM	31.7	C
5	Roth Road @ McKinley Ave	SSSC	D	AM	13.7 (SBLR)	B
				PM	18.7 (SBLR)	C
6	Roth Road @ Airport Way	Signal	D	AM	28.4	C
				PM	15.8	B

Side Street Stop Controlled (SSSC) intersection LOS corresponds to the worst approach.

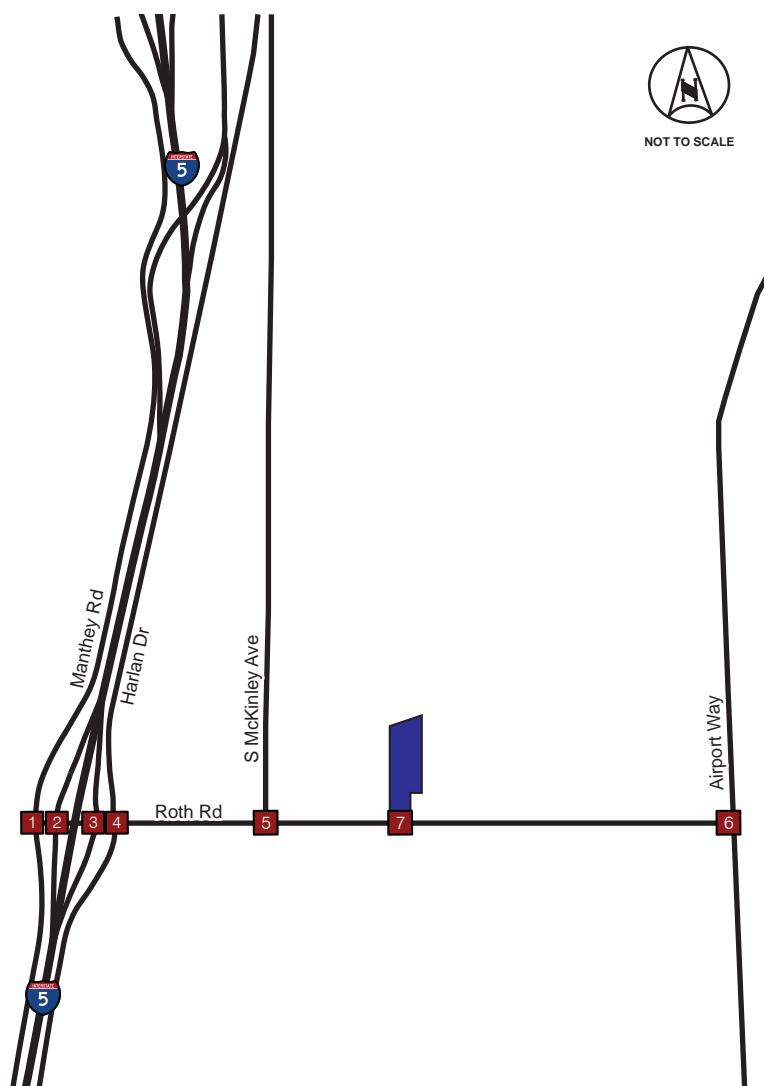
Bolded represents unacceptable conditions.

Table 15 – Cumulative (2040) Roadway Level of Service

Location	Daily Capacity*	Daily Volume	LOS
Roth Rd, west of McKinley Ave	35,000	12,730	A
Roth Road, east of McKinley Ave	35,000	13,890	A

Trans Truck System Truck Facility - Traffic Impact Analysis

1 ↳ 100 / 130 ↲ 90 / 90 Manthey Rd	2 ↳ 70 / 86 ↳ 260 / 100 Roth Rd	3 ↳ 150 / 80 ↳ 10 / 20 ↳ 300 / 300 I-5 SB Ramp	4 ↳ 80 / 70 ↳ 30 / 60 ↳ 40 / 60 Harlan Dr	5 ↳ 100 / 150 ↳ 160 / 90 150 / 130 80 / 60 Roth Rd	6 ↳ 180 / 100 ↳ 220 / 220 Roth Rd	7 ↳ 150 / 80 ↳ 330 / 370 ↳ 370 / 375 ↳ 290 / 250 I-5 NB Ramp
100 / 76 295 / 279 180 / 260	42 / 62 270 / 335 40 / 60	100 / 100 160 / 90	320 / 250 50 / 70 51 / 86	40 / 80 288 / 298	30 / 70 204 / 318	100 / 60 30 / 20 250 / 194
220 / 316	260 / 356					
Site Driveway						
Roth Rd						



LEGEND	
XX/YY	AM/PM Volumes
#	Study Intersection
.Signal	Signalized Intersection
●	Stop Controlled Approach
■	Project Location

Table 16 – Cumulative (2040) Freeway Level of Service

I-5				Cumulative (2040)	
Direction	Segment	Type	Peak Hour	Density ^a	LOS
NB	South of Roth Road	Diverge	AM	23.6	C
			PM	26.5	C
	North of Roth Road	Merge	AM	28.4	D
			PM	32.5	D
SB	North of Roth Road	Diverge	AM	26.2	C
			PM	24.3	C
	South of Roth Road	Merge	AM	29.4	D
			PM	27.7	C

Notes:

a- Density measured in passenger cars/lane/mile (pc/ln/mi)

b- **Bold** represents unacceptable operations

CUMULATIVE (2040) PLUS PROPOSED PROJECT CONDITIONS

Peak-hour traffic associated with the proposed Project was added to the Cumulative (2040) traffic volumes previously noted, and levels of service were determined at the study intersections. Cumulative (2040) plus Proposed Project peak-hour turning movement volumes are presented in **Figure 10**. Analysis worksheets for this scenario are provided in **Appendix F**.

Table 17 provides the intersection operating conditions, **Table 18** provides the roadway operating conditions, and **Table 19** provides the freeway operating conditions for this analysis scenario. As shown, the study facilities operate from LOS B to LOS D during the AM and PM peak-hours.

Table 17 – Cumulative (2040) plus Proposed Project Intersection Levels of Service

ID	Intersection	Control	Threshold	Peak Hour	Cumulative (2040)		Cumulative (2040) plus Project	
					Delay (sec)	LOS	Delay (sec)	LOS
1	Roth Road @ Manthey Rd	SSSC	E	AM	22.7 (WBL)	C	22.7 (WBL)	C
				PM	15.3 (WBL)	C	15.3 (WBL)	C
2	Roth Road @ I-5 SB Ramp	AWSC	D	AM	16.2	C	16.5	C
				PM	14.1	B	14.2	B
3	Roth Road @ I-5 NB Ramp	AWSC	D	AM	27.4	D	28.8	D
				PM	18.9	C	19.4	C
4	Roth Road @ Harlan Road	Signal	D	AM	46.1	D	48.6	D
				PM	31.7	C	32.7	C
5	Roth Road @ McKinley Ave	SSSC	D	AM	13.7 (SBLR)	B	14 (SBLR)	B
				PM	18.7 (SBLR)	C	19.3 (SBLR)	C
6	Roth Road @ Airport Way	Signal	D	AM	28.4	C	29.0	C
				PM	15.8	B	15.9	B
7	Roth Road @ Site Driveway	SSSC	D	AM	NOT ANALYZED IN THIS SCENARIO		10.7 (SBLR)	B
				PM	THIS SCENARIO		11.9 (SBLR)	B

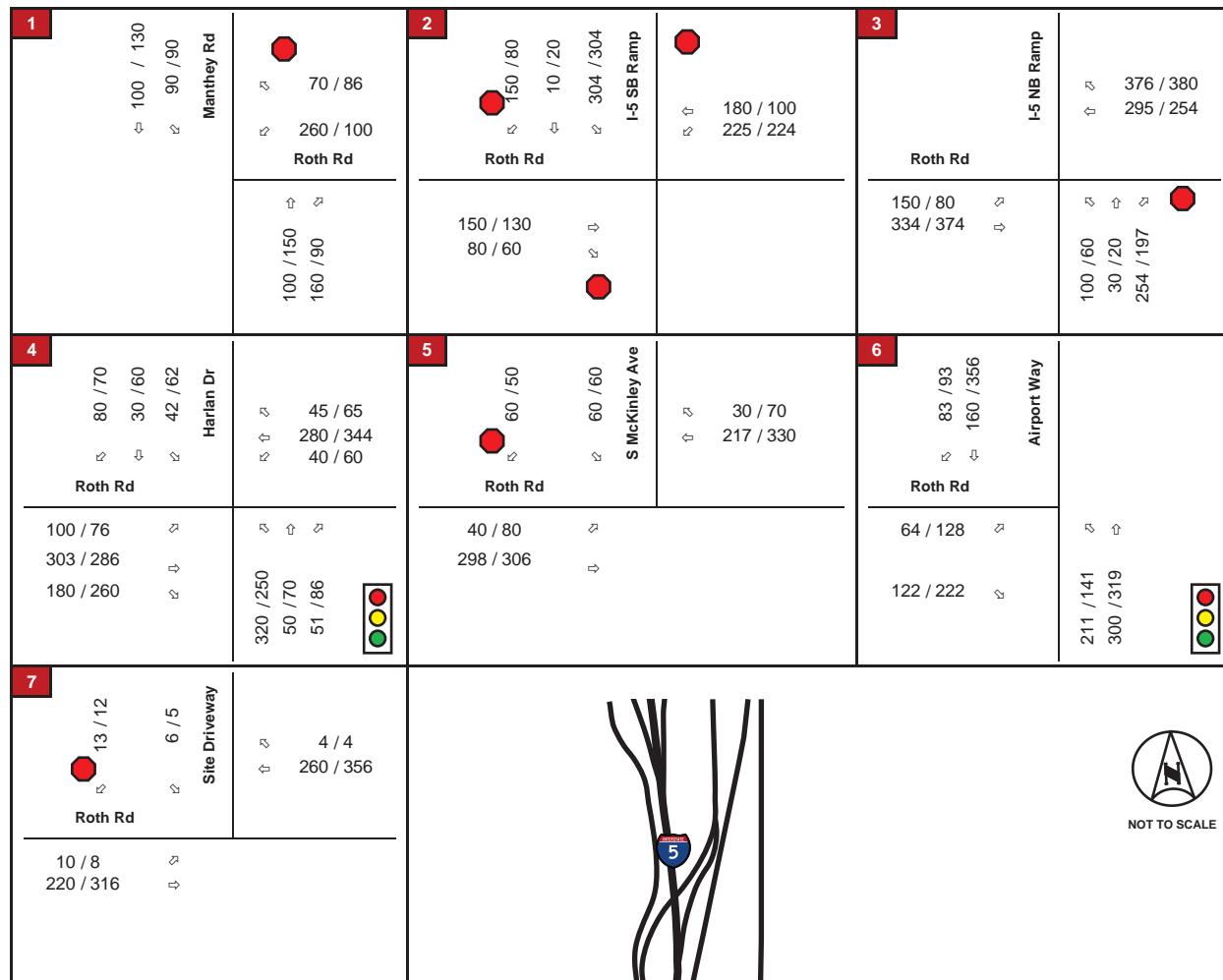
Notes:

Side Street Stop Controlled (SSSC) intersection LOS corresponds to the worst approach.

Bolded represents unacceptable conditions.

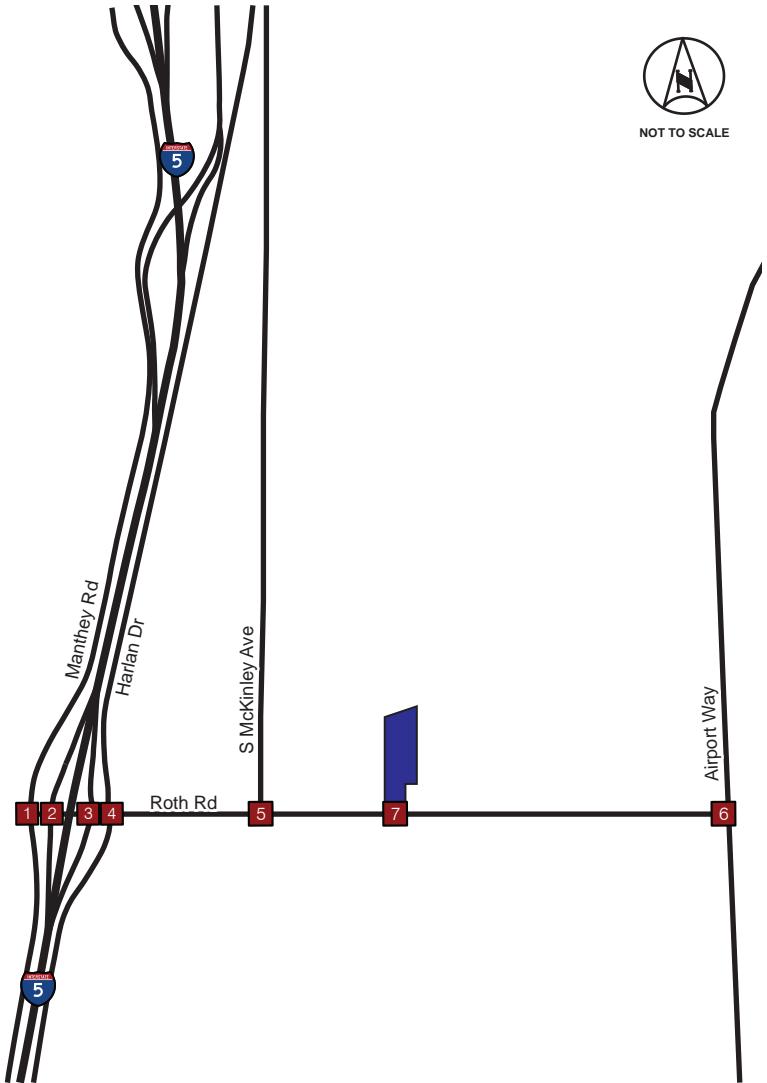
Shaded represents a significant impact.

Trans Truck System Truck Facility - Traffic Impact Analysis



NOT TO SCALE

LEGEND	
XX/YY	AM/PM Volumes
#	Study Intersection
	Signalized Intersection
	Stop Controlled Approach
	Project Location



Kimley»Horn Cumulative (2040) plus Proposed Project Peak-Hour Traffic Volumes Figure 10

Table 18 – Cumulative (2040) plus Proposed Project Roadway Level of Service

Location	Daily Capacity*	Daily Volume	LOS
Roth Rd, west of McKinley Ave	35,000	12,860	A
Roth Road, east of McKinley Ave	35,000	14,020	A

Table 19 – Cumulative (2040) plus Proposed Project Freeway Level of Service

I-5				Cumulative (2040)		Cumulative (2040) plus Proposed Project	
Direction	Segment	Type	Peak Hour	Density ^a	LOS	Density ^a	LOS
NB	South of Roth Road	Diverge	AM	23.6	C	23.6	C
			PM	26.5	C	26.6	C
	North of Roth Road	Merge	AM	28.4	D	28.4	D
			PM	32.5	D	32.5	D
SB	North of Roth Road	Diverge	AM	26.2	C	26.2	C
			PM	24.3	C	24.3	C
	South of Roth Road	Merge	AM	29.4	D	29.4	D
			PM	27.7	C	27.7	C

Notes:

a- Density measured in passenger cars/lane/mile (pc/ln/mi)

b- **Bold** represents unacceptable operations

IMPACTS AND MITIGATION

Standards of Significance

Project impacts were determined by comparing conditions with the proposed Project to those without the project. Impacts are created when traffic from the proposed Project forces the LOS to fall below a specific threshold. The proposed Project study facilities are under the jurisdiction of San Joaquin County, the City of Lathrop, Caltrans, or the City of Manteca. The County standards specify the following:

“As defined in the San Joaquin County 2010 General Plan, adopted in 1992, all County roadways shall operate at a LOS of C or better (except in a City sphere of influence where the City had adopted LOS D); intersections shall operate at an overall LOS D or better on minor arterials and roadways of higher classification; and LOS C on all other roads; all freeways and State highways shall operate at a LOS D. The methods contained in the ‘Transportation Research Board, 1997 Highway Capacity Manual’ (or latest edition) shall be used to determine LOS...

If the LOS for conditions at a given location is already at an unacceptable LOS, then the impacts must be assessed in terms of...delay (for intersection approaches)...If the delay at a given intersection approach under the ‘Existing plus Approved Projects plus Proposed Project’ conditions or the “Cumulative plus Proposed Project” conditions exceeds the delay for the same intersection approach under ‘Existing plus Approved Projects Conditions’ or ‘Cumulative Conditions’, then mitigation measures that would return the delay to the ‘Existing’ or ‘Cumulative’ level must be identified.”

Table 20 presents the significance threshold criteria for each jurisdiction.

Table 20 – Significance Criteria

Jurisdiction	Signalized Intersections	Side-Street Stop Control	All-Way Stop Control	Roadway Segment	Delay Threshold for Intersections with Unacceptable "No Project" LOS
San Joaquin County	LOS D	LOS D	LOS D	LOS C	N/A
City of Lathrop	LOS D	LOS E	LOS D	LOS D	≥ 3.0 seconds
Caltrans	LOS D	LOS D	LOS D	N/A	≥ 0.1 seconds
City of Manteca	LOS D	LOS D	LOS D	LOS D	≥ 3.0 seconds

Source: Sunrise Trucking Parking Lot, San Joaquin County Public Works Department, MRO Engineers, Inc., November 21, 2013.

Impacts and Mitigation

Existing (2019) plus Approved and Pending Project plus Proposed Project Conditions

As reflected in **Table 11** and **Table 13**, the addition of the proposed Project results in two (2) significant impact. The following is a discussion of the impacts and their associated mitigations. Analysis worksheets for this scenario are provided in **Appendix G**.

Impacts:

Intersections

I1. Intersection #2, Roth Road @ I-5 SB Ramp

As shown in **Table 5** and **Table 8**, this intersection operates at an unacceptable LOS under Existing (2019) plus Approved and Pending Projects conditions and the addition of the project causes operations at the intersection to worsen. As shown in **Table 11**, the proposed Project increases the delay during both the AM peak-hour. ***This is a significant impact.***

I2. Intersection #4, Roth Road @ Harlan Road

As shown in **Table 5** and **Table 8**, this intersection operates at an unacceptable LOS under Existing (2019) plus Approved and Pending Projects conditions and the addition of the project causes operations at the intersection to worsen. As shown in **Table 11**, the proposed Project increases the delay during both the AM and PM peak-hours. ***This is a significant impact.***

Mitigations:

Intersections

M1. Intersection #2, Roth Road @ I-5 SB Ramp

The significant impact at this intersection during the AM peak-hour can be mitigated by converting the intersection to all-way stop control. This mitigation is consistent with improvements identified in the City of Lathrop's *2018 Traffic Monitoring Program*⁴ which identifies the need for conversion to all-way stop control under Existing (2019) plus Approved and Pending Projects conditions. The implementation of this mitigation results in acceptable LOS B in both the AM PM peak-hours (**Table 21**). With this improvement, this impact would be ***less than significant.***

M2. Intersection #4, Roth Road @ Harlan Road

The significant impact at this intersection during the AM and PM peak-hours can be mitigated by converting the intersection to a traffic signal. This mitigation is consistent with improvements identified in the City of Lathrop's *2018 Traffic Monitoring Program*⁴, which identifies the need to move the Roth Road intersection with Harlan Road at least 600-feet to the east and signalize the new intersection under Year 2020 conditions. The implementation of this mitigation results in acceptable LOS D in the AM and PM peak-hours (**Table 21**). With this improvement, this impact would be ***less than significant.***

Table 21 – Intersection Levels of Service – Existing plus Approved and Pending Projects plus Proposed Project Mitigated Conditions

ID	Intersection	Control	Peak Hour	Existing (2019) plus Approved Projects plus Project		Existing (2019) plus Approved Projects plus Project - Mitigated	
				Delay (sec)	LOS	Delay (sec)	LOS
2	Roth Road @ I-5 SB Ramp	SSSC	AM	49.7 (SBL)	E	13.7	B
			PM	34.8 (SBL)	D	12.3	B
4	Roth Road @ Harlan Road	AWSC	AM	64.6	F	38.3	D
			PM	152.1	F	35.6	D

Notes:

Side Street Stop Controlled (SSSC) intersection LOS corresponds to the worst approach.

Bolded represents unacceptable conditions.

Shaded represents a significant impact.

Cumulative (2040) plus Proposed Project Conditions

As reflected in **Table 17**, **Table 18**, and **Table 19**, the addition of the proposed Project does not result in significant impacts as defined by the County. Accordingly, ***no mitigations are required*** for this scenario.

OTHER CONSIDERATIONS

95th Percentile Queues

Vehicle queuing for critical movements at the study intersections was evaluated for the eastbound movements along Roth Road at the intersection of Roth Road and Harlan Road (Intersection #4). The City of Lathrop's 2018 *Traffic Monitoring Program*⁴ identifies the need to move the Roth Road @ Harlan Road intersection at least 600-feet to the east and signalize the new intersection under Year 2020 conditions. The 95th percentile queues represent a worst-case condition, as 95-percent of the time, vehicle queues are anticipated to less than the calculated lengths. **Table 22** compares the calculated 95th percentile queues to available vehicle storage lengths. Analysis worksheets are provided in **Appendix B-F**.

As shown in **Table 22**, the calculated 95th percentile queues are not anticipated to exceed available storage capacity.

Access

The site plan for the proposed Project (**Figure 2**) was qualitatively reviewed for general access and on-site circulation. According to the site plan, access to the site will be provided from Roth Road via one project driveway. The proposed Project access will be restricted by a gate.

Table 22 – 95th Percentile Queues

Intersection / Analysis Scenario	Movement	AM Peak-Hour		PM Peak-Hour	
		Available Storage (ft)	95 th % Queue (ft)	Available Storage (ft)	95 th % Queue (ft)
#4, Roth Road @ Harlan Road	EBL				
Existing (2019)	100		15	100	8
Existing (2019) plus Approved/Pending Project			20		25
Existing (2019) plus Approved/Pending Project plus proposed			20		25
Cumulative (2040)		700	152	700	126
Cumulative (2040) plus proposed Project			152		126
#4, Roth Road @ Harlan Road	EBR				
Existing (2019)	100		30	100	53
Existing (2019) plus Approved/Pending Project			40		73
Existing (2019) plus Approved/Pending Project plus proposed			40		73
Cumulative (2040)		700	47	700	54
Cumulative (2040) plus proposed Project			47		54

Source: *Highway Capacity Manual (HCM) 2010* methodology per Synchro[®] v10.

CONCLUSIONS

Based upon the analysis documented in this report, the following conclusions are offered:

- The proposed Project is estimated to generate 370 new external daily trips, 33 new external trips occurring during the AM peak-hour, and 29 new external trips during the PM peak-hour.
- As defined by the County, the addition of the proposed Project results in two (2) significant impacts to the Roth Road intersections with the I-5 SB Ramps and Harlan Road (Intersection #2 and Intersection #4) under Existing (2019) plus Approved and Pending Projects plus Proposed Project Conditions. These two impacts can be mitigated to be ***less than significant***.
- As defined by the County, the addition of the proposed Project to does not result in significant impacts to any of the study intersections under Cumulative (2040) plus Proposed Project Conditions.
- Based on this analysis, the 95th percentile vehicles queues are not anticipated to exceed available storage for the eastbound movement at the intersection of Roth Road and Harlan Road (Intersection #4).

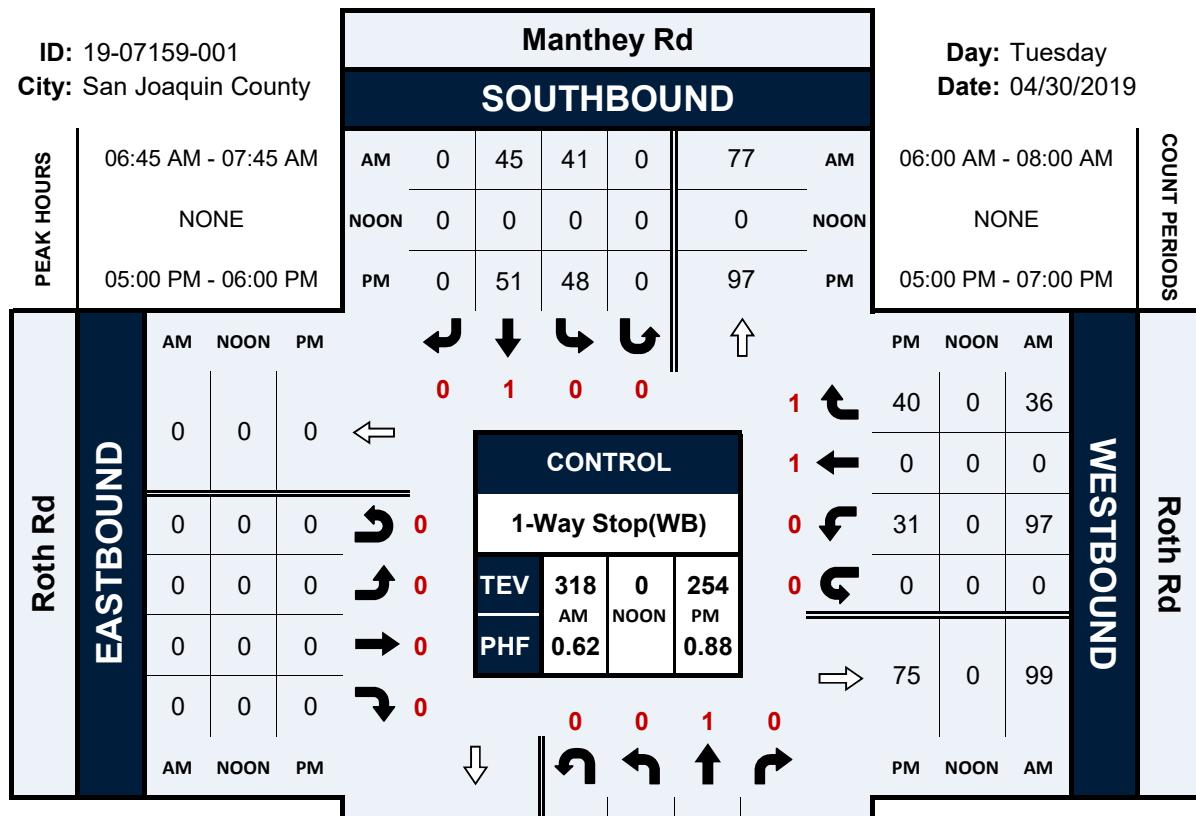
Appendix A

Traffic Count Data Sheets

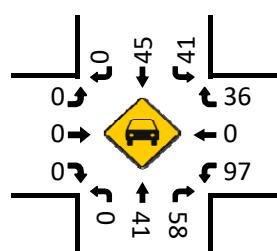
Manthey Rd & Roth Rd

Peak Hour Turning Movement Count

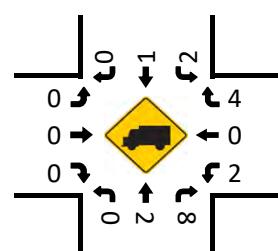
ID: 19-07159-001
City: San Joaquin County



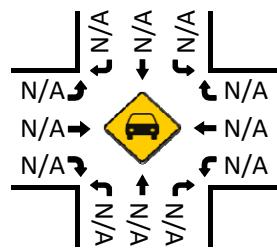
Total Vehicles (AM)



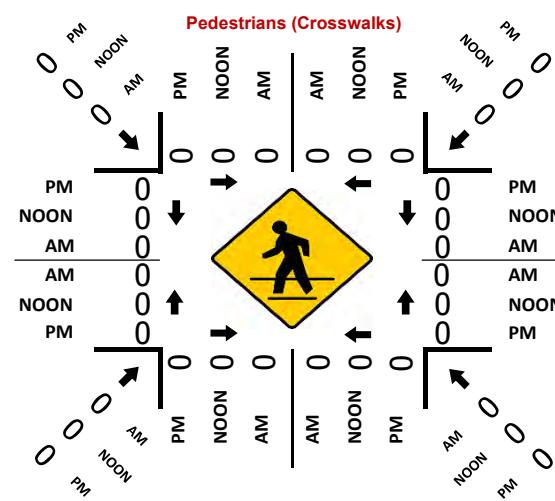
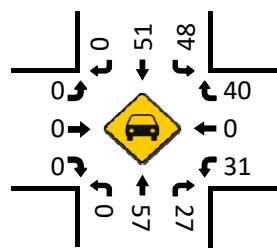
HT (AM)



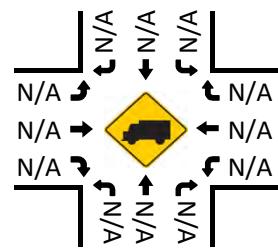
Total Vehicles (Noon)



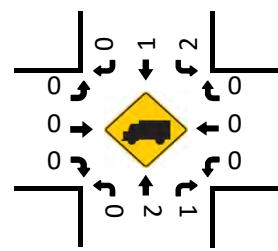
Total Vehicles (PM)



HT (NOON)



HT (PM)

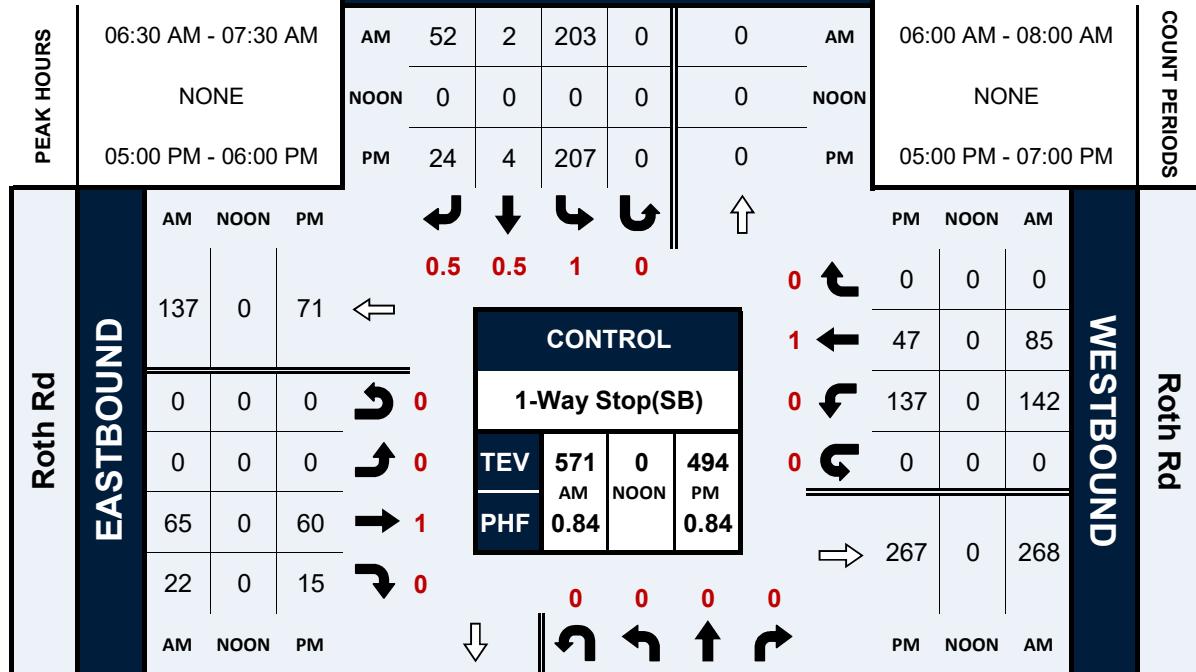


I-5 SB Ramps & Roth Rd

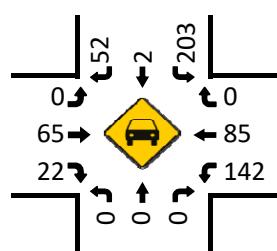
Peak Hour Turning Movement Count

ID: 19-07159-002
City: San Joaquin County

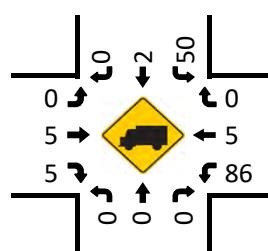
Day: Tuesday
Date: 04/30/2019



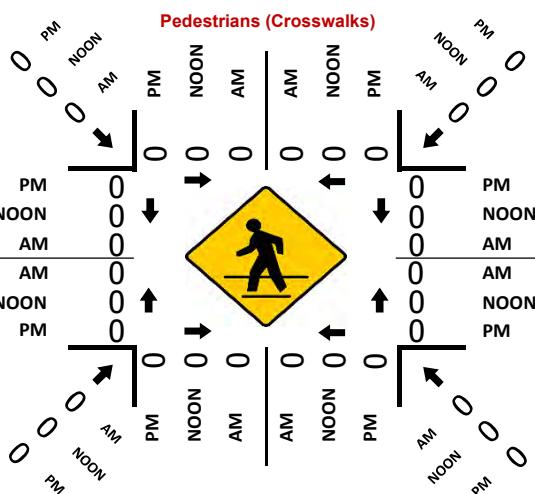
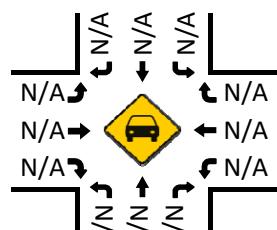
Total Vehicles (AM)



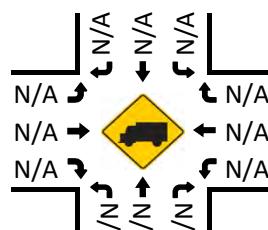
HT (AM)



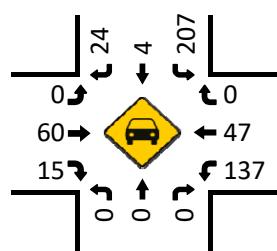
Total Vehicles (Noon)



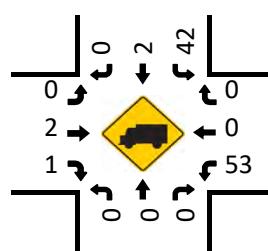
HT (NOON)



Total Vehicles (PM)



HT (PM)

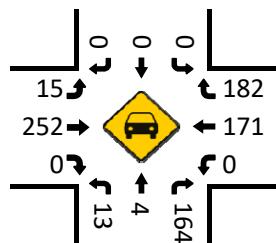
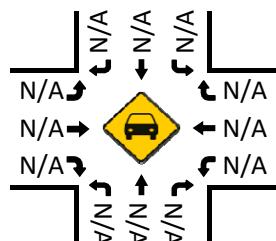
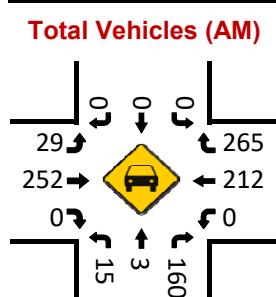


I-5 NB Ramps & Roth Rd

Peak Hour Turning Movement Count

ID: 19-07159-003
City: San Joaquin County

PEAK HOURS	07:00 AM - 08:00 AM			
	NONE			
05:00 PM - 06:00 PM				
Roth Rd	EASTBOUND	AM	NOON	PM
		227	0	184
		0	0	0
		29	0	15
		252	0	252
		0	0	0
		AM	NOON	PM

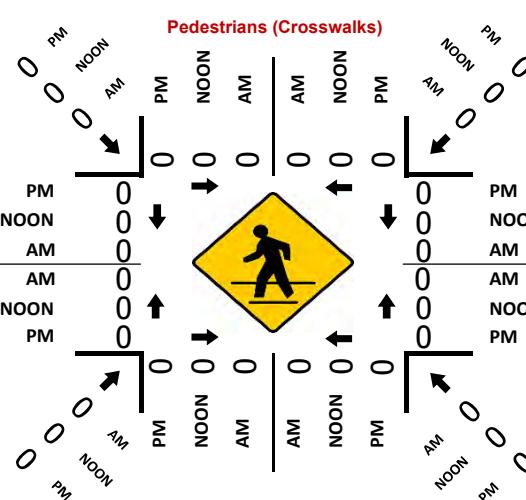


I-5 NB Ramps					
SOUTHBOUND					
AM	0	0	0	0	297 AM
NOON	0	0	0	0	0 NOON
PM	0	0	0	0	201 PM





PM	0	0	13	4	164	PM
NOON	0	0	0	0	0	NOON
AM	0	0	15	3	160	AM

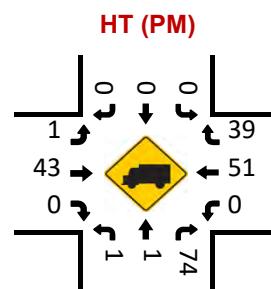
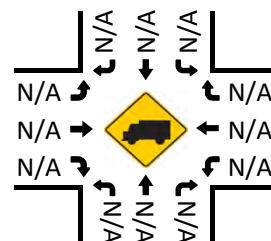
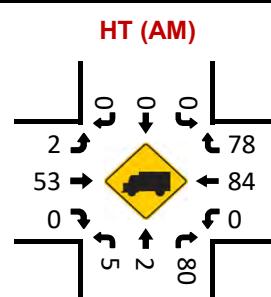


Day: Tuesday
Date: 04/30/2019

06:00 AM - 08:00 AM
NONE
05:00 PM - 07:00 PM

PM	NOON	AM
182	0	265
171	0	212
0	0	0
1	0	0
417	0	412
PM	NOON	AM

WESTBOUND

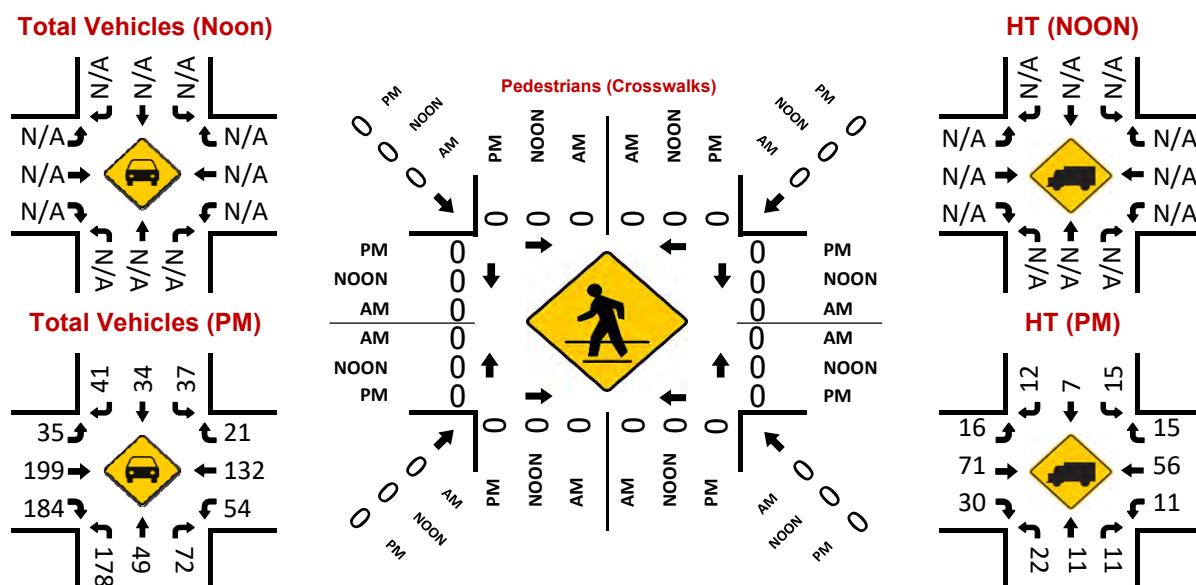
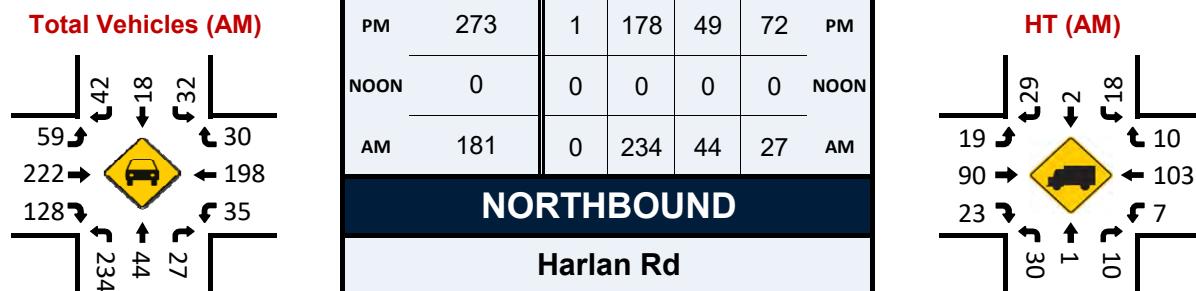
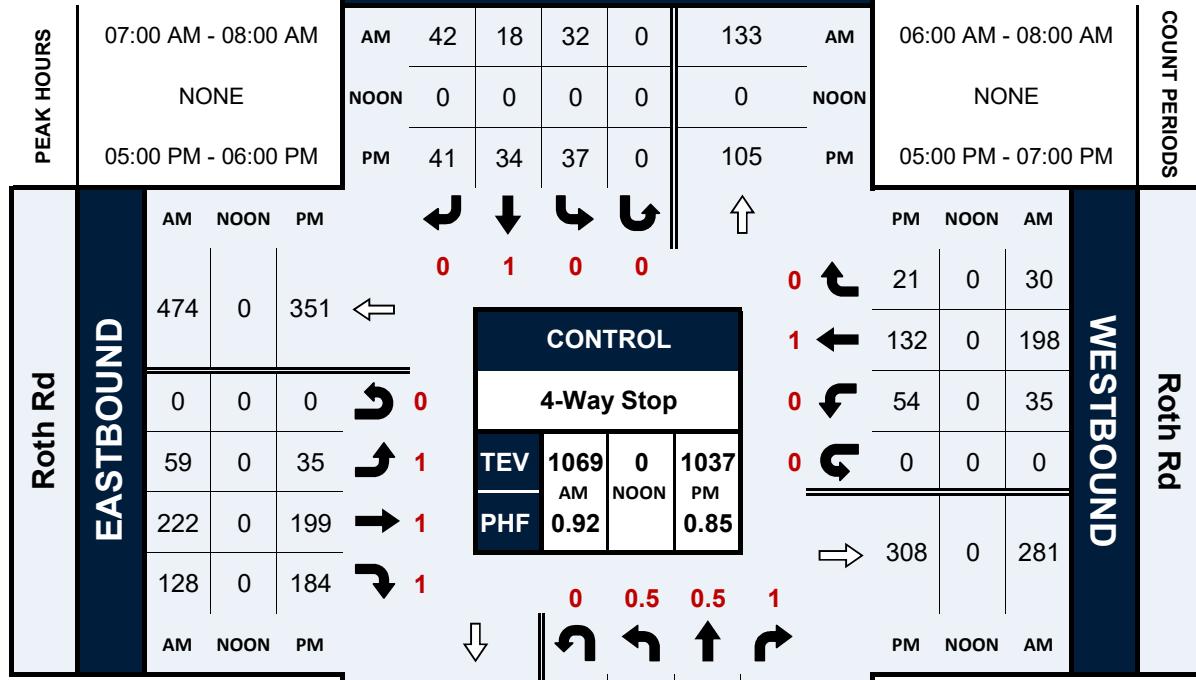


Harlan Rd & Roth Rd

Peak Hour Turning Movement Count

ID: 19-07159-004
City: San Joaquin County

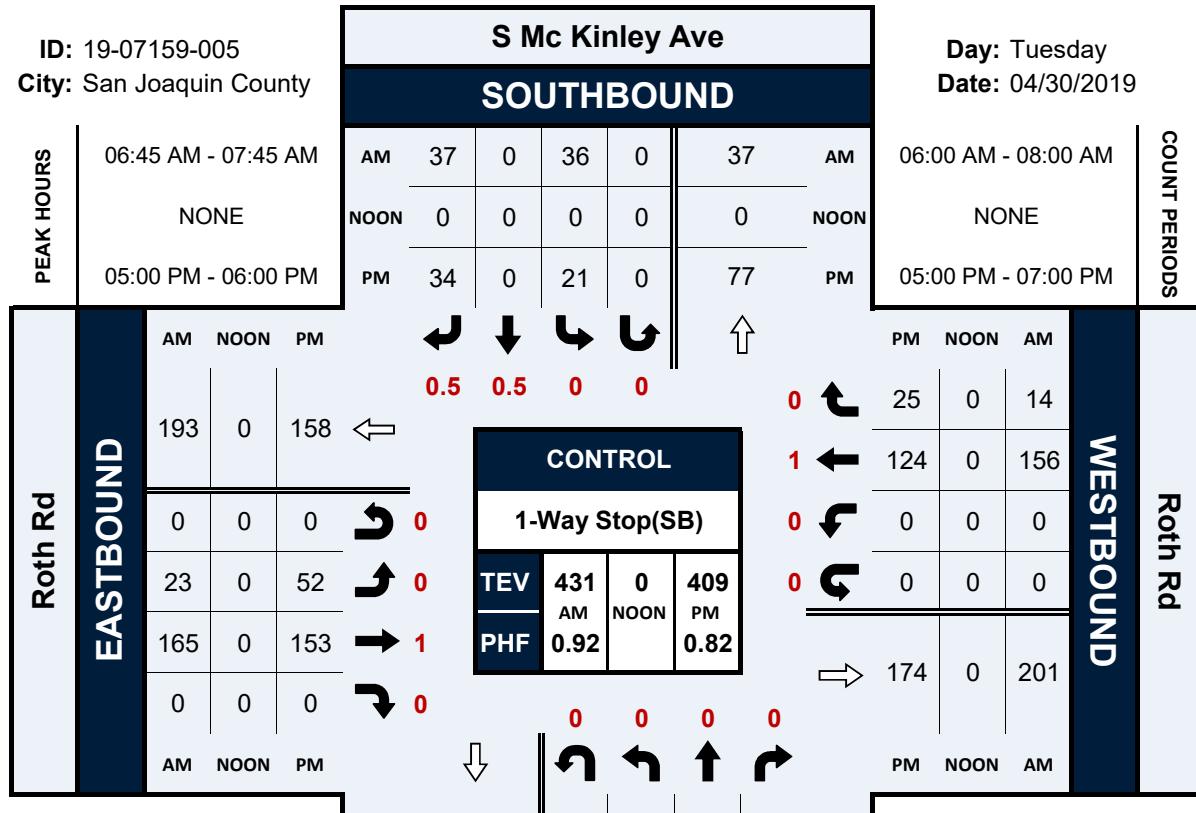
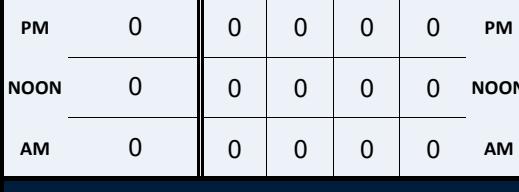
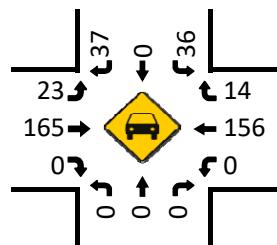
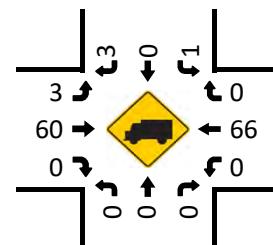
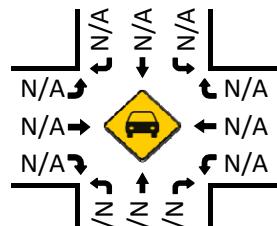
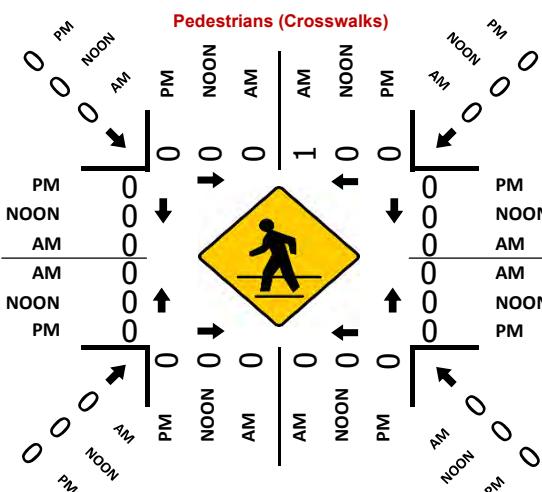
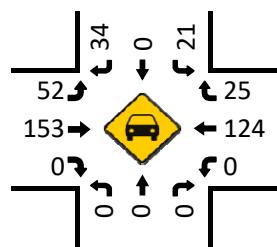
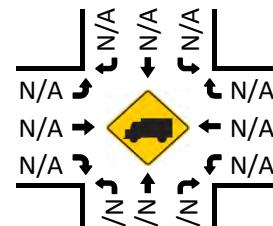
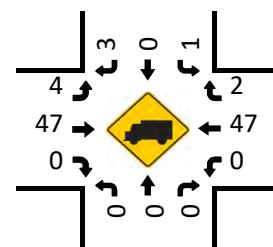
Day: Tuesday
Date: 04/30/2019



S Mc Kinley Ave & Roth Rd**Peak Hour Turning Movement Count**

ID: 19-07159-005
City: San Joaquin County

Day: Tuesday
Date: 04/30/2019

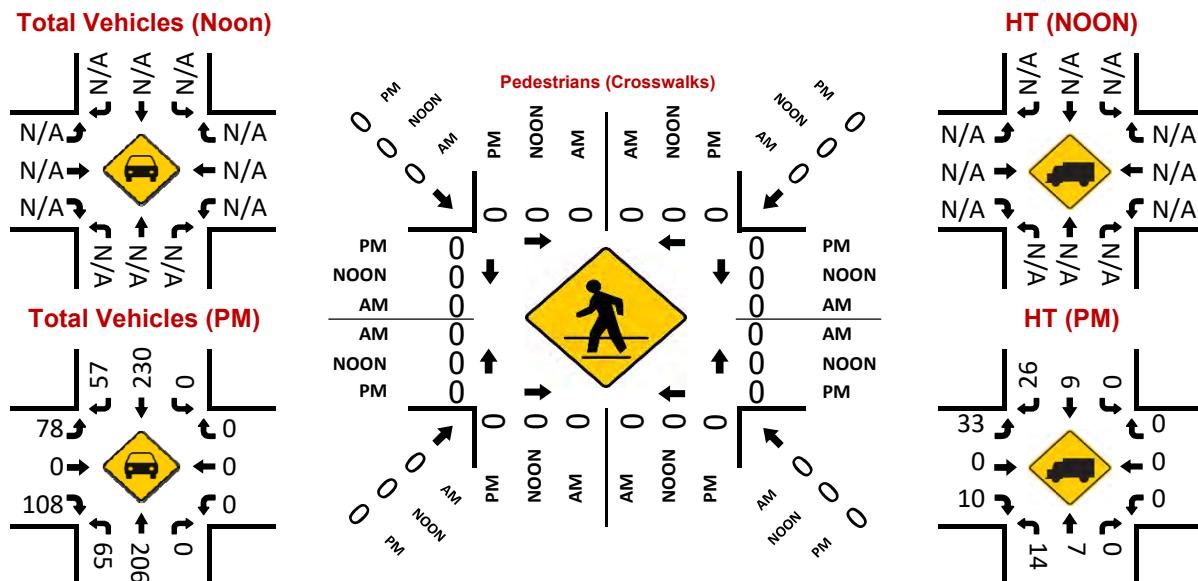
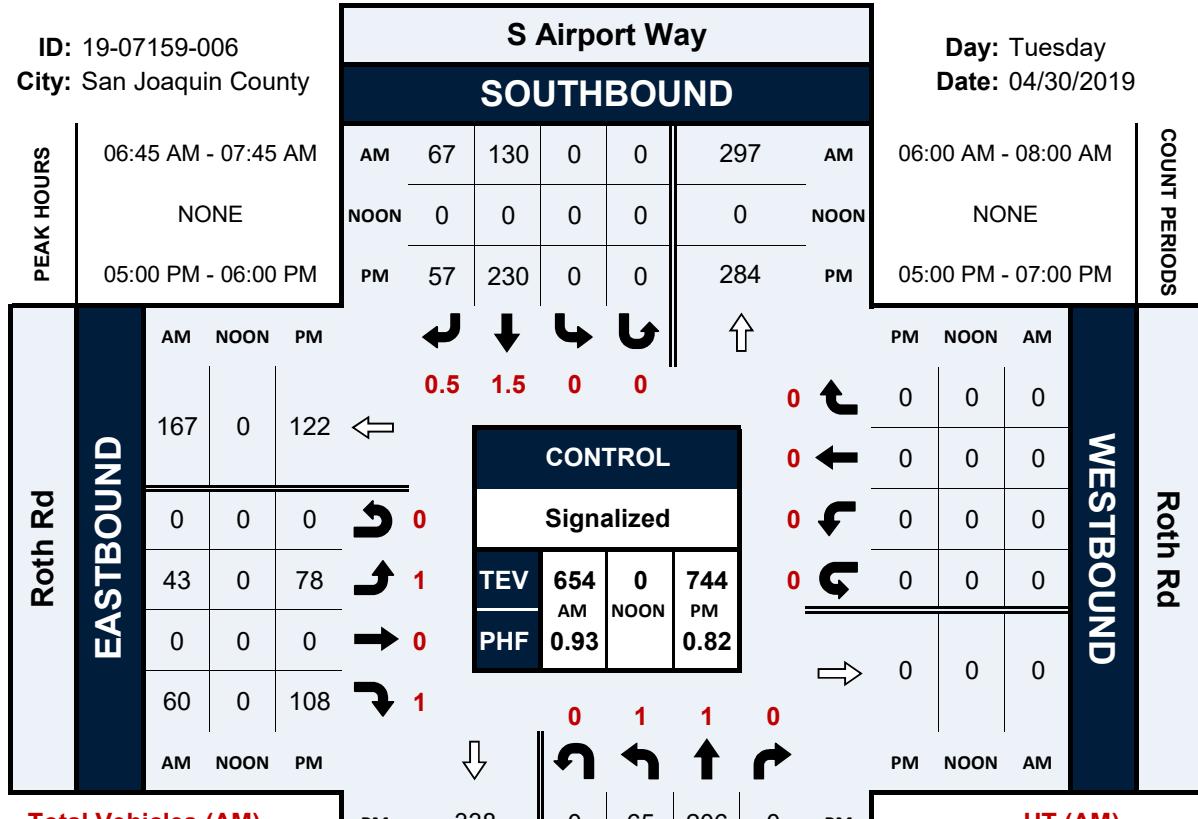
**Total Vehicles (AM)****HT (AM)****Total Vehicles (Noon)****Total Vehicles (PM)****HT (NOON)****HT (PM)**

S Airport Way & Roth Rd

Peak Hour Turning Movement Count

ID: 19-07159-006
City: San Joaquin County

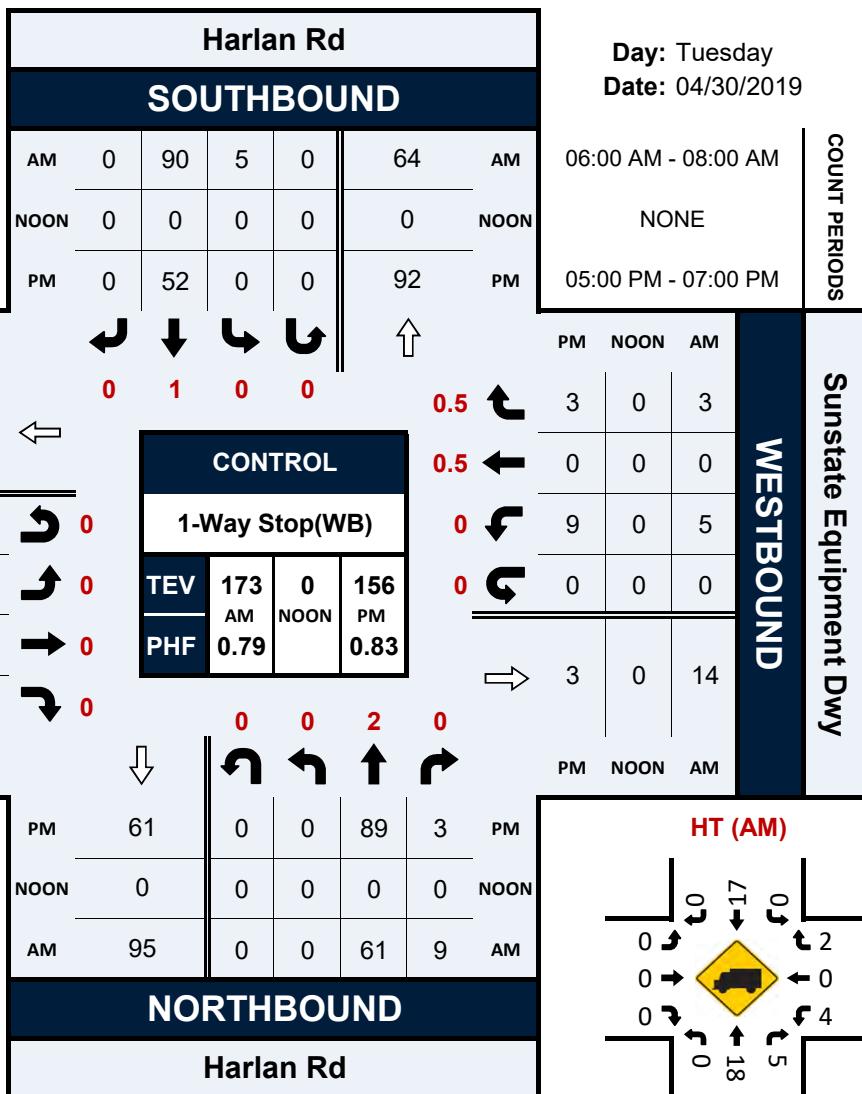
Day: Tuesday
Date: 04/30/2019



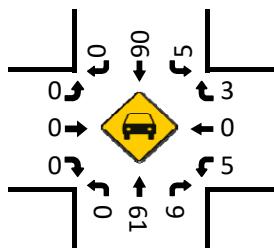
Harlan Rd & Sunstate Equipment Dwy

Peak Hour Turning Movement Count

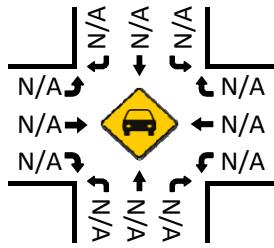
ID: 19-07159-007
City: San Joaquin County



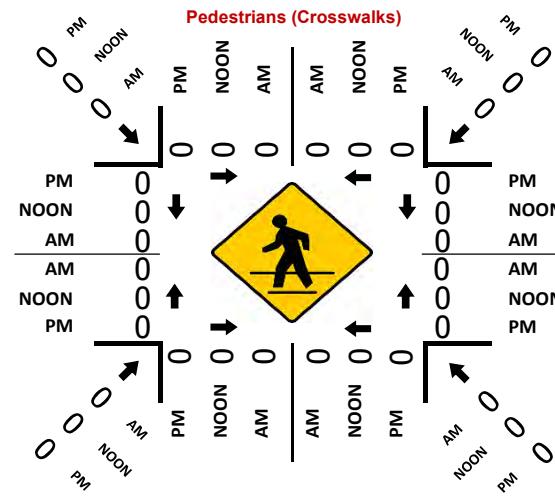
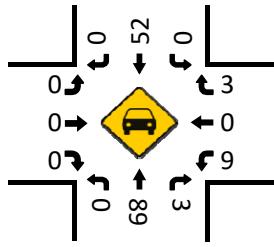
Total Vehicles (AM)



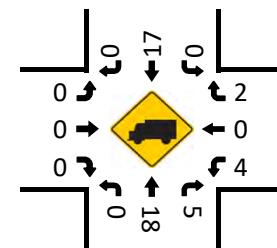
Total Vehicles (Noon)



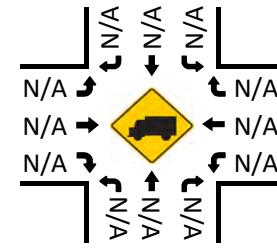
Total Vehicles (PM)



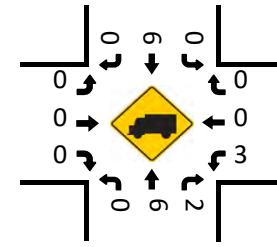
HT (AM)



HT (Noon)



HT (PM)



VOLUME

Harlan Rd Bet. Roth Rd & Equipment St

Day: Tuesday
Date: 4/30/2019City: French Camp
Project #: CA19_7160_001

DAILY TOTALS				NB 1,413	SB 1,246	EB 0	WB 0	Total 2,659			
AM Period	NB	SB	EB	WB	TOTAL	PM Period	NB	SB	EB	WB	TOTAL
00:00	2	5			7	12:00	22	29			51
00:15	2	4			6	12:15	37	22			59
00:30	1	3			4	12:30	29	23			52
00:45	3	8	3	15	23	12:45	19	107	26	100	45 207
01:00	2	4			6	13:00	18	18			36
01:15	1	5			6	13:15	27	24			51
01:30	3	1			4	13:30	23	24			47
01:45	4	10	1	11	21	13:45	19	87	21	87	40 174
02:00	3	1			4	14:00	26	18			44
02:15	9	4			13	14:15	32	22			54
02:30	2	5			7	14:30	22	25			47
02:45	6	20	3	13	33	14:45	25	105	16	81	41 186
03:00	3	1			4	15:00	24	15			39
03:15	3	2			5	15:15	24	25			49
03:30	12	5			17	15:30	30	26			56
03:45	2	20	8	16	36	15:45	36	114	19	85	55 199
04:00	2	6			8	16:00	34	36			70
04:15	4	7			11	16:15	31	23			54
04:30	4	9			13	16:30	20	20			40
04:45	5	15	6	28	43	16:45	16	101	19	98	35 199
05:00	7	6			13	17:00	24	25			49
05:15	13	8			21	17:15	17	27			44
05:30	12	7			19	17:30	20	9			29
05:45	12	44	10	31	75	17:45	16	77	11	72	27 149
06:00	13	8			21	18:00	18	12			30
06:15	9	10			19	18:15	15	19			34
06:30	10	14			24	18:30	15	18			33
06:45	20	52	13	45	97	18:45	11	59	12	61	23 120
07:00	28	12			40	19:00	15	8			23
07:15	14	18			32	19:15	13	8			21
07:30	30	16			46	19:30	16	14			30
07:45	27	99	21	67	166	19:45	14	58	12	42	26 100
08:00	20	16			36	20:00	10	3			13
08:15	16	18			34	20:15	8	8			16
08:30	21	12			33	20:30	10	3			13
08:45	17	74	18	64	138	20:45	7	35	2	16	9 51
09:00	16	20			36	21:00	4	6			10
09:15	22	17			39	21:15	8	7			15
09:30	16	15			31	21:30	11	7			18
09:45	25	79	20	72	151	21:45	4	27	4	24	8 51
10:00	18	20			38	22:00	6	11			17
10:15	22	31			53	22:15	4	4			8
10:30	27	21			48	22:30	5	5			10
10:45	20	87	15	87	174	22:45	6	21	5	25	11 46
11:00	24	22			46	23:00	6	2			8
11:15	20	24			44	23:15	5	7			12
11:30	30	20			50	23:30	7	3			10
11:45	19	93	26	92	185	23:45	3	21	2	14	5 35
TOTALS	601	541			1142	TOTALS	812	705			1517
SPLIT %	52.6%	47.4%			42.9%	SPLIT %	53.5%	46.5%			57.1%

DAILY TOTALS				NB 1,413	SB 1,246	EB 0	WB 0	Total 2,659
AM Peak Hour	11:30	11:45		11:45	PM Peak Hour	15:30	15:15	15:30
AM Pk Volume	108	100		207	PM Pk Volume	131	106	235
Pk Hr Factor	0.730	0.862		0.877	Pk Hr Factor	0.910	0.736	0.839
7 - 9 Volume	173	131	0	304	4 - 6 Volume	178	170	348
7 - 9 Peak Hour	07:00	07:15		07:00	4 - 6 Peak Hour	16:00	16:00	16:00
7 - 9 Pk Volume	99	71	0	166	4 - 6 Pk Volume	101	98	199
Pk Hr Factor	0.825	0.845	0.000	0.865	Pk Hr Factor	0.743	0.681	0.711

VOLUME

Harlan Rd Bet. Roth Rd & Equipment St

Day: Wednesday
Date: 5/1/2019City: French Camp
Project #: CA19_7160_001

DAILY TOTALS				NB 1,638	SB 1,348	EB 0	WB 0	Total 2,986			
AM Period	NB	SB	EB	WB	TOTAL	PM Period	NB	SB	EB	WB	TOTAL
00:00	2	2			4	12:00	24	34			58
00:15	3	4			7	12:15	27	25			52
00:30	1	2			3	12:30	22	30			52
00:45	3	9	4	12	21	12:45	23	96	25	114	48 210
01:00	1	1			2	13:00	29	22			51
01:15	2	2			4	13:15	26	18			44
01:30	1	1			2	13:30	22	31			53
01:45	4	8	1	5	13	13:45	26	103	16	87	42 190
02:00	3	4			7	14:00	26	23			49
02:15	6	3			9	14:15	25	24			49
02:30	11	1			12	14:30	38	14			52
02:45	7	27	3	11	38	14:45	20	109	18	79	38 188
03:00	3	4			7	15:00	17	17			34
03:15	4	2			6	15:15	32	26			58
03:30	3	8			11	15:30	31	25			56
03:45	5	15	3	17	32	15:45	31	111	14	82	45 193
04:00	4	5			9	16:00	51	27			78
04:15	8	9			17	16:15	33	23			56
04:30	8	8			16	16:30	28	29			57
04:45	6	26	7	29	55	16:45	28	140	21	100	49 240
05:00	7	5			12	17:00	42	28			70
05:15	11	6			17	17:15	18	22			40
05:30	8	6			14	17:30	15	22			37
05:45	14	40	15	32	72	17:45	11	86	18	90	29 176
06:00	18	6			24	18:00	20	18			38
06:15	12	6			18	18:15	15	18			33
06:30	15	9			24	18:30	14	11			25
06:45	26	71	8	29	100	18:45	14	63	9	56	23 119
07:00	15	13			28	19:00	6	12			18
07:15	18	17			35	19:15	8	6			14
07:30	26	8			34	19:30	8	11			19
07:45	30	89	12	50	139	19:45	6	28	10	39	16 67
08:00	22	20			42	20:00	10	4			14
08:15	19	17			36	20:15	9	5			14
08:30	29	25			54	20:30	7	10			17
08:45	16	86	18	80	166	20:45	5	31	13	32	18 63
09:00	26	11			37	21:00	15	10			25
09:15	28	17			45	21:15	20	8			28
09:30	21	20			41	21:30	22	10			32
09:45	34	109	14	62	171	21:45	17	74	7	35	24 109
10:00	13	19			32	22:00	18	7			25
10:15	38	24			62	22:15	15	10			25
10:30	14	26			40	22:30	21	18			39
10:45	19	84	19	88	172	22:45	24	78	14	49	38 127
11:00	20	28			48	23:00	21	16			37
11:15	32	30			62	23:15	12	8			20
11:30	22	30			52	23:30	16	6			22
11:45	20	94	44	132	226	23:45	12	61	8	38	20 99
TOTALS	658	547			1205	TOTALS	980	801			1781
SPLIT %	54.6%	45.4%			40.4%	SPLIT %	55.0%	45.0%			59.6%

DAILY TOTALS				NB 1,638	SB 1,348	EB 0	WB 0	Total 2,986
AM Peak Hour	09:00	11:15		11:15	PM Peak Hour	15:30	12:00	16:00
AM Pk Volume	109	138		236	PM Pk Volume	146	114	240
Pk Hr Factor	0.801	0.784		0.922	Pk Hr Factor	0.716	0.838	0.769
7 - 9 Volume	175	130	0	305	4 - 6 Volume	226	190	416
7 - 9 Peak Hour	07:45	08:00		07:45	4 - 6 Peak Hour	16:00	16:15	16:00
7 - 9 Pk Volume	100	80	0	174	4 - 6 Pk Volume	140	101	240
Pk Hr Factor	0.833	0.800	0.000	0.806	Pk Hr Factor	0.686	0.871	0.769

Prepared by NDS/ATD

VOLUME

Roth Rd W/O Mc Kinley Ave

Day: Tuesday
Date: 4/30/2019

City: Lathrop
Project #: CA19_7160_002

DAILY TOTALS				NB 0	SB 0	EB 3,622	WB 3,350				Total 6,972	
AM Period	NB	SB	EB	WB	TOTAL		PM Period	NB	SB	EB	WB	TOTAL
00:00			12	20	32		12:00		65	51	116	
00:15			7	7	14		12:15		55	44	99	
00:30			14	4	18		12:30		70	55	125	
00:45			7	40	4	35	12:45		56	246	49	199
01:00			13	6	19		13:00		60	49	109	
01:15			5	12	17		13:15		68	59	127	
01:30			10	12	22		13:30		80	58	138	
01:45			19	47	15	45	13:45		63	271	53	219
02:00			10	9	19		14:00		73	61	134	
02:15			5	14	19		14:15		57	58	115	
02:30			9	14	23		14:30		61	59	120	
02:45			12	36	15	52	14:45		74	265	53	231
03:00			10	8	18		15:00		62	59	121	
03:15			9	11	20		15:15		50	52	102	
03:30			17	31	48		15:30		85	59	144	
03:45			13	49	13	63	15:45		86	283	50	220
04:00			8	26	34		16:00		62	46	108	
04:15			7	22	29		16:15		75	44	119	
04:30			15	25	40		16:30		67	63	130	
04:45			10	40	16	89	16:45		58	262	45	198
05:00			12	33	45		17:00		63	29	92	
05:15			21	27	48		17:15		53	48	101	
05:30			23	44	67		17:30		56	47	103	
05:45			24	80	45	149	17:45		41	213	27	151
06:00			13	39	52		18:00		53	41	94	
06:15			29	46	75		18:15		46	36	82	
06:30			46	42	88		18:30		57	41	98	
06:45			52	140	38	165	18:45		37	193	23	141
07:00			46	49	95		19:00		43	20	63	
07:15			41	68	109		19:15		25	23	48	
07:30			46	51	97		19:30		28	26	54	
07:45			48	181	42	210	19:45		36	132	25	94
08:00			37	66	103		20:00		22	24	46	
08:15			53	37	90		20:15		22	28	50	
08:30			68	69	137		20:30		29	13	42	
08:45			57	215	43	215	20:45		23	96	17	82
09:00			41	40	81		21:00		26	26	52	
09:15			59	49	108		21:15		24	16	40	
09:30			68	40	108		21:30		13	22	35	
09:45			52	220	43	172	21:45		17	80	11	75
10:00			46	53	99		22:00		12	6	18	
10:15			68	58	126		22:15		13	14	27	
10:30			41	46	87		22:30		12	11	23	
10:45			57	212	51	208	22:45		18	55	20	51
11:00			46	50	96		23:00		12	12	24	
11:15			51	55	106		23:15		16	16	32	
11:30			49	85	134		23:30		13	9	22	
11:45			69	215	50	240	23:45		10	51	46	19
TOTALS			1475	1643	3118		TOTALS		2147	1707	3854	
SPLIT %			47.3%	52.7%	44.7%		SPLIT %		55.7%	44.3%	55.3%	

DAILY TOTALS	NB	SB	EB	WB	Total 6,972						
	0	0	3,622	3,350							
AM Peak Hour	11:45	10:45	11:15	PM Peak Hour	15:30	13:15	13:15				
AM Pk Volume	259	241	475	PM Pk Volume	308	231	515				
Pk Hr Factor	0.925	0.709	0.886	Pk Hr Factor	0.895	0.947	0.933				
7 - 9 Volume	0	0	396	425	821	4 - 6 Volume	0	0	475	349	824
7 - 9 Peak Hour			08:00	07:15	08:00	4 - 6 Peak Hour			16:15	16:00	16:00
7 - 9 Pk Volume	0	0	215	227	430	4 - 6 Pk Volume	0	0	263	198	460
Pk Hr Factor	0.000	0.000	0.790	0.835	0.785	Pk Hr Factor	0.000	0.000	0.877	0.786	0.885

Prepared by NDS/ATD

VOLUME

Roth Rd W/O Mc Kinley Ave

Day: Wednesday
Date: 5/1/2019

City: Lathrop
Project #: CA19_7160_002

DAILY TOTALS				NB 0	SB 0	EB 3,508	WB 3,145				Total 6,653
AM Period	NB	SB	EB	WB	TOTAL	PM Period	NB	SB	EB	WB	TOTAL
00:00			10	12	22	12:00		66	38		104
00:15			9	4	13	12:15		46	70		116
00:30			6	7	13	12:30		71	46		117
00:45			6	31	3	12:45		62	245	54	208
01:00			3	20	23	13:00		68	51		119
01:15			6	9	15	13:15		68	44		112
01:30			5	12	17	13:30		79	57		136
01:45			10	24	3	13:45		65	280	39	191
02:00			11	7	18	14:00		64	54		118
02:15			8	7	15	14:15		58	43		101
02:30			6	13	19	14:30		72	50		122
02:45			10	35	20	14:45		48	242	39	186
03:00			11	10	21	15:00		65	49		114
03:15			15	14	29	15:15		51	45		96
03:30			11	19	30	15:30		91	63		154
03:45			10	47	13	15:45		61	268	40	197
04:00			9	27	36	16:00		65	46		111
04:15			14	28	42	16:15		55	45		100
04:30			23	27	50	16:30		64	38		102
04:45			8	54	23	16:45		48	232	46	175
05:00			12	29	41	17:00		62	44		106
05:15			13	20	33	17:15		53	41		94
05:30			31	29	60	17:30		56	38		94
05:45			23	79	44	17:45		44	215	43	166
06:00			13	49	62	18:00		36	25		61
06:15			23	34	57	18:15		51	24		75
06:30			33	34	67	18:30		41	24		65
06:45			51	120	32	18:45		32	160	21	94
07:00			27	53	80	19:00		41	23		64
07:15			46	50	96	19:15		28	23		51
07:30			39	51	90	19:30		25	19		44
07:45			48	160	47	19:45		28	122	19	84
08:00			37	43	80	20:00		25	31		56
08:15			47	46	93	20:15		22	18		40
08:30			52	58	110	20:30		32	19		51
08:45			58	194	54	20:45		19	98	18	86
09:00			58	48	106	21:00		17	24		41
09:15			48	34	82	21:15		26	18		44
09:30			50	53	103	21:30		22	13		35
09:45			57	213	54	21:45		15	80	15	70
10:00			70	56	126	22:00		19	13		32
10:15			46	54	100	22:15		19	10		29
10:30			61	50	111	22:30		17	30		47
10:45			71	248	56	22:45		22	77	8	61
11:00			62	64	126	23:00		10	5		15
11:15			59	48	107	23:15		11	18		29
11:30			59	61	120	23:30		11	8		19
11:45			60	240	62	23:45		12	44	5	36
TOTALS			1445	1591	3036	TOTALS		2063	1554		3617
SPLIT %			47.6%	52.4%	45.6%	SPLIT %		57.0%	43.0%		54.4%

DAILY TOTALS	NB	SB	EB	WB	Total 6,653						
	0	0	3,508	3,145							
AM Peak Hour	10:30	11:00	10:45	PM Peak Hour	13:00	12:15	12:45				
AM Pk Volume	253	235	480	PM Pk Volume	280	221	483				
Pk Hr Factor	0.891	0.918	0.945	Pk Hr Factor	0.886	0.789	0.888				
7 - 9 Volume	0	0	354	402	756	4 - 6 Volume	0	0	447	341	788
7 - 9 Peak Hour			08:00	07:00	08:00	4 - 6 Peak Hour			16:00	16:00	16:00
7 - 9 Pk Volume	0	0	194	201	395	4 - 6 Pk Volume	0	0	232	175	407
Pk Hr Factor	0.000	0.000	0.836	0.948	0.882	Pk Hr Factor	0.000	0.000	0.892	0.951	0.917

Prepared by NDS/ATD

VOLUME

Roth Rd E/O Mc Kinley Ave

Day: Tuesday
Date: 4/30/2019

City: Manteca
Project #: CA19 7160 003

DAILY TOTALS		NB		SB		EB		WB						Total
		0		0		3,309		3,058						6,367
AM Period	NB	SB	EB	WB	TOTAL	PM Period	NB	SB	EB	WB	TOTAL			
00:00			8	17	25	12:00			63	44	107			
00:15			7	5	12	12:15			51	39	90			
00:30			13	4	17	12:30			62	53	115			
00:45			7	35	41	12:45			43	219	48	184	91	403
01:00			13	4	17	13:00			56	49	105			
01:15			6	7	13	13:15			57	61	118			
01:30			11	10	21	13:30			68	60	128			
01:45			14	44	14	13:45			63	244	51	221	114	465
02:00			12	6	18	14:00			64	58	122			
02:15			3	14	17	14:15			49	56	105			
02:30			10	12	22	14:30			53	63	116			
02:45			11	36	13	14:45			67	233	55	232	122	465
03:00			8	6	14	15:00			62	65	127			
03:15			11	10	21	15:15			37	50	87			
03:30			18	27	45	15:30			72	66	138			
03:45			16	53	10	15:45			77	248	57	238	134	486
04:00			8	20	28	16:00			53	59	112			
04:15			10	19	29	16:15			58	44	102			
04:30			16	16	32	16:30			53	60	113			
04:45			11	45	18	16:45			51	215	46	209	97	424
05:00			21	26	47	17:00			54	31	85			
05:15			24	24	48	17:15			44	50	94			
05:30			26	32	58	17:30			38	44	82			
05:45			28	99	37	17:45			35	171	22	147	57	318
06:00			16	34	50	18:00			43	34	77			
06:15			33	39	72	18:15			33	35	68			
06:30			54	29	83	18:30			44	33	77			
06:45			56	159	33	18:45			28	148	23	125	51	273
07:00			55	41	96	19:00			32	11	43			
07:15			49	53	102	19:15			22	20	42			
07:30			46	51	97	19:30			23	24	47			
07:45			40	190	44	19:45			33	110	27	82	60	192
08:00			46	48	94	20:00			19	17	36			
08:15			49	43	92	20:15			20	22	42			
08:30			68	48	116	20:30			21	12	33			
08:45			56	219	39	20:45			19	79	10	61	29	140
09:00			48	42	90	21:00			21	15	36			
09:15			54	39	93	21:15			20	13	33			
09:30			61	35	96	21:30			12	20	32			
09:45			42	205	39	21:45			15	68	10	58	25	126
10:00			41	46	87	22:00			9	4	13			
10:15			64	51	115	22:15			10	13	23			
10:30			42	45	87	22:30			13	11	24			
10:45			55	202	52	22:45			14	46	10	38	24	84
11:00			45	48	93	23:00			9	8	17			
11:15			46	51	97	23:15			14	17	31			
11:30			46	78	124	23:30			11	4	15			
11:45			62	199	45	23:45			8	42	6	35	14	77
TOTALS			1486	1428	2914	TOTALS			1823	1630				3453
SPLIT %			51.0%	49.0%	45.8%	SPLIT %			52.8%	47.2%				54.2%

DAILY TOTALS	NB	SB	EB	WB	Total 6,367				
	0	0	3,309	3,058					
AM Peak Hour	11:45	10:45	11:15	PM Peak Hour	15:30	14:15	15:00		
AM Pk Volume	238	229	435	PM Pk Volume	260	239	486		
Pk Hr Factor	0.944	0.734	0.877	Pk Hr Factor	0.844	0.919	0.880		
7 - 9 Volume	0	0	409	367	776	386	356	742	
7 - 9 Peak Hour			08:00	07:15	08:00	4 - 6 Volume	0	0	16:00
7 - 9 Pk Volume	0	0	219	196	397	4 - 6 Peak Hour			16:00
Pk Hr Factor	0.000	0.000	0.805	0.925	0.856	4 - 6 Pk Volume	0	0	209
						Pk Hr Factor	0.000	0.000	424
							0.931	0.871	0.938

Prepared by NDS/ATD

VOLUME

Roth Rd E/O Mc Kinley Ave

Day: Wednesday
Date: 5/1/2019

City: Manteca
Project #: CA19 7160 003

DAILY TOTALS				NB 0	SB 0	EB 3,283	WB 2,925					Total 6,208
AM Period	NB	SB	EB	WB	TOTAL	PM Period	NB	SB	EB	WB	TOTAL	
00:00			5	12	17	12:00		63	43		106	
00:15			8	4	12	12:15		47	68		115	
00:30			7	8	15	12:30		56	50		106	
00:45			6	26	0	12:45		55	221	55	216	110 437
01:00			4	15	19	13:00		71	44		115	
01:15			7	6	13	13:15		57	41		98	
01:30			4	9	13	13:30		80	54		134	
01:45			9	24	3	13:45		52	260	47	186	99 446
02:00			11	6	17	14:00		58	49		107	
02:15			7	5	12	14:15		56	51		107	
02:30			6	12	18	14:30		59	58		117	
02:45			10	34	19	14:45		43	216	61	219	104 435
03:00			11	10	21	15:00		58	51		109	
03:15			13	12	25	15:15		52	46		98	
03:30			11	16	27	15:30		76	72		148	
03:45			13	48	12	15:45		56	242	42	211	98 453
04:00			7	21	28	16:00		61	50		111	
04:15			16	18	34	16:15		50	48		98	
04:30			23	20	43	16:30		50	36		86	
04:45			9	55	20	16:45		52	213	50	184	102 397
05:00			18	21	39	17:00		55	46		101	
05:15			16	15	31	17:15		47	35		82	
05:30			39	18	57	17:30		52	39		91	
05:45			27	100	34	17:45		41	195	52	172	93 367
06:00			14	44	58	18:00		29	26		55	
06:15			23	23	46	18:15		39	27		66	
06:30			40	31	71	18:30		33	28		61	
06:45			57	134	25	18:45		22	123	22	103	44 226
07:00			30	41	71	19:00		29	20		49	
07:15			44	42	86	19:15		23	23		46	
07:30			43	35	78	19:30		19	19		38	
07:45			53	170	38	19:45		27	98	18	80	45 178
08:00			43	40	83	20:00		13	23		36	
08:15			50	37	87	20:15		22	15		37	
08:30			49	46	95	20:30		29	15		44	
08:45			55	197	35	20:45		19	83	15	68	34 151
09:00			52	45	97	21:00		17	20		37	
09:15			46	28	74	21:15		28	14		42	
09:30			50	43	93	21:30		18	15		33	
09:45			57	205	47	21:45		15	78	13	62	28 140
10:00			58	55	113	22:00		19	14		33	
10:15			40	46	86	22:15		18	21		39	
10:30			58	44	102	22:30		20	15		35	
10:45			67	223	51	22:45		19	76	9	59	28 135
11:00			57	60	117	23:00		8	6		14	
11:15			50	45	95	23:15		9	9		18	
11:30			53	57	110	23:30		10	6		16	
11:45			66	226	66	23:45		9	36	4	25	13 61
TOTALS			1442	1340	2782	TOTALS			1841	1585		3426
SPLIT %			51.8%	48.2%	44.8%	SPLIT %			53.7%	46.3%		55.2%

DAILY TOTALS	NB	SB	EB	WB	Total 6,208						
	0	0	3,283	2,925							
AM Peak Hour	10:30	11:30	11:30	PM Peak Hour	12:45	14:45	14:45				
AM Pk Volume	232	234	463	PM Pk Volume	263	230	459				
Pk Hr Factor	0.866	0.860	0.877	Pk Hr Factor	0.822	0.799	0.775				
7 - 9 Volume	0	0	367	314	681	4 - 6 Volume	0	0	408	356	764
7 - 9 Peak Hour			08:00	07:45	07:45	4 - 6 Peak Hour			16:00	16:00	16:00
7 - 9 Pk Volume	0	0	197	161	356	4 - 6 Pk Volume	0	0	213	184	397
Pk Hr Factor	0.000	0.000	0.895	0.875	0.937	Pk Hr Factor	0.000	0.000	0.873	0.920	0.894

Appendix B

*Analysis Worksheets for
Existing (2019) Conditions*

Trans Truck System Truck Facility TIA
1: Roth Rd & Manthey Rd

Existing
AM PEAK HOUR

Intersection						
Int Delay, s/veh	5.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑			↖
Traffic Vol, veh/h	97	36	41	58	41	45
Future Vol, veh/h	97	36	41	58	41	45
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	65	65	52	52	74	74
Heavy Vehicles, %	6	6	6	6	6	6
Mvmt Flow	149	55	79	112	55	61
Major/Minor						
Conflicting Flow All	306	135	0	0	191	0
Stage 1	135	-	-	-	-	-
Stage 2	171	-	-	-	-	-
Critical Hdwy	6.46	6.26	-	-	4.16	-
Critical Hdwy Stg 1	5.46	-	-	-	-	-
Critical Hdwy Stg 2	5.46	-	-	-	-	-
Follow-up Hdwy	3.554	3.354	-	-	2.254	-
Pot Cap-1 Maneuver	678	903	-	-	1359	-
Stage 1	882	-	-	-	-	-
Stage 2	849	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	650	903	-	-	1359	-
Mov Cap-2 Maneuver	650	-	-	-	-	-
Stage 1	845	-	-	-	-	-
Stage 2	849	-	-	-	-	-
Approach						
	WB	NB	SB			
HCM Control Delay, s	11.4	0	3.7			
HCM LOS	B					
Minor Lane/Major Mvmt						
	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	650	903	1359	-
HCM Lane V/C Ratio	-	-	0.23	0.061	0.041	-
HCM Control Delay (s)	-	-	12.2	9.2	7.8	0
HCM Lane LOS	-	-	B	A	A	A
HCM 95th %tile Q(veh)	-	-	0.9	0.2	0.1	-

Trans Truck System Truck Facility TIA
2: Roth Rd & I-5 SB Ramp

Existing
AM PEAK HOUR

Intersection												
Int Delay, s/veh 9.9												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			↑	↑	↑	↑↑				↑	↑	
Traffic Vol, veh/h	0	65	22	142	85	0	0	0	0	203	2	52
Future Vol, veh/h	0	65	22	142	85	0	0	0	0	203	2	52
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	40	300	-	-	-	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	16974	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	56	56	56	79	79	79	92	92	92	88	88	88
Heavy Vehicles, %	12	12	12	40	40	40	40	40	40	20	20	20
Mvmt Flow	0	116	39	180	108	0	0	0	0	231	2	59
Major/Minor												
Major1			Major2			Minor2						
Conflicting Flow All	-	0	0	155	0	0			604	623	54	
Stage 1	-	-	-	-	-	-			468	468	-	
Stage 2	-	-	-	-	-	-			136	155	-	
Critical Hdwy	-	-	-	4.7	-	-			6.9	6.8	7.2	
Critical Hdwy Stg 1	-	-	-	-	-	-			6.1	5.8	-	
Critical Hdwy Stg 2	-	-	-	-	-	-			5.7	5.8	-	
Follow-up Hdwy	-	-	-	2.58	-	-			3.69	4.19	3.49	
Pot Cap-1 Maneuver	0	-	-	1204	-	0			411	372	950	
Stage 1	0	-	-	-	-	0			556	524	-	
Stage 2	0	-	-	-	-	0			843	732	-	
Platoon blocked, %	-	-	-	-	-	-						
Mov Cap-1 Maneuver	-	-	-	1204	-	-			349	0	950	
Mov Cap-2 Maneuver	-	-	-	-	-	-			349	0	-	
Stage 1	-	-	-	-	-	-			473	0	-	
Stage 2	-	-	-	-	-	-			843	0	-	
Approach												
EB			WB			SB						
HCM Control Delay, s	0			5.3					19.6			
HCM LOS									C			
Minor Lane/Major Mvmt												
EBT EBR WBL WBT SBLn1 SBLn2												
Capacity (veh/h)	-	-	1204	-	349	481						
HCM Lane V/C Ratio	-	-	0.149	-	0.441	0.287						
HCM Control Delay (s)	-	-	8.5	-	23.2	15.5						
HCM Lane LOS	-	-	A	-	C	C						
HCM 95th %tile Q(veh)	-	-	0.5	-	2.2	1.2						

Trans Truck System Truck Facility TIA
3: I-5 NB Ramp & Roth Rd

Existing
AM PEAK HOUR

Intersection												
Int Delay, s/veh 2.4												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑			↑	↑	↓	↓	↑			
Traffic Vol, veh/h	29	252	0	0	212	265	15	3	160	0	0	0
Future Vol, veh/h	29	252	0	0	212	265	15	3	160	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	300	-	-	-	-	50	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	16965	-	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	81	81	81	93	93	93	92	92	92
Heavy Vehicles, %	20	20	20	34	34	34	49	49	49	40	40	40
Mvmt Flow	32	277	0	0	262	327	16	3	172	0	0	0
Major/Minor												
Major1		Major2			Minor1							
Conflicting Flow All	589	0	-	-	-	0	767	930	139			
Stage 1	-	-	-	-	-	-	341	341	-			
Stage 2	-	-	-	-	-	-	426	589	-			
Critical Hdwy	4.4	-	-	-	-	-	7.335	7.235	7.635			
Critical Hdwy Stg 1	-	-	-	-	-	-	6.535	6.235	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	6.135	6.235	-			
Follow-up Hdwy	2.39	-	-	-	-	-	3.9655	4.4655	3.7655			
Pot Cap-1 Maneuver	886	-	0	0	-	-	282	210	765			
Stage 1	-	-	0	0	-	-	586	548	-			
Stage 2	-	-	0	0	-	-	550	410	-			
Platoon blocked, %	-	-	-	-	-	-	-	-	-			
Mov Cap-1 Maneuver	886	-	-	-	-	-	272	0	765			
Mov Cap-2 Maneuver	-	-	-	-	-	-	272	0	-			
Stage 1	-	-	-	-	-	-	565	0	-			
Stage 2	-	-	-	-	-	-	550	0	-			
Approach												
EB		WB			NB							
HCM Control Delay, s	1			0			11.9					
HCM LOS							B					
Minor Lane/Major Mvmt												
NBLn1		NBLn2		EBL	EBT	WBT	WBR					
Capacity (veh/h)	272	765	886	-	-	-	-	-	-	-	-	
HCM Lane V/C Ratio	0.071	0.225	0.036	-	-	-	-	-	-	-	-	
HCM Control Delay (s)	19.2	11.1	9.2	-	-	-	-	-	-	-	-	
HCM Lane LOS	C	B	A	-	-	-	-	-	-	-	-	
HCM 95th %tile Q(veh)	0.2	0.9	0.1	-	-	-	-	-	-	-	-	

Trans Truck System Truck Facility TIA
4: Harlan Rd & Roth Rd

Existing
AM PEAK HOUR

Intersection												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↔	↔	↔	↑	↑	↑	↔	↔	
Traffic Vol, veh/h	59	222	128	35	198	30	234	44	27	32	18	42
Future Vol, veh/h	59	222	128	35	198	30	234	44	27	32	18	42
Peak Hour Factor	0.94	0.94	0.94	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Heavy Vehicles, %	32	32	32	46	46	46	13	13	13	53	53	53
Mvmt Flow	63	236	136	44	251	38	296	56	34	41	23	53
Number of Lanes	1	1	1	0	1	0	0	1	1	0	1	0
Approach												
Opposing Approach	WB		EB		NB		SB		NB			
Opposing Lanes	1		3		1		2		NB			
Conflicting Approach Left	SB		NB		EB		WB					
Conflicting Lanes Left	1		2		3		1					
Conflicting Approach Right	NB		SB		WB		EB					
Conflicting Lanes Right	2		1		1		3					
HCM Control Delay	17.9		47.9		46.7		18.1					
HCM LOS	C		E		E		C					
Lane												
	NBLn1	NBLn2	EBLn1	EBLn2	EBLn3	WBLn1	SBLn1					
Vol Left, %	84%	0%	100%	0%	0%	13%	35%					
Vol Thru, %	16%	0%	0%	100%	0%	75%	20%					
Vol Right, %	0%	100%	0%	0%	100%	11%	46%					
Sign Control	Stop											
Traffic Vol by Lane	278	27	59	222	128	263	92					
LT Vol	234	0	59	0	0	35	32					
Through Vol	44	0	0	222	0	198	18					
RT Vol	0	27	0	0	128	30	42					
Lane Flow Rate	352	34	63	236	136	333	116					
Geometry Grp	8	8	7	7	7	8	8					
Degree of Util (X)	0.879	0.074	0.157	0.557	0.294	0.857	0.332					
Departure Headway (Hd)	8.992	7.839	9.012	8.496	7.773	9.27	10.261					
Convergence, Y/N	Yes											
Cap	405	457	398	425	462	393	351					
Service Time	6.74	5.586	6.765	6.248	5.525	7.021	8.026					
HCM Lane V/C Ratio	0.869	0.074	0.158	0.555	0.294	0.847	0.33					
HCM Control Delay	50.2	11.2	13.4	21.5	13.8	47.9	18.1					
HCM Lane LOS	F	B	B	C	B	E	C					
HCM 95th-tile Q	8.8	0.2	0.6	3.3	1.2	8.2	1.4					

Trans Truck System Truck Facility TIA
5: Roth Rd/Roth Road & McKinley Ave

Existing
AM PEAK HOUR

Intersection						
Int Delay, s/veh	2.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	
Traffic Vol, veh/h	23	165	156	14	36	37
Future Vol, veh/h	23	165	156	14	36	37
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	84	84	79	79	79	79
Heavy Vehicles, %	34	34	39	39	6	6
Mvmt Flow	27	196	197	18	46	47
Major/Minor						
Major1		Major2		Minor2		
Conflicting Flow All	215	0	-	0	456	206
Stage 1	-	-	-	-	206	-
Stage 2	-	-	-	-	250	-
Critical Hdwy	4.44	-	-	-	6.46	6.26
Critical Hdwy Stg 1	-	-	-	-	5.46	-
Critical Hdwy Stg 2	-	-	-	-	5.46	-
Follow-up Hdwy	2.506	-	-	-	3.554	3.354
Pot Cap-1 Maneuver	1186	-	-	-	555	824
Stage 1	-	-	-	-	819	-
Stage 2	-	-	-	-	782	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1186	-	-	-	541	824
Mov Cap-2 Maneuver	-	-	-	-	541	-
Stage 1	-	-	-	-	798	-
Stage 2	-	-	-	-	782	-
Approach						
EB		WB		SB		
HCM Control Delay, s	1		0		11.4	
HCM LOS					B	
Minor Lane/Major Mvmt						
EBL		EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1186	-	-	-	655	
HCM Lane V/C Ratio	0.023	-	-	-	0.141	
HCM Control Delay (s)	8.1	0	-	-	11.4	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0.1	-	-	-	0.5	

Trans Truck System Truck Facility TIA
6: Roth Rd & Airport Way

Existing
AM PEAK HOUR

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	43	60	100	254	130	67
Future Volume (veh/h)	43	60	100	254	130	67
Initial Q (Q _b), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1470	1470	1796	1796	1589	1589
Adj Flow Rate, veh/h	54	75	120	306	140	72
Peak Hour Factor	0.80	0.80	0.83	0.83	0.93	0.93
Percent Heavy Veh, %	29	29	7	7	21	21
Cap, veh/h	132	117	156	1231	952	465
Arrive On Green	0.09	0.09	0.09	0.69	0.48	0.48
Sat Flow, veh/h	1400	1246	1711	1796	2045	959
Grp Volume(v), veh/h	54	75	120	306	106	106
Grp Sat Flow(s),veh/h/ln	1400	1246	1711	1796	1509	1416
Q Serve(g_s), s	1.5	2.4	2.8	2.6	1.6	1.7
Cycle Q Clear(g_c), s	1.5	2.4	2.8	2.6	1.6	1.7
Prop In Lane	1.00	1.00	1.00			0.68
Lane Grp Cap(c), veh/h	132	117	156	1231	731	686
V/C Ratio(X)	0.41	0.64	0.77	0.25	0.14	0.15
Avail Cap(c_a), veh/h	617	549	230	1231	731	686
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	17.4	17.8	18.1	2.4	5.8	5.9
Incr Delay (d2), s/veh	2.0	5.7	8.9	0.5	0.4	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	0.2	1.3	0.4	0.4	0.4
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	19.5	23.5	27.0	2.9	6.3	6.4
LnGrp LOS	B	C	C	A	A	A
Approach Vol, veh/h	129			426	212	
Approach Delay, s/veh	21.8			9.7	6.3	
Approach LOS		C		A	A	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+R _c), s	32.5		8.3	8.2	24.3	
Change Period (Y+R _c), s	4.5		4.5	4.5	4.5	
Max Green Setting (Gmax), s	28.0		18.0	5.5	18.0	
Max Q Clear Time (g_c+l1), s	4.6		4.4	4.8	3.7	
Green Ext Time (p_c), s	1.8		0.3	0.0	1.0	
Intersection Summary						
HCM 6th Ctrl Delay			10.8			
HCM 6th LOS			B			

Trans Truck System Truck Facility TIA
1: Roth Rd & Manthey Rd

Existing
PM PEAK HOUR

Intersection						
Int Delay, s/veh	4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑		↑	↑
Traffic Vol, veh/h	31	40	57	27	48	51
Future Vol, veh/h	31	40	57	27	48	51
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	89	89	88	88	77	77
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	35	45	65	31	62	66
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	271	81	0	0	96	0
Stage 1	81	-	-	-	-	-
Stage 2	190	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	718	979	-	-	1498	-
Stage 1	942	-	-	-	-	-
Stage 2	842	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	687	979	-	-	1498	-
Mov Cap-2 Maneuver	687	-	-	-	-	-
Stage 1	901	-	-	-	-	-
Stage 2	842	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	9.6	0		3.6		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	687	979	1498	-
HCM Lane V/C Ratio	-	-	0.051	0.046	0.042	-
HCM Control Delay (s)	-	-	10.5	8.9	7.5	0
HCM Lane LOS	-	-	B	A	A	A
HCM 95th %tile Q(veh)	-	-	0.2	0.1	0.1	-

Trans Truck System Truck Facility TIA
2: Roth Rd & I-5 SB Ramp

Existing
PM PEAK HOUR

Intersection												
Int Delay, s/veh 9.9												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			↑	↑	↑					↑	↑	
Traffic Vol, veh/h	0	60	15	137	47	0	0	0	0	207	4	24
Future Vol, veh/h	0	60	15	137	47	0	0	0	0	207	4	24
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	40	300	-	-	-	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	16974	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	81	81	81	92	92	92	85	85	85
Heavy Vehicles, %	4	4	4	23	23	23	2	2	2	15	15	15
Mvmt Flow	0	67	17	169	58	0	0	0	0	244	5	28
Major/Minor												
Major1			Major2			Minor2						
Conflicting Flow All	-	0	0	84	0	0	472	480	29			
Stage 1	-	-	-	-	-	-	396	396	-			
Stage 2	-	-	-	-	-	-	76	84	-			
Critical Hdwy	-	-	-	4.445	-	-	6.825	6.725	7.125			
Critical Hdwy Stg 1	-	-	-	-	-	-	6.025	5.725	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	5.625	5.725	-			
Follow-up Hdwy	-	-	-	2.4185	-	-	3.6425	4.1425	3.4425			
Pot Cap-1 Maneuver	0	-	-	1380	-	0	508	461	1001			
Stage 1	0	-	-	-	-	0	618	576	-			
Stage 2	0	-	-	-	-	0	912	798	-			
Platoon blocked, %	-	-	-	-	-	-						
Mov Cap-1 Maneuver	-	-	-	1380	-	-	446	0	1001			
Mov Cap-2 Maneuver	-	-	-	-	-	-	446	0	-			
Stage 1	-	-	-	-	-	-	543	0	-			
Stage 2	-	-	-	-	-	-	912	0	-			
Approach												
EB			WB			SB						
HCM Control Delay, s	0			5.9			16.1					
HCM LOS							C					
Minor Lane/Major Mvmt												
EBT			EBR			WBL			SBLn1			
Capacity (veh/h)	-	-	-	1380	-	-	446	520				
HCM Lane V/C Ratio	-	-	-	0.123	-	-	0.364	0.219				
HCM Control Delay (s)	-	-	-	8	-	-	17.6	13.9				
HCM Lane LOS	-	-	-	A	-	-	C	B				
HCM 95th %tile Q(veh)	-	-	-	0.4	-	-	1.6	0.8				

Trans Truck System Truck Facility TIA
3: I-5 NB Ramp & Roth Rd

Existing
PM PEAK HOUR

Intersection													
Int Delay, s/veh 2.8													
Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑				↑	↑		↑	↑	0	0	0
Traffic Vol, veh/h	15	252	0	1	0	171	182	13	4	164	0	0	0
Future Vol, veh/h	15	252	0	1	0	171	182	13	4	164	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	-	None	-	-	None	-	-	None
Storage Length	300	-	-	-	-	-	50	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	-	0	-	-	0	-	16965	-	-
Grade, %	-	0	-	-	-	0	-	-	0	-	0	-	-
Peak Hour Factor	87	87	87	82	82	82	82	82	82	82	92	92	92
Heavy Vehicles, %	14	14	14	20	20	20	20	29	29	29	2	2	2
Mvmt Flow	17	290	0	1	0	209	222	16	5	200	0	0	0
Major/Minor													
Major1			Major2			Minor1							
Conflicting Flow All	431	0	-	290	-	-	0	644	757	145			
Stage 1	-	-	-	-	-	-	-	324	324	-			
Stage 2	-	-	-	-	-	-	-	320	433	-			
Critical Hdwy	4.31	-	-	7.2	-	-	-	7.035	6.935	7.335			
Critical Hdwy Stg 1	-	-	-	-	-	-	-	6.235	5.935	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	-	5.835	5.935	-			
Follow-up Hdwy	2.333	-	-	3.29	-	-	-	3.7755	4.2755	3.5755			
Pot Cap-1 Maneuver	1056	-	0	697	0	-	-	373	297	805			
Stage 1	-	-	0	-	0	-	-	642	595	-			
Stage 2	-	-	0	-	0	-	-	668	527	-			
Platoon blocked, %	-	-	-	-	-	-	-						
Mov Cap-1 Maneuver	1056	-	-	555	-	-	-	366	0	805			
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	366	0	-			
Stage 1	-	-	-	-	-	-	-	630	0	-			
Stage 2	-	-	-	-	-	-	-	668	0	-			
Approach													
EB			WB			NB							
HCM Control Delay, s	0.5			0				11.3					
HCM LOS								B					
Minor Lane/Major Mvmt													
	NBLn1	NBLn2	EBL	EBT	WBT	WBR							
Capacity (veh/h)	366	805	1056	-	-	-							
HCM Lane V/C Ratio	0.057	0.248	0.016	-	-	-							
HCM Control Delay (s)	15.4	10.9	8.5	-	-	-							
HCM Lane LOS	C	B	A	-	-	-							
HCM 95th %tile Q(veh)	0.2	1	0.1	-	-	-							

Trans Truck System Truck Facility TIA
4: Harlan Rd & Roth Rd

Existing
PM PEAK HOUR

Intersection													
Intersection Delay, s/veh	20.6												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↔	↔				↑	↑	↔	↔	
Traffic Vol, veh/h	35	199	184	54	132	21	1	178	49	72	37	34	41
Future Vol, veh/h	35	199	184	54	132	21	1	178	49	72	37	34	41
Peak Hour Factor	0.84	0.84	0.84	0.80	0.80	0.80	0.81	0.81	0.81	0.81	0.74	0.74	0.74
Heavy Vehicles, %	20	20	20	31	31	31	11	11	11	11	23	23	23
Mvmt Flow	42	237	219	68	165	26	1	220	60	89	50	46	55
Number of Lanes	1	1	1	0	1	0	0	0	1	1	0	1	0
Approach	EB		WB			NB			SB				
Opposing Approach	WB		EB			SB			NB				
Opposing Lanes	1		3			1			2				
Conflicting Approach Left	SB		NB			EB			WB				
Conflicting Lanes Left	1		2			3			1				
Conflicting Approach Right	NB		SB			WB			EB				
Conflicting Lanes Right	2		1			1			3				
HCM Control Delay	16.2		26.5			23.7			17.5				
HCM LOS	C		D			C			C				
Lane	NBLn1	NBLn2	EBLn1	EBLn2	EBLn3	WBLn1	SBLn1						
Vol Left, %	78%	0%	100%	0%	0%	26%	33%						
Vol Thru, %	22%	0%	0%	100%	0%	64%	30%						
Vol Right, %	0%	100%	0%	0%	100%	10%	37%						
Sign Control	Stop												
Traffic Vol by Lane	228	72	35	199	184	207	112						
LT Vol	179	0	35	0	0	54	37						
Through Vol	49	0	0	199	0	132	34						
RT Vol	0	72	0	0	184	21	41						
Lane Flow Rate	281	89	42	237	219	259	151						
Geometry Grp	8	8	7	7	7	8	8						
Degree of Util (X)	0.671	0.184	0.095	0.506	0.424	0.644	0.384						
Departure Headway (Hd)	8.687	7.567	8.303	7.69	6.972	8.962	9.143						
Convergence, Y/N	Yes												
Cap	419	477	434	467	513	406	395						
Service Time	6.387	5.267	6.003	5.489	4.77	6.662	6.858						
HCM Lane V/C Ratio	0.671	0.187	0.097	0.507	0.427	0.638	0.382						
HCM Control Delay	27.4	12	11.9	18.2	14.9	26.5	17.5						
HCM Lane LOS	D	B	B	C	B	D	C						
HCM 95th-tile Q	4.8	0.7	0.3	2.8	2.1	4.4	1.8						

Intersection						
Int Delay, s/veh	2.5					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	
Traffic Vol, veh/h	52	153	124	25	21	34
Future Vol, veh/h	52	153	124	25	21	34
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	85	74	74	76	76
Heavy Vehicles, %	20	20	28	28	4	4
Mvmt Flow	61	180	168	34	28	45
Major/Minor						
Major1		Major2		Minor2		
Conflicting Flow All	202	0	-	0	487	185
Stage 1	-	-	-	-	185	-
Stage 2	-	-	-	-	302	-
Critical Hdwy	4.3	-	-	-	6.44	6.24
Critical Hdwy Stg 1	-	-	-	-	5.44	-
Critical Hdwy Stg 2	-	-	-	-	5.44	-
Follow-up Hdwy	2.38	-	-	-	3.536	3.336
Pot Cap-1 Maneuver	1269	-	-	-	536	852
Stage 1	-	-	-	-	842	-
Stage 2	-	-	-	-	745	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1269	-	-	-	508	852
Mov Cap-2 Maneuver	-	-	-	-	508	-
Stage 1	-	-	-	-	797	-
Stage 2	-	-	-	-	745	-
Approach						
EB		WB		SB		
HCM Control Delay, s	2		0		11	
HCM LOS					B	
Minor Lane/Major Mvmt						
EBL		EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1269	-	-	-	677	
HCM Lane V/C Ratio	0.048	-	-	-	0.107	
HCM Control Delay (s)	8	0	-	-	11	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0.2	-	-	-	0.4	

Trans Truck System Truck Facility TIA
6: Roth Rd & Airport Way

Existing
PM PEAK HOUR

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	78	108	65	206	230	57
Future Volume (veh/h)	78	108	65	206	230	57
Initial Q (Q _b), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1633	1633	1811	1811	1737	1737
Adj Flow Rate, veh/h	98	135	82	261	267	66
Peak Hour Factor	0.80	0.80	0.79	0.79	0.86	0.86
Percent Heavy Veh, %	18	18	6	6	11	11
Cap, veh/h	222	197	125	1175	1243	302
Arrive On Green	0.14	0.14	0.07	0.65	0.47	0.47
Sat Flow, veh/h	1555	1384	1725	1811	2720	639
Grp Volume(v), veh/h	98	135	82	261	166	167
Grp Sat Flow(s),veh/h/ln	1555	1384	1725	1811	1650	1622
Q Serve(g_s), s	2.5	4.0	2.0	2.6	2.5	2.6
Cycle Q Clear(g_c), s	2.5	4.0	2.0	2.6	2.5	2.6
Prop In Lane	1.00	1.00	1.00			0.39
Lane Grp Cap(c), veh/h	222	197	125	1175	779	766
V/C Ratio(X)	0.44	0.68	0.66	0.22	0.21	0.22
Avail Cap(c_a), veh/h	649	577	220	1175	779	766
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	16.9	17.6	19.5	3.1	6.7	6.7
Incr Delay (d2), s/veh	1.4	4.1	5.7	0.4	0.6	0.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9	3.2	0.9	0.5	0.8	0.8
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	18.3	21.7	25.2	3.5	7.3	7.4
LnGrp LOS	B	C	C	A	A	A
Approach Vol, veh/h	233			343	333	
Approach Delay, s/veh	20.3			8.7	7.3	
Approach LOS		C		A	A	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s	32.5		10.7	7.6	24.9	
Change Period (Y+Rc), s	4.5		4.5	4.5	4.5	
Max Green Setting (Gmax), s	28.0		18.0	5.5	18.0	
Max Q Clear Time (g_c+l1), s	4.6		6.0	4.0	4.6	
Green Ext Time (p_c), s	1.5		0.6	0.0	1.6	
Intersection Summary						
HCM 6th Ctrl Delay			11.2			
HCM 6th LOS			B			

Appendix C

*Analysis Worksheets for
Existing (2019) plus Approved and Pending Project Conditions*

Trans Truck System Truck Facility TIA
1: Roth Rd & Manthey Rd

Existing + AP
AM PEAK HOUR

Intersection						
Int Delay, s/veh	5.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑			↖
Traffic Vol, veh/h	97	36	45	63	41	45
Future Vol, veh/h	97	36	45	63	41	45
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	65	65	52	52	74	74
Heavy Vehicles, %	6	6	6	6	6	6
Mvmt Flow	149	55	87	121	55	61
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	319	148	0	0	208	0
Stage 1	148	-	-	-	-	-
Stage 2	171	-	-	-	-	-
Critical Hdwy	6.46	6.26	-	-	4.16	-
Critical Hdwy Stg 1	5.46	-	-	-	-	-
Critical Hdwy Stg 2	5.46	-	-	-	-	-
Follow-up Hdwy	3.554	3.354	-	-	2.254	-
Pot Cap-1 Maneuver	666	888	-	-	1339	-
Stage 1	870	-	-	-	-	-
Stage 2	849	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	637	888	-	-	1339	-
Mov Cap-2 Maneuver	637	-	-	-	-	-
Stage 1	833	-	-	-	-	-
Stage 2	849	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	11.6	0		3.7		
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	637	888	1339	-
HCM Lane V/C Ratio	-	-	0.234	0.062	0.041	-
HCM Control Delay (s)	-	-	12.4	9.3	7.8	0
HCM Lane LOS	-	-	B	A	A	A
HCM 95th %tile Q(veh)	-	-	0.9	0.2	0.1	-

Trans Truck System Truck Facility TIA
2: Roth Rd & I-5 SB Ramp

Existing + AP
AM PEAK HOUR

Intersection												
Int Delay, s/veh 16.9												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	84	22	172	85	0	0	0	0	275	5	52
Future Vol, veh/h	0	84	22	172	85	0	0	0	0	275	5	52
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	40	300	-	-	-	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	16974	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	56	56	56	79	79	79	92	92	92	88	88	88
Heavy Vehicles, %	12	12	12	40	40	40	40	40	40	20	20	20
Mvmt Flow	0	150	39	218	108	0	0	0	0	313	6	59
Major/Minor												
Major1			Major2			Minor2						
Conflicting Flow All	-	0	0	189	0	0			714	733	54	
Stage 1	-	-	-	-	-	-			544	544	-	
Stage 2	-	-	-	-	-	-			170	189	-	
Critical Hdwy	-	-	-	4.7	-	-			6.9	6.8	7.2	
Critical Hdwy Stg 1	-	-	-	-	-	-			6.1	5.8	-	
Critical Hdwy Stg 2	-	-	-	-	-	-			5.7	5.8	-	
Follow-up Hdwy	-	-	-	2.58	-	-			3.69	4.19	3.49	
Pot Cap-1 Maneuver	0	-	-	1166	-	0			350	320	950	
Stage 1	0	-	-	-	-	0			506	483	-	
Stage 2	0	-	-	-	-	0			812	706	-	
Platoon blocked, %	-	-	-	-	-	-						
Mov Cap-1 Maneuver	-	-	-	1166	-	-			~ 285	0	950	
Mov Cap-2 Maneuver	-	-	-	-	-	-			~ 285	0	-	
Stage 1	-	-	-	-	-	-			411	0	-	
Stage 2	-	-	-	-	-	-			812	0	-	
Approach												
EB			WB			SB						
HCM Control Delay, s	0			5.9					34.8			
HCM LOS									D			
Minor Lane/Major Mvmt												
EBT			EBR			WBL			SBLn1			
Capacity (veh/h)	-	-	-	1166	-	-	285	382				
HCM Lane V/C Ratio	-	-	-	0.187	-	-	0.731	0.442				
HCM Control Delay (s)	-	-	-	8.8	-	-	45.5	21.7				
HCM Lane LOS	-	-	-	A	-	-	E	C				
HCM 95th %tile Q(veh)	-	-	-	0.7	-	-	5.3	2.2				
Notes												
-: Volume exceeds capacity			\$: Delay exceeds 300s			+: Computation Not Defined			*: All major volume in platoon			

Intersection												
Int Delay, s/veh 2.8												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑			↑	↑	↓	↓	↑	0	0	0
Traffic Vol, veh/h	43	316	0	0	225	284	15	3	200	0	0	0
Future Vol, veh/h	43	316	0	0	225	284	15	3	200	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	300	-	-	-	-	50	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	16965	-	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	81	81	81	93	93	93	92	92	92
Heavy Vehicles, %	20	20	20	34	34	34	49	49	49	40	40	40
Mvmt Flow	47	347	0	0	278	351	16	3	215	0	0	0
Major/Minor												
Major1		Major2			Minor1							
Conflicting Flow All	629	0	-	-	-	0	895	1070	174			
Stage 1	-	-	-	-	-	-	441	441	-			
Stage 2	-	-	-	-	-	-	454	629	-			
Critical Hdwy	4.4	-	-	-	-	-	7.335	7.235	7.635			
Critical Hdwy Stg 1	-	-	-	-	-	-	6.535	6.235	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	6.135	6.235	-			
Follow-up Hdwy	2.39	-	-	-	-	-	3.9655	4.4655	3.7655			
Pot Cap-1 Maneuver	854	-	0	0	-	-	231	170	723			
Stage 1	-	-	0	0	-	-	515	488	-			
Stage 2	-	-	0	0	-	-	532	391	-			
Platoon blocked, %	-	-	-	-	-	-						
Mov Cap-1 Maneuver	854	-	-	-	-	-	218	0	723			
Mov Cap-2 Maneuver	-	-	-	-	-	-	218	0	-			
Stage 1	-	-	-	-	-	-	487	0	-			
Stage 2	-	-	-	-	-	-	532	0	-			
Approach												
EB		WB			NB							
HCM Control Delay, s	1.1			0			13					
HCM LOS							B					
Minor Lane/Major Mvmt												
NBLn1		NBLn2		EBL	EBT	WBT	WBR					
Capacity (veh/h)	218	723	854	-	-	-						
HCM Lane V/C Ratio	0.089	0.297	0.055	-	-	-						
HCM Control Delay (s)	23.1	12.1	9.5	-	-	-						
HCM Lane LOS	C	B	A	-	-	-						
HCM 95th %tile Q(veh)	0.3	1.2	0.2	-	-	-						

Intersection												
Intersection Delay, s/veh	57.9											
Intersection LOS	F											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↔	↔	↔	↑	↑	↑	↔	↔	↔
Traffic Vol, veh/h	77	295	144	35	229	42	234	44	51	39	18	66
Future Vol, veh/h	77	295	144	35	229	42	234	44	51	39	18	66
Peak Hour Factor	0.94	0.94	0.94	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Heavy Vehicles, %	32	32	32	46	46	46	13	13	13	53	53	53
Mvmt Flow	82	314	153	44	290	53	296	56	65	49	23	84
Number of Lanes	1	1	1	0	1	0	0	1	1	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			3			1			2		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			2			3			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	2			1			1			3		
HCM Control Delay	29.2			108.6			61.1			24		
HCM LOS	D			F			F			C		
Lane	NBLn1	NBLn2	EBLn1	EBLn2	EBLn3	WBLn1	SBLn1					
Vol Left, %	84%	0%	100%	0%	0%	11%	32%					
Vol Thru, %	16%	0%	0%	100%	0%	75%	15%					
Vol Right, %	0%	100%	0%	0%	100%	14%	54%					
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol by Lane	278	51	77	295	144	306	123					
LT Vol	234	0	77	0	0	35	39					
Through Vol	44	0	0	295	0	229	18					
RT Vol	0	51	0	0	144	42	66					
Lane Flow Rate	352	65	82	314	153	387	156					
Geometry Grp	8	8	7	7	7	8	8					
Degree of Util (X)	0.955	0.155	0.217	0.787	0.354	1.095	0.47					
Departure Headway (Hd)	10.227	9.058	9.939	9.418	8.689	10.177	11.506					
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes	Yes					
Cap	359	398	364	386	417	356	315					
Service Time	7.927	6.758	7.639	7.118	6.389	7.957	9.206					
HCM Lane V/C Ratio	0.981	0.163	0.225	0.813	0.367	1.087	0.495					
HCM Control Delay	69.9	13.4	15.4	39.2	16.1	108.6	24					
HCM Lane LOS	F	B	C	E	C	F	C					
HCM 95th-tile Q	10.3	0.5	0.8	6.7	1.6	14.3	2.4					

Intersection						
Int Delay, s/veh						
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	
Traffic Vol, veh/h	29	288	204	17	36	37
Future Vol, veh/h	29	288	204	17	36	37
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	84	84	79	79	79	79
Heavy Vehicles, %	34	34	39	39	6	6
Mvmt Flow	35	343	258	22	46	47
Major/Minor						
Major1		Major2		Minor2		
Conflicting Flow All	280	0	-	0	682	269
Stage 1	-	-	-	-	269	-
Stage 2	-	-	-	-	413	-
Critical Hdwy	4.44	-	-	-	6.46	6.26
Critical Hdwy Stg 1	-	-	-	-	5.46	-
Critical Hdwy Stg 2	-	-	-	-	5.46	-
Follow-up Hdwy	2.506	-	-	-	3.554	3.354
Pot Cap-1 Maneuver	1119	-	-	-	409	760
Stage 1	-	-	-	-	767	-
Stage 2	-	-	-	-	659	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1119	-	-	-	393	760
Mov Cap-2 Maneuver	-	-	-	-	393	-
Stage 1	-	-	-	-	737	-
Stage 2	-	-	-	-	659	-
Approach						
EB		WB		SB		
HCM Control Delay, s	0.8		0		13.4	
HCM LOS					B	
Minor Lane/Major Mvmt						
EBL		EBT		WBT		SBLn1
Capacity (veh/h)	1119	-	-	-	-	520
HCM Lane V/C Ratio	0.031	-	-	-	-	0.178
HCM Control Delay (s)	8.3	0	-	-	-	13.4
HCM Lane LOS	A	A	-	-	-	B
HCM 95th %tile Q(veh)	0.1	-	-	-	-	0.6

Trans Truck System Truck Facility TIA
6: Roth Rd & Airport Way

Existing + AP
AM PEAK HOUR

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	53	70	115	286	147	79
Future Volume (veh/h)	53	70	115	286	147	79
Initial Q (Q _b), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1470	1470	1796	1796	1589	1589
Adj Flow Rate, veh/h	66	88	139	345	158	85
Peak Hour Factor	0.80	0.80	0.83	0.83	0.93	0.93
Percent Heavy Veh, %	29	29	7	7	21	21
Cap, veh/h	145	129	175	1218	903	462
Arrive On Green	0.10	0.10	0.10	0.68	0.47	0.47
Sat Flow, veh/h	1400	1246	1711	1796	2011	988
Grp Volume(v), veh/h	66	88	139	345	122	121
Grp Sat Flow(s),veh/h/ln	1400	1246	1711	1796	1509	1411
Q Serve(g_s), s	1.8	2.8	3.3	3.2	1.9	2.1
Cycle Q Clear(g_c), s	1.8	2.8	3.3	3.2	1.9	2.1
Prop In Lane	1.00	1.00	1.00			0.70
Lane Grp Cap(c), veh/h	145	129	175	1218	705	659
V/C Ratio(X)	0.45	0.68	0.80	0.28	0.17	0.18
Avail Cap(c_a), veh/h	611	543	228	1218	705	659
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	17.4	17.8	18.1	2.6	6.4	6.4
Incr Delay (d2), s/veh	2.2	6.2	13.6	0.6	0.5	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	2.1	1.8	0.5	0.5	0.5
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	19.6	24.0	31.7	3.2	6.9	7.0
LnGrp LOS	B	C	C	A	A	A
Approach Vol, veh/h	154			484	243	
Approach Delay, s/veh	22.1			11.4	7.0	
Approach LOS		C		B	A	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+R _c), s	32.5			8.8	8.7	23.8
Change Period (Y+R _c), s	4.5			4.5	4.5	4.5
Max Green Setting (Gmax), s	28.0			18.0	5.5	18.0
Max Q Clear Time (g_c+l1), s	5.2			4.8	5.3	4.1
Green Ext Time (p_c), s	2.1			0.4	0.0	1.1
Intersection Summary						
HCM 6th Ctrl Delay			12.1			
HCM 6th LOS			B			

Intersection						
Int Delay, s/veh	4.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑		↑	↑
Traffic Vol, veh/h	48	86	112	53	63	51
Future Vol, veh/h	48	86	112	53	63	51
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	89	89	88	88	77	77
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	54	97	127	60	82	66
Major/Minor						
Conflicting Flow All	387	157	0	0	187	0
Stage 1	157	-	-	-	-	-
Stage 2	230	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	616	889	-	-	1387	-
Stage 1	871	-	-	-	-	-
Stage 2	808	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	578	889	-	-	1387	-
Mov Cap-2 Maneuver	578	-	-	-	-	-
Stage 1	818	-	-	-	-	-
Stage 2	808	-	-	-	-	-
Approach						
	WB	NB	SB			
HCM Control Delay, s	10.4	0	4.3			
HCM LOS	B					
Minor Lane/Major Mvmt						
	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	578	889	1387	-
HCM Lane V/C Ratio	-	-	0.093	0.109	0.059	-
HCM Control Delay (s)	-	-	11.9	9.5	7.8	0
HCM Lane LOS	-	-	B	A	A	A
HCM 95th %tile Q(veh)	-	-	0.3	0.4	0.2	-

Trans Truck System Truck Facility TIA
2: Roth Rd & I-5 SB Ramp

Existing + AP
PM PEAK HOUR

Intersection												
Int Delay, s/veh 13.9												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	80	36	171	96	0	0	0	0	266	4	38
Future Vol, veh/h	0	80	36	171	96	0	0	0	0	266	4	38
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	40	300	-	-	-	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	16974	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	81	81	81	92	92	92	85	85	85
Heavy Vehicles, %	4	4	4	23	23	23	2	2	2	15	15	15
Mvmt Flow	0	90	40	211	119	0	0	0	0	313	5	45
Major/Minor												
Major1			Major2			Minor2						
Conflicting Flow All	-	0	0	130	0	0			651	671	60	
Stage 1	-	-	-	-	-	-			541	541	-	
Stage 2	-	-	-	-	-	-			110	130	-	
Critical Hdwy	-	-	-	4.445	-	-			6.825	6.725	7.125	
Critical Hdwy Stg 1	-	-	-	-	-	-			6.025	5.725	-	
Critical Hdwy Stg 2	-	-	-	-	-	-			5.625	5.725	-	
Follow-up Hdwy	-	-	-	2.4185	-	-			3.6425	4.1425	3.4425	
Pot Cap-1 Maneuver	0	-	-	1324	-	0			393	356	956	
Stage 1	0	-	-	-	-	0			519	494	-	
Stage 2	0	-	-	-	-	0			879	761	-	
Platoon blocked, %	-	-	-	-	-	-						
Mov Cap-1 Maneuver	-	-	-	1324	-	-			331	0	956	
Mov Cap-2 Maneuver	-	-	-	-	-	-			331	0	-	
Stage 1	-	-	-	-	-	-			436	0	-	
Stage 2	-	-	-	-	-	-			879	0	-	
Approach												
EB			WB			SB						
HCM Control Delay, s	0			5.3					26.8			
HCM LOS									D			
Minor Lane/Major Mvmt												
		EBT	EBR	WBL	WBT	SBLn1	SBLn2					
Capacity (veh/h)	-	-	1324	-	331	412						
HCM Lane V/C Ratio	-	-	0.159	-	0.63	0.373						
HCM Control Delay (s)	-	-	8.2	-	32.7	18.8						
HCM Lane LOS	-	-	A	-	D	C						
HCM 95th %tile Q(veh)	-	-	0.6	-	4	1.7						

Intersection													
Int Delay, s/veh													
Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑				↑	↑		↑	↑			
Traffic Vol, veh/h	35	311	0	1	0	231	375	36	5	194	0	0	0
Future Vol, veh/h	35	311	0	1	0	231	375	36	5	194	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	-	None	-	-	None	-	-	None
Storage Length	300	-	-	-	-	-	50	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	-	0	-	-	0	-	16965	-	-
Grade, %	-	0	-	-	-	0	-	-	0	-	0	-	-
Peak Hour Factor	87	87	87	82	82	82	82	82	82	82	92	92	92
Heavy Vehicles, %	14	14	14	20	20	20	20	29	29	29	2	2	2
Mvmt Flow	40	357	0	1	0	282	457	44	6	237	0	0	0
Major/Minor													
Major1			Major2			Minor1							
Conflicting Flow All	739	0	-	357	-	-	0	948	1178	179			
Stage 1	-	-	-	-	-	-	-	437	437	-			
Stage 2	-	-	-	-	-	-	-	511	741	-			
Critical Hdwy	4.31	-	-	7.2	-	-	-	7.035	6.935	7.335			
Critical Hdwy Stg 1	-	-	-	-	-	-	-	6.235	5.935	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	-	5.835	5.935	-			
Follow-up Hdwy	2.333	-	-	3.29	-	-	-	3.7755	4.2755	3.5755			
Pot Cap-1 Maneuver	802	-	0	628	0	-	-	236	162	763			
Stage 1	-	-	0	-	0	-	-	558	525	-			
Stage 2	-	-	0	-	0	-	-	538	373	-			
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-			
Mov Cap-1 Maneuver	802	-	-	468	-	-	-	223	0	763			
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	223	0	-			
Stage 1	-	-	-	-	-	-	-	528	0	-			
Stage 2	-	-	-	-	-	-	-	538	0	-			
Approach													
EB			WB			NB							
HCM Control Delay, s	1			0			14.2						
HCM LOS							B						
Minor Lane/Major Mvmt													
Capacity (veh/h)	223	763	802	-	-	-	-	-	-	-	-	-	
HCM Lane V/C Ratio	0.224	0.31	0.05	-	-	-	-	-	-	-	-	-	
HCM Control Delay (s)	25.7	11.8	9.7	-	-	-	-	-	-	-	-	-	
HCM Lane LOS	D	B	A	-	-	-	-	-	-	-	-	-	
HCM 95th %tile Q(veh)	0.8	1.3	0.2	-	-	-	-	-	-	-	-	-	

Intersection													
Intersection Delay, s/veh		141.9											
Intersection LOS		F											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↔	↔				↑	↑	↔	↔	
Traffic Vol, veh/h	76	279	184	54	335	62	1	206	59	86	60	53	65
Future Vol, veh/h	76	279	184	54	335	62	1	206	59	86	60	53	65
Peak Hour Factor	0.84	0.84	0.84	0.80	0.80	0.80	0.81	0.81	0.81	0.81	0.74	0.74	0.74
Heavy Vehicles, %	20	20	20	31	31	31	11	11	11	11	23	23	23
Mvmt Flow	90	332	219	68	419	78	1	254	73	106	81	72	88
Number of Lanes	1	1	1	0	1	0	0	0	1	1	0	1	0
Approach	EB		WB			NB				SB			
Opposing Approach	WB		EB			SB				NB			
Opposing Lanes	1		3			1				2			
Conflicting Approach Left	SB		NB			EB				WB			
Conflicting Lanes Left	1		2			3				1			
Conflicting Approach Right	NB		SB			WB				EB			
Conflicting Lanes Right	2		1			1				3			
HCM Control Delay	39		361.2			63.9				43.5			
HCM LOS	E		F			F				E			
Lane	NBLn1	NBLn2	EBLn1	EBLn2	EBLn3	WBLn1	SBLn1						
Vol Left, %	78%	0%	100%	0%	0%	12%	34%						
Vol Thru, %	22%	0%	0%	100%	0%	74%	30%						
Vol Right, %	0%	100%	0%	0%	100%	14%	37%						
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop						
Traffic Vol by Lane	266	86	76	279	184	451	178						
LT Vol	207	0	76	0	0	54	60						
Through Vol	59	0	0	279	0	335	53						
RT Vol	0	86	0	0	184	62	65						
Lane Flow Rate	328	106	90	332	219	564	241						
Geometry Grp	8	8	7	7	7	8	8						
Degree of Util (X)	0.956	0.277	0.247	0.862	0.526	1.718	0.726						
Departure Headway (Hd)	12.343	11.184	11.482	10.954	10.214	10.973	12.92						
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes	Yes						
Cap	296	324	315	332	355	338	281						
Service Time	10.043	8.884	9.182	8.654	7.914	8.673	10.62						
HCM Lane V/C Ratio	1.108	0.327	0.286	1	0.617	1.669	0.858						
HCM Control Delay	78.7	18.1	17.9	54.8	23.7	361.2	43.5						
HCM Lane LOS	F	C	C	F	C	F	E						
HCM 95th-tile Q	9.5	1.1	1	7.9	2.9	35.4	5.2						

Intersection						
Int Delay, s/veh						
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	
Traffic Vol, veh/h	67	298	318	38	21	45
Future Vol, veh/h	67	298	318	38	21	45
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	85	74	74	76	76
Heavy Vehicles, %	20	20	28	28	4	4
Mvmt Flow	79	351	430	51	28	59
Major/Minor						
Major1		Major2		Minor2		
Conflicting Flow All	481	0	-	0	965	456
Stage 1	-	-	-	-	456	-
Stage 2	-	-	-	-	509	-
Critical Hdwy	4.3	-	-	-	6.44	6.24
Critical Hdwy Stg 1	-	-	-	-	5.44	-
Critical Hdwy Stg 2	-	-	-	-	5.44	-
Follow-up Hdwy	2.38	-	-	-	3.536	3.336
Pot Cap-1 Maneuver	994	-	-	-	280	600
Stage 1	-	-	-	-	634	-
Stage 2	-	-	-	-	600	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	994	-	-	-	252	600
Mov Cap-2 Maneuver	-	-	-	-	252	-
Stage 1	-	-	-	-	571	-
Stage 2	-	-	-	-	600	-
Approach						
EB		WB		SB		
HCM Control Delay, s	1.6		0		15.9	
HCM LOS					C	
Minor Lane/Major Mvmt						
EBL		EBT		WBT		SBLn1
Capacity (veh/h)	994	-	-	-	-	417
HCM Lane V/C Ratio	0.079	-	-	-	-	0.208
HCM Control Delay (s)	8.9	0	-	-	-	15.9
HCM Lane LOS	A	A	-	-	-	C
HCM 95th %tile Q(veh)	0.3	-	-	-	-	0.8

Trans Truck System Truck Facility TIA
6: Roth Rd & Airport Way

Existing + AP
PM PEAK HOUR

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	125	169	103	319	356	91
Future Volume (veh/h)	125	169	103	319	356	91
Initial Q (Q _b), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1633	1633	1811	1811	1737	1737
Adj Flow Rate, veh/h	156	211	130	404	414	106
Peak Hour Factor	0.80	0.80	0.79	0.79	0.86	0.86
Percent Heavy Veh, %	18	18	6	6	11	11
Cap, veh/h	316	281	164	1092	1072	272
Arrive On Green	0.20	0.20	0.10	0.60	0.41	0.41
Sat Flow, veh/h	1555	1384	1725	1811	2694	661
Grp Volume(v), veh/h	156	211	130	404	261	259
Grp Sat Flow(s),veh/h/ln	1555	1384	1725	1811	1650	1618
Q Serve(g_s), s	4.1	6.7	3.4	5.3	5.1	5.2
Cycle Q Clear(g_c), s	4.1	6.7	3.4	5.3	5.1	5.2
Prop In Lane	1.00	1.00	1.00			0.41
Lane Grp Cap(c), veh/h	316	281	164	1092	678	665
V/C Ratio(X)	0.49	0.75	0.79	0.37	0.38	0.39
Avail Cap(c_a), veh/h	603	537	204	1092	678	665
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	16.4	17.4	20.6	4.7	9.6	9.6
Incr Delay (d2), s/veh	1.2	4.0	15.5	1.0	1.6	1.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.4	0.3	1.9	1.5	1.8	1.8
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	17.6	21.4	36.1	5.7	11.2	11.3
LnGrp LOS	B	C	D	A	B	B
Approach Vol, veh/h	367			534	520	
Approach Delay, s/veh	19.8			13.1	11.3	
Approach LOS	B			B	B	
Timer - Assigned Phs	2			4	5	6
Phs Duration (G+Y+Rc), s	32.5			13.9	8.9	23.6
Change Period (Y+Rc), s	4.5			4.5	4.5	4.5
Max Green Setting (Gmax), s	28.0			18.0	5.5	18.0
Max Q Clear Time (g_c+l1), s	7.3			8.7	5.4	7.2
Green Ext Time (p_c), s	2.5			0.9	0.0	2.4
Intersection Summary						
HCM 6th Ctrl Delay				14.1		
HCM 6th LOS				B		

Appendix D

*Analysis Worksheets for
Existing (2019) plus Approved and Pending Project plus Proposed Project Conditions*

Intersection						
Int Delay, s/veh	5.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑			↑
Traffic Vol, veh/h	97	36	45	63	41	45
Future Vol, veh/h	97	36	45	63	41	45
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	65	65	52	52	74	74
Heavy Vehicles, %	6	6	6	6	6	6
Mvmt Flow	149	55	87	121	55	61
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	319	148	0	0	208	0
Stage 1	148	-	-	-	-	-
Stage 2	171	-	-	-	-	-
Critical Hdwy	6.46	6.26	-	-	4.16	-
Critical Hdwy Stg 1	5.46	-	-	-	-	-
Critical Hdwy Stg 2	5.46	-	-	-	-	-
Follow-up Hdwy	3.554	3.354	-	-	2.254	-
Pot Cap-1 Maneuver	666	888	-	-	1339	-
Stage 1	870	-	-	-	-	-
Stage 2	849	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	637	888	-	-	1339	-
Mov Cap-2 Maneuver	637	-	-	-	-	-
Stage 1	833	-	-	-	-	-
Stage 2	849	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	11.6	0		3.7		
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	637	888	1339	-
HCM Lane V/C Ratio	-	-	0.234	0.062	0.041	-
HCM Control Delay (s)	-	-	12.4	9.3	7.8	0
HCM Lane LOS	-	-	B	A	A	A
HCM 95th %tile Q(veh)	-	-	0.9	0.2	0.1	-

Trans Truck System Truck Facility TIA
2: Roth Rd & I-5 SB Ramp

Existing + AP plus Project
AM PEAK HOUR

Intersection												
Int Delay, s/veh 18.1												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	84	22	177	85	0	0	0	0	279	5	52
Future Vol, veh/h	0	84	22	177	85	0	0	0	0	279	5	52
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	40	300	-	-	-	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	16974	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	56	56	56	79	79	79	92	92	92	88	88	88
Heavy Vehicles, %	12	12	12	40	40	40	40	40	40	20	20	20
Mvmt Flow	0	150	39	224	108	0	0	0	0	317	6	59
Major/Minor												
Major1			Major2			Minor2						
Conflicting Flow All	-	0	0	189	0	0			726	745	54	
Stage 1	-	-	-	-	-	-			556	556	-	
Stage 2	-	-	-	-	-	-			170	189	-	
Critical Hdwy	-	-	-	4.7	-	-			6.9	6.8	7.2	
Critical Hdwy Stg 1	-	-	-	-	-	-			6.1	5.8	-	
Critical Hdwy Stg 2	-	-	-	-	-	-			5.7	5.8	-	
Follow-up Hdwy	-	-	-	2.58	-	-			3.69	4.19	3.49	
Pot Cap-1 Maneuver	0	-	-	1166	-	0			344	315	950	
Stage 1	0	-	-	-	-	0			499	476	-	
Stage 2	0	-	-	-	-	0			812	706	-	
Platoon blocked, %	-	-	-	-	-	-						
Mov Cap-1 Maneuver	-	-	-	1166	-	-			~ 278	0	950	
Mov Cap-2 Maneuver	-	-	-	-	-	-			~ 278	0	-	
Stage 1	-	-	-	-	-	-			403	0	-	
Stage 2	-	-	-	-	-	-			812	0	-	
Approach												
EB			WB			SB						
HCM Control Delay, s	0			6					37.6			
HCM LOS									E			
Minor Lane/Major Mvmt												
EBT			EBR			WBL			SBLn1			
Capacity (veh/h)	-	-	-	1166	-	-	278	372				
HCM Lane V/C Ratio	-	-	-	0.192	-	-	0.76	0.458				
HCM Control Delay (s)	-	-	-	8.8	-	-	49.7	22.6				
HCM Lane LOS	-	-	-	A	-	-	E	C				
HCM 95th %tile Q(veh)	-	-	-	0.7	-	-	5.7	2.3				
Notes												
~: Volume exceeds capacity	\$: Delay exceeds 300s	+: Computation Not Defined	*	*: All major volume in platoon								

Intersection												
Int Delay, s/veh 2.8												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑			↑	↑	↓	↓	↑	0	0	0
Traffic Vol, veh/h	43	320	0	0	230	290	15	3	204	0	0	0
Future Vol, veh/h	43	320	0	0	230	290	15	3	204	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	300	-	-	-	-	50	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	16965	-	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	81	81	81	93	93	93	92	92	92
Heavy Vehicles, %	20	20	20	34	34	34	49	49	49	40	40	40
Mvmt Flow	47	352	0	0	284	358	16	3	219	0	0	0
Major/Minor												
Major1		Major2			Minor1							
Conflicting Flow All	642	0	-	-	-	0	909	1088	176			
Stage 1	-	-	-	-	-	-	446	446	-			
Stage 2	-	-	-	-	-	-	463	642	-			
Critical Hdwy	4.4	-	-	-	-	-	7.335	7.235	7.635			
Critical Hdwy Stg 1	-	-	-	-	-	-	6.535	6.235	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	6.135	6.235	-			
Follow-up Hdwy	2.39	-	-	-	-	-	3.9655	4.4655	3.7655			
Pot Cap-1 Maneuver	844	-	0	0	-	-	225	165	721			
Stage 1	-	-	0	0	-	-	511	485	-			
Stage 2	-	-	0	0	-	-	526	385	-			
Platoon blocked, %	-	-	-	-	-	-						
Mov Cap-1 Maneuver	844	-	-	-	-	-	212	0	721			
Mov Cap-2 Maneuver	-	-	-	-	-	-	212	0	-			
Stage 1	-	-	-	-	-	-	482	0	-			
Stage 2	-	-	-	-	-	-	526	0	-			
Approach												
EB		WB			NB							
HCM Control Delay, s	1.1			0			13.1					
HCM LOS							B					
Minor Lane/Major Mvmt												
	NBLn1	NBLn2	EBL	EBT	WBT	WBR						
Capacity (veh/h)	212	721	844	-	-	-						
HCM Lane V/C Ratio	0.091	0.304	0.056	-	-	-						
HCM Control Delay (s)	23.7	12.2	9.5	-	-	-						
HCM Lane LOS	C	B	A	-	-	-						
HCM 95th %tile Q(veh)	0.3	1.3	0.2	-	-	-						

Intersection												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↔	↔	↑	↑	↑	↑	↔	↔	
Traffic Vol, veh/h	77	303	144	35	239	45	234	44	51	41	18	66
Future Vol, veh/h	77	303	144	35	239	45	234	44	51	41	18	66
Peak Hour Factor	0.94	0.94	0.94	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Heavy Vehicles, %	32	32	32	46	46	46	13	13	13	53	53	53
Mvmt Flow	82	322	153	44	303	57	296	56	65	52	23	84
Number of Lanes	1	1	1	0	1	0	0	1	1	0	1	0
Approach												
Opposing Approach	WB		EB		NB		SB		NB			
Opposing Lanes	1		3		1		2		NB			
Conflicting Approach Left	SB		NB		EB		WB					
Conflicting Lanes Left	1		2		3		1					
Conflicting Approach Right	NB		SB		WB		EB					
Conflicting Lanes Right	2		1		1		3					
HCM Control Delay	31.5		128.6		61.9		24.7					
HCM LOS	D		F		F		C					
Lane												
	NBLn1	NBLn2	EBLn1	EBLn2	EBLn3	WBLn1	SBLn1					
Vol Left, %	84%	0%	100%	0%	0%	11%	33%					
Vol Thru, %	16%	0%	0%	100%	0%	75%	14%					
Vol Right, %	0%	100%	0%	0%	100%	14%	53%					
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol by Lane	278	51	77	303	144	319	125					
LT Vol	234	0	77	0	0	35	41					
Through Vol	44	0	0	303	0	239	18					
RT Vol	0	51	0	0	144	45	66					
Lane Flow Rate	352	65	82	322	153	404	158					
Geometry Grp	8	8	7	7	7	8	8					
Degree of Util (X)	0.956	0.156	0.218	0.814	0.357	1.154	0.48					
Departure Headway (Hd)	10.404	9.233	10.062	9.54	8.81	10.291	11.7					
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes	Yes					
Cap	352	391	359	384	411	355	310					
Service Time	8.104	6.933	7.762	7.24	6.51	8.017	9.4					
HCM Lane V/C Ratio	1	0.166	0.228	0.839	0.372	1.138	0.51					
HCM Control Delay	70.8	13.6	15.5	42.8	16.3	128.6	24.7					
HCM Lane LOS	F	B	C	E	C	F	C					
HCM 95th-tile Q	10.2	0.5	0.8	7.2	1.6	16.1	2.5					

Intersection						
Int Delay, s/veh	2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	
Traffic Vol, veh/h	29	298	217	17	36	37
Future Vol, veh/h	29	298	217	17	36	37
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	84	84	79	79	79	79
Heavy Vehicles, %	34	34	39	39	6	6
Mvmt Flow	35	355	275	22	46	47
Major/Minor						
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	297	0	-	0	711	286
Stage 1	-	-	-	-	286	-
Stage 2	-	-	-	-	425	-
Critical Hdwy	4.44	-	-	-	6.46	6.26
Critical Hdwy Stg 1	-	-	-	-	5.46	-
Critical Hdwy Stg 2	-	-	-	-	5.46	-
Follow-up Hdwy	2.506	-	-	-	3.554	3.354
Pot Cap-1 Maneuver	1102	-	-	-	394	744
Stage 1	-	-	-	-	753	-
Stage 2	-	-	-	-	651	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1102	-	-	-	378	744
Mov Cap-2 Maneuver	-	-	-	-	378	-
Stage 1	-	-	-	-	723	-
Stage 2	-	-	-	-	651	-
Approach						
Approach	EB	WB	SB			
HCM Control Delay, s	0.7	0	13.7			
HCM LOS			B			
Minor Lane/Major Mvmt						
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1102	-	-	-	504	
HCM Lane V/C Ratio	0.031	-	-	-	0.183	
HCM Control Delay (s)	8.4	0	-	-	13.7	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0.1	-	-	-	0.7	

Trans Truck System Truck Facility TIA
6: Roth Rd & Airport Way

Existing + AP plus Project
AM PEAK HOUR



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	57	72	116	286	147	82
Future Volume (veh/h)	57	72	116	286	147	82
Initial Q (Q _b), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1470	1470	1796	1796	1589	1589
Adj Flow Rate, veh/h	71	90	140	345	158	88
Peak Hour Factor	0.80	0.80	0.83	0.83	0.93	0.93
Percent Heavy Veh, %	29	29	7	7	21	21
Cap, veh/h	150	133	176	1214	886	469
Arrive On Green	0.11	0.11	0.10	0.68	0.46	0.46
Sat Flow, veh/h	1400	1246	1711	1796	1987	1009
Grp Volume(v), veh/h	71	90	140	345	123	123
Grp Sat Flow(s), veh/h/ln	1400	1246	1711	1796	1509	1407
Q Serve(g_s), s	2.0	2.9	3.3	3.2	2.0	2.1
Cycle Q Clear(g_c), s	2.0	2.9	3.3	3.2	2.0	2.1
Prop In Lane	1.00	1.00	1.00			0.72
Lane Grp Cap(c), veh/h	150	133	176	1214	701	654
V/C Ratio(X)	0.47	0.68	0.80	0.28	0.18	0.19
Avail Cap(c_a), veh/h	608	541	227	1214	701	654
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	17.4	17.8	18.2	2.7	6.5	6.5
Incr Delay (d2), s/veh	2.3	5.9	13.9	0.6	0.5	0.6
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.6	0.2	1.8	0.6	0.5	0.6
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	19.7	23.7	32.0	3.3	7.0	7.1
LnGrp LOS	B	C	C	A	A	A
Approach Vol, veh/h	161			485	246	
Approach Delay, s/veh	21.9			11.6	7.1	
Approach LOS		C		B	A	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+R _c), s	32.5			8.9	8.8	23.7
Change Period (Y+R _c), s	4.5			4.5	4.5	4.5
Max Green Setting (Gmax), s	28.0			18.0	5.5	18.0
Max Q Clear Time (g_c+l1), s	5.2			4.9	5.3	4.1
Green Ext Time (p_c), s	2.1			0.4	0.0	1.2
Intersection Summary						
HCM 6th Ctrl Delay			12.2			
HCM 6th LOS			B			

Intersection						
Int Delay, s/veh	0.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	
Traffic Vol, veh/h	10	159	174	4	6	13
Future Vol, veh/h	10	159	174	4	6	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	40	40	2	2	2	2
Mvmt Flow	11	173	189	4	7	14
Major/Minor						
Major1		Major2		Minor2		
Conflicting Flow All	193	0	-	0	386	191
Stage 1	-	-	-	-	191	-
Stage 2	-	-	-	-	195	-
Critical Hdwy	4.5	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.56	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1182	-	-	-	617	851
Stage 1	-	-	-	-	841	-
Stage 2	-	-	-	-	838	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1182	-	-	-	611	851
Mov Cap-2 Maneuver	-	-	-	-	611	-
Stage 1	-	-	-	-	833	-
Stage 2	-	-	-	-	838	-
Approach						
EB		WB		SB		
HCM Control Delay, s	0.5		0		9.9	
HCM LOS					A	
Minor Lane/Major Mvmt						
EBL		EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1182	-	-	-	757	
HCM Lane V/C Ratio	0.009	-	-	-	0.027	
HCM Control Delay (s)	8.1	0	-	-	9.9	
HCM Lane LOS	A	A	-	-	A	
HCM 95th %tile Q(veh)	0	-	-	-	0.1	

Intersection						
Int Delay, s/veh	4.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑		↑	↑
Traffic Vol, veh/h	48	86	112	53	63	51
Future Vol, veh/h	48	86	112	53	63	51
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	89	89	88	88	77	77
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	54	97	127	60	82	66
Major/Minor						
Conflicting Flow All	387	157	0	0	187	0
Stage 1	157	-	-	-	-	-
Stage 2	230	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	616	889	-	-	1387	-
Stage 1	871	-	-	-	-	-
Stage 2	808	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	578	889	-	-	1387	-
Mov Cap-2 Maneuver	578	-	-	-	-	-
Stage 1	818	-	-	-	-	-
Stage 2	808	-	-	-	-	-
Approach						
	WB	NB	SB			
HCM Control Delay, s	10.4	0	4.3			
HCM LOS	B					
Minor Lane/Major Mvmt						
	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	578	889	1387	-
HCM Lane V/C Ratio	-	-	0.093	0.109	0.059	-
HCM Control Delay (s)	-	-	11.9	9.5	7.8	0
HCM Lane LOS	-	-	B	A	A	A
HCM 95th %tile Q(veh)	-	-	0.3	0.4	0.2	-

Intersection												
Int Delay, s/veh 14.6												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑	↑	↑	↑↑					↑	↑	
Traffic Vol, veh/h	0	80	36	175	96	0	0	0	0	270	4	38
Future Vol, veh/h	0	80	36	175	96	0	0	0	0	270	4	38
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	40	300	-	-	-	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	16974	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	81	81	81	92	92	92	85	85	85
Heavy Vehicles, %	4	4	4	23	23	23	2	2	2	15	15	15
Mvmt Flow	0	90	40	216	119	0	0	0	0	318	5	45
Major/Minor												
Major1			Major2			Minor2						
Conflicting Flow All	-	0	0	130	0	0			661	681	60	
Stage 1	-	-	-	-	-	-			551	551	-	
Stage 2	-	-	-	-	-	-			110	130	-	
Critical Hdwy	-	-	-	4.445	-	-			6.825	6.725	7.125	
Critical Hdwy Stg 1	-	-	-	-	-	-			6.025	5.725	-	
Critical Hdwy Stg 2	-	-	-	-	-	-			5.625	5.725	-	
Follow-up Hdwy	-	-	-	2.4185	-	-			3.6425	4.1425	3.4425	
Pot Cap-1 Maneuver	0	-	-	1324	-	0			387	351	956	
Stage 1	0	-	-	-	-	0			513	489	-	
Stage 2	0	-	-	-	-	0			879	761	-	
Platoon blocked, %	-	-	-	-	-	-						
Mov Cap-1 Maneuver	-	-	-	1324	-	-			324	0	956	
Mov Cap-2 Maneuver	-	-	-	-	-	-			324	0	-	
Stage 1	-	-	-	-	-	-			429	0	-	
Stage 2	-	-	-	-	-	-			879	0	-	
Approach												
EB			WB			SB						
HCM Control Delay, s	0			5.3					28.3			
HCM LOS									D			
Minor Lane/Major Mvmt												
		EBT	EBR	WBL	WBT	SBLn1	SBLn2					
Capacity (veh/h)	-	-	1324	-	324	403						
HCM Lane V/C Ratio	-	-	0.163	-	0.654	0.385						
HCM Control Delay (s)	-	-	8.2	-	34.8	19.4						
HCM Lane LOS	-	-	A	-	D	C						
HCM 95th %tile Q(veh)	-	-	0.6	-	4.3	1.8						

Intersection													
Int Delay, s/veh 3.2													
Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑				↑	↑	↑	↑	↑	0	0	0
Traffic Vol, veh/h	35	315	0	1	0	235	380	36	5	197	0	0	0
Future Vol, veh/h	35	315	0	1	0	235	380	36	5	197	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	-	None	-	-	None	-	-	None
Storage Length	300	-	-	-	-	-	50	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	-	0	-	-	0	-	16965	-	-
Grade, %	-	0	-	-	-	0	-	-	0	-	0	-	-
Peak Hour Factor	87	87	87	82	82	82	82	82	82	82	92	92	92
Heavy Vehicles, %	14	14	14	20	20	20	20	29	29	29	2	2	2
Mvmt Flow	40	362	0	1	0	287	463	44	6	240	0	0	0
Major/Minor													
Major1			Major2			Minor1							
Conflicting Flow All	750	0	-	362	-	-	0	961	1194	181			
Stage 1	-	-	-	-	-	-	-	442	442	-			
Stage 2	-	-	-	-	-	-	-	519	752	-			
Critical Hdwy	4.31	-	-	7.2	-	-	-	7.035	6.935	7.335			
Critical Hdwy Stg 1	-	-	-	-	-	-	-	6.235	5.935	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	-	5.835	5.935	-			
Follow-up Hdwy	2.333	-	-	3.29	-	-	-	3.7755	4.2755	3.5755			
Pot Cap-1 Maneuver	794	-	0	623	0	-	-	231	158	761			
Stage 1	-	-	0	-	0	-	-	554	522	-			
Stage 2	-	-	0	-	0	-	-	533	369	-			
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-			
Mov Cap-1 Maneuver	794	-	-	462	-	-	-	219	0	761			
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	219	0	-			
Stage 1	-	-	-	-	-	-	-	524	0	-			
Stage 2	-	-	-	-	-	-	-	533	0	-			
Approach													
EB			WB			NB							
HCM Control Delay, s	1			0			14.4						
HCM LOS							B						
Minor Lane/Major Mvmt													
	NBLn1	NBLn2	EBL	EBT	WBT	WBR							
Capacity (veh/h)	219	761	794	-	-	-							
HCM Lane V/C Ratio	0.228	0.316	0.051	-	-	-							
HCM Control Delay (s)	26.2	11.9	9.8	-	-	-							
HCM Lane LOS	D	B	A	-	-	-							
HCM 95th %tile Q(veh)	0.9	1.4	0.2	-	-	-							

Intersection													
Intersection Delay, s/veh	152.1												
Intersection LOS	F												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↓	↓	↓	↑	↑	↑	↑	↓	↓	↓
Traffic Vol, veh/h	76	286	184	54	344	65	1	206	59	86	62	53	65
Future Vol, veh/h	76	286	184	54	344	65	1	206	59	86	62	53	65
Peak Hour Factor	0.84	0.84	0.84	0.80	0.80	0.80	0.81	0.81	0.81	0.81	0.74	0.74	0.74
Heavy Vehicles, %	20	20	20	31	31	31	11	11	11	11	23	23	23
Mvmt Flow	90	340	219	68	430	81	1	254	73	106	84	72	88
Number of Lanes	1	1	1	0	1	0	0	0	1	1	0	1	0
Approach	EB			WB			NB			SB			
Opposing Approach	WB			EB			SB			NB			
Opposing Lanes	1			3			1			2			
Conflicting Approach Left	SB			NB			EB			WB			
Conflicting Lanes Left	1			2			3			1			
Conflicting Approach Right	NB			SB			WB			EB			
Conflicting Lanes Right	2			1			1			3			
HCM Control Delay	41.8			386.2			65.3			45.3			
HCM LOS	E			F			F			E			
Lane	NBLn1	NBLn2	EBLn1	EBLn2	EBLn3	WBLn1	SBLn1						
Vol Left, %	78%	0%	100%	0%	0%	12%	34%						
Vol Thru, %	22%	0%	0%	100%	0%	74%	29%						
Vol Right, %	0%	100%	0%	0%	100%	14%	36%						
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop						
Traffic Vol by Lane	266	86	76	286	184	463	180						
LT Vol	207	0	76	0	0	54	62						
Through Vol	59	0	0	286	0	344	53						
RT Vol	0	86	0	0	184	65	65						
Lane Flow Rate	328	106	90	340	219	579	243						
Geometry Grp	8	8	7	7	7	8	8						
Degree of Util (X)	0.96	0.278	0.248	0.886	0.528	1.775	0.738						
Departure Headway (Hd)	12.53	11.369	11.631	11.102	10.361	11.038	13.099						
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes	Yes						
Cap	292	319	311	330	350	336	277						
Service Time	10.23	9.069	9.331	8.802	8.061	8.738	10.799						
HCM Lane V/C Ratio	1.123	0.332	0.289	1.03	0.626	1.723	0.877						
HCM Control Delay	80.4	18.4	18.1	59.5	24.1	386.2	45.3						
HCM Lane LOS	F	C	C	F	C	F	E						
HCM 95th-tile Q	9.5	1.1	1	8.3	2.9	37.4	5.3						

Intersection						
Int Delay, s/veh						
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	
Traffic Vol, veh/h	67	306	330	38	21	45
Future Vol, veh/h	67	306	330	38	21	45
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	85	74	74	76	76
Heavy Vehicles, %	20	20	28	28	4	4
Mvmt Flow	79	360	446	51	28	59
Major/Minor						
Major1		Major2		Minor2		
Conflicting Flow All	497	0	-	0	990	472
Stage 1	-	-	-	-	472	-
Stage 2	-	-	-	-	518	-
Critical Hdwy	4.3	-	-	-	6.44	6.24
Critical Hdwy Stg 1	-	-	-	-	5.44	-
Critical Hdwy Stg 2	-	-	-	-	5.44	-
Follow-up Hdwy	2.38	-	-	-	3.536	3.336
Pot Cap-1 Maneuver	980	-	-	-	271	588
Stage 1	-	-	-	-	623	-
Stage 2	-	-	-	-	594	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	980	-	-	-	244	588
Mov Cap-2 Maneuver	-	-	-	-	244	-
Stage 1	-	-	-	-	560	-
Stage 2	-	-	-	-	594	-
Approach						
EB		WB		SB		
HCM Control Delay, s	1.6		0		16.3	
HCM LOS					C	
Minor Lane/Major Mvmt						
EBL		EBT		WBT		SBLn1
Capacity (veh/h)	980	-	-	-	-	406
HCM Lane V/C Ratio	0.08	-	-	-	-	0.214
HCM Control Delay (s)	9	0	-	-	-	16.3
HCM Lane LOS	A	A	-	-	-	C
HCM 95th %tile Q(veh)	0.3	-	-	-	-	0.8

Trans Truck System Truck Facility TIA
6: Roth Rd & Airport Way

Existing + AP plus Project
PM PEAK HOUR

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	128	171	104	319	356	93
Future Volume (veh/h)	128	171	104	319	356	93
Initial Q (Q _b), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1633	1633	1811	1811	1737	1737
Adj Flow Rate, veh/h	160	214	132	404	414	108
Peak Hour Factor	0.80	0.80	0.79	0.79	0.86	0.86
Percent Heavy Veh, %	18	18	6	6	11	11
Cap, veh/h	320	284	166	1089	1060	274
Arrive On Green	0.21	0.21	0.10	0.60	0.41	0.41
Sat Flow, veh/h	1555	1384	1725	1811	2683	671
Grp Volume(v), veh/h	160	214	132	404	262	260
Grp Sat Flow(s),veh/h/ln	1555	1384	1725	1811	1650	1616
Q Serve(g_s), s	4.2	6.8	3.5	5.3	5.2	5.3
Cycle Q Clear(g_c), s	4.2	6.8	3.5	5.3	5.2	5.3
Prop In Lane	1.00	1.00	1.00			0.41
Lane Grp Cap(c), veh/h	320	284	166	1089	674	660
V/C Ratio(X)	0.50	0.75	0.79	0.37	0.39	0.39
Avail Cap(c_a), veh/h	601	535	204	1089	674	660
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	16.4	17.4	20.6	4.8	9.7	9.7
Incr Delay (d2), s/veh	1.2	4.0	15.9	1.0	1.7	1.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.4	5.3	2.0	1.5	1.8	1.8
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	17.6	21.4	36.5	5.7	11.4	11.5
LnGrp LOS	B	C	D	A	B	B
Approach Vol, veh/h	374			536	522	
Approach Delay, s/veh	19.8			13.3	11.4	
Approach LOS	B			B	B	
Timer - Assigned Phs	2			4	5	6
Phs Duration (G+Y+Rc), s	32.5			14.1	9.0	23.5
Change Period (Y+Rc), s	4.5			4.5	4.5	4.5
Max Green Setting (Gmax), s	28.0			18.0	5.5	18.0
Max Q Clear Time (g_c+l1), s	7.3			8.8	5.5	7.3
Green Ext Time (p_c), s	2.5			0.9	0.0	2.4
Intersection Summary						
HCM 6th Ctrl Delay				14.3		
HCM 6th LOS				B		

Intersection						
Int Delay, s/veh	0.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	
Traffic Vol, veh/h	8	316	356	4	5	12
Future Vol, veh/h	8	316	356	4	5	12
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	9	343	387	4	5	13
Major/Minor						
Major1		Major2		Minor2		
Conflicting Flow All	391	0	-	0	750	389
Stage 1	-	-	-	-	389	-
Stage 2	-	-	-	-	361	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1168	-	-	-	379	659
Stage 1	-	-	-	-	685	-
Stage 2	-	-	-	-	705	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1168	-	-	-	375	659
Mov Cap-2 Maneuver	-	-	-	-	375	-
Stage 1	-	-	-	-	678	-
Stage 2	-	-	-	-	705	-
Approach						
EB		WB		SB		
HCM Control Delay, s	0.2		0		11.9	
HCM LOS					B	
Minor Lane/Major Mvmt						
EBL		EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1168	-	-	-	539	
HCM Lane V/C Ratio	0.007	-	-	-	0.034	
HCM Control Delay (s)	8.1	0	-	-	11.9	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.1	

Appendix E

*Analysis Worksheets for
Cumulative (2040) Conditions*

Trans Truck System Truck Facility TIA
1: Roth Rd & Manthey Rd

Cumulative
AM PEAK HOUR

Intersection										
Int Delay, s/veh	9.3									
Movement	WBL	WBR	NBT	NBR	SBL	SBT				
Lane Configurations	↑	↑	↑		↑					
Traffic Vol, veh/h	260	70	100	160	90	100				
Future Vol, veh/h	260	70	100	160	90	100				
Conflicting Peds, #/hr	0	0	0	0	0	0				
Sign Control	Stop	Stop	Free	Free	Free	Free				
RT Channelized	-	None	-	None	-	None				
Storage Length	0	0	-	-	-	-				
Veh in Median Storage, #	0	-	0	-	-	0				
Grade, %	0	-	0	-	-	0				
Peak Hour Factor	92	92	92	92	92	92				
Heavy Vehicles, %	6	6	6	6	6	6				
Mvmt Flow	283	76	109	174	98	109				
Major/Minor										
Minor1		Major1		Major2						
Conflicting Flow All	501	196	0	0	283	0				
Stage 1	196	-	-	-	-	-				
Stage 2	305	-	-	-	-	-				
Critical Hdwy	6.46	6.26	-	-	4.16	-				
Critical Hdwy Stg 1	5.46	-	-	-	-	-				
Critical Hdwy Stg 2	5.46	-	-	-	-	-				
Follow-up Hdwy	3.554	3.354	-	-	2.254	-				
Pot Cap-1 Maneuver	523	835	-	-	1257	-				
Stage 1	828	-	-	-	-	-				
Stage 2	739	-	-	-	-	-				
Platoon blocked, %			-	-	-	-				
Mov Cap-1 Maneuver	480	835	-	-	1257	-				
Mov Cap-2 Maneuver	480	-	-	-	-	-				
Stage 1	759	-	-	-	-	-				
Stage 2	739	-	-	-	-	-				
Approach										
WB		NB		SB						
HCM Control Delay, s	19.9		0		3.8					
HCM LOS	C									
Minor Lane/Major Mvmt										
		NBT	NBR	WBLn1	WBLn2	SBL				
Capacity (veh/h)	-		480	835	1257	-				
HCM Lane V/C Ratio	-		0.589	0.091	0.078	-				
HCM Control Delay (s)	-		22.7	9.7	8.1	0				
HCM Lane LOS	-		C	A	A	A				
HCM 95th %tile Q(veh)	-		3.7	0.3	0.3	-				

Trans Truck System Truck Facility TIA
2: Roth Rd & I-5 SB Ramp

Cumulative
AM PEAK HOUR

Intersection												
Intersection Delay, s/veh		16.2										
Intersection LOS		C										
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	150	80	220	180	0	0	0	0	300	10	150
Future Vol, veh/h	0	150	80	220	180	0	0	0	0	300	10	150
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	12	12	12	40	40	40	2	2	2	20	20	20
Mvmt Flow	0	163	87	239	196	0	0	0	0	326	11	163
Number of Lanes	0	1	1	1	2	0	0	0	0	1	1	0
Approach	EB		WB			SB						
Opposing Approach	WB		EB									
Opposing Lanes	3		2			0						
Conflicting Approach Left	SB					WB						
Conflicting Lanes Left	2		0			3						
Conflicting Approach Right			SB			EB						
Conflicting Lanes Right	0		2			2						
HCM Control Delay	13.5		16.2			17.5						
HCM LOS	B		C			C						
Lane	EBLn1	EBLn2	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2					
Vol Left, %	0%	0%	100%	0%	0%	100%	27%					
Vol Thru, %	100%	0%	0%	100%	100%	0%	5%					
Vol Right, %	0%	100%	0%	0%	0%	0%	68%					
Sign Control	Stop											
Traffic Vol by Lane	150	80	220	90	90	240	220					
LT Vol	0	0	220	0	0	240	60					
Through Vol	150	0	0	90	90	0	10					
RT Vol	0	80	0	0	0	0	150					
Lane Flow Rate	163	87	239	98	98	261	239					
Geometry Grp	8	8	8	8	8	8	8					
Degree of Util (X)	0.352	0.171	0.551	0.211	0.143	0.555	0.453					
Departure Headway (Hd)	7.78	7.061	8.288	7.777	5.249	7.772	6.924					
Convergence, Y/N	Yes											
Cap	465	510	438	465	675	467	524					
Service Time	5.492	4.772	5.988	5.477	3.047	5.472	4.624					
HCM Lane V/C Ratio	0.351	0.171	0.546	0.211	0.145	0.559	0.456					
HCM Control Delay	14.7	11.2	20.7	12.5	8.9	19.7	15.2					
HCM Lane LOS	B	B	C	B	A	C	C					
HCM 95th-tile Q	1.6	0.6	3.2	0.8	0.5	3.3	2.3					

Intersection												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑			↑	↑	↑	↑	↑	0	0	0
Traffic Vol, veh/h	150	330	0	0	290	370	100	30	250	0	0	0
Future Vol, veh/h	150	330	0	0	290	370	100	30	250	0	0	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	20	20	20	34	34	34	49	49	49	2	2	2
Mvmt Flow	163	359	0	0	315	402	109	33	272	0	0	0
Number of Lanes	1	2	0	0	1	1	0	1	1	0	0	0
Approach												
Opposing Approach	WB		WB		NB							
Opposing Lanes	2		3		0							
Conflicting Approach Left			NB		EB							
Conflicting Lanes Left	0		2		3							
Conflicting Approach Right	NB				WB							
Conflicting Lanes Right	2		0		2							
HCM Control Delay	16.1		37.9		23.5							
HCM LOS	C		E		C							
Lane												
	NBLn1	NBLn2	EBLn1	EBLn2	EBLn3	WBLn1	WBLn2					
Vol Left, %	77%	0%	100%	0%	0%	0%	0%					
Vol Thru, %	23%	0%	0%	100%	100%	100%	0%					
Vol Right, %	0%	100%	0%	0%	0%	0%	100%					
Sign Control	Stop											
Traffic Vol by Lane	130	250	150	165	165	290	370					
LT Vol	100	0	150	0	0	0	0					
Through Vol	30	0	0	165	165	290	0					
RT Vol	0	250	0	0	0	0	370					
Lane Flow Rate	141	272	163	179	179	315	402					
Geometry Grp	8	8	8	8	8	8	8					
Degree of Util (X)	0.382	0.65	0.414	0.429	0.325	0.74	0.863					
Departure Headway (Hd)	9.723	8.616	9.136	8.621	6.522	8.448	7.727					
Convergence, Y/N	Yes											
Cap	370	418	393	418	549	429	467					
Service Time	7.495	6.387	6.901	6.386	4.286	6.213	5.491					
HCM Lane V/C Ratio	0.381	0.651	0.415	0.428	0.326	0.734	0.861					
HCM Control Delay	18.4	26.1	18.2	17.8	12.4	31.8	42.7					
HCM Lane LOS	C	D	C	C	B	D	E					
HCM 95th-tile Q	1.7	4.5	2	2.1	1.4	6	8.9					

Trans Truck System Truck Facility TIA
4: Harlan Rd & Roth Rd

Cumulative
AM PEAK HOUR

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (veh/h)	100	295	180	40	270	42	320	50	51	40	30	80
Future Volume (veh/h)	100	295	180	40	270	42	320	50	51	40	30	80
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1426	1426	1426	1218	1218	1218	1707	1707	1707	1115	1115	1115
Adj Flow Rate, veh/h	109	321	196	43	293	46	348	54	55	43	33	87
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	32	32	32	46	46	46	13	13	13	53	53	53
Cap, veh/h	129	505	428	45	306	48	384	266	271	43	40	105
Arrive On Green	0.09	0.35	0.35	0.04	0.30	0.30	0.24	0.34	0.34	0.04	0.15	0.15
Sat Flow, veh/h	1358	1426	1208	1160	1028	161	1626	775	790	1061	271	715
Grp Volume(v), veh/h	109	321	196	43	0	339	348	0	109	43	0	120
Grp Sat Flow(s),veh/h/ln	1358	1426	1208	1160	0	1189	1626	0	1565	1061	0	986
Q Serve(g_s), s	6.3	15.1	10.0	3.0	0.0	22.5	16.7	0.0	3.9	3.2	0.0	9.5
Cycle Q Clear(g_c), s	6.3	15.1	10.0	3.0	0.0	22.5	16.7	0.0	3.9	3.2	0.0	9.5
Prop In Lane	1.00		1.00	1.00		0.14	1.00		0.50	1.00		0.73
Lane Grp Cap(c), veh/h	129	505	428	45	0	354	384	0	537	43	0	145
V/C Ratio(X)	0.85	0.64	0.46	0.96	0.00	0.96	0.91	0.00	0.20	1.00	0.00	0.83
Avail Cap(c_a), veh/h	140	505	428	85	0	354	423	0	597	122	0	232
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	35.8	21.6	20.0	38.5	0.0	27.7	29.8	0.0	18.6	38.5	0.0	33.2
Incr Delay (d2), s/veh	33.7	2.6	0.8	55.1	0.0	36.7	21.6	0.0	0.2	64.0	0.0	12.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.3	5.1	2.8	1.5	0.0	9.7	8.6	0.0	1.4	1.6	0.0	2.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	69.5	24.2	20.8	93.7	0.0	64.4	51.4	0.0	18.8	102.5	0.0	45.7
LnGrp LOS	E	C	C	F	A	E	D	A	B	F	A	D
Approach Vol, veh/h	626				382			457			163	
Approach Delay, s/veh	31.0				67.7			43.6			60.7	
Approach LOS	C				E			D			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	7.7	32.0	7.6	32.9	23.5	16.3	12.1	28.4				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	9.2	30.6	5.9	26.3	20.9	18.9	8.3	23.9				
Max Q Clear Time (g_c+l1), s	5.2	5.9	5.0	17.1	18.7	11.5	8.3	24.5				
Green Ext Time (p_c), s	0.0	0.6	0.0	1.9	0.3	0.3	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay				46.1								
HCM 6th LOS				D								

Intersection						
Int Delay, s/veh						
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	
Traffic Vol, veh/h	40	288	204	30	60	60
Future Vol, veh/h	40	288	204	30	60	60
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	34	34	39	39	6	6
Mvmt Flow	43	313	222	33	65	65
Major/Minor						
Major1		Major2		Minor2		
Conflicting Flow All	255	0	-	0	638	239
Stage 1	-	-	-	-	239	-
Stage 2	-	-	-	-	399	-
Critical Hdwy	4.44	-	-	-	6.46	6.26
Critical Hdwy Stg 1	-	-	-	-	5.46	-
Critical Hdwy Stg 2	-	-	-	-	5.46	-
Follow-up Hdwy	2.506	-	-	-	3.554	3.354
Pot Cap-1 Maneuver	1145	-	-	-	435	790
Stage 1	-	-	-	-	791	-
Stage 2	-	-	-	-	669	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1145	-	-	-	415	790
Mov Cap-2 Maneuver	-	-	-	-	415	-
Stage 1	-	-	-	-	755	-
Stage 2	-	-	-	-	669	-
Approach						
EB		WB		SB		
HCM Control Delay, s	1		0		13.7	
HCM LOS					B	
Minor Lane/Major Mvmt						
EBL		EBT		WBT		SBLn1
Capacity (veh/h)	1145	-	-	-	-	544
HCM Lane V/C Ratio	0.038	-	-	-	-	0.24
HCM Control Delay (s)	8.3	0	-	-	-	13.7
HCM Lane LOS	A	A	-	-	-	B
HCM 95th %tile Q(veh)	0.1	-	-	-	-	0.9

Trans Truck System Truck Facility TIA
6: Roth Rd & Airport Way

Cumulative
AM PEAK HOUR

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	60	120	210	300	160	80
Future Volume (veh/h)	60	120	210	300	160	80
Initial Q (Q _b), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1470	1470	1796	1796	1589	1589
Adj Flow Rate, veh/h	65	130	228	326	174	87
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	29	29	7	7	21	21
Cap, veh/h	198	177	218	1167	827	395
Arrive On Green	0.14	0.14	0.13	0.65	0.42	0.42
Sat Flow, veh/h	1400	1246	1711	1796	2060	947
Grp Volume(v), veh/h	65	130	228	326	131	130
Grp Sat Flow(s),veh/h/ln	1400	1246	1711	1796	1509	1418
Q Serve(g_s), s	1.8	4.3	5.5	3.3	2.4	2.5
Cycle Q Clear(g_c), s	1.8	4.3	5.5	3.3	2.4	2.5
Prop In Lane	1.00	1.00	1.00			0.67
Lane Grp Cap(c), veh/h	198	177	218	1167	630	592
V/C Ratio(X)	0.33	0.74	1.04	0.28	0.21	0.22
Avail Cap(c_a), veh/h	585	520	218	1167	630	592
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	16.7	17.7	18.8	3.2	8.0	8.1
Incr Delay (d2), s/veh	1.0	5.9	73.1	0.6	0.7	0.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	0.3	6.3	0.7	0.7	0.7
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	17.6	23.6	91.9	3.8	8.8	8.9
LnGrp LOS	B	C	F	A	A	A
Approach Vol, veh/h	195			554	261	
Approach Delay, s/veh	21.6			40.1	8.8	
Approach LOS		C		D	A	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s	32.5		10.6	10.0	22.5	
Change Period (Y+Rc), s	4.5		4.5	4.5	4.5	
Max Green Setting (Gmax), s	28.0		18.0	5.5	18.0	
Max Q Clear Time (g_c+l1), s	5.3		6.3	7.5	4.5	
Green Ext Time (p_c), s	1.9		0.5	0.0	1.2	
Intersection Summary						
HCM 6th Ctrl Delay			28.4			
HCM 6th LOS			C			

Trans Truck System Truck Facility TIA
1: Roth Rd & Manthey Rd

Cumulative
PM PEAK HOUR

Intersection						
Int Delay, s/veh	4.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑		↑	↑
Traffic Vol, veh/h	100	86	150	90	90	130
Future Vol, veh/h	100	86	150	90	90	130
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	109	93	163	98	98	141
Major/Minor						
Conflicting Flow All	549	212	0	0	261	0
Stage 1	212	-	-	-	-	-
Stage 2	337	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	497	828	-	-	1303	-
Stage 1	823	-	-	-	-	-
Stage 2	723	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	456	828	-	-	1303	-
Mov Cap-2 Maneuver	456	-	-	-	-	-
Stage 1	756	-	-	-	-	-
Stage 2	723	-	-	-	-	-
Approach						
	WB	NB	SB			
HCM Control Delay, s	12.8	0	3.3			
HCM LOS	B					
Minor Lane/Major Mvmt						
	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	456	828	1303	-
HCM Lane V/C Ratio	-	-	0.238	0.113	0.075	-
HCM Control Delay (s)	-	-	15.3	9.9	8	0
HCM Lane LOS	-	-	C	A	A	A
HCM 95th %tile Q(veh)	-	-	0.9	0.4	0.2	-

Trans Truck System Truck Facility TIA
2: Roth Rd & I-5 SB Ramp

Cumulative
PM PEAK HOUR

Intersection												
Intersection Delay, s/veh	14.1											
Intersection LOS	B											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	130	60	220	100	0	0	0	0	300	20	80
Future Vol, veh/h	0	130	60	220	100	0	0	0	0	300	20	80
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	4	4	4	23	23	23	2	2	2	15	15	15
Mvmt Flow	0	141	65	239	109	0	0	0	0	326	22	87
Number of Lanes	0	1	1	1	2	0	0	0	0	1	1	0
Approach	EB		WB		SB							
Opposing Approach	WB		EB									
Opposing Lanes	3		2		0							
Conflicting Approach Left	SB				WB							
Conflicting Lanes Left	2		0		3							
Conflicting Approach Right			SB		EB							
Conflicting Lanes Right	0		2		2							
HCM Control Delay	11.7		14.9		14.5							
HCM LOS	B		B		B							
Lane	EBLn1	EBLn2	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2					
Vol Left, %	0%	0%	100%	0%	0%	100%	49%					
Vol Thru, %	100%	0%	0%	100%	100%	0%	10%					
Vol Right, %	0%	100%	0%	0%	0%	0%	41%					
Sign Control	Stop											
Traffic Vol by Lane	130	60	220	50	50	204	196					
LT Vol	0	0	220	0	0	204	96					
Through Vol	130	0	0	50	50	0	20					
RT Vol	0	60	0	0	0	0	80					
Lane Flow Rate	141	65	239	54	54	222	213					
Geometry Grp	8	8	8	8	8	8	8					
Degree of Util (X)	0.276	0.115	0.497	0.105	0.073	0.439	0.39					
Departure Headway (Hd)	7.038	6.322	7.475	6.967	4.842	7.129	6.586					
Convergence, Y/N	Yes											
Cap	509	564	481	513	735	503	545					
Service Time	4.808	4.091	5.237	4.728	2.602	4.885	4.342					
HCM Lane V/C Ratio	0.277	0.115	0.497	0.105	0.073	0.441	0.391					
HCM Control Delay	12.5	9.9	17.5	10.6	8	15.4	13.5					
HCM Lane LOS	B	A	C	B	A	C	B					
HCM 95th-tile Q	1.1	0.4	2.7	0.3	0.2	2.2	1.8					

Intersection													
Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑				↑	↑	↑	↑	↑			
Traffic Vol, veh/h	80	370	0	1	0	250	375	60	20	194	0	0	0
Future Vol, veh/h	80	370	0	1	0	250	375	60	20	194	0	0	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	14	14	14	20	20	20	20	29	29	29	2	2	2
Mvmt Flow	87	402	0	1	0	272	408	65	22	211	0	0	0
Number of Lanes	1	2	0	0	0	1	1	0	1	1	0	0	0
Approach													
Opposing Approach	WB		EB		NB								
Opposing Lanes	2		3		0								
Conflicting Approach Left			NB		EB								
Conflicting Lanes Left	0		2		3								
Conflicting Approach Right	NB				WB								
Conflicting Lanes Right	2		0		2								
HCM Control Delay	13.5		24		15.9								
HCM LOS	B		C		C								
Lane													
	NBLn1	NBLn2	EBLn1	EBLn2	EBLn3	WBLn1	WBLn2						
Vol Left, %	75%	0%	100%	0%	0%	0%	0%						
Vol Thru, %	25%	0%	0%	100%	100%	100%	100%						
Vol Right, %	0%	100%	0%	0%	0%	0%	100%						
Sign Control	Stop												
Traffic Vol by Lane	80	194	80	185	185	251	375						
LT Vol	60	0	80	0	0	0	0						
Through Vol	20	0	0	185	185	251	0						
RT Vol	0	194	0	0	0	0	375						
Lane Flow Rate	87	211	87	201	201	273	408						
Geometry Grp	8	8	8	8	8	8	8						
Degree of Util (X)	0.213	0.452	0.198	0.43	0.319	0.559	0.755						
Departure Headway (Hd)	8.814	7.721	8.207	7.696	5.715	7.382	6.668						
Convergence, Y/N	Yes												
Cap	408	466	438	469	629	488	542						
Service Time	6.56	5.467	5.946	5.435	3.454	5.12	4.405						
HCM Lane V/C Ratio	0.213	0.453	0.199	0.429	0.32	0.559	0.753						
HCM Control Delay	13.9	16.7	13	16.1	11.1	19.1	27.2						
HCM Lane LOS	B	C	B	C	B	C	D						
HCM 95th-tile Q	0.8	2.3	0.7	2.1	1.4	3.4	6.6						

Trans Truck System Truck Facility TIA
4: Harlan Rd & Roth Rd

Cumulative
PM PEAK HOUR

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (veh/h)	76	279	260	60	335	62	250	70	86	60	60	70
Future Volume (veh/h)	76	279	260	60	335	62	250	70	86	60	60	70
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No		No		No	
Adj Sat Flow, veh/h/ln	1604	1604	1604	1441	1441	1441	1737	1737	1737	1559	1559	1559
Adj Flow Rate, veh/h	83	303	283	65	364	67	272	76	93	65	65	76
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	20	20	20	31	31	31	11	11	11	23	23	23
Cap, veh/h	100	570	483	74	407	75	317	194	237	77	87	102
Arrive On Green	0.07	0.36	0.36	0.05	0.34	0.34	0.19	0.27	0.27	0.05	0.13	0.13
Sat Flow, veh/h	1527	1604	1359	1372	1184	218	1654	711	870	1485	655	766
Grp Volume(v), veh/h	83	303	283	65	0	431	272	0	169	65	0	141
Grp Sat Flow(s),veh/h/ln	1527	1604	1359	1372	0	1401	1654	0	1580	1485	0	1421
Q Serve(g_s), s	3.6	10.2	11.5	3.2	0.0	19.8	10.8	0.0	5.9	2.9	0.0	6.5
Cycle Q Clear(g_c), s	3.6	10.2	11.5	3.2	0.0	19.8	10.8	0.0	5.9	2.9	0.0	6.5
Prop In Lane	1.00		1.00	1.00		0.16	1.00		0.55	1.00		0.54
Lane Grp Cap(c), veh/h	100	570	483	74	0	482	317	0	432	77	0	190
V/C Ratio(X)	0.83	0.53	0.59	0.87	0.00	0.89	0.86	0.00	0.39	0.84	0.00	0.74
Avail Cap(c_a), veh/h	128	631	535	152	0	589	403	0	641	226	0	446
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	31.3	17.4	17.8	31.8	0.0	21.1	26.5	0.0	20.1	31.9	0.0	28.3
Incr Delay (d2), s/veh	28.0	0.8	1.4	25.0	0.0	14.2	13.8	0.0	0.6	20.6	0.0	5.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.1	3.6	3.5	1.5	0.0	7.8	5.2	0.0	2.1	1.5	0.0	2.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	59.3	18.2	19.2	56.8	0.0	35.3	40.3	0.0	20.6	52.5	0.0	33.9
LnGrp LOS	E	B	B	E	A	D	D	A	C	D	A	C
Approach Vol, veh/h	669			496			441			206		
Approach Delay, s/veh	23.7			38.1			32.8			39.8		
Approach LOS	C			D			C			D		
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	8.0	23.0	8.2	28.6	17.5	13.5	9.0	27.8				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	10.3	27.5	7.5	26.7	16.5	21.3	5.7	28.5				
Max Q Clear Time (g_c+l1), s	4.9	7.9	5.2	13.5	12.8	8.5	5.6	21.8				
Green Ext Time (p_c), s	0.0	0.9	0.0	2.4	0.3	0.6	0.0	1.5				
Intersection Summary												
HCM 6th Ctrl Delay				31.7								
HCM 6th LOS				C								

Intersection										
Int Delay, s/veh	3.2									
Movement	EBL	EBT	WBT	WBR	SBL	SBR				
Lane Configurations		↑	↑		↑					
Traffic Vol, veh/h	80	298	318	70	60	50				
Future Vol, veh/h	80	298	318	70	60	50				
Conflicting Peds, #/hr	0	0	0	0	0	0				
Sign Control	Free	Free	Free	Free	Stop	Stop				
RT Channelized	-	None	-	None	-	None				
Storage Length	-	-	-	-	0	-				
Veh in Median Storage, #	-	0	0	-	0	-				
Grade, %	-	0	0	-	0	-				
Peak Hour Factor	92	92	92	92	92	92				
Heavy Vehicles, %	20	20	28	28	4	4				
Mvmt Flow	87	324	346	76	65	54				
Major/Minor										
Major1		Major2		Minor2						
Conflicting Flow All	422	0	-	0	882	384				
Stage 1	-	-	-	-	384	-				
Stage 2	-	-	-	-	498	-				
Critical Hdwy	4.3	-	-	-	6.44	6.24				
Critical Hdwy Stg 1	-	-	-	-	5.44	-				
Critical Hdwy Stg 2	-	-	-	-	5.44	-				
Follow-up Hdwy	2.38	-	-	-	3.536	3.336				
Pot Cap-1 Maneuver	1047	-	-	-	314	659				
Stage 1	-	-	-	-	684	-				
Stage 2	-	-	-	-	607	-				
Platoon blocked, %	-	-	-	-	-	-				
Mov Cap-1 Maneuver	1047	-	-	-	282	659				
Mov Cap-2 Maneuver	-	-	-	-	282	-				
Stage 1	-	-	-	-	615	-				
Stage 2	-	-	-	-	607	-				
Approach										
EB		WB		SB						
HCM Control Delay, s	1.9		0		18.7					
HCM LOS	C									
Minor Lane/Major Mvmt										
EBL		EBT	WBT	WBR	SBLn1					
Capacity (veh/h)	1047		-	-	-	381				
HCM Lane V/C Ratio	0.083		-	-	-	0.314				
HCM Control Delay (s)	8.8		0	-	-	18.7				
HCM Lane LOS	A		A	-	-	C				
HCM 95th %tile Q(veh)	0.3		-	-	-	1.3				

Trans Truck System Truck Facility TIA
6: Roth Rd & Airport Way

Cumulative
PM PEAK HOUR

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	125	220	140	319	356	91
Future Volume (veh/h)	125	220	140	319	356	91
Initial Q (Q _b), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1633	1633	1811	1811	1737	1737
Adj Flow Rate, veh/h	136	239	152	347	387	99
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	18	18	6	6	11	11
Cap, veh/h	343	306	191	1068	1002	254
Arrive On Green	0.22	0.22	0.11	0.59	0.38	0.38
Sat Flow, veh/h	1555	1384	1725	1811	2695	660
Grp Volume(v), veh/h	136	239	152	347	243	243
Grp Sat Flow(s),veh/h/ln	1555	1384	1725	1811	1650	1618
Q Serve(g_s), s	3.5	7.7	4.1	4.6	5.1	5.2
Cycle Q Clear(g_c), s	3.5	7.7	4.1	4.6	5.1	5.2
Prop In Lane	1.00	1.00	1.00			0.41
Lane Grp Cap(c), veh/h	343	306	191	1068	634	622
V/C Ratio(X)	0.40	0.78	0.80	0.32	0.38	0.39
Avail Cap(c_a), veh/h	590	525	200	1068	634	622
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	15.8	17.4	20.6	4.9	10.6	10.6
Incr Delay (d2), s/veh	0.7	4.4	19.0	0.8	1.8	1.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.2	6.0	2.5	1.3	1.8	1.8
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	16.5	21.8	39.5	5.8	12.3	12.4
LnGrp LOS	B	C	D	A	B	B
Approach Vol, veh/h	375			499	486	
Approach Delay, s/veh	19.9			16.0	12.4	
Approach LOS	B			B	B	
Timer - Assigned Phs	2			4	5	6
Phs Duration (G+Y+Rc), s	32.5			15.0	9.8	22.7
Change Period (Y+Rc), s	4.5			4.5	4.5	4.5
Max Green Setting (Gmax), s	28.0			18.0	5.5	18.0
Max Q Clear Time (g_c+l1), s	6.6			9.7	6.1	7.2
Green Ext Time (p_c), s	2.1			0.8	0.0	2.2
Intersection Summary						
HCM 6th Ctrl Delay				15.8		
HCM 6th LOS				B		

Appendix F

*Analysis Worksheets for
Cumulative (2040) plus Proposed Project Conditions*

Intersection										
Int Delay, s/veh	9.3									
Movement	WBL	WBR	NBT	NBR	SBL	SBT				
Lane Configurations	↑	↑	↑		↑					
Traffic Vol, veh/h	260	70	100	160	90	100				
Future Vol, veh/h	260	70	100	160	90	100				
Conflicting Peds, #/hr	0	0	0	0	0	0				
Sign Control	Stop	Stop	Free	Free	Free	Free				
RT Channelized	-	None	-	None	-	None				
Storage Length	0	0	-	-	-	-				
Veh in Median Storage, #	0	-	0	-	-	0				
Grade, %	0	-	0	-	-	0				
Peak Hour Factor	92	92	92	92	92	92				
Heavy Vehicles, %	6	6	6	6	6	6				
Mvmt Flow	283	76	109	174	98	109				
Major/Minor										
Minor1		Major1		Major2						
Conflicting Flow All	501	196	0	0	283	0				
Stage 1	196	-	-	-	-	-				
Stage 2	305	-	-	-	-	-				
Critical Hdwy	6.46	6.26	-	-	4.16	-				
Critical Hdwy Stg 1	5.46	-	-	-	-	-				
Critical Hdwy Stg 2	5.46	-	-	-	-	-				
Follow-up Hdwy	3.554	3.354	-	-	2.254	-				
Pot Cap-1 Maneuver	523	835	-	-	1257	-				
Stage 1	828	-	-	-	-	-				
Stage 2	739	-	-	-	-	-				
Platoon blocked, %	-	-	-	-	-	-				
Mov Cap-1 Maneuver	480	835	-	-	1257	-				
Mov Cap-2 Maneuver	480	-	-	-	-	-				
Stage 1	759	-	-	-	-	-				
Stage 2	739	-	-	-	-	-				
Approach										
WB		NB		SB						
HCM Control Delay, s	19.9		0		3.8					
HCM LOS	C									
Minor Lane/Major Mvmt										
NBT		NBR	WBLn1	WBLn2	SBL	SBT				
Capacity (veh/h)	-	-	480	835	1257	-				
HCM Lane V/C Ratio	-	-	0.589	0.091	0.078	-				
HCM Control Delay (s)	-	-	22.7	9.7	8.1	0				
HCM Lane LOS	-	-	C	A	A	A				
HCM 95th %tile Q(veh)	-	-	3.7	0.3	0.3	-				

Intersection												
Intersection Delay, s/veh	16.5											
Intersection LOS	C											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	150	80	225	180	0	0	0	0	304	10	150
Future Vol, veh/h	0	150	80	225	180	0	0	0	0	304	10	150
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	12	12	12	40	40	40	2	2	2	20	20	20
Mvmt Flow	0	163	87	245	196	0	0	0	0	330	11	163
Number of Lanes	0	1	1	1	2	0	0	0	0	1	1	0
Approach	EB			WB			SB					
Opposing Approach	WB			EB								
Opposing Lanes	3			2			0					
Conflicting Approach Left	SB						WB					
Conflicting Lanes Left	2			0			3					
Conflicting Approach Right				SB			EB					
Conflicting Lanes Right	0			2			2					
HCM Control Delay	13.6			16.6			17.8					
HCM LOS	B			C			C					
Lane	EBLn1	EBLn2	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2					
Vol Left, %	0%	0%	100%	0%	0%	100%	29%					
Vol Thru, %	100%	0%	0%	100%	100%	0%	4%					
Vol Right, %	0%	100%	0%	0%	0%	0%	67%					
Sign Control	Stop											
Traffic Vol by Lane	150	80	225	90	90	240	224					
LT Vol	0	0	225	0	0	240	64					
Through Vol	150	0	0	90	90	0	10					
RT Vol	0	80	0	0	0	0	150					
Lane Flow Rate	163	87	245	98	98	261	243					
Geometry Grp	8	8	8	8	8	8	8					
Degree of Util (X)	0.354	0.171	0.564	0.212	0.143	0.557	0.463					
Departure Headway (Hd)	7.814	7.094	8.307	7.797	5.268	7.796	6.962					
Convergence, Y/N	Yes											
Cap	463	508	438	463	672	465	521					
Service Time	5.527	4.807	6.007	5.497	3.066	5.496	4.662					
HCM Lane V/C Ratio	0.352	0.171	0.559	0.212	0.146	0.561	0.466					
HCM Control Delay	14.8	11.3	21.2	12.6	9	19.9	15.5					
HCM Lane LOS	B	B	C	B	A	C	C					
HCM 95th-tile Q	1.6	0.6	3.4	0.8	0.5	3.3	2.4					

Intersection												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑			↑	↑	↑	↑	↑	0	0	0
Traffic Vol, veh/h	150	334	0	0	295	376	100	30	254	0	0	0
Future Vol, veh/h	150	334	0	0	295	376	100	30	254	0	0	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	20	20	20	34	34	34	49	49	49	2	2	2
Mvmt Flow	163	363	0	0	321	409	109	33	276	0	0	0
Number of Lanes	1	2	0	0	1	1	0	1	1	0	0	0
Approach												
Opposing Approach	WB		EB		NB							
Opposing Lanes	2		3		0							
Conflicting Approach Left			NB		EB							
Conflicting Lanes Left	0		2		3							
Conflicting Approach Right	NB				WB							
Conflicting Lanes Right	2		0		2							
HCM Control Delay	16.3		40.3		24.3							
HCM LOS	C		E		C							
Lane												
	NBLn1	NBLn2	EBLn1	EBLn2	EBLn3	WBLn1	WBLn2					
Vol Left, %	77%	0%	100%	0%	0%	0%	0%					
Vol Thru, %	23%	0%	0%	100%	100%	100%	100%					
Vol Right, %	0%	100%	0%	0%	0%	0%	100%					
Sign Control	Stop											
Traffic Vol by Lane	130	254	150	167	167	295	376					
LT Vol	100	0	150	0	0	0	0					
Through Vol	30	0	0	167	167	295	0					
RT Vol	0	254	0	0	0	0	376					
Lane Flow Rate	141	276	163	182	182	321	409					
Geometry Grp	8	8	8	8	8	8	8					
Degree of Util (X)	0.384	0.665	0.416	0.438	0.332	0.757	0.882					
Departure Headway (Hd)	9.78	8.672	9.192	8.677	6.577	8.494	7.772					
Convergence, Y/N	Yes											
Cap	367	417	391	414	545	426	466					
Service Time	7.555	6.447	6.96	6.445	4.344	6.261	5.539					
HCM Lane V/C Ratio	0.384	0.662	0.417	0.44	0.334	0.754	0.878					
HCM Control Delay	18.6	27.2	18.4	18.1	12.6	33.4	45.8					
HCM Lane LOS	C	D	C	C	B	D	E					
HCM 95th-tile Q	1.8	4.7	2	2.2	1.4	6.3	9.4					

Trans Truck System Truck Facility TIA
4: Harlan Rd & Roth Rd

Cumulative plus Project
AM PEAK HOUR

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (veh/h)	100	303	180	40	280	45	320	50	51	42	30	80
Future Volume (veh/h)	100	303	180	40	280	45	320	50	51	42	30	80
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1426	1426	1426	1218	1218	1218	1707	1707	1707	1115	1115	1115
Adj Flow Rate, veh/h	109	329	196	43	304	49	348	54	55	46	33	87
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	32	32	32	46	46	46	13	13	13	53	53	53
Cap, veh/h	129	505	428	45	305	49	384	263	268	47	40	105
Arrive On Green	0.09	0.35	0.35	0.04	0.30	0.30	0.24	0.34	0.34	0.04	0.15	0.15
Sat Flow, veh/h	1358	1426	1208	1160	1024	165	1626	775	790	1061	271	715
Grp Volume(v), veh/h	109	329	196	43	0	353	348	0	109	46	0	120
Grp Sat Flow(s),veh/h/ln	1358	1426	1208	1160	0	1189	1626	0	1565	1061	0	986
Q Serve(g_s), s	6.3	15.6	10.0	3.0	0.0	23.8	16.7	0.0	4.0	3.5	0.0	9.5
Cycle Q Clear(g_c), s	6.3	15.6	10.0	3.0	0.0	23.8	16.7	0.0	4.0	3.5	0.0	9.5
Prop In Lane	1.00			1.00		0.14	1.00		0.50	1.00		0.73
Lane Grp Cap(c), veh/h	129	505	428	45	0	354	384	0	531	47	0	145
V/C Ratio(X)	0.85	0.65	0.46	0.96	0.00	1.00	0.91	0.00	0.21	0.99	0.00	0.83
Avail Cap(c_a), veh/h	140	505	428	85	0	354	423	0	597	122	0	232
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	35.8	21.8	20.0	38.5	0.0	28.2	29.8	0.0	18.8	38.4	0.0	33.2
Incr Delay (d2), s/veh	33.7	3.0	0.8	55.1	0.0	47.2	21.6	0.0	0.2	57.9	0.0	12.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.3	5.3	2.8	1.5	0.0	11.0	8.6	0.0	1.4	1.6	0.0	2.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	69.5	24.7	20.8	93.7	0.0	75.3	51.4	0.0	19.0	96.2	0.0	45.7
LnGrp LOS	E	C	C	F	A	E	D	A	B	F	A	D
Approach Vol, veh/h	634				396			457			166	
Approach Delay, s/veh	31.2				77.3			43.7			59.7	
Approach LOS	C				E			D			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	8.0	31.7	7.6	32.9	23.5	16.3	12.1	28.4				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	9.2	30.6	5.9	26.3	20.9	18.9	8.3	23.9				
Max Q Clear Time (g_c+l1), s	5.5	6.0	5.0	17.6	18.7	11.5	8.3	25.8				
Green Ext Time (p_c), s	0.0	0.6	0.0	1.9	0.3	0.3	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay				48.6								
HCM 6th LOS				D								

Intersection						
Int Delay, s/veh						
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	
Traffic Vol, veh/h	40	298	217	30	60	60
Future Vol, veh/h	40	298	217	30	60	60
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	34	34	39	39	6	6
Mvmt Flow	43	324	236	33	65	65
Major/Minor						
Major1		Major2		Minor2		
Conflicting Flow All	269	0	-	0	663	253
Stage 1	-	-	-	-	253	-
Stage 2	-	-	-	-	410	-
Critical Hdwy	4.44	-	-	-	6.46	6.26
Critical Hdwy Stg 1	-	-	-	-	5.46	-
Critical Hdwy Stg 2	-	-	-	-	5.46	-
Follow-up Hdwy	2.506	-	-	-	3.554	3.354
Pot Cap-1 Maneuver	1130	-	-	-	420	776
Stage 1	-	-	-	-	780	-
Stage 2	-	-	-	-	661	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1130	-	-	-	401	776
Mov Cap-2 Maneuver	-	-	-	-	401	-
Stage 1	-	-	-	-	744	-
Stage 2	-	-	-	-	661	-
Approach						
EB		WB		SB		
HCM Control Delay, s	1		0		14	
HCM LOS					B	
Minor Lane/Major Mvmt						
EBL		EBT		WBT		SBLn1
Capacity (veh/h)	1130	-	-	-	-	529
HCM Lane V/C Ratio	0.038	-	-	-	-	0.247
HCM Control Delay (s)	8.3	0	-	-	-	14
HCM Lane LOS	A	A	-	-	-	B
HCM 95th %tile Q(veh)	0.1	-	-	-	-	1

Trans Truck System Truck Facility TIA
6: Roth Rd & Airport Way

Cumulative plus Project
AM PEAK HOUR

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	64	122	211	300	160	83
Future Volume (veh/h)	64	122	211	300	160	83
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1470	1470	1796	1796	1589	1589
Adj Flow Rate, veh/h	70	133	229	326	174	90
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	29	29	7	7	21	21
Cap, veh/h	204	181	217	1162	814	402
Arrive On Green	0.15	0.15	0.13	0.65	0.42	0.42
Sat Flow, veh/h	1400	1246	1711	1796	2037	967
Grp Volume(v), veh/h	70	133	229	326	132	132
Grp Sat Flow(s),veh/h/ln	1400	1246	1711	1796	1509	1415
Q Serve(g_s), s	1.9	4.4	5.5	3.4	2.4	2.6
Cycle Q Clear(g_c), s	1.9	4.4	5.5	3.4	2.4	2.6
Prop In Lane	1.00	1.00	1.00			0.68
Lane Grp Cap(c), veh/h	204	181	217	1162	628	588
V/C Ratio(X)	0.34	0.73	1.05	0.28	0.21	0.22
Avail Cap(c_a), veh/h	582	518	217	1162	628	588
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	16.6	17.7	18.9	3.3	8.1	8.1
Incr Delay (d2), s/veh	1.0	5.7	75.9	0.6	0.8	0.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	3.2	6.5	0.7	0.7	0.7
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	17.6	23.4	94.8	3.9	8.9	9.0
LnGrp LOS	B	C	F	A	A	A
Approach Vol, veh/h	203			555	264	
Approach Delay, s/veh	21.4			41.4	8.9	
Approach LOS	C			D	A	
Timer - Assigned Phs	2			4	5	6
Phs Duration (G+Y+Rc), s	32.5			10.8	10.0	22.5
Change Period (Y+Rc), s	4.5			4.5	4.5	4.5
Max Green Setting (Gmax), s	28.0			18.0	5.5	18.0
Max Q Clear Time (g_c+l1), s	5.4			6.4	7.5	4.6
Green Ext Time (p_c), s	1.9			0.5	0.0	1.2
Intersection Summary						
HCM 6th Ctrl Delay				29.0		
HCM 6th LOS				C		

Intersection						
Int Delay, s/veh	0.5					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	
Traffic Vol, veh/h	10	220	260	4	6	13
Future Vol, veh/h	10	220	260	4	6	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	11	239	283	4	7	14
Major/Minor						
Major1		Major2		Minor2		
Conflicting Flow All	287	0	-	0	546	285
Stage 1	-	-	-	-	285	-
Stage 2	-	-	-	-	261	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1275	-	-	-	499	754
Stage 1	-	-	-	-	763	-
Stage 2	-	-	-	-	783	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1275	-	-	-	494	754
Mov Cap-2 Maneuver	-	-	-	-	494	-
Stage 1	-	-	-	-	755	-
Stage 2	-	-	-	-	783	-
Approach						
EB		WB		SB		
HCM Control Delay, s	0.3		0		10.7	
HCM LOS					B	
Minor Lane/Major Mvmt						
EBL		EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1275	-	-	-	647	
HCM Lane V/C Ratio	0.009	-	-	-	0.032	
HCM Control Delay (s)	7.8	0	-	-	10.7	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.1	

Intersection						
Int Delay, s/veh	4.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑		↑	↑
Traffic Vol, veh/h	100	86	150	90	90	130
Future Vol, veh/h	100	86	150	90	90	130
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	109	93	163	98	98	141
Major/Minor						
Conflicting Flow All	549	212	0	0	261	0
Stage 1	212	-	-	-	-	-
Stage 2	337	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	497	828	-	-	1303	-
Stage 1	823	-	-	-	-	-
Stage 2	723	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	456	828	-	-	1303	-
Mov Cap-2 Maneuver	456	-	-	-	-	-
Stage 1	756	-	-	-	-	-
Stage 2	723	-	-	-	-	-
Approach						
	WB	NB	SB			
HCM Control Delay, s	12.8	0	3.3			
HCM LOS	B					
Minor Lane/Major Mvmt						
	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	456	828	1303	-
HCM Lane V/C Ratio	-	-	0.238	0.113	0.075	-
HCM Control Delay (s)	-	-	15.3	9.9	8	0
HCM Lane LOS	-	-	C	A	A	A
HCM 95th %tile Q(veh)	-	-	0.9	0.4	0.2	-

Intersection												
Intersection Delay, s/veh		14.2										
Intersection LOS		B										
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	130	60	224	100	0	0	0	0	304	20	80
Future Vol, veh/h	0	130	60	224	100	0	0	0	0	304	20	80
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	4	4	4	23	23	23	2	2	2	15	15	15
Mvmt Flow	0	141	65	243	109	0	0	0	0	330	22	87
Number of Lanes	0	1	1	1	2	0	0	0	0	1	1	0
Approach	EB		WB			SB						
Opposing Approach	WB		EB									
Opposing Lanes	3		2			0						
Conflicting Approach Left	SB					WB						
Conflicting Lanes Left	2		0			3						
Conflicting Approach Right			SB			EB						
Conflicting Lanes Right	0		2			2						
HCM Control Delay	11.8		15.2			14.6						
HCM LOS	B		C			B						
Lane	EBLn1	EBLn2	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2					
Vol Left, %	0%	0%	100%	0%	0%	100%	49%					
Vol Thru, %	100%	0%	0%	100%	100%	0%	10%					
Vol Right, %	0%	100%	0%	0%	0%	0%	41%					
Sign Control	Stop											
Traffic Vol by Lane	130	60	224	50	50	207	197					
LT Vol	0	0	224	0	0	207	97					
Through Vol	130	0	0	50	50	0	20					
RT Vol	0	60	0	0	0	0	80					
Lane Flow Rate	141	65	243	54	54	225	214					
Geometry Grp	8	8	8	8	8	8	8					
Degree of Util (X)	0.278	0.115	0.507	0.105	0.073	0.446	0.394					
Departure Headway (Hd)	7.07	6.354	7.496	6.987	4.862	7.15	6.61					
Convergence, Y/N	Yes											
Cap	506	562	480	511	733	503	543					
Service Time	4.839	4.123	5.257	4.748	2.621	4.906	4.366					
HCM Lane V/C Ratio	0.279	0.116	0.506	0.106	0.074	0.447	0.394					
HCM Control Delay	12.6	10	17.8	10.6	8	15.6	13.6					
HCM Lane LOS	B	A	C	B	A	C	B					
HCM 95th-tile Q	1.1	0.4	2.8	0.3	0.2	2.3	1.9					

Intersection													
Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑				↑	↑	↑	↑	↑			
Traffic Vol, veh/h	80	374	0	1	0	254	380	60	20	197	0	0	0
Future Vol, veh/h	80	374	0	1	0	254	380	60	20	197	0	0	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	14	14	14	20	20	20	20	29	29	29	2	2	2
Mvmt Flow	87	407	0	1	0	276	413	65	22	214	0	0	0
Number of Lanes	1	2	0	0	0	1	1	0	1	1	0	0	0
Approach	EB			WB			NB						
Opposing Approach	WB			EB									
Opposing Lanes	2			3			0						
Conflicting Approach Left				NB			EB						
Conflicting Lanes Left	0			2			3						
Conflicting Approach Right	NB						WB						
Conflicting Lanes Right	2			0			2						
HCM Control Delay	13.7			24.9			16.1						
HCM LOS	B			C			C						
Lane	NBLn1	NBLn2	EBLn1	EBLn2	EBLn3	WBLn1	WBLn2						
Vol Left, %	75%	0%	100%	0%	0%	0%	0%						
Vol Thru, %	25%	0%	0%	100%	100%	100%	0%						
Vol Right, %	0%	100%	0%	0%	0%	0%	100%						
Sign Control	Stop												
Traffic Vol by Lane	80	197	80	187	187	255	380						
LT Vol	60	0	80	0	0	0	0						
Through Vol	20	0	0	187	187	255	0						
RT Vol	0	197	0	0	0	0	380						
Lane Flow Rate	87	214	87	203	203	277	413						
Geometry Grp	8	8	8	8	8	8	8						
Degree of Util (X)	0.214	0.462	0.199	0.437	0.325	0.571	0.769						
Departure Headway (Hd)	8.86	7.767	8.25	7.739	5.759	7.417	6.703						
Convergence, Y/N	Yes												
Cap	405	465	436	465	624	488	541						
Service Time	6.607	5.513	5.992	5.481	3.5	5.157	4.442						
HCM Lane V/C Ratio	0.215	0.46	0.2	0.437	0.325	0.568	0.763						
HCM Control Delay	14	17	13	16.4	11.3	19.6	28.4						
HCM Lane LOS	B	C	B	C	B	C	D						
HCM 95th-tile Q	0.8	2.4	0.7	2.2	1.4	3.5	6.9						

Trans Truck System Truck Facility TIA
4: Harlan Rd & Roth Rd

Cumulative plus Project
PM PEAK HOUR

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (veh/h)	76	286	260	60	344	65	250	70	86	62	60	70
Future Volume (veh/h)	76	286	260	60	344	65	250	70	86	62	60	70
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No		No		No	
Adj Sat Flow, veh/h/ln	1604	1604	1604	1441	1441	1441	1737	1737	1737	1559	1559	1559
Adj Flow Rate, veh/h	83	311	283	65	374	71	272	76	93	67	65	76
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	20	20	20	31	31	31	11	11	11	23	23	23
Cap, veh/h	101	581	493	75	413	78	316	192	235	80	87	102
Arrive On Green	0.07	0.36	0.36	0.05	0.35	0.35	0.19	0.27	0.27	0.05	0.13	0.13
Sat Flow, veh/h	1527	1604	1359	1372	1177	223	1654	711	870	1485	655	766
Grp Volume(v), veh/h	83	311	283	65	0	445	272	0	169	67	0	141
Grp Sat Flow(s),veh/h/ln	1527	1604	1359	1372	0	1400	1654	0	1580	1485	0	1421
Q Serve(g_s), s	3.7	10.6	11.6	3.3	0.0	21.0	11.0	0.0	6.1	3.1	0.0	6.6
Cycle Q Clear(g_c), s	3.7	10.6	11.6	3.3	0.0	21.0	11.0	0.0	6.1	3.1	0.0	6.6
Prop In Lane	1.00		1.00	1.00		0.16	1.00		0.55	1.00		0.54
Lane Grp Cap(c), veh/h	101	581	493	75	0	492	316	0	426	80	0	188
V/C Ratio(X)	0.83	0.54	0.57	0.87	0.00	0.91	0.86	0.00	0.40	0.84	0.00	0.75
Avail Cap(c_a), veh/h	126	617	523	148	0	575	394	0	627	221	0	437
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	32.0	17.5	17.8	32.5	0.0	21.4	27.2	0.0	20.7	32.5	0.0	29.0
Incr Delay (d2), s/veh	29.0	0.8	1.4	24.7	0.0	16.3	14.7	0.0	0.6	19.8	0.0	5.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.1	3.8	3.5	1.6	0.0	8.5	5.4	0.0	2.2	1.5	0.0	2.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	61.0	18.3	19.2	57.2	0.0	37.7	41.9	0.0	21.3	52.3	0.0	34.8
LnGrp LOS	E	B	B	E	A	D	D	A	C	D	A	C
Approach Vol, veh/h	677			510			441			208		
Approach Delay, s/veh	23.9			40.2			34.0			40.5		
Approach LOS	C			D			C			D		
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	8.2	23.2	8.3	29.6	17.7	13.7	9.1	28.8				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	10.3	27.5	7.5	26.7	16.5	21.3	5.7	28.5				
Max Q Clear Time (g_c+l1), s	5.1	8.1	5.3	13.6	13.0	8.6	5.7	23.0				
Green Ext Time (p_c), s	0.0	0.9	0.0	2.5	0.3	0.6	0.0	1.4				
Intersection Summary												
HCM 6th Ctrl Delay				32.7								
HCM 6th LOS				C								

Intersection						
Int Delay, s/veh	3.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	
Traffic Vol, veh/h	80	306	330	70	60	50
Future Vol, veh/h	80	306	330	70	60	50
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	20	20	28	28	4	4
Mvmt Flow	87	333	359	76	65	54
Major/Minor						
Major1		Major2		Minor2		
Conflicting Flow All	435	0	-	0	904	397
Stage 1	-	-	-	-	397	-
Stage 2	-	-	-	-	507	-
Critical Hdwy	4.3	-	-	-	6.44	6.24
Critical Hdwy Stg 1	-	-	-	-	5.44	-
Critical Hdwy Stg 2	-	-	-	-	5.44	-
Follow-up Hdwy	2.38	-	-	-	3.536	3.336
Pot Cap-1 Maneuver	1035	-	-	-	305	648
Stage 1	-	-	-	-	675	-
Stage 2	-	-	-	-	601	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1035	-	-	-	274	648
Mov Cap-2 Maneuver	-	-	-	-	274	-
Stage 1	-	-	-	-	605	-
Stage 2	-	-	-	-	601	-
Approach						
EB		WB		SB		
HCM Control Delay, s	1.8		0		19.3	
HCM LOS					C	
Minor Lane/Major Mvmt						
EBL		EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1035	-	-	-	371	
HCM Lane V/C Ratio	0.084	-	-	-	0.322	
HCM Control Delay (s)	8.8	0	-	-	19.3	
HCM Lane LOS	A	A	-	-	C	
HCM 95th %tile Q(veh)	0.3	-	-	-	1.4	

Trans Truck System Truck Facility TIA
6: Roth Rd & Airport Way

Cumulative plus Project
PM PEAK HOUR

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	128	222	141	319	356	93
Future Volume (veh/h)	128	222	141	319	356	93
Initial Q (Q _b), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1633	1633	1811	1811	1737	1737
Adj Flow Rate, veh/h	139	241	153	347	387	101
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	18	18	6	6	11	11
Cap, veh/h	346	308	192	1066	993	256
Arrive On Green	0.22	0.22	0.11	0.59	0.38	0.38
Sat Flow, veh/h	1555	1384	1725	1811	2683	670
Grp Volume(v), veh/h	139	241	153	347	244	244
Grp Sat Flow(s),veh/h/ln	1555	1384	1725	1811	1650	1616
Q Serve(g_s), s	3.6	7.8	4.1	4.6	5.1	5.2
Cycle Q Clear(g_c), s	3.6	7.8	4.1	4.6	5.1	5.2
Prop In Lane	1.00	1.00	1.00			0.41
Lane Grp Cap(c), veh/h	346	308	192	1066	631	618
V/C Ratio(X)	0.40	0.78	0.80	0.33	0.39	0.39
Avail Cap(c_a), veh/h	589	524	199	1066	631	618
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	15.8	17.4	20.6	5.0	10.6	10.7
Incr Delay (d2), s/veh	0.8	4.4	19.1	0.8	1.8	1.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.2	6.0	2.5	1.3	1.8	1.8
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	16.6	21.8	39.7	5.8	12.4	12.6
LnGrp LOS	B	C	D	A	B	B
Approach Vol, veh/h	380			500	488	
Approach Delay, s/veh	19.9			16.2	12.5	
Approach LOS	B			B	B	
Timer - Assigned Phs	2			4	5	6
Phs Duration (G+Y+Rc), s	32.5			15.1	9.8	22.7
Change Period (Y+Rc), s	4.5			4.5	4.5	4.5
Max Green Setting (Gmax), s	28.0			18.0	5.5	18.0
Max Q Clear Time (g_c+l1), s	6.6			9.8	6.1	7.2
Green Ext Time (p_c), s	2.1			0.8	0.0	2.2
Intersection Summary						
HCM 6th Ctrl Delay				15.9		
HCM 6th LOS				B		

Intersection						
Int Delay, s/veh	0.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	
Traffic Vol, veh/h	8	316	356	4	5	12
Future Vol, veh/h	8	316	356	4	5	12
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	9	343	387	4	5	13
Major/Minor						
Major1		Major2		Minor2		
Conflicting Flow All	391	0	-	0	750	389
Stage 1	-	-	-	-	389	-
Stage 2	-	-	-	-	361	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1168	-	-	-	379	659
Stage 1	-	-	-	-	685	-
Stage 2	-	-	-	-	705	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1168	-	-	-	375	659
Mov Cap-2 Maneuver	-	-	-	-	375	-
Stage 1	-	-	-	-	678	-
Stage 2	-	-	-	-	705	-
Approach						
EB		WB		SB		
HCM Control Delay, s	0.2		0		11.9	
HCM LOS					B	
Minor Lane/Major Mvmt						
EBL		EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1168	-	-	-	539	
HCM Lane V/C Ratio	0.007	-	-	-	0.034	
HCM Control Delay (s)	8.1	0	-	-	11.9	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.1	

Appendix G

*Analysis Worksheets for
Mitigated Conditions*

Intersection												
Intersection Delay, s/veh		13.7										
Intersection LOS		B										
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	84	22	177	85	0	0	0	0	279	5	52
Future Vol, veh/h	0	84	22	177	85	0	0	0	0	279	5	52
Peak Hour Factor	0.56	0.56	0.56	0.79	0.79	0.79	0.92	0.92	0.92	0.88	0.88	0.88
Heavy Vehicles, %	12	12	12	40	40	40	40	40	40	20	20	20
Mvmt Flow	0	150	39	224	108	0	0	0	0	317	6	59
Number of Lanes	0	1	1	1	2	0	0	0	0	1	1	0
Approach	EB		WB			SB						
Opposing Approach	WB		EB									
Opposing Lanes	3		2			0						
Conflicting Approach Left	SB					WB						
Conflicting Lanes Left	2		0			3						
Conflicting Approach Right			SB			EB						
Conflicting Lanes Right	0		2			2						
HCM Control Delay	12		14.5			13.8						
HCM LOS	B		B			B						
Lane	EBLn1	EBLn2	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2					
Vol Left, %	0%	0%	100%	0%	0%	100%	66%					
Vol Thru, %	100%	0%	0%	100%	100%	0%	3%					
Vol Right, %	0%	100%	0%	0%	0%	0%	31%					
Sign Control	Stop											
Traffic Vol by Lane	84	22	177	43	43	170	166					
LT Vol	0	0	177	0	0	170	109					
Through Vol	84	0	0	43	43	0	5					
RT Vol	0	22	0	0	0	0	52					
Lane Flow Rate	150	39	224	54	54	193	188					
Geometry Grp	8	8	8	8	8	8	8					
Degree of Util (X)	0.292	0.069	0.472	0.106	0.07	0.383	0.353					
Departure Headway (Hd)	6.998	6.284	7.59	7.083	4.667	7.134	6.741					
Convergence, Y/N	Yes											
Cap	513	568	475	505	764	505	533					
Service Time	4.756	4.041	5.342	4.835	2.418	4.884	4.491					
HCM Lane V/C Ratio	0.292	0.069	0.472	0.107	0.071	0.382	0.353					
HCM Control Delay	12.6	9.5	17	10.7	7.8	14.3	13.2					
HCM Lane LOS	B	A	C	B	A	B	B					
HCM 95th-tile Q	1.2	0.2	2.5	0.4	0.2	1.8	1.6					

Trans Truck System Truck Facility TIA
4: Harlan Rd & Roth Rd

Existing + AP plus Project - Mit
AM PEAK HOUR

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↑	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖
Traffic Volume (veh/h)	77	303	144	35	239	45	234	44	51	41	18	66
Future Volume (veh/h)	77	303	144	35	239	45	234	44	51	41	18	66
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		No
Adj Sat Flow, veh/h/ln	1426	1426	1426	1218	1218	1218	1707	1707	1707	1115	1115	1115
Adj Flow Rate, veh/h	82	322	153	44	303	57	296	56	65	52	23	84
Peak Hour Factor	0.94	0.94	0.94	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Percent Heavy Veh, %	32	32	32	46	46	46	13	13	13	53	53	53
Cap, veh/h	96	514	436	47	330	62	339	214	249	53	29	107
Arrive On Green	0.07	0.36	0.36	0.04	0.33	0.33	0.21	0.30	0.30	0.05	0.14	0.14
Sat Flow, veh/h	1358	1426	1208	1160	997	188	1626	721	836	1061	210	767
Grp Volume(v), veh/h	82	322	153	44	0	360	296	0	121	52	0	107
Grp Sat Flow(s),veh/h/ln	1358	1426	1208	1160	0	1185	1626	0	1557	1061	0	977
Q Serve(g_s), s	4.3	13.4	6.6	2.7	0.0	20.9	12.6	0.0	4.2	3.5	0.0	7.6
Cycle Q Clear(g_c), s	4.3	13.4	6.6	2.7	0.0	20.9	12.6	0.0	4.2	3.5	0.0	7.6
Prop In Lane	1.00		1.00	1.00		0.16	1.00		0.54	1.00		0.79
Lane Grp Cap(c), veh/h	96	514	436	47	0	392	339	0	463	53	0	136
V/C Ratio(X)	0.86	0.63	0.35	0.93	0.00	0.92	0.87	0.00	0.26	0.98	0.00	0.79
Avail Cap(c_a), veh/h	123	537	455	97	0	438	420	0	635	145	0	280
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	32.9	18.9	16.8	34.3	0.0	23.0	27.4	0.0	19.2	34.0	0.0	29.8
Incr Delay (d2), s/veh	34.6	2.2	0.5	45.3	0.0	23.0	15.5	0.0	0.3	53.4	0.0	9.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.3	4.4	1.8	1.3	0.0	7.9	6.1	0.0	1.5	1.7	0.0	2.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	67.5	21.1	17.3	79.5	0.0	46.1	42.9	0.0	19.5	87.4	0.0	39.5
LnGrp LOS	E	C	B	E	A	D	D	A	B	F	A	D
Approach Vol, veh/h		557			404			417			159	
Approach Delay, s/veh		26.9			49.7			36.1			55.2	
Approach LOS		C			D			D			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	8.1	25.8	7.4	30.3	19.4	14.5	9.6	28.2				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	9.8	29.2	6.0	27.0	18.5	20.5	6.5	26.5				
Max Q Clear Time (g_c+l1), s	5.5	6.2	4.7	15.4	14.6	9.6	6.3	22.9				
Green Ext Time (p_c), s	0.0	0.6	0.0	2.0	0.3	0.4	0.0	0.8				
Intersection Summary												
HCM 6th Ctrl Delay			38.3									
HCM 6th LOS			D									

Intersection												
Intersection Delay, s/veh		12.3										
Intersection LOS		B										
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	80	36	175	96	0	0	0	0	270	4	38
Future Vol, veh/h	0	80	36	175	96	0	0	0	0	270	4	38
Peak Hour Factor	0.89	0.89	0.89	0.81	0.81	0.81	0.92	0.92	0.92	0.85	0.85	0.85
Heavy Vehicles, %	4	4	4	23	23	23	2	2	2	15	15	15
Mvmt Flow	0	90	40	216	119	0	0	0	0	318	5	45
Number of Lanes	0	1	1	1	2	0	0	0	0	1	1	0
Approach	EB		WB			SB						
Opposing Approach	WB		EB									
Opposing Lanes	3		2			0						
Conflicting Approach Left	SB					WB						
Conflicting Lanes Left	2		0			3						
Conflicting Approach Right			SB			EB						
Conflicting Lanes Right	0		2			2						
HCM Control Delay	10.3		12.7			12.7						
HCM LOS	B		B			B						
Lane	EBLn1	EBLn2	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2					
Vol Left, %	0%	0%	100%	0%	0%	100%	72%					
Vol Thru, %	100%	0%	0%	100%	100%	0%	3%					
Vol Right, %	0%	100%	0%	0%	0%	0%	25%					
Sign Control	Stop											
Traffic Vol by Lane	80	36	175	48	48	159	153					
LT Vol	0	0	175	0	0	159	111					
Through Vol	80	0	0	48	48	0	4					
RT Vol	0	36	0	0	0	0	38					
Lane Flow Rate	90	40	216	59	59	187	180					
Geometry Grp	8	8	8	8	8	8	8					
Degree of Util (X)	0.167	0.067	0.421	0.107	0.072	0.352	0.322					
Departure Headway (Hd)	6.7	5.987	7.018	6.512	4.394	6.761	6.449					
Convergence, Y/N	Yes											
Cap	535	597	514	551	814	532	559					
Service Time	4.444	3.73	4.754	4.248	2.129	4.491	4.178					
HCM Lane V/C Ratio	0.168	0.067	0.42	0.107	0.072	0.352	0.322					
HCM Control Delay	10.8	9.2	14.8	10	7.5	13.1	12.2					
HCM Lane LOS	B	A	B	A	A	B	B					
HCM 95th-tile Q	0.6	0.2	2.1	0.4	0.2	1.6	1.4					

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖↑	↑↗	↖↑	↖↑	↖↑	↖↑	↖↑	↖↑	↖↑	↖↑	↖↑	↖↑
Traffic Volume (veh/h)	76	286	185	54	344	65	207	59	86	62	53	65
Future Volume (veh/h)	76	286	185	54	344	65	207	59	86	62	53	65
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No		No		No	
Adj Sat Flow, veh/h/ln	1604	1604	1604	1441	1441	1441	1737	1737	1737	1559	1559	1559
Adj Flow Rate, veh/h	90	340	220	68	430	81	256	73	106	84	72	88
Peak Hour Factor	0.84	0.84	0.84	0.80	0.80	0.80	0.81	0.81	0.81	0.74	0.74	0.74
Percent Heavy Veh, %	20	20	20	31	31	31	11	11	11	23	23	23
Cap, veh/h	110	643	545	80	456	86	377	163	236	102	241	294
Arrive On Green	0.07	0.40	0.40	0.06	0.39	0.39	0.25	0.25	0.25	0.07	0.38	0.38
Sat Flow, veh/h	1527	1604	1359	1372	1179	222	1139	640	930	1485	638	780
Grp Volume(v), veh/h	90	340	220	68	0	511	256	0	179	84	0	160
Grp Sat Flow(s),veh/h/ln	1527	1604	1359	1372	0	1401	1139	0	1570	1485	0	1419
Q Serve(g_s), s	4.8	13.3	9.5	4.0	0.0	29.0	17.8	0.0	7.9	4.6	0.0	6.5
Cycle Q Clear(g_c), s	4.8	13.3	9.5	4.0	0.0	29.0	17.8	0.0	7.9	4.6	0.0	6.5
Prop In Lane	1.00		1.00	1.00		0.16	1.00		0.59	1.00		0.55
Lane Grp Cap(c), veh/h	110	643	545	80	0	542	377	0	399	102	0	535
V/C Ratio(X)	0.82	0.53	0.40	0.85	0.00	0.94	0.68	0.00	0.45	0.83	0.00	0.30
Avail Cap(c_a), veh/h	121	643	545	130	0	571	441	0	487	117	0	630
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	37.6	18.7	17.6	38.4	0.0	24.3	29.5	0.0	25.8	37.8	0.0	18.0
Incr Delay (d2), s/veh	32.1	0.8	0.5	24.2	0.0	23.8	3.4	0.0	0.8	33.0	0.0	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.7	4.8	2.9	1.9	0.0	12.5	5.0	0.0	2.9	2.6	0.0	2.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	69.7	19.6	18.1	62.6	0.0	48.2	32.9	0.0	26.6	70.8	0.0	18.3
LnGrp LOS	E	B	B	E	A	D	C	A	C	E	A	B
Approach Vol, veh/h	650				579			435			244	
Approach Delay, s/veh	26.0				49.9			30.3			36.4	
Approach LOS	C			D			C			D		
Timer - Assigned Phs	1	2	3	4		6	7	8				
Phs Duration (G+Y+Rc), s	10.1	25.4	9.3	37.4		35.5	10.4	36.3				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5		4.5	4.5	4.5				
Max Green Setting (Gmax), s	6.5	25.5	7.8	32.2		36.5	6.5	33.5				
Max Q Clear Time (g_c+l1), s	6.6	19.8	6.0	15.3		8.5	6.8	31.0				
Green Ext Time (p_c), s	0.0	1.1	0.0	2.7		1.0	0.0	0.9				
Intersection Summary												
HCM 6th Ctrl Delay			35.6									
HCM 6th LOS			D									