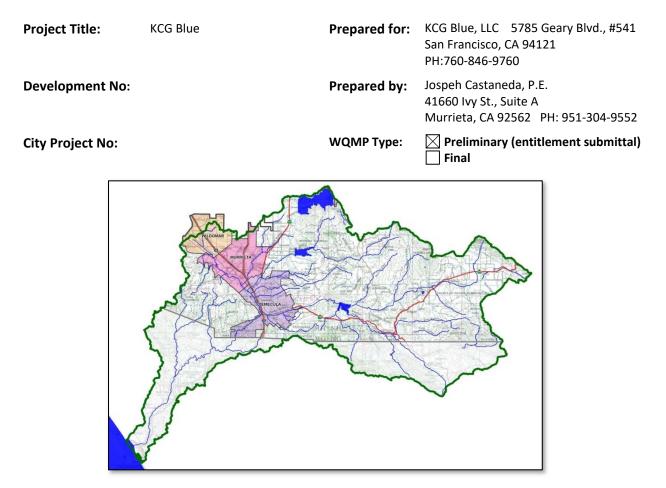
Appendix 10.0

Project Specific Water Quality Management Plan



Project Specific Water Quality Management Plan (WQMP)

A Template for preparing Project Specific Water Quality Management Plans (WQMPs) for Priority Development Projects located in the City of Wildomar.



Original Date Prepared: June 5, 2019

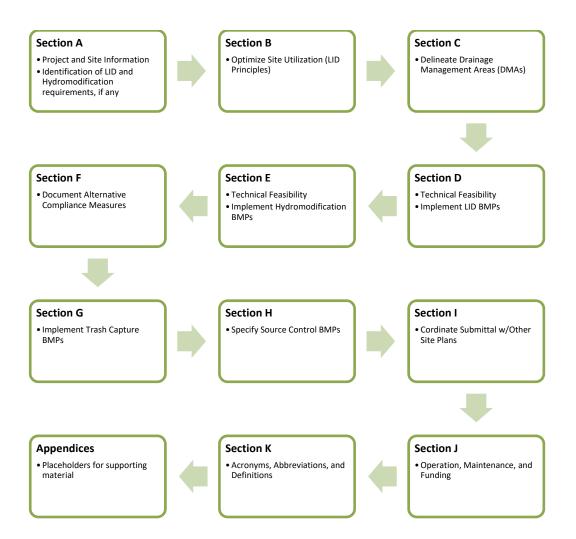
Revision Summary (post WQMP acceptance):

		09/03/2019			
MARK	BY	DATE		APPRV.	DATE
ENGINEER		NEER	REVISIONS	(CITY

Prepared for Compliance with Regional Board Order No. <u>**R9-2013-0001**</u> as amended by Order No. **R9-2015-0001** and Order No. **R9-2015-0100**

A Brief Introduction

The Regional Municipal Separate Stormwater Sewer System (MS4) Permit¹ requires that a Project-Specific WQMP be prepared for all development projects within the Santa Margarita Region (SMR) that meet the 'Priority Development Project' categories and thresholds listed in the SMR Water Quality Management Plan (WQPM). This Project-Specific WQMP Template for Development Projects in the **Santa Margarita Region** has been prepared to help document compliance and prepare a WQMP submittal. Below is a flowchart for the layout of this Template that will provide the steps required to document compliance.



¹ Order No. R9-2013-0001 as amended by Order Nos. R9-2015-0001 and R9-2015-0100, NPDES No. CAS0109266, National Pollutant Discharge Elimination System (NPDES) Permit and Waste Discharge Requirements for Discharges from the MS4s Draining the Watersheds within the San Diego Region, California Regional Water Quality Control Board, May 8, 2013.

OWNER'S CERTIFICATION

This Project-Specific WQMP has been prepared for KCG Blue, LLC by JLC Engineering & Consulting, Inc. for the KCG Blue Shooting Range project.

This WQMP is intended to comply with the requirements of the City of Wildomar for Wildomar Municipal Code Ch. 13.12 which includes the requirement for the preparation and implementation of a Project-Specific WQMP.

The undersigned, while owning the property/project described in the preceding paragraph, shall be responsible for the implementation and funding of this WQMP and will ensure that this WQMP is amended as appropriate to reflect up-to-date conditions on the site. In addition, the property owner accepts responsibility for interim operation and maintenance of storm water Best Management Practices until such time as this responsibility is formally transferred to a subsequent owner. This WQMP will be reviewed with the facility operator, facility supervisors, employees, tenants, maintenance and service contractors, or any other party (or parties) having responsibility for implementing portions of this WQMP. At least one copy of this WQMP will be maintained at the project site or project office in perpetuity. The undersigned is authorized to certify and to approve implementation of this WQMP. The undersigned is aware that implementation of this WQMP is enforceable under the City of Wildomar Water Quality Ordinance (Wildomar Municipal Code Ch. 13.12).

"I, the undersigned, certify under penalty of law that the provisions of this WQMP have been reviewed and accepted and that the WQMP will be transferred to future successors in interest."

Owner's Signature

Date

Owner's Printed Name

Owner's Title/Position

PREPARER'S CERTIFICATION

"The selection, sizing and design of stormwater treatment and other stormwater quality and quantity control Best Management Practices (BMPs) in this plan meet the requirements of Regional Water Quality Control Board Order No. **R9-2013-0001** as amended by Order Nos. **R9-2015-0001 and R9-2015-0100**."

righ & Dartuele

Preparer's Signature

Date

Preparer's Printed Name

Preparer's Licensure:



Preparer's Title/Position

Table of Contents

Section A: Project and Site Information	0
 A.1 Maps and Site Plans A.2 Identify Receiving Waters A.3 Drainage System Susceptibility to Hydromodification A.4 Additional Permits/Approvals required for the Project: Section B: Optimize Site Utilization (LID Principles) 	
Section C: Delineate Drainage Management Areas (DMAs)	9
Section D: Implement LID BMPs	20
D.1 Full Infiltration Applicability D.2 Biofiltration Applicability D.3 Feasibility Assessment Summaries D.4 LID BMP Sizing	22 24
Section E: Implement Hydrologic Control BMPs and Sediment Supply BMPs	29
E.1 Hydrologic Control BMP Selection E.2 Hydrologic Control BMP Sizing E.3 Implement Sediment Supply BMPs Section F: Alternative Compliance	30 30
 F.1 Identify Pollutants of Concern. F.2 Treatment Control BMP Selection F.3 Sizing Criteria. F.4 Hydrologic Performance Standard – Alternative Compliance Approach Section G: Implement Trash Capture BMPs. 	
Section H: Source Control BMPs	43
Section I: Coordinate Submittal with Other Site Plans	45
Section J: Operation, Maintenance and Funding	46
Section K: Acronyms, Abbreviations and Definitions	47

List of Tables

Table A-1 Identification of Receiving Waters	2
Table A-2 Identification of Susceptibility to Hydromodification	2
Table A-3 Other Applicable Permits	3
Table C-1 DMA Identification	9
Table C-2 Type 'A', Self-Treating Areas	17
Table C-3 Type 'B', Self-Retaining Areas	18
Table C-4 Type 'C', Areas that Drain to Self-Retaining Areas	18
Table C-5 Type 'D', Areas Draining to BMPs	19
Table D-1 Infiltration Feasibility	21
Table D-2 Geotechnical Concerns for Onsite Infiltration	22
Table D-3 Evaluation of Biofiltration BMP Feasibility	23
Table D-4 Proprietary BMP Approval Requirement Summary	23
Table D-5 LID Prioritization Summary Matrix	24
Table D-6 DCV Calculations for LID BMPs	26
Table D-7 LID BMP Sizing	27
Table E-1 Hydrologic Control BMP Sizing	30
Table E-2 Triad Assessment Summary	33
Table F-1 Summary of Approved 2010 303(d) listed waterbodies and associated pollutants of co	oncern for
the Riverside County SMR Region and downstream waterbodies	36
Table F-2 Potential Pollutants by Land Use Type	37
Table F-3 Treatment Control BMP Selection	
Table F-4 Treatment Control BMP Sizing	
Table F-5 Offsite Hydrologic Control BMP Sizing	
Table G-1 Sizing Trash Capture BMPs	40
Table G-2 Approximate precipitation depth/intensity values for calculation of the Trash Capture	e Design
Storm	41
Table G-3 Trash Capture BMPs	41
Table I-1 Construction Plan Cross-reference	45
Table I-2 Other Applicable Permits	45

List of Appendices

Appendix 1: Maps and Site Plans	54
Appendix 2: Construction Plans	
Appendix 3: Soils Information	
Appendix 4: Historical Site Conditions	60
Appendix 5: LID Infeasibility	61

Appendix 6: BMP Design Details	62
Appendix 7: Hydromodification	63
Appendix 8: Source Control	68
Appendix 9: O&M	69
Appendix 10: Educational Materials	60

Section A: Project and Site Information

Use the table below to compile and summarize basic site information that will be important for completing subsequent steps. Subsections A.1 through A.4 provide additional detail on documentation of additional project and site information.

PROJECT INFORMATION				
Type of PDP:	New Development			
Type of Project:	Light Industrial			
Planning Area:	N/A			
Community Name:	N/A			
Development Name:	N/A			
PROJECT LOCATION				
Latitude & Longitude (DMS):		N 33°37'35", W 117°17'21"		
Project Watershed and Sub-\	Watershed:	Santa Ana River/San Jacinto River Subwatershed Santa Ana Hydrologic Unit Number 802.31		
24-Hour 85 th Percentile Storr	n Depth (inches):	0.72		
Is project subject to Hydrome	odification requirements?	Y N (Select based on Sec	tion A.3)	
APN(s):		367-020-038		
Map Book and Page No.:		Map Book 10 Page 58-75/Thomas Bros. Pg. 897 Grid A-4		
PROJECT CHARACTERISTICS				
Proposed or Potential Land L	Jse(s)		Indoor S	Shooting Range
Proposed or Potential SIC Co	de(s)		7997	
Existing Impervious Area of P	Project Footprint (SF)		0 SF	
Total area of <u>proposed</u> Impe	rvious Surfaces within the Pro	oject Limits (SF)/or Replacement	86,318 9	sf
Total Project Area (ac)			1.92 Acı	res
Does the project consist of o	ffsite road improvements?		🖂 Y	□ N
Does the project propose to	construct unpaved roads?		Y	N
Is the project part of a larger	nt (phased project)?	Y	N	
Is the project exempt from H	ydromodification Performan	ce Standards?	Y	🖂 N
Does the project propose the	e use of Alternative Compliar	nce to satisfy BMP requirements?	Y	\boxtimes N
		diment performance standards)	_	
	Has preparation of Project-Specific WQMP included coordination with other site plans?			
EXISTING SITE CHARACTERISTICS			—	
Is the project located within any Multi-Species Habitat Conservation Plan area (MSHCP $\[]$ Y $\[]$ N				⊠ N
Criteria Cell?)				
Are there any natural hydrologic features on the project site?			Υ	N
Is a Geotechnical Report atta		Y		
If no Geotech. Report, list the Natural Resources Conservation Service (NRCS) soils type(s) Soil Type B & C				e B & C
present on the site (A, B, C and/or D)				
<u>Provide a brief description of the project</u> : The project site is a proposed commercial office building that will include a building partiag area landscaped area streat frontige improvements and a biofiltration basis. The project site will				

building, parking area, landscaped area, street frontage improvements, and a biofiltration basin. The project site will drain the northerly and westerly 2.07 acres of developed area to a proposed biofiltration basin located at the northerly portion of the project site. Flows will be treated for water quality purposes prior to being pumped into a concrete ditch

within the Mission Trail right-of-way. The south westerly portion of the project site will discharge at the southerly project boundary via a dispersion area that will disperse the flows in a sheet flow condition to mimic the existing condition.

The pre-project condition flows currently flow from easterly to south westerly and sheet flows across the southerly boundary of the project site. The current land use is a mix of ½ acre residential homes, undeveloped land use and commercial land use for the existing paved Bundy Canyon Road. The post-project condition will keep this flow pattern. The post-project will intercept the offsite flows emanating from the east via a U Channel along the easterly and southerly project boundry. The onsite flows tributary to the porous pavers will be collected in this U Channel and both the offsite and treated onsite flows will be dispersed through the dispersion area. The offsite flows from the Bundy Canyon Road improvements will be collected via a proposed catch basin. The flows from the Bundy Canyon Road improvements, as well as the onsite flows from the north and westerly portion of the site, will be conveyed via sheet flow and storm drain to the biofiltration basin. The low flows from the underdrains will be pumped from the basin, and flows in excess of the water quality volume will be conveyed directly out through the outlet structure. Flows will then discharge into a proposed concrete channel within the Mission Trail right-of-way.

A.1 Maps and Site Plans

When completing your Project-Specific WQMP, include a map of the Project vicinity and existing site. In addition, include all grading, drainage, landscape/plant palette and other pertinent construction plans in Appendix 2. At a **minimum**, your WQMP Site Plan should include the following:

- Vicinity and location maps
- Parcel Boundary and Project Footprint
- Existing and Proposed Topography
- Drainage Management Areas (DMAs)
- Proposed Structural Best Management Practices (BMPs)
- Drainage Paths
- Drainage infrastructure, inlets, overflows

- Source Control BMPs
- Site Design BMPs
- Buildings, Roof Lines, Downspouts
- Impervious Surfaces
- Pervious Surfaces (i.e. Landscaping)
- Standard Labeling

Use your discretion on whether or not you may need to create multiple sheets or can appropriately accommodate these features on one or two sheets. Keep in mind that the Copermittee plan reviewer must be able to easily analyze your Project utilizing this template and its associated site plans and maps. Complete the checklists in Appendix 1 to verify that all exhibits and components are included.

A.2 Identify Receiving Waters

Using Table A-1 below, list in order of upstream to downstream, the Receiving Waters that the Project site is tributary to. Continue to fill each row with the Receiving Water's 303(d) listed impairments (if any), designated Beneficial Uses, and proximity, if any, to a RARE Beneficial Use. Include a map of the Receiving Waters in Appendix 1. This map should identify the path of the storm water discharged from the site all the way to the outlet of the Santa Margarita River to the Pacific Ocean. Use the most recent 303(d) list available from the State Water Resources Control Board Website.

(http://www.waterboards.ca.gov/sandiego/water issues/programs/basin plan/)

Receiving Waters USEPA Approved 303(d) List Impairments		Designated Beneficial Uses	Proximity to RARE Beneficial Use
Lake Elsinore	Nutrients, Organic Enrichment/Low Dissolved Oxygen, PCBs, Sediment Toxicity	MUN, REC1, REC2, WARM, WILD	N/A

Table A-1 Identification of Receiving Waters

A.3 Drainage System Susceptibility to Hydromodification

Using Table A-2 below, list in order of the point of discharge at the project site down to the Santa Margarita River², each drainage system or receiving water that the project site is tributary to. Continue to fill each row with the material of the drainage system, and any exemption (if applicable). Based on the results, summarize the applicable hydromodification performance standards that will be documented in Section E. Exempted categories of receiving waters include:

- Existing storm drains that discharge directly to water storage reservoirs, lakes, or enclosed embayments, or
- Conveyance channels whose bed and bank are concrete lined all the way from the point of discharge to water storage reservoirs, lakes, enclosed embayments, or the Pacific Ocean.
- Other water bodies identified in an approved Watershed Management Area Analysis (WMAA) (See Exhibit G to the WQMP)

Include a map exhibiting each drainage system and the associated susceptibility in Appendix 1.

Drainage System	Drainage System Material	Hydromodification Exemption	Hydromodification Exempt	
Mission Trails Culvert	Reinforced Concrete Pipe	This area is exempt based on location. This drainage system is located in the Santa Ana Watershed and drains to Lake Elsinore.	⊠Y □N	
Corydon Road Culvert	Reinforced Concrete Pipe	This area is exempt based on location. This drainage system is located in the Santa Ana Watershed and drains to Lake Elsinore.	⊠Y □N	
			□Y □N	
Summary of Perform	mance Standards	I		
Hydromodification Exempt – Select if "Y" is selected in the Hydromodification Exempt column above, project is exempt from hydromodification requirements.				
Not Exempt -Select if "N" is selected in any row of the Hydromodification Exempt column above. Project is subject to hydrologic control requirements and may be subject to sediment supply requirements.				

Table A-2 Identification of Susceptibility to Hydromodification

² Refer to Exhibit G of the WQMP for a map of exempt and potentially exempt areas. These maps are from the Draft SMR WMAA as of January 5, 2018 and will be replaced upon acceptance of the SMR WMAA.

A.4 Additional Permits/Approvals required for the Project:

 Table A-3 Other Applicable Permits

Agency	Permit Re	quired
State Department of Fish and Game, 1602 Streambed Alteration Agreement	□ Y	N 🛛
State Water Resources Control Board, Clean Water Act Section 401 Water Quality Certification	□ Y	N 🛛
US Army Corps of Engineers, Clean Water Act Section 404 Permit	□ Y	N 🛛
US Fish and Wildlife, Endangered Species Act Section 7 Biological Opinion	□ Y	N 🛛
Statewide Construction General Permit Coverage	X 🛛	□ N
Statewide Industrial General Permit Coverage	□ Y	N 🛛
Western Riverside Multiple Species Habitat Conservation Plan (MSHCP) Consistency Approval (e.g., Joint Project Review (JPR), Determination of Biological Equivalent or Superior Preservation (DBESP))	ΓY	N 🛛
Other (please list in the space below as required)	ΓY	N 🛛

If yes is answered to any of the questions above, the Copermittee may require proof of approval/coverage from those agencies as applicable including documentation of any associated requirements that may affect this Project-Specific WQMP.

Section B: Optimize Site Utilization (LID Principles)

Review of the information collected in Section 'A' will aid in identifying the principal constraints on site design and selection of LID BMPs as well as opportunities to reduce imperviousness and incorporate LID Principles into the site and landscape design. For example, constraints might include impermeable soils, high groundwater, groundwater pollution or contaminated soils, steep slopes, geotechnical instability, high-intensity land use, heavy pedestrian or vehicular traffic, utility locations or safety concerns. Opportunities might include existing natural areas, low areas, oddly configured or otherwise unbuildable parcels, easements and landscape amenities including open space and buffers (which can double as locations for LID Bioretention BMPs), and differences in elevation (which can provide hydraulic head). Prepare a brief narrative for each of the site optimization strategies described below. This narrative will help you as you proceed with your Low Impact Development (LID) design and explain your design decisions to others.

Apply the following LID Principles to the layout of the Priority Development Project (PDP) to the extent they are applicable and feasible. Putting thought upfront about how best to organize the various elements of a site can help to significantly reduce the PDP's potential impact on the environment and reduce the number and size of Structural LID BMPs that must be implemented. Integrate opportunities to accommodate the following LID Principles within the preliminary PDP site layout to maximize implementation of LID Principles.

Site Optimization

Complete checklist below to determine applicable Site Design BMPs for your site.

Project- Specific WQMP Site Design BMP Checklist

The following questions below are based upon Section 3.2 of the SMR WQMP will help you determine how to best optimize your site and subsequently identify opportunities and/or constraints, and document compliance.

SITE DESIGN REQUIREMENTS

Answer the following questions below by indicating "Yes," "No," or "N/A" (Not Applicable). Justify all "No" and "N/A" answers by inserting a narrative at the end of the section. The narrative should include identification and justification of any constraints that would prevent the use of those categories of LID BMPs. Upon identifying Site Design BMP opportunities, include these on your WQMP Site plan in Appendix 1.

Did you identify and preserve existing drainage patterns?

Integrating existing drainage patterns into the site plan helps to maintain the time of concentration and infiltration rates of runoff, decreasing peak flows, and may also help preserve the contribution of Critical Coarse Sediment (i.e., Bed Sediment Supply) from the PDP to the Receiving Water. Preserve existing drainage patterns by:

• Minimizing unnecessary site grading that would eliminate small depressions, where appropriate add additional "micro" storage throughout the site landscaping.

🛛 Yes 🗌 No 🗌 N/A

Yes No 🛛 N/A

- Where possible conform the PDP site layout along natural landforms, avoid excessive grading and disturbance of vegetation and soils, preserve or replicate the sites natural drainage features and patterns.
- Set back PDP improvements from creeks, wetlands, riparian habitats and any other natural water bodies.
- Use existing and proposed site drainage patterns as a natural design element, rather than using expensive impervious conveyance systems. Use depressed landscaped areas, vegetated buffers, and bioretention areas as amenities and focal points within the site and landscape design.

Discuss how this was included or provide a discussion/justification for "No" or "N/A" answer. The project will provide a drainage concept that will maintain the current existing drainage patterns as they currently exist. All project flows will be captured and treated prior to being discharged along the southerly property boundary utilizing a dispersion channel to mimic the sheet flow pattern that currently exists, or through a channel within the Mission Trail right-of-way.

Did you identify and protect existing vegetation?

Identify any areas containing dense native vegetation or well-established trees, and try to avoid disturbing these areas. Soils with thick, undisturbed vegetation have a much higher capacity to store and infiltrate runoff than do disturbed soils. Reestablishment of a mature vegetative community may take decades. Sensitive areas, such as streams and floodplains should also be avoided.

- Define the development envelope and protected areas, identifying areas that are most suitable for development and areas that should be left undisturbed.
- Establish setbacks and buffer zones surrounding sensitive areas.
- Preserve significant trees and other natural vegetation where possible.

Discuss how this was included or provide a discussion/justification for "No" or "N/A" answer. The project site is currently barren with no well-established vegetation.

	Project- Specific WQMP Site Design BMP Checklist		
	Did you identify and preserve natural infiltration capacity?		
□ Yes ⊠ No □ N/A	A key component of LID is taking advantage of a site's natural infiltration and storage capacity. A site survey and geotechnical investigation can help define areas with high potential for infiltration and surface storage.		
	 Identify opportunities to locate LID Principles and Structural BMPs in highly pervious areas. Doing so will maximize infiltration and limit the amount of runoff generated. Concentrate development on portions of the site with less permeable soils, and preserve areas that can promote infiltration. 		
Discuss how this was included or provide a discussion/justification for "No" or "N/A" answer. The project site was found to have infiltration rates that made infiltration BMPs infeasible. As a result of the low rates, a biofiltration basin is proposed that will allow for some infiltration to occur, as well as porous pavers for self-retaining areas that promote micro-infiltration.			
⊠ Yes □ No □ N/A	 Did you minimize impervious area? Look for opportunities to limit impervious cover through identification of the smallest possible land area that can be practically impacted or disturbed during site development. Limit overall coverage of paving and roofs. This can be accomplished by designing compact, taller structures, narrower and shorter streets and sidewalks, clustering buildings and sharing driveways, smaller parking lots (fewer stalls, smaller stalls, and more efficient lanes), and indoor or underground parking. Inventory planned impervious areas on your preliminary site plan. Identify where permeable pavements, or other permeable materials, such as crushed aggregate, turf block, permeable modular blocks, pervious concrete or pervious asphalt could be substituted for impervious concrete or asphalt paving. This will help reduce the amount of Runoff that may need to be addressed through Structural BMPs. Examine site layout and circulation patterns and identify areas where landscaping can be substituted for pavement, such as for overflow parking. Consider green roofs. Green roofs are roofing systems that provide a layer of soil/vegetative cover over a waterproofing membrane. A green roof mimics predevelopment conditions by filtering, absorbing, and evapotranspiring precipitation to help manage the effects of an otherwise impervious rooftop. 		
Discuss how this was included or provide a discussion/justification for "No" or "N/A" answer. Based on the proposed project type, the impervious area was limited to the maximum extent practical. Landscaped areas are provided and biofiltration basins are proposed to offset the impacts of the proposed imperious cover that was created. Additionally, porous pavers will be utilized to minimize the impervious area.			

	Project- Specific WQMP Site Design BMP Checklist	
	Did you identify and disperse runoff to adjacent pervious areas or small collection areas? Look for opportunities to direct runoff from impervious areas to adjacent landscaping, other pervious areas, or small collection areas where such runoff may be retained. This is sometimes referred to as reducing Directly Connected Impervious Areas.	
⊠ Yes □ No □ N/A	 Direct roof runoff into landscaped areas such as medians, parking islands, planter boxes, etc., and/or areas of pervious paving. Instead of having landscaped areas raised above the surrounding impervious areas, design them as depressed areas that can receive Runoff from adjacent impervious pavement. For example, a lawn or garden depressed 3"-4" below surrounding walkways or driveways provides a simple but quite functional landscape design element. Detain and retain runoff throughout the site. On flatter sites, smaller Structural BMPs may be interspersed in landscaped areas among the buildings and paving. On hillside sites, drainage from upper areas may be collected in conventional catch basins and piped to landscaped areas and LID BMPs and/or Hydrologic Control BMPs in lower areas. Low retaining walls may also be used to create terraces that can accommodate LID BMPs. Wherever possible, direct drainage from landscaped slopes offsite and not to impervious surfaces like parking lots. Reduce curb maintenance and provide for allowances for curb cuts. Design landscaped areas or other pervious areas to receive and infiltrate runoff from nearby impervious areas. Use Tree Wells to intercept, infiltrate, and evapotranspire precipitation and runoff before it reaches structural BMPs. Tree wells can be used to limit the size of Drainage Management Areas that must be treated by structural BMPs. Guidelines for Tree Wells are included in the Tree Well Fact Sheet in the LID BMP Design Handbook. 	
Discuss how this was included or provide a discussion/justification for "No" or "N/A" answer. Due to the size of the project, runoff from all impervious areas are directed or piped directly to the proposed biofiltration basin or porous pavers.		
	Did you utilize native or drought tolerant species in site landscaping?	
🛛 Yes 🗌 No 🗌 N/A	Wherever possible, use native or drought tolerant species within site landscaping instead of alternatives. These plants are uniquely suited to local soils and climate and can reduce the overall demands for potable water use associated with irrigation.	
	ncluded or provide a discussion/justification for "No" or "N/A" answer. caping will be utilized where appropriate to the maximum extent practical.	

	Project- Specific WQMP Site Design BMP Checklist		
	Did implement harvest and use of runoff?		
	Under the Regional MS4 Permit, Harvest and Use BMPs must be employed to reduce runoff on any site where they are applicable and feasible. However, Harvest and Use BMPs are effective for retention of stormwater runoff only when there is adequate demand for non-potable water during the wet season. If demand for non-potable water is not sufficiently large, the actual retention of stormwater runoff will be diminished during larger storms or during back-to-back storms.		
☐ Yes ⊠ No ☐ N/A	 For the purposes of planning level Harvest and Use BMP feasibility screening, Harvest and Use is only considered to be a feasible if the total average wet season demand for non-potable water is sufficiently large to use the entire DCV within 72 hours. If the average wet season demand for non-potable water is not sufficiently large to use the entire DCV within 72 hours, then Harvest and Use is not considered to be feasible and need not be considered further. The general feasibility and applicability of Harvest and Use BMPs should consider: Any downstream impacts related to water rights that could arise from capturing storm water (not common). Conflicts with recycled water used – where the project is conditioned to use recycled water for irrigation, this should be given priority over storm water capture as it is a year-round supply of water. Code Compliance - If a particular use of captured storm water, and/or available methods for storage of captured storm water would be contrary to building codes in effect at the time of approval of the preliminary Project-Specific WQMP, then an evaluation of harvesting and use for that use would not be required. Wet season demand – the applicant shall demonstrate, to the acceptance of the [Insert Jurisdiction], that there is adequate demand for harvested water during the wet season to drain the system in a reasonable amount of time. 		
	wet season to drain the system in a reasonable amount of time.		
Discuss how this was	included or provide a discussion/justification for "No" or "N/A" answer.		
The project does not generate the required demand needed to make harvest and use feasible for this project.			
	Did you keep the runoff from sediment producing pervious area hydrologically separate from developed areas that require treatment?		
🗌 Yes 🗌 No 🖾 N/A	Pervious area that qualify as self-treating areas or off-site open space should be kept separate from drainage to structural BMPs whenever possible. This helps limit the required size of structural BMPs, helps avoid impacts to sediment supply, and helps reduce clogging risk to BMPs.		
Discuss how this was	included or provide a discussion/justification for "No" or "N/A" answer.		
This project will not result in any sediment producing pervious areas.			

Section C: Delineate Drainage Management Areas (DMAs)

This section provides streamlined guidance and documentation of the DMA delineation and categorization process, for additional information refer to the procedure in Section 3.3 of the SMR WQMP which discusses the methods of delineating and mapping your project site into individual DMAs. Complete Steps 1 to 4 to successfully delineate and categorize DMAs.

Step 1: Identify Surface Types and Drainage Pathways

Carefully delineate pervious areas and impervious areas (including roofs) throughout site and identify overland flow paths and above ground and below ground conveyances. Also identify common points (such as BMPs) that these areas drain to.

Step 2: DMA Delineation

Use the information in Step 1 to divide the entire PDP site into individual, discrete DMAs. Typically, lines delineating DMAs follow grade breaks and roof ridge lines. Where possible, establish separate DMAs for each surface type (e.g., landscaping, pervious paving, or roofs). Assign each DMA a unique code and determine its size in square feet. The total area of your site should total the sum of all of your DMAs (unless water from outside the project limits comingles with water from inside the project limits, i.e. run-on). Complete Table C-1

DMA Name or Identification	Surface Type(s) ¹	Area (Sq. Ft.)	DMA Туре
DMA A	Mixed	90,169	
DMA B	Mixed	9,583	
DMA C	Landscape	3,920	Taha
			To be
			Determined
			in Step 3

Table C-1 DMA Identification

Add Columns as Needed

Step 3: DMA Classification

Determine how drainage from each DMA will be handled by using information from Steps 1 and 2 and by completing Steps 3.A to 3.C. Each DMA will be classified as one of the following four types:

- Type 'A': Self-Treating Areas:
- Type 'C': Areas Draining to Self-Retaining Areas
- Type 'B': Self-Retaining Areas
- Type 'D': Areas Draining to BMPs

Step 3.A – Identify Type 'A' Self-Treating Area

Indicate if the DMAs meet the following criteria by answering "Yes" or "No".

🛛 Yes 🗌 No

Area is undisturbed from their natural condition OR restored with Native and/or California Friendly vegetative covers.

🛛 Yes 🗌 No	Area is irrigated, if at all, with appropriate low water use irrigation systems
	to prevent irrigation runoff.
	Runoff from the area will not comingle with runoff from the developed
🖂 Yes 🗌 No	portion of the site, or across other landscaped areas that do not meet the
	above criteria.

If all answers indicate "Yes," complete Table C-2 to document the DMAs that are classified as Self-Treating Areas.

Table C-2 Type 'A', Self-Treating Areas

DMA Name or Identification	Area (Sq. Ft.)	Stabilization Type	Irrigation Type (if any)
DMA C	3,920	Landscape	Drip or equivalent

Step 3.B – Identify Type 'B' Self-Retaining Area and Type 'C' Areas Draining to Self-Retaining Areas

Type 'B' Self-Retaining Area: A Self-Retaining Area is shallowly depressed 'micro infiltration' areas designed to retain the Design Storm rainfall that reaches the area, without producing any Runoff.

Indicate if the DMAs meet the following criteria by answering "Yes," "No," or "N/A".

🗌 Yes 🗌 No 🖂 N/A	Slopes will be graded toward the center of the pervious area.
Yes 🗌 No 🗌 N/A	Soils will be freely draining to not create vector or nuisance conditions.
🗌 Yes 🗌 No 🔀 N/A	Inlet elevations of area/overflow drains, if any, should be clearly specified to be three inches or more above the low point to promote ponding.
🛛 Yes 🗌 No 🗌 N/A	Pervious pavements (e.g., crushed stone, porous asphalt, pervious concrete, or permeable pavers) can be self-retaining when constructed with a gravel base course four or more inches deep below any underdrain discharge elevation.

If all answers indicate "Yes," DMAs may be categorized as Type 'B', proceed to identify Type 'C' Areas Draining to Self-Retaining Areas.

Type 'C' Areas Draining to Self-Retaining Areas: Runoff from impervious or partially pervious areas can be managed by routing it to Self-Retaining Areas consistent with the LID Principle discussed in SMR WQMP Section 3.2.5 for 'Dispersing Runoff to Adjacent Pervious Areas'.

Indicate if the DMAs meet the following criteria by answering "Yes" or "No".

🛛 Yes 🗌 No

The drainage from the tributary area must be directed to and dispersed within the Self-Retaining Area.

Yes No Area must be designed to retain the entire Design Storm runoff without flowing offsite.

If all answers indicate "Yes," DMAs may be categorized as Type 'C'.

Complete Table C-3 and Table C-4 to identify Type 'B' Self-Retaining Areas and Type 'C' Areas Draining to Self-Retaining Areas.

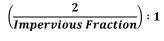
Table C-3 Type 'B', Self-Retaining Area	as
---	----

Self-Retaining Area			Type 'C' DMAs that are draining to the Self-Retaining Area			
DMA	Post-project	Area (square feet)	Storm Depth (inches)		[C] from Table C-4=	Required Retention Depth (inches)
Name/ ID	surface type	[A]	[B]	DMA Name / ID	[C]	$[D] = [B] + \frac{[B] \cdot [C]}{[A]}$
DMA B-1	Porous Pavers	3,459	0.72	dma b-2	5264.5	1.82

Table C-4 Type 'C', Areas that Drain to Self-Retaining Areas

	DMA				Receivin	g Self-Retainii	ng DMA
DMA Name/ ID	Area (square feet)	Post-project surface type	Runoff factor	Product		Area (square feet)	Ratio
	[A]		[B]	[C] = [A] x [B]	DMA name /ID	[D]	[C]/[D]
DMA B-2a	5,166	Asphalt/Concrete	1.0	5,166	DMA B-1	3,459	1.52
DMA B-2b	985	Landscape	0.1	98.5			
				∑ = 5264.5			

<u>Note:</u> (See Section 3.3 of SMR WQMP) Ensure that partially pervious areas draining to a Self-Retaining area do not exceed the following ratio:



(Tributary Area: Self-Retaining Area)

Step 3.C – Identify Type 'D' Areas Draining to BMPs

Areas draining to BMPs are those that could not be fully managed through LID Principles (DMA Types A through C) and will instead drain to an LID BMP and/or a Conventional Treatment BMP designed to manage water quality impacts from that area, and Hydromodification where necessary.

Complete Table C-5 to document which DMAs are classified as Areas Draining to BMPs

Т	able C-5 Type 'D', Areas Draining	to BMPs		
	DMA Name or ID	BMP Name or ID Receiving Runoff from DMA		
DMA A Biofiltration A				

<u>Note</u>: More than one DMA may drain to a single LID BMP; however, one DMA may not drain to more than one BMP.

Section D: Implement LID BMPs

The Regional MS4 Permit requires the use of LID BMPs to provide retention or treatment of the DCV and includes a BMP hierarchy which requires Full Retention BMPs (Priority 1) to be considered before Biofiltration BMPs (Priority 2) and Flow-Through Treatment BMPs and Alternative Compliance BMPs (Priority 3). LID BMP selection must be based on technical feasibility and should be considered early in the site planning and design process. Use this section to document the selection of LID BMPs for each DMA. Note that feasibility is based on the DMA scale and may vary between DMAs based on site conditions.

D.1 Full Infiltration Applicability

An assessment of the feasibility of utilizing full infiltration BMPs is required for all projects, *except where it can be shown that site design LID principals fully retain the DCV (i.e., all DMAs are Type A, B, or C), or where Harvest and Use BMPs fully retain the DCV. Check the following box if applicable:*

Site design LID principals fully retain the DCV (i.e., all DMAs are Type A, B, or C), (Proceed to Section E).

If the above box remains unchecked, perform a site-specific evaluation of the feasibility of Infiltration BMPs using each of the applicable criteria identified in Chapter 2.3.3 of the SMR WQMP and complete the remainder of Section D.1.

Geotechnical Report

A Geotechnical Report or Phase I Environmental Site Assessment may be required by the Copermittee to confirm present and past site characteristics that may affect the use of Infiltration BMPs. In addition, the Copermittee, at their discretion, may not require a geotechnical report for small projects as described in Chapter 2 of the SMR WQMP. If a geotechnical report has been prepared, include it in Appendix 3. In addition, if a Phase I Environmental Site Assessment has been prepared, include it in Appendix 4.

Infiltration Feasibility

Table D-1 below is meant to provide a simple means of assessing which DMAs on your site support Infiltration BMPs and is discussed in the SMR WQMP in Chapter 2.3.3. Check the appropriate box for each question and then list affected DMAs as applicable. If additional space is needed, add a row below the corresponding answer. Table D-1 Infiltration Feasibility

able D-1 Infiltration Feasibility		
Downstream Impacts (SMR WQMP Section 2.3.3.a)		
Does the project site	YES	NO
have any DMAs where infiltration would negatively impact downstream water rights or other Beneficial Uses ³ ?		Х
If Yes, list affected DMAs:		
Groundwater Protection (SMR WQMP Section 2.3.3.b)		
Does the project site	YES	NO
have any DMAs with industrial, and other land uses that pose a high threat to water quality, which cannot be		Х
treated by Bioretention BMPs? Or have DMAs with active industrial process areas?		
If Yes, list affected DMAs:		
have any DMAs with a seasonal high groundwater mark shallower than 10 feet?		Х
If Yes, list affected DMAs:		
have any DMAs located within 100 feet horizontally of a water supply well?		Х
If Yes, list affected DMAs:		
have any DMAs that would restrict BMP locations to within a 2:1 (horizontal: vertical) influence line extending		X
from any septic leach line?		
If Yes, list affected DMAs:	ļ	
have any DMAs been evaluated by a licensed Geotechnical Engineer, Hydrogeologist, or Environmental Engineer,		х
who has concluded that the soils do not have adequate physical and chemical characteristics for the		
protection of groundwater, and has treatment provided by amended media layers in Bioretention BMPs been		
considered in evaluating this factor?		
If Yes, list affected DMAs:		
Public Safety and Offsite Improvements (SMR WQMP Section 2.3.3.c)	VEC	
Does the project site	YES	NO
have any areas identified by the geotechnical report as posing a public safety risk where infiltration of stormwater		X
could have a negative impact?		
If Yes, list affected DMAs:		
Infiltration Characteristics For LID BMPs (SMR WQMP Section 2.3.3.d)	1/50	
Does the project site	YES	NO
have factored infiltration rates of less than 0.8 inches / hour?	Х	
(Note: on a case-by-case basis, the City may allow a factor of safety as low as 1.0 to support selection of full		
infiltration BMPs. Therefore, measured infiltration rates could be as low as 0.8 in/hr to support full infiltration. A		
higher factor of safety would be required for design in accordance with the LID BMP Deign Handbook).		
If Yes, list affected DMAs: DMA A & B were determined to have rates below 0.8 in/hr based on infiltration		
Cut/Fill Conditions (SMR WQMP Section 2.3.3.e)		
Does the project site	YES	NO
have significant cut and/or fill conditions that would preclude in-situ testing of infiltration rates at the final		Х
infiltration surface?		^
If Yes, list affected DMAs:		L
Other Site-Specific Factors (SMR WQMP Section 2.3.3.f)		
Does the project site	YES	NO
have DMAs where the geotechnical investigation discovered other site-specific factors that would preclude		X
effective and/or safe infiltration?		^
Describe here:		

If you answered "Yes" to any of the questions above for any DMA, Infiltration BMPs that rely solely on infiltration should not be used for those DMAs and you should proceed to the assessment for Biofiltration

³ Such a condition must be substantiated by sufficient modeling to demonstrate an impact and would be subject to [Insert Jurisdiction] discretion. There is not a standardized method for assessing this criterion. Water rights evaluations should be site-specific.

BMPs below. Biofiltration BMPs that provide partial infiltration may still be feasible and should be assessed in Section D.2. Summarize concerns identified in the Geotechnical Report, if any, that resulted in a "YES" response above in the table below.

DED-2 Geoteennear concerns for onsite initiation					
Type of Geotechnical Concern	DMAs Feasible (By Name or ID)	DMAs Infeasible (By Name or ID)			
Collapsible Soil	А, В & С				
Expansive Soil	А, В & С				
Slopes	А, В & С				
Liquefaction	А, В & С				
Other					

Table D-2	Geotechnical	Concerns for	Onsite Infiltration

D.2 Biofiltration Applicability

This section should document the applicability of biofiltration BMPs for Type D DMAs that are not feasible for full infiltration BMPs. The key decisions to be documented in this section include:

- 1. Are biofiltration BMPs with partial infiltration feasible?
 - a. Biofiltration BMPs must be designed to maximize incidental infiltration via a partial infiltration design unless it is demonstrated that this design is not feasible.
 - b. These designs can be used at sites with low infiltration rates where other feasibility factors do not preclude incidental infiltration.

Document summary in Table D-3.

- 2. If not, what are the factors that require the use of biofiltration with no infiltration? This may include:
 - a. Geotechnical hazards
 - b. Water rights issues
 - c. Water balance issues
 - d. Soil contamination or groundwater quality issues
 - e. Very low infiltration rates (factored rates < 0.1 in/hr)
 - f. Other factors, demonstrated to the acceptance of the City

If this applies to any DMAs, then rationale must be documented in Table D-3.

- 3. Are biofiltration BMPs infeasible?
 - a. If yes, then provide a site-specific analysis demonstrating the technical infeasibility of all LID BMPs has been performed and is included in Appendix 5. If you plan to submit an analysis demonstrating the technical infeasibility of LID BMPs, request a pre-submittal meeting with the Copermittee with jurisdiction over the Project site to discuss this option. Proceed to Section F to document your alternative compliance measures.

Table D-3	Evaluation	of Biofiltration	BMP Feasibility
-----------	------------	------------------	-----------------

	Is Partial/ Incidental	
	Infiltration Allowable?	Basis for Infeasibility of Partial Infiltration (provide summary and
DMA ID	(Y/N)	include supporting basis if partial infiltration not feasible)
DMA A	Y	

Proprietary Biofiltration BMP Approval Criteria

If the project will use proprietary BMPs as biofiltration BMPs, then this section is completed to document that the proprietary BMPs are selected in accordance with Section 2.3.7 of the SMR WQMP. Proprietary Biofiltration BMPs must meet both of the following approval criteria:

- 1. Approval Criteria for All Proprietary BMPs, and
- 2. Acceptance Criteria for Proprietary Biofiltration BMPs.

When the use of proprietary biofiltration BMPs is proposed to meet the Pollutant Control performance standards, use Table D-4 to document that appropriate approval criteria have been met for the proposed BMPs. Add additional rows to document approval criteria are met for each type of BMP proposed.

Proposed Proprietary Biofiltration BMP	Approval Criteria	Notes/Comments
	Proposed BMP has an active TAPE GULD Certification for the project pollutants of concern ⁴ or equivalent 3 rd party demonstrated performance.	Insert text here
	The BMP is used in a manner consistent with manufacturer guidelines and conditions of its third-party certification.	Insert text here
Insert BMP Name and Manufacturer Here	The BMP includes biological features including vegetation supported by engineered or other growing media.	Describe features here.
	The BMP is designed to maximize infiltration, or supplemental infiltration is provided to achieve retention equivalent to Biofiltration with Partial Infiltration BMPs if factored infiltration rate is between 0.1 and 0.8 inches/hour.	Describe supplemental retention practices if applicable.

Table D-4 Proprietary BMP Approval Requirement Summary

⁴ Use Table F-1 and F-2 to identify and document the pollutants of concern and include these tables in Appendix 5.

report,

The BMP is sized using one of two Biofiltration LID sizing options in Section	List sizing method used, resulting size (i.e. volume or flow), and provided size
2.3.2 of the SRM WQMP.	(for proposed unit)

D.3 Feasibility Assessment Summaries

From the Infiltration, Biofiltration with Partial Infiltration and Biofiltration with No Infiltration Sections above, complete Table D-5 below to summarize which LID BMPs are technically feasible, and which are not, based upon the established hierarchy.

		2. Biofiltration	3. Biofiltration	No LID (Alternative
		with Partial	with No	Compliance)
DMA Name/ID	1. Infiltration	Infiltration	Infiltration	
DMA A		\square		
DMA B	\square	\square		

For those DMAs where LID BMPs are not feasible, provide a narrative in Table D-6 below summarizing why they are not feasible, include your technical infeasibility criteria in Appendix 5, and proceed to Section F below to document Alternative Compliance measures for those DMAs. Recall that each proposed DMA must pass through the LID BMP hierarchy before alternative compliance measures may be considered.

This is based on the clarification letter titled "San Diego Water Board's Expectations of Documentation to Support a Determination of Priority Development Project Infiltration Infeasibility" (April 28, 2017, Via email from San Diego Regional Water Quality Control Board to San Diego County Municipal Storm Water Copermittees⁵).

copi	ermittees j.	
Table	D-6 Summary of Infeasibility Docu	umentation
		Narrative Summary (include reference to applicable appendix/attachment/
	Question	as applicable)
a)	When in the entitlement	N/A
	process did a	
	geotechnical engineer	
	analyze the site for	
	infiltration feasibility?	
b)	When in the entitlement	N/A
	process were other	
	investigations conducted	
	(e.g., groundwater	
	quality, water rights) to	

⁵ http://www.projectcleanwater.org/download/pdp-infiltration-infeasibility/

	evaluate infiltration	
	feasibility?	
c)	What was the scope and results of testing, if conducted, or rationale for why testing was not needed to reach findings?	N/A
d)	What public health and safety requirements affected infiltration locations?	N/A
e)	What were the conclusions and recommendations of the geotechnical engineer and/or other professional responsible for other investigations?	N/A
f)	What was the history of design discussions between the permittee and applicant for the proposed project, resulting in the final design determination related locations feasible for infiltration?	N/A
g)	What site design alternatives were considered to achieve infiltration or partial infiltration on site?	N/A
h)	What physical impairments (i.e., fire road egress, public safety considerations, utilities) and public safety concerns influenced site layout and infiltration feasibility?	N/A
i)	What LID Principles (site design BMPs) were included in the project site design?	N/A

D.4 LID BMP Sizing

Each LID BMP must be designed to ensure that the DCV will be captured by the selected BMPs with no discharge to the storm drain or surface waters during the DCV size storm. Infiltration BMPs must at minimum be sized to capture the DCV to achieve pollutant control requirements.

Biofiltration BMPs must at a minimum be sized to:

- Treat 1.5 times the DCV not reliably retained on site using a volume-base or flow-based sizing method, or
- Include static storage volume, including pore spaces and pre-filter detention volume, at least 0.75 times the portion of the DCV not reliably retained on site.

First, calculate the DCV for each LID BMP using the V_{BMP} worksheet in Appendix F of the LID BMP Design Handbook. Second, design the LID BMP to meet the required V_{BMP} using the methods included in Section 3 of the LID BMP Design Handbook. Utilize the worksheets found in the LID BMP Design Handbook or consult with the Copermittee to assist you in correctly sizing your LID BMPs. Use Table D-7 below to document the DCV for each LID BMP. Provide the completed design procedure sheets for each LID BMP in Appendix 6. You may add additional rows to the table below as needed.

DMA Type/ID DMA A	DMA (square feet) [A]	Post-Project Surface Type	Effective Impervious Fraction, I _f [B]	DMA Runoff Factor [C]	DMA Areas x Runoff Factor [A] x [C]	Biofiltro	ntion Basin A	
DMA A-1	81,152	Asphalt/Concrete	1.0	0.89	72225.28			
DMA A-2	9,017	Landscape	0.1	0.11	991.87			
						Design Storm Depth (in)	DCV, V_{BMP} (cubic feet)	Proposed Volume on Plans (cubic feet)
	90,169				73217.15 Σ= [D]	0.72 [E]	4,058 ft ³ [F] = $\frac{[D]x[E]}{12}$	4,548.8 ft ³ [G]

 Table D-7 DCV Calculations for LID BMPs

[B], [C] is obtained as described in Section 2.6.1.b of the SMR WQMP

[E] is obtained from Exhibit A in the SMR WQMP

[G] is obtained from a design procedure sheet, such as in LID BMP Design Handbook and placed in Appendix 6.

Complete Table D-8 below to document the Design Capture Volume and the Proposed Volume for each LID BMP. You can add rows to the table as needed. Alternatively, the Santa Margarita Hydrology Model (SMRHM) can be used to size LID BMPs to address the DCV and, if applicable, to size Hydrologic Control BMPs to meet the Hydrologic Performance Standard described in the SMR WQMP, as identified in Section E.

Table D-8 LID BMP Sizing

BMP Name /	DMA No.	BMP Type / Description	Design Capture	Proposed Volume
ID			Volume (ft ³)	(ft ³)
Biofiltration A	DMA A	Biofiltration Basin	4,058	4,548.8

If biofiltration will include a capped underdrain, then include sizing calculations demonstrating that the BMP will meet infiltration sizing requirements with the underdrain capped and also meet biofiltration sizing requirements if the underdrain is uncapped.

The project site will treat the pollutants of concern via a biofiltration basin, porous paver self-retaining areas, and landscaped self-treating areas. The biofiltration basin will treat the pollutants of concern via biofiltration through the soil media. The self-retaining porous paver areas are considered a form of micro-infiltration, and therefore treat the pollutants of concern via micro-infiltration. The self-treating area has virtually no impervious area draining to it and consists nearly entirely of landscaped area, therefore the area is deemed self-treating through the landscaped area.

Flows within the biofiltration will either be pumped out of the biofiltration basin (for the water quality flows that biofiltrate through the soil media into the underdrain system) due to the lack of an existing storm drain within proximity of the biofiltration basin, and the depth at which the underdrain system is located. The flows in excess of the required water quality volume (that pond higher than 0.5 feet above the soil media surface) will be conveyed via an outlet structure directly to the outlet storm drain. This storm drain then discharges into a proposed concrete channel located within the Mission Trail right-of-way. The porous pavers will be designed to provide the minimum 3" ponding below the perforated outlet pipe to adhere to the self-retaining area standards. Flows from the underdrain and peak surface flows will be conveyed to the U Channel along the southerly boundary of the project site. This channel will convey flows to a rip rap dispersion area in which flows will be dispersed in a sheet flow manner, mimicking the existing condition flows.

The project site uses LID principles, which includes the biofiltration basin, porous pavers/self-retaining areas, and self-treating areas.

Lake Elsinore is evaporating faster than runoff from natural precipitation can recharge it. Requiring infiltration of 85% of runoff events for projects tributary to Lake Elsinore would only exacerbate current water quality problems associated with Pollutant concentration due to lake water evaporation. In cases where rainfall events have low potential to recharge Lake Elsinore (i.e. no hydraulic connection between groundwater to Lake Elsinore, or other factors), requiring infiltration of Urban Runoff from projects is

counterproductive to the overall watershed goals. Project proponents, in these cases, would be allowed to discharge Urban Runoff, provided they used equally effective filtration-based BMPs.

Section E: Implement Hydrologic Control BMPs and Sediment **Supply BMPs**

If a completed Table 1.2 demonstrates that the project is exempt from Hydromodification Performance Standards, specify N/A and proceed to Section G.



N/A Project is Exempt from Hydromodification Performance Standards.

If a PDP is not exempt from hydromodification requirements than the PDP must satisfy the requirements of the performance standards for hydrologic control BMPs and Sediment Supply BMPs. The PDP may choose to satisfy hydrologic control requirements using onsite or offsite BMPs (i.e. Alternative Compliance). Sediment supply requirements cannot be met via alternative compliance. If N/A is not selected above, select one of the two options below and complete the applicable sections.

Project is Not Hydromodification Exempt and chooses to implement Hydrologic Control and Sediment Supply BMPs Onsite (complete Section E).

Project is Not Hydromodification Exempt and chooses to implement Hydrologic Control Requirements using Alternative Compliance (complete Section F). Selection of this option must be approved by the Copermittee.

E.1 Hydrologic Control BMP Selection

Capture of the DCV and achievement of the Hydrologic Performance Standard may be met by combined and/or separate structural BMPs. The user should consider the full suite of Hydrologic Control BMPs to manage runoff from the post-development condition and meet the Hydrologic Performance Standard identified in this section.

The Hydrologic Performance Standard consists of matching or reducing the flow duration curve of postdevelopment conditions to that of pre-existing, naturally occurring conditions, for the range of geomorphically significant flows (10% of the 2-year runoff event up to the 10-year runoff event). Select each of the hydrologic control BMP types that are applied to meet the above performance standard on the site.

- LID principles as defined in Section 3.2 of the SMR WQMP.

Structural LID BMPs that may be modified or enlarged, if necessary, beyond the DCV.

Structural Hydrologic Control BMPs that are distinct from the LID BMPs above. The LID BMP Design Handbook provides information not only on Hydrologic Control BMP design, but also on BMP design to meet the combined LID requirement and Hydrologic Performance Standard. The Handbook specifies the type of BMPs that can be used to meet the Hydrologic Performance Standard.

E.2 Hydrologic Control BMP Sizing

Hydrologic Control BMPs must be designed to ensure that the flow duration curve of the postdevelopment DMA will not exceed that of the pre-existing, naturally occurring, DMA for the range of geomorphically significant flows. Using SMRHM, (or another acceptable continuous simulation model if approved by the Copermittee) the applicant shall demonstrate that the performance of the Hydrologic Control BMPs complies with the Hydrologic Performance Standard. Complete Table E-1 below and identify, for each DMA, the type of Hydrologic Control BMP, if the SMRHM model confirmed the management (Identified as "passed" in SMRHM), the total volume capacity of the Hydrologic Control BMP, the Hydrologic Control BMP footprint at top floor elevation, and the drawdown time of the Hydrologic Control BMP. SMRHM summary reports should be documented in Appendix 7. Refer to the SMRHM Guidance Document for additional information on SMRHM. You can add rows to the table as needed.

 Table E-1
 Hydrologic Control BMP Sizing

BMP	DMA	BMP Type / Description	SMRHM	BMP Volume	BMP	Drawdown
Name / ID	No.		Passed	(ac-ft)	Footprint (ac)	time (hr)

*Hydromodification calculations were determined using the HydroMod Spreadsheet that was developed by Riverside County Transportation Department. Print out of the results are included in Appendix 7.

If a bioretention BMP with capped underdrain is used and hydromodification requirements apply, then sizing calculations must demonstrate that the BMP meets flow duration control criteria with the underdrain capped and uncapped. Both calculations must be included.

E.3 Implement Sediment Supply BMPs

The sediment supply performance standard applies to PDPs for which hydromodification applied that have the potential to impact Potential Critical Coarse Sediment Yield Areas. Refer to Exhibit G of the WQMP to determine if there are onsite Potential Critical Coarse Sediment Yield Areas or Potential Sediment Source Areas. Select one of the two options below and include the Potential Critical Coarse Sediment Yield Area Exhibit showing your project location in Appendix 7.

- There are no mapped Potential Critical Coarse Sediment Yield Areas or Potential Sediment Source Areas on the site. The Sediment Supply Performance Standard is met with no further action.
- There are mapped Potential Critical Coarse Sediment Yield Areas or Potential Sediment Source Areas on the site, the Sediment Supply Performance Standard will be met through Option 1 or Option 2 below.

The applicant may refer to Section 3.6.4 of the SMR WQMP for a description of the methodology to meet the Sediment Supply Performance Standard. Select the applicable compliance pathway and complete the

appropriate sections to demonstrate compliance with the Sediment Supply Performance Standard if the second box is selected above:

- Avoid impacts related to any PDP activities to Potential Critical Coarse Sediment Yield Areas. Proceed to Section E.3.1.
- Complete a Site-Specific Critical Coarse Sediment Analysis. Proceed to Section E.3.2.

E.3.1 Option 1: Avoid Potential Critical Coarse Sediment Yield Areas and Potential Sediment Source Areas

The simplest approach for complying with the Sediment Supply Performance Standard is to avoid impacts to areas identified as Potential Critical Coarse Sediment Yield Areas or Potential Sediment Supply Areas. If a portion of PDP is identified as a Potential Critical Coarse Sediment Yield Area or a Potential Sediment Source Area, that PDP may still achieve compliance with the Sediment Supply Performance Standards if Potential Critical Coarse Sediment Yield Areas are avoided, i.e. areas are not developed and thereby delivery of Critical Coarse Sediment to the receiving waters is not impeded by site developments.

Provide a narrative describing how the PDP has avoided impacts to Potential Critical Coarse Sediment Yield Areas and/or Potential Sediment Source Areas below.

Insert narrative description here

If it is not feasible to avoid these areas, proceed to Option 2 to complete a Site-Specific Critical Coarse Sediment Analysis.

E.3.2 Option 2: Site-Specific Critical Coarse Sediment Analysis

Perform a stepwise assessment to ensure the maintenance of the pre-project source(s) of Critical Coarse Sediment (i.e., Bed Sediment Supply):

- 1. Determine whether the site or a portion of the site is a Significant Source of Bed Sediment Supply to the Receiving Channel (i.e., an actual verified Critical Coarse Sediment Yield Area);
- 2. Avoid areas identified as actual verified Critical Coarse Sediment Yield Areas in the PDP design and maintain pathways for discharge of Bed Sediment Supply from these areas to receiving waters.

Step 1: Identify if the site is an actual verified Critical Coarse Sediment Yield Area supplying Bed Sediment Supply to the receiving channel

Step 1.A – Is the Bed Sediment of onsite streams similar to that of receiving streams?

Rate the similarity:	🗌 High
	🗌 Medium
	Low

Results from the geotechnical and sieve analysis to be performed both onsite and in the receiving channel should be documented in Appendix 7. Of particular interest, the results of the sieve

analysis, the soil erodibility factor, a description of the topographic relief of the project area, and the lithology of onsite soils should be reported in Appendix 7.

□ **Step 1.B** – Are onsite streams capable of delivering Bed Sediment Supply from the site, if any, to the receiving channel?

Rate the potential:	🗌 High
	🗌 Medium
	Low

Results from the analyses of the sediment delivery potential to the receiving channel should be documented in Appendix 7 and identify, at a minimum, the Sediment Source, the distance to the receiving channel, the onsite channel density, the project watershed area, the slope, length, land use, and rainfall intensity.

Step 1.C – Will the receiving channel adversely respond to a change in Bed Sediment Load?

Rate the need for bed sediment supply:

High
Medium
Low

Results from the in-stream analysis to be performed both onsite should be documented in Appendix 7. The analysis should, at a minimum, quantify the bank stability and the degree of incision, provide a gradation of the Bed Sediment within the receiving channel, and identify if the channel is sediment supply-limited.

Step 1.D – Summary of Step 1

Summarize in Table E.3 the findings of Step 1 and associate a score (in parenthesis) to each step. The sum of the three individual scores determines if a stream is a significant contributor to the receiving stream.

- Sum is equal to or greater than eight Site is a significant source of sediment bed material

 all on-site streams must be preserved or by-passed within the site plan. The applicant
 shall proceed to Step 2 for all onsite streams.
- Sum is greater than five but lower than eight. Site is a source of sediment bed material some of the on-site streams must be preserved (with identified streams noted). The applicant shall proceed to Step 2 for the identified streams only.
- Sum is equal to or lower than five. Site is not a significant source of sediment bed material. The applicant may advance to Section F.

Table E-2 Triad Step	Assessment Summary Rating				
1.A	🗌 High (3)	🗌 Medium (2)	🗌 Low (1)		
1.B	🗌 High (3)	🗌 Medium (2)	🗌 Low (1)		
1.C	High (3)	🗌 Medium (2)	🗌 Low (1)		
Significant					

Step 2: Avoid Development of Critical Coarse Sediment Yield Areas, Potential Sediment Sources Areas, and Preserve Pathways for Transport of Bed Sediment Supply to Receiving Waters

Onsite streams identified as a actual verified Critical Coarse Sediment Yield Areas should be avoided in the site design and transport pathways for Critical Coarse Sediment should be preserved

Check those that apply:

The site design does avoid all onsite channels identified as actual verified Critical Coarse Sediment Yield Areas

AND

The drainage design bypasses flow and sediment from onsite upstream drainages identified as actual verified Critical Coarse Sediment Yield Areas to maintain Critical Coarse Sediment supply to receiving waters

(If both are yes, the applicant may disregard subsequent steps of Section E.3 and directly advance directly to Section G).

- Or -

The site design **does NOT avoid** all onsite channels identified as actual verified Critical Coarse Sediment Yield Areas

OR

The project impacts transport pathways of Critical Coarse Sediment from onsite upstream drainages.

(If either of these are the case, the applicant may proceed with the subsequent steps of Section E.3).

Provide in Appendix 7 a site map that identifies all onsite channels and highlights those onsite channels that were identified as a Significant Source of Bed Sediment. The site map shall demonstrate, if feasible, that the site design avoids those onsite channels identified as a Significant Source of Bed Sediment. In addition, the applicant shall describe the characteristics of each onsite channel identified as a Significant Source of Bed Sediment. If the design plan cannot avoid the onsite channels, please provide a rationale for each channel individually.

The site map shall demonstrate that the drainage design bypasses those onsite channels that supply Critical Coarse Sediment to the receiving channel(s). In addition, the applicant shall describe the characteristics of each onsite channel identified as an actual verified Critical Coarse Sediment Yield Area.

Identified Channel #1 - Insert narrative description here

Identified Channel #2 - Insert narrative description here Identified Channel #3 - Insert narrative description here

E.3.3 Sediment Supply BMPs to Result in No Net Impact to Downstream Receiving Waters

If impacts to Critical Coarse Sediment Yield Areas cannot be avoided, sediment supply BMPs must be implemented such there is no net impact to receiving waters. Sediment supply BMPs may consist of approaches that permit flux of bed sediment supply from Critical Coarse Sediment Yield Areas within the project boundary. This approach is subject to acceptance by the [Insert Jurisdiction]. It may require extensive documentation and analysis by qualified professionals to support this demonstration.

Appendix H of the San Diego Model BMP Design Manual provides additional information on site-specific investigation of Critical Coarse Sediment Supply areas.

http://www.projectcleanwater.org/download/2018-model-bmp-design-manual/

If applicable, insert narrative description here

Documentation of sediment supply BMPs should be detailed in Appendix 7.

Section F: Alternative Compliance

SECTION NOT APPLICABLE

Alternative Compliance may be used to achieve compliance with pollutant control and/or hydromodification requirements for a given PDP. Alternative Compliance may be used under two scenarios, check the applicable box if the PDP is proposing to use Alternative Compliance to satisfy all or a portion of the Pollutant Control and/or Hydrologic Control requirements (but not sediment supply requirements)

- ☐ If it is not feasible to fully implement Infiltration or Biofiltration BMPs at a PDP site, Flow-Through Treatment Control BMPs may be used to treat pollutants contained in the portion of DCV not reliably retained on site and Alternative Compliance measures must also be implemented to mitigate for those pollutants in the DCV that are not retained or removed on site prior to discharging to a receiving water.
- Alternative Compliance is selected to comply with either pollutant control or hydromodification flow control requirements even if complying with these requirements is potentially feasible on-site. If such voluntary Alternative Compliance is implemented, Flow-Through Treatment Control BMPs must still be used to treat those pollutants in the portion of the DCV not reliably retained on site prior to discharging to a receiving water.

Refer to Section 2.7 of the SMR WQMP and consult the City for currently available Alternative Compliance pathways. Coordinate with the Copermittee if electing to participate in Alternative Compliance and complete the sections below to document implementation of the Flow-Through BMP component of the program.

F.1 Identify Pollutants of Concern

The purpose of this section is to help you appropriately plan for mitigating your Pollutants of Concern in lieu of implementing LID BMPs and to document compliance and.

Utilize

Table A-1 from Section A, which noted your project's Receiving Waters, to identify impairments for Receiving Waters (including downstream receiving waters) by completing Table F-1. Table F-1 includes the watersheds identified as impaired in the Approved 2010 303(d) list; check box corresponding with the PDP's receiving water. The most recent 303(d) lists are available from the State Water Resources Control Board website:

https://www.waterboards.ca.gov/water_issues/programs/tmdl/integrated2010.shtml).https://www.waterboards.ca.gov/water_issues/programs/tmdl/integrated2010.shtml.

er Body	Nutrients ¹	Metals ²	Toxicity	Bacteria and Pathogens	Pesticides and Herbicides	Sulfate	Total Dissolved Solids
De Luz Creek	Х	Х				Х	
Long Canyon Creek		Х		Х	Х		
Murrieta Creek	Х	Х	Х		Х		
Redhawk Channel	Х	Х		Х	Х		Х
Santa Gertudis Creek	Х	Х		Х	Х		
Santa Margarita Estuary	Х						
Santa Margarita River (Lower)	Х			Х			
Santa Margarita River (Upper)	Х		Х				
Temecula Creek	Х	Х	Х		Х		Х
Warm Springs Creek	Х	Х		Х	Х		

 Table F-1 Summary of Approved 2010 303(d) listed waterbodies and associated pollutants of concern for the Riverside County

 SMR Region and downstream waterbodies.

¹Nutrients include nitrogen, phosphorus and eutrophic conditions caused by excess nutrients.

² Metals includes copper, iron, and manganese.

Use Table F-2 to identify the pollutants identified with the project site. Indicate the applicable PDP Categories and/or Project Features by checking the boxes that apply. If the identified General Pollutant Categories are the same as those listed for your Receiving Waters, then these will be your Pollutants of Concern; check the appropriate box or boxes in the last row.

	Table F-2 Potential Pollutants by Land Use Type										
	Priority Development	General P	ollutant (Categories							
	roject Categories and/or lect Features (check those that apply)	Bacterial Indicators	Metals	Nutrients	Pesticides	Toxic Organic Compounds	Sediments	Trash & Debris	Oil & Grease	Total Dissolved Solids	Sulfate
	Detached Residential Development	Р	N	Ρ	Ρ	Ν	Ρ	Ρ	Ρ	N	Ν
	Attached Residential Development	Р	N	Ρ	Ρ	Ν	Ρ	Ρ	P ⁽²⁾	N	Ν
	Commercial/Industrial Development	P ⁽³⁾	P ⁽⁷⁾	P ⁽¹⁾	P ⁽¹⁾	Р	P ⁽¹⁾	Ρ	Р	N	N
	Automotive Repair Shops	N	Р	Ν	N	P ^(4, 5)	N	Ρ	Р	N	N
	Restaurants (>5,000 ft²)	Р	N	Ν	P ⁽¹⁾	N	N	Ρ	Р	N	N
	Hillside Development (>5,000 ft ²)	Р	N	Ρ	Р	Ν	Ρ	Ρ	Р	N	N
	Parking Lots (>5,000 ft²)	P ⁽⁶⁾	P ⁽⁷⁾	P ⁽¹⁾	P ⁽¹⁾	P ⁽⁴⁾	Ρ	Ρ	Р	N	N
	Streets, Highways, and Freeways	P ⁽⁶⁾	P ⁽⁷⁾	P ⁽¹⁾	P ⁽¹⁾	P ⁽⁴⁾	Ρ	Ρ	Р	N	N
	Retail Gasoline Outlets	Ν	P ⁽⁷⁾	Ν	Ν	P ⁽⁴⁾	Ν	Р	Р	Ν	Ν
Р	Project Priority ollutant(s) of Concern										

Table F-2 Potential Pollutants by Land Use Type

P = Potential

N = Not Potential

⁽¹⁾ A potential Pollutant if non-native landscaping exists or is proposed onsite; otherwise not expected

⁽²⁾ A potential Pollutant if the project includes uncovered parking areas; otherwise not expected

⁽³⁾ A potential Pollutant is land use involving animal waste products; otherwise not expected

⁽⁴⁾ Including petroleum hydrocarbons

⁽⁵⁾ Including solvents

⁽⁶⁾ Bacterial indicators are routinely detected in pavement runoff

⁽⁷⁾ A potential source of metals, primarily copper and zinc. Iron, magnesium, and aluminum are commonly found in the environment and are commonly associated with soils, but are not primarily of anthropogenic stormwater origin in the municipal environment.

F.2 Treatment Control BMP Selection

Treatment Control BMPs typically provide proprietary treatment mechanisms to treat potential Pollutants in runoff, but do not sustain significant biological processes. Treatment Control BMPs must be selected to address the Project Priority Pollutants of Concern (identified above) and meet the acceptance criteria described in Section 2.3.7 of the SMR WQMP. Documentation of acceptance criteria must be included in Appendix 6. In addition, ensure that proposed Treatment Control BMPs are properly identified on the WQMP Site Plan in Appendix 1.

Table F-3 Treatment Control BMP Selection

Selected Treatment Control BMP Name or ID ¹	Priority Pollutant(s) of Concern to Mitigate ²	Removal Efficiency Percentage ³

¹ Treatment Control BMPs must not be constructed within Receiving Waters. In addition, a proposed Treatment Control BMP may be listed more than once if they possess more than one qualifying pollutant removal efficiency.

² Cross Reference Table E.1 above to populate this column.

³ As documented in a Copermittee Approved Study and provided in Appendix 6.

F.3 Sizing Criteria

Utilize Table F-4 below to appropriately size flow-through BMPs to the DCV, or Design Flow Rate, as applicable. Please reference Chapter 3.5.1 of the SMR WQMP for further information.

DMA Type/ID	DMA Area (square feet) [A]	Post- Project Surface Type	Effective Impervious Fraction, I _f [B]	DMA Runoff Factor	DMA Areas x Runoff Factor [A] x [C]		BMP Name / ntifier Here
						Design Storm (in)	Design Flow Rate (cfs)
	$A_T = \Sigma[A]$				Σ= [D]	[E]	$[F] = \frac{[D]x[E]}{[G]}$

[B], [C] is obtained as described in Section 2.6.1.b from the SMR WQMP

[E] either 0.2 inches or 2 times the 85th percentile hourly rainfall intensity [G] = 43,560,.

F.4 Hydrologic Performance Standard – Alternative Compliance Approach

Alternative compliance options are only available if the governing Copermittee has acknowledged the infeasibility of onsite Hydrologic Control BMPs and approved an alternative compliance approach. See Section 3.5 and 3.6 of the SMR WQMP.

Select the pursued alternative and describe the specifics of the alternative:

□ Offsite Hydrologic Control Management within the same channel system

Insert narrative description here

□ In-Stream Restoration Project

Insert narrative description here

For Offsite Hydrologic Control BMP Option

Each Hydrologic Control BMP must be designed to ensure that the flow duration curve of the postdevelopment DMA will not exceed that of the pre-existing, naturally occurring, DMA by more than ten percent over a one-year period. Using SMRHM, the applicant shall demonstrate that the performance of each designed Hydrologic Control BMP is equivalent with the Hydrologic Performance Standard for onsite conditions. Complete Table F-5 below and identify, for each Hydrologic Control BMP, the equivalent DMA the Hydrologic Control BMP mitigates, that the SMRHM model passed, the total volume capacity of the BMP, the BMP footprint at top floor elevation, and the drawdown time of the BMP. SMRHM summary reports for the alternative approach should be documented in Appendix 7. Refer to the SMRHM Guidance Document for additional information on SMRHM. You can add rows to the table as needed.

BMP Name / Type	Equivalent	SMRHM	BMP Volume	BMP	Drawdown
	DMA (ac)	Passed	(ac-ft)	Footprint (ac)	time (hr)

Table F-5 Offsite Hydrologic Control BMP Sizing

For Instream Restoration Option

Attach to Appendix 7 the technical report detailing the condition of the receiving channel subject to the proposed hydrologic and sediment regimes. Provide the full design plans for the in-stream restoration project that have been approved by the Copermittee. Utilize the San Diego Regional Water Quality Equivalency Guidance Document.

Section G: Implement Trash Capture BMPs

SECTION NOT APPLICABLE

The City may require full trash capture BMPs to be installed as part of the project. Consult with the City to determine applicability.

Trash Capture BMPs may be applicable to Type 'D' DMAs, as defined in Section 2.3.4 of the SMR WQMP. Trash Capture BMPs are designed to treat Q_{TRASH} , the runoff flow rate generated during the 1-year 1-hour precipitation depth. Utilize Table G-1 to size Trash Capture BMP. Refer to

Table G-2 to determine the Trash Capture Design Storm Intensity (E).

Table G-1 Sizing Trash Capture BMPs

DMA Type/ID A	DMA Area (square feet) [A]	Post-Project Surface Type	Effective Impervious Fraction, I _f [B]	DMA Runoff Factor	DMA Areas x Runoff Factor [A] × [C]	Enter BMP Na	me / Identifier Here
DMA A-1	81,152	Asphalt/Concrete	1.0	0.89	72225.28		
DMA A-2	9,017	Landscape	0.1	0.11	991.87		
						Trash	
						Capture	Trash Capture
						Design Storm Intensity (in)	Design Flow Rate (cubic feet or cfs)
	90,169				73217.15 Σ= [D]	0.37 [E]	$[F] = \frac{[D]x[E]}{[G]}$

[B], [C] is obtained as described in Section 2.6.1.b from the SMR WQMP

Table G-2 Approximate precipitation depth/intensity values for calculation of the Trash Capture Design Storm

City	1-year 1-hour Precipitation Depth/Intensity (inches/hr)
Murrieta	0.47
Temecula	0.50
Wildomar	0.37

Use Table G-3 to summarize and document the selection and sizing of Trash Capture BMPs.

Т	able G-3 Trash Captur	e BMPs			
				Required Trash	Provided Trash
		DMA		Capture Flowrate	Capture Flowrate
	BMP Name / ID	No(s)	BMP Type / Description	(cfs)	(cfs)
	BIOFILTRATION	DMA A	Biofiltration Basin with partial	0.62	**
	А		infiltration		

** Trash capture will be provided by the Biofiltration Basin. Per the Santa Margarita River 2018 Water Quality Management Plan:

"Full Trash Capture BMPs may include catch basin insert devices, in line high flow devices, and LID BMPs. Full Trash Capture Devices must be certified by the State Water Resources Control Boards and for devices within County of Riverside right-of-way or requiring County of Riverside maintenance, the specific State certified BMP must be approved by County of Riverside. These BMPs are intended to remove all particles larger than 5 mm from runoff during most storm events."

The SMR WQMP references the Fact sheets for all Certified Full Trash Capture BMPs that are available from the State Water Resources Control Board Trash Implementation Website, including Multi-Benefit

Treatment Systems including LID. The State Water Resources Control Boards Certified Multi-Benefit Treatment Systems Complying with Trash Full Capture System Requirements states that the State Water Board Executive Director certifies a Multi-Benefit Treatment System that performs and complies with the following criteria:

- 1. Traps all particles that are 5 mm or greater up to the regional specific design flow or corresponding volume;
- 2. Complies with one of the following trash treatment designs applicable to the Multi-Benefit Treatment System:
 - a. Flow-based design that includes:
 - i. A trash treatment capacity equal to or greater than the peak runoff flow collected during the region specific one-year, one-hour storm event from the applicable drainage area, or
 - ii. A trash treatment capacity equal to or greater than the corresponding flow capacity; or
 - b. Volume-based design that includes a trash treatment capacity that is:
 - i. Equal to or greater than the volumetric sizing criteria for treatment systems in the applicable State or Regional Water Board Storm water permit, and
 - ii. Equal or greater than the volume generated from a one-year, one-hour storm event.
- 3. Incorporates an operation and maintenance plan sufficient to ensure that the captured trash does not migrate from the site; and
- 4. Is constructed per design plans that are stamped and signed by a registered California licensed professional civil engineer.

Based upon the above criteria, the Biofiltration Basin meets these parameters. The unit hydrograph calculation included in Appendix 6 for the 1-year, 1-hour storm event indicates that a total of 2,688 cu. ft. of volume is generated. The Bifiltration Basin has a total surface volume of 3,726 cu. ft., which is more than sufficient volume. This volume was calculated by taking the bottom surface area of 1,863 sq. ft., and multiplying by 0.5' of ponded depth above the soil media (not accounting for side slopes), by 3' and 0.3 to account for the soil media and void raito, and by 1.5' and 0.4 for the gravel layer. The outlet structure within the Biofiltration Basin will include screens that will prevent particles greater than 5 mm from passing. An operations and maintenance plan will be provided during final engineering and will includes specific maintenance activities related to trash capture. All designs will be signed and stamped by a registered civil engineer.

Section H: Source Control BMPs

Source Control BMPs include permanent, structural features that may be required in your Project plans, such as roofs over and berms around trash and recycling areas, and Operational BMPs, such as regular sweeping and "housekeeping," that must be implemented by the site's occupant or user. The Maximum Extent Practicable (MEP) standard typically requires both types of BMPs. In general, Operational Source Control BMPs cannot be substituted for a feasible and effective Structural Source Control BMP. Complete checklist below to determine applicable Source Control BMPs for your site.

Project-Specific WQMP Source Control BMP Checklist

All development projects must implement Source Control BMPs. Source Control BMPs are used to minimize pollutants that may discharge to the MS4. Refer to Chapter 3 (Section 3.8) of the SMR WQMP for additional information. Complete Steps 1 and 2 below to identify Source Control BMPs for the project site.

STEP 1: IDENTIFY POLLUTANT SOURCES

Review project site plans and identify the applicable pollutant sources. "Yes" indicates that the pollutant source is applicable to project site. "No" indicates that the pollutant source is not applicable to project site.

🖂 Yes 🗌 No	Storm Drain Inlets	🗌 Yes 🔀 No	Outdoor storage areas
🗌 Yes 🔀 No	Floor Drains	🗌 Yes 🔀 No	Material storage areas
🗌 Yes 🔀 No	Sump Pumps	🗌 Yes 🔀 No	Fueling areas
🔀 Yes 🗌 No	Pets Control/Herbicide Application	🗌 Yes 🔀 No	Loading Docks
🗌 Yes 🔀 No	Food Service Areas	🔀 Yes 🗌 No	Fire Sprinkler Test/Maintenance water
🔀 Yes 🗌 No	Trash Storage Areas	🔀 Yes 🗌 No	Plazas, Sidewalks and Parking Lots
🗌 Yes 🔀 No	Industrial Processes	🗌 Yes 🔀 No	Pools, Spas, Fountains and other water features
🗌 Yes 🔀 No	Vehicle and Equipment Cleaning and Maintenance/Repair Areas		

STEP 2: REQUIRED SOURCE CONTROL BMPs

List each Pollutant source identified above in column 1 and fill in the corresponding Structural Source Control BMPs and Operational Control BMPs by referring to the Stormwater Pollutant Sources/Source Control Checklist included in Appendix 8. The resulting list of structural and operational source control BMPs must be implemented as long as the associated sources are present on the project site. Add additional rows as needed.

Pollutant Source	Structural Source Control BMP	Operational Source Control BMP
Storm Drain Inlets	Mark "Only Rain Down the Storm Drain" or similar. Catch basin markers may be available from the Riverside County Flood Control and Water Conservation District, call 951.955.1200 to verify.	 Maintain and periodically repaint or replace markers. Provide stormwater pollution prevention information to new site owners, lessees, employees or operators.
Pest Control/Herbicide Application	 Note building design features that discourage entry of pests. Preserve existing native trees, shrubs, and ground cover to the maximum extent practicable. 	 Maintain landscaping using minimum or no pesticides. See applicable operational BMPs in "What you should know for Landscape and Gardening"

	 Design landscaping to minimize irrigation and runoff to promote surface infiltration where appropriate, and to minimize the use of fertilizers and pesticides that can contribute to stormwater pollution. Where landscaped areas are used to retain or detain stormwater, specify plants that are tolerant of saturated soil conditions. Consider using pest-resistant plants, especially adjacent to hardscape. To insure successful establishment, select plans appropriate to site soils, slopes, climate, sun, wind, rain, landuse, air movement, ecological consistency, and plant interactions. 	 Provide Integrated Pest Management information to new owners, lessees and operators
Trash Storage Areas	 Trash receptacles will be covered or closed at all times. Signs will be posted on dumpsters stating "Do not dump hazardous materials here" or similar. 	Provide adequate number of receptacles. Inspect receptacles regularly; repair or replace leaky receptacles. Keep receptacles covered. Prohibit/prevent dumping or liquid or hazardous wastes. Post "no hazardous materials" signs. Inspect and pick up litter daily and clean up spills immediately. Keep spill control materials available on-site. See Fact Sheet SC-34, "Waste Handling and Disposal" in the CASQA Stormwater Quality Handbook at <u>www.cabmphandbooks.com</u>
Fire Sprinkler Test/Maintenance Water	Provide a means to drain fire sprinkler test water to the sanitary sewer.	See the note in Face Sheet SC-41, "Building and Grounds Maintenance," in the CASQA Stormwater Quality Handbooks at <u>www.cabmphandbooks.com</u> .
Plazas, sidewalks and parking lots		Sweep sidewalks regularly to prevent accumulation of litter and debris. Collect debris from pressure washing to prevent entry into storm drain system.

Section I: Coordinate Submittal with Other Site Plans

Populate Table I-1 below to assist the plan checker in an expeditious review of your project. During construction and at completion, City inspectors will verify the installation of BMPs against the approved plans. The first two columns will contain information that was prepared in previous steps, while the last column will be populated with the corresponding plan sheets. This table is to be completed with the submittal of your final Project-Specific WQMP.

Will be provided during Final Engineering

BMP	No. or ID	BMP Identifier and Description	Corresponding Plan Sheet(s)
	A	Biofiltration Basin "A"	WQMP Site Plan
	В	Porous Pavers "B"	WQMP Site Plan

 Table I-1 Construction Plan Cross-reference

Note that the updated table — or Construction Plan WQMP Checklist — is **only a reference tool** to facilitate an easy comparison of the construction plans to your Project-Specific WQMP. The Copermittee with jurisdiction over the Project site can advise you regarding the process required to propose changes to the approved Project-Specific WQMP.

Use Table I-2 to identify other applicable permits that may impact design of the site. If yes is answered to any of the items below, the Copermittee may require proof of approval/coverage from those agencies as applicable including documentation of any associated requirements that may affect this Project-Specific WQMP.

 Table I-2
 Other
 Applicable
 Permits

Agency	Permit Required	
State Department of Fish and Game, 1602 Streambed Alteration Agreement	□ Y	N 🛛
State Water Resources Control Board, Clean Water Act Section 401 Water Quality Certification	□ Y	N 🛛
US Army Corps of Engineers, Clean Water Act Section 404 Permit	□ Y	N 🛛
US Fish and Wildlife, Endangered Species Act Section 7 Biological Opinion	□ Y	N 🛛
Statewide Construction General Permit Coverage	×	□ N
Statewide Industrial General Permit Coverage	□ Y	N 🛛
Western Riverside MSHCP Consistency Approval (e.g., JPR, DBESP)	□ Y	N 🛛
Other (please list in the space below as required)	Υ	□ N

Section J: Operation, Maintenance and Funding

The Copermittee with jurisdiction over the Project site will periodically verify that BMPs on your Project are maintained and continue to operate as designed. To make this possible, the Copermittee will require that you include in Appendix 9 of this Project-Specific WQMP:

- 1. A means to finance and implement maintenance of BMPs in perpetuity, including replacement cost.
- 2. Acceptance of responsibility for maintenance from the time the BMPs are constructed until responsibility for operation and maintenance is legally transferred. A warranty covering a period following construction may also be required.
- 3. An outline of general maintenance requirements for the Stormwater BMPs you have selected.
- 4. Figures delineating and designating pervious and impervious areas, location, and type of Stormwater BMP, and tables of pervious and impervious areas served by each facility. Geolocating the BMPs using a coordinate system of latitude and longitude is recommended to help facilitate a future statewide database system.
- 5. A separate list and location of self-retaining areas or areas addressed by LID Principles that do not require specialized Operations and Maintenance or inspections but will require typical landscape maintenance as noted in Chapter 5, in the SMR WQMP. Include a brief description of typical landscape maintenance for these areas.

The Copermittee with jurisdiction over the Project site will also require that you prepare and submit a detailed BMP Operation and Maintenance Plan that sets forth a maintenance schedule for each of the BMPs built on your site. An agreement assigning responsibility for maintenance and providing for inspections and certification may also be required.

Details of these requirements and instructions for preparing a BMP Operation and Maintenance Plan are in Chapter 5 of the SMR WQMP.

Maintenance Mechanism: Property Owners

Will the proposed BMPs be maintained by a Homeowners' Association (HOA) or Property Owners Association (POA)?



Include your Operation and Maintenance Plan and Maintenance Mechanism in Appendix 9. Additionally, include all pertinent forms of educational materials for those personnel that will be maintaining the proposed BMPs within this Project-Specific WQMP in Appendix 10.

Section K: Acronyms, Abbreviations and Definitions

Regional MS4 Permit	and Order No. R9-2015-0100 an NPDES Permit issued by the San Diego Regional Water Quality Control Board.
Applicant	Public or private entity seeking the discretionary approval of new or replaced improvements from the Copermittee with jurisdiction over the project site. The Applicant has overall responsibility for the implementation and the approval of a Priority Development Project. The WQMP uses consistently the term "user" to refer to the applicant such as developer or project proponent. The WQMP employs also the designation "user" to identify the Registered Professional Civil Engineer responsible for submitting the Project-Specific WQMP, and designing the required BMPs.
Best Management	Defined in 40 CFR 122.2 as schedules of activities, prohibitions of
Practice (BMP)	practices, maintenance procedures, and other management practices to prevent or reduce the pollution of waters of the United States. BMPs also include treatment requirements, operating procedures and practices to control plant site runoff, spillage or leaks, sludge or waste disposal, or drainage from raw material storage. In the case of municipal storm water permits, BMPs are typically used in place of numeric effluent limits.
BMP Fact Sheets	BMP Fact Sheets are available in the LID BMP Design Handbook. Individual BMP Fact Sheets include sitting considerations, and design and sizing guidelines for seven types of structural BMPs (infiltration basin, infiltration trench, permeable pavement, harvest-and-use, bioretention, extended detention basin, and sand filter).
California	Publisher of the California Stormwater Best Management Practices
Stormwater Quality	Handbooks, available at
Association (CASQA)	www.cabmphandbooks.com.
Conventional Treatment Control BMP	volume reduction as LID BMPs, and commonly require more specialized maintenance than LID BMPs. As such, the Regional MS4 Permit and this WQMP require the use of LID BMPs wherever feasible, before Conventional Treatment BMPs can be considered or implemented.
Copermittees	The Regional MS4 Permit identifies the Cities of Murrieta, Temecula, and Wildomar, the County, and the District, as Copermittees for the SMR.
County	The abbreviation refers to the County of Riverside in this document.

CEQA	California Environmental Quality Act - a statute that requires state and local agencies to identify the significant environmental impacts of their actions and to avoid or mitigate those impacts, if feasible.
CIMIS	California Irrigation Management Information System - an integrated network of 118 automated active weather stations all over California managed by the California Department of Water Resources.
CWA	Clean Water Act - is the primary federal law governing water pollution. Passed in 1972, the CWA established the goals of eliminating releases of high amounts of toxic substances into water, eliminating additional water pollution by 1985, and ensuring that surface waters would meet standards necessary for human sports and recreation by 1983. CWA Section 402(p) is the federal statute requiring NPDES permits for discharges from MS4s.
CWA Section 303(d)	Impaired water in which water quality does not meet applicable
Waterbody	water quality standards and/or is not expected to meet water quality standards, even after the application of technology based pollution controls required by the CWA. The discharge of urban runoff to these water bodies by the Copermittees is significant because these discharges can cause or contribute to violations of
	applicable water quality standards.
Design Storm	The Regional MS4 Permit has established the 85th percentile, 24- hour storm event as the "Design Storm". The applicant may refer to Exhibit A to identify the applicable Design Storm Depth (D85) to the project.
DCV	Design Capture Volume (DCV) is the volume of runoff produced from the Design Storm to be mitigated through LID Retention BMPs, Other LID BMPs and Volume Based Conventional Treatment BMPs, as appropriate.
Design Flow Rate	The design flow rate represents the minimum flow rate capacity that flow-based conventional treatment control BMPs should treat to the MEP, when considered.
DCIA	Directly Connected Impervious Areas - those impervious areas that are hydraulically connected to the MS4 (i.e. street curbs, catch basins, storm drains, etc.) and thence to the structural BMP without flowing over pervious areas.
Discretionary	A decision in which a Copermittee uses its judgment in deciding
Approval	whether and how to carry out or approve a project.
District	Riverside County Flood Control and Water Conservation District.
DMA	A Drainage Management Area - a delineated portion of a project site that is hydraulically connected to a common structural BMP or conveyance point. The Applicant may refer to Section 3.3 for further guidelines on how to delineate DMAs.

t a f i	Refers to the amount of time the design volume takes to pass through the BMP. The specified or incorporated drawdown times are to ensure that adequate contact or detention time has occurred for treatment, while not creating vector or other nuisance issues. It is important to abide by the drawdown time requirements stated in the fact sheet for each specific BMP.
	Area which 1) is suitable for a BMP (for example, if infiltration is
r i	potentially feasible for the site based on infeasibility criteria, infiltration must be allowed over this area) and 2) receives runoff from impervious areas.
ESA A	An Environmental Sensitive Area (ESA) designates an area "in
	which plants or animals life or their habitats are either rare or
e	especially valuable because of their special nature or role in an
e	ecosystem and which would be easily disturbed or degraded by
1	numan activities and developments". (Reference: California Public
F	Resources Code § 30107.5).
ET I	Evapotranspiration (ET) is the loss of water to the atmosphere by
t	the combined processes of evaporation (from soil and plant
s	surfaces) and transpiration (from plant tissues). It is also an
i	ndicator of how much water crops, lawn, garden, and trees need
f	for healthy growth and productivity
FAR 7	The Floor Area Ratio (FAR) is the total square feet of a building
Ċ	divided by the total square feet of the lot the building is located
C	on.
Flow-Based BMP H	Flow-based BMPs are conventional treatment control BMPs that
a	are sized to treat the design flow rate.
FPPP F	Facility Pollution Prevention Plan
HCOC I	Hydrologic Condition of Concern - Exists when the alteration of a
s	site's hydrologic regime caused by development would cause
s	significant impacts on downstream channels and aquatic habitats,
a	alone or in conjunction with impacts of other projects.
HMP I	Hydromodification Management Plan – Plan defining Performance
9	Standards for PDPs to manage increases in runoff discharge rates
a	and durations.
Hydrologic Control	BMP to mitigate the increases in runoff discharge rates and
BMP G	durations and meet the Performance Standards set forth in the HMP.
HSG I	Hydrologic Soil Groups - soil classification to indicate the
r	minimum rate of infiltration obtained for bare soil after prolonged
ν.	wetting. The HSGs are A (very low runoff potential/high
i	nfiltration rate), B, C, and D (high runoff potential/very low
i	nfiltration rate)

Hydromodification	The Regional MS4 Permit identifies that increased volume, velocity,
	frequency and discharge duration of storm water runoff from developed areas has the potential to greatly accelerate downstream erosion, impair stream habitat in natural drainages, and negatively impact beneficial uses.
JRMP	A separate Jurisdictional Runoff Management Plan (JRMP) has been developed by each Copermittee and identifies the local programs and activities that the Copermittee is implementing to meet the Regional MS4 Permit requirements.
LID	Low Impact Development (LID) is a site design strategy with a goal of maintaining or replicating the pre-development hydrologic regime through the use of design techniques. LID site design BMPs help preserve and restore the natural hydrologic cycle of the site, allowing for filtration and infiltration which can greatly reduce the volume, peak flow rate, velocity, and pollutant loads of storm water runoff.
LID BMP	A type of storm water BMP that is based upon Low Impact Development concepts. LID BMPs not only provide highly effective treatment of storm water runoff, but also yield potentially significant reductions in runoff volume – helping to mimic the pre- project hydrologic regime, and also require less ongoing maintenance than Treatment Control BMPs. The applicant may refer to Chapter 2.
LID BMP Design Handbook	The LID BMP Design Handbook was developed by the Copermittees to provide guidance for the planning, design and maintenance of LID BMPs which may be used to mitigate the water quality impacts of PDPs within the County.
LID Bioretention BMP	LID Bioretention BMPs are bioretention areas are vegetated (i.e., landscaped) shallow depressions that provide storage, infiltration, and evapotranspiration, and provide for pollutant removal (e.g., filtration, adsorption, nutrient uptake) by filtering storm water through the vegetation and soils. In bioretention areas, pore spaces and organic material in the soils help to retain water in the form of soil moisture and to promote the adsorption of pollutants (e.g., dissolved metals and petroleum hydrocarbons) into the soil matrix. Plants use soil moisture and promote the drying of the soil through transpiration. The Regional MS4 Permit defines "retain" as to keep or hold in a particular place, condition, or position without discharge to surface waters.
LID Biofiltration BMP	BMPs that reduce stormwater pollutant discharges by intercepting rainfall on vegetative canopy, and through incidental infiltration and/or evapotranspiration, and filtration, and other biological and chemical processes. As storm water passes down through the planting soil, pollutants are filtered, adsorbed, biodegraded, and sequestered by the soil and plants, and collected through an underdrain.

LID Harvest and Reuse BMP	BMPs used to facilitate capturing storm water runoff for later use without negatively impacting downstream water rights or other
	Beneficial Uses.
LID Infiltration BMP	BMPs to reduce storm water runoff by capturing and infiltrating the runoff into in-situ soils or amended onsite soils. Typical LID Infiltration BMPs include infiltration basins, infiltration trenches and pervious pavements.
LID Retention BMP	BMPs to ensure full onsite retention without runoff of the DCV such as infiltration basins, bioretention, chambers, trenches, permeable pavement and pavers, harvest and reuse.
LID Principles	Site design concepts that prevent or minimize the causes (or drivers) of post-construction impacts, and help mimic the pre- development hydrologic regime.
MEP	Maximum Extent Practicable - standard established by the 1987 amendments to the Clean Water Act (CWA) for the reduction of Pollutant discharges from MS4s. Refer to Attachment C of the Regional MS4 Permit for a complete definition of MEP.
MF	Multi-family – zoning classification for parcels having 2 or more living residential units.
MS4	Municipal Separate Storm Sewer System (MS4) is a conveyance or system of conveyances (including roads with drainage systems, municipal streets, catch basins, curbs, gutters, ditches, man-made channels, or storm drains): (i) Owned or operated by a State, city, town, borough, county, parish, district, association, or other public body (created by or pursuant to State law) having jurisdiction over disposal of sewage, industrial wastes, storm water, or other wastes, including special districts under State law such as a sewer district, flood control district or drainage district, or similar entity, or an Indian tribe or an authorized Indian tribal organization, or designated and approved management agency under section 208 of the CWA that discharges to waters of the United States; (ii) Designated or used for collecting or conveying storm water; (iii) Which is not a combined sewer; (iv) Which is not part of the Publicly Owned Treatment Works (POTW) as defined at 40 CFR 122.26.
New Development	Defined by the Regional MS4 Permit as 'Priority Development
Project	Projects' if the project, or a component of the project meets the categories and thresholds described in Section 1.1.1.
NPDES	National Pollution Discharge Elimination System - Federal
	program for issuing, modifying, revoking and reissuing, terminating, monitoring and enforcing permits, and imposing and enforcing pretreatment requirements, under Sections 307, 318, 402, and 405 of the CWA.
NRCS	Natural Resources Conservation Service

PDP	Priority Development Project - Includes New Development and Redevelopment project categories listed in Provision E.3.b of the Regional MS4 Permit.
Priority Pollutants of	Pollutants expected to be present on the project site and for which
Concern	a downstream water body is also listed as Impaired under the CWA
Concern	Section 303(d) list or by a TMDL.
Project-Specific	A plan specifying and documenting permanent LID Principles and
WQMP	storm water BMPs to control post-construction Pollutants and
VVQIVIF	storm water runoff for the life of the PDP, and the plans for
	operation and maintenance of those BMPs for the life of the project.
Receiving Waters	Waters of the United States.
Redevelopment	The creation, addition, and or replacement of impervious surface
Project	on an already developed site. Examples include the expansion of a
Troject	building footprint, road widening, the addition to or replacement
	of a structure, and creation or addition of impervious surfaces.
	Replacement of impervious surfaces includes any activity that is
	not part of a routine maintenance activity where impervious
	material(s) are removed, exposing underlying soil during
	construction. Redevelopment does not include trenching and
	resurfacing associated with utility work; resurfacing existing
	roadways; new sidewalk construction, pedestrian ramps, or bike
	lane on existing roads; and routine replacement of damaged
	pavement, such as pothole repair.
	Project that meets the criteria described in Section 1.
Runoff Fund	Runoff Funds have not been established by the Copermittees and
	are not available to the Applicant.
	If established, a Runoff Fund will develop regional mitigation
	projects where PDPs will be able to buy mitigation credits if it is
	determined that implementing onsite controls is infeasible.
San Diego Regional	San Diego Regional Water Quality Control Board - The term
Board	"Regional Board", as defined in Water Code section 13050(b), is
	intended to refer to the California Regional Water Quality Control
	Board for the San Diego Region as specified in Water Code Section
	13200. State agency responsible for managing and regulating water
	quality in the SMR.
SCCWRP	Southern California Coastal Water Research Project
Site Design BMP	Site design BMPs prevent or minimize the causes (or drivers) of
	post-construction impacts, and help mimic the pre-development
	hydrologic regime.
SF	Parcels with a zoning classification for a single residential unit.
SMC	Southern California Stormwater Monitoring Coalition
SMR	The Santa Margarita Region (SMR) represents the portion of the
	Santa Margarita Watershed that is included within the County of
	Riverside.

Source Control BMP	Source Control BMPs land use or site planning practices, or
	structural or nonstructural measures that aim to prevent runoff
	pollution by reducing the potential for contamination at the source
	of pollution. Source control BMPs minimize the contact between
	Pollutants and runoff.
Structural BMP	Structures designed to remove pollutants from stormwater runoff
	and mitigate hydromodification impacts.
SWPPP	Storm Water Pollution Prevention Plan
Tentative Tract Map	Tentative Tract Maps are required for all subdivision creating five
	(5) or more parcels, five (5) or more condominiums as defined in
	Section 783 of the California Civil Code, a community apartment
	project containing five (5) or more parcels, or for the conversion of
	a dwelling to a stock cooperative containing five (5) or more
	dwelling units.
TMDL	Total Maximum Daily Load - the maximum amount of a Pollutant
	that can be discharged into a waterbody from all sources (point and
	non-point) and still maintain Water Quality Standards. Under
	CWA Section 303(d), TMDLs must be developed for all
	waterbodies that do not meet Water Quality Standards after
	application of technology-based controls.
USEPA	United States Environmental Protection Agency
Volume-Based BMP	Volume-Based BMPs applies to BMPs where the primary mode of
	pollutant removal depends upon the volumetric capacity such as
	detention, retention, and infiltration systems.
WQMP	Water Quality Management Plan
Wet Season	The Regional MS4 Permit defines the wet season from October 1
	through April 30.

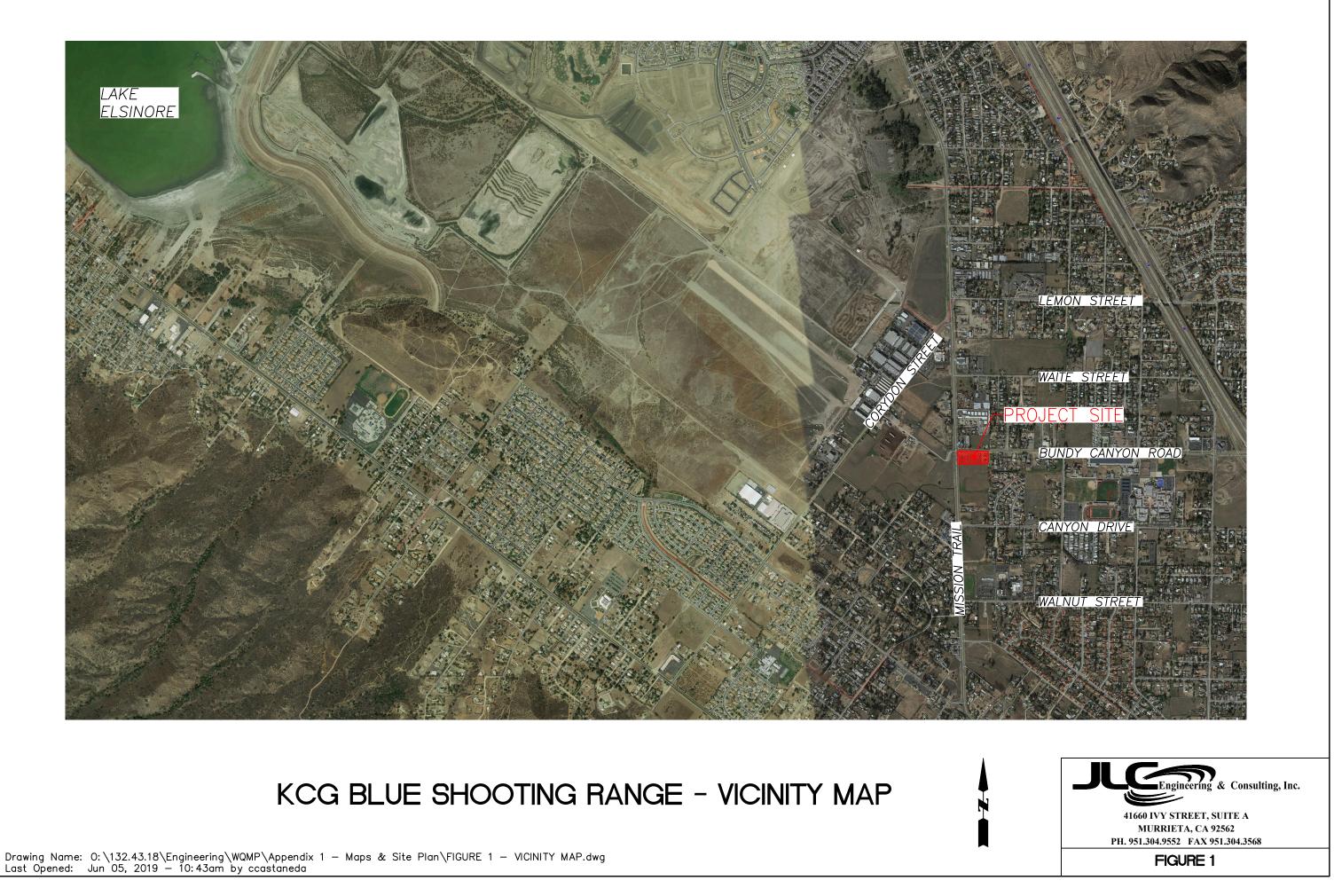
Appendix 1: Maps and Site Plans

Location Map, WQMP Site Plan and Receiving Waters Map

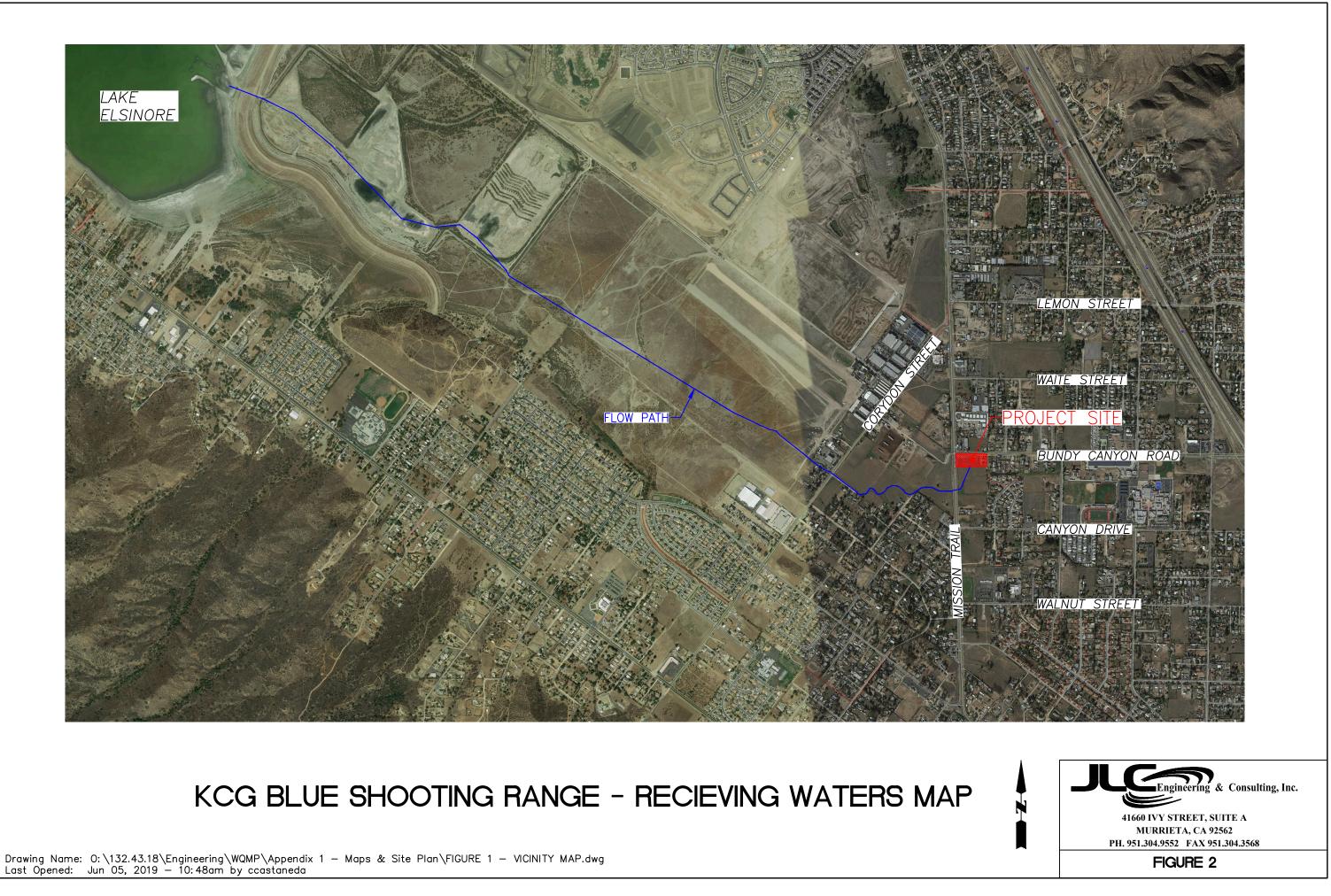
Complete the checklist below to verify all exhibits and components are included in the Project-Specific WQMP. Refer Section 4 of the SMR WQMP and Section D of this Template.

	Map and Site Plan Checklist				
Indicate all	Indicate all Maps and Site Plans are included in your Project-Specific WQMP by checking the boxes below.				
	Vicinity and Location Map				
	Existing Site Map (unless exiting conditions are included in WQMP Site Plan)				
	WQMP Site Plan				
	Parcel Boundary and Project Footprint				
	Existing and Proposed Topography				
	Drainage Management Areas (DMAs)				
	Proposed Structural Best Management Practices (BMPs)				
	Drainage Paths				
	Drainage infrastructure, inlets, overflows				
	Source Control BMPs				
	Site Design BMPs				
	Buildings, Roof Lines, Downspouts				
	Impervious Surfaces				
	Pervious Surfaces (i.e. Landscaping)				
	Standard Labeling				

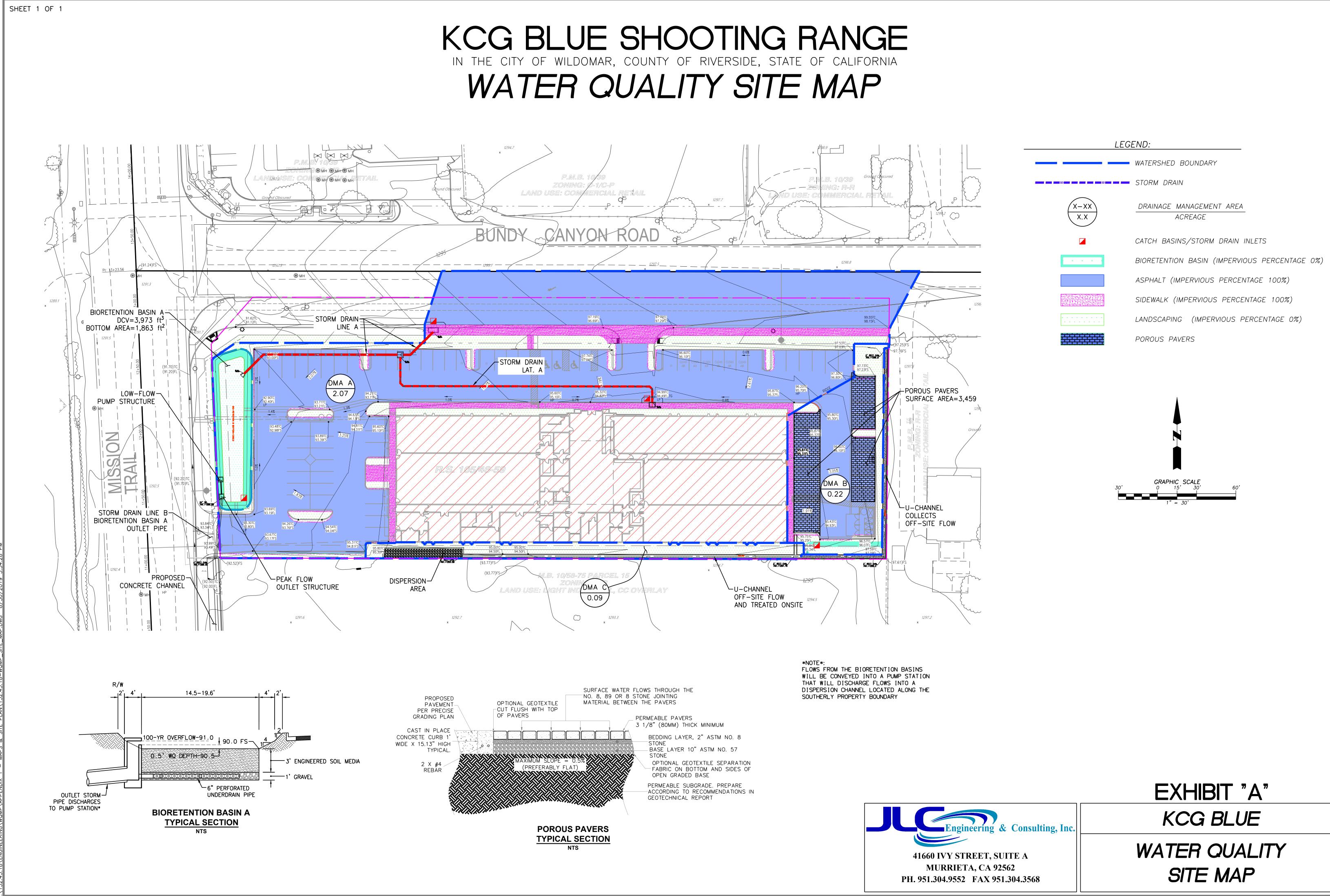
Vicinity Map



Receiving Waters Map



WQMP Site Plan



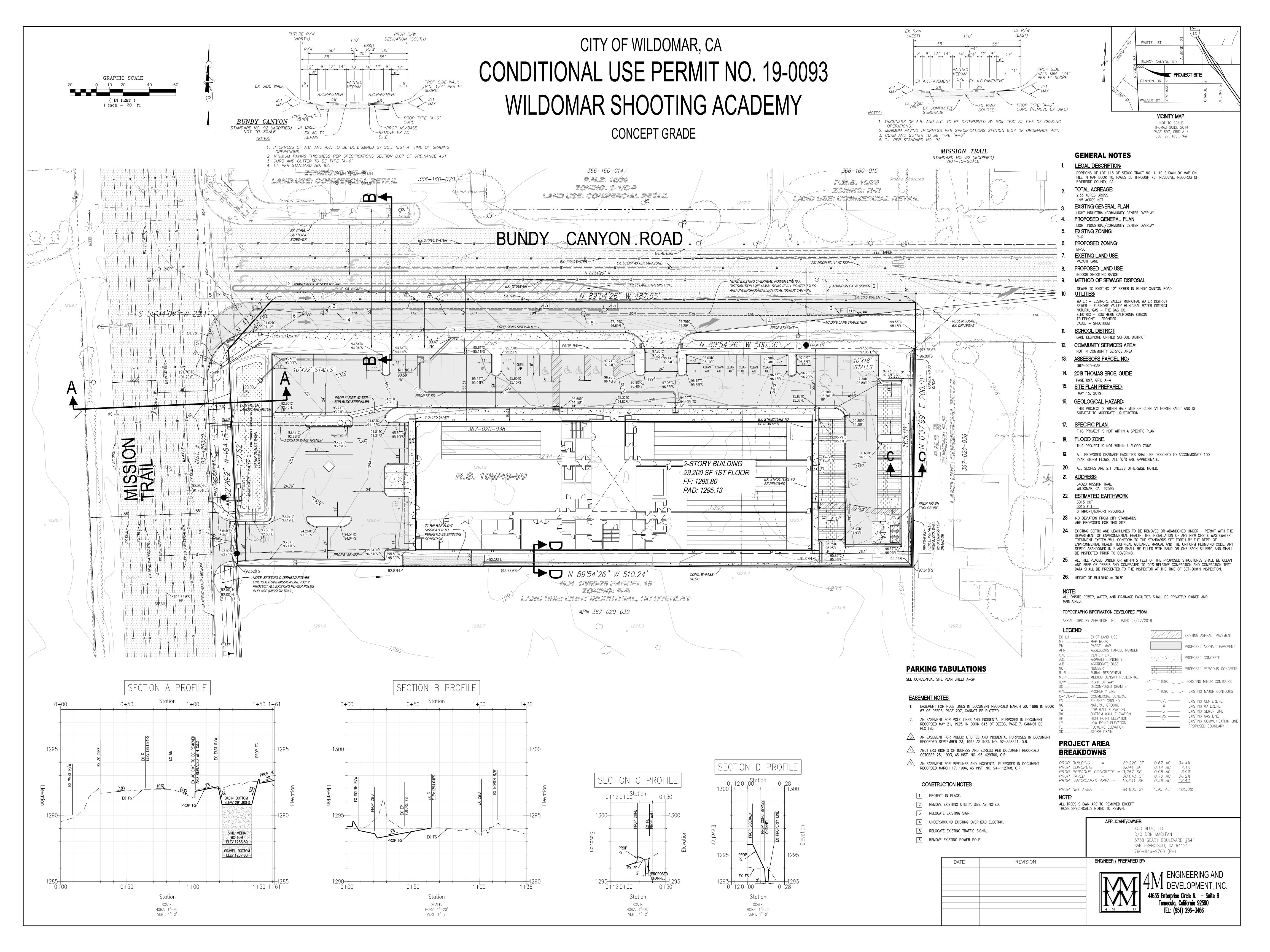
Appendix 2: Construction Plans

Grading and Drainage Plans

Examples of material to provide in Appendix 2 may include but are not limited to the following:

- Site grading plans from the Project's Civil Plan Set,
- Drainage plans showing the exiting condition and proposed drainage system from the project's drainage report,
- Other plan sheets containing elements that impact site grading and drainage.

Refer to Section 4 of the SMR WQMP and Section I of this Template.



Appendix 3: Soils Information

Geotechnical Study, Other Infiltration Testing Data, and/or Other Documentation

Examples of material to provide in Appendix 3 may include but are not limited to the following:

- Geotechnical Study/Report prepared for the project,
- Additional soils testing data (if not included in the Geotechnical Study),
- Exhibits/Maps/Other Documentation of the Hydrologic Soils Groups (HSG)s at the project site.

This information should support the Full Infiltration Applicability, and Biofiltration Applicability sections of this Template. Refer to Section 2.3 of the SMR WQMP and Sections A and D of this Template.

Earth Strata Geotechnical Services, Inc. Geotechnical, Environmental and Materials Testing Consultants

> Project No. 182250-12A MDMG Project 1585 KCG BLU, LLC

September 20, 2018

Mr. Don Maclean **KCG BLUE, LLC** 3961 Citrus Drive Fallbrook, CA 92028

Subject: Double Ring Infiltration Testing Report, Proposed Gun Shooting Range and Tactical Training Facility, Assessor Parcel Number 367-020-038, Lot Number 115 of Sedco Tract 1 Subdivision, Located at 34020 Mission Trail, City of Wildomar, Riverside County, California

Earth Strata Geotechnical Services is pleased to present this infiltration feasibility report for the proposed commercial development, Assessor Parcel Number 367-020-038, located at 34020 Mission Trail in the City of Wildomar of Riverside County, California. The purpose of our study was to determine the infiltration rates and physical characteristics of the subsurface earth materials at the approximate depth of the proposed WQMP area within the proposed development. This feasibility report provides the infiltration rates to be used for the design and the development of the water quality management plan, where applicable.

PROPERTY DESCRIPTION

The subject property is located at 34020 Mission Trail in the City of Wildomar, Riverside County, California. The approximate location of the site is shown on the Vicinity Map, Figure 1.

The subject property is comprised of approximately 2.33 acres of developed land with an existing singlefamily residence and accessory building to be removed. Topographic relief at the subject property is relatively low with the terrain being generally flat. Elevations at the site is approximately 1,305 feet above mean sea level (msl). Drainage within the subject property generally flows to the northwest.

The site is currently bordered by residential development to the east, commercial development to the north, and vacant property to the west and south. Most of the vegetation on the site consists of moderate amounts of annual weeds/grasses.

PROPOSED CONSTRUCTION

The proposed commercial development is expected to consist of concrete, wood or steel framed twostory structure utilizing slab on grade construction with associated streets, parking, landscape areas, and utilities. The current development plans include demolition of the existing single-family residence and accessory building and construction of one (1) building pad for the proposed two-story structure.

SUBSURFACE EXPLORATION AND INFILTRATION TESTING

SUBSURFACE EXPLORATION

Subsurface exploration of the subject site consisted of four exploratory borings within the proposed development for geotechnical evaluation purposes to a maximum depth of 31.5 feet, conducted on August 21, 2018. The approximate locations of the exploratory excavations are shown on the attached Infiltration Location Map, Plate 1.

EARTH MATERIALS

The earth materials on the site are primarily comprised of topsoil and Quaternary alluvial materials. A general description of the dominant earth materials observed on the site is provided below:

- <u>Topsoil (no map symbol)</u>: Residual topsoil, encountered in the upper 1 foot, blankets the site and underlying alluvium. These materials were noted to be generally strong brown to dark brown, silty sand and clayey sand which were very porous, dry and in a loose to medium dense state.
- <u>Quaternary Young Alluvial Fan Deposits (map symbol Qyv)</u>: Quaternary young alluvial fan deposits were encountered at the surface and beneath the topsoil to the full depth of our exploration. These young alluvial deposits consist predominately of interlayered strong brown, yellowish brown to gray brown, fine to coarse grained silty sand, clayey sand, and sandy silt. These deposits were generally noted to be in a dry to moist, loose to very dense state.

GROUNDWATER

Groundwater was not observed within the exploratory borings excavated to a depth of 31.5 feet.

INFILTRATION TESTING

The double ring infiltrometer test method was utilized to perform a total of four (4) infiltration tests on August 30, 2018 to evaluate near surface infiltration rates in order to estimate the amount of storm water runoff that can infiltrate into the onsite water quality treatment plan areas. The infiltration tests were performed in general accordance with the requirements of double ring infiltration testing, ASTM D3385 and Appendix A of the Riverside County Flood Control and Water Conservation District.

The infiltration tests were performed using double ring infiltrometer and Mariotte tubes at a depth of 5 feet below existing grades. The locations of the infiltration tests are indicated on the attached infiltration Location Map, Plate 1. The double ring infiltrometer tests were located by property boundary measurement on the site plan and by using geographic features. Infiltration test data recorded in the field are summarized in the following table and is included within Appendix B including the graph of Infiltration Rate versus Elapsed Time.

INFILTRATION TEST SUMMARY

TEST NUMBER	INFILTRATION HOLE DEPTH (ft.)	INFILTRATION RATE (in/hr)	DESCRIPTION
DR-1	5	0.27	Clayey SAND
DR-2	5	0.22	Clayey SAND
DR-3	5	1.89	Silty SAND
DR-4	5	0.25	Sandy CLAY

The infiltration test rates ranged from 0.22 to 1.89 inches per hour (in/hr).

CONCLUSIONS AND RECOMMENDATIONS

<u>General</u>

From geotechnical and engineering geologic points of view, the proposed WQMP areas, where tested, is considered suitable for infiltration for the proposed development, provided the following conclusions and recommendations are incorporated into the plans and are implemented during construction.

Groundwater

Groundwater was not observed during our subsurface exploration. Potential groundwater impact is considered very low to low. Local well data indicates regional groundwater highs approximately 385 feet below existing surface, which meets the minimum separation of >10 feet from the bottom of infiltration facility to the groundwater mark.

Geologic/ Geotechnical Screening

These proposed WQMP areas in the vicinity of DR-1 and DR-2 (see Plate 1) are located at a lower elevation than the proposed structures in competent native earth materials.

The proposed structures will be supported by compacted fill and competent alluvium, with groundwater at a depth of approximately 385 feet. As such, the potential for earthquake induced liquefaction and lateral spreading beneath the proposed structures is considered very low to remote due to the recommended compacted fill, relatively low groundwater level, and the dense nature of the deeper onsite earth materials.

Therefore, infiltration within the proposed WQMP areas will not encroach on any proposed structures and will not increase the risk of geologic hazards.

Preliminary laboratory test results indicate onsite earth materials exhibit an expansion potential of **LOW** as classified in accordance with 2016 CBC Section 1803.5.3 and ASTM D4829.

Recommended Factor of Safety

The recommended factor of safety for the infiltration design is 3.

Based on the data presented in this report and the recommendations set forth herein, it is the opinion of Earth Strata Geotechnical Services that the WQMP area can be designed for an infiltration rate of 0.26 inches per hour.

GRADING PLAN REVIEW AND CONSTRUCTION SERVICES

This report has been prepared for the exclusive use of **Mr. Don McClean** and their authorized representative. It likely does not contain sufficient information for other parties or other uses. Earth Strata Geotechnical Services should be engaged to review the final design plans and specifications prior to construction. This is to verify that the recommendations contained in this report have been properly incorporated into the project plans and specifications. Should Earth Strata Geotechnical Services not be accorded the opportunity to review the project plans and specifications, we are not responsibility for misinterpretation of our recommendations.

Earth Strata should be retained to provide observations during construction to validate this report. In order to allow for design changes in the event that the subsurface conditions differ from those anticipated prior to construction.

Earth Strata should review any changes in the project and modify and approve in writing the conclusions and recommendations of this report. This report and the drawings contained within are intended for design input purposes only and are not intended to act as construction drawings or specifications. In the event that conditions encountered during grading or construction operations appear to be different than those indicated in this report, this office should be notified immediately, as revisions may be required.

REPORT LIMITATIONS

Our services were performed using the degree of care and skill ordinarily exercised, under similar circumstances, by reputable soils engineers and geologists, practicing at the time and location this report was prepared. No other warranty, expressed or implied, is made as to the conclusions and professional advice included in this report.

Earth materials vary in type, strength, and other geotechnical properties between points of observation and exploration. Groundwater and moisture conditions can also vary due to natural processes or the works of man on this or adjacent properties. As a result, we do not and cannot have complete knowledge of the subsurface conditions beneath the subject property. No practical study can completely eliminate uncertainty with regard to the anticipated geotechnical conditions in connection with a subject property.

The conclusions and recommendations within this report are based upon the findings at the points of observation and are subject to confirmation by Earth Strata during construction. This report is considered valid for a period of one year from the time the report was issued.

This report was prepared with the understanding that it is the responsibility of the owner or their representative, to ensure that the conclusions and recommendations contained herein are brought to the attention of the other project consultants and are incorporated into the plans and specifications. The owners' contractor should properly implement the conclusions and recommendations during grading and construction, and notify the owner if they consider any of the recommendations presented herein to be unsafe or unsuitable.

Respectfully submitted,

EARTH STRATA GEOTECHNICAL SERVICES

Milalo

Stephen M. Poole, PE 40219 President Principal Engineer

SMP/jf/snj

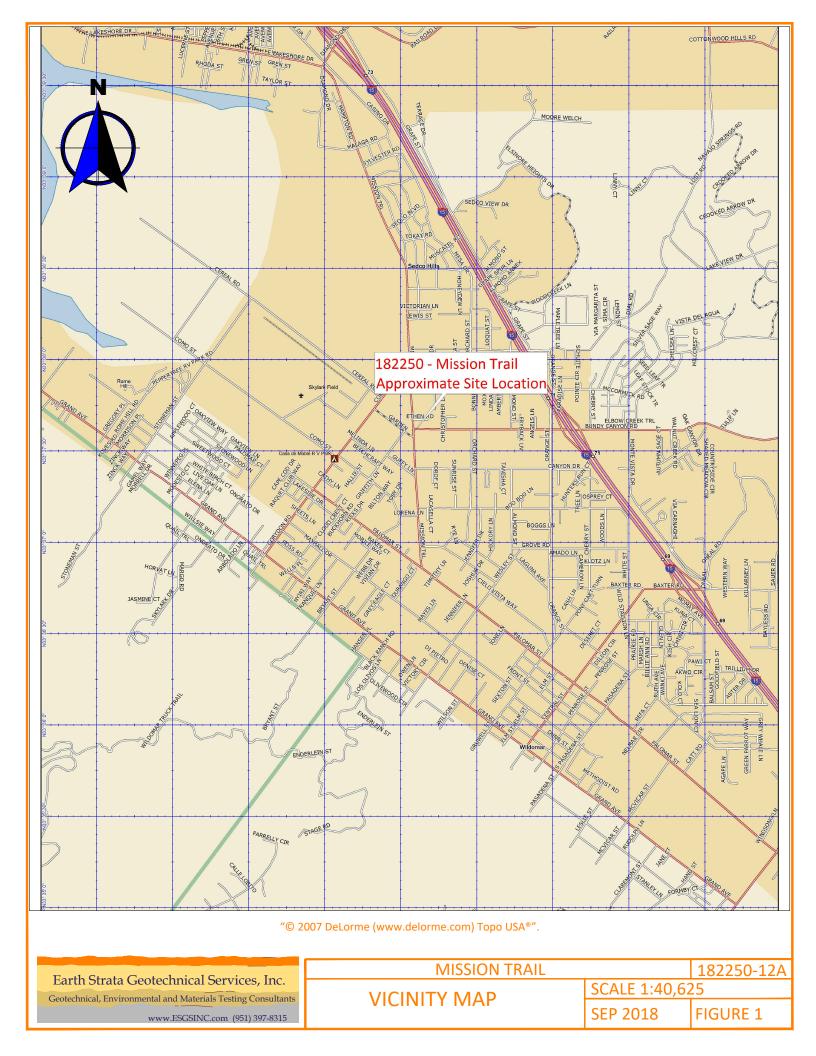
Distribution: (1) Addressee

Attachments: Figure 1 – Vicinity Map (*Rear of Text*) Appendix A – Exploratory Logs (*Rear of Text*) Appendix B – Infiltration Test Sheets (*Rear of Text*) Plate 1 – Infiltration Location Map (*Rear of Text*)

EARTH STRATA GEOTECHNICAL SERVICES 5 Project Number 182250-12A



September 20, 2018



APPENDIX A EXPLORATORY LOGS

						Geo	otechnical Boring Log B-1
Date: August 21, 2018							Project Name: Mission Trail Page: 1 of 2
Project Number: 182250-10A							Logged By: JF
Drilling Company: Drilling It							Type of Rig: B-61
Drive Weight (lbs): 140							Drop (in): 30 Hole Diameter (in): 8
Top of Hole Elevation (ft): See Map					e Map		Hole Location: See Geotechnical Map
Depth (ft)	Blow Count Per	Foot	Sample Depth	Dry Density (pcf)	Moisture (%)	Classification Symbol	MATERIAL DESCRIPTION
0		0-	-5'				Topsoil
						SC	Clayey SAND; dark brown, dry, dense, fine to medium sand, trace gravel
	2	5 2.	.5'	115.3	9.0		Quaternary Young Alluvial Fan Deposits (Qyv)
						ML	Sandy SILT; medium brown, dry, medium dense, fine to coarse sand, trace clay
							and gravel
5 -	2	1 5	5'	104.7	6.4	SM	Silty SAND; yellowish brown, dry, dense, fine to coarse sand
	4	0 7.	.5'	118.4	10.5	CL	Sandy CLAY with Silt; yellowish red to brown, dry, dense, fine to coarse sand,
				110.1	10.5	02	trace gravel
10 -	3	3 1	.0'	119.8	14.6		
						SM	Silty SAND; strong brown, slightly moist, dense, fine to medium sand, trace clay
15 -	<u> </u>						
	5		.5'	123.5	10.4	ML	Sandy SILT; strong brown, dry, dense, fine to medium sand
20 -							
20	6	1 2	.0'	118.5	14.0	SM	Silty SAND; yellowish brown, slighty mosit, very dense, fine to medium sand
25 -	4	1 2	5'	117.5	14.7	ML	Sandy SILT; yellowish brown to grayish brown, moist, dense, fine sand
30							
	42	184 F	Rem	ningto	n Ave	nue, T	Temecula, CA 92590 WWW.ESGSINC.com (951) 397-8315

	Geotechnical Boring Log B-1														
	ugust 21,						2 of 2								
	Number:					Logged By: JF									
_	Company					Type of Rig: B-61									
	Veight (lb	-				Drop (in): 30 Hole Diameter (in): 8									
Top of	Hole Elev	ation		e Map	1	Hole Location: See Geotechnical Map									
	Blow Count Per Foot	th	Dry Density (pcf)	(uo										
ff)	unt	Sample Depth	ity	Moisture (%)	Classification Symbol										
t) (t	, Cour Foot	le [ens	ture	assificati Symbol										
Depth (ft)	No	dme	Γ	lois	Clas										
	В	Si	D	2		MATERIAL DESCRIPTION									
30	53	30'	112.4	15.1		Yellowish gray to strong brown, very dense below 30 feet									
	\sim														
						Total Depth: 31.5 feet									
						No Groundwater									
35 -															
55															
40 -															
45 -															
	_														
	_														
	_														
50 -															
	_														
	_														
	_														
55 -															
	H														
60															
	42184	4 Ren	ningto	n Ave	nue, T	Earth Strata Geotechnical Services Femecula, CA 92590 Geotechnical, Environmental and Materials Testing Construction www.ESGSINC.com (951)	sultants								

	Geotechnical Boring Log B-2 Date: August 21, 2018 Project Name: Mission Trail Page: * of *										
	-						Project Name: Mission Trail Page: * of *				
Project							Logged By: JF				
Drilling		-	-	-			Type of Rig: B-61				
Drive V			-				Drop (in): 30 Hole Diameter (in): 8				
Top of	HC		ation	(ft): See	e Map		Hole Location: See Geotechnical Map				
Depth (ft)		Blow Count Per Foot	Sample Depth	Dry Density (pcf)	Moisture (%)	Classification Symbol	MATERIAL DESCRIPTION				
0							Quaternary Young Alluvial Fan Deposits (Qyv)				
	Π										
		48	2.5'	118.5	6.5	SM	Silty SAND; brown, dry, dense, fine to coarse sand, trace clay and gravel				
_							Strong brown below 4 feet				
5		34	5'	118.2	10.8	SC	Clayey SAND; dark reddish brown, dry, dense, fine to coarse sand				
		36	7.5'	115.4	11.5	SM	Silty SAND; yellowish brown, dry, dense, fine to coarse sand, trace clay				
10		90/9"	10'	128.8	7.1		Light yellowish brown, very dense below 10 feet				
15											
13		74	15'	132.5	8.6		Light reddish brown to strong brown, with clay				
		/									
							Total Depth: 16.5 feet				
							No Groundwater				
20											
20											
25											
25											
30											
		42184	4 Ren	ningto	n Ave	nue, T	Earth Strata Geotechnical Services, Inc. Geotechnical, Environmental and Materials Testing Consultants www.ESGSINC.com (951) 397-8315				

						Geo	otechnical Boring Log B-3								
Date: A							Project Name: Mission Trail Page: 1 of 1								
Project							Logged By: JF								
Drilling			-	-			ype of Rig: B-61 rop (in): 30 Hole Diameter (in): 8								
Drive V			-	0 (ft): See	Man		Hole Location: See Geotechnical Map								
			ation		e iviap										
f)		Blow Count Per Foot	Sample Depth	Dry Density (pcf)	Moisture (%)	Classification Symbol									
t) (1		/ Cour Foot	ole E	ens	ture	assificati Symbol									
Depth (ft)		Blow	amp	Iry D	lois	Cla									
		ш	S		2		MATERIAL DESCRIPTION								
0	Ц						Topsoil								
	μ					SM	Silty SAND; strong brown, dry, loose, fine to coarse sand, trace gravel								
		10	2.5'	98.6	9.9		Quaternary Young Alluvial Fan Deposits (Qyv)								
						SM	Silty SAND; brown, moist, loose, fine to coarse sand								
5		10													
		18	5'	113.4	7.5	SP-SC	Poorly-Graded SAND with Clay; dark reddish brown, moist, medium dense, fine								
		25					to coarse sand								
		35	7.5'	120.1	14.2	SC	Clayey SAND; strong brown to yellowish brown, moist, dense, fine to coarse								
							sand, trace gravel								
10		38	10'	114.0	17.4	<u> </u>	City CAND, growich brown, moist dance fing to modium conducith alow								
		30	10	114.8	17.4	SIVI	Silty SAND; grayish brown, moist, dense, fine to medium sand with clay								
	Η														
	Η														
	H														
15		41	15'	125.4	13.5	SC	Clayey SAND; yellowish red, moist, dense, fine to coarse sand								
	Π						Total Depth: 16.5 feet								
	Π						No Groundwater								
20	Π														
20	Π														
	Π														
	Π														
25															
25	Ш														
	Ц														
	Ц														
30															
		1710	1 Ron	ningto	η Ανο	пиа т	Earth Strata Geotechnical Services, Inc. remecula, CA 92590 Geotechnical Environmental and Materials Testing Consultants								
		74104		inigio	ii Ave	nue, I									
							www.ESGSINC.com (951) 397-8315								

	Geotechnical Boring Log B-4 Date: August 21, 2018 Project Name: Mission Trail Page: 1 of 1										
						Project Name: Mission Trail Page: 1 of 1					
	Number:					Logged By: JF					
_	Company		-			Type of Rig: B-61					
-	eight (lb	-				Drop (in): 30 Hole Diameter (in): 8					
Top of F	lole Elev	ation (e Map	1	Hole Location: See Geotechnical Map					
Depth (ft)	Blow Count Per Foot	Sample Depth	Dry Density (pcf)	Moisture (%)	Classification Symbol	MATERIAL DESCRIPTION					
0		0-5'				<u>Topsoil</u>					
	\sim				SC	Clayey SAND; strong brown, dry, medium dense, fine to medium sand, trace					
		\searrow				gravel					
						Quaternary Young Alluvial Fan Depsoits (Qyv)					
					SM	Silty SAND; brown, moist, medium dense to dense, fine to coarse sand					
5 -						Total Depth: 5 feet					
						No Groundwater					
10											
10 -											
15 -											
20 -											
25 -	┥										
20											
30											
	42184	4 Rem	ningto	n Ave	nue, T	Temecula, CA 92590 WWW.ESGSINC.com (951) 397-8315					

APPENDIX B

INFILTRATION TEST SHEETS

Test No.	DR-1	Location	5	See Map	כ		Turf	-Tec In	ternatio	<mark>onal - F</mark>	Record	Chart f	or IN10	<mark>)-W - (1</mark>	<mark>2 & 24 In</mark>	ich In
	dentification:		A	1			Constants		Area cm2	Depth of Liquid (cm)	Container Number		Marriotte 1	Гube Volun		Earth Geotechni
Test Loc		DR-1					Inner Ring		729				3000			
Liquid Us Tested B		TAP WATI MM/DI	рн:	8.0 Date		80/2018	Annular Ri		2189 I maintained			Float Valv	o () Mari	iotte Tubes	10000	
		> 30 Feet		Depth of		4.5'			Depth of O			9 cm	Other	olle Tubes		
Deptilito	Water table.	2 00 1 000				1.0		r enetration		ator rung	•	0.011	O line			
							Flow R	eadings	A	-	Inf	iltration Rat	es		Ground Tem	perature
Trial #	Start / End	Date MM/DD/YY	Time HR:MIN	Time Increment /(Total)	Elapsed Time (Min)	Inner Ring Reading cm	Inner Maroitte Tube Flow (ml)	Annular Space Reading cm	Annular Space Marriotte Tube Flow (ml)	Liquid Temp ºF	Inner Infiltration Rate cm/h	Inner Infiltration Rate In/h	Annular Infiltration Rate cm/h	Annular Infiltration Rate In/h	Ground Temp Depth (cm)	Temp at Depth (c
	Start Test	8/30/2018														
1	End Test Start Test	8/30/2018 8/30/2018	10:36 10:36	0:30		6.00	0	6.00	2500		0.00	0.00	2.28	0.90		
2	End Test	8/30/2018	11:06	0:30		6.00	400	6.00	2000		1.10	0.43	1.83	0.72		
	Start Test	8/30/2018	11:06	0:30	00											
3	End Test Start Test	8/30/2018 8/30/2018	<u>11:36</u> 11:36	1:30 0:30		6.00	250	6.00	2000		0.69	0.27	1.83	0.72		
4	End Test	8/30/2018	12:06	2:00		6.00	250	6.00	2000		0.69	0.27	1.83	0.72		
-	Start Test	8/30/2018				0.00	050	0.00	4500		0.00	0.27	4.07	0.54		
5	End Test Start Test	8/30/2018 8/30/2018	12:36 12:36	2:30 0:30		6.00	250	6.00	1500		0.69	0.27	1.37	0.54		
6	End Test	8/30/2018	13:06	3:00	180	6.00	200	6.00	1500		0.55	0.22	1.37	0.54		
7	Start Test End Test	8/30/2018 8/30/2018	13:06 13:36	0:30		6.00	250	6.00	1500		0.69	0.27	1.37	0.54		
	Start Test	8/30/2018	13:36	0:30	240											
8	End Test	8/30/2018	14:06	4:00		6.00	250	6.00	1500		0.69	0.27	1.37	0.54		
							-									
							-									
									-							
									-							
									-							
									-							
							-		-		-					
									-							
											-					
							_		-							
L																

nfiltration Rings)

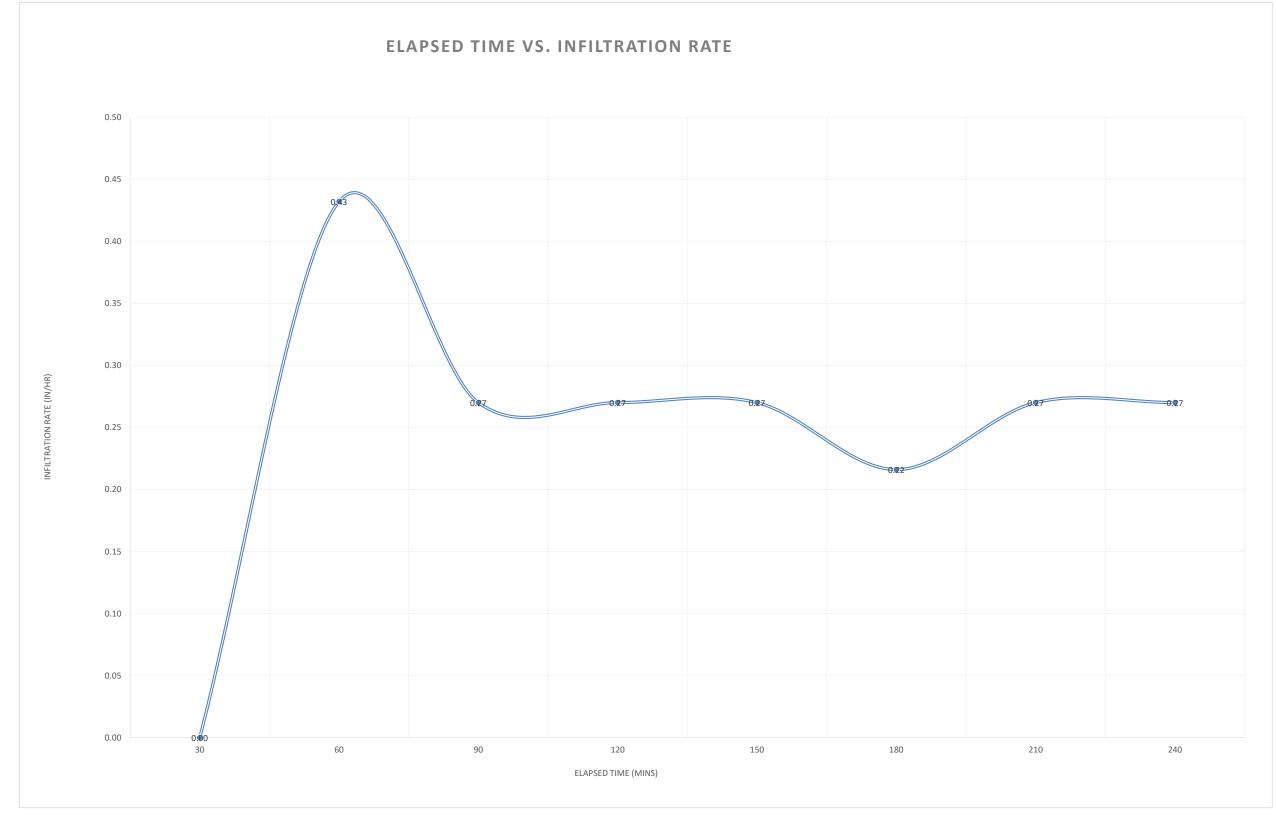
rth Strata Geotechnical Services, Inc.

hnical, Environmental and Materials Testing Consultants

е	Remarks
at (c)	Weather conditions Etc
	urf-lec nternational

Project Identification:	182250-12	A	
Test Location:	DR-1		
Liquid Used:	TAP WATE	pH:	8.0
Tested By:	MM/DI		
Depth to water table:	> 30 Feet		

Earth Strata Geotechnical Services, Inc. Geotechnical, Environmental and Materials Testing Consultants



Test No.	DR-2	Location	S	See Map)		Turi	-Tec In	ternatio	<mark>nal - F</mark>	Record	Chart f	or IN10	<mark>)-W - (1</mark>	<mark>2 & 24 In</mark>	ch In
	dentification:		A				Constants		Area cm2	Depth of Liquid (cm)	Container Number	-	Marriotte 1	Гube Volum		Earth Geotechni
Test Loc		DR-2					Inner Ring		729				5000			
Liquid U		TAP WAT	pH:	8.0			Annular Ri		2189						10000	
Tested B		MM/DI		Date		0/2018		Liquid level maintained (X) Flow Valve () Float Valve () Mariotte Tubes Penetration Depth of Outer Ring: 9 cm Other								
Depth to	water table:	> 30 Feet		Depth of	lest	4.5'		Penetration	Depth of O	uter Ring	:	9 cm	Other			
							Flow R	eadings			Inf	iltration Rat	es		Ground Tem	perature
Trial #	Start / End	Date MM/DD/YY	Time HR:MIN	Time Increment /(Total)	Elapsed Time (Min)	Inner Ring Reading cm	Inner Maroitte	Annular Space Reading cm	Annular Space Marriotte Tube Flow (ml)	Liquid Temp ºF	Inner	Inner Infiltration Rate In/h	Annular Infiltration	Annular Infiltration Rate In/h	Ground Temp Depth (cm)	Temp a Depth (c
	Start Test	8/30/2018	10:52		30											
1	End Test	8/30/2018	11:22	0:30		6.00	100	6.00	8500		0.27	0.11	7.77	3.06		
2	Start Test End Test	8/30/2018 8/30/2018	11:22 11:52	0:30		6.00	100	6.00	7000		0.27	0.11	6.40	2.52		
	Start Test	8/30/2018				0.00	100	0.00	7000		0.27	0.11	0.40	2.52		
3	End Test	8/30/2018	12:38	1:30		6.00	100	6.00	4500		0.27	0.11	4.11	1.62		
	Start Test	8/30/2018										0.40		4.00		
4	End Test Start Test	8/30/2018 8/30/2018	13:08 13:08	2:00 0:30		6.00	400	6.00	3000		1.10	0.43	2.74	1.08		
5	End Test	8/30/2018	13:38	2:30	150	6.00	200	6.00	4500		0.55	0.22	4.11	1.62		
	Start Test	8/30/2018					_					0.00		0.00		
6	End Test Start Test	8/30/2018 8/30/2018	<mark>14:08</mark> 14:08	3:00 0:30		6.00	200	6.00	2500		0.55	0.22	2.28	0.90		
7	End Test	8/30/2018	14:38	3:30	210	6.00	200	6.00	2500		0.55	0.22	2.28	0.90		
8	Start Test End Test	8/30/2018 8/30/2018	14:38 15:08	0:30 4:00		6.00	200	6.00	2500		0.55	0.22	2.28	0.90		
							_		-							
									-							
							-		-							
							-		-							
							-									
									-							
							-		-							
									-							
											-					
l				8							•					

nfiltration Rings)

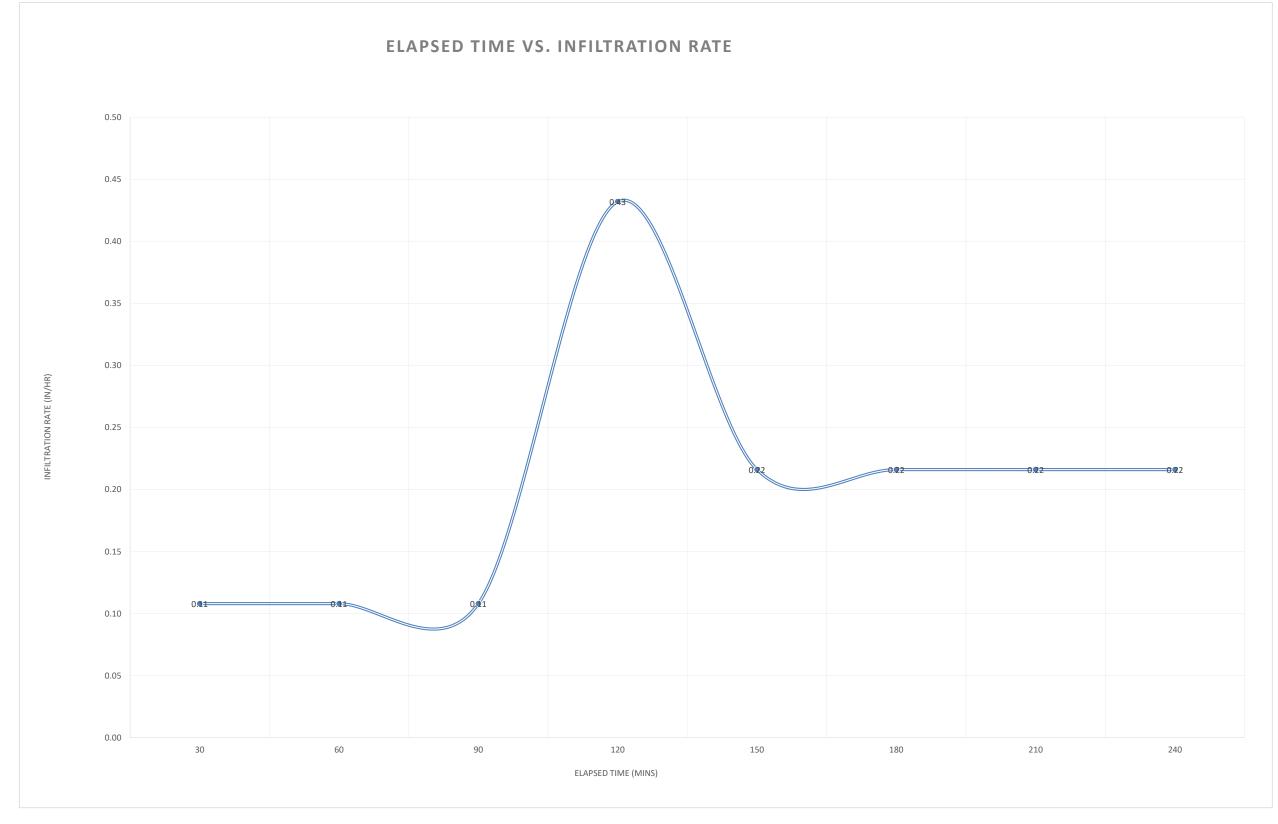
rth Strata Geotechnical Services, Inc.

hnical, Environmental and Materials Testing Consultants

е	Remarks
at (c)	Weather conditions Etc
	urf-lec nternational

Project Identification:	182250-12	A	
Test Location:	DR-2		
Liquid Used:	TAP WATE	pH:	8.0
Tested By:	MM/DI		
Depth to water table:	> 30 Feet		

Earth Strata Geotechnical Services, Inc. Geotechnical, Environmental and Materials Testing Consultants



Project Identification: 182250-12A Constants Inter Cin2 Container Marriotte Tube Volume Test Location: DR-3 DR-3 Inner Ring 729 10.0 1 3000 Liquid Used: TAP WATE pH: 8.0 Annular Ring 2189 10.0 2 10000 Tested By: MM/DI Date 8/30/2018 Liquid level maintained (X) Flow Valve () Float Valve () Mariotte Tubes Depth to water table: > 30 Feet Depth of Test 4.5' Penetration Depth of Outer Ring: 9 cm Other Trial # Start / End Date Time Increment Inner Ring Annular Space Annular Annular Annular Annular Infiltration Infi	Test No.	DR-3	Location	5	See Map)		Tur	-Tec In	ternatic	<mark>onal - F</mark>	Record	Chart f	or IN10	<mark>)-W - (1</mark>	<mark>2 & 24 In</mark>	ich In
Test Location: DR-3 Inner King 729 10.0 1 3000 Tested By: MM/DI Date 8/30/2018 Liquid Used Flow VATE pH: 8.0 Annular Ring 2189 10.0 2 10000	-			A	1					Area cm2	Liquid (cm)	Container Number		Marriotte 1	Гube Volun		Earth Geotechni
Tested By: MM/DI Date 8/30/2018 Liquid level maintained (X) Flow Valve () Float Valve () Mariotte Tubes Depth to water table: > 30 Feet Depth of Test 4.5' Penetration Depth of Outer Ring: 9 cm Other Trial # Start / End Date MM/DI Time Increment (Total) Time Increment (Total) Time Increment (Total) Flow Readings Annular Space Readings Infiltration Rates Ground Tempera 1 End Test 8/30/2018 15:16 0:30 30 6:00 750 6:00 3000 2:06 0.81 2:74 1.08 2 End Test 8/30/2018 15:46 0:30 6:00 2:00 6:00 2:06 0.81 2:74 1.08 1.44 1.62 3:65 1.44 1.62 3:65 1.44 1.62 3:65 1.44 1.62 3:65 1.44 1.62 3:65 1.44 1.62 3:65 1.44 1.62 3:65 1.44 4:6 1.62 4:6 3:65 1.44 4:6	-							-									
Depth to water table: > 30 Feet Depth of Test 4.5' Penetration Depth of Outer Ring: 9 cm Other Trial # Start / End Date MM/DD/YY Time HR:MIN Time (Total) Time (Total) Time Increment (Total) Elapsed Time (Min) Inner Ring Reading cm Annular Space (m) Annular Space Marriotte Tube Flow (m) Annular Space (m) Inner Tugqid Annular Infiltration Rate cm/h Annular Infiltration Rate m/h Infiltration Rate m/h				pH:			0/0040	Annular Ri									
Trial # Start / End Date MM/DD/YY Time Increment (Total) Time Increment (Total) Elapsed Inner Ring Reading cm Inner Ring Maroitte Tube Flow (ml) Annular Space Reading cm Annular Space Marriotte Tube Flow (ml) Annular Inmer Maroitte Tube Flow (ml) Inner Inmer Maroitte Tube Flow (ml) Inner Inmer Maroitte Tube Flow (ml) Inner Inmer Maroitte Tube Flow (ml) Inner Inmer Infiltration Rate m/h Annular Infiltration Rate m/h Annular Infiltration Rate m/h Ground Tempera Annular Infiltration Rate m/h Ground Tempera Infiltration Rate m/h 1 Start Test 8/30/2018 15:16 0:30 30 6.00 750 6.00 3000 2.06 0.81 2.74 1.08 2 End Test 8/30/2018 16:16 0:30 60 6.00 2000 6.00 4000 5.49 2.16 3.65 1.44 3 End Test 8/30/2018 16:16 0:30 90 6.00 2250 6.00 4000 5.49 2.16 3.65 1.44 4 End Test 8/30/2018 17:16 2:00 120 6.00 45:00															lotte l'udes	i	
Trial # Start / End Date MM/DD/YY Time HR:MIN Time Increment (Total) Elapsed Time (Min) Inner Maroitte Tube Flow, (ml) Annular Space Reading cm Liquid Marriotte Tube Flow, (ml) Liquid Temp *F Inner Infiltration Rate In/n Annular Annular Reading cm Annular Marriotte (ml) Liquid Space Reading cm Inner Infiltration (ml) Inner Infiltration (ml) Annular Marriotte Tube Flow, (ml) Liquid Temp *F Inner Infiltration Rate In/n Annular Annular Reading cm Annular Marriotte (ml) Inner Space Reading cm Inner Marriotte Tube Flow, (ml) Liquid Temp *F Inner Infiltration Rate In/n Annular Annular Reading cm Annular Marriotte (ml) Inner Temp *F 1 End Test 8/30/2018 15:16 0:30 30 6.00 750 6.00 3000 2.06 0.81 2.74 1.08 6 2 End Test 8/30/2018 16:16 0:30 60 2000 6.00 4000 5.49 2.16 3.65 1.44 6 3 End Test 8/30/2018 16:46 0:30 120 6.00 2200 6.00 4000	Deptilito	water table.	2 30 1 661	Deptil of rest			4.5		i eneratio			•	3 011	other			
Trial # Start / End Date MM/DD/YV Time HR:MIN Imme (min) Elapsed Time (Min) Annular meating on (Min) Annular rule Annular space (min) Space Marriotte Tube Flow (min) Liquid Space Marriotte Tube Flow (min) Inner Temp % Inner Infiltration Rate cm/h Annular Infiltration Rate cm/h Annular Infiltratio								Flow R	eadings			Inf	iltration Rat	es		Ground Tem	perature
1 End Test 8/30/2018 15:46 0:30 30 6.00 750 6.00 3000 2.06 0.81 2.74 1.08 1.08 2 End Test 8/30/2018 15:46 0:30 60 6.00 2000 6.00 4000 5.49 2.16 3.65 1.44 1.60 1.00 <th>Trial #</th> <th>Start / End</th> <th></th> <th></th> <th>Increment</th> <th>Time</th> <th>Inner Ring</th> <th>Maroitte Tube Flow</th> <th>Space</th> <th>Space Marriotte Tube Flow</th> <th></th> <th>Infiltration</th> <th>Infiltration</th> <th>Infiltration</th> <th>Infiltration</th> <th></th> <th>Temp at Depth (c</th>	Trial #	Start / End			Increment	Time	Inner Ring	Maroitte Tube Flow	Space	Space Marriotte Tube Flow		Infiltration	Infiltration	Infiltration	Infiltration		Temp at Depth (c
1 End rest 8/30/2018 15:46 0:30 6.00 750 6.00 3000 2.06 0.01 2.14 1.06 2 End Test 8/30/2018 16:16 1.00 60 600 2000 6.00 4000 5.49 2.16 3.65 1.44 3 End Test 8/30/2018 16:16 1.00 600 2000 6.00 4000 5.49 2.16 3.65 1.44 4 End Test 8/30/2018 16:46 1:30 90 6.00 2000 6.00 4000 5.49 2.16 3.65 1.44 4 End Test 8/30/2018 16:46 1:30 90 6.00 2250 6.00 4500 6.17 2.43 4.11 1.62 5 End Test 8/30/2018 17:46 2:30 150 6.00 1350 6.00 4500 3.70 1.46 4.11 1.62 5 End Test 8/30/2018 17:46 2:30 160 150 6.00 4500 3.70 1.46 4.11																	
2 End Test 8/30/2018 16:16 1:00 00 6.00 2000 6.00 4000 5.49 2.16 3.65 1.44 1.44 3 End Test 8/30/2018 16:16 0:30 90 6.00 2000 6.00 4000 5.49 2.16 3.65 1.44 </td <td>1</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>6.00</td> <td>750</td> <td>6.00</td> <td>3000</td> <td></td> <td>2.06</td> <td>0.81</td> <td>2.74</td> <td>1.08</td> <td></td> <td></td>	1						6.00	750	6.00	3000		2.06	0.81	2.74	1.08		
Start Test 8/30/2018 16:16 0:30 90 6.00 2000 6.00 4000 5.49 2.16 3.65 1.44 4 Start Test 8/30/2018 16:46 0:30 120 6.00 2250 6.00 4000 5.49 2.16 3.65 1.44 4 End Test 8/30/2018 17:16 2:00 120 6.00 2250 6.00 4500 6.17 2.43 4.11 1.62 5 End Test 8/30/2018 17:16 0:30 150 6.00 1350 6.00 4500 3.70 1.46 4.11 1.62 5 End Test 8/30/2018 17:46 2:30 150 6.00 1350 6.00 4500 3.70 1.46 4.11 1.62 6 End Test 8/30/2018 18:16 3:00 180 6.00 6.00 6.00 5.08 2.00 5.48 2.16 7 End Test 8/30/2018	2			15:46			6.00	2000	6.00	4000		5.49	2.16	3.65	1.44		
3 End Test 8/30/2018 16:46 11:30 6:00 2000 6:00 4000 5:49 2:10 3:65 1:44 1:40 1:40 4 End Test 8/30/2018 16:46 0:30 120 6:00 2250 6:00 4000 5:49 2:10 3:65 1:44 1:62 4 End Test 8/30/2018 17:16 0:30 120 6:00 2250 6:00 4500 6:17 2:43 4:11 1:62 6:00 6:00 1:50 6:00 4500 6:00 6:07 2:43 4:11 1:62 6:00 6:00 1:50 6:00 4500 6:00 6:07 2:43 4:11 1:62 6:00 6:00 1:50 6:00 4500 6:00 6:07 7:44 4:11 1:62 6:00 6:00 1:50 6:00 4500 3:70 1:46 4:11 1:62 6:00 6:00 1:50 6:00 6:00 6:00 6:00 6:00 6:00 6:00 6:00 6:00 6:00 6:00 6:00 <		Start Test	8/30/2018	16:16	0:30	00											
4 End Test 8/30/2018 17:16 2:00 120 6.00 2250 6.00 4500 6.17 2.43 4.11 1.62 5 Start Test 8/30/2018 17:16 0:30 150 6.00 1350 6.00 4500 3.70 1.46 4.11 1.62 5 End Test 8/30/2018 17:46 2:30 150 6.00 1350 6.00 4500 3.70 1.46 4.11 1.62 6 End Test 8/30/2018 17:46 0:30 180 6.00 1850 6.00 6000 5.08 2.00 5.48 2.16 6 End Test 8/30/2018 18:16 0:30 210 6.00 1750 6.00 4500 4.80 5.08 2.00 5.48 2.16 7 End Test 8/30/2018 18:46 3:30 210 6.00 1750 6.00 4500 4.80 1.89 4.11 1.62 7 End Test 8/30/2018 18:46 3:30 210 6.00 1750 <	3						6.00	2000	6.00	4000		5.49	2.16	3.65	1.44		
Start Test 8/30/2018 17:16 0:30 0:30 150 Image: constraint of the start of the	4						6.00	2250	6.00	4500		6.17	2.43	4.11	1.62		
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	_				0:30	150									4.00		
6 End Test 8/30/2018 18:16 3:00 180 6.00 1850 6.00 6000 5.08 2.00 5.48 2.16 7 Start Test 8/30/2018 18:16 0:30 210 6.00 1750 6.00 4500 4.80 1.89 4.11 1.62	5						6.00	1350	6.00	4500		3.70	1.46	4.11	1.62		
7 End Test 8/30/2018 18:46 3:30 210 6.00 1750 6.00 4500 4.80 1.89 4.11 1.62 Start Test 8/30/2018 18:46 0:30 240 0	6	End Test	8/30/2018	18:16	3:00	180	6.00	1850	6.00	6000		5.08	2.00	5.48	2.16		
	7		8/30/2018			210	6.00	1750	6.00	4500		4.80	1.89	4.11	1.62		
	8						6.00	1750	6.00	4500		4.80	1.89	4.11	1.62		
								-		-							
										-							
										-							
								-		-							
										-							
Image: state of the state o																	
										-		-					
Image: state in the state i																	
$ \begin{bmatrix} 1 & 1 & 1 & 1 & 1 \\ 1 & 1 & 1 & 1 & 1 \\ 1 & 1 &$												-					
												<u> </u>					
Image: state in the state																	
Image: state of the state																	

nfiltration Rings)

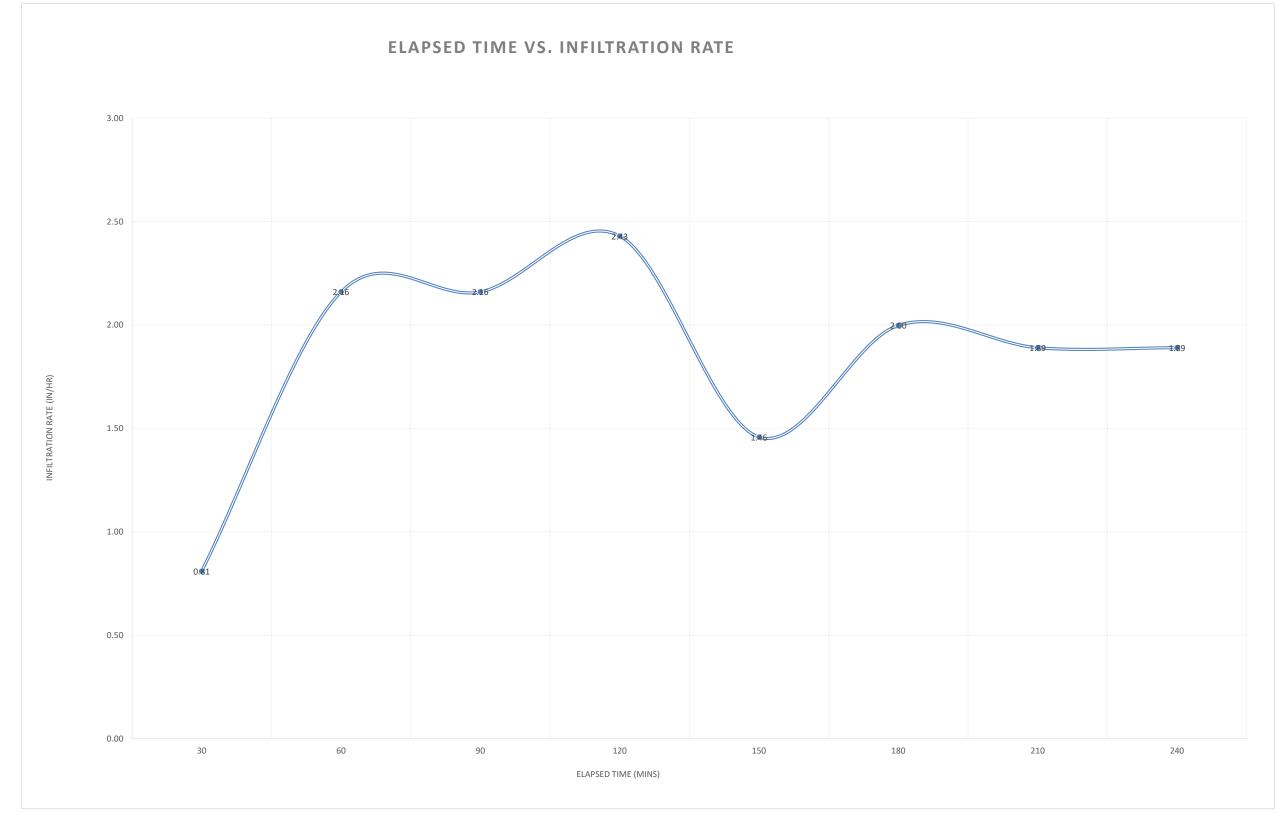
rth Strata Geotechnical Services, Inc.

hnical, Environmental and Materials Testing Consultants

е	Remarks
at (c)	Weather conditions Etc
	urf-lec nternational

Project Identification:	182250-12	A	
Test Location:	DR-3		
Liquid Used:	TAP WATE	pH:	8.0
Tested By:	MM/DI		
Depth to water table:	> 30 Feet		

Earth Strata Geotechnical Services, Inc. Geotechnical, Environmental and Materials Testing Consultants



	Test No.	DR-4	Location	5	See Map	ט		Turf	-Tec In	ternatic	<mark>onal - F</mark>	Record	Chart f	or IN10	<mark>)-W - (1</mark>	<mark>2 & 24 In</mark>	ch In
Test Location: DR-4 Inner Ring 7/29 10.0 1 3000 3000 Tested By: MM/DI Date 8/30/2018 Liquid Used Tested By: Other 10000 1 100000 100000 1000				A	1					Area cm2	Liquid (cm)	Container Number		Marriotte 1	Γube Volun		Geotechni
Tested By: MM/D Date 8/30/2018 Liquid level maintained (X) Flow Valve () Float Valve () Mariotte Tubes Depth of Test 4.5' Penetration Depth of Outer Ring: 9 cm Other Trial # Start / End Date 8/30/2018 Inner Ring Reading in (Total) Inner Ring Reading in (Total) Inner Ring Reading in (Total) Annular Space (Reading in (T	-																
Depth to water table: > 30 Feet Depth of Test 4.5' Penetration Depth of Outer Ring: 9 cm Other Trial # Start / End Date MM/DD/YY Time HR:MN Time Increment ((Total) Time Increment ((Total) Flow Readings Annular Space (m) Annular Space Reading in Annular Space (m) Annular Space Reading in Infiltration (m) Infiltration Rate m/h Annular Infiltration Rate m/h Annular Infiltration				pH:			0/2019	Annular Ri						n () Mari	otto Tuboo		
Trial # Start / End Date MM/DD/YY Time HR:MIN Time Increment (Total) Inner Increment (Total) Inner Maroitte Tube Flow (ml) Inner Indrement Temp at Space Reading in Tube Flow (ml) Inner Instruct Temp at Space Inner Infiltration Rate In/h Annular Infiltration Rate In/h Annular Infiltra		•												<u> </u>	otte ludes		
Trial # Start / End Date MM/DD/Y Time HR:MIN Time Increment (Total) Elapsed Fine (Min) Inner Marrite (Min) Inner Marrite (Min) Annular Marrite (Min) Annul	Deptil to	water table.	2 30 1 661		Deptilo	1631	4.5		renetration			•	3 011	Other			
Trial # Start / End Date MM/DD/YY Time R:MM/D/Y Time (Total) Time (Total) Inner (Total) Inner (Total) Annular Maritier Space Reading in (m) Space Reading in (m) Space Reading in (m) Space Reading in (m) Liquid Instruct (m) Inner Instruct (m)								Flow R	eadings			Inf	iltration Rat	tes		Ground Tem	perature
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	Trial #	Start / End			Increment	Time		Maroitte Tube Flow	Space	Space Marriotte Tube Flow		Infiltration	Infiltration	Infiltration	Infiltration		Temp at Depth (c
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $																	
2 End Test 8/30/2018 16:39 1:00 600 4.00 925.83 1:00 5560 2.54 1.00 5.08 2.00 3 End Test 8/30/2018 16:39 0:30 90 4.00 2.00 2.00 2.54 1.00 2.56 1.57	1											5.08	2.00	7.62	3.00		
Start Test 8/30/2018 16:39 0:30 90 4.00 2.00 2.780 2.54 1.00 2.54 1.00 4 End Test 8/30/2018 17:09 0:30 120 3.50 925 1.50 2780 2.54 1.00 2.54 1.00 4 End Test 8/30/2018 17:09 0:30 120 3.50 1.50 2780 0.63 0.25 1.00 2.54 1.00<	2											2.54	1.00	5.08	2.00		
3 End Test 8/30/2018 17:09 1:30 90 3:50 925 1:50 2780 2.54 1:00 2.54	_	Start Test	8/30/2018	16:39	0:30	00	4.00		2.00			2.04					
4 End Test 8/30/2018 17:39 2:00 120 3.38 230 1.25 1390 0.63 0.25 1.27 0.50 5 Start Test 8/30/2018 17:39 0:30 150 4.00 2.00 2.00 0.63 0.25 1.27 0.50 5 End Test 8/30/2018 18:09 2:30 150 3.88 230 1.75 1390 0.63 0.25 1.27 0.50 6 Start Test 8/30/2018 18:09 0:30 3.88 1.75 1390 0.63 0.25 1.27 0.50 6 End Test 8/30/2018 18:39 3:00 3.88 1.75 1390 0.63 0.25 1.27 0.50 6 End Test 8/30/2018 18:39 3:00 3.75 230 1.50 1390 0.63 0.25 1.27 0.50 7 End Test 8/30/2018 19:09 3:30 210 3.63 230 1.25 1390 0.63 0.25 1.27 0.50 0.50	3											2.54	1.00	2.54	1.00		
Start Test 8/30/2018 17:39 0:30 4.00 2.00 0.00 0.63 0.25 1.27 0.50 End Test 8/30/2018 18:09 2:30 1.00 3.88 230 1.75 1390 0.63 0.25 1.27 0.50 6 Start Test 8/30/2018 18:09 0:30 3.88 230 1.75 1390 0.63 0.25 1.27 0.50 6 End Test 8/30/2018 18:39 3:00 3.75 230 1.50 1390 0.63 0.25 1.27 0.50 7 Start Test 8/30/2018 18:39 0:30 210 3.75 230 1.50 1390 0.63 0.25 1.27 0.50 7 Start Test 8/30/2018 19:09 3:30 210 3.75 1.30 0.63 0.25 1.27 0.50 8/30/2018 19:09 3:30 210 3.63 230 1.25 1390 0.63 <td>4</td> <td></td> <td>0.63</td> <td>0.25</td> <td>1.27</td> <td>0.50</td> <td></td> <td></td>	4											0.63	0.25	1.27	0.50		
5 End Test 8/30/2018 18:09 2:30 3.88 2:30 1.75 1390 0.63 0.25 1.27 0.50 0.50 6 Start Test 8/30/2018 18:09 0:30 180 3.88 1.75 1390 0.63 0.25 1.27 0.50 0.50 6 End Test 8/30/2018 18:39 3:00 3.75 230 1.50 1390 0.63 0.25 1.27 0.50 0.50 7 End Test 8/30/2018 18:39 0:30 210 3.75 1.50 1.50 0.63 0.25 1.27 0.50 0.50 7 End Test 8/30/2018 19:09 3:30 210 3.75 1.50 1.50 0.63 0.25 1.27 0.50 0.50 8/30/2018 19:09 3:30 210 3.63 230 1.25 1390 0.63 0.25 1.27 0.50 0.50 8/30/2018 19:09 3:30 240 4.50 1.25 1390 0.63 0.25 1.27 <th< td=""><td></td><td>Start Test</td><td>8/30/2018</td><td>17:39</td><td>0:30</td><td>150</td><td>4.00</td><td></td><td>2.00</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<>		Start Test	8/30/2018	17:39	0:30	150	4.00		2.00								
6 End Test 8/30/2018 18:39 3:00 180 3.75 230 1.50 1390 0.63 0.25 1.27 0.50 7 Start Test 8/30/2018 18:39 0:30 210 3.75 230 1.50 1390 0.63 0.25 1.27 0.50 7 End Test 8/30/2018 19:09 3:30 210 3.63 230 1.25 1390 0.63 0.25 1.27 0.50 5 Start Test 8/30/2018 19:09 0:30 240 4.50 1.25 1390 0.63 0.25 1.27 0.50	5											0.63	0.25	1.27	0.50		
7 End Test 8/30/2018 19:09 3:30 210 3.63 230 1.25 1390 0.63 0.25 1.27 0.50 Start Test 8/30/2018 19:09 0:30 240 4.50 1.25 1390 0.63 0.25 1.27 0.50	6											0.63	0.25	1.27	0.50		
	7		8/30/2018									0.63	0.25	1.27	0.50		
	8											0.63	0.25	1.27	0.50		
								_		-							
										-							
Image								_				_					
Image: series of the series										-							
Image: state in the state i																	
								-		-							
										-		-					
										-							
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$								_		-		_					
										-							
														<u> </u>			
												_					

nfiltration Rings)

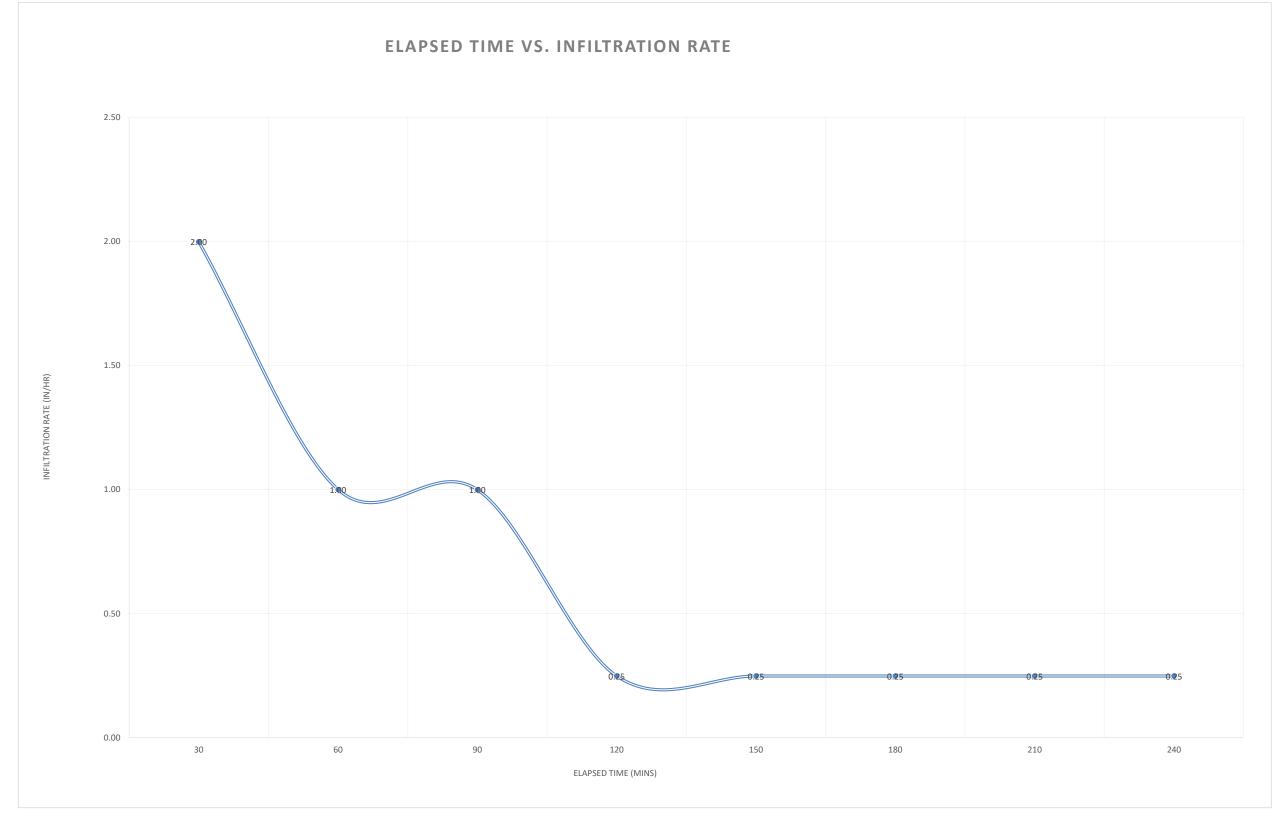
rth Strata Geotechnical Services, Inc.

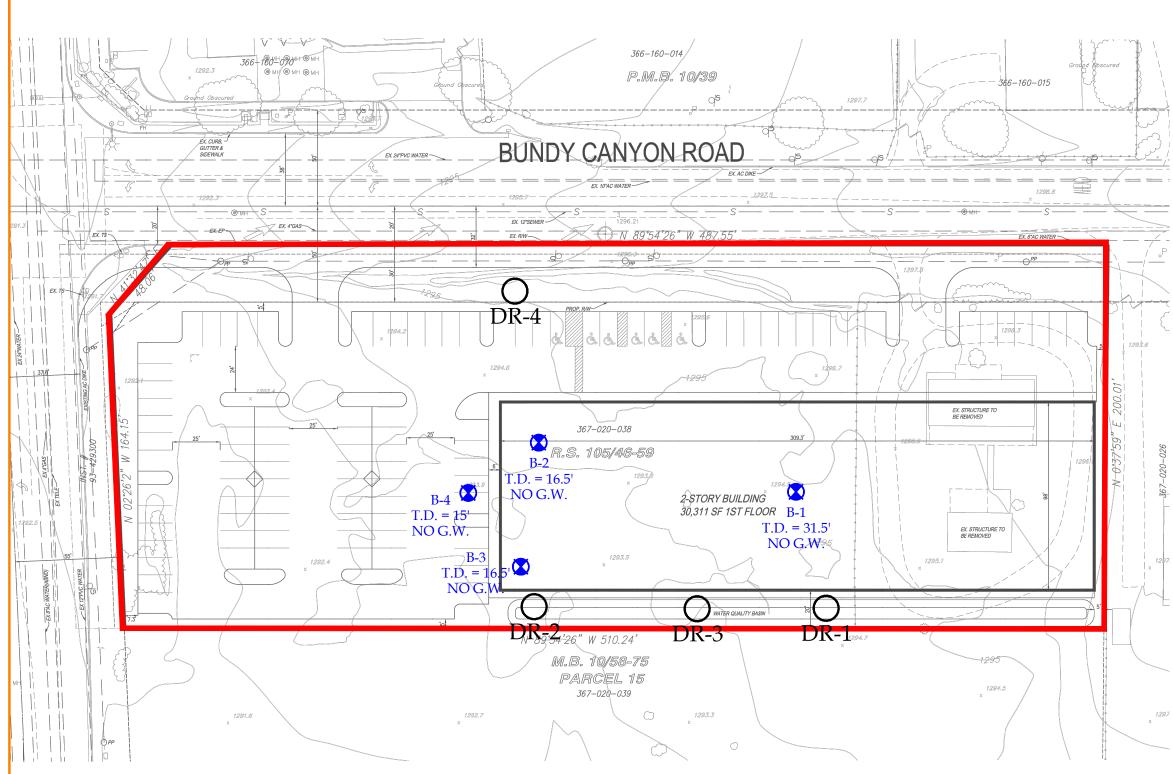
hnical, Environmental and Materials Testing Consultants

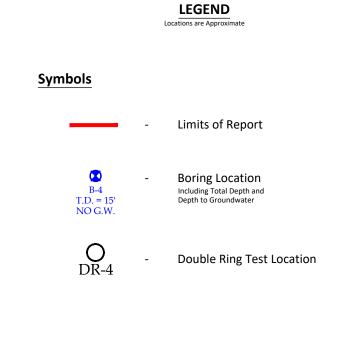
е	Remarks
at (c)	Weather conditions Etc
	urf-lec nternational

Project Identification:	182250-12	A	
Test Location:	DR-4		
Liquid Used:	TAP WATE	pH:	8.0
Tested By:	MM/DI		
Depth to water table:	> 30 Feet		

Earth Strata Geotechnical Services, Inc. Geotechnical, Environmental and Materials Testing Consultants









INFILTRATION MAP

LOCATED AT 34020 MISSION TRAIL CITY OF WILDOMAR, RIVERSIDE COUNTY, CALIFORNIA APN 367-020-038

PROJECT	MISSION TRAIL		
CLIENT	MR. DON MACLEAN		
PROJECT NO.	182250-12A		
DATE	SEPTEMBER 2018		
SCALE	1:50		
DWG XREFS			
REVISION			
DRAWN BY	JDG	PLATE	1 OF 1

Earth Strata Geotechnical Services, Inc.

Geotechnical, Environmental and Materials Testing Consultants

Appendix 4: Historical Site Conditions

Phase I Environmental Site Assessment or Other Information on Past Site Use

Examples of material to provide in Appendix 4 may include but are not limited to the following:

- Environmental Site Assessments conducted for the project,
- Other information on Past Site Use that impacts the feasibility of LID BMP implementation on the site.

This information should support the Full Infiltration Applicability, and Biofiltration Applicability sections of this Template. Refer to Section 2.3 of the SMR WQMP and Sections D of this Template.

Appendix 5: LID Feasibility Supplemental Information

Information that supports or supplements the determination of LID technical feasibility documented in Section D

Examples of material to provide in Appendix 5 may include but are not limited to the following:

- Technical feasibility criteria for DMAs
- Site specific analysis of technical infeasibility of all LID BMPs (if Alternative Compliance is needed)
- Documentation of Approval criteria for Proprietary Biofiltration BMPs

This information should support the Full Infiltration Applicability, and Biofiltration Applicability sections of this Template. Refer to Section 2.3 of the SMR WQMP and Sections D of this Template.

Appendix 6: LID BMP Design Details

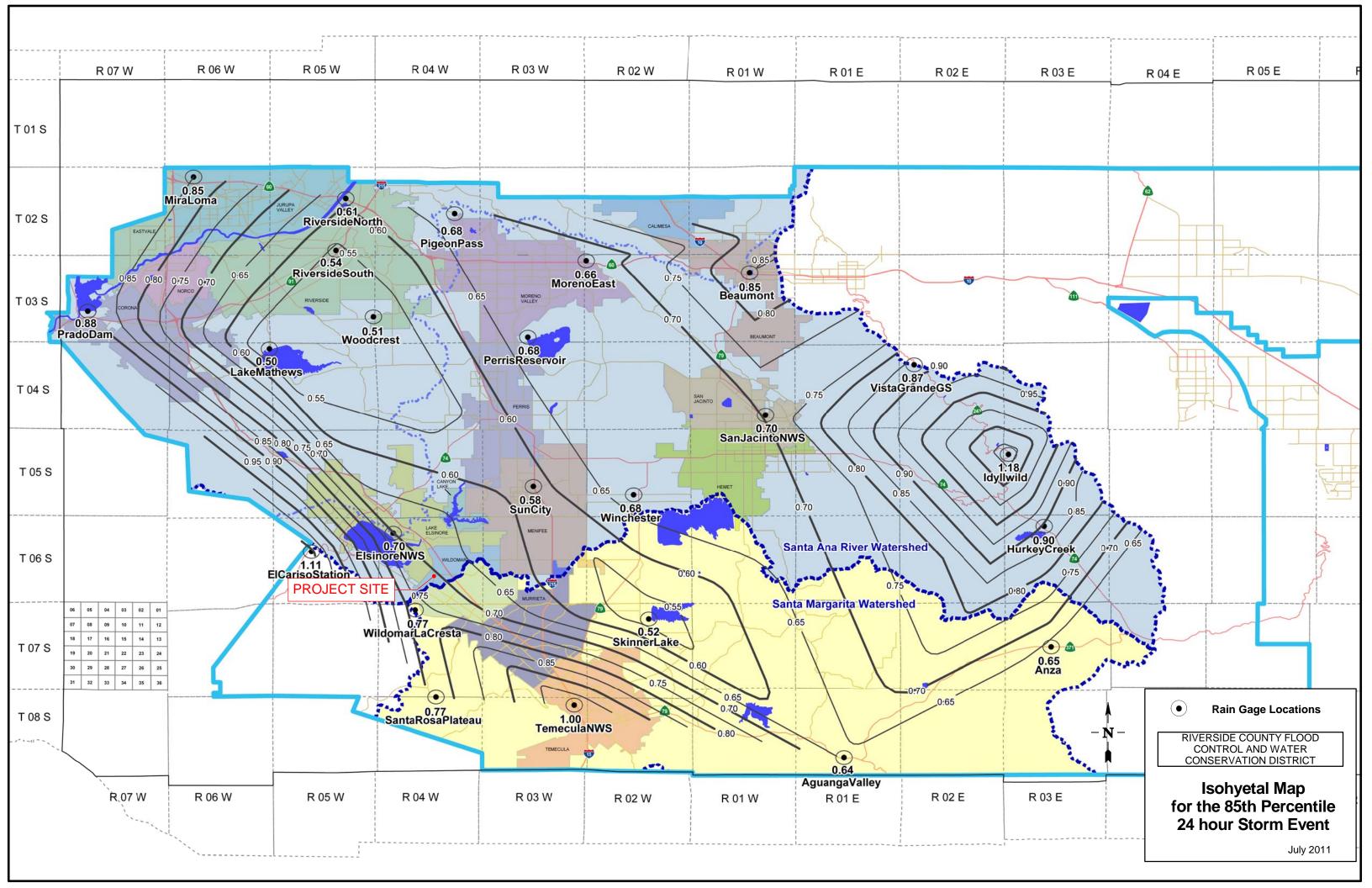
BMP Sizing, Design Details and other Supporting Documentation to supplement Section D

Examples of material to provide in Appendix 6 may include but are not limited to the following:

- DCV calculations,
- LID BMP sizing calculations from Exhibit C of the SMR WQMP
- Design details/drawings from manufacturers for proprietary BMPs

This information should support the Full Infiltration Applicability, and Biofiltration Applicability sections of this Template. Refer to Section 3.4 of the SMR WQMP and Sections D.4 of this Template.

Rainfall Map



Santa Margarita VBMP Spreadsheet

	<mark>Iargarita W</mark> 1 Volume, V _{BMP}		Legend:		_ `	uired Entries sulated Cells
		be used in conjunction with	BMP designs fro	m the LID BMF		
Company Name	JLC Engineerin	g		Date 6/	4/2019	
Designed by	CRC		County/Ci	ty Case No		
Company Project Nu	mber/Name	KCG Blue				
Drainage Area Numb	per/Name	DMA A				
Enter the Area Tribut	-		-	.07 acres		
85 th Per	centile, 24-hour	Rainfall Depth, from th	ne Isohyetal Ma	ap in Handbo	ok Appendix	E
Site Location				Township	6S	
				Range	4W	
				Section	27	
Enter the 85 th Pe	ercentile, 24-hour	Rainfall Depth		D ₈₅ =	0.72	
	D	etermine the Effective	Impervious Fra	action		
Type of post-dev (use pull down r	velopment surfac	e cover	Mixed Surfac	e Types		
Effective Imperv				$I_f =$	0.91	
	Calculate the cor	nposite Runoff Coeffic	ient, C for the	BMP Tributa	ry Area	
		on the WEF/ASCE M			-	
	$78I_{f}^{2} + 0.774I_{f} + 0$		emou	C =	0.74	
		Determine Design Stor	age Volume, V	/ _{BMP}		
Calculate V _U , th	e 85% Unit Stora	age Volume $V_U = D_{85}$	x C	$V_u =$	0.54	(in*ac)/ac
Calculate the de	sign storage volu	me of the BMP, V_{BMP} .				
V_{BMP} (ft ³)=		$A_{\rm T}$ (ac) x 43,560 (ft 12 (in/ft)	2/ac)	V _{BMP} =	4,058	ft ³
Notes:						

Biofiltration Design Spreadsheet

Biofiltration with Pa	artial Infiltration Facility -	BMP ID	T 1	Required	Entries	
Desig	n Procedure	А	Legend:	Calculate		
Company Name:	JLC Enginee	ring		Date:	6/4/2019	
Designed by:	Jilleen Ferr		County/City	y Case No.:	KCG	
		Design Volume				
Enter the area	tributary to this feature			$A_T =$	2.07	acres
Enter V _{BMP} de	termined from Section 2.1 o	f this Handbook		V _{BMP} =	4,058	ft ³
	timate of footprint of BMP, ributary impervious area)	Area _{BMP} (Guidance: A	A reasonable starting	Area _{BMP} =	1,863	ft ²
should be the con ponding elevation	hall be measured at the mid-pondi tour that is midway between the f n of the basin. The underlying grav ems with vertical walls, the effecti	loor of the basin and th vel layer (infiltration st	e maximum water c orage layer) should	quality		
	Portion of	of DCV Reliably R	etained			
Depth of Grave	l Infiltration Storage Layer (18	" minimum; 30" ma	ximum)	dg=	18.0	inches
	Reliably Retained via Infiltrat	-	el Layer			23
$V_{\text{retained}} = d_g$	$_{g}(in) \ge 0.4 \ge Area_{BMP}(ft^{2}) \ge 0.4 = 0.$	1/12		$V_{Retained} =$	1117.8	ft^3
Portion of V _{BN}	AP not Reliably Retained					
V _{Not Reliably}	$_{\text{Retained}} = V_{\text{BMP}} - V_{\text{Retained}}$		$V_{ m Not \ Reli}$	ably Retained =	2940.2	ft^3
	Biofiltration with Pa	artial Retention Fac	cility Surface Are	ea		
Depth of Surfa	ace Ponding Layer (6" minin	num, 12" maximur	n)	$d_{\rm P} =$	6.0	inches
Depth of Engin	neered Soil Media (24" to 36	6"; 18" if vertically	constrained)	$d_s =$	36.0	inches
Design Media	Filtration Rate (2.5 in/hr)			$I_{design} =$	2.5	in/hr
Allowable Rou	uting Period, T _{routing} (5 hrs)			$T_{routing} =$	5.0	hr
	iltration Depth, d_{E_bio} $(d_P + (0.3 \text{ x } d_S) + (I_{design} * T_{design})$	(ft)		$d_{E_{bio}} =$	2.4	ft
1	c Depth, $d_{E_bio_static}$ = (d_P + (0.3 * d_S)) (ft)		($d_{E_{bio_{static}}} = $	1.4	ft
V _{biofiltered} =	$d_{E_{bio}} * Area_{BMP}$			$V_{biofiltered} =$	4548.8	ft ³
V _{biofiltered_sta}	$_{tic} = d_{E_{bio_{static}}} * Area_{BMP}$		V_{bic}	ofiltered_static =	2608.2	ft ³
	Siz	ing Option 1 Resu	lt			

Riverside County-SMR LID BMP Design Handbook February 2018

	Criteria 1:	$V_{biofiltered (with routing)} > 150\% of V_{not reliably retained}$	Results:	PASS				
		Sizing Option 2 Result						
	Criteria 2:	$V_{biofiltered_static} > 0.75 \text{ x } V_{Not Reliably Retained}$	Results:	PASS				
		Note						
	If neither of these criteria are met, then increase retention depth, increase footprint, or both, and rerun calculations. This calculation is inherently iterative.							
		Biofiltration with Partial Retention Facility Properties						
	Side Slopes in	n Partial Retention with Biofiltration Facility	z =	4 :1				
	Diameter of U	Jnderdrain		6 inches				
	Longitudinal	Slope of Site (3% maximum)		1.1 %				
	Check Dam S	Spacing		0 feet				
	Describe Veg	etation:						
Notes	•							

100-Year, 1-Hour Unit Hydrograph

Unit Hydrograph Analysis Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1 Study date 08/30/19 File: POST11.out _____ Riverside County Synthetic Unit Hydrology Method RCFC & WCD Manual date - April 1978 Program License Serial Number 433 _____ English (in-lb) Input Units Used English Rainfall Data (Inches) Input Values Used English Units used in output format _____ KCG BLUE POST-PROJECT CONDITION HYDROLOGY UNIT HYDROGRAPH ANALYSIS, 1-YEAR, 1-HOUR STORM EVENT AREA TRIBUTARY TO BIORETENTION BASIN "A" FILENAME: POST _____ Drainage Area = 2.07(Ac.) = 0.003 Sq. Mi. Drainage Area for Depth-Area Areal Adjustment = 2.07(Ac.) = 0.003 Sq. Mi. USER Entry of lag time in hours Lag time = 0.086 Hr. Lag time = 5.17 Min. 25% of lag time = 1.29 Min. 40% of lag time = 2.07 Min. Unit time = 5.00 Min. Duration of storm = 1 Hour(s)User Entered Base Flow = 0.00(CFS) 2 YEAR Area rainfall data: Rainfall(In)[2] Weighting[1*2] Area(Ac.)[1] 1.30 2.07 0.63 100 YEAR Area rainfall data: Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2] 2.07 1.55 3.21 STORM EVENT (YEAR) = 1.00 Area Averaged 2-Year Rainfall = 0.630(In) Area Averaged 100-Year Rainfall = 1.550(In) Point rain (area averaged) = 0.467(In) Areal adjustment factor = 100.00 % Adjusted average point rain = 0.467(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 2.070 68.90 0.900 Total Area Entered = 2.07(Ac.) RI RI Infil. Rate Impervious Adj. Infil. Rate Area% F
 AMC2
 AMC-1
 (In/Hr)
 (Dec.)
 (In/Hr)

 68.9
 49.7
 0.575
 0.900
 0.109
 1.000
 0.109

 Sum (F) =
 0.109
 0.109
 0.109
 0.109
 0.109

Area averaged mean soil loss (F) (In/Hr) = 0.109
Minimum soil loss rate ((In/Hr)) = 0.055
(for 24 hour storm duration)
Soil low loss rate (decimal) = 0.720
Slope of intensity-duration curve for a 1 hour storm =0.4800

Unit Hydrograph VALLEY S-Curve

	ime period rs)	Time % of lag	Distributio Graph %	n Unit Hydrograg (CFS)
1	0.083	96.787	18.216	0.380
2	0.167	193.573	48.105	1.004
3	0.250	290.360	16.147	0.337
4	0.333	387.147	7.237	0.151
5	0.417	483.933	4.110	0.086
6	0.500	580.720	2.666	0.056
7	0.583	677.507	1.703	0.036
8	0.667	774.293	1.075	0.022
9	0.750	871.080	0.741	0.015
		Su	m = 100.000	Sum= 2.086

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit			Storm Rain				
		Percent		Max			
1			0.247		(0.178)		
2	0.17		0.252	0.109	(0.182)	0.14	13
3			0.303		(0.218)		93
4	0.33	5.40	0.303	0.109	(0.218)	0.19	93
5	0.42	5.70	0.319	0.109	(0.230)	0.21	LO
6	0.50	6.40	0.359	0.109	(0.258)	0.24	19
7	0.58	7.90	0.443	0.109	(0.319)	0.33	33
8	0.67	9.10	0.510	0.109	(0.367)	0.40	01
9	0.75	12.80	0.717	0.109	(0.516)	0.60	08
10	0.83	25.60	1.435	0.109	(1.033)	1.32	25
11	0.92	7.90	0.443	0.109	(0.319)	0.33	33
12	1.00		0.275	0.109	(0.198)	0.16	55
		(Loss R	ate Not Used)				
S	um =	100.0			Sum =	4.3	
	Flood	volume = E	ffective rain	fall 0.3	36(In)		
			2.1(Ac.)/[(:			Ac.Ft)	
			= 0.11(In		(-		
			= 0.019 (Ad				
			0.47(In)				
			2687.8				
	Total	soil loss	= 821	1 Cubic Feet	F		
			of this hydrog				
	+++++	++++++++++	+++++++++++++++++++++++++++++++++++++++			++++++++++	+++++
				R STOI			
			lunoff				
			ograph in 5			FS))	
Time	(h+m) V		t Q(CFS) 0				10 0
0+	5	0.0004	0.05 O	1		1	1
			0.19 QV			ĺ	İ
			0.26 QV				
			0.34 Q V				
51		5.0050	0.01 10 1	I	I	I	I

0+25	0.0084	0.37	Q V				
0+30	0.0113	0.42	Q V		İ		Í
0+35	0.0148	0.51	Q V		İ		Í
0+40	0.0192	0.64	Q	V	İ		Í
0+45	0.0248	0.82	Q	V	İ		Í
0+50	0.0341	1.35	Q		V	İ	Í
0+55	0.0464	1.78	Q		i	v	i
1+ 0	0.0533	1.01	Q		İ	V	Í
1+ 5	0.0572	0.57	Q		İ	V	Í
1+10	0.0592	0.28	Q		İ		vİ
1+15	0.0603	0.16	Q		İ		V
1+20	0.0610	0.10	Q		i	ĺ	v
1+25	0.0614	0.06	Q		İ		vİ
1+30	0.0616	0.03	Q		İ		vj
1+35	0.0617	0.01	Q		İ		V
1+40	0.0617	0.00	Q		İ		V

Appendix 7: Hydromodification

Supporting Detail Relating to compliance with the Hydromodification Performance Standards

Examples of material to provide in Appendix 7 may include but are not limited to the following:

- Hydromodification Exemption Exhibit,
- Potential Critical Coarse Sediment Yield Area Mapping
- Hydromodification BMP sizing calculations,
- SMRHM report files,
- Site-Specific Critical Coarse Sediment Analysis,
- Design details/drawings from manufacturers for proprietary BMPs

This information should support the hydromodification exemption (if applicable) and hydrologic control BMP and Sediment Supply BMP sections of this Template. Refer to Section 2.4 and 3.6 of the SMR WQMP and Sections E of this Template.

The project site discharges to Lake Elsinore, which is in the Santa Ana Watershed. Based upon the HCOC Applicability Map from the Hydromodification Susceptibility Documentation Report and Mapping for the Santa Ana Region, the project site is exempt from addressing hydromodifications. However, the project will treat the required TMDL's and Constituents of Concern as required by the Santa Margarita Watershed WQMP.

Appendix 8: Source Control

Pollutant Sources/Source Control Checklist

Include a copy of the completed Pollutant Sources/Source Control Checklist used to document Source Control BMPs in Section H of this Template.

Appendix 9: O&M

Operation and Maintenance Plan and Documentation of Finance, Maintenance and Recording Mechanisms

Include the completed Operation and Maintenance Plan in this Appendix along with additional documentation of Finance and Maintenance Recording Mechanisms for the site. Refer to Sections 3.10 and 5 of the SMR WQMP and Section J of this Template.

Appendix 10: Educational Materials

BMP Fact Sheets, Maintenance Guidelines and Other End-User BMP Information

Examples of material to provide in Appendix 10 may include but are not limited to the following:

- BMP Fact Sheets for proposed BMPs form Exhibit C: LID BMP Design Handbook of the SMR WQMP,
- Source control information and training material for site owners and operators,
- O&M training material,
- Other educational/training material related to site drainage and BMPs.





andscaping and garden maintenance activities can be major contributors to water pollution. Soils, yard wastes, over-watering and garden chemicals become part of the urban runoff mix that winds its way through streets, gutters and storm drains before entering lakes, rivers, streams, etc. Urban runoff pollution contaminates water and harms aquatic life!

In Riverside County, report illegal discharges into the storm drain, call 1-800-506-2555 "Only Rain Down the Storm Drain"

Important Links:

Riverside County Household Hazardous Waste Collection Information 1-800-304-2226 or <u>www.rivcowm.org</u>

> Riverside County Backyard Composting Program 1-800-366-SAVE

Integrated Pest Management (IPM)Solutions www.ipm.ucdavis.edu

California Master Gardener Programs www.mastergardeners.org www.camastergardeners.ucdavis.edu

California Native Plant Society www.cnps.org

The Riverside County "Only Rain Down the Storm Drain" Pollution Prevention Program gratefully acknowledges Orange County's Storm Water Program for their contribution to this brochure.



...Only Rain Down ...the Storm Drain

What you should know for... Landscape and Gardening

Best Management tips for:

- Professionals
- Novices
- Landscapers
- Gardeners
- Cultivators





Tips for Landscape & Gardening

This brochure will help you to get the most of your lawn and gardening efforts and keep our waterways clean. Clean waterways provide recreation, establish thriving fish habitats, secure safe sanctuaries for wildlife, and add beauty to our communities. NEVER allow gardening products or waste water to enter the street, gutter or storm drain.

General Landscaping Tips

- Protect stockpiles and materials from wind and rain by storing them under tarps or secured plastic sheeting.
- Prevent erosion of slopes by planting fastgrowing, dense ground covering plants. These will shield and bind the soil.
- Plant native vegetation to reduce the amount of water, fertilizers and pesticides applied to the landscape.
- Never apply pesticides or fertilizers when rain is predicted within the next 48 hours.

Garden & Lawn Maintenance

Do not overwater. Use irrigation practices such as drip irrigation, soaker hoses or microspray systems. Periodically inspect and fix leaks and misdirected sprinklers. • Do not rake or blow leaves, clippings or pruning waste into the street, gutter or storm

drain. Instead, dispose of green waste by composting, hauling it to a permitted landfill, or recycling it through your city's program.



- Consider recycling your green waste and adding "nature's own fertilizer" to your lawn or garden.
- Read labels and use only as directed. Do not over-apply pesticides or fertilizers. Apply to spots as needed, rather than blanketing an entire area.
- Store pesticides, fertilizers and other chemicals in a dry covered area to prevent exposure that may result in the deterioration of containers and packaging.
- Rinse empty pesticide containers and re-use rinse water as you would use the product. Do not dump rinse water down storm drains or sewers. Dispose of empty containers in the trash.
- When available, use non-toxic alternatives to traditional pesticides, and use pesticides specifically designed to control the pest you are targeting.

- Try natural long-term common sense solutions first. Integrated Pest Management (IPM) can provide landscaping guidance and solutions, such as:
 - Physical Controls Try hand picking, barriers, traps or caulking holes to control weeds and pests.
 - Biological Controls Use predatory insects to control harmful pests.
 - Chemical Controls Check out <u>www.ipm.ucdavis.edu</u> before using chemicals. Remember, all chemicals should be used cautiously and in moderation.
- If fertilizer is spilled, sweep up the spill before irrigating. If the spill is liquid, apply an absorbent material such as cat litter, and then sweep it up and dispose of it in the trash.
- Take unwanted pesticides to a Household Waste Collection Center to be recycled.
- Dumping toxics into the street, gutter or storm drain is illegal!

<u>www.bewaterwise.com</u> Great water conservation tips and drought tolerant garden designs.

<u>www.ourwaterourworld.com</u> Learn how to safely manage home and garden pests.

Additional information can also be found on the back of this brochure.

Helpful telephone numbers and links:

Riverside County Stormwater	Protection Partners
Flood Control District	(951) 955-1200
County of Riverside	(951) 955-1000
City of Banning	(951) 922-3105
City of Beaumont	(951) 769-8520
City of Calimesa	(909) 795-9801
City of Canyon Lake	(951) 244-2955
Cathedral City	(760) 770-0327
City of Coachella	(760) 398-4978
City of Corona	(951) 736-2447
City of Desert Hot Springs	(760) 329-6411
City of Eastvale	(951) 361-0900
City of Hemet	(951) 765-2300
City of Indian Wells	(760) 346-2489
City of Indio	(760) 391-4000
City of Lake Elsinore	(951) 674-3124
City of La Quinta	(760) 777-7000
City of Menifee	(951) 672-6777
City of Moreno Valley	(951) 413-3000
City of Murrieta	(951) 304-2489
City of Norco	(951) 270-5607
City of Palm Desert	(760) 346-0611
City of Palm Springs	(760) 323-8299
City of Perris	(951) 943-6100
City of Rancho Mirage	(760) 324-4511
City of Riverside	(951) 361-0900
City of San Jacinto	(951) 654-7337
City of Temecula	(951) 694-6444
City of Wildomar	(951) 677-7751

REPORT ILLEGAL STORM DRAIN DISPOSAL 1-800-506-2555 or e-mail us at <u>fcnpdes@rcflood.org</u>

 Riverside County Flood Control and Water Conservation District <u>www.rcflood.org</u>

Online resources include:

- California Storm Water Quality Association
 <u>www.casqa.org</u>
- State Water Resources Control Board <u>www.waterboards.ca.gov</u>
- Power Washers of North America
 <u>www.thepwna.org</u>

Stormwater Pollution

What you should know for ..

Outdoor Cleaning Activities and Professional Mobile Service Providers



Storm drain pollution prevention information for:

- Car Washing / Mobile Detailers
- Window and Carpet Cleaners
- Power Washers
- Waterproofers / Street Sweepers
- Equipment cleaners or degreasers and all mobile service providers

Do you know where street flows actually go?

Storm drains are NOT connected to sanitary sewer systems and treatment plants!



The primary purpose of storm drains is to carry rain water away from developed areas to prevent flooding. Pollutants discharged to storm drains are transported directly into rivers, lakes and streams. Soaps, degreasers, automotive fluids, litter and a host of materials are washed off buildings, sidewalks, plazas and parking areas. Vehicles and equipment must be properly managed to prevent the pollution of local waterways.

Unintentional spills by mobile service operators can flow into storm drains and pollute our waterways. Avoid mishaps. Always have a Spill Response Kit on hand to clean up unintentional spills. Only emergency <u>Mechanical</u> repairs should be done in City streets, using drip pans for spills. <u>Plumbing</u> should be done on private property. Always store chemicals in a leak-proof container and keep covered when not in use. <u>Window/Power</u> <u>Washing</u> waste water shouldn't be released into the streets, but should be disposed of in a sanitary sewer, landscaped area or in the soil. Soiled <u>Carpet Cleaning</u> wash water should be filtered before being discharged into the sanitary sewer. Dispose of all filter debris properly. <u>Car Washing/Detailing</u> operators should wash cars on private property and use a regulated hose nozzle for water flow control and runoff prevention. Capture and dispose of waste water and chemicals properly. Remember, storm drains are for receiving rain water runoff only.

REPORT ILLEGAL STORM DRAIN DISPOSAL 1-800-506-2555

Help Protect Our WaterWays! Use these guidelines for Outdoor Cleaning Activities and Wash Water Disposal

Did you know that disposing of pollutants into the street, gutter, storm drain or body of water is PROHIBITED by law and can result in stiff penalties?

Best Management Practices

Waste wash water from Mechanics, Plumbers, Window/Power Washers, Carpet Cleaners, Car Washing and Mobile Detailing activities may contain significant quantities of motor oil, grease, chemicals, dirt, detergents, brake pad dust, litter and other materials.

Best Management Practices, or BMPs as they are known, are guides to prevent pollutants from entering the storm drains. *Each of us* can do our part to keep stormwater clean by using the suggested BMPs below:

Simple solutions for both light and heavy duty jobs:

Do...consider dry cleaning methods first such as a mop, broom, rag or wire brush. Always keep a spill response kit on site.

Do...prepare the work area before power cleaning by using sand bags, rubber mats, vacuum booms, containment pads or temporary berms to keep wash water <u>away</u> from the gutters and storm drains.

Do...use vacuums or other machines to remove and collect loose debris or litter before applying water.

Do...obtain the property owner's permission to dispose of *small amounts* of power washing waste water on to landscaped, gravel or unpaved surfaces.

Do...check your local sanitary sewer agency's policies on wash water disposal regulations before disposing of wash water into the sewer. (See list on reverse side)

Do...be aware that if discharging to landscape areas, soapy wash water may damage landscaping. Residual wash water may remain on paved surfaces to evaporate. Sweep up solid residuals and dispose of properly. Vacuum booms are another option for capturing and collecting wash water.

Do...check to see if local ordinances prevent certain activities.

Do not let...wash or waste water from sidewalk, plaza or building cleaning go into a street or storm drain.



Report illegal storm drain disposal Call Toll Free 1-800-506-2555

Using Cleaning Agents

Try using biodegradable/phosphate-free products. They are easier on the environment, but don't confuse them with being toxic free. Soapy water entering the storm drain system <u>can</u> impact the delicate aquatic environment.



When cleaning surfaces with a *high-pressure washer* or *steam cleaner*, additional precautions should be taken to prevent the discharge of pollutants into the storm drain system. These two methods of surface cleaning can loosen additional material that can contaminate local waterways.

Think Water Conservation

Minimize water use by using high pressure, low volume nozzles. Be sure to check all hoses for leaks. Water is a precious resource, don't let it flow freely and be sure to shut it off in between uses.

Screening Wash Water

Conduct thorough dry cleanup before washing exterior surfaces, such as buildings and decks *with loose paint*, sidewalks or plaza areas. Keep debris from entering the storm drain after cleaning by first passing the wash water through a "20 mesh" or finer screen to catch the solid materials, then dispose of the mesh in a refuse container. Do not let the remaining wash water enter a street, gutter or storm drain.

Drain Inlet Protection & Collection of Wash Water

- Prior to any washing, block all storm drains with an impervious barrier such as sandbags or berms, or seal the storm drain with plugs or other appropriate materials.
- Create a containment area with berms and traps or take advantage of a low spot to keep wash water contained.
- Wash vehicles and equipment on grassy or gravel areas so that the wash water can seep into the ground.
- Pump or vacuum up all wash water in the contained area.

Concrete/Coring/Saw Cutting and Drilling Projects

Protect any down-gradient inlets by using dry activity techniques whenever possible. If water is used, minimize the amount of water used during the coring/drilling or saw cutting process. Place a barrier of sandbags and/or absorbent berms to protect the storm drain inlet or watercourse. Use a shovel or wet vacuum to remove the residue from the pavement. Do not wash residue or particulate matter into a storm drain inlet or watercourse.



Anderstanding Stormwater A Citizen's Guide to



EPA 833-B-03-002 Bency United States

anuary 2003

or visit www.epa.gov/npdes/stormwater www.epa.gov/nps

For more information contact:

muois shi veila



What is stormwater runoff?

Why is stormwater runof



Stormwater runoff occurs when precipitation from rain or snowmelt flows over the ground. Impervious surfaces like driveways, sidewalks, and streets prevent stormwater from naturally soaking into the ground.

The effects of pollution

Polluted stormwater runoff can have many adverse effects on plants, fish, animals, and people.

- Sediment can cloud the water and make it difficult or impossible for aquatic plants to grow. Sediment also can destroy aquatic habitats.
- Excess nutrients can cause algae blooms. When algae die, they sink to the bottom and decompose in a process that removes oxygen from the water. Fish and other aquatic organisms can't exist in water with low dissolved oxygen levels.





a problem?



Stormwater can pick up debris, chemicals, dirt, and other pollutants and flow into a storm sewer system or directly to a lake, stream, river, wetland, or coastal water. Anything that enters a storm sewer system is discharged untreated into the waterbodies we use for swimming, fishing, and providing drinking water.

- Bacteria and other pathogens can wash into swimming areas and create health hazards, often making beach closures necessary.
- Debris—plastic bags, six-pack rings, bottles, and cigarette butts—washed into waterbodies can choke, suffocate, or disable aquatic life like ducks, fish, turtles, and birds.
- Household hazardous wastes like insecticides, pesticides, paint, solvents, used motor oil, and other auto fluids can poison aquatic life. Land animals and people can become sick or die from eating diseased fish and shellfish or ingesting polluted water.



 Polluted stormwater often affects drinking water sources. This, in turn, can affect human health and increase drinking water treatment costs.

Stormwater Pollution Solutions

Septic

poorly

systems



Recycle or properly dispose of household products that contain chemicals, such as insecticides, pesticides, paint, solvents, and used motor oil and other auto fluids. Don't pour them onto the ground or into storm drains.

Lawn care

Excess fertilizers and pesticides applied to lawns and gardens wash off and pollute streams. In addition, yard clippings and leaves can wash



into storm drains and contribute nutrients and organic matter to streams.

- Don't overwater your lawn. Consider using a soaker hose instead of a sprinkler.
- Use pesticides and fertilizers sparingly. When use is necessary, use these chemicals in the recommended amounts. Use organic mulch or safer pest control methods whenever possible.
- Compost or mulch yard waste. Don't leave it in the street or sweep it into storm drains or streams.
- Cover piles of dirt or mulch being used in landscaping projects.

Auto care

Washing your car and degreasing auto parts at home can send detergents and other contaminants through the storm sewer system. Dumping automotive fluids into storm drains has the same result as dumping the materials directly into a waterbody.

- Use a commercial car wash that treats or recycles its wastewater, or wash your car on your yard so the water infiltrates into the ground.
- Repair leaks and dispose of used auto fluids and batteries at designated drop-off or recycling locations.







Permeable Pavement—Traditional concrete and asphalt don't allow water to soak into the ground. Instead these surfaces rely on storm drains to divert unwanted water. Permeable pavement systems allow rain and snowmelt to soak through, decreasing stormwater runoff.

Rain Barrels—You can collect rainwater from rooftops in mosquitoproof containers. The water can be used later on lawn or garden areas.



Rain Gardens and Grassy Swales—Specially designed areas planted



rainwater to collect and soak into the ground. Rain from rooftop areas or paved areas can be diverted into these areas rather than into storm drains.

Vegetated Filter Strips—Filter strips are areas of native grass or plants created along roadways or streams. They trap the pollutants stormwater picks up as it flows across driveways and streets.

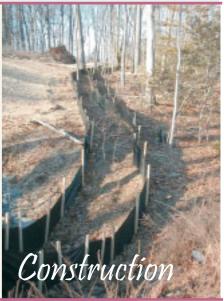


Dirt, oil, and debris that collect in parking lots and paved areas can be washed into the storm sewer system and eventually enter local waterbodies.

- Sweep up litter and debris from sidewalks, driveways and parking lots, especially around storm drains.
- Cover grease storage and dumpsters and keep them clean to avoid leaks.
- Report any chemical spill to the local hazardous waste cleanup team. They'll know the best way to keep spills from harming the environment.

Erosion controls that aren't maintained can cause excessive amounts of sediment and debris to be carried into the stormwater system. Construction vehicles can leak fuel, oil, and other harmful fluids that can be picked up by stormwater and deposited into local waterbodies.

- Divert stormwater away from disturbed or exposed areas of the construction site.
- Install silt fences, vehicle mud removal areas, vegetative cover, and other sediment and erosion controls and properly maintain them, especially after rainstorms.
- Prevent soil erosion by minimizing disturbed areas during construction projects, and seed and mulch bare areas as soon as possible.





Lack of vegetation on streambanks can lead to erosion. Overgrazed pastures can also contribute excessive amounts of sediment to local waterbodies. Excess fertilizers and pesticides can poison aquatic animals and lead to destructive algae blooms. Livestock in streams can contaminate waterways with bacteria, making them unsafe for human contact. Automotive acilities



septic systems release nutrients and pathogens (bacteria and viruses) that can be picked up by stormwater and discharged into nearby waterbodies. Pathogens can cause public health problems and environmental concerns.

- Inspect your system every 3 years and pump your tank as necessary (every 3 to 5 years).
- Don't dispose of household hazardous waste in sinks or toilets.

Pet waste Pet waste can be

a major source of bacteria and excess nutrients in local waters.

- When walking your pet, remember to pick up the waste and dispose of it properly. Flushing pet waste is the best disposal method. Leaving pet waste on the ground increases public health risks by allowing harmful bacteria and nutrients to wash into the storm drain and eventually into local waterbodies.



- Keep livestock away from streambanks and provide them a water source away from waterbodies.
- Store and apply manure away from waterbodies and in accordance with a nutrient management plan.
- Vegetate riparian areas along waterways.
- Rotate animal grazing to prevent soil erosion in fields.
- Apply fertilizers and pesticides according to label instructions to save money and minimize pollution.

Improperly managed logging operations can result in erosion and sedimentation.

- Conduct preharvest planning to prevent erosion and lower costs.
- Use logging methods and equipment that minimize soil disturbance.
- Plan and design skid trails, yard areas, and truck access roads to minimize stream crossings and avoid disturbing the forest floor.
- Construct stream crossings so that they minimize erosion and physical changes to streams.
- Expedite revegetation of cleared areas.



Uncovered fueling stations allow spills to be washed into storm drains. Cars waiting to be repaired can leak fuel, oil, and other harmful fluids that can be picked up by stormwater.

- Clean up spills immediately and properly dispose of cleanup materials.
- Provide cover over fueling stations and design or retrofit facilities for spill containment.
- Properly maintain fleet vehicles to prevent oil, gas, and other discharges from being washed into local waterbodies.
- Install and maintain oil/water separators.