Appendix 10.0

Preliminary Drainage Study for St. Frances Catholic Church

PRELIMINARY DRAINAGE STUDY

FOR

ST. FRANCES CATHOLIC CHURCH

21591 LEMON STREET WILDOMAR, CA 92595

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PRELIMINARY DRAINAGE STUDY ST. FRANCES CATHOLIC CHURCH

I. PURPOSE AND SCOPE

The following preliminary drainage study has been conducted on behalf of the Diocese of San Bernardino. This drainage study is for the proposed expansion of the existing church site St. Frances of Rome located in Wildomar, CA. The site is currently three separate properties: APN 366-170-005-4, 366-170-058-2, and 366-330-011-3. These three properties are being merged into a single 11.24-acre parcel. The project site is located on the south side of Lemon Street, just east of Orchard Street.

The purpose of this drainage study is to determine the required drainage improvements for the project site. The scope of work consists of the following:

- 1. Perform undeveloped and developed studies for the 100-year storm event using the rational method.
- 2. Perform undeveloped and developed studies for the 100-year storm event using the synthetic unit hydrograph method.
- 3. Analyze the offsite drainage and determine its impact on the job site.
- 4. Determine the required mitigation and flow values that will enter and exit each of the water quality basins.

The project site will accommodate a proposed church building as well as new office buildings in a future phase. The northeast office building will have changes in order to make it ADA compliant. The southern parking lot is currently a paved area used for overflow parking. This paving will be removed and repaved as a part of this project.

II. EXISTING & ULTIMATE DRAINAGE CONDITIONS

The project site is currently split into three properties: the existing church site (366-170-058-2), an office building in the northeast corner (366-170-005-4), and the overflow parking area to the south (366-330-011-3).

An existing drainage course bisects the project site, running from east to west between the existing church site to the north and the existing parking lot at the south end of the project site. This drainage course bisecting the project site is "Line E" as detailed within the Sedco Master Drainage Plan and exists in the form of a Riverside County Flood Control easement that can be viewed within the undeveloped and developed drainage maps within Appendix E of this report. The portion of the project site north of this existing drainage course (church site and office building) drain from the northeast corner to southwest, with the flows generated by this area being deposited into Line E. The portion of the project site east of the existing drainage course (overflow parking area) generally sheet flows from east to west. The north half of the overflow parking lot also has a slight fall to the north, with the flows from this area also being deposited into Line E.

Offsite flows currently enter the project site at each of the three properties. The existing church site has a slope running along its east boundary. The runoff from the top of this slope, which is comprised of the backyards of 4 residential houses on the west side of Wagon Rim Court, flow down the slope and enter the church's parking lot (Appendix A: Figure 9). These flows are conveyed south through the project site and are deposited into the drainage course bisecting the project site (Line E).

The portion of Lemon Street north of the existing office building at the northeast corner of the site, is currently unimproved (Appendix A: Figure 10). As a result, offsite flows being conveyed from east to west along Lemon Street currently overtop the existing street section and enter the project site east of the office building. These flows are conveyed south through the project site and are deposited into the drainage course bisecting the project site (Line E).

Offsite flows enter the overflow parking lot, at the south end of the project site, along the eastern boundary line. Like the parking lot itself, the offsite area east of the parking lot sheet flows from east to west, with a slight fall to the north towards the natural drainage course.

A. SEDCO MASTER DRAINAGE PLAN

In March of 1982, Kenneth L. Edwards completed the "Master Drainage Plan for the Sedco Area" for the Riverside County Flood Control and Water Conservation District. In the Sedco MDP, Line E, E-1, and E-2 are designed to "accept the flows released from the Freeway culverts between Line Street and Waite Street."

Within the Sedco MDP, Line E-1 is intended to carry 230 CFS from a 66" CSP and 24" CSP between Lemon Street and Dorothy Lane, south to Line E. It is also

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designed to carry 205 CFS from the 84" CSP located between Dorothy Lane and Waite Street, north to Line E. Line E-2 is intended to carry 65 CFS discharged from the 2-24" CSPs and the 30" CSP located between Lime and Lemon Street to Line E by way of an 30" RCP storm drain to be located within an existing easement running along the eastern property line of the project site.

Line E is designed to carry the 450 CFS of flows from west to east, along an alignment approximately halfway between Lemon and Waite Street. An existing drainage course along the alignment of Line E exists between Almond Street and the western boundary of the project site, though it is not in the form of the trapezoidal channel outline within the Sedco MDP. The MDP outline the flows from Line E to be carried west, to Mission Trail, where they will then be directed northwest. After crossing Corydon Road, the flows are to be spread out into sheet flow conditions and are then directed into Lake Elsinore. None of these 3 lines currently exist in their ultimate condition.

B. FLOWS EAST OF HIGHWAY 15

As discussed in the EXISTING & ULTIMATE DRAINAGE CONDITION section of this report, offsite flows enter the project site via overtopping of the curb along Lemon Street. These flows originate east of the project site. Approximately 274.15 acres of hillside, houses, and undeveloped area east of Interstate-15 drain from northeast to the southwest, crossing Interstate-15 by way of a 66" CSP, a 30" CSP, and 3-24" CSPs. For the larger storm events, the capacity of these five storm drains limit the amount of runoff generated by the 274.15 acres that can cross the highway.

The bulk of the flows crossing Interstate-15, do so via a 66" CSP located just south of Lemon Street. The flows from this pipe are discharged into a concrete basin west of Interstate-15, that runs parallel to the highway (Appendix A: Figure 11). As outlined within the Sedco MDP, the flows from the 66" CSP are designed to flow southwest to Line E by way of Line E-1. However, due to Line E-1 having not yet been constructed as an outlet for the concrete basin, during large storm events the basin exceeds its capacity and overflows. These flows are directed north into Lemon Street where they are then conveyed west.

Flows also cross Interstate-15 by way of a 24" CSP just north of Lemon Street, a 24" pipe just south of Lime Street, and a 30" CSP also located just south of Lime Street. The flows from these 3 pipes discharge west of Interstate-15 between Lime and Lemon Street and are conveyed southwest to Lemon Street as intended within the Sedco MDP.

C. LEMON STREET FLOWS

These five storm drains outlined in the above section, confluence with approximately 31.85 acres comprised of Interstate-15 and housing west of the freeway.

This runoff drains to Lemon Street in the current drainage condition, where the flows are conveyed west. The flows currently overtop the project site along the western portion of its border with Lemon Street.

Of these 5 storm drains that cross Interstate-15, only the 24" CSP located south of Victorian Lane is designed to flow within Lemon Street. The 66" CSP is designed to discharge into Line E-1 and then Line E as outlined in the Sedco MDP. The flows from the 2-24" CSPs and the 30" CSP located between Lime and Lemon Street are designed to flow to Lemon Street, however, the flows are then intended to be carried out of Lemon Street and into Line E by way of Line E-2. Line E-2 is set to be located along the eastern boundary of the project site.

As Line E-2 is not yet constructed, the flows from these 4 storm drains as well as the 31.85 acres east of Interstate-15 that are designed to enter Line E before reaching the project site, are instead conveyed west via Lemon Street to the project site. As Lemon Street was not designed to carry these flows, they currently overtop the project site along its north boundary and are conveyed south to Line E.

D. OFFSITE EAST FLOWS

As discussed previously, offsite flows enter the overflow parking lot along the eastern boundary line. Like the parking lot itself, the offsite area east of the parking lot sheet flows from east to west, with a slight fall to the north towards the natural drainage course.

The area directly east of the church site between Lemon Street and Dorothy Lane do not enter the project site before entering Line E. Though this area generally sheet flows from east to west, a channel was constructed at the southwest end of Wagon Rim Court that conveys the flows generated by this area, as well as the 24" CSP south of the 66" CSP, directly into Line E.

Flows south of Dorothy Lane and north of Waite Street, sheet flow from east to west across Mojonnier Way.

E. DOWNSTREAM FLOWS

All flows entering Line E are intended to flow west to Mission Trail via a trapezoidal channel located halfway between Lemon Street and Waite Street. Due to this trapezoidal channel having not yet been constructed, the flows instead leave the proposed Line E alignment at the western boundary of the project site. These flows are conveyed southwest to a vacant lot located north of Waite Street. The flows are spread out into sheet flow conditions and conveyed west to Mission Trail.

III. METHODOLOGY / PROPOSED DRAINAGE FACILITIES

A. ONSITE BASINS

The project site will mitigate the developed drainage flows down to 90% of the undeveloped flow values. This will be done by way of three basins that will also be used for Water Quality Management needs. Two basins will be located at the southeastern and southwestern corners of the lot that holds the current church site. A third basin will be located on the southern lot that is currently used for overflow parking. The drainage areas for each of these basins can be found on the Developed Drainage Map located within Appendix E of this report. Additionally, area C_1 as labeled on the Developed Drainage Map, will also accommodate the flows from offsite drainage area F_3 . Due to this, the calculations for offsite area F_3 will be included with the onsite calculations.

The rational method will be used in order to determine the flow values that will need to enter into each basin from the project site. Offsite runoff entering the project site along the drive isle entrances on Lemon Street, will also pass through the basins (no mitigation or storage provided for these flows). Due to these flows entering the project site via overflow in excess of the capacity of Lemon Street in addition to the upstream point of these flows being over 2-miles upstream of the basins, the time of concentration at which point these offsite flows will reach the basins is much later than when the runoff from the project site will do so. As such, the greater value between the amount of flow entering the project site and the flow values from the project site as determined by the rational method, will be used as the flow values that will need to enter into each basin.

The Synthetic Unit Hydrograph Method will be used in order to route the storm runoff through the basins in order to assure the flow values are being reduced to 90% of the undeveloped drainage condition. Though areas C_1 and D_1 on the Developed Drainage Map drain to the same area in the undeveloped condition, they will be split up into two separate areas in the undeveloped condition in order to determine the difference in flow generated by the new project development. Additionally, the offsite area F_3 will be mitigated within basin C_1 . As such, a combined unit hydrograph for area C_1 and F_3 will be conducted and routed through basin C_1 . Below is a summary of the required and provided mitigation by each basin:

Mitigation

Area	Storm Event		Flow _{DEVELOPED} (CFS)	Mitigation _{REQUIRED} (CFS)	Mitigation _{PROVIDED} (CFS)
$C_1, D_1, \& F_3$	100 Year - 24 Hour		5.630	0.706	2.870
E ₁	100 Year - 24 Hour	1.785	1.692	0.085	0.711

Figure 1: Mitigation Summary

The values within the Mitigation_{REQUIRED} column of Figure 1 are derived in the Synthetic Unit Hydrograph section of this report. For a further breakdown of the

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mitigation from each basin $(C_1, D_1, and E_1)$ see the summary within the Routing Analysis section of this report.

A Synthetic Unit Hydrograph will also be completed in order to calculate the flow generated by the offsite areas tributary to the project site. The offsite area will be divided into four areas: the portion east of the freeway (F_1), area west of the freeway tributary to Lemon Street(F_2), the area entering the existing church site (366-170-058-2) by way of the slope along the church's eastern boundary (F_3), and the area entering the overflow parking lot (F_4).

B. OFFSITE FLOWS TO LEMON STREET

In order to accommodate the large offsite flows discussed in the EXISTING & ULTIMATE DRAINAGE CONDITION section of this report, the project proposes the construction of an additional 6-inch concrete curb to be located just north of the existing tubular steel fence which is located at the 10' setback line south of the Lemon Street right-of-way. It is not possible to locate this curb at the street right-of-way line due to the existence of multiple underground utility boxes in this area.

Additionally, there is a parcel of land located at the north east corner of the project 150' wide that is a part of this application. Lemon Street is not improved to its full $\frac{1}{2}$ width along this frontage. The close proximity of the existing building on this side dictates a 2' ± retaining wall to be located south of the ultimate right-of-way line across this parcel. The height of this wall will be extended to 1' above the finished grade of the parkway to preclude the existing flow from Lemon Street from entering this site at this point.

This proposed curb and wall extension serve two main purposes:

1. To prevent the comingling of offsite flows with onsite flows for the 2-year storm events.

2. To assure that the runoff generated by the 100-year storm overtops the street and flows into the project site at specified points so that these flows can be directed through the site. This overtopping will occur at the east and west driveway entrances to the church site, where the street will retain the capacity it currently has in the pre-developed state.

The storm runoff flows along Lemon Street from east to west. These flows will then overtop into the driveway entrances to the south, with these overtopping flows running perpendicular to their original flow path within Lemon Street. This scenario shares many of the same characteristics as stormwater flowing along a gutter and then entering a curb inlet catch basin perpendicular to the original flow path. As such, the driveway entrances were modelled as curb inlet catch basins using the following equation:

$$Q = 0.049W^{1.4} \left(\frac{D}{S}\right)^{0.155}$$

In order to determine the amount of flow entering the project site at the two driveway entrances during the 100-year storm event, first the cross section of these drive isles must be analyzed. The cross section of each of the driveway entrances will be plotted with the 100 year-storm water surface level shown as a dashed line. The depth of this dashed line above the elevation of the driveway entrance will be used as the depth of the curb inlet catch basin within the model.

C. OFFSITE FLOWS TO OVERFLOW PARKING LOT

The offsite drainage areas tributary to the southeastern portion of the site can be broken up into three categories: the area tributary to the drainage course bisecting the project site (Line E), the area tributary to the southern parking lot, and the area tributary to Mojonnier Way. As the drainage course bisecting the site (Line E) is the ultimate drainage course for most of the area as set by Riverside County's Master Plan of Drainage, all flows tributary to this drainage course in the predeveloped condition will remain so in the developed condition whenever possible.

There is approximately 4.79 acres of offsite drainage area tributary to the parking lot at the southern end of the project site. The flows generated by this area will be intercepted by a rectangular concrete channel at the eastern end of the parking lot and conveyed north. These flows will then be discharged into Line E as this is the ultimate condition of the runoff generated by the offsite area.

South of the 4.79 acres tributary to the parking lot is an offsite area tributary to Mojonnier Way. Due to grade constrains, Mojonnier Way cannot fall from north to south. As such, to prevent these offsite flows from flowing north along Mojonnier Way and comingling with the onsite flows by entering the basin at the south end of the project site, it is necessary to design Mojonnier Way as a tilt section. This section will enable these offsite flows to retain the same flow path they have in the predeveloped condition, sheet flowing from east to west along Mojonnier Way. Additionally, a buffer strip consisting of a 13' wide by 2' deep section of filter material will be added at the west end of the tilt section in order to treat the flows after they cross Mojonnier Way.

IV. RATIONAL METHOD RESULTS

The following is a summary of the rational method calculations that can be found within the Appendix F of this report. A breakdown of each drainage area can be found within the drainage maps located in Appendix E of this report.

Undeveloped

		Flow (CFS)	T _C (Minutes)
$A_1, A_2, \& F_3$	100 Year - 1 Hour	22.813	14.02
B ₁	100 Year - 1 Hour	9.221	6.201

Developed							
Area	Storm Event	Flow (CFS)	T _C (Minutes)				
$C_1 \& F_3$	100 Year - 1 Hour	11.214	11.284				
D_1	100 Year - 1 Hour	14.356	10.607				
E ₁	100 Year - 1 Hour	9.327	5.894				

Figure 3: Developed Rational Summary Table

As discussed previously in the Methodology section of this report, areas C_1 and D_1 in the developed drainage condition correspond to the combined area A_1 and A_2 in the undeveloped drainage condition. The flow values in Figure 3 represent the amount of flow that will need to enter each basin. Basins D_1 and E_1 will need to accommodate an inlet flow of 14.356 CFS and 9.327 CFS respectively. Basin C_1 will need to accommodate an inlet flow of 16.8 CFS, as the flow overtopping into the project site is greater than the 11.214 determined by the Rational Method.

V. SYNTHETIC UNIT HYDROGRAPH RESULTS

The following is a summary of the synthetic unit hydrograph calculations that can be found within the Appendix G of this report. A breakdown of each drainage area can be found within the drainage maps located in Appendix E of this report.

Undeveloped

Area	Storm Event	Flow (CFS)	Volume (Acre-Ft)
$A_1, A_2, \& F_3$	100 Year - 24 Hour	5.471	2.3258
B ₁	100 Year - 24 Hour	1.785	0.9972

Figure 4: Undeveloped Synthetic Unit Hydrograph Summary Table

Developed

Area	Storm Event	Flow (CFS)	Volume (Acre-Ft)
$C_1 \& F_3$	100 Year - 24 Hour	2.480	1.0589
D_1	100 Year - 24 Hour	3.150	1.4978
E ₁	100 Year - 24 Hour	1.692	0.9089

Figure 5: Developed Synthetic Unit Hydrograph Summary Table

The required mitigation for the project site was found by subtracting 90% of the flow values within Figure 4 from the respective flow values within Figure 5. These unit hydrographs were used for the routing analysis found in the next section of this report.

VI. ROUTING ANALYSIS RESULTS

The following is a summary of the routing calculations that can be found within the Appendix H of this report.

Routing

Area	Storm Event	Inflow (CFS)	Outflow _{SD} (CFS)	Outflow _{INFELTRATION} (CFS)	Mitigation (CFS)	Volume Required (Acre-Ft)
$C_1 \& F_3$	100 Year - 24 Hour	2.480	1.115	0.158	1.365	0.330
D_1	100 Year - 24 Hour	3.150	1.645	0.242	1.505	0.368
E1	100 Year - 24 Hour	1.692	0.981	0.167	0.711	0.173

Figure 6: WQMP Basin Routing Summary Table

The basins make use of infiltration using a factor of safety of 2. The soils report supporting the infiltration rate used within this report can be found in the Geotechnical Report within Appendix K of this report. The outflow due to infiltration was subtracted from the outflow values generated from the routing analysis in order to determine the outflow discharged from the project site by each basin. The mitigation provided by each basin is defined as the flowing:

Mitigation_{PROVIDED} = Inflow – Outflow_{STORM DRAIN}.

The configuration of each basin and its outlet structure can be found within Appendix I of this report.

VII. OFFSITE FLOW RESULTS

The offsite Synthetic Unit Hydrographs were conducted for areas F_1 and F_2 for both the 2-year, 10, year, and 100-year storm events over the 1-hour and 24-hour durations, in order to determine when offsite flows will enter the project site. Additionally, the 100-year storm events were conducted for the area F4, in order to determine the maximum amount of flow that will need to be conveyed by the drainage ditch running along the eastern boundary of the overflow parking lot. Below is a summary of these Synthetic Unit Hydrograph calculations that can be found within Appendix G of this report.

Area	Storm Event	Flow (CFS)	Volume (Acre-Ft)
	2 Year - 1 Hour	95.798	4.4502
	2 Year - 24 Hour	18.394	11.9659
F_1	10 Year - 1 Hour	252.596	14.3024
1.1	10 Year - 24 Hour	87.760	35.5258
	100 Year - 1 Hour	432.186	25.4624
	100 Year - 24 Hour	164.612	73.2901
	2 Year - 1 Hour	20.312	0.6664
	2 Year - 24 Hour	4.539	2.7580
	10 Year - 1 Hour	44.742	1.4784
F ₂	10 Year - 24 Hour	9.299	4.7248
	100 Year - 1 Hour	76.786	2.7652
	100 Year - 24 Hour	18.862	8.5557
E	100 Year - 1 Hour	11.605	0.4074
F ₄	100 Year - 24 Hour	2.747	1.3010

Offsite

Figure 7: Offsite Synthetic Unit Hydrograph Summary Table

The flow values from the area east of the freeway are limited by the total storm drain capacity of the CSP pipes that cross the freeway (286.82). All flows originating east of Interstate-15 in excess of this capacity will be reduced to the storm drain capacity. As such, the

total flow to Lemon Street will be equal the lesser of the flow generated by area F_1 or 286.82 CFS added to the flow generated by area F_2 . The following is a summary of the flow values reaching Lemon Street:

Lemon Stree	7CT 10WS
Storm Event	Flow (CFS)
2 Year - 1 Hour	116.110
2 Year - 24 Hour	22.933
10 Year - 1 Hour	297.338
10 Year - 24 Hour	97.059
100 Year - 1 Hour	363.606
100 Year - 24 Ноц	ır 183.474

Lemon Street Flows

Figure 8: Offsite Flows Reaching Lemon Street

After plotting profile of this flow from the above storm events with respect to the drive isle entrance cross sections, it was determined that the 100-year storm and 10-year storm events would overtop the drive isle entrances, but the 2-year storm event would not. The 10-year storm event was found to overtop the eastern drive isle entrance at a depth of 0.24 feet above the elevation of the drive isle entrance, however, the flows do not overtop the western drive isle entrance into this site.

The maximum amount of flow entering the project site was found by analyzing the depth of the 100-year storm event above each of the drive isle entrances. The 100-year 1-hour water surface was found to be 0.30 feet above the capacity of the eastern drive isle and 0.05 feet above the capacity of the western drive isle. Using the equation for calculating the capacity of a curb inlet catch basin, it was found that 16.8 CFS and 12.1 CFS would enter the east and west driveway entrances respectively. This is less than the flow capacity of the parking lot (Appendix J), so these flows will be able to be conveyed through the parking lot and to the basins. These flows will not be mitigated by the proposed drainage facilities and will pass through the project site and basins to Line E, their pre-development downstream point.

It was found that a 3-foot-wide by 1.76' deep rectangular gutter would be sufficient to convey 11.605 CFS generated by offsite area F4 north to Line E with 1-foot of freeboard.

VIII. CONCLUSION

The above discussed analysis and proposed improvements will provide that this site will be developed in conformance with applicable regulations.

APPENDICES

APPENDIX "A"

EXISTING DRAINAGE CONDITION PHOTOGRAPHS / EXHIBITS

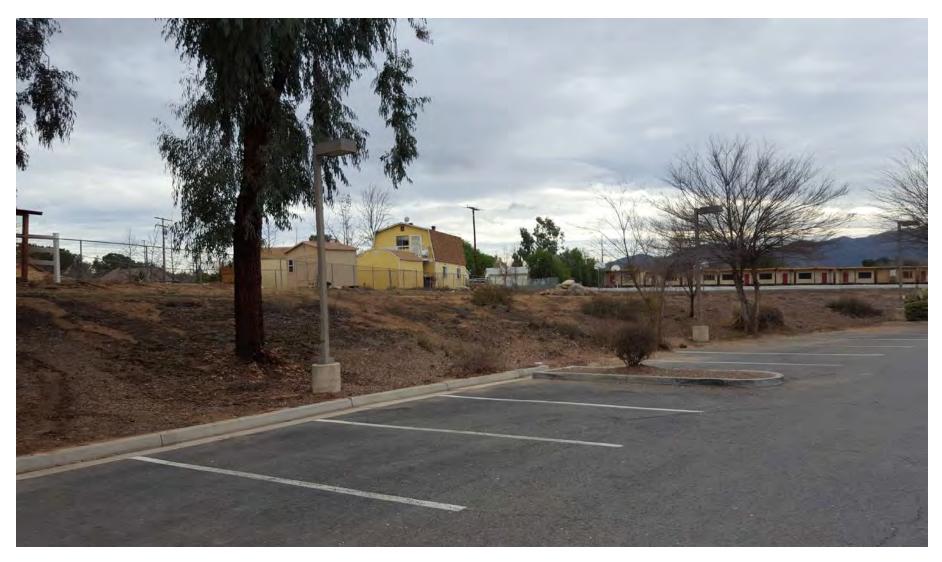


Figure 9: Slope Along East Boundary of Project Site.



Figure 10: Lemon Street North of Existing Office Building

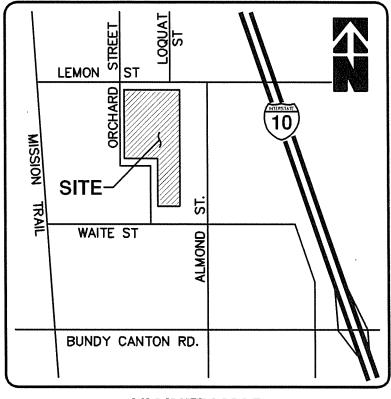


Figure 11: 66" CSP Outlet Basin West of Interstate-15

APPENDIX "B"

VICINITY MAP

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VICINITY MAP

APPENDIX "C" NOAA POINT PRECIPITATION

Precipitation Frequency Data Server





POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Marlin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

Duration		Average recurrence interval (years)										
	1	2	5	10	25	50	100	200	500	1000		
5-min	0.081 (0.068-0.098)	0.110 (0.092-0.133)	0.150 (0.125-0.181)	0.183 (0.151-0.223)	0.229 (0.183-0.290)	0.266 (0.208-0.344)	0.305 (0.232-0.405)	0.346 (0.256-0.473)	0.403 (0.285-0.576)	0.449 (0.306-0.664		
10-min	0.117 (0.098-0.140)	0.158 (0.132-0.190)	0.214 (0.179-0.259)	0.262 (0.217-0.319)	0.329 (0.263-0.415)	0.382 (0.298-0.494)	0.438 (0.333-0.580)	0.496 (0.367-0.678)	0.578 (0.409-0.825)	0.643 (0.439-0.952		
15-min	0.141 (0.118-0.170)	0.191 (0.160-0.230)	0.259 (0.217-0.314)	0.317 (0.262-0.386)	0.398 (0.318-0.502)	0.462 (0.361-0.597)	0.529 (0.403-0.702)	0.600 (0.444-0.820)	0.699 (0.495-0.998)	0.778 (0.531-1.15)		
30-min	0.220 (0.185-0.265)	0.298 (0.250-0.360)	0.405 (0.338-0.490)	0.494 (0.409-0.603)	0.621 (0.496-0.785)	0.721 (0.564-0.932)	0.826 (0.629-1.10)	0.937 (0.693-1.28)	1.09 (0.773-1.56)	1.22 (0.829-1.80)		
60-min	0.345 (0.290-0.416)	0.468 (0.392-0.564)	0.635 (0.530-0.768)	0.775 (0.642-0.946)	0.973 (0.778-1.23)	1.13 (0.884-1.46)	1.30 (0.987-1.72)	1.47 (1.09-2.01)	1.71 (1.21-2.44)	1.91 (1.30-2.82)		
2-hr	0.521 (0.437-0.627)	0.682 (0.571-0.822)	0.901 (0.752-1.09)	1.08 (0.898-1.32)	1.34 (1.07-1.70)	1.55 (1.21-2.00)	1.76 (1.34-2.34)	1.99 (1.47-2.72)	2.30 (1.63-3.29)	2.55 (1.74-3.77)		
3-hr	0.643 (0.540-0.775)	0.835 (0.699-1.01)	1.09 (0.913-1.32)	1.31 (1.09-1.60)	1.61 (1.29-2.04)	1.86 (1.45-2.40)	2.11 (1.60-2.79)	2.37 (1.75-3.24)	2.74 (1.94-3.90)	3.02 (2.06-4.47)		
6-hr	0.918 (0.770-1.11)	1.19 (0.997-1.44)	1.56 (1.30-1.88)	1.86 (1.54-2.27)	2.28 (1.82-2.88)	2.61 (2.04-3.38)	2.95 (2.25-3.92)	3.31 (2.44-4.52)	3.80 (2.69-5.42)	4.18 (2.85-6.19)		
12-hr	1.19 (1.00-1.44)	1.60 (1.34-1.93)	2.13 (1.78-2.58)	2.57 (2.12-3.13)	3.16 (2.52-3.99)	3.61 (2.82-4.66)	4.06 (3.10-5.39)	4.53 (3.35-6.19)	5.16 (3.65-7.37)	5.65 (3.85-8.36)		
24-hr	1.58 (1.40-1.83)	2.23 (1.97-2.58)	3.06 (2.69-3.55)	3.72 (3.25-4.35)	4.61 (3.90-5.56)	5.28 (4.38-6.50)	5.95 (4.82-7.50)	6.63 (5.23-8.58)	7.54 (5.71-10.2)	8.23 (6.03-11.5)		
2-day	1.90 (1.68-2.19)	2.76 (2.44-3.19)	3.87 (3.41-4.49)	4.77 (4.17-5.57)	5.98 (5.06-7.21)	6.89 (5.71-8.48)	7.81 (6.33-9.84)	8.75 (6.90-11.3)	10.0 (7.59-13.5)	11.0 (8.04-15.3)		
3-day	2.07 (1.83-2.39)	3.06 (2.70-3.54)	4.35 (3.83-5.04)	5.40 (4.72-6.31)	6.82 (5.77-8.22)	7.90 (6.56-9.73)	9.01 (7.30-11.3)	10.1 (8.00-13.1)	11.7 (8.86-15.7)	12.9 (9.45-17.9)		
4-day	2.24 (1.98-2.58)	3.33 (2.94-3.85)	4.77 (4.20-5.53)	5.95 (5.20-6.95)	7.55 (6.39-9.10)	8.79 (7.29-10.8)	10.1 (8.15-12.7)	11.4 (8.97-14.7)	13.2 (9.97-17.7)	14.6 (10.7-20.3)		
7-day	2.53 (2.23-2.92)	3.77 (3.33-4.36)	5.45 (4.80-6.32)	6.84 (5.98-7.99)	8.77 (7.42-10.6)	10.3 (8.54-12.7)	11.9 (9.61-14.9)	13.5 (10.7-17.5)	15.8 (12.0-21.3)	17.6 (12.9-24.5)		
10-day	2.66 (2.35-3.07)	3.97 (3.51-4.59)	5.76 (5.07-6.68)	7.26 (6.35-8.48)	9.38 (7.94-11.3)	11.1 (9.17-13.6)	12.8 (10.4-16.1)	14.7 (11.6-19.0)	17.3 (13.1-23.3)	19.4 (14.2-27.0)		
20-day	3.11 (2.75-3.59)	4.67 (4.12-5.40)	6.85 (6.03-7.94)	8.72 (7.62-10.2)	11.4 (9.66-13.8)	13.6 (11.3-16.8)	16.0 (12.9-20.1)	18.5 (14.6-23.9)	22.1 (16.7-29.7)	25.0 (18.3-34.8)		
30-day	3.68 (3.25-4.25)	5.48 (4.84-6.33)	8.04 (7.08-9.32)	10.3 (8.97-12.0)	13.5 (11.5-16.3)	16.2 (13.5-20.0)	19.1 (15.5-24.1)	22.3 (17.6-28.8)	26.8 (20.3-36.1)	30.6 (22.4-42.6)		
45-day	4.25 (3.75-4.90)	6.25 (5.52-7.23)	9.14 (8.05-10.6)	11.7 (10.2-13.7)	15.5 (13.1-18.7)	18.7 (15.5-23.0)	22.1 (17.9-27.9)	25.9 (20.4-33.5)	31.4 (23.8-42.3)	36.1 (26.4-50.2)		
60-day	4.89 (4.32-5.65)	7.08 (6.25-8.19)	10.3 (9.06-11.9)	13.1 (11.5-15.4)	17.4 (14.7-21.0)	21.0 (17.4-25.9)	25.0 (20.3-31.5)	29.4 (23.2-38.0)	35.8 (27.1-48.2)	41.2 (30.2-57.4)		

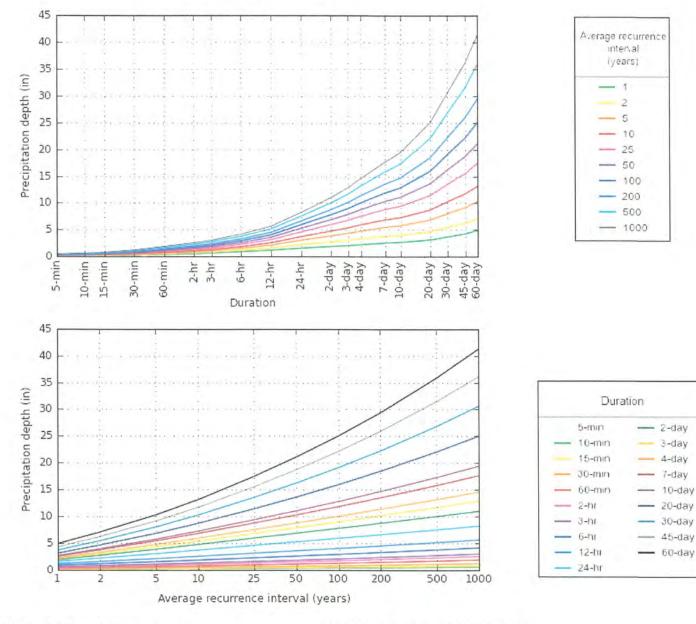
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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PF graphical



PDS-based depth-duration-frequency (DDF) curves Latitude: 33.6337°, Longitude: -117.2823°

NOAA Atlas 14, Volume 6, Version 2

Created (GMT): Thu Jan 10 23:56:23 2019

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Maps & aerials

Small scale terrain

Precipitation Frequency Data Server



Large scale terrain





Large scale aerial

Precipitation Frequency Data Server

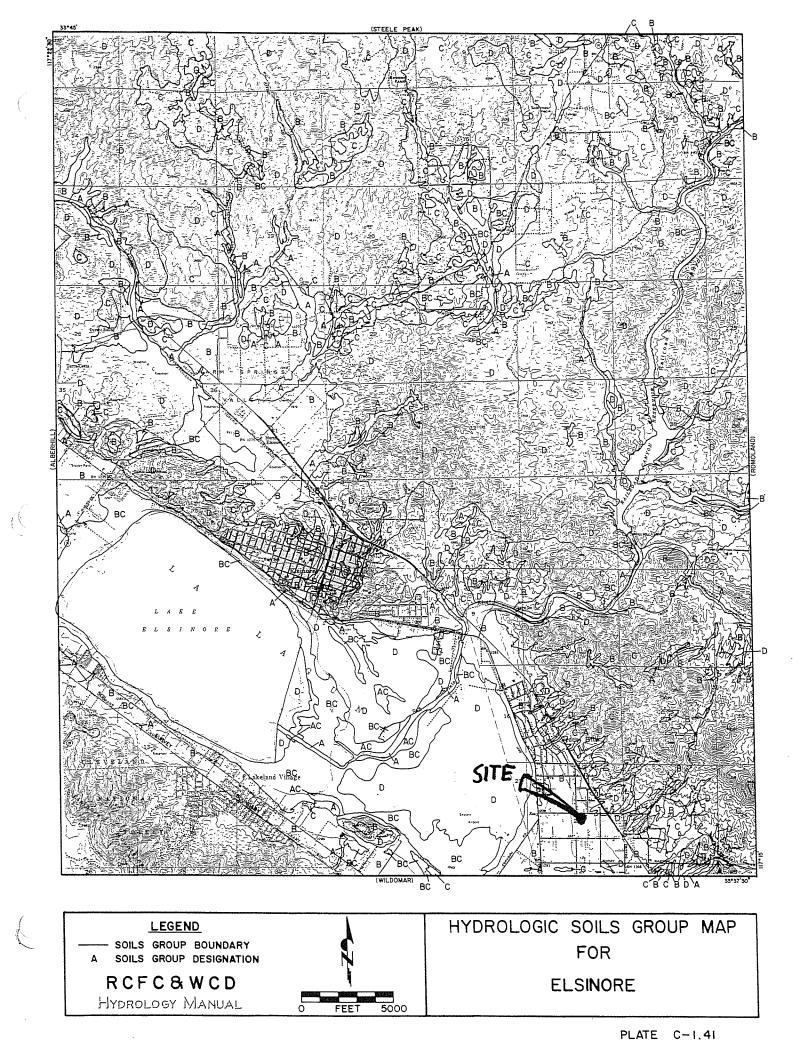


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US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: HDSC.Questions@noaa.gov

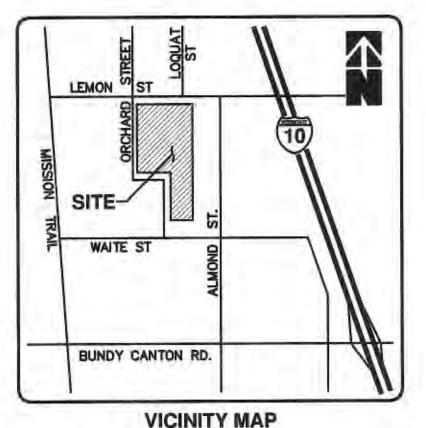
Disclaimer

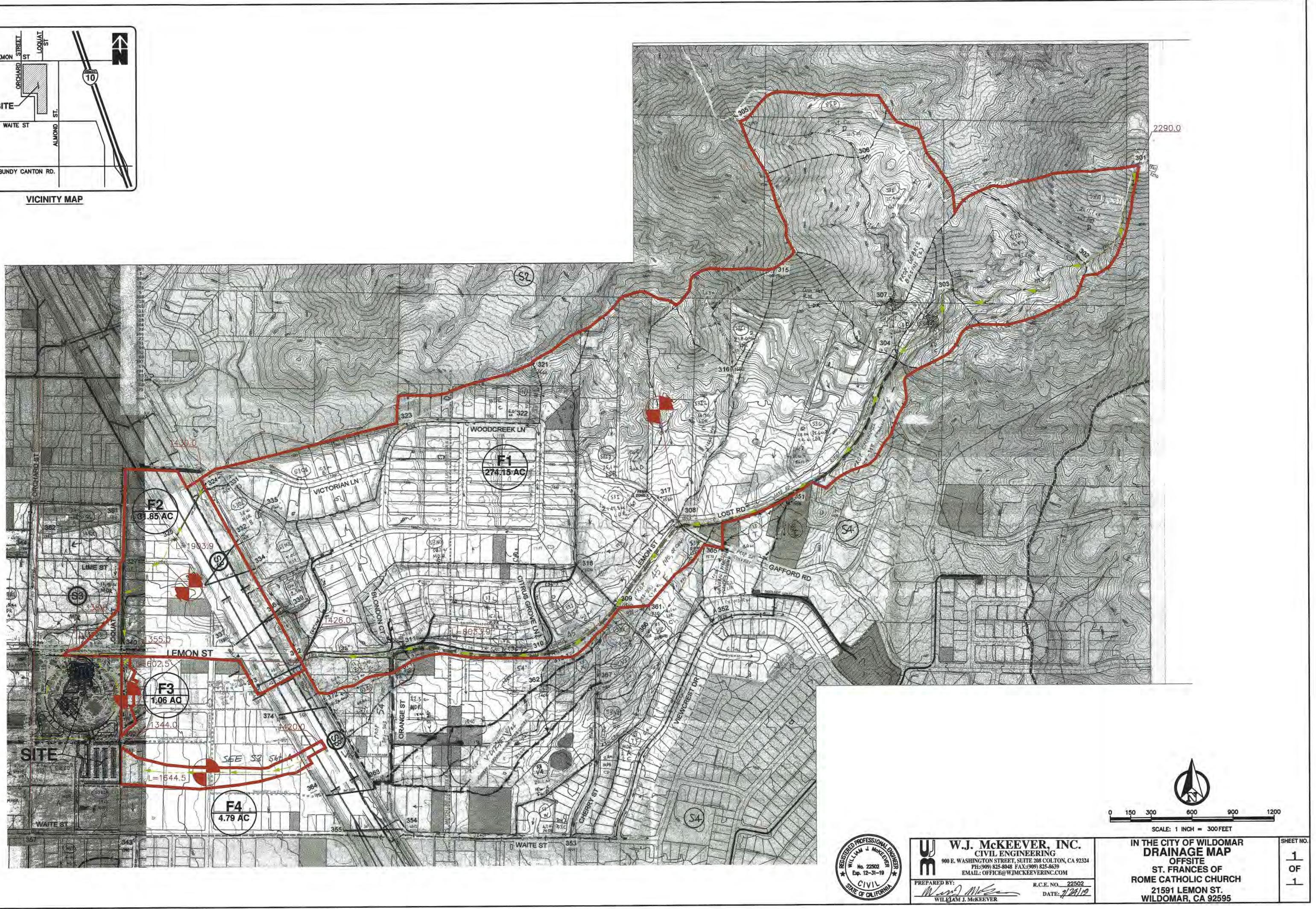
APPENDIX "D" HYDROLOGIC SOIL MAP

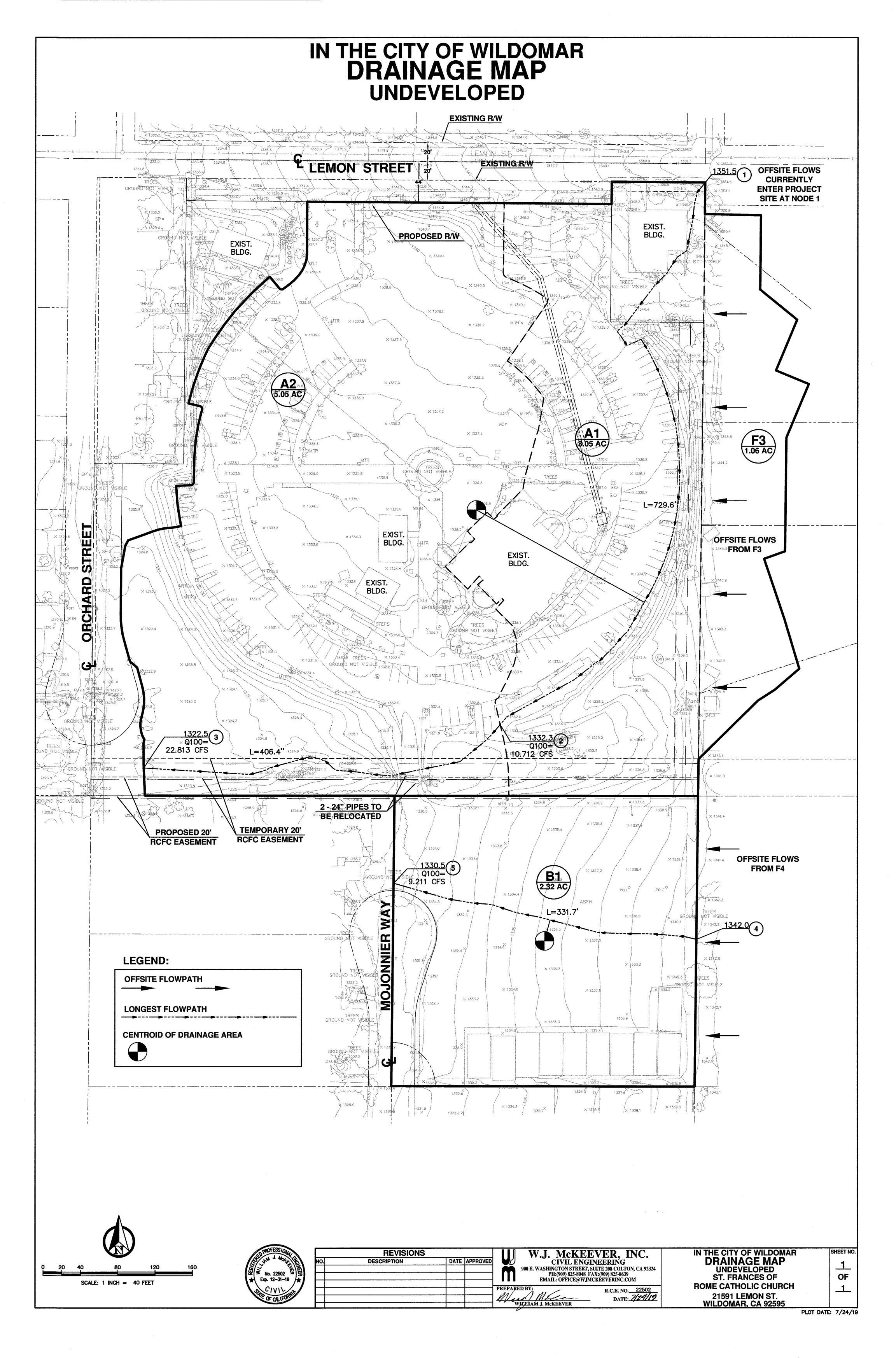


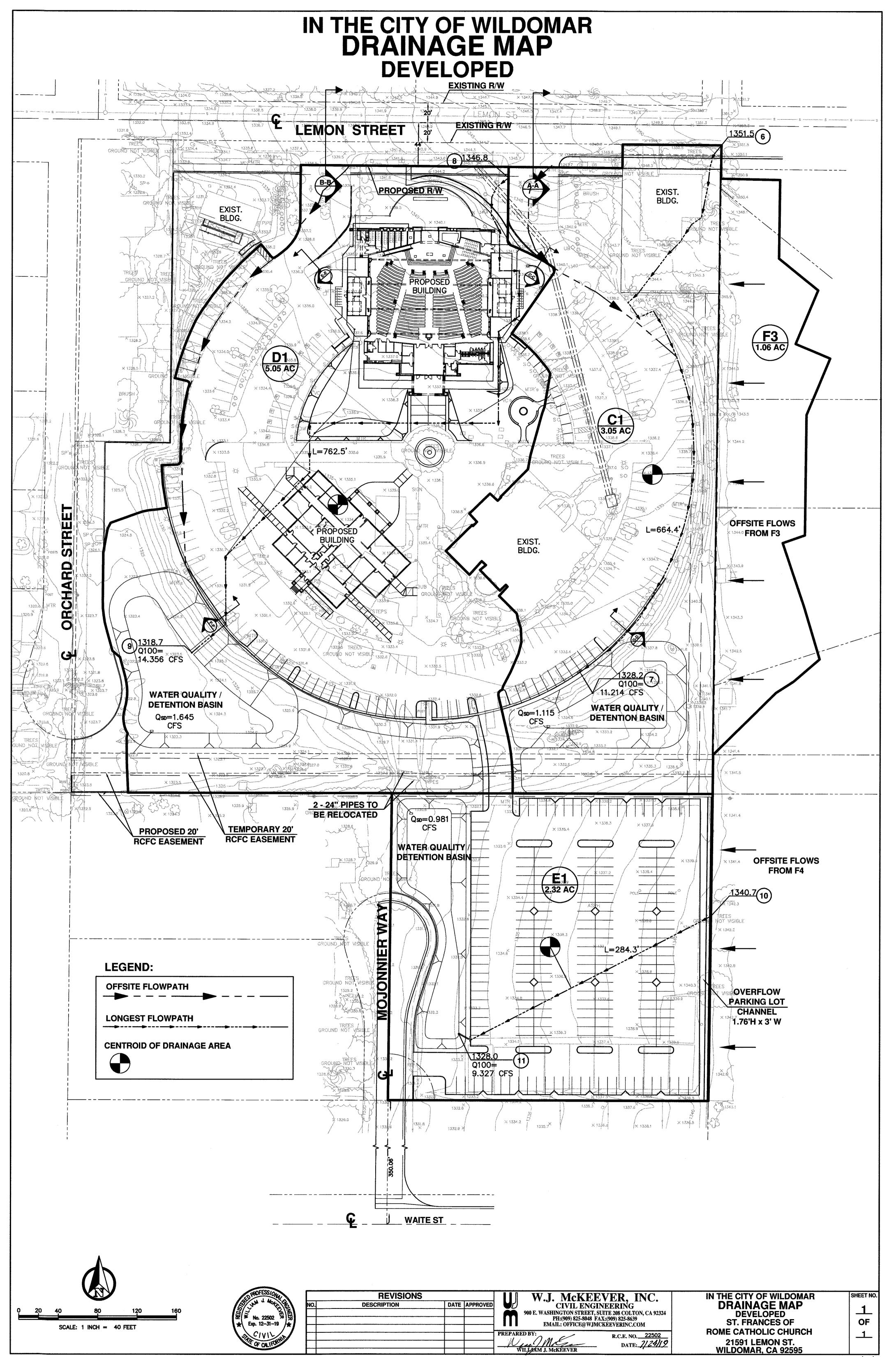
APPENDIX "E"

DRAINAGE MAPS

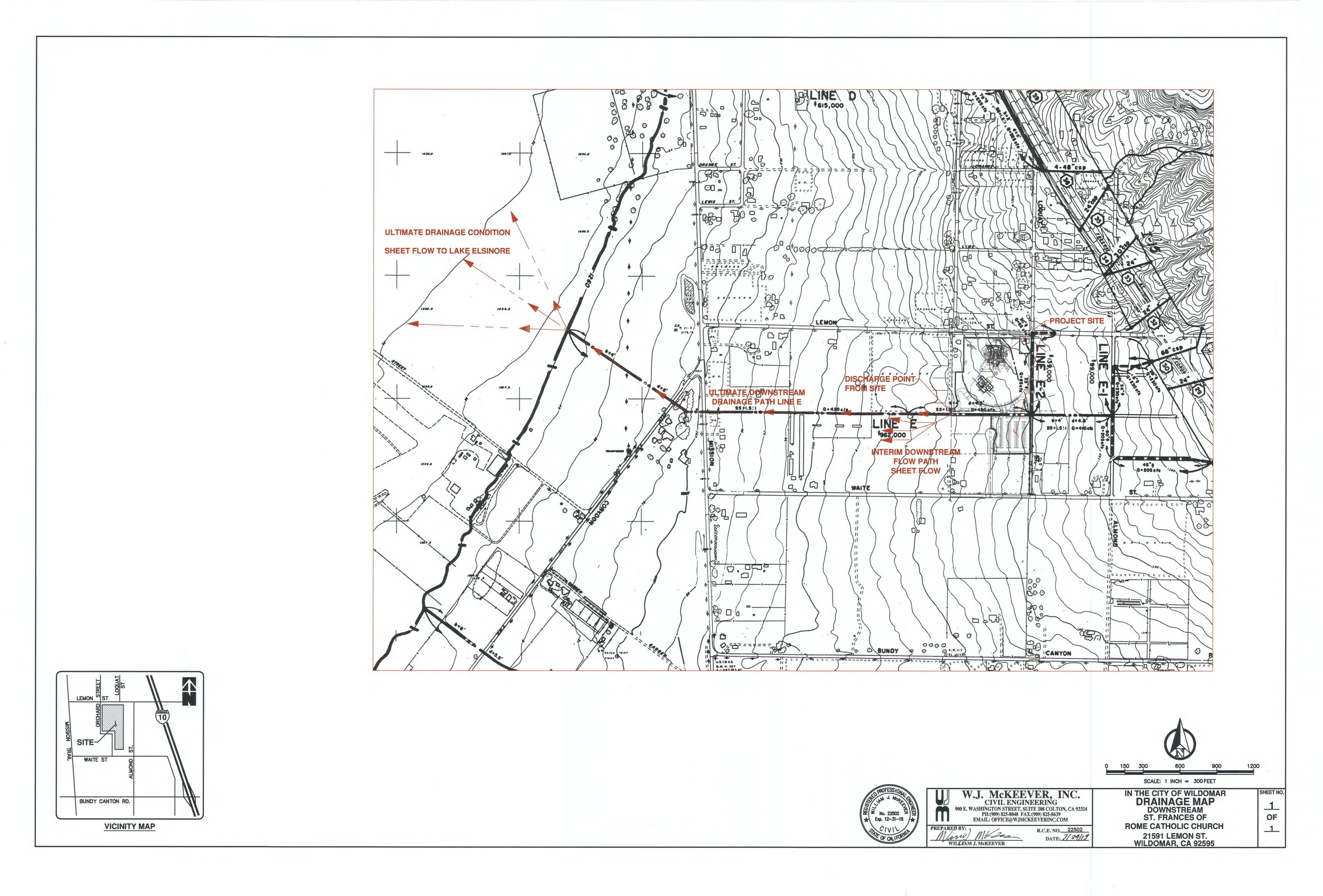








PLOT DATE: 7/24/19



APPENDIX "F"

RATIONAL METHOD CALCULATIONS

UNDEVELOPED 100 YEAR – 1 HOUR

Riverside County Rational Hydrology Program CIVILCADD/CIVILDESIGN Engineering Software, (c) 1989 - 2005 Version 7.1 Rational Hydrology Study Date: 07/23/19 File:c318und.out _____ ******** Hydrology Study Control Information ********* English (in-lb) Units used in input data file ____ Program License Serial Number 6222 St. Frances of Rome - Wildomar Undeveloped 100-Year 1-Hour Rational Method Hydrology Program based on Riverside County Flood Control & Water Conservation District 1978 hydrology manual Storm event (year) = 100.00 Antecedent Moisture Condition = 2 Standard intensity-duration curves data (Plate D-4.1) For the [Elsinore-Wildomar] area used. 10 year storm 10 minute intensity = 2.320(In/Hr) 10 year storm 60 minute intensity = 0.980(In/Hr) 100 year storm 10 minute intensity = 3.540(In/Hr) 100 year storm 60 minute intensity = 1.500(In/Hr) Storm event year = 100.0Calculated rainfall intensity data: 1 hour intensity = 1.500(In/Hr)Slope of intensity duration curve = 0.4800 Process from Point/Station 1.000 to Point/Station 2.000 **** INITIAL AREA EVALUATION **** Initial area flow distance = 729.600(Ft.) Top (of initial area) elevation = 1351.500(Ft.) Bottom (of initial area) elevation = 1332.300(Ft.) Difference in elevation = 19.200(Ft.) Slope = 0.02632 s(percent) = 2.63 $TC = k(0.433) * [(length^3)/(elevation change)]^{0.2}$ Initial area time of concentration = 12.511 min. Rainfall intensity = 3.184(In/Hr) for a 100.0 year storm USER INPUT of soil data for subarea Runoff Coefficient = 0.819 Decimal fraction soil group A = 0.000Decimal fraction soil group B = 1.000Decimal fraction soil group C = 0.000Decimal fraction soil group D = 0.000

```
RI index for soil (AMC 2) = 71.88
Pervious area fraction = 0.642; Impervious fraction = 0.358
Initial subarea runoff = 10.712(CFS)
Total initial stream area =
                               4.110(Ac.)
Pervious area fraction = 0.642
2.000 to Point/Station
Process from Point/Station
                                                           3.000
**** NATURAL CHANNEL TIME + SUBAREA FLOW ADDITION ****
Top of natural channel elevation = 1332.300(Ft.)
End of natural channel elevation = 1322.500(Ft.)
Length of natural channel = 406.400 (Ft.)
Estimated mean flow rate at midpoint of channel =
                                                17.292(CFS)
Natural valley channel type used
L.A. County flood control district formula for channel velocity:
Velocity(ft/s) = (7 + 8(q(English Units)^{.352})(slope^{0.5})
Velocity using mean channel flow = 4.48(Ft/s)
Correction to map slope used on extremely rugged channels with
drops and waterfalls (Plate D-6.2)
     Normal channel slope = 0.0241
Corrected/adjusted channel slope = 0.0241
Travel time = 1.51 \text{ min}. TC = 14.02 \text{ min}.
Adding area flow to channel
USER INPUT of soil data for subarea
Runoff Coefficient = 0.795
Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 1.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil (AMC 2) = 65.97
Pervious area fraction = 0.627; Impervious fraction = 0.373
Rainfall intensity = 3.014(In/Hr) for a 100.0 year storm
Subarea runoff = 12.102(CFS) for 5.050(Ac.)
Total runoff =
                 22.813(CFS)
                                 Total area =
                                                   9.160(Ac.)
Process from Point/Station
                              4.000 to Point/Station
                                                           5.000
**** INITIAL AREA EVALUATION ****
Initial area flow distance = 331.700(Ft.)
Top (of initial area) elevation = 1342.000(Ft.)
Bottom (of initial area) elevation = 1330.500(Ft.)
Difference in elevation = 11.500(Ft.)
Slope = 0.03467 s(percent)=
                                 3.47
TC = k(0.311) * [(length^3) / (elevation change)]^{0.2}
Initial area time of concentration = 6.201 min.
Rainfall intensity =
                        4.459(In/Hr) for a 100.0 year storm
USER INPUT of soil data for subarea
Runoff Coefficient = 0.891
Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 1.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
```

RI index for soil(AMC 2) = 81.09 Pervious area fraction = 0.146; Impervious fraction = 0.854 Initial subarea runoff = 9.221(CFS) Total initial stream area = 2.320(Ac.) Pervious area fraction = 0.146 End of computations, total study area = 11.48 (Ac.) The following figures may be used for a unit hydrograph study of the same area.

Area averaged pervious area fraction(Ap) = 0.535Area averaged RI index number = 71.1

DEVELOPED 100 Year – 1 Hour

Riverside County Rational Hydrology Program CIVILCADD/CIVILDESIGN Engineering Software, (c) 1989 - 2005 Version 7.1 Rational Hydrology Study Date: 07/23/19 File:c318dev.out ******** Hydrology Study Control Information ********* English (in-lb) Units used in input data file Program License Serial Number 6222 St. Frances of Rome - Wildomar Developed 100-Year 1-Hour Rational Method Hydrology Program based on Riverside County Flood Control & Water Conservation District 1978 hydrology manual Storm event (year) = 100.00 Antecedent Moisture Condition = 2 Standard intensity-duration curves data (Plate D-4.1) For the [Elsinore-Wildomar] area used. 10 year storm 10 minute intensity = 2.320(In/Hr) 10 year storm 60 minute intensity = 0.980(In/Hr) 100 year storm 10 minute intensity = 3.540(In/Hr) 100 year storm 60 minute intensity = 1.500(In/Hr) Storm event year = 100.0Calculated rainfall intensity data: 1 hour intensity = 1.500(In/Hr)Slope of intensity duration curve = 0.4800 Process from Point/Station 6.000 to Point/Station 7.000 **** INITIAL AREA EVALUATION **** Initial area flow distance = 664.400(Ft.) Top (of initial area) elevation = 1351.500(Ft.) Bottom (of initial area) elevation = 1328.200(Ft.) Difference in elevation = 23.300(Ft.) Slope = 0.03507 s(percent) = 3.51 $TC = k(0.429) * [(length^3) / (elevation change)]^{0.2}$ Initial area time of concentration = 11.284 min. Rainfall intensity = 3.345(In/Hr) for a 100.0 year storm USER INPUT of soil data for subarea Runoff Coefficient = 0.816Decimal fraction soil group A = 0.000Decimal fraction soil group B = 1.000Decimal fraction soil group C = 0.000Decimal fraction soil group D = 0.000

```
RI index for soil (AMC 2) = 69.51
Pervious area fraction = 0.630; Impervious fraction = 0.370
Initial subarea runoff = 11.214(CFS)
Total initial stream area =
                               4.110(Ac.)
Pervious area fraction = 0.630
Process from Point/Station
                           8.000 to Point/Station
                                                            9.000
**** INITIAL AREA EVALUATION ****
Initial area flow distance = 762.500(Ft.)
Top (of initial area) elevation = 1346.800(Ft.)
Bottom (of initial area) elevation = 1318.700(Ft.)
Difference in elevation = 28.100(Ft.)
Slope = 0.03685 s(percent) =
                              3.69
TC = k(0.385) * [(length^3)/(elevation change)]^{0.2}
Initial area time of concentration = 10.607 min.
Rainfall intensity =
                       3.446(In/Hr) for a 100.0 year storm
USER INPUT of soil data for subarea
Runoff Coefficient = 0.825
Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 1.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil(AMC \overline{2}) = 63.96
Pervious area fraction = 0.466; Impervious fraction = 0.534
Initial subarea runoff = 14.356(CFS)
Total initial stream area =
                              5.050(Ac.)
Pervious area fraction = 0.466
Process from Point/Station
                              10.000 to Point/Station 11.000
**** INITIAL AREA EVALUATION ****
Initial area flow distance = 284.300(Ft.)
Top (of initial area) elevation = 1340.700(Ft.)
Bottom (of initial area) elevation = 1328.000(Ft.)
Difference in elevation =
                           12.700(Ft.)
Slope = 0.04467 s(percent) = 4.47
TC = k(0.330) * [(length^3) / (elevation change)]^{0.2}
Initial area time of concentration = 5.894 min.
Rainfall intensity =
                        4.569(In/Hr) for a 100.0 year storm
USER INPUT of soil data for subarea
Runoff Coefficient = 0.880
Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 1.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil(AMC \overline{2}) = 72.89
Pervious area fraction = 0.228; Impervious fraction = 0.772
Initial subarea runoff = 9.327(CFS)
Total initial stream area =
                                 2.320(Ac.)
Pervious area fraction = 0.228
End of computations, total study area =
                                               11.48 (Ac.)
The following figures may
be used for a unit hydrograph study of the same area.
Area averaged pervious area fraction(Ap) = 0.477
```

Area averaged RI index number = 67.8

APPENDIX "G"

SYNTHETIC UNIT HYDROGRAPH CALCULATIONS

ONSITE

.

UNDEVELOPED 100 Year – 24 Hour AREA A₁, A₂, & F₃

```
Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318undA1A2F324100.out
_____
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
St. Frances of Rome - Wildomar
Undeveloped A1, A2, & F3
100-Year 24-Hour
_____
Drainage Area = 9.16(Ac.) = 0.014 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 9.16(Ac.) = 0.014 Sq.
Length along longest watercourse = 1136.00(Ft.)
Length along longest watercourse measured to centroid = 523.10(Ft.)
Length along longest watercourse = 0.215 Mi.
Length along longest watercourse measured to centroid = 0.099 Mi.
Difference in elevation = 29.00(Ft.)
Slope along watercourse = 134.7887 Ft./Mi.
Average Manning's 'N' = 0.020
Lag time = 0.044 Hr.
Lag time = 2.63 Min.
25% of lag time = 0.66 Min.
40% of lag time = 1.05 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
             Rainfall(In)[2] Weighting[1*2]
Area(Ac.)[1]
      9.16
               2.23
                                  20.43
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
      9.16
                 5.95
                                     54.50
STORM EVENT (YEAR) = 100.00
```

Mi.

Area Averaged 2-Year Rainfall = 2.230(In) Area Averaged 100-Year Rainfall = 5.950(In) Point rain (area averaged) = 5.950(In) Areal adjustment factor = 100.00 % Adjusted average point rain = 5.950(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious 🖇 3.05067.770.4835.05065.970.3731.06078.000.000 Total Area Entered = 9.16(Ac.) RI RI Infil. Rate Impervious Adj. Infil. Rate Area% F

 AMC2
 AMC-2
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 67.8
 67.8
 0.386
 0.483
 0.218
 0.333
 0.073

 66.0
 66.0
 0.406
 0.373
 0.269
 0.551
 0.149

 78.0
 78.0
 0.268
 0.000
 0.268
 0.116
 0.031

 Sum(F) = 0.252Area averaged mean soil loss (F) (In/Hr) = 0.252Minimum soil loss rate ((In/Hr)) = 0.126(for 24 hour storm duration) Soil low loss rate (decimal) = 0.607 Unit Hydrograph DESERT S-Curve Unit Hydrograph Data _____ Unit time period $% \left({{\mathcal{T}}_{{\rm{D}}}} \right)$ Time % of lag $% \left({{\mathcal{T}}_{{\rm{D}}}} \right)$ Distribution $% \left({{\mathcal{T}}_{{\rm{D}}}} \right)$ Unit Hydrograph Graph % (CFS) (hrs) -

 1
 0.083
 190.242
 40.394

 2
 0.167
 380.485
 46.243

 3
 0.250
 570.727
 9.403

 4
 0.333
 760.970
 2.999

 5
 0.417
 951.212
 0.961

 3.729 4.269 0.868 0.277 0.089 Sum = 100.000 Sum = 9.232_____

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit	Time	Pattern	Storm Rain	Loss rate(In./Hr)		Effective
	(Hr.)	Percent	(In/Hr)	Max	Low	(In/Hr)
1	0.08	0.07	0.048	(0.447)	0.029	0.019
2	0.17	0.07	0.048	(0.445)	0.029	0.019
3	0.25	0.07	0.048	(0.444)	0.029	0.019
4	0.33	0.10	0.071	(0.442)	0.043	0.028
5	0.42	0.10	0.071	(0.440)	0.043	0.028
6	0.50	0.10	0.071	(0.438)	0.043	0.028
7	0.58	0.10	0.071	(0.437)	0.043	0.028
8	0.67	0.10	0.071	(0.435)	0.043	0.028
9	0.75	0.10	0.071	(0.433)	0.043	0.028
10	0.83	0.13	0.095	(0.432)	0.058	0.037
11	0.92	0.13	0.095	(0.430)	0.058	0.037

	12	1.00	0.13	0.095	(0.428)	0.058	0.037
÷	13	1.08	0.10	0.071	(0.426)	0.043	0.028
	14	1.17	0.10	0.071	(0.425)	0.043	0.028
	15	1.25	0.10	0.071	(0.423)	0.043	0.028
	16	1.33	0.10	0.071	(0.421)	0.043	0.028
	17 18	1.42 1.50	0.10 0.10	0.071 0.071	(0.420) (0.418)	0.043 0.043	0.028 0.028
	19	1.58	0.10	0.071	(0.416)	0.043	0.028
	20	1.67	0.10	0.071	(0.415)	0.043	0.028
	21	1.75	0.10	0.071	(0.413)	0.043	0.028
	22	1.83	0.13	0.095	(0.411)	0.058	0.037
	23 24	1.92 2.00	0.13 0.13	0.095 0.095	(0.410) (0.408)	0.058 0.058	0.037 0.037
	25	2.00	0.13	0.095	(0.408)	0.058	0.037
	26	2.17	0.13	0.095	(0.405)	0.058	0.037
	27	2.25	0.13	0.095	(0.403)	0.058	0.037
	28	2.33	0.13	0.095	(0.402)	0.058	0.037
	29 30	2.42 2.50	0.13 0.13	0.095 0.095	(0.400) (0.398)	0.058 0.058	0.037 0.037
	31	2.50	0.13	0.119	(0.398) (0.397)	0.038	0.047
	32	2.67	0.17	0.119	(0.395)	0.072	0.047
	33	2.75	0.17	0.119	(0.393)	0.072	0.047
	34	2.83	0.17	0.119	(0.392)	0.072	0.047
	35 36	2.92 3.00	0.17 0.17	0.119 0.119	(0.390) (0.389)	0.072 0.072	0.047 0.047
	37	3.08	0.17	0.119	(0.387)	0.072	0.047
	38	3.17	0.17	0.119	(0.385)	0.072	0.047
	39	3.25	0.17	0.119	(0.384)	0.072	0.047
	40	3.33 3.42	0.17 0.17	0.119	(0.382)	0.072 0.072	0.047
	41 42	3.42 3.50	0.17	0.119 0.119	(0.381) (0.379)	0.072	0.047 0.047
	43	3.58	0.17	0.119	(0.377)	0.072	0.047
	44	3.67	0.17	0.119	(0.376)	0.072	0.047
	45	3.75	0.17	0.119	(0.374)	0.072	0.047
	46 47	3.83 3.92	0.20 0.20	0.143 0.143	(0.373) (0.371)	0.087 0.087	0.056 0.056
	48	4.00	0.20	0.143	(0.369)	0.087	0.056
	49	4.08	0.20	0.143	(0.368)	0.087	0.056
	50	4.17	0.20	0.143	(0.366)	0.087	0.056
	51 52	4.25	0.20 0.23	0.143	(0.365)	0.087	0.056
	52	4.33 4.42	0.23	0.167 0.167	(0.363) (0.362)	0.101 0.101	0.065 0.065
	54	4.50	0.23	0.167	(0.360)	0.101	0.065
	55	4.58	0.23	0.167	(0.359)	0.101	0.065
	56	4.67	0.23	0.167	(0.357)	0.101	0.065
	57 58	4.75 4.83	0.23 0.27	0.167 0.190	(0.355) (0.354)	0.101 0.116	0.065 0.075
	59	4.92	0.27	0.190	(0.352)	0.116	0.075
	60	5.00	0.27	0.190	(0.351)	0.116	0.075
	61	5.08	0.20	0.143	(0.349)	0.087	0.056
	62	5.17	0.20	0.143	(0.348)	0.087	0.056
	63 64	5.25 5.33	0.20 0.23	0.143 0.167	(0.346) (0.345)	0.087 0.101	0.056 0.065
	65	5.42	0.23	0.167	(0.343)	0.101	0.065
	66	5.50	0.23	0.167	(0.342)	0.101	0.065
	67	5.58	0.27	0.190	(0.340)	0.116	0.075
	68 69	5.67 5.75	0.27 0.27	0.190 0.190	(0.339) (0.337)	$0.116 \\ 0.116$	0.075 0.075
	70	5.83	0.27	0.190	(0.336)	0.116	0.075
	71	5.92	0.27	0.190	(0.334)	0.116	0.075

72 73 74 75 76 77 80 81 83 84 86 87 88 90 92 93 94 96 97 99 90 101 103 104 105 106 107 108 9121 2123 124 125 126 127 82 122 122 122 122 122 122 122 122 122	6.00 6.08 6.17 6.25 6.33 6.42 6.50 6.58 6.75 6.892 7.08 7.17 7.253 7.58 7.592 8.08 8.58 7.53 9.008 9.253 9.58 9.500 10.253 10.253 10.258 10.5	0.27 0.30 0.30 0.30 0.30 0.30 0.33 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.53 0.57 0.57 0.57 0.63 0.63 0.67 0.70 0.70 0.70 0.73 0.50 0.50 0.50 0.50 0.50 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.50	0.190 0.214 0.214 0.214 0.214 0.214 0.214 0.238 0.357 0.357 0.357 0.357 0.351 0.381 0.405 0.405 0.452 0.476 0.500 0.500 0.524 0.357 0	$ \left(\begin{array}{c} 0.333 \\ 0.331 \\ (0.331 \\ (0.330 \\ (0.328 \\ (0.327 \\ (0.325 \\ (0.324 \\ (0.323 \\ (0.321 \\ (0.320 \\ (0.321 \\ (0.320 \\ (0.312 \\ (0.317 \\ (0.315 \\ (0.317 \\ (0.315 \\ (0.314 \\ (0.312 \\ (0.311 \\ (0.310 \\ (0.303 \\ (0.307 \\ (0.305 \\ (0.303 \\ (0.307 \\ (0.303 \\ (0.307 \\ (0.303 \\ (0.303 \\ (0.307 \\ (0.303 \\ (0.303 \\ (0.303 \\ (0.303 \\ (0.303 \\ (0.298 \\ (0.297 \\ (0.298 \\ (0.297 \\ (0.296 \\ (0.294 \\ (0.293 \\ (0.292 \\ (0.292 \\ (0.292 \\ (0.292 \\ (0.292 \\ (0.281 \\ (0.285 \\ (0.285 \\ (0.283 \\ (0.282 \\ (0.281 \\ (0.273 \\ 0.277 \\ 0.275 \\ 0.274 \\ 0.273 \\ 0.277 \\ 0.275 \\ 0.271 \\ 0.273 \\ 0.271 \\ 0.270 \\ 0.269 \\ 0.268 \\ 0.266 \\ (0.265 \\ (0.265 \\ (0.262 \\ (0.261 \\ (0.262 \\ (0.259 \\ 0.257 \\ 0.257 \\ 0.256 \\ 0.255 \\ 0.2$	0.116 0.130 0.130 0.130 0.130 0.130 0.130 0.144 0.144 0.144 0.144 0.144 0.144 0.144 0.144 0.144 0.144 0.159 0.159 0.159 0.159 0.159 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.217 0.217 0.217 0.217 0.217 0.217 0.231 0.231 0.231 0.231 0.231 0.231 0.231 0.246 0.246 0.274 (0.289) (0.289) (0.289) (0.303) (0.318) (0.318) (0.318) (0.318) (0.217 0.217	0.075 0.084 0.084 0.084 0.084 0.084 0.094 0.094 0.094 0.094 0.094 0.094 0.094 0.094 0.094 0.094 0.094 0.094 0.103 0.103 0.112 0.122 0.122 0.122 0.122 0.122 0.122 0.122 0.140 0.140 0.140 0.140 0.140 0.150 0.150 0.159 0.159 0.159 0.159 0.159 0.159 0.159 0.159 0.159 0.159 0.159 0.159 0.159 0.150 0.227 0.228 0.227 0.228 0.255 0.256 0.257 0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.212 0.221 0.222 0.221

$\begin{array}{c} 132\\ 133\\ 134\\ 135\\ 136\\ 137\\ 138\\ 140\\ 142\\ 143\\ 144\\ 145\\ 146\\ 147\\ 148\\ 149\\ 150\\ 151\\ 152\\ 156\\ 157\\ 158\\ 160\\ 162\\ 163\\ 164\\ 165\\ 166\\ 167\\ 168\\ 169\\ 171\\ 172\\ 173\\ 174\\ 175\\ 177\\ 178\\ 177\\ 178\\ 179\end{array}$	$\begin{array}{c} 11.00\\ 11.08\\ 11.17\\ 11.25\\ 11.33\\ 11.42\\ 11.50\\ 11.58\\ 11.67\\ 11.75\\ 11.83\\ 11.92\\ 12.00\\ 12.08\\ 12.17\\ 12.25\\ 12.33\\ 12.42\\ 12.50\\ 12.58\\ 12.67\\ 12.75\\ 12.83\\ 12.92\\ 13.00\\ 13.08\\ 13.17\\ 13.25\\ 13.33\\ 13.42\\ 13.50\\ 13.58\\ 13.67\\ 13.75\\ 13.83\\ 13.42\\ 13.50\\ 13.58\\ 13.67\\ 13.75\\ 13.83\\ 13.92\\ 14.00\\ 14.08\\ 14.17\\ 14.25\\ 13.83\\ 13.92\\ 14.00\\ 14.08\\ 14.75\\ 14.58\\ 14.50\\ 14.58\\ 14.67\\ 14.58\\ 14.67\\ 14.58\\ 14.67\\ 14.58\\ 14.67\\ 14.58\\ 14.67\\ 14.58\\ 14.92\\ 14$	0.67 0.63 0.63 0.63 0.63 0.63 0.57 0.57 0.57 0.57 0.60 0.60 0.83 0.83 0.83 0.83 0.93 0.93 0.93 0.93 0.97 0.97 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.23 1.13 1.13 1.13 1.13 1.23 1.13 1.23 1.231	0.476 0.452 0.452 0.452 0.452 0.452 0.452 0.452 0.452 0.405 0.405 0.405 0.405 0.595 0.595 0.595 0.619 0.619 0.666 0.666 0.666 0.666 0.690 0.809 0.809 0.809 0.809 0.809 0.809 0.809 0.809 0.809 0.809 0.809 0.547 0.549 0.595 0.595	0.239 0.238 0.237 0.235 0.234 0.233 0.232 0.231 0.230 0.228 0.227 0.226 0.225 0.224 0.223 0.222 0.220 0.219 0.219 0.218 0.217 0.216 0.215 0.214 0.213 0.212 0.214 0.213 0.212 0.214 0.213 0.212 0.214 0.213 0.212 0.214 0.209 0.208 0.207 0.206 0.205 0.204 0.201 0.200 0.201 0.200 0.201	(0.289) (0.274) (0.274) (0.274) (0.274) (0.274) (0.274) (0.274) (0.246) (0.246) (0.246) (0.260) (0.260) (0.260) (0.361) (0.361) (0.361) (0.376) (0.376) (0.404) (0.404) (0.404) (0.404) (0.419) (0.419) (0.419) (0.491) (0.491) (0.491) (0.491) (0.491) (0.491) (0.491) (0.491) (0.491) (0.332) (0.332) (0.332) (0.332) (0.332) (0.332) (0.332) (0.332) (0.332) (0.332) (0.332) (0.332) (0.376) (0.361) (0.361) (0.361) (0.361)	0.225 0.202 0.204 0.205 0.206 0.207 0.208 0.162 0.163 0.164 0.191 0.360 0.361 0.362 0.361 0.362 0.387 0.388 0.439 0.440 0.465 0.466 0.588 0.590 0.591 0.592 0.593 0.333 0.334 0.335 0.337 0.338 0.334 0.335 0.337 0.338 0.337 0.338 0.337 0.338 0.337 0.338 0.337 0.338 0.337 0.338 0.337 0.338 0.337 0.338 0.337 0.338 0.337 0.338 0.337 0.338 0.337 0.338 0.337 0.338 0.337 0.338 0.434 0.417 0.418 0.419 0.396 0.397
171 172 173 174 175 176 177	14.25 14.33 14.42 14.50 14.58 14.67 14.75	0.90 0.87 0.87 0.87 0.87 0.87 0.87 0.87	0.643 0.619 0.619 0.619 0.619 0.619 0.619	0.206 0.205 0.204 0.203 0.202 0.201 0.201 0.200	(0.390) (0.376) (0.376) (0.376) (0.376) (0.376) (0.376) (0.376)	0.436 0.414 0.415 0.416 0.417 0.418 0.419

193 194 195 196 197 198 199 200 201 202 203 203 204	$\begin{array}{c} 16.00\\ 16.08\\ 16.17\\ 16.25\\ 16.33\\ 16.42\\ 16.50\\ 16.58\\ 16.67\\ 16.75\\ 16.83\\ 16.92\\ 17.00\\ 17.08\\ 17.17\\ 17.25\\ 17.33\\ 17.42\\ 17.50\\ 17.58\\ 17.67\\ 17.75\\ 17.83\\ 17.92\\ 18.00\\ 18.08\\ 18.17\\ 18.25\\ 18.33\\ 18.42\\ 18.50\\ 18.58\\ 18.67\\ 18.75\\ 18.83\\ 18.92\\ 19.00\\ 19.08\\ 19.17\\ 19.25\\ 19.33\\ 19.42\\ 19.50\\ 19.58\\ 19.67\\ 19.58\\ 10.67\\ 10.58\\ 10.68\\ 10$	0.63 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.10 0.10 0.10 0.10 0.10 0.10 0.10 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.13 0.10	0.452 0.095 0.095 0.095 0.095 0.095 0.095 0.071 0.071 0.071 0.071 0.071 0.071 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.095 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071	0.185 (0.184) (0.183) (0.182) (0.181) (0.180) (0.180) (0.170) (0.179) (0.177) (0.177) (0.175) (0.174) (0.175) (0.172) (0.169) (0.168) (0.166) (0.166) (0.166) (0.166) (0.161) (0.162) (0.162) (0.162) (0.162) (0.162) (0.162) (0.163) (0.158) (0.158) (0.157) (0.158) (0.155) (0.157) (0.150) (0.150) (0.149) (0.144) (0.143) (0.143) (0.143)	(0.274) 0.058 0.058 0.058 0.058 0.058 0.043 0.043 0.043 0.043 0.043 0.043 0.043 0.072 0.072 0.072 0.072 0.072 0.072 0.072 0.072 0.072 0.072 0.072 0.072 0.072 0.072 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.043 0	0.267 0.037 0.037 0.037 0.037 0.037 0.037 0.028 0.028 0.028 0.028 0.028 0.028 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.037 0.028 0
242	20.17	0.10	0.071	(0.145)	0.043	0.028
243	20.25	0.10	0.071	(0.145)	0.043	0.028
244	20.33	0.10	0.071	(0.144)	0.043	0.028
245	20.42	0.10	0.071	(0.143)	0.043	0.028

252 253	21.00	0.07	0.048	(0.139)	0.029	0.01	
254	21.08	0.10 0.10	0.071 0.071	(0.139) 0.138)	0.043	0.02	8
255 256	21.25 21.33	0.10 0.07	0.071 0.048	(0.043 0.029	0.02 0.01	
257	21.42	0.07	0.048	ì		0.029	0.01	
258	21.50	0.07	0.048	(0.029	0.01	
259 260	21.58 21.67	0.10 0.10	0.071 0.071	(0.043 0.043	0.02	
261	21.75	0.10	0.071	(0.043	0.02	
262	21.83	0.07	0.048	(,	0.029	0.01	
263 264	21.92 22.00	0.07 0.07	0.048 0.048	(0.029 0.029	0.01 0.01	
265	22.08	0.10	0.071	(0.043	0.02	
266	22.17	0.10	0.071	(•	0.043	0.02	
267 268	22.25 22.33	0.10 0.07	0.071 0.048	(0.132) 0.131)	0.043 0.029	0.02 0.01	
269	22.42	0.07	0.048	(0.029	0.01	
270	22.50	0.07	0.048	(0.131)	0.029	0.01	
271 272	22.58 22.67	0.07 0.07	0.048 0.048	(0.130) 0.130)	0.029 0.029	0.01 0.01	
273	22.75	0.07	0.048	(0.130)	0.029	0.01	
274	22.83	0.07	0.048	(,	0.029	0.01	
275 276	22.92 23.00	0.07 0.07	0.048 0.048	(0.129) 0.129)	0.029 0.029	0.01 0.01	
277	23.08		0.048	(0.128)	0.029	0.01	
278	23.17	0.07	0.048	(0.128)	0.029	0.01	
279 280	23.25 23.33	0.07 0.07	0.048 0.048	(0.128) 0.127)	0.029 0.029	0.01 0.01	
281	23.42	0.07	0.048	(0.127)	0.029	0.01	
282	23.50		0.048	(0.127)	0.029	0.01	
283 284	23.58 23.67	0.07 0.07	0.048 0.048	(0.127) 0.127)	0.029 0.029	0.01 0.01	
285	23.75	0.07	0.048	(0.126)	0.029	0.01	
286 287	23.83 23.92	0.07	0.048	(,	0.029	0.01	
288	23.92	0.07 0.07	0.048 0.048	(0.126) 0.126)	0.029 0.029	0.01 0.01	
		(Loss Rate N		ţ	,			
	Sum =	100.0 volume = Effe	atima nai	nf-11	3.05(In	Sum =	36.6	
	times	s area	0.2(Ac.)/[(In)/(5.05(II) Ft.)] =	2.3(Ac.F	't)	
	Total	s area 5 soil loss = soil loss = rainfall =	2.90(In)		``	,	
	Total	soil loss =	2.216(Ac.Ft)				
	Flood	volume =	101310.2	Cubic	: Feet			
	Total	soil loss =	9652	8.6 Cu	bic Feet			
		flow rate of				1(CFS)		
	+++++	* + + + + + + + + + + + + + + + + + + +					• + + + + + + + + +	++++
			n o f f	Н	STORM ydrogr	aph		
					nute interva	als ((CFS))		
Tin		Volume Ac.Ft						10.0
		0.0005 0.0015						
()+10	0.0015	0.15 Q	2	I	1	1	l

$\begin{array}{cccccccccccccccccccccccccccccccccccc$	
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$10+15 \\ 10+20 \\ 10+25 \\ 10+30 \\ 10+35 \\ 10+40 \\ 10+45 \\ 10+50 \\ 10+55 \\ 11+0 \\ 11+5 \\ 11+10 \\ 11+15 \\ 11+20 \\ 11+25 \\ 11+30 \\ 11+35 \\ 11+40 \\ 11+45 \\ 11+55 \\ 12+0 \\ 12+5 \\ 12+10 \\ 12+5 \\ 12+20 \\ 12+25 \\ 12+20 \\ 12+5 \\ 12+30 \\ 12+35 \\ 12+40 \\ 12+45 \\ 12+50 \\ 12+55 \\ 12+50 \\ 12+55 \\ 13+0 \\ 0$	0.6627 0.6717 0.6806 0.7005 0.7138 0.7276 0.7416 0.7558 0.7700 0.7838 0.7968 0.8099 0.8229 0.8360 0.8492 0.8613 0.8720 0.8825 0.8937 0.9055 0.9176 0.9341 0.9556 0.9781 1.0017 1.0261 1.0507 1.0767 1.1042 1.1320 1.1606 1.1900 1.2197	$\begin{array}{c} 1.34\\ 1.31\\ 1.30\\ 1.30\\ 1.59\\ 1.93\\ 2.01\\ 2.04\\ 2.06\\ 2.07\\ 1.99\\ 1.90\\ 1.99\\ 1.90\\ 1.99\\ 1.90\\ 1.92\\ 1.75\\ 1.56\\ 1.53\\ 1.61\\ 1.72\\ 1.75\\ 2.39\\ 3.12\\ 3.27\\ 3.42\\ 3.55\\ 3.58\\ 3.77\\ 3.99\\ 4.04\\ 4.15\\ 4.27\\ 4.30\\ \end{array}$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
13+10 $13+15$ $13+20$ $13+25$ $13+30$ $13+35$ $13+40$ $13+45$ $13+55$ $14+0$ $14+55$ $14+10$ $14+15$ $14+20$ $14+25$ $14+20$ $14+25$ $14+30$ $14+35$ $14+40$ $14+45$ $14+50$ $14+55$ $15+0$ $15+5$ $15+10$	1.2889 1.3260 1.3635 1.4011 1.4388 1.4932 1.5151 1.5366 1.5579 1.5794 1.6033 1.6302 1.6576 1.6847 1.7112 1.7377 1.7642 1.7907 1.8173 1.8433 1.8687 1.9188 1.9429	5.28 5.40 5.44 5.46 5.47 4.50 3.18 3.12 3.10 3.11 3.48 3.90 3.99 3.93 3.85 3.84 3.84 3.85 3.84 3.85 3.86 3.78 3.69 3.69 3.50	$\begin{vmatrix} QV & & QV & & \\ QV & & QV & & \\ QV & VV & \\ VV & VV & \\ VV & VV & \\ QV & VV & \\ VV & VV & $

.

19+55 2.2558 0.18 Q I I V 20+0 2.2570 0.18 Q I I V 20+5 2.2584 0.21 Q I I V

24+10 2.3257 0.02 Q V 24+15 2.3257 0.01 Q V 24+20 2.3258 0.00 Q V	20+15 20+20 20+25 20+30 20+35 20+40 20+45 20+50 20+55 21+0 21+5 21+10 21+15 21+20 21+25 21+30 21+35 21+40 21+45 21+50 21+55 22+0 22+55 22+10 22+55 22+10 22+25 22+20 22+25 22+30 22+35 22+40 22+35 22+40 22+55 22+40 22+55 22+40 22+55 22+10 22+55 22+10 22+55 22+10 22+55 22+30 22+55 22+40 22+55 22+40 22+55 23+10 23+55 23+10 23+25 23+20 23+25 23+40 23+25 23+40 23+45 23+40 23+45 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+25 23+40 23+45 23+40 23+45 23+40 23+45 23+40 23+45 23+40 23+45 23+40 23+45 23+40 23+45 23+40 23+45 23+40 23+45 23+40 23+45 23+40 23+45 23+40 23+45 23+40 23+45 23+40 23+45 23+40 23+45 23+40 23+45 23+50 23+55 24+0 23+55 24+0 23+55 24+0 23+55 24+0 24+55 24+50 24+55	2.2619 2.2637 2.2655 2.2673 2.2690 2.2708 2.2726 2.2742 2.2754 2.2754 2.2754 2.2766 2.2781 2.2831 2.2843 2.2856 2.2870 2.2887 2.2905 2.2920 2.2933 2.2945 2.2905 2.2920 2.2933 2.2945 2.2959 2.2959 2.2959 2.2959 2.2959 2.2959 2.2959 2.2959 2.2959 2.2959 2.2959 2.2959 2.3034 2.3009 2.3022 2.3034 2.3058 2.3070 2.3082 2.3094 2.3117 2.3129 2.3141 2.3153 2.3165 2.3177 2.3189 2.3201 2.3213 2.3225 2.3236 2.3248 2.3255	0.26 0.26 0.26 0.26 0.26 0.22 0.18 0.22 0.18 0.22 0.18 0.22 0.22 0.18 0.25 0.26 0.22 0.18 0.25 0.26 0.22 0.18 0.25 0.26 0.22 0.18 0.25 0.26 0.22 0.18 0.21 0.25 0.26 0.22 0.18 0.21 0.25 0.22 0.18 0.21 0.25 0.22 0.18 0.21 0.25 0.22 0.18 0.21 0.25 0.22 0.18 0.21 0.25 0.22 0.18 0.21 0.25 0.22 0.18 0.21 0.25 0.22 0.18 0.21 0.25 0.22 0.18 0.21 0.25 0.22 0.18 0.21 0.25 0.22 0.18 0.21 0.25 0.22 0.18 0.21 0.25 0.21 0.21 0.25 0.22 0.18 0.21 0.25 0.22 0.18 0.17				V V V V V V V V	
24+ 0 2.3248 0.17 Q V 24+ 5 2.3255 0.10 Q V 24+10 2.3257 0.02 Q V 24+15 2.3257 0.01 Q V				Q			V	
24+5 2.3255 0.10 Q V 24+10 2.3257 0.02 Q V 24+15 2.3257 0.01 Q V								
24+10 2.3257 0.02 Q V 24+15 2.3257 0.01 Q V								
24+15 2.3257 0.01 Q V					1			
					1			
Z4+ZU Z.3Z58 U.UU Q V					1	1		
	24+20	2.3258	0.00	Q			V I	

UNDEVELOPED 100 YEAR – 24 HOUR AREA B₁

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Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318undB124100.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
_____
St. Frances of Rome - Wildomar
Undeveloped B1
100-Year 24-Hour
_____
Drainage Area = 2.32(Ac.) = 0.004 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 2.32(Ac.) = 0.004 Sq.
Length along longest watercourse = 331.70(Ft.)
Length along longest watercourse measured to centroid = 158.90(Ft.)
Length along longest watercourse = 0.063 Mi.
Length along longest watercourse measured to centroid = 0.030 Mi.
Difference in elevation = 11.50(Ft.)
Slope along watercourse = 183.0570 Ft./Mi.
Average Manning's 'N' = 0.015
Lag time = 0.012 Hr.
Lag time = 0.74 Min.
25% of lag time = 0.19 Min.
40% of lag time = 0.30 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
              Rainfall(In)[2] Weighting[1*2]
Area(Ac.)[1]
      2.32
                                      5.17
                    2.23
100 YEAR Area rainfall data:
Area(Ac.)[1]
             Rainfall(In)[2] Weighting[1*2]
      2.32
              5.95
                                  13.80
STORM EVENT (YEAR) = 100.00
```

Mi.

Area Averaged 2-Year Rainfall = 2.230(In) Area Averaged 100-Year Rainfall = 5.950(In) Point rain (area averaged) = 5.950(In) Areal adjustment factor = 100.00 % Adjusted average point rain = 5.950(In) Sub-Area Data:
 Area(Ac.)
 Runoff Index
 Impervio

 2.320
 81.09
 0.854
 Runoff Index Impervious % Total Area Entered = 2.32(Ac.)
 RI
 RI
 Infil. Rate
 Impervious
 Adj. Infil. Rate
 Area%
 F

 AMC2
 AMC-2
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 81.1
 81.1
 0.232
 0.854
 0.054
 1.000
 0.054
 Sum (F) = 0.054Area averaged mean soil loss (F) (In/Hr) = 0.054Minimum soil loss rate ((In/Hr)) = 0.027(for 24 hour storm duration) Soil low loss rate (decimal) = 0.217 Unit Hydrograph DESERT S-Curve _____ Unit Hydrograph Data _____ Unit time period Time % of lag Distribution Unit Hydrograph (hrs) Graph % (CFS) 1 0.083 674.993 78.099 2 0.167 1349.985 21.901 1.826 349.98521.9010.512Sum = 100.000Sum=2.338

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit	Time	Pattern	Storm Rain	Loss rate(In./Hr)		Effective
	(Hr.)	Percent	(In/Hr)	Max	Low	(In/Hr)
1	0.08	0.07	0.048	(0.095)	0.010	0.037
2	0.17	0.07	0.048	(0.095)	0.010	0.037
3	0.25	0.07	0.048	(0.094)	0.010	0.037
4	0.33	0.10	0.071	(0.094)	0.015	0.056
5	0.42	0.10	0.071	(0.094)	0.015	0.056
6	0.50	0.10	0.071	(0.093)	0.015	0.056
7	0.58	0.10	0.071	(0.093)	0.015	0.056
8	0.67	0.10	0.071	(0.092)	0.015	0.056
9	0.75	0.10	0.071	(0.092)	0.015	0.056
10	0.83	0.13	0.095	(0.092)	0.021	0.075
11	0.92	0.13	0.095	(0.091)	0.021	0.075
12	1.00	0.13	0.095	(0.091)	0.021	0.075
13	1.08	0.10	0.071	(0.091)	0.015	0.056
14	1.17	0.10	0.071	(0.090)	0.015	0.056
15	1.25	0.10	0.071	(0.090)	0.015	0.056
16	1.33	0.10	0.071	(0.090)	0.015	0.056
17	1.42	0.10	0.071	(0.089)	0.015	0.056
18	1.50	0.10	0.071	(0.089)	0.015	0.056

19 21 22 22 22 22 22 22 23 33 33 33 33 33 33	$\begin{array}{c} 1.58\\ 1.67\\ 1.75\\ 1.83\\ 2.00\\ 2.17\\ 2.33\\ 2.58\\$	0.10 0.10 0.10 0.10 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.20 0.27 0.27 0.27 0.30 0.30 0.30	0.071 0.071 0.071 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.119 0.143 0.143 0.143 0.143 0.143 0.143 0.143 0.167 0.167 0.167 0.167 0.167 0.167 0.167 0.167 0.167 0.167 0.167 0.167 0.167 0.167 0.167 0.190 0.190 0.190 0.190 0.190 0.190 0.190 0.190 0.190 0.190 0.190 0.190 0.190 0.190 0.190 0.214 0.214 0.214	(0.088) (0.088) (0.087) (0.087) (0.087) (0.087) (0.086) (0.086) (0.086) (0.085) (0.085) (0.084) (0.084) (0.084) (0.083) (0.083) (0.082) (0.082) (0.082) (0.081) (0.081) (0.081) (0.081) (0.081) (0.081) (0.081) (0.081) (0.080) (0.080) (0.079) (0.079) (0.079) (0.079) (0.078) (0.078) (0.077) (0.077) (0.077) (0.075) (0.071) (0.071) (0.071) (0.071) (0.071) (0.070) (0.070) (0.070) (0.070)	0.015 0.015 0.021 0.021 0.021 0.021 0.021 0.021 0.021 0.021 0.026 0.031 0.031 0.031 0.031 0.036 0.036 0.036 0.036 0.031 0.041 0.041 0.041 0.041 0.041 0.041 0.041 0.041 0.046 0.046 0.046	0.056 0.056 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.073 0.093 0.112 0.112 0.112 0.112 0.130 0.130 0.130 0.130 0.130 0.149 0.149 0.149 0.149 0.149 0.149 0.149 0.149 0.149

79 80 81 82 83 84 85 87 89 90 91 92 93 94 95 96 97 99 90 101 102 103 104 105 106 107 108 90 111 112 123 124 126 127 128 130 131 132 134 135 136	6.58 6.67 6.75 6.83 6.92 7.00 7.08 7.17 7.25 7.33 7.42 7.58 7.67 7.75 8.00 8.08 8.17 8.25 8.33 8.42 8.50 8.58 8.75 8.33 8.42 8.00 9.08 9.17 9.25 9.33 9.25 9.67 9.58 9.67 9.58 9.67 9.58 9.67 9.58 9.67 9.58 9.67 9.58 9.67 9.58 9.67 9.58 9.67 9.58 9.67 9.58 10.25 10.25 10.25 10.50 10.50 10.50 10.50 10.50 10.50 10.67 10.50 10.67 10.83 10.92 11.00 11.08 1.17 11.25 11.33	0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.37 0.37 0.37 0.40 0.40 0.40 0.40 0.43 0.43 0.43 0.43 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.53 0.53 0.53 0.57 0.63 0.63 0.67 0.70 0.70 0.70 0.70 0.73 0.50 0.67 0.67 0.67 0.67 0.67 0.67 0.67 0.67 0.67 0.67 0.63	0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.262 0.262 0.262 0.262 0.286 0.286 0.286 0.309 0.309 0.309 0.357 0.35	(0.069) (0.068) (0.068) (0.067) (0.067) (0.067) (0.066) (0.066) (0.066) (0.065) (0.065) (0.065) (0.064) 0.064 0.063 0.063 0.063 0.063 0.063 0.063 0.062 0.062 0.062 0.062 0.062 0.061 0.061 0.061 0.061 0.061 0.061 0.061 0.061 0.062 0.059 0.050 0.050 0.055	0.052 0.052 0.052 0.052 0.052 0.052 0.052 0.052 0.052 0.052 0.057 0.057 0.057 0.062 0.062 0.062 0.062 0.067 (0.067) (0.077) (0.077) (0.077) (0.077) (0.077) (0.077) (0.077) (0.083) (0.083) (0.083) (0.083) (0.083) (0.083) (0.083) (0.088) (0.088) (0.088) (0.098) (0.098) (0.098) (0.103) (0.098) (0.098)	0.186 0.186 0.186 0.186 0.186 0.186 0.186 0.205 0.205 0.224 0.224 0.224 0.224 0.246 0.246 0.246 0.294 0.294 0.294 0.295 0.295 0.320 0.320 0.320 0.320 0.320 0.344 0.345 0.393 0.393 0.393 0.393 0.417 0.417 0.417 0.418 0.442 0.442 0.442 0.467 0.301 0.301 0.301 0.302 0.302 0.302 0.302 0.421 0.422 0

139	11.58	0.57	0.405	0.052	(0.088)	0.353
140	11.67	0.57	0.405	0.051	(0.088)	0.353
141	11.75	0.57	0.405	0.051	(0.088)	0.354
142	11.83	0.60	0.428	0.051		0.378
143	11.92	0.60	0.428	0.051	(0.093)	0.378
144	12.00	0.60	0.428	0.050	(0.093)	0.378
145	12.08	0.83	0.595	0.050	(0.129)	0.545
146	12.17	0.83	0.595	0.050	(0.129)	0.545
147	12.25	0.83	0.595	0.050	(0.129)	0.545
148	12.33	0.87	0.619	0.049	(0.134)	0.570
149	12.42	0.87	0.619	0.049	(0.134)	0.570
150	12.50	0.87	0.619	0.049	(0.134)	0.570
151	12.58	0.93	0.666	0.049	(0.145)	0.618
152	12.67	0.93	0.666	0.048	(0.145)	0.618
153	12.75	0.93	0.666	0.048	(0.145)	0.618
154	12.83	0.97	0.690	0.048	(0.150)	0.642
155	12.92	0.97	0.690	0.048	(0.150)	0.643
156	13.00	0.97	0.690	0.047	(0.150)	0.643
157	13.08	1.13	0.809	0.047	(0.176)	0.762
158	13.17	1.13	0.809	0.047	(0.176)	0.762
159	13.25	1.13	0.809	0.047	(0.176)	0.763
160 161	13.33 13.42	1.13 1.13	0.809 0.809	0.046 0.046	(0.176)	0.763
162	13.50	1.13	0.809	0.046	(0.176)	0.763 0.763
163	13.58	0.77	0.547	0.046	(0.119)	0.502
164	13.67	0.77	0.547	0.045	(0.119)	0.502
165	13.75	0.77	0.547	0.045	(0.119)	0.502
166	13.83	0.77	0.547	0.045	(0.119)	0.502
167	13.92	0.77	0.547	0.045	(0.119)	0.503
168 169	14.00 14.08	0.77 0.90	0.547 0.643	0.045 0.044	(0.119)	0.503
170	14.17	0.90	0.643	0.044	(0.139)	0.598 0.599
171	14.25	0.90	0.643	$0.044 \\ 0.044$	(0.139)	0.599
172	14.33	0.87	0.619		(0.134)	0.575
173	14.42	0.87	0.619	0.043	(0.134)	0.575
174	14.50	0.87	0.619	0.043	(0.134)	0.576
175	14.58	0.87	0.619	0.043	(0.134)	0.576
176	14.67	0.87	0.619	0.043	(0.134)	0.576
177	14.75	0.87	0.619	0.043		0.576
178	14.83	0.83	0.595	0.042	(0.129)	0.553
179	14.92	0.83	0.595	0.042	(0.129)	0.553
180	15.00	0.83	0.595	0.042	(0.129)	0.553
181	15.08	0.80	0.571	0.042	(0.124)	0.530
182	15.17	0.80	0.571	0.041	(0.124)	0.530
183	15.25	0.80	0.571	0.041	(0.124)	0.530
184 185	15.33	0.77	0.547	0.041	(0.119)	0.506
186	15.42	0.77	0.547	0.041	(0.119)	0.507
	15.50	0.77	0.547	0.041	(0.119)	0.507
187	15.58	0.63	0.452	0.040	(0.098)	0.412
188	15.67	0.63	0.452	0.040	(0.098)	0.412
189	15.75	0.63	0.452	0.040	(0.098)	0.412
190	15.83	0.63	0.452	0.040	(0.098)	0.412
191	15.92	0.63	0.452	0.040	(0.098)	0.413
192	16.00	0.63	0.452	0.039	(0.098)	0.413
193	16.08	0.13	0.095	(0.039)	0.021	0.075
194	16.17	0.13	0.095	(0.039)	0.021	0.075
195	16.25	0.13	0.095	(0.039)	0.021	0.075
196	16.33	0.13	0.095	(0.039)	0.021	0.075
197	16.42	0.13	0.095	(0.038)	0.021	0.075
198	16.50	0.13	0.095	(0.038)	0.021	0.075

$\begin{array}{c} 199\\ 200\\ 201\\ 202\\ 203\\ 204\\ 205\\ 206\\ 207\\ 208\\ 209\\ 210\\ 211\\ 212\\ 213\\ 214\\ 215\\ 216\\ 217\\ 218\\ 220\\ 221\\ 222\\ 223\\ 224\\ 225\\ 226\\ 227\\ 228\\ 229\\ 230\\ 231\\ 232\\ 233\\ 235\\ 236\\ 237\\ 238\\ 239\\ 240\\ 241\\ 242\\ 243\\ 244\\ 245\\ 246\\ 247\\ 248\\ 245\\ 246\\ 247\\ 248\\ 245\\ 246\\ 247\\ 248\\ 245\\ 246\\ 247\\ 248\\ 245\\ 246\\ 247\\ 248\\ 245\\ 246\\ 247\\ 248\\ 245\\ 246\\ 247\\ 248\\ 245\\ 246\\ 247\\ 248\\ 245\\ 246\\ 247\\ 248\\ 245\\ 246\\ 247\\ 248\\ 245\\ 246\\ 247\\ 248\\ 245\\ 246\\ 247\\ 248\\ 245\\ 246\\ 247\\ 248\\ 245\\ 251\\ 251\\ 251\\ 251\\ 251\\ 251\\ 251\\ 25$	$16.58 \\ 16.67 \\ 16.75 \\ 16.83 \\ 16.92 \\ 17.00 \\ 17.08 \\ 17.17 \\ 17.25 \\ 17.33 \\ 17.42 \\ 17.50 \\ 17.58 \\ 17.67 \\ 17.75 \\ 17.83 \\ 17.92 \\ 18.00 \\ 18.08 \\ 18.17 \\ 18.25 \\ 18.33 \\ 18.42 \\ 18.50 \\ 18.58 \\ 18.57 \\ 18.58 \\ 18.67 \\ 18.75 \\ 18.83 \\ 18.92 \\ 19.00 \\ 19.08 \\ 19.17 \\ 19.25 \\ 19.33 \\ 19.42 \\ 19.50 \\ 19.58 \\ 19.50 \\ 19.75 \\ 19.83 \\ 19.92 \\ 20.00 \\ 20.88 \\ 19.75 \\ 19.75 \\ 19.83 \\ 19.92 \\ 20.00 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.83 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.58 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.75 \\ 20.83 \\ 20.67 \\ 20.83 \\ 20.8$	0.10 0.10 0.10 0.10 0.10 0.10 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.13 0.10 0.07	0.071 0.071 0.071 0.071 0.071 0.071 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.095 0.071 0	(0.038) (0.038) (0.038) (0.038) (0.037) (0.037) (0.037) (0.037) (0.036) (0.036) (0.036) (0.036) (0.036) (0.036) (0.036) (0.035) (0.034) (0.034) (0.034) (0.034) (0.034) (0.033) (0.033) (0.033) (0.033) (0.033) (0.032) (0.032) (0.032) (0.032) (0.032) (0.032) (0.031) (0.031) (0.031) (0.030) (0.	0.015 0.015 0.015 0.015 0.015 0.026 0.026 0.026 0.026 0.026 0.026 0.026 0.026 0.026 0.021 0.015 0.01	0.056 0.056 0.056 0.056 0.056 0.093 0.093 0.093 0.093 0.093 0.093 0.093 0.093 0.093 0.093 0.093 0.093 0.093 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.056 0
246	20.50	0.10	0.071	(0.030)	0.015	0.056
247	20.58	0.10	0.071	(0.030)	0.015	0.056
248	20.67	0.10	0.071	(0.030)	0.015	0.056
249	20.75	0.10	0.071	(0.030)	0.015	0.056

	21.67 21.75 21.83 21.92 22.00 22.08 22.17 22.25 22.33 22.42 22.50 22.58 22.67 22.75 22.83 22.92 23.00 23.08 23.17 23.25 23.33 23.42 23.50 23.58 23.50 23.58 23.50 23.58 23.50 23.58 23.50 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.58 23.67 23.75 23.83 23.92 24.00 50 50 51 51 51 51 51 51 51 51 51 51 51 51 51	0.10 0.10 0.07 0.07 0.07 0.10 0.10 0.10 0.07	0.071 0.048 0.048 0.048 0.071 0.071 0.071 0.048 0.049 0.153 (F 5.95(Ir 43439.1 6669 F	(((((((((((((((((((0.029) 0.028) 0.028) 0.028) 0.028) 0.028) 0.028) 0.028) 0.028) 0.028) 0.028) 0.028) 0.028) 0.028) 0.027] 0.027] 0.027] 0.027] 0.027] 0.027] 0.027] 0.027]	Sum = In) 1.0(Ac.) 785(CFS) ++++++++++ M	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	56 56 37 37 56 56 56 56 37 37 37 37 37 37 37 37 37 37 37 37 37
						vals ((CFS)		
							·	
Tim 	e(h+m) 	Volume Ac.Ft	= Q(CFS)	0	2.5	5.0	7.5	10.0
0 0 0 0 0	+ 5 +10 +15 +20 +25 +30 +35	0.0005 0.0011 0.0017 0.0025 0.0034 0.0043 0.0052	0.07 Q 0.09 Q 0.09 Q 0.12 Q 0.13 Q 0.13 Q 0.13 Q					

0+50	0.0081	0.16 Q	1			
0+55 1+ 0	0.0093 0.0105	0.17 Q 0.17 Q				
1+ 5	0.0115	0.14 Q				
1+10	0.0124	0.13 Q			-	
1+15	0.0133	0.13 Q				1
1+20 1+25	0.0142 0.0151	0.13 Q 0.13 Q				
1+30	0.0160	0.13 Q	1		a statement of the stat	
1+35	0.0169	0.13 Q	- Marine		1	
1+40	0.0178	0.13 Q				
1+45 1+50	0.0187 0.0199	0.13 Q 0.16 Q			1	
1+55	0.0211	0.16 Q 0.17 Q				1
2+ 0	0.0223	0.17 Q			E	
2+ 5	0.0235	0.17 Q				
2+10	0.0247	0.17 Q				1
2+15 2+20	0.0259 0.0271	0.17 QV 0.17 QV			ł	1
2+25	0.0283	0.17 QV				
2+30	0.0295	0.17 QV				
2+35	0.0309	0.21 QV				
2+40 2+45	0.0324 0.0339	0.22 QV 0.22 QV			1	1
2+45	0.0354	0.22 QV 0.22 QV		1		
2+55	0.0369	0.22 QV	İ			
3+ 0	0.0384	0.22 QV		1		
3+ 5	0.0399 0.0414	0.22 QV 0.22 QV				
3+10 3+15	0.0414	0.22 QV 0.22 QV				
3+20	0.0444	0.22 QV				
3+25	0.0459	0.22 QV		I		
3+30	0.0474	0.22 QV			E	
3+35 3+40	0.0489 0.0504	0.22 QV 0.22 Q V				
3+45	0.0519	0.22 Q V				
3+50	0.0536	0.25 QV				
3+55	0.0554	0.26 QV				
4+ 0 4+ 5	0.0573 0.0591	0.26 QV 0.26 QV				
4+10	0.0609	0.26 QV				
4+15	0.0627	0.26 QV				
4+20	0.0647	0.30 QV				1
4+25 4+30	0.0668 0.0689	0.31 QV 0.31 QV		1		1
4+35	0.0710	0.31 QV				
4 + 4 0	0.0731	0.31 QV	5 Television	L.	l	
4+45	0.0752	0.31 Q V				
4+50 4+55	0.0775 0.0799	0.34 Q V 0.35 Q V	1		1	
4+33 5+ 0	0.0823	0.35 IQ V			1	
5+ 5	0.0843	0.28 Q V		L	Ì	
5+10	0.0861	0.26 Q V	NUMBER	I		
5+15	0.0879	0.26 Q V		1	-	
5+20 5+25	0.0899 0.0920	0.30 Q V 0.31 Q V		ł	1	1
5+30	0.0941	0.31 Q V		E	*	
5+35	0.0965	0.34 Q V		stant		
5+40	0.0989	0.35 Q V	1			
5+45	0.1013	0.35 Q V	l	I		l

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6+ 0 0.1085 0.35 $ $	
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$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
	$\begin{array}{c c c c c c c c c c c c c c c c c c c $
	V V V V V V V V

20+25 0.9673 0.13 Q V 20+30 0.9682 0.13 Q V V 20+35 0.9691 0.13 Q I I V V 20+40 0.9700 0.13 Q I I V V

20+50	0.9716	0.10 Q	1		a constantino de la c	V I
20+55	0.9722	0.09 Q		1		V
21+ 0	0.9728	0.09 Q		*****		V
21+ 5	0.9736	0.12 Q		1		V I
21+10	0.9745	0.13 Q				VI
21+15	0.9754	0.13 Q		1		VI
21+20	0.9761	0.10 Q'				V
21+25	0.9767	0.09 Q				VI
21+30	0.9773	0.09 Q		1	1	VI
21+35	0.9781	0.12 Q			1	V I
21+40	0.9790	0.13 Q				VI
21+45	0.9799	0.13 Q	Land			V
21+50	0.9806	0.10 Q	1			V
21+55	0.9812	0.09 Q				V I
22+ 0	0.9818	0.09 Q	1			V
22+ 5	0.9826	0.12 Q				VI
22+10	0.9835	0.13 Q				V
22+15	0.9844	0.13 Q				V
22+20	0.9851	0.10 Q				V
22+25	0.9857	0.09 Q			1	V
22+30	0.9863	0.09 Q			1	VI
22+35	0.9869	0.09 Q				VI
22+40	0.9875	0.09 Q		1		V
22+45	0.9881	0.09 Q				V
22+50	0.9887	0.09 Q	a de la compansión de la c	1	-	VI
22+55	0.9893	0.09 Q		ł		VI
23+ 0	0.9899	0.09 Q	500 P	1		VI
23+ 5	0.9905	0.09 Q		I		VI
23+10	0.9911	0.09 Q	1		1	V
23+15	0.9917	0.09 Q				VI
23+20	0.9923	0.09 Q	-		1	VI
23+25	0.9929	0.09 Q			l	VI
23+30	0.9935	0.09 Q			1	VI
23+35	0.9941	0.09 Q	1			V
23+40	0.9947	0.09 Q		1		V I
23+45	0.9953	0.09 Q				VI
23+50	0.9959	0.09 Q		1		VI
23+55	0.9965	0.09 Q	I	1		V
24+ 0	0.9971	0.09 Q			·	V
24+ 5	0.9972	0.02 Q				VI

DEVELOPED 100 YEAR – 24 HOUR AREA C₁ & F₃

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Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318devC1F324100.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
St. Frances of Rome - Wildomar
Developed C1 & F3
100-Year 24-Hour
_____
Drainage Area = 4.11(Ac.) = 0.006 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment =
                                         4.11(Ac.) = 0.006 Sq.
Length along longest watercourse = 664.40(Ft.)
Length along longest watercourse measured to centroid = 398.60(Ft.)
Length along longest watercourse = 0.126 Mi.
Length along longest watercourse measured to centroid = 0.075 Mi.
Difference in elevation = 23.30(Ft.)
Slope along watercourse = 185.1656 Ft./Mi.
Average Manning's 'N' = 0.020
Lag time = 0.030 Hr.
Lag time = 1.82 Min.
25% of lag time = 0.46 Min.
40% of lag time = 0.73 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
              Rainfall(In)[2] Weighting[1*2]
Area(Ac.)[1]
      4.11
                                      9.17
                    2.23
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
      4.11
                                     24.45
               5.95
STORM EVENT (YEAR) = 100.00
```

Mi.

Area Averaged 2-Year Rainfall = 2.230(In) Area Averaged 100-Year Rainfall = 5.950(In) Point rain (area averaged) = 5.950(In) Areal adjustment factor = 100.00 % Adjusted average point rain = 5.950(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious 💡 3.05063.610.4981.06078.000.000 3.050 Total Area Entered = 4.11(Ac.) RI RI Infil. Rate Impervious Adj. Infil. Rate Area% F

 AMC2
 AMC-2
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 63.6
 63.6
 0.431
 0.498
 0.238
 0.742
 0.176

 78.0
 78.0
 0.268
 0.000
 0.268
 0.258
 0.069

 Sum (F) = 0.246Area averaged mean soil loss (F) (In/Hr) = 0.246Minimum soil loss rate ((In/Hr)) = 0.123(for 24 hour storm duration) Soil low loss rate (decimal) = 0.604 _____ Unit Hydrograph DESERT S-Curve Unit Hydrograph Data Unit time period Time % of lag Distribution Unit Hydrograph (hrs) Graph % (CFS) _____ 10.083274.71953.24120.167549.43840.40730.250824.1566.352 2.205 1.674 0.263 Sum = 100.000 Sum= 4.142 _____

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit	Time	Pattern	Storm Rain		e(In./Hr)	Effective
	(Hr.)	Percent	(In/Hr)	Max	Low	(In/Hr)
1	0.08	0.07	0.048	(0.435)	0.029	0.019
2	0.17	0.07	0.048	(0.434)	0.029	0.019
3	0.25	0.07	0.048	(0.432)	0.029	0.019
4	0.33	0.10	0.071	(0.430)	0.043	0.028
5	0.42	0.10	0.071	(0.429)	0.043	0.028
6	0.50	0.10	0.071	(0.427)	0.043	0.028
7	0.58	0.10	0.071	(0.425)	0.043	0.028
8	0.67	0.10	0.071	(0.424)	0.043	0.028
9	0.75	0.10	0.071	(0.422)	0.043	0.028
10	0.83	0.13	0.095	(0.420)	0.058	0.038
11	0.92	0.13	0.095	(0.419)	0.058	0.038
12	1.00	0.13	0.095	(0.417)	0.058	0.038
13	1.08	0.10	0.071	(0.415)	0.043	0.028
14	1.17	0.10	0.071	(0.414)	0.043	0.028
15	1.25	0.10	0.071	(0.412)	0.043	0.028

17 19 22 22 22 22 22 22 22 22 22 22 22 22 22	$\begin{array}{c} 1.33\\ 1.42\\ 1.50\\ 1.58\\ 1.67\\ 1.75\\ 1.89\\ 2.08\\ 7.53\\ 2.58\\ 7.5\\ 5.58\\ 5.58\\ 5.58\\ 5.58\\ 5.58\\ 5.58\\ 5.58\\ 5.67\\ 5.89\\ 2.08\\ 7.5\\ 5.58\\ 5.67\\ 5.58\\ 5.67\\ 5.89\\ 2.08\\ 7.5\\ 5.67\\ 5.89\\ 2.08\\ 7.5\\ 7.5\\ 5.89\\ 2.08\\ 7.5\\ 7.5\\ 5.89\\ 2.08\\ 7.5\\ 7.5\\ 5.89\\ 2.08\\ 7.5\\ 7.5\\ 5.89\\ 2.08\\ 7.5\\ 7.5\\ 5.89\\ 2.08\\ 7.5\\ 7.5\\ 7.5\\ 7.5\\ 7.5\\ 7.5\\ 7.5\\ 7.5$	0.10 0.10 0.10 0.10 0.10 0.10 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.20 0.20 0.20 0.20 0.20 0.20 0.20 0.20 0.23	0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.119 0.143 0.143 0.143 0.143 0.143 0.143 0.143 0.143 0.143 0.167 0.167 0.167 0.167 0.167 0.167 0.190 0.19	(0.410) (0.409) (0.407) (0.405) (0.401) (0.399) (0.397) (0.396) (0.394) (0.393) (0.391) (0.389) (0.388) (0.386) (0.385) (0.381) (0.381) (0.377) (0.375) (0.374) (0.372) (0.372) (0.372) (0.372) (0.360) (0.361) (0.361) (0.364) (0.361) (0.361) (0.361) (0.355) (0.357) (0.355) (0.354) (0.352) (0.351) (0.348) (0.344) (0.344) (0.352) (0.351) (0.342) (0.344) (0.345) (0.342) (0.334) (0.334) (0.334) (0.334) (0.334) (0.332) (0.332) (0.324) (0.322) (0.324) (0.322) (0.324) (0.322) (0.324) (0.322) (0.324) (0.322) (0.324) (0.322) (0.324) (0.	0.043 0.043 0.043 0.043 0.043 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.072 0.07	0.028 0.028 0.028 0.028 0.028 0.028 0.038 0.038 0.038 0.038 0.038 0.038 0.038 0.038 0.038 0.038 0.038 0.038 0.038 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.057 0.075 0

$\begin{array}{c} 136\\ 137\\ 138\\ 139\\ 140\\ 142\\ 143\\ 144\\ 145\\ 146\\ 147\\ 148\\ 149\\ 150\\ 152\\ 155\\ 156\\ 157\\ 158\\ 160\\ 162\\ 163\\ 166\\ 167\\ 168\\ 169\\ 171\\ 177\\ 176\\ 177\\ 178\\ 182\\ 183\\ 185\\ 186\\ 187\\ 188\end{array}$	$\begin{array}{c} 11.33\\ 11.42\\ 11.50\\ 11.58\\ 11.67\\ 11.75\\ 11.83\\ 11.92\\ 12.00\\ 12.08\\ 12.17\\ 12.25\\ 12.33\\ 12.42\\ 12.50\\ 12.58\\ 12.67\\ 12.75\\ 12.83\\ 12.92\\ 13.00\\ 13.08\\ 13.17\\ 13.25\\ 13.33\\ 13.42\\ 13.50\\ 13.58\\ 13.67\\ 13.75\\ 13.83\\ 13.92\\ 14.00\\ 13.58\\ 13.67\\ 13.75\\ 13.83\\ 13.92\\ 14.00\\ 14.08\\ 14.17\\ 14.25\\ 13.83\\ 13.92\\ 14.00\\ 14.08\\ 14.17\\ 14.25\\ 14.33\\ 14.42\\ 14.50\\ 14.58\\ 14.67\\ 14.75\\ 14.83\\ 14.92\\ 15.00\\ 15.08\\ 15.17\\ 15.25\\ 15.33\\ 15.42\\ 15.50\\ 15.58\\ 15.67\\ \end{array}$	0.63 0.63 0.63 0.57 0.57 0.57 0.60 0.60 0.60 0.83 0.83 0.83 0.83 0.87 0.93 0.93 0.97 0.97 0.97 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.57 0.77 0.90 0.87 0.87 0.83 0.83 0.80 0.80 0.77 0.77 0.77 0.77 0.77 0.77 0.77 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.87 0.87 0.83 0.83 0.80 0.80 0.63	0.452 0.452 0.452 0.405 0.405 0.405 0.428 0.428 0.428 0.595 0.595 0.595 0.619 0.619 0.666 0.666 0.666 0.666 0.666 0.690 0.690 0.809 0.809 0.809 0.809 0.809 0.809 0.809 0.547 0.54	0.231 0.230 0.229 0.228 0.227 0.226 0.225 0.223 0.222 0.221 0.220 0.219 0.218 0.217 0.216 0.215 0.214 0.212 0.211 0.209 0.209 0.208 0.207 0.206 0.209 0.208 0.207 0.206 0.205 0.204 0.205 0.204 0.205 0.204 0.202 0.201 0.202 0.201 0.202 0.201 0.199 0.198 0.197 0.196 0.195 0.191 0.192 0.189 0.185 0.184	$ \left(\begin{array}{c} 0.273 \\ 0.273 \\ (0.273 \\ 0.273 \\ (0.244 \\) \\ (0.244 \\) \\ (0.244 \\) \\ (0.259 \\) \\ (0.259 \\) \\ (0.259 \\) \\ (0.259 \\) \\ (0.259 \\) \\ (0.359 \\) \\ (0.359 \\) \\ (0.374 \\) \\ (0.374 \\) \\ (0.374 \\) \\ (0.374 \\) \\ (0.403 \\) \\ (0.403 \\) \\ (0.403 \\) \\ (0.403 \\) \\ (0.403 \\) \\ (0.403 \\) \\ (0.403 \\) \\ (0.447 \\) \\ (0.447 \\) \\ (0.447 \\) \\ (0.447 \\) \\ (0.4489 \\) \\ (0.4489 \\) \\ (0.489 \\) \\ (0.489 \\) \\ (0.489 \\) \\ (0.489 \\) \\ (0.489 \\) \\ (0.331 \\) \\ (0.331 \\) \\ (0.331 \\) \\ (0.331 \\) \\ (0.374 \\) \\ (0.331 \\) \\ (0.331 \\) \\ (0.273 \\)$	0.212 0.214 0.215 0.168 0.170 0.171 0.196 0.366 0.367 0.368 0.393 0.394 0.444 0.445 0.446 0.471 0.472 0.473 0.593 0.595 0.596 0.597 0.598 0.3340 0.341 0.342 0.343 0.341 0.342 0.341 0.342 0.343 0.440 0.341 0.442 0.441 0.442 0.441 0.442 0.343 0.341 0.342 0.343 0.440 0.341 0.422 0.421 0.422 0.423 0.424 0.402 0.381 0.361 0.267 0.268
184	15.33	0.77	0.547	0.188	(0.331)	0.360
185	15.42	0.77	0.547	0.187	(0.331)	0.361
186	15.50	0.77	0.547	0.186	(0.331)	0.361
187	15.58	0.63	0.452	0.185	(0.273)	0.267

196 197 198 199 200 201 202 203 204 205 206 207 208 209 210 211 212 213 214 215 216 217 218 220 221 222 223 224 225 226 227 228 230 231 232 233 234 235 236 237 238 239 240 242 243 234 242 244 245	16.33 16.42 16.50 16.58 16.75 16.75 16.83 16.92 17.00 17.08 17.17 17.25 17.33 17.42 17.50 17.58 17.67 17.75 17.83 17.92 18.00 18.08 18.17 18.25 18.33 18.42 18.50 18.58 18.75 18.58 18.75 18.83 18.92 19.00 19.08 19.17 19.25 19.33 19.42 19.50 19.58 19.75 19.58 19.75 19.75 19.83 19.92 20.00 20.08 20.17 20.25 20.33 20.42	0.13 0.13 0.13 0.10 0.10 0.10 0.10 0.10 0.10 0.10 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.13 0.10 0.10 0.07 0.00 0.10	0.095 0.095 0.095 0.071 0.071 0.071 0.071 0.071 0.071 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.095 0.071 0	$ \left(\begin{array}{c} 0.177 \\ (0.176 \\ (0.175 \\ (0.175 \\ (0.171 \\ (0.173 \\) \\ (0.172 \\ (0.171 \\ (0.170 \\) \\ (0.170 \\ (0.170 \\ (0.169 \\) \\ (0.167 \\) \\ (0.166 \\) \\ (0.165 \\) \\ (0.165 \\) \\ (0.165 \\) \\ (0.165 \\) \\ (0.165 \\) \\ (0.165 \\) \\ (0.165 \\) \\ (0.165 \\) \\ (0.166 \\) \\ (0.166 \\) \\ (0.166 \\) \\ (0.161 \\) \\ (0.160 \\) \\ (0.162 \\) \\ (0.161 \\) \\ (0.160 \\) \\ (0.158 \\) \\ (0.157 \\) \\ (0.157 \\) \\ (0.157 \\) \\ (0.157 \\) \\ (0.157 \\) \\ (0.157 \\) \\ (0.157 \\) \\ (0.155 \\) \\ (0.157 \\) \\ (0.154 \\) \\ (0.154 \\) \\ (0.155 \\) \\ (0.151 \\) \\ (0.151 \\) \\ (0.151 \\) \\ (0.151 \\) \\ (0.151 \\) \\ (0.151 \\) \\ (0.151 \\) \\ (0.142 \\) \\ (0.144 \\) \\ (0.143 \\) \\ (0.143 \\) \\ (0.142 \\) \\ (0.141 \\) \\ (0.141 \\) \\ (0.141 \\) \\ (0.141 \\) \\ (0.142 \\) \\ (0.142 \\) \\ (0.141 \\) \\ (0.141 \\) \\ (0.142 \\) \\ (0.142 \\) \\ (0.141 \\) \\ (0.141 \\) \\ (0.142 \\) \\ (0.141 \\) \\ (0.141 \\) \\ (0.142 \\) \\ (0.142 \\) \\ (0.142 \\) \\ (0.141 \\) \\ (0.142 \\) \\ (0.141 \\) \\ (0.142 \\) \\ ($	0.058 0.058 0.043 0.043 0.043 0.043 0.043 0.043 0.043 0.043 0.072 0.072 0.072 0.072 0.072 0.072 0.072 0.072 0.072 0.072 0.072 0.072 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.058 0.043 0	0.038 0.038 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.038 0.038 0.038 0.038 0.038 0.038 0.038 0.038 0.028
238	19.83	0.07	0.048	<pre>(0.144)</pre>	0.029	0.019
239	19.92	0.07	0.048	(0.143)	0.029	0.019
240	20.00	0.10	0.048	(0.143)	0.029	0.028
241	20.08	0.10	0.071	(0.142)	0.043	0.028
242	20.17	0.10	0.071	(0.141)	0.043	0.028
243	20.25	0.10	0.071	(0.141)	0.043	0.028

256 257 258 260 261 262 263 264 265 266 267 270 271 272 273 274 275 276 277 278 281 283 284 285 284 285 286 283 284 285 286 283	22.42 22.50 22.58 22.67 22.75 22.83 22.92 23.00 23.08 23.17 23.25 23.33 23.42 23.50 23.58 23.50 23.58 23.67 23.75 23.83 23.92 24.00 Sum =	0.07 0.07 0.10 0.10 0.07	0.048 0.071 0.071 0.048 0.		$\begin{array}{c} 0.130)\\ 0.129)\\ 0.129)\\ 0.128)\\ 0.128)\\ 0.128)\\ 0.128)\\ 0.127)\\ 0.127)\\ 0.127)\\ 0.127)\\ 0.126)\\ 0.126)\\ 0.126)\\ 0.126)\\ 0.125)\\ 0.125)\\ 0.125)\\ 0.125)\\ 0.125)\\ 0.124)\\ 0.124)\\ 0.124)\\ 0.124)\\ 0.123)\\ 0.123)\\ 0.123)\\ 0.123)\\ 0.123)\\ 0.123)\\ 0.123)\\ 0.123)\end{array}$	Sum =		P P <td< th=""></td<>
	Total	s area soil loss =	2.86(ln)		1.1(Ac.F	t)	
		soil loss = rainfall =		Ac.Ft) n)				
		volume = soil loss =	46126.9	Cubic				
	Peak	flow rate of	this hydro	ograph	n = 2.	480(CFS)		
	+++++						+++++++++++++++++++++++++++++++++++++++	 + + +
		R 1	24 – HO 1 n o f f		STOR ydrog			
		Hydrod	graph in .	5 M:	inute inter	vals ((CFS))		
Tim	ne(h+m)	Volume Ac.Ft	Ç(CFS)	0	2.5	5.0	7.5	10.0
)+10)+15)+20)+25	0.0028	0.07 Q 0.08 Q 0.10 Q					

0+35 0+40 0+45 0+50 0+55 1+0 1+5 1+10 1+15 1+20 1+25 1+30 1+35 1+40 1+45 1+50	0.0044 0.0052 0.0060 0.0070 0.0091 0.0100 0.0109 0.0117 0.0125 0.0133 0.0141 0.0149 0.0157 0.0165 0.0175	0.12 0.12 0.14 0.15 0.16 0.14 0.12 0.12 0.12 0.12 0.12 0.12 0.12 0.12	Q Q Q Q Q Q Q Q Q Q			
1+55 2+0 2+5 2+10 2+15 2+20 2+25 2+30 2+35 2+40 2+45 2+50 2+55 3+0 3+15 3+20 3+25 3+30 3+35 3+40 3+45 2+50 3+45 3+45 3+45 3+45 3+45 3+45 3+45 3+45 3+45 3+45 3+45 3+45 3+40 3+55 3+20 3+55 3+20 3+55 3+20 3+25 3+35 3+25 3+35 3+25 3+35 3+25 3+35 3+25 3+35 3+40 3+25 3+35 3+25 3+35 3+40 3+25 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+35 3+40 3+45 3+555 3+555 3+555 3+555 3+5555 3+5555 3+5555 3+555555555555555555555555555	0.0185 0.0196 0.0207 0.0218 0.0228 0.0239 0.0250 0.0261 0.0273 0.0286 0.0300 0.0313 0.0326 0.0340 0.0353 0.0367 0.0380 0.0394 0.0394 0.0407 0.0421 0.0434 0.0447 0.0447 0.0461 0.0476	0.15 0.16 0.16 0.16 0.16 0.16 0.16 0.16 0.16	О О <			
3+50 3+55 4+0 4+5 4+10 4+15 4+20 4+25 4+30 4+35 4+40 4+45 4+50 4+55 5+0 5+5 5+10 5+15 5+20 5+25 5+30	0.0476 0.0492 0.0508 0.0524 0.0556 0.0574 0.0593 0.0611 0.0630 0.0649 0.0668 0.0688 0.0709 0.0731 0.0750 0.0750 0.0782 0.0819 0.0837	0.22 0.23 0.23 0.23 0.23 0.23 0.23 0.26 0.27 0.27 0.27 0.27 0.27 0.29 0.31 0.31 0.21 0.23 0.26 0.27 0.27	QV QV QV QV QV QV QV QV QV QV QV QV QV Q			

5+35 5+40 5+450 5+450 5+450 5+50 6+1150 6+2250 6+405 6+405 6+405 6+405 6+405 6+505 7++1150 7++22305 7++4505 8+1050 8+1050 8+250 8+1050 8+250 8+450 8+250 8+450 8+250 8+450 8+250 8+450 8+450 8+450 8+450 8+450 8+450 8+450 8+450 8+450 8+450 8+450 8+450 8+450 8+450 8+450 8+450 8+450 8+450 8+450 8+550 9+1150 9+120 9+120 9+120 9+450 9+450 8+5000 8+5000 8+5000 8+5000 8+5000 8+5000 8+5000 8+5000 8+50000	0.0858 0.0879 0.0900 0.0922 0.0944 0.0965 0.0988 0.1012 0.1036 0.1060 0.1085 0.1109 0.1135 0.1161 0.1188 0.1215 0.1242 0.1242 0.1242 0.1269 0.1270 0.1323 0.1350 0.1378 0.1407 0.1437 0.1437 0.1468 0.1500 0.1532 0.1566 0.1601 0.1636 0.1674 0.1714 0.1754 0.1794 0.1794 0.1835 0.1875 0.1917 0.1960 0.2003 0.2047 0.2033 0.2047 0.2033 0.2138 0.2187 0.2238 0.2290 0.2345 0.2404 0.2463 0.2527 0.2593 0.2661 0.2732	0.29 0.31 0.31 0.31 0.31 0.33 0.35 0.35 0.35 0.35 0.35 0.35 0.39 0.55 0.62 0.66 0.62 0.66 0.71 0.85 0.86 0.92 0.97 0.98 1.04	$\begin{bmatrix} Q & V & & V & \\ Q & V & V & \\ Q & $		
9+20 9+25 9+30 9+35 9+40	0.2345 0.2404 0.2463 0.2527 0.2593	0.81 0.85 0.86 0.92 0.97	Q V Q V Q V Q V Q V		

$\begin{array}{c} 10+35\\ 10+40\\ 10+45\\ 10+50\\ 10+55\\ 11+0\\ 11+5\\ 11+10\\ 11+15\\ 11+20\\ 11+25\\ 11+30\\ 11+35\\ 11+40\\ 11+45\\ 11+55\\ 12+0\\ 12+55\\ 12+10\\ 12+15\\ 12+20\\ 12+55\\ 12+10\\ 12+15\\ 12+20\\ 12+55\\ 12+30\\ 12+35\\ 12+40\\ 12+45\\ 12+55\\ 12+50\\ 12+55\\ 13+0\\ 13+55\\ 13+10\\ 13+55\\ 13+10\\ 13+55\\ 13+55\\ 13+40\\ 13+55\\ 14+55\\$	0.3196 0.3259 0.324 0.3324 0.3389 0.3454 0.3520 0.3583 0.3643 0.3703 0.3764 0.3824 0.3886 0.3940 0.3989 0.4038 0.4090 0.4146 0.4202 0.4284 0.4386 0.4490 0.4599 0.4711 0.4824 0.4599 0.4711 0.55751 0.5598 0.5751 0.5598 0.5751 0.5918 0.6088 0.6258 0.6258 0.6429 0.6600 0.5751 0.5918 0.6088 0.6258 0.6258 0.6429 0.7026 0.7124 0.7222 0.7335 0.7458 0.77847 0.7947 0.8067 0.8188 0.8309 0.8427 0.8542 0.8542 0.8542	0.77 0.91 0.94 0.95 0.95 0.96 0.91 0.87 0.87 0.87 0.88 0.89 0.71 0.71 0.76 0.81 0.72 1.47 1.52 1.62 1.62 1.64 1.75 1.83 1.95 1.95 1.95 1.95 1.95 1.95 1.95 1.95 1.95 1.95 1.95 1.95 1.96 2.23 2.43 2.47 2.48 1.91 1.47 1.41 1.42 1.42 1.42 1.42 1.42 1.42 1.42 1.42 1.42 1.42 1.42 1.42 1.42 1.42 1.42 1.42 1.42 1.42 1.64 1.95 1.95 1.95 1.96 2.23 2.43 1.91 1.47 1.41 1.42 1.42 1.64 1.75 1.67 1.67 1.67 1.67 1.67 1.67 1.67 1.75		7 7 V V V V V V V V V V V V V	
14+40 14+45 14+50 14+55	0.8188 0.8309 0.8427 0.8542	1.75 1.76 1.71 1.67	Q Q Q Q	 	V V V V

15+35 0.938 $15+40$ 0.946 $15+55$ 0.961 $15+55$ 0.969 $16+0$ 0.977 $16+5$ 0.981 $16+10$ 0.983 $16+10$ 0.983 $16+15$ 0.984 $16+25$ 0.986 $16+30$ 0.987 $16+35$ 0.989 $16+40$ 0.989 $16+50$ 0.990 $16+55$ 0.991 $17+0$ 0.992 $17+5$ 0.993 $17+10$ 0.994 $17+15$ 0.996 $17+20$ 0.997 $17+25$ 0.998 $17+30$ 1.000 $17+35$ 1.001 $17+40$ 1.002 $17+45$ 1.004 $17+55$ 1.006 $18+0$ 1.017 $18+5$ 1.017 $18+5$ 1.017 $18+5$ 1.017 $18+5$ 1.012 $19+10$ 1.023 $19+15$ 1.026 $19+55$ 1.027 $19+45$ 1.026 $19+55$ 1.026 $20+15$ 1.026 $20+15$ 1.037 $20+20$ 1.037 $20+20$ 1.037 $20+5$ 1.037 $20+5$ 1.037 $20+5$ 1.037 $20+15$ 1.037 $20+15$ 1.037 $20+15$ 1.037 $20+20$ 1.037 $20+30$ 1.037 $20+30$ 1.037 $20+5$ 1.037	$\begin{array}{cccccccccccccccccccccccccccccccccccc$			V V V V V V V V
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20+35	1.0335	0.12	Q I			V	1
20+40	1.0343	0.12	Q			V	
20+45	1.0351	0.12	QI			V	
20+50	1.0358	0.10	Q			V	-
20+55	1.0363	0.08	Q I			V	
21+ 0	1.0369	0.08	Q I			V	
21+ 5	1.0375	0.10	Q			V	ł
21+10	1.0383	0.11	Q			V	
21+15	1.0391	0.12	Q		-	V	
21+20	1.0398	0.10	Q I		and a second sec	V	
21+25	1.0403	0.08	Q		1	V	
21+30	1.0409	0.08	QI			V	
21+35	1.0416	0.10	Q I			V	
21+40	1.0424	0.11	QI			V	
21+45	1.0432	0.12	Q			V	
21+50	1.0438	0.10	Q I			V	
21+55 22+ 0	1.0444 1.0449	0.08 0.08	Q I		1	V V	
22+ 0	1.0449	0.08 0.10	Q Q		1 1		1
22+10	1.0450	0.10	Q I			V	
22+10	1.0472	0.11	Q I		1		
22+20	1.0479	0.10	Q I		1		
22+25	1.0484	0.08	Q I				r
22+30	1.0490	0.08	Q I				
22+35	1.0495	0.08	Q				ri -
22+40	1.0500	0.08	Q			V	
22+45	1.0506	0.08	Q			V	7
22+50	1.0511	0.08	Q			V	7
22+55	1.0516	0.08	Q			V	7
23+ 0	1.0522	0.08	Q				7
23+ 5	1.0527	0.08	Q	1			7
23+10	1.0533	0.08	Q				7
23+15	1.0538	0.08	Q				7
23+20 23+25	1.0543 1.0549	0.08	Q	1	1	•	7
23+25	1.0549	0.08 0.08	Q	l	1	•	7 7
23+35	1.0560	0.08	Q Q	1	1		7
23+35	1.0565	0.08	Q	1			7
23+45	1.0570	0.08	Q	1		•	7
23+50	1.0576	0.08	Q	1	1		7
23+55	1.0581	0.08	Q				7
24+ 0	1.0586	0.08	Q		1		7
24+ 5	1.0589	0.04	Q	1			7
24+10	1.0589	0.00	Q		1		V

DEVELOPED 100 YEAR – 24 HOUR AREA D₁

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Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File; c318devD124100.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
 English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
 St. Frances of Rome - Wildomar
Developed D1
100-Year 24-Hour
Drainage Area = 5.05(Ac.) = 0.008 Sg. Mi.
Drainage Area for Depth-Area Areal Adjustment = 5.05(Ac.) = 0.008 Sq.
Length along longest watercourse = 762.50(Ft.)
Length along longest watercourse measured to centroid = 532.10(Ft.)
Length along longest watercourse = 0.144 Mi.
Length along longest watercourse measured to centroid = 0.101 Mi.
Difference in elevation = 28.10(Ft.)
Slope along watercourse = 194.5810 Ft./Mi.
Average Manning's 'N' = 0.020
Lag time = 0.035 Hr.
Lag time = 2.12 Min.
25% of lag time = 0.53 Min.
40% of lag time = 0.85 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
      5.05
               2.23
                                     11.26
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
5.05 5.95 30.05
STORM EVENT (YEAR) = 100.00
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Mi.

Area Averaged 2-Year Rainfall = 2.230(In) Area Averaged 100-Year Rainfall = 5.950(In) Point rain (area averaged) = 5.950(In) Areal adjustment factor = 100.00 % Adjusted average point rain = 5.950(In) Sub-Area Data: Area(Ac.)Runoff IndexImpervious %5.05063.960.534 Total Area Entered = 5.05(Ac.)
 RI
 RI
 Infil. Rate Impervious
 Adj. Infil. Rate Area%
 F

 AMC2
 AMC-2
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 64.0
 64.0
 0.427
 0.534
 0.222
 1.000
 0.222

 Sum
 (E)
 0.22
 1.000
 0.222
 Sum (F) = 0.222Area averaged mean soil loss (F) (In/Hr) = 0.222Minimum soil loss rate ((In/Hr)) = 0.111 (for 24 hour storm duration) Soil low loss rate (decimal) = 0.473 Unit Hydrograph DESERT S-Curve _____ Unit Hydrograph Data Unit time period Time % of lag Distribution Unit Hydrograph Graph % (CFS) (hrs) 2.445 2.190 0.359 0.095 Sum = 100.000 Sum= 5.089

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit	Time	Pattern	Storm Rain		oss rate		Effective
	(Hr.)	Percent	(In/Hr)		Max	Low	(In/Hr)
1	0.08	0.07	0.048	(0.393)	0.023	0.025
2	0.17	0.07	0.048	(0.392)	0.023	0.025
3	0.25	0.07	0.048	(0.390)	0.023	0.025
4	0.33	0.10	0.071	(0.389)	0.034	0.038
5	0.42	0.10	0.071	(0.387)	0.034	0.038
6	0.50	0.10	0.071	(0.386)	0.034	0.038
-7	0.58	0.10	0.071	(0.384)	0.034	0.038
8	0.67	0.10	0.071	(0.383)	0.034	0.038
9	0.75	0.10	0.071	(0.381)	0.034	0.038
10	0.83	0.13	0.095	(0.380)	0.045	0.050
11	0.92	0.13	0.095	(0.378)	0.045	0.050
12	1.00	0.13	0.095	(0.377)	0.045	0.050
13	1.08	0.10	0.071	(0.375)	0.034	0.038
14	1.17	0.10	0.071	(0.374)	0.034	0.038
15	1.25	0.10	0.071	(0.372)	0.034	0.038
16	1.33	0.10	0.071	(0.371)	0.034	0.038

27 28 30 32 33 35 37 39 01 23 44 44 45 55 55 55 55 56 61 23 45 67 89 01 23 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 56 78 90 12 34 55 55 55 55 55 55 55 55 55 55 55 55 55	$\begin{array}{c} 1.42\\ 1.50\\ 1.58\\ 1.67\\ 1.75\\ 1.83\\ 1.92\\ 2.08\\ 2.17\\ 2.25\\ 2.342\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 3.58\\ 3.58\\ 3.58\\ 3.58\\ 3.58\\ 3.58\\ 3.58\\ 3.58\\ 3.67\\ 5.83\\ 2.58\\ 3.67\\ 5.83\\ 2.58\\ 5.67\\ 5.25\\ 5.33\\ 2.58\\ 5.58\\ 5.58\\ 5.58\\ 5.58\\ 5.58\\ 5.58\\ 5.58\\ 5.58\\ 5.68\\ 5.58\\ 5.58\\ 5.68\\ 5.58\\ 5.58\\ 5.68\\ 5.58\\ 5.68\\ 5.58\\ 5.58\\ 5.68\\ 5.58\\ 5.68\\ 5.58\\ 5.68\\ 5.58\\ 5.68\\ 5.58\\ 5.68\\ 5.58\\ 5.68\\ 5.68\\ 5.58\\ 5.68$	0.10 0.10 0.10 0.10 0.10 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.20 0.220 0.20	0.071 0.071 0.071 0.071 0.071 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.119 0.143 0.143 0.143 0.143 0.143 0.143 0.167 0.190 0	$ \left(\begin{array}{c} 0.369 \right) \\ (0.368) \\ (0.366) \\ (0.365) \\ (0.361) \\ (0.362) \\ (0.361) \\ (0.359) \\ (0.359) \\ (0.358) \\ (0.355) \\ (0.355) \\ (0.352) \\ (0.350) \\ (0.349) \\ (0.349) \\ (0.344) \\ (0.346) \\ (0.344) \\ (0.344) \\ (0.344) \\ (0.342) \\ (0.342) \\ (0.342) \\ (0.339) \\ (0.338) \\ (0.332) \\ (0.333) \\ (0.332) \\ (0.332) \\ (0.332) \\ (0.322) \\ (0.324) \\ (0.322) \\ (0.322) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.3$	0.034 0.034 0.034 0.034 0.034 0.034 0.045 0.045 0.045 0.045 0.045 0.045 0.045 0.045 0.045 0.056 0.068 0.068 0.068 0.079 0.079 0.079 0.079 0.079 0.079 0.079 0.079 0.079 0.079 0.079 0.079 0.079 0.079 0.079 0.079 0.090 0	0.038 0.038 0.038 0.038 0.038 0.038 0.050 0.050 0.050 0.050 0.050 0.050 0.050 0.050 0.063 0.075 0

$\begin{array}{cccccccccccccccccccccccccccccccccccc$	0.30 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.37 0.37 0.37 0.37 0.40 0.40 0.40 0.43 0.43 0.43 0.43 0.50 0.50 0.50 0.50 0.50 0.50 0.53 0.57 0.57 0.63 0.63 0.67 0.70 0.73 0.73 0.73 0.50 0.50 0.50 0.57 0.63 0.67 0.73 0.50 0.50 0.50 0.50 0.50 0.57 0.63 0.67 0.73 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.57 0.63 0.67 0.73 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.67	0.214 0.214 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.262 0.262 0.262 0.262 0.262 0.262 0.286 0.309 0.309 0.357 0.357 0.357 0.357 0.357 0.357 0.357 0.357 0.357 0.3405 0.405 0.405 0.405 0.405 0.452 0.476 0.524 0.557 0.357 0.357 0.357 0.357 0.357 0.357 0.357 0.357 0.357 0.357 0.357 0.357 0.476 0.500 0.500 0.500 0.500 0.500 0.500 0.500 0.500 0.500 0.500 0.500 0.500	$ \left(\begin{array}{c} 0.286 \right) \\ (0.285 \right) \\ (0.281 \right) \\ (0.281) \\ (0.281) \\ (0.280) \\ (0.279) \\ (0.279) \\ (0.278) \\ (0.276) \\ (0.275) \\ (0.271) \\ (0.271) \\ (0.270) \\ (0.271) \\ (0.270) \\ (0.269) \\ (0.268) \\ (0.266) \\ (0.265) \\ (0.266) \\ (0.263) \\ (0.261) \\ (0.263) \\ (0.261) \\ (0.263) \\ (0.261) \\ (0.263) \\ (0.261) \\ (0.263) \\ (0.263) \\ (0.263) \\ (0.263) \\ (0.263) \\ (0.263) \\ (0.263) \\ (0.263) \\ (0.263) \\ (0.263) \\ (0.257) \\ (0.255) \\ (0.257) \\ (0.255) \\ (0.257) \\ (0.257) \\ (0.255) \\ (0.257) \\ (0.257) \\ (0.253) \\ (0.252) \\ (0.253) \\ (0.252) \\ (0.251) \\ (0.244) \\ (0.244) \\ (0.244) \\ (0.242) \\ (0.244) \\ (0.242) \\ (0.244) \\ (0.242) \\ (0.244) \\ (0.242) \\ (0.233) \\ (0.237) \\ (0.231) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.223) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.224) \\ (0.223) \\ (0.224) \\ (0.224) \\ (0.224) \\ (0.224) \\ (0.224) \\ (0.224) \\ (0.225) \\ (0.225) \\ (0.225) \\ (0.225) \\ (0.225) \\ (0.224) \\ (0.22$	0.101 0.101 0.113 0.113 0.113 0.113 0.113 0.113 0.113 0.113 0.113 0.113 0.113 0.124 0.124 0.124 0.124 0.125 0.135 0.135 0.146 0.169 0.169 0.169 0.169 0.169 0.169 0.169 0.169 0.169 0.169 0.169 0.169 0.169 0.169 0.180 0.180 0.191 0.191 0.191 0.214 0.214 0.214 0.214 0.225 0.236 0.225 0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25	0.113 0.125 0.125 0.125 0.125 0.125 0.125 0.125 0.125 0.125 0.125 0.125 0.125 0.138 0.138 0.138 0.151 0.163 0.163 0.163 0.163 0.163 0.188 0.188 0.188 0.188 0.188 0.188 0.188 0.201 0.201 0.201 0.201 0.213 0.213 0.238 0.238 0.238 0.238 0.238 0.238 0.225 0.263 0.263 0.263 0.263 0.263 0.287 0.288 0.287 0.288 0.289 0.188 0.188 0.188 0.188 0.188 0.251 0.251 0.251 0.251 0.251 0.252 0.252 0.253 0.254 0.255	
128 10.67	0.67	0.476	(0.225)	0.225	0.251	
129 10.75	0.67	0.476	0.224	(0.225)	0.252	
130 10.83	0.67	0.476	0.223	(0.225)	0.253	

$\begin{array}{c} 137\\ 138\\ 139\\ 140\\ 141\\ 142\\ 143\\ 144\\ 145\\ 146\\ 147\\ 148\\ 152\\ 155\\ 155\\ 155\\ 155\\ 155\\ 166\\ 166\\ 167\\ 168\\ 167\\ 172\\ 177\\ 178\\ 180\\ 181\\ 182\\ 183\\ 186\\ 187\\ 1889\\ 190\\ 191\\ 202\\ 181\\ 182\\ 185\\ 186\\ 187\\ 189\\ 190\\ 191\\ 202\\ 182\\ 185\\ 186\\ 187\\ 1889\\ 190\\ 191\\ 202\\ 180\\ 180\\ 180\\ 180\\ 180\\ 180\\ 180\\ 190\\ 190\\ 190\\ 190\\ 100\\ 100\\ 100\\ 10$	$\begin{array}{c} 11.42\\ 11.50\\ 11.58\\ 11.67\\ 11.75\\ 11.83\\ 11.92\\ 12.00\\ 12.08\\ 12.17\\ 12.25\\ 12.33\\ 12.42\\ 12.50\\ 12.58\\ 12.67\\ 12.75\\ 12.83\\ 12.92\\ 13.00\\ 13.08\\ 13.17\\ 13.25\\ 13.33\\ 13.42\\ 13.50\\ 13.58\\ 13.67\\ 13.75\\ 13.83\\ 13.92\\ 14.00\\ 14.08\\ 14.17\\ 14.25\\ 14.33\\ 14.42\\ 14.50\\ 14.58\\ 14.67\\ 14.75\\ 14.83\\ 14.92\\ 15.00\\ 15.08\\ 15.17\\ 15.58\\ 15.58\\ 15.58\\ 15.58\\ 15.58\\ 15.58\\ 15.58\\ 15.58\\ 15.58\\ 15.58\\ 15.58\\ 15.67\\ 15.67\\ 15.68\\ 15.67\\ 15.68\\ 15.67\\ 15.68\\ 15.67\\ 15.58\\ 15.67\\ 15.68\\ 15.68\\ 15$	0.63 0.63 0.57 0.57 0.57 0.60 0.60 0.60 0.83 0.83 0.83 0.87 0.93 0.93 0.93 0.97 0.97 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.677 0.77 0.77 0.77 0.77 0.77 0.77 0.87 0.87 0.83 0.63	0.452 0.452 0.405 0.405 0.428 0.428 0.428 0.595 0.595 0.595 0.619 0.619 0.666 0.666 0.666 0.666 0.690 0.809 0.809 0.809 0.809 0.809 0.809 0.547 0.547 0.547 0.547 0.547 0.547 0.619 0.619 0.619 0.619 0.619 0.619 0.619 0.619 0.619 0.619 0.595 0.59	$ \left(\begin{array}{c} 0.216 \right) \\ (0.215 \right) \\ (0.213 \right) \\ (0.212 \right) \\ (0.211 \right) \\ (0.210 \right) \\ (0.209) \\ (0.209) \\ (0.208) \\ 0.207 \\ 0.206 \\ 0.205 \\ 0.204 \\ 0.203 \\ 0.202 \\ 0.201 \\ 0.200 \\ 0.199 \\ 0.198 \\ 0.197 \\ 0.196 \\ 0.195 \\ 0.197 \\ 0.196 \\ 0.195 \\ 0.191 \\ 0.190 \\ 0.191 \\ 0.190 \\ 0.189 \\ 0.188 \\ 0.187 \\ 0.188 \\ 0.187 \\ 0.186 \\ 0.185 \\ 0.184 \\ 0.183 \\ 0.182 \\ 0.181 \\ 0.180 \\ 0.177 \\ 0.178 \\ 0.177 \\ 0.176 \\ 0.175 \\ 0.174 \\ 0.173 \\ 0.172 \\ 0.171 \\ 0.170 \\ 0.169 \\ 0.168 \\ 0.165 \\ 0.165 \\ 0.165 \\ 0.165 \\ 0.165 \\ 0.164 \\ 0.163 \\ \end{array} $	0.214 0.214 0.191 0.191 0.203 0.203 0.203 (0.281) (0.281) (0.281) (0.293) (0.293) (0.293) (0.293) (0.315) (0.315) (0.326) (0.326) (0.326) (0.383) (0.383) (0.383) (0.383) (0.259) (0.259) (0.259) (0.259) (0.259) (0.259) (0.259) (0.259) (0.259) (0.259) (0.259) (0.259) (0.259) (0.259) (0.259) (0.293) (0.214) (0.238 0.238 0.213 0.213 0.213 0.213 0.226 0.226 0.226 0.226 0.388 0.390 0.415 0.416 0.417 0.465 0.466 0.467 0.492 0.493 0.494 0.614 0.615 0.616 0.617 0.618 0.358 0.359 0.360 0.361 0.362 0.363 0.459 0.460 0.441 0.420 0.361 0.362 0.363 0.440 0.441 0.442 0.361 0.362 0.363 0.459 0.460 0.441 0.420 0.441 0.422 0.399 0.400 0.401 0.378 0.378 0.379 0.285 0.286 0.287 0.288 0.288 0.288
189	15.75	0.63	0.452	0.165	(0.214)	0.287
190	15.83	0.63	0.452	0.165	(0.214)	0.288
191	15.92	0.63	0.452	0.164	(0.214)	0.288

197	16.42	0.13	0.095	(0.159)	0.045	0.050
198	16.50	0.13	0.095	(0.158)	0.045	0.050
199 200	16.58 16.67	0.10 0.10	0.071 0.071	(0.157) (0.156)	0.034 0.034	0.038 0.038
200	16.75	0.10	0.071	(0.156)	0.034	0.038
202	16.83	0.10	0.071	(0.155)	0.034	0.038
203	16.92	0.10	0.071	(0.154)	0.034	0.038
204 205	17.00 17.08	0.10 0.17	0.071 0.119	(0.153) (0.153)	0.034 0.056	0.038 0.063
206	17.17	0.17	0.119	(0.152)	0.056	0.063
207	17.25	0.17	0.119	(0.151)	0.056	0.063
208 209	17.33 17.42	0.17 0.17	0.119 0.119	(0.150) (0.149)	0.056 0.056	0.063 0.063
210	17.50	0.17	0.119	(0.149)	0.056	0.063
211	17.58	0.17	0.119	(0.148)	0.056	0.063
212 213	17.67	0.17	0.119	(0.147)	0.056	0.063
213 214	17.75 17.83	0.17 0.13	0.119 0.095	(0.147) (0.146)	0.056 0.045	0.063 0.050
215	17.92	0.13	0.095	(0.145)	0.045	0.050
216	18.00	0.13	0.095	(0.144)	0.045	0.050
217 218	18.08 18.17	0.13 0.13	0.095 0.095	(0.144) (0.143)	0.045 0.045	0.050 0.050
219	18.25	0.13	0.095	(0.142)	0.045	0.050
220	18.33	0.13	0.095	(0.142)	0.045	0.050
221 222	18.42 18.50	0.13 0.13	0.095 0.095	(0.141) (0.140)	0.045 0.045	0.050 0.050
223	18.58	0.10	0.071	(0.139)	0.034	0.038
224	18.67	0.10	0.071	(0.139)	0.034	0.038
225 226	18.75 18.83	0.10 0.07	0.071	(0.138)	0.034 0.023	0.038
226	10.03 18.92	0.07	0.048 0.048	(0.137) (0.137)	0.023	0.025 0.025
228	19.00	0.07	0.048	(0.136)	0.023	0.025
229	19.08	0.10	0.071	(0.136)	0.034	0.038
230 231	19.17 19.25	0.10 0.10	0.071 0.071	(0.135) (0.134)	0.034 0.034	0.038 0.038
232	19.33	0.13	0.095	(0.134)	0.045	0.050
233	19.42	0.13	0.095	(0.133)	0.045	0.050
234 235	19.50 19.58	0.13 0.10	0.095 0.071	(0.132) (0.132)	0.045 0.034	0.050 0.038
236	19.67	0.10	0.071	(0.131)	0.034	0.038
237	19.75	0.10	0.071	(0.131)	0.034	0.038
238 239	19.83 19.92	0.07 0.07	0.048 0.048	(0.130) (0.129)	0.023 0.023	0.025 0.025
240	20.00	0.07	0.048	(0.129)	0.023	0.025
241	20.08	0.10	0.071	(0.128)	0.034	0.038
242	20.17	0.10	0.071	(0.128)	0.034	0.038
243 244	20.25 20.33	0.10 0.10	0.071 0.071	(0.127) (0.127)	0.034 0.034	0.038 0.038
245	20.42	0.10	0.071	(0.126)	0.034	0.038
246	20.50	0.10	0.071	(0.126)	0.034	0.038
247 248	20.58 20.67	0.10 0.10	0.071 0.071	(0.125) (0.124)	0.034 0.034	0.038 0.038
249	20.75	0.10	0.071	(0.124)	0.034	0.038
250	20.83	0.07	0.048	(0.123)	0.023	0.025
251 252	20.92 21.00	0.07 0.07	0.048 0.048	(0.123) (0.122)	0.023 0.023	0.025 0.025
253	21.00	0.10	0.048	(0.122)	0.034	0.023
254	21.17	0.10	0.071	(0.122)	0.034	0.038
255 256	21.25	0.10	0.071	(0.121)	0.034	0.038
256	21.33	0.07	0.048	(0.121)	0.023	0.025

time Total Total Total Flood Total Peak	0.10 0.07	ective rainf 5.0(Ac.)/[(I 2.39(In 1.006(Ac 5.95(In) 65244.0 C 43827. this hydrog	.Ft)	In) 1.5(Ac. 150(CFS) ++++++++++	0.025 0.025 0.025 0.025 0.025 0.025 42.7 Ft)	
			H y d r o g Minute inter)	
Time(h+m)	Volume Ac.Ft	Q(CFS) 0	2.5	5.0	7.5	10.0
0+ 5 0+10 0+15 0+20 0+25 0+30 0+35	0.0012 0.0021 0.0032 0.0045 0.0058	0.12 Q 0.13 Q 0.16 Q 0.19 Q 0.19 Q				

			_			
0+40	0.0084	0.19	Q			
0+45	0.0097	0.19	Q			
0+50	0.0113	0.22	Q			
0+55	0.0130	0.25	Q			
1+ 0	0.0147	0.25	VQ			
1+ 5	0.0163	0.22	Q			
1+10	0.0176	0.20	Q			1
1+15	0.0190	0.19	Q			1
1+20	0.0203	0.19	Q			
1+25	0.0216	0.19 '	Q			No. of Contract of
1+30	0.0229	0.19	Q			Internet
1+35	0.0242	0.19	Q			1
1+40	0.0256	0.19	Q			
1+45	0.0269	0.19	Q			
1+50	0.0284	0.22	Q			
1+55	0.0301	0.25	Q		verenet	
2+ 0	0.0319	0.25	VQ			
2+ 5	0.0336	0.26	VQ	1		-
2+10	0.0354	0.26	VQ			
2+15	0.0372	0.26	VQ			
2+20	0.0389	0.26	IQ			
2+25	0.0407	0.26	IQ			
2+30	0.0424	0.26	IQ		1	
2+35	0.0444	0.29	IQ	1		1
2+40	0.0466	0.31	IQ	l		
2+45	0.0488	0.32	IQ	1	1	
2+50	0.0510	0.32	IQ			
2+55	0.0532	0.32	ÍQ	Ì		
3+ 0	0.0554	0.32	ÍQ	1		
3+ 5	0.0576	0.32	ÍQ			
3+10	0.0598	0.32	Q			
3+15	0.0620	0.32	IQ			
3+20	0.0642	0.32	IQ			
3+25	0.0664	0.32	IQ			1
3+30	0.0686	0.32	Q	1		1
3+35	0.0708	0.32	IQ			
3+40	0.0730	0.32	IQ			
3+45	0.0752	0.32	QV			1
3+50	0.0776	0.35	QV			
3+55	0.0802	0.38	QV			
4+ 0	0.0828	0.38	QV			ļ
4+ 5	0.0854	0.38	QV			
4+10	0.0881	0.38	QV			
4+15	0.0907	0.38	QV			
4+20	0.0936	0.41	QV	1		
4+25	0.0966	0.44	QV	1		E.
4+30	0.0997	0.45	QV			
4+35	0.1028	0.45	QV			1
4+40	0.1058	0.45	QV			
4+45	0.1089	0.45	QV	1	1	
4+50	0.1122	0.48	IQV		1	4
4+55	0.1157	0.51	I QV	ł		1
5+ 0	0.1192	0.51	QV	-	l	1
5+ 5	0.1223	0.45	IQ V		l	1
5+10	0.1250	0.39	IQ V	1	1	
5+15	0.1277	0.39	IQ V	1		
5+20	0.1305	0.41	IQ V	l	l	a constantino de la c
5+25	0.1336	0.44	IQ V	l		
5+30	0.1366	0.45	IQ V	N	l	
5+35	0.1399	0.48	IQ V	I	1	And the second se

5+40 5+45 5+50 5+55 6+0 6+10 6+15 6+225 6+30 6+450 6+450 6+450 6+550 7+50 7+105 7+250 7+30 7+350 7+500 7+550 8+100 8+200 8+100 8+200 8+100 8+200 8+100 8+200 8+200 8+100 8+200 8+200 8+100 8+200 8+200 8+200 8+100 8+2	0.1434 0.1469 0.1504 0.1539 0.1575 0.1612 0.1651 0.1691 0.1730 0.1770 0.1809 0.1851 0.1895 0.1939 0.1939 0.1983 0.2027 0.2071 0.2114 0.2158 0.2202 0.2249 0.2249 0.2297 0.2345 0.2395 0.2448 0.2500 0.2555 0.2612 0.2669 0.2731 0.2796 0.2862 0.2928	0.51 0.51 0.51 0.51 0.51 0.57 0.57 0.57 0.61 0.63 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.63 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.95 0.95 0.96	QV QV <td< th=""><th></th></td<>	
8+30 8+35 8+40 8+45 8+50 8+55 9+0 9+5 9+10 9+15 9+25 9+30 9+25 9+30 9+35 9+40 9+45 9+55 10+0 10+5 10+10 10+25 10+20 10+35	0.3060 0.3128 0.3198 0.3268 0.3340 0.3415 0.3490 0.3569 0.3651 0.3735 0.3908 0.3996 0.4086 0.4178 0.4270 0.4367 0.4467 0.4467 0.4568 0.4652 0.4721 0.4788 0.4920 0.4986 0.5062	0.99 1.02 1.05 1.08 1.15 1.20 1.21 1.24 1.27 1.28 1.31 1.34 1.34 1.34 1.40 1.45 1.47 1.22 1.00 0.96 0.96 1.11	$\begin{vmatrix} Q & V \\ Q $	

18+25 1.4224 0.26 Q V 18+30 1.4241 0.26 Q V 18+35 1.4257 0.22 Q V V 18+35 1.4257 0.22 Q V 18+40 1.4270 0.20 Q V 18+45 1.4283 0.19 Q V 18+50 1.4295 0.16 Q V 18+55 1.4304 0.13 Q V 19+0 1.4313 0.13 Q V 19+5 1.4324 0.16 Q V	18+30 1.4241 0.26 Q I V 18+35 1.4257 0.22 Q V V 18+40 1.4270 0.20 Q I V V 18+45 1.4283 0.19 Q I V V 18+50 1.4295 0.16 Q I V V 18+55 1.4304 0.13 Q I V V 19+0 1.4313 0.13 Q I V V	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
	19+101.43360.19Q V19+151.43490.19Q V19+201.43650.22Q V19+251.43820.25Q V19+301.43990.25 Q V19+351.44150.22Q VV	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$

20+40	1.4574	0.19 Q			V
20+45	1.4587	0.19 Q		1	I V I
			l	1	
20+50	1.4598	0.16 Q			V I
20+55	1.4607	0.13 Q			V
21+ 0	1.4616	0.13 Q	1	1	V
21+ 5	1.4627	_	1	1	V
		-	1	I	
21+10	1.4640	0.19 Q	1		V
21+15	1.4653	0.19 Q	1		V
21+20	1.4664	0.16 Q	1		V
21+25	1.4673		1	1	V V
		-	1		
21+30	1.4682	0.13 Q	and the second se	1	I VI
21+35	1.4693	0.16 Q			V I
21+40	1.4706	0.19 Q	1		V
21+45	1.4719	-		1	V
-					
21+50	1.4730	0.16 Q			V
21+55	1.4739	0.13 Q			V
22+ 0	1.4748	0.13 Q	1	1	V V
22+ 5	1.4759	0.16 Q	1		V
22+10	1.4772	-		1	
					V V
22+15	1.4785	0.19 Q			V
22+20	1.4796	0.16 Q			V
22+25	1.4805	0.13 Q		1	I VI
22+30	1.4814	0.13 Q			i Vi
22+35	1.4823	0.13 Q			V V
22+40	1.4832	0.13 Q		l	V V
22+45	1.4840	0.13 Q			V
22+50	1.4849	0.13 Q	1		V
22+55	1,4858	0.13 Q	1		V V
23+ 0	1.4867	0.13 Q	1	1	V V
23+ 5	1.4876	0.13 Q			I VI
23+10	1.4884	0.13 Q	1		I V I
23+15	1.4893	0.13 Q		1	V V
23+20	1.4902	0.13 Q	1	1	V V
23+25	1.4911	0.13 Q	1	1	V
			1	1	
23+30	1.4920	0.13 Q			V
23+35	1.4928	0.13 Q			V
23+40	1.4937	0.13 Q			V V
23+45	1.4946	0.13 Q		1	V
23+50	1.4955	0.13 Q	1	1	V
			1	1	
23+55	1.4964	0.13 Q		ł	V
24+ 0	1.4972	0.13 Q			V V
24+ 5	1.4977	0.07 Q			[V]
24+10	1.4978	0.01 Q	1	1	I VI
24+15	1.4978	0.00 Q	r I	+	V V
24,10	1.10/0	0.00 Y	ŀ	I	1 V

DEVELOPED 100 YEAR – 24 HOUR AREA E₁

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Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318devE124100.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
    English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
St. Frances of Rome - Wildomar
Developed E1
100-Year 24-Hour
Drainage Area = 2.32(Ac.) = 0.004 Sg. Mi.
Drainage Area for Depth-Area Areal Adjustment = 2.32(Ac.) = 0.004 Sq.
Length along longest watercourse = 284.30(Ft.)
Length along longest watercourse measured to centroid = 156.10(Ft.)
Length along longest watercourse = 0.054 Mi.
Length along longest watercourse measured to centroid = 0.030 Mi.
Difference in elevation = 12.70(Ft.)
Slope along watercourse = 235.8635 Ft./Mi.
Average Manning's 'N' = 0.015
Lag time = 0.011 Hr.
Lag time = 0.66 Min.
25% of lag time = 0.17 Min.
40% of lag time = 0.26 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
      2.32
               2.23
                                       5.17
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
2.32 5.95 13.80
                                     13.80
STORM EVENT (YEAR) = 100.00
```

Mi.

Area Averaged 2-Year Rainfall = 2.230(In) Area Averaged 100-Year Rainfall = 5.950(In) Point rain (area averaged) = 5.950(In) Areal adjustment factor = 100.00 % Adjusted average point rain = 5.950(In) Sub-Area Data: Area (Ac.) Runoff Index Impervious % 2.320 72.89 0.772 Total Area Entered = 2.32(Ac.)
 RI
 RI
 Infil. Rate Impervious
 Adj. Infil. Rate Area%
 F

 AMC2
 AMC-2
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 72.9
 72.9
 0.328
 0.772
 0.100
 1.000
 0.100
 Sum (F) = 0.100Area averaged mean soil loss (F) (In/Hr) = 0.100Minimum soil loss rate ((In/Hr)) = 0.050(for 24 hour storm duration) Soil low loss rate (decimal) = 0.282 Unit Hydrograph DESERT S-Curve Unit Hydrograph Data Unit time period Time % of lag Distribution Unit Hydrograph (hrs) Graph % (CFS) 10.083756.12880.4031.88020.1671512.25619.5970.458 $\begin{array}{rcl} 512.256 & 19.597 & 0.430 \\ \text{Sum} = 100.000 & \text{Sum} = & 2.338 \end{array}$

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit	Time	Pattern	Storm Rain	Loss rate		Effective
	(Hr.)	Percent	(In/Hr)	Max	Low	(In/Hr)
1	0.08	0.07	0.048	(0.177)	0.013	0.034
2	0.17	0.07	0.048	(0.177)	0.013	0.034
3	0.25	0.07	0.048	(0.176)	0.013	0.034
4	0.33	0.10	0.071	(0.175)	0.020	0.051
5	0.42	0.10	0.071	(0.175)	0.020	0.051
6	0.50	0.10	0.071	(0.174)	0.020	0.051
7	0.58	0.10	0.071	(0.173)	0.020	0.051
8	0.67	0.10	0.071	(0.173)	0.020	0.051
9	0.75	0.10	0.071	(0.172)	0.020	0.051
10	0.83	0.13	0.095	(0.171)	0.027	0.068
11	0.92	0.13	0.095	(0.171)	0.027	0.068
12	1.00	0.13	0.095	(0.170)	0.027	0.068
13	1.08	0.10	0.071	(0.169)	0.020	0.051
14	1.17	0.10	0.071	(0.169)	0.020	0.051
15	1.25	0.10	0.071	(0.168)	0.020	0.051
16	1.33	0.10	0.071	(0.167)	0.020	0.051
17	1.42	0.10	0.071	(0.167)	0.020	0.051
18	1.50	0.10	0.071	(0.166)	0.020	0.051

19 20 22 23 24 26 78 90 12 33 35 37 89 01 23 44 44 44 45 67 89 01 23 45 67 89 01 23 45 66 78 90 12 34 55 55 55 55 55 55 55 55 55 55 55 55 55	$\begin{array}{c} 1.58\\ 1.67\\ 1.75\\ 1.83\\ 2.008\\ 2.25\\ 2.34\\ 2.58$	0.10 0.10 0.10 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.20 0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.27	0.071 0.071 0.071 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.119 0.143 0.143 0.143 0.143 0.143 0.143 0.167 0.167 0.167 0.167 0.167 0.167 0.167 0.167 0.167 0.190 0.19	(0.165) (0.163) (0.163) (0.163) (0.162) (0.161) (0.161) (0.160) (0.159) (0.159) (0.159) (0.159) (0.157) (0.157) (0.157) (0.157) (0.157) (0.155) (0.155) (0.154) (0.154) (0.154) (0.152) (0.152) (0.152) (0.151) (0.150) (0.150) (0.150) (0.144) (0.147) (0.147) (0.147) (0.144) (0.142) (0.142) (0.141) (0.142) (0.139) (0.137) (0.137) (0.133) (0.132) (0.132) (0.132)	0.020 0.020 0.027 0.027 0.027 0.027 0.027 0.027 0.027 0.027 0.027 0.027 0.027 0.034 0.040 0.040 0.040 0.040 0.040 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.054 0	0.051 0.051 0.051 0.068 0.068 0.068 0.068 0.068 0.068 0.068 0.068 0.068 0.085 0.0103 0.120 0.120 0.120 0.137
69 70 71 72	5.75 5.83 5.92 6.00	0.27 0.27 0.27 0.27	0.190 0.190 0.190 0.190 0.190	(0.134) (0.133) (0.133) (0.132)	0.054 0.054 0.054 0.054	0.137 0.137 0.137 0.137

$\begin{array}{c} 139\\ 140\\ 141\\ 142\\ 143\\ 144\\ 145\\ 146\\ 147\\ 148\\ 149\\ 150\\ 151\\ 155\\ 156\\ 157\\ 158\\ 160\\ 162\\ 163\\ 166\\ 167\\ 168\\ 9\\ 171\\ 177\\ 178\\ 179\\ 180\\ 181\\ 182\\ 183\\ 185\\ 186\\ 188\\ 189\end{array}$	$\begin{array}{c} 11.58\\ 11.67\\ 11.75\\ 11.83\\ 11.92\\ 12.00\\ 12.08\\ 12.17\\ 12.25\\ 12.33\\ 12.42\\ 12.50\\ 12.58\\ 12.67\\ 12.75\\ 12.83\\ 12.92\\ 13.00\\ 13.08\\ 13.17\\ 13.25\\ 13.33\\ 12.92\\ 13.00\\ 13.08\\ 13.17\\ 13.25\\ 13.33\\ 13.42\\ 13.50\\ 13.58\\ 13.67\\ 13.75\\ 13.83\\ 13.92\\ 14.00\\ 14.08\\ 14.17\\ 14.25\\ 13.83\\ 13.92\\ 14.00\\ 14.08\\ 14.17\\ 14.25\\ 13.83\\ 13.92\\ 14.00\\ 14.58\\ 14.67\\ 14.58\\ 14.50\\ 14.58\\ 14.67\\ 14.58\\ 14.67\\ 14.58\\ 14.50\\ 14.58\\ 14.67\\ 15.25\\ 15.33\\ 15.42\\ 15.50\\ 15.58\\ 15.57\\ 15.58\\ 15.67\\ 15.75\\ \end{array}$	0.57 0.57 0.57 0.60 0.60 0.60 0.83 0.83 0.83 0.87 0.93 0.93 0.93 0.97 0.97 0.97 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.63 0.87 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.77 0.77 0.77 0.77 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.83 0.83 0.83 0.83 0.80 0.80 0.77 0.77 0.77 0.77 0.77 0.90 0.97 0.77 0.90 0.83 0.83 0.83 0.80 0.77	0.405 0.405 0.405 0.428 0.428 0.428 0.595 0.595 0.595 0.619 0.619 0.666 0.666 0.666 0.690 0.809 0.809 0.809 0.809 0.809 0.809 0.809 0.547 0.547 0.547 0.547 0.547 0.547 0.547 0.547 0.547 0.643 0.643 0.619 0.619 0.619 0.619 0.619 0.619 0.619 0.619 0.619 0.595 0.59	0.096 0.095 0.095 0.094 0.094 0.093 0.092 0.092 0.092 0.092 0.091 0.091 0.090 0.090 0.089 0.089 0.089 0.087 0.087 0.087 0.087 0.087 0.085 0.085 0.085 0.085 0.084 0.084 0.084 0.084 0.084 0.083 0.082 0.081 0.081 0.081 0.081 0.081 0.081 0.081 0.081 0.081 0.081 0.081 0.081 0.081 0.081 0.081 0.081 0.081 0.081 0.079 0.079 0.079 0.079 0.077 0.077 0.077 0.075 0	$ \left(\begin{array}{c} 0.114 \\ (0.114 \\ (0.121 \\ (0.121) \\ (0.121) \\ (0.121) \\ (0.121 \\ (0.121) \\ (0.121) \\ (0.168 \\ (0.168) \\ (0.168 \\ (0.175) \\ (0.175) \\ (0.175 \\ (0.175 \\ (0.175 \\ (0.175 \\ (0.188 \\) \\ (0.188 \\ (0.188 \\) \\ (0.188 \\ (0.195 \\ (0.195 \\ (0.195 \\ (0.195 \\ (0.228 \\ (0.154 \\ (0.154 \\ (0.175 \\ (0.175 \\ (0.175 \\ (0.175 \\ (0.175 \\ (0.175 \\ (0.175 \\ (0.175 \\ (0.175 \\ (0.175 \\ (0.168 \\ (0.161 \\ (0.161 \\ (0.161 \\ (0.161 \\ (0.154 \\ (0.128 \\ (0$	0.308 0.309 0.309 0.334 0.334 0.334 0.335 0.502 0.502 0.502 0.503 0.527 0.527 0.528 0.576 0.576 0.577 0.601 0.601 0.602 0.721 0.722 0.722 0.723 0.723 0.723 0.462 0.463 0.463 0.463 0.463 0.463 0.463 0.464 0.560 0.561 0.537 0.538 0.538 0.539 0.539 0.539 0.5316 0.516 0.517 0.493 0.494 0.494 0.471 0.472 0.377 0.378
183	15.25	0.80	0.571	0.077	<pre>(0.161) (0.154) (0.154) (0.154) (0.128) (0.128)</pre>	0.494
184	15.33	0.77	0.547	0.077		0.471
185	15.42	0.77	0.547	0.076		0.471
186	15.50	0.77	0.547	0.076		0.472
187	15.58	0.63	0.547	0.075		0.377
188	15.67	0.63	0.452	0.075		0.377

199 200 201 202 203	16.58 16.67 16.75 16.83 16.92	0.10 0.10 0.10 0.10 0.10 0.10	0.071 0.071 0.071 0.071 0.071 0.071	(0.071) (0.071) (0.070) (0.070) (0.069)	0.020 0.020 0.020 0.020 0.020 0.020	0.051 0.051 0.051 0.051 0.051
203 204 205 206 207 208	17.00 17.08 17.17 17.25 17.33	0.10 0.17 0.17 0.17 0.17	0.071 0.119 0.119 0.119 0.119 0.119	(0.069) (0.069) (0.068) (0.068) (0.068) (0.068)	0.020 0.034 0.034 0.034 0.034 0.034	0.051 0.085 0.085 0.085 0.085 0.085
209 210 211 212 213	17.42 17.50 17.58 17.67 17.75	0.17 0.17 0.17 0.17 0.17	0.119 0.119 0.119 0.119 0.119 0.119	(0.067) (0.067) (0.067) (0.066) (0.066)	0.034 0.034 0.034 0.034 0.034	0.085 0.085 0.085 0.085 0.085 0.085
214 215 216 217 218	17.83 17.92 18.00 18.08 18.17	0.13 0.13 0.13 0.13 0.13 0.13	0.095 0.095 0.095 0.095 0.095	(0.066) (0.065) (0.065) (0.065) (0.064)	0.027 0.027 0.027 0.027 0.027	0.068 0.068 0.068 0.068 0.068
219 220 221 222 223 224	18.25 18.33 18.42 18.50 18.58 18.67	0.13 0.13 0.13 0.13 0.10 0.10	0.095 0.095 0.095 0.095 0.095 0.071 0.071	(0.064) (0.064) (0.064) (0.063) (0.063) (0.063)	0.027 0.027 0.027 0.027 0.020 0.020	0.068 0.068 0.068 0.068 0.051 0.051
225 226 227 228 229	18.75 18.83 18.92 19.00 19.08	0.10 0.07 0.07 0.07 0.07 0.10	0.071 0.048 0.048 0.048 0.048 0.071	(0.062) (0.062) (0.062) (0.061) (0.061)	0.020 0.013 0.013 0.013 0.013 0.020	0.051 0.034 0.034 0.034 0.034
230 231 232 233 234	19.17 19.25 19.33 19.42 19.50	0.10 0.10 0.13 0.13 0.13	0.071 0.071 0.095 0.095 0.095	(0.061) (0.061) (0.060) (0.060) (0.060)	0.020 0.020 0.027 0.027 0.027	0.051 0.051 0.068 0.068 0.068
235 236 237 238 239	19.58 19.67 19.75 19.83 19.92	0.10 0.10 0.10 0.07 0.07	0.071 0.071 0.071 0.048 0.048	(0.059) (0.059) (0.059) (0.059) (0.059) (0.058)	0.020 0.020 0.020 0.013 0.013	0.051 0.051 0.051 0.034 0.034
240 241 242 243 244 245	20.00 20.08 20.17 20.25 20.33 20.42	0.07 0.10 0.10 0.10 0.10 0.10 0.10	0.048 0.071 0.071 0.071 0.071 0.071	(0.058) (0.058) (0.058) (0.057) (0.057) (0.057)	0.013 0.020 0.020 0.020 0.020 0.020 0.020	0.034 0.051 0.051 0.051 0.051 0.051
246 247 248 249 250	20.50 20.58 20.67 20.75 20.83	0.10 0.10 0.10 0.10 0.10 0.07	0.071 0.071 0.071 0.071 0.071 0.048	(0.057) (0.056) (0.056) (0.056) (0.056) (0.056)	0.020 0.020 0.020 0.020 0.020 0.013	0.051 0.051 0.051 0.051 0.051 0.034
251 252 253 254 255	20.92 21.00 21.08 21.17 21.25	0.07 0.07 0.10 0.10 0.10	0.048 0.048 0.071 0.071 0.071	(0.055) (0.055) (0.055) (0.055) (0.055)	0.013 0.013 0.020 0.020 0.020	0.034 0.034 0.051 0.051 0.051
256 257 258	21.33 21.42 21.50	0.07 0.07 0.07	0.048 0.048 0.048	(0.054) (0.054) (0.054)	0.013 0.013 0.013	0.034 0.034 0.034

259 21.5 260 21.6 261 21.7 262 21.8 263 21.9 264 22.0 265 22.0 266 22.1 267 22.2 268 22.3 269 22.4 270 22.5 271 22.5 272 22.6 273 22.7 274 22.8 275 22.9 276 23.0 277 23.0 277 23.0 278 23.1 279 23.2 280 23.3 281 23.4 282 23.5 283 23.5 284 23.6 285 23.7 286 23.8 287 23.9 288 24.0 Sum =	7 0.10 5 0.10 3 0.07 2 0.07 0 0.07 8 0.10 7 0.10 5 0.10 3 0.07 2 0.07 0 0.07 8 0.07 7 0.07 5 0.07 3 0.07 2 0.07 0 0.07 8 0.07 7 0.07 5 0.07 3 0.07 2 0.07 0 0.07 8 0.07 7 0.07 5 0.07 3 0.07 2 0.07 3 0.07 3 0.07 2 0.07 3 0.07 2 0.07 3 0.07 3 0.07 2 0.07 3 0.07 3 0.07 2 0.07 3 0.07 3 0.07 2 0.07 3 0.07		(0.054) (0.053) (0.053) (0.053) (0.053) (0.053) (0.052) (0.052) (0.052) (0.052) (0.052) (0.052) (0.052) (0.052) (0.052) (0.052) (0.051) (0.051) (0.051) (0.051) (0.051) (0.051) (0.051) (0.051) (0.051) (0.051) (0.051) (0.051) (0.051) (0.051) (0.051) (0.051) (0.050) (0.0	0.020 0.020 0.020 0.013 0.013 0.013 0.013 0.020 0.020 0.020 0.020 0.013	0.051 0.051 0.034 0.034 0.034 0.051 0.051 0.051 0.034	
tim Tota Tota Tota Floo		2.3(Ac.)/[1.25(2 0.241(7 5.95(Ir 39593.3 1051	Ac.Ft) n) Cubic Feet 5.0 Cubic Feet		't)	
 Pea	k flow rate of	this hydro	ograph = 1.	692(CFS)	·	
++++		24 - H O	++++++++++++++++++++++++++++++++++++++	М	+++++++++++++++++++++++++++++++++++++++	++
	Hydrog	raph in	5 Minute inter	vals ((CFS))		
Time(h+m) Volume Ac.Ft	Q(CFS)	0 2.5	5.0	7.5	10.
0+10 0+15 0+20 0+25	0.0004 0.0010 0.0015 0.0023 0.0031 0.0040 0.0048 0.0056 0.0064					

5+50 5+50 6+10 6+10 6+20 6+30 6+450 6+450 6+450 6+450 6+550 7+10 7+20 7+20 7+30 7+45 7+30 7+45 7+55 8+10 8+2250 8+10 8+2250 8+450 8+100 8+100 8+100 8+100 8+100 8+100 8+100 8+100 8+100 8+100 8+	0.0951 0.0973 0.0995 0.1019 0.1044 0.1069 0.1094 0.1118 0.1143 0.1170 0.1225 0.1253 0.1280 0.1308 0.1335 0.1363 0.1390 0.1420 0.14481 0.1513 0.1546 0.1579 0.1615 0.1650 0.1686 0.1726 0.1768 0.1892 0.1892 0.1850 0.1892 0.1933 0.1976 0.2020 0.2064 0.2111 0.2158 0.2205 0.2258 0.2258 0.2258 0.2258 0.2267 0.2647 0.2647 0.2670 0.2734 0.2800 0.2867 0.2935 0.2935 0.2931 0.3064 0.3146 0.3187 0.3244 0.3304 0.3365	0.32 0.32 0.32 0.32 0.35 0.36 0.36 0.36 0.36 0.40 0.60 0.60 0.60 0.92 0.92 0.92 0.92 0.98 0.60 0.60 0.60 0.60 0.60 0.60 0.60 0.60 0.85 0.80 0.92 0.92 0.92 0.98 0.60 0.60 0.60 0.60 0.60 0.60 0.60 0.82 0.88 0.88	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	7 7			
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$\begin{array}{c} 10+50\\ 10+55\\ 11+0\\ 11+5\\ 11+10\\ 11+15\\ 11+20\\ 11+25\\ 11+30\\ 11+35\\ 11+40\\ 11+45\\ 11+50\\ 11+55\\ 12+0\\ 12+55\\ 12+10\\ 12+25\\ 12+10\\ 12+25\\ 12+20\\ 12+55\\ 12+20\\ 12+55\\ 12+40\\ 12+55\\ 12+30\\ 12+55\\ 12+30\\ 12+55\\ 13+10\\ 13+55\\ 13+10\\ 13+55\\ 13+20\\ 13+55\\ 13+10\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+40\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+40\\ 13+55\\ 13+50\\ 13+55\\ 13+40\\ 14+55\\ 13+50\\ 13+55\\ 14+0\\ 14+55\\ 13+50\\ 15+5\\ 15+10\\ 15+5\\ 15+10\\ 15+5\\ 15+10\\ 15+5\\ 15+10\\ 15+35\\ 15+40\\ 15+45\\ 15+$	0.88 0.88 0.88 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.74 0.72 0.73 1.10 1.17 1.18 1.22 1.23 1.32 1.35 1.35 1.35 1.35 1.69 1.20 1.31 1.27 1.31 1.27 1.31 1.21 1.17 1.16 1.26 1.26 1.22 1.21 1.17 1.16 1.10 0.92 0.88			
		0.88 0.84 0.83 0.83 0.83 0.83 0.74 0.72 0.72 0.72 0.77 0.78 0.78 1.10 1.17 1.18 1.22 1.23 1.35 1.35 1.35 1.35 1.35 1.35 1.69 1.69 1.69 1.69 1.69 1.69 1.69 1.69 1.69 1.69 1.69 1.69 1.69 1.69 1.69 1.20 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.20 1.21 1.31 1.27 1.26 1.26 1.26 1.22 1.21 1.17 1.16 1.121 1.17 1.16 1.20 1.10 1.10 1.0	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{c c c c c c c c c c c c c c c c c c c $

$\begin{array}{l} 15+50\\ 15+50\\ 16+&5\\ 16+&0\\ 16+&5\\ 16+10\\ 16+25\\ 16+20\\ 16+25\\ 16+30\\ 16+35\\ 16+40\\ 16+45\\ 16+50\\ 16+55\\ 17+&0\\ 17+&5\\ 17+10\\ 17+55\\ 17+20\\ 17+25\\ 17+30\\ 17+55\\ 17+30\\ 17+55\\ 17+30\\ 17+55\\ 17+30\\ 17+55\\ 17+35\\ 17+40\\ 17+55\\ 18+10\\ 18+55\\ 18+55\\ 18+55\\ 18+45\\ 18+55\\ 18+55\\ 18+45\\ 18+55\\ 18+45\\ 18+55\\ 18+55\\ 19+&0\\ 19+55\\ 19+30\\ 19+35\\ 19+40\\ 19+45\\ 19+55\\ 20+&0\\ 20+35\\ 20+45\\ 20+45\\ 20+45\\ 19+55\\ 20+45\\ 20+45\\ 20+45\\ 19+55\\ 20+45\\ 19+55\\ 20+45\\ 20+45\\ 19+55\\ 20+45\\ 20+45\\ 19+55\\ 20+45\\ 20+45\\ 19+55\\ 20+45\\ 20+45\\ 19+55\\ 20+45\\ 20$	0.8163 0.8224 0.8285 0.8305 0.8316 0.8327 0.8338 0.8349 0.8360 0.8360 0.8369 0.8377 0.8386 0.8394 0.8402 0.8402 0.8410 0.8423 0.8451 0.8451 0.8451 0.84520 0.8520 0.8520 0.8556 0.8556 0.8557 0.8578 0.8589 0.8600 0.8578 0.8589 0.8600 0.8611 0.8622 0.8633 0.8642 0.8650 0.8651 0.8642 0.8650 0.8642 0.8651 0.8641 0.8622 0.8633 0.8642 0.8651 0.8651 0.8710 0.8710 0.8721 0.8741 0.8749 0.8741 0.8749 0.8741 0.8749 0.8741 0.8749 0.8741 0.8749 0.8741 0.8749 0.8741 0.8749 0.8741 0.8749 0.8741 0.8749 0.8741 0.8742 0.8742 0.8840 0.8848	$\begin{array}{cccccccccccccccccccccccccccccccccccc$			$\begin{array}{c} v \\ v $
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$\begin{array}{c} 20+50\\ 20+55\\ 21+0\\ 21+5\\ 21+10\\ 21+15\\ 21+20\\ 21+25\\ 21+30\\ 21+35\\ 21+40\\ 21+45\\ 21+50\\ 21+55\\ 22+0\\ 22+55\\ 22+10\\ 22+55\\ 22+20\\ 22+25\\ 22+20\\ 22+25\\ 22+30\\ 22+25\\ 22+30\\ 22+35\\ 22+40\\ 22+45\\ 22+55\\ 23+0\\ 23+5\\ 23+10\\ 23+5\\ 23+10\\ 23+5\\ 23+20\\ 23+25\\ 23+30\\ 23+25\\ 23+30\\ 23+35\\ 23+40\\ 23+45\\ 23+50\\ 23+55\\ 23+40\\ 23+55\\ 23+50\\ 23+55\\ 23+50\\ 23+55\\ 24+0\\ 24+5\\ 3+50\\ 23+55\\ 24+0\\ 24+5\\ 3+50\\ 3+50\\ 3+50\\ 3+55\\ 3+55\\ 3+50\\ 3+55\\ 3+$	0.8854 0.8860 0.8865 0.8873 0.88973 0.8890 0.8890 0.8901 0.8907 0.8914 0.8923 0.8931 0.8937 0.8942 0.8942 0.8948 0.8956 0.8964 0.8972 0.8978 0.8978 0.8984 0.8995 0.9000 0.9006 0.9011 0.9017 0.9022 0.9028 0.9033 0.9033 0.9039 0.9044 0.9055 0.9061 0.9072 0.9077 0.9083 0.9089	0.09 0.08 0.08 0.11 0.12 0.09 0.08 0.08 0.11 0.12 0.12 0.09 0.08 0.08 0.08 0.08 0.08 0.08 0.08	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~				V V V V V V V V V V	
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OFFSITE

2 YEAR – 1 HOUR OFFSITE F₁

Unit Hydrograph Analysis Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1 Study date 07/22/19 File: c318offsiteF112.out Riverside County Synthetic Unit Hydrology Method RCFC & WCD Manual date - April 1978 Program License Serial Number 6222 English (in-lb) Input Units Used English Rainfall Data (Inches) Input Values Used English Units used in output format St. Frances of Rome - Wildomar Offsite (F1) 2-Year 1-Hour _____ Drainage Area = 274.15(Ac.) = 0.428 Sg. Mi. Drainage Area for Depth-Area Areal Adjustment = 274.15(Ac.) = 0.428 Sg. Length along longest watercourse = 8033.90(Ft.) Length along longest watercourse measured to centroid = 4835.60(Ft.) Length along longest watercourse = 1.522 Mi. Length along longest watercourse measured to centroid = 0.916 Mi. Difference in elevation = 864.00(Ft.) Slope along watercourse = 567.8338 Ft./Mi. Average Manning's 'N' = 0.040 Lag time = 0.326 Hr. Lag time = 19.58 Min. 25% of lag time = 4.90 Min. 40% of lag time = 7.83 Min. Unit time = 5.00 Min. Duration of storm = 1 Hour(s)User Entered Base Flow = 0.00(CFS) 2 YEAR Area rainfall data: Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2] 274.15 0.47 128.30 100 YEAR Area rainfall data: Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2] 274.15 1.30 356.39 STORM EVENT (YEAR) = 2.00

Area Averaged 2-Year Rainfall = 0.468(In) Area Averaged 100-Year Rainfall = 1.300(In) Point rain (area averaged) = 0.468(In) Areal adjustment factor = 99.75 % Adjusted average point rain = 0.467(In) Sub-Area Data: Area(Ac.)Runoff IndexImpervious %274.15082.550.169 Total Area Entered = 274.15(Ac.) RI RI Infil. Rate Impervious Adj. Infil. Rate Area% F
 AMC2
 AMC-1
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 82.5
 66.6
 0.399
 0.169
 0.338
 1.000
 0.338
 Sum (F) = 0.338Area averaged mean soil loss (F) (In/Hr) = 0.338Minimum soil loss rate ((In/Hr)) = 0.169(for 24 hour storm duration) Soil low loss rate (decimal) = 0.765Slope of intensity-duration curve for a 1 hour storm =0.4800 Unit Hydrograph DESERT S-Curve _____ Unit Hydrograph Data Unit time period Time % of lag Distribution Unit Hydrograph (hrs) Graph % (CFS) _____ 1.272 3.514 1.133 3.131 0.925 2.555 0.854 2.359 0.704 1.944 0.646 1.785 1.366 0.494 0.438 1.210 0.285 0.788 0.262 0.723 0.300 0.829 0.306 0.847 0.306 0.847

28	2.333	714.901	0.281		0.777	
29	2.417	740.434	0.172		0.476	
30	2.500	765.966	0.160		0.441	
31	2.583	791.498	0.168		0.463	
			Sum = 100.000	Sum=	276.292	

(Hr.) 1 0.08 2 0.17 3 0.25 4 0.33 5 0.42 6 0.50 7 0.58 8 0.67 9 0.75 10 0.83 11 0.92 12 1.00 Sum = Flood times Total Total Flood Total Flood Total Peak	<pre>6.40 7.90 9.10 12.80 25.60 7.90 4.90 (Loss Rate N 100.0 volume = Effe s area 274 soil loss = soil loss = rainfall = volume = soil loss = flow rate of ++++++++++++</pre>	<pre>In/Hr) 0.246 0.252 0.303 0.303 0.319 0.359 0.443 0.510 0.717 1.434 0.443 0.275 ot Used) ctive rainf .1(Ac.)/[(I</pre>	<pre>Max (0.338) (0.338) (0.338) (0.338) (0.338) (0.338) (0.338) (0.338) 0.338 0.338 0.338 (0.338) all 0.19 all 0.19 .Ft) ubic Feet 3 Cubic Feet araph = 99 </pre>	Low 0.189 0.193 0.231 0.244 0.274 (0.339) (0.390) (0.549) (1.097) (0.339) 0.210 Sum = 9(In) 4.5(Ac 5.798(CFS) ++++++++++ M	(In/Hr) 0.0 0.0 0.0 0.0 0.1 0.1 0.1 0.3 1.0 0.1 0.2 2.3 .Ft)	58 59 71 75 84 04 71 379 996 04 965
	Hydrogr	aph in 5	Minute int	ervals ((CFS		
Time(h+m)	Volume Ac.Ft	Q(CFS) C	25.0	50.0	75.0	100.0
0+ 5 0+10 0+15	0.0015 0.0091 0.0326 0.0817 0.1506 0.2350 0.3324 0.4450	0.22 Q 1.10 Q 3.41 VQ 7.13 V Q 10.00 V 12.26 V	 			

1+15	3.1734	46.81			QI	VI		
1+20	3.4047	33.59		l Q	1	V	.	
1+25	3.5796	25.39		Q	1		V	
1+30	3.7175	20.02	1	QI	l		V	
1+35	3.8295	16.26	Q				V	
1+40	3.9242	13.75	I Q				V	
1+45	4.0035	11.51	I Q		1		V (
1+50	4.0681	9.39	ΙQ				V I	
1+55	4.1237	8.07	I Q	1			VI	
2+ 0	4.1720	7.01	ΙQ				V	
2+ 5	4.2141	6.12	I Q				V	
2+10	4.2498	5.18	ΙQ	Readoute			V I	
2+15	4.2816	4.61	IQ		1		V I	
2+20	4.3085	3.91	IQ		*****	I	V I	
2+25	4.3322	3.44	IQ				V I	
2+30	4.3513	2.78	I Q			-	VI	
2+35	4.3677	2.37	Q	******			V	
2+40	4.3801	1.81	Q	l			V	
2+45	4.3916	1.67	Q	allow the second se			V	
2+50	4.4032	1.69	Q	1	l	l	V	
2+55	4.4145	1.64	Q		le contra de la contra de		V	
3+ 0	4.4250	1.53	Q				V	
3+ 5	4.4340	1.30	Q				V	
3+10	4.4402	0.90	Q				V	
3+15	4.4454	0.76	Q		l		V	
3+20	4.4494	0.58	Q				V	ļ
3+25	4.4500	0.08	Q				V	1
3+30	4.4502	0.03	Q		1		V	l

2 YEAR – 24 HOUR OFFSITE F₁

```
Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/22/19 File: c318offsiteF1242.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
_____
English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
St. Frances of Rome - Wildomar
Offsite (F1)
2-Year 24-Hour
_____
Drainage Area = 274.15(Ac.) = 0.428 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 274.15(Ac.) = 0.428 Sq.
Length along longest watercourse = 8033.90(Ft.)
Length along longest watercourse measured to centroid = 4835.60(Ft.)
Length along longest watercourse = 1.522 Mi.
Length along longest watercourse measured to centroid = 0.916 Mi.
Difference in elevation = 864.00(Ft.)
Slope along watercourse = 567.8338 Ft./Mi.
Average Manning's 'N' = 0.040
Lag time = 0.326 Hr.
Lag time = 19.58 Min.
25% of lag time = 4.90 Min.
40% of lag time = 7.83 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
Area(Ac.)[1]
             Rainfall(In)[2] Weighting[1*2]
                                  611.35
     274.15
                2.23
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
     274.15
               5.95
                                1631.19
STORM EVENT (YEAR) = 2.00
```

Area Averaged 2-Year Rainfall = 2.230(In) Area Averaged 100-Year Rainfall = 5.950(In) Point rain (area averaged) = 2.230(In) Areal adjustment factor = 99.95 % Adjusted average point rain = 2.229(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 274.150 82.55 0.169 Total Area Entered = 274.15(Ac.) RI Infil. Rate Impervious Adj. Infil. Rate Area% F RI AMC2 AMC-1(In/Hr)(Dec.%)(In/Hr)(Dec.)(In/Hr)82.566.60.3990.1690.3381.0000.338 Sum (F) = 0.338Area averaged mean soil loss (F) (In/Hr) = 0.338Minimum soil loss rate ((In/Hr)) = 0.169 (for 24 hour storm duration) Soil low loss rate (decimal) = 0.765Unit Hydrograph DESERT S-Curve _____ Unit Hydrograph Data Unit time period Time % of lag Distribution _ Unit Hydrograph (hrs)Graph %10.08325.5321.39020.16751.0645.43230.25076.59714.01640.333102.12921.84150.417127.66114.44560.500153.1938.75470.583178.7256.24180.667204.2584.72890.750229.7903.691100.833255.3222.953110.917280.8542.536121.000306.3862.148131.08331.9191.672141.167357.4511.448151.250382.9831.272161.333408.5151.133171.417434.0470.925181.500459.5800.854191.583485.1120.704201.667510.6440.646211.750536.1760.494221.833561.7080.438231.917587.2400.285242.000612.7730.262252.083638.3050.300262.167663.8370.306272.250689.3690.306282.333714.9010.281292.417740.434</td (hrs) Graph % (CFS) 3.840 15.008 38.724 60.345 39.909 24.186 17.242 13.064 10.197 8.159 7.008 5.935 4.619 4.001 3.514 3.131 2.555 2.359 1.944 1.785 1.366 1.210 0.788 0.723 0.829 0.847 0.847 0.777 0.476

30	2.500	765.966	0.160		0.441
31	2.583	791.498	0.168		0.463
		ç	Sum = 100.000	Sum=	276.292

Unit I (Pattern Percent	Storm Rain (In/Hr)	Loss rate(In./ Max Low		Effective (In/Hr)
	0.08	0.07	0.018	(0.600)	0.014	0.004
).17	0.07	0.018	(0.597)	0.014	0.004
).25	0.07	0.018	(0.595)	0.014	0.004
).33	0.10	0.027	(0.593)	0.020	0.006 0.006
).42).50	0.10 0.10	0.027 0.027	(0.591) (0.588)	0.020 0.020	0.006
	D.58	0.10	0.027	(0.586)	0.020	0.006
	D.67	0.10	0.027	(0.584)	0.020	0.006
	0.75	0.10	0.027	(0.581)	0.020	0.006
	0.83	0.13	0.036	(0.579)	0.027	0.008
	0.92	0.13	0.036	(0.577)	0.027	0.008
	1.00	0.13	0.036	(0.575)	0.027	0.008
	1.08 1.17	0.10 0.10	0.027	(0.572) (0.570)	0.020 0.020	0.006 0.006
	1.25	0.10	0.027 0.027	(0.568)	0.020	0.008
	1.33	0.10	0.027	(0.565)	0.020	0.006
	1.42	0.10	0.027	(0.563)	0.020	0.006
	1.50	0.10	0.027	(0.561)	0.020	0.006
	1.58	0.10	0.027	(0.559)	0.020	0.006
	1.67	0.10	0.027	(0.556)	0.020	0.006
	1.75	0.10	0.027	(0.554)	0.020	0.006
	1.83 1.92	0.13 0.13	0.036 0.036	(0.552) (0.550)	0.027 0.027	0.008 0.008
	2.00	0.13	0.036	(0.548)	0.027	0.008
	2.08	0.13	0.036	(0.545)	0.027	0.008
	2.17	0.13	0.036	(0.543)	0.027	0.008
	2.25	0.13	0.036	(0.541)	0.027	0.008
	2.33	0.13	0.036	(0.539)	0.027	0.008
	2.42	0.13	0.036	(0.537)	0.027	0.008
	2.50 2.58	0.13 0.17	0.036 0.045	(0.534) (0.532)	0.027 0.034	0.008 0.010
	2.50	0.17	0.045	(0.530)	0.034	0.010
	2.75	0.17	0.045	(0.528)	0.034	0.010
	2.83	0.17	0.045	(0.526)	0.034	0.010
	2.92	0.17	0.045	(0.523)	0.034	0.010
	3.00	0.17	0.045	(0.521)	0.034	0.010
	3.08	0.17	0.045	(0.519)	0.034	0.010
	3.17 3.25	0.17 0.17	0.045 0.045	(0.517) (0.515)	0.034 0.034	$0.010 \\ 0.010$
	3.33	0.17	0.045	(0.513)	0.034	0.010
	3.42	0.17	0.045	(0.511)	0.034	0.010
	3.50	0.17	0.045	(0.508)	0.034	0.010
	3.58	0.17	0.045	(0.506)	0.034	0.010
	3.67	0.17	0.045	(0.504)	0.034	0.010
	3.75	0.17	0.045	(0.502)	0.034	0.010 0.013
	3.83 3.92	0.20 0.20	0.053 0.053	(0.500) (0.498)	0.041 0.041	0.013
	4.00	0.20	0.053	(0.498)	0.041	0.013
	4.08	0.20	0.053	(0.494)	0.041	0.013
				-		

50 52 53 55 55 56 66 66 66 66 66 66 77 77 77 77 77 78 90 12 34 56 78 90 99 99 99 99 99 99 99 99 99 99 99 99	4.17 4.25 4.33 4.425 4.58 4.58 4.58 4.75 5.25	0.20 0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.27 0.27 0.27 0.20 0.20 0.20 0.20 0.23 0.23 0.23 0.27 0.27 0.27 0.27 0.27 0.27 0.27 0.27 0.30 0.30 0.30 0.30 0.33 0.50	0.053 0.053 0.062 0.062 0.062 0.062 0.062 0.062 0.071 0.071 0.071 0.053 0.053 0.053 0.062 0.062 0.062 0.062 0.062 0.062 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.080 0.089 0.0107 0.107 0.107 0.107 0.107 0.107 0.107 0.116 0.114 0.134 0.134 0.134 0.134 0.134 0.143 0.143	(0.492) (0.489) (0.487) (0.483) (0.481) (0.479) (0.477) (0.475) (0.473) (0.471) (0.469) (0.469) (0.467) (0.465) (0.463) (0.461) (0.465) (0.463) (0.461) (0.459) (0.457) (0.453) (0.451) (0.445) (0.447) (0.445) (0.447) (0.445) (0.443) (0.447) (0.443) (0.437) (0.433) (0.431) (0.429) (0.427) (0.425) (0.423) (0.421) (0.421) (0.412) (0.412) (0.412) (0.412) (0.400) (0.399) (0.391) (0.389) (0.380) (0.390) (0.390) (0.390) (0.390) (0.	0.041 0.043 0.048 0.048 0.048 0.048 0.048 0.048 0.055 0.055 0.055 0.041 0.041 0.041 0.041 0.041 0.048 0.048 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.061 0.061 0.061 0.061 0.061 0.063 0.068 0.082 0.082 0.082 0.089 0.089 0.102 0	0.013 0.013 0.015 0.015 0.015 0.015 0.015 0.017 0.017 0.017 0.017 0.013 0.013 0.013 0.013 0.015 0.015 0.015 0.015 0.017 0.017 0.017 0.017 0.017 0.017 0.017 0.017 0.019 0.019 0.019 0.019 0.019 0.019 0.019 0.019 0.021 0.023 0.023 0.023 0.025 0.025 0.027 0.031 0.031 0.031 0.034 0.034
101	8.42	0.50	0.134	(0.391)	0.102	0.031
102	8.50	0.50	0.134	(0.389)	0.102	0.031
103	8.58	0.53	0.143	(0.388)	0.109	0.034

$\begin{array}{c} 110\\ 111\\ 112\\ 113\\ 114\\ 115\\ 116\\ 117\\ 118\\ 120\\ 121\\ 122\\ 123\\ 124\\ 125\\ 126\\ 127\\ 128\\ 130\\ 131\\ 132\\ 133\\ 134\\ 135\\ 136\\ 137\\ 138\\ 140\\ 141\\ 142\\ 143\\ 144\\ 145\\ 146\\ 147\\ 148\\ 149\\ 150\\ 151\\ 152\\ 156\\ 157\\ 158\\ 159\\ 159\\ 159\\ 159\\ 150\\ 157\\ 158\\ 159\\ 150\\ 157\\ 158\\ 159\\ 150\\ 157\\ 158\\ 159\\ 150\\ 157\\ 158\\ 159\\ 150\\ 157\\ 158\\ 159\\ 150\\ 157\\ 158\\ 159\\ 150\\ 157\\ 158\\ 159\\ 150\\ 157\\ 158\\ 159\\ 150\\ 157\\ 158\\ 159\\ 150\\ 157\\ 158\\ 159\\ 150\\ 157\\ 158\\ 159\\ 150\\ 157\\ 158\\ 159\\ 150\\ 150\\ 150\\ 150\\ 150\\ 150\\ 150\\ 150$	9.17 9.25 9.33 9.42 9.50 9.58 9.67 9.75 9.83 9.92 10.00 10.25 10.33 10.42 10.50 10.58 10.67 10.75 10.83 10.92 11.00 11.08 11.17 11.25 11.33 11.42 11.50 11.58 11.67 11.58 11.67 11.58 11.67 11.58 12.00 12.08 12.00 12.08 12.00 12.08 12.00 12.08 12.00 12.58 12.67 12.58 12.67 12.58 12.67 12.58 12.67 12.58 12.92 13.00 13.08 13.17 13.25	0.63 0.67 0.67 0.67 0.70 0.70 0.70 0.73 0.73 0.73 0.50 0.50 0.50 0.50 0.50 0.50 0.67 0.67 0.67 0.67 0.63 0.97 0.97 0.97 0.97 0.97 1.13 1.13 1.13	0.169 0.169 0.178 0.178 0.178 0.178 0.187 0.187 0.196 0.196 0.196 0.196 0.134 0.134 0.134 0.134 0.134 0.134 0.178 0.178 0.178 0.178 0.178 0.178 0.178 0.169 0.162 0.152 0.152 0.152 0.152 0.250 0.223 0.223 0.223 0.250 0.259 0.259 0.259 0.259 0.303 0.303 0.303	(0.375) (0.373) (0.371) (0.370) (0.368) (0.366) (0.364) (0.363) (0.361) (0.359) (0.357) (0.356) (0.354) (0.352) (0.341) (0.342) (0.342) (0.342) (0.342) (0.342) (0.344) (0.342) (0.337) (0.337) (0.335) (0.332) (0.322) (0.321) (0.311) (0.310) (0.302) (0.299) (0.297) (0.296) (0.294)	0.130 0.130 0.136 0.136 0.136 0.136 0.143 0.143 0.143 0.143 0.150 0.150 0.150 0.150 0.102 0.102 0.102 0.102 0.102 0.102 0.102 0.102 0.102 0.136 0.136 0.136 0.136 0.136 0.136 0.130 0.130 0.130 0.130 0.130 0.130 0.130 0.130 0.130 0.130 0.130 0.130 0.130 0.130 0.130 0.116 0.116 0.116 0.116 0.1171 0.171 0.171 0.177 0.177 0.177 0.177 0.191 0.191 0.198 0.198 0.232 0.232	0.040 0.042 0.042 0.042 0.042 0.044 0.044 0.044 0.046 0.046 0.046 0.031 0.031 0.031 0.031 0.031 0.031 0.031 0.031 0.042 0.040 0.040 0.040 0.040 0.040 0.040 0.040 0.040 0.040 0.054 0.052 0.052 0.052 0.052 0.052 0.052 0.054 0.059 0.059 0.059 0.061 0.071 0.071 0.071
153 154 155 156 157 158	12.75 12.83 12.92 13.00 13.08 13.17	0.93 0.97 0.97 1.13 1.13	0.250 0.259 0.259 0.259 0.303 0.303	(0.303) (0.302) (0.300) (0.299) (0.297) (0.296)	0.191 0.198 0.198 0.198 0.232 0.232	0.059 0.061 0.061 0.061 0.071 0.071

17114172141731417414175141761417714177141791417914180151811518215183151841518515186151901519115192161931619416195161971619816199162011620316204172051720717208172091721117212172131721417215172161821718218182201822118221182211822318	3.330.3.420.3.500.3.580.	90 0.241 87 0.232 87 0.232 87 0.232 87 0.232 87 0.232 87 0.232 87 0.232 83 0.223 83 0.225 77 0.205 63 0.169 63 0.169 63 0.169 63 0.036 13 0.036 13 0.036 13 0.036 13 0.036 13 0.036 13 0.036 13 0.036 13 0.036 13 0.036 13 0.036 13 0.036 13 0.036 13 0	0.277) 0.275) 0.274) 0.273) 0.271) 0.270) 0.268) 0.267) 0.266) 0.264) 0.263) 0.262) 0.260) 0.259) 0.258) 0.256) 0.252) 0.251) 0.252) 0.251) 0.252) 0.251) 0.248) 0.247) 0.246) 0.247) 0.246) 0.243) 0.242) 0.241) 0.242) 0.242) 0.237) 0.236) 0.235) 0.237) 0.236) 0.231) 0.231) 0.231) 0.231) 0.231) 0.222) 0.222) 0.222) 0.222) 0.221) 0.222) 0.221) 0.221) 0.221) 0.217) 0.216) 0.213)	0.184 0.184 0.177 0.177 0.177 0.177 0.177 0.177 0.171 0.171 0.171 0.164 0.164 0.164 0.164 0.157 0.157 0.157 0.157 0.130 0.130 0.130 0.130 0.130 0.27 0.027 0.027 0.027 0.027 0.027 0.027 0.020 0.020 0.020 0.020 0.020 0.020 0.020 0.020 0.020 0.020 0.020 0.020 0.020 0.020 0.020 0.020 0.020 0.034 0.034 0.034 0.034 0.034 0.034 0.034 0.034 0.034 0.034 0.034 0.034 0.027 0.020 0.020	0.057 0.054 0.054 0.054 0.054 0.054 0.052 0.052 0.052 0.050 0.050 0.050 0.050 0.048 0.048 0.048 0.048 0.040 0.008 0.008 0.008 0.008 0.008 0.006 0.006 0.010 0.010 0.010 0.010 0.010 0.010 0.010 0.010 0.010 0.010 0.010 0.008 0.00
220 18 221 18 222 18 223 18 224 18 225 18 226 18 227 18 228 19	3.33 0. 3.42 0. 3.50 0. 3.58 0. 3.67 0. 3.75 0. 3.83 0. 3.92 0. 3.00 0.	13 0.036 13 0.036 13 0.036 13 0.036	0.216) 0.215) 0.214)	0.027 0.027 0.027	0.008 0.008 0.008

$\begin{array}{c} 230\\ 231\\ 232\\ 233\\ 234\\ 235\\ 236\\ 237\\ 238\\ 240\\ 241\\ 242\\ 243\\ 244\\ 245\\ 246\\ 247\\ 248\\ 255\\ 256\\ 257\\ 258\\ 260\\ 261\\ 262\\ 263\\ 266\\ 267\\ 268\\ 270\\ 271\\ 273\\ 274\\ 275\\ 276\\ 277\\ 278\\ 276\\ 277\\ 278\\ 280\end{array}$	$\begin{array}{c} 19.17\\ 19.25\\ 19.33\\ 19.42\\ 19.50\\ 19.58\\ 19.67\\ 19.75\\ 19.83\\ 19.92\\ 20.00\\ 20.08\\ 20.17\\ 20.25\\ 20.33\\ 20.42\\ 20.50\\ 20.58\\ 20.67\\ 20.75\\ 20.83\\ 20.92\\ 21.00\\ 21.08\\ 21.17\\ 21.25\\ 21.33\\ 21.42\\ 21.50\\ 21.67\\ 21.75\\ 21.83\\ 21.42\\ 21.50\\ 21.67\\ 21.75\\ 21.83\\ 21.42\\ 22.00\\ 22.08\\ 22.17\\ 22.25\\ 22.33\\ 22.42\\ 22.50\\ 22.58\\ 22.67\\ 22.58\\ 22.50\\ 22.58\\ 22.67\\ 22.58\\ 22.58\\ 22.67\\ 22.58\\ 22.67\\ 22.58\\ 22.67\\ 22.58\\ 22.67\\ 22.58\\ 22$	0.10 0.13 0.13 0.13 0.13 0.13 0.10 0.10 0.10 0.10 0.07 0.07 0.07 0.10 0.07	0.027 0.027 0.036 0.036 0.036 0.027 0.028 0.018 0.	$ \left(\begin{array}{c} 0.206 \\ (0.205 \\ (0.204 \\) \\ (0.203 \\) \\ (0.202 \\) \\ (0.201 \\) \\ (0.200 \\) \\ (0.200 \\) \\ (0.200 \\) \\ (0.200 \\) \\ (0.200 \\) \\ (0.199 \\) \\ (0.197 \\) \\ (0.197 \\) \\ (0.196 \\) \\ (0.196 \\) \\ (0.196 \\) \\ (0.197 \\) \\ (0.196 \\) \\ (0.197 \\) \\ (0.197 \\) \\ (0.197 \\) \\ (0.197 \\) \\ (0.197 \\) \\ (0.187 \\) \\ (0.187 \\) \\ (0.187 \\) \\ (0.187 \\) \\ (0.187 \\) \\ (0.187 \\) \\ (0.187 \\) \\ (0.187 \\) \\ (0.187 \\) \\ (0.187 \\) \\ (0.187 \\) \\ (0.187 \\) \\ (0.188 \\) \\ (0.187 \\) \\ (0.187 \\) \\ (0.187 \\) \\ (0.181 \\) \\ (0.181 \\) \\ (0.181 \\) \\ (0.181 \\) \\ (0.172 \\) \\ (0.177 \\) \\ (0.177 \\) \\ (0.177 \\) \\ (0.177 \\) \\ (0.177 \\) \\ (0.173 \\) \\ (0.172 \\) \\ (0.171 \\) \\ ($	0.020 0.027 0.027 0.027 0.020 0.020 0.020 0.020 0.014 0.014 0.014 0.020 0.014 0	0.006 0.008 0.008 0.008 0.008 0.008 0.006 0.006 0.004 0.004 0.006 0.004 0
273 274 275 276 277 278	22.75 22.83 22.92 23.00 23.08 23.17	0.07 0.07 0.07 0.07 0.07 0.07 0.07	0.018 0.018 0.018 0.018 0.018 0.018 0.018	(0.174) (0.173) (0.173) (0.173) (0.172) (0.172)	0.014 0.014 0.014 0.014 0.014 0.014	0.004 0.004 0.004 0.004 0.004 0.004
283 284 285 286 287 288	23.67 23.75 23.83 23.92 24.00	0.07 0.07 0.07 0.07 0.07 0.07 (Loss Rate	0.018 0.018 0.018 0.018 0.018 0.018	(0.170) (0.170) (0.169) (0.169) (0.169)	$\begin{array}{c} 0.014\\ 0.014\\ 0.014\\ 0.014\\ 0.014\\ 0.014\\ 0.014 \end{array}$	0.004 0.004 0.004 0.004 0.004 0.004

Flood times Total Total Total Flood Total	s area 274 soil loss = soil loss = rainfall = volume = soil loss =	38.953(Ac.Ft)	Ft.)] = Feet pic Feet	In) 12.0(Ac	6.3 .Ft)	
		 +++++++++++++++++++++++++++++++				 ++++
		24 – HOUR noff H				
	Hydrogr	aph in 5 Mi	nute inter))	
		Q(CFS) 0		10.0	15.0	20.0
0+ 5	0.0001	0.02 Q				
0+10 0+15	0.0007	0.08 Q 0.24 Q				
0+15	0.0023 0.0058	0.50 VO		1		E I
0+25	0.0106	0.70 VÕ	1	ł		
0+30	0.0167	0.88 VQ				
0+35	0.0241	1.08 V Q	I			
0+40	0.0326 0.0416	1.22 V Q 1.31 V Q				
0+45	0.0416	1.31 V Q				
		1.39 V Q				
	0.0614	1.48 V Q				l.
1+ 5	0 08/6	1.61 V Q 1.76 V O				
1+10	0.0973	1.61 V Q 1.76 V Q 1.85 V Q		1	1	
1+15	0.1100	1.84 V Q		1	1	
	0.1223	1.78 V Q	l	i		
1+25	0.1342	1.74 V Q		I		ļ
1+30	0 1/61	1./3 V Q		1	1	I
1+35	0.1580	1.72 V Q				I
1 + 40	0.1699	1.72 V Q				
1+45	0.1817	1.72 V Q			-	l
1+50 1+55	0.1937 0.2059	1.73 V Q 1.77 V Q	l.		1	1
1+55 2+ 0	0.2059	1.77 V Q 1.85 V Q		1	1	
2+ 0 2+ 5	0.2322	1.85 V Q 1.98 V Q	1	ł	1	
2+10	0.2464	2.06 V Q	(1	1	
2+15	0.2610	2.12 V Q	l			
2+20	0.2758	2.16 V Q	Ì	Ì	Ì	
2+25	0.2909	2.19 V Q	-		1	44444
2+30	0.3061	2.21 V Q		l		
2+35	0.3215	2.24 V Q				
2+40	0.3372	2.28 V Q				-
2+45	0.3535	2.37 V Q		-	1	
2+50	0.3708	2.51 V Q	1	1		
2+55	0.3888	2.60 IV Q				2
3+ 0 2+ 5	0.4071	2.66 V Q				1
3+ 5	0.4257	2.70 IV Q		1		1
3+10	0.4445 0.4635	2.73 V Q 2.76 V Q	1	l		1
3+15						

$\begin{array}{cccccccccccccccccccccccccccccccccccc$	2.81 2.82 2.83 2.84	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		
--	------------------------------------	---	--	--

13+257.1437 $13+30$ 7.2692 $13+35$ 7.3959 $13+40$ 7.5216 $13+45$ 7.6422 $13+50$ 7.7541 $13+55$ 7.8603 $14+0$ 7.9634 $14+0$ 8.1649 $14+15$ 8.2664 $14+20$ 8.3703 $14+25$ 8.4756 $14+30$ 8.5809 $14+35$ 8.6860 $14+40$ 8.7907 $14+45$ 8.8952 $14+55$ 9.1036 $15+0$ 9.2070 $15+5$ 9.3093 $15+10$ 9.4108 $15+15$ 9.5114 $15+25$ 9.7091 $15+30$ 9.8064 $15+35$ 9.9022 $15+40$ 9.9963 $15+45$ 10.0874 $15+55$ 10.2589 $16+0$ 10.3416 $16+5$ 10.4222 $16+10$ 10.4986 $16+15$ 10.5659 $16+20$ 10.6196 $16+25$ 10.6641 $16+30$ 10.7030 $16+55$ 10.8228 $16+55$ 10.8228 $16+55$ 10.8459 $17+0$ 10.8672 $17+5$ 10.9669 $17+20$ 10.9462 $17+25$ 10.9669 $17+35$ 11.0082 $17+40$ 11.0287 $17+45$ 11.0492 $17+55$ 11.0894 $18+5$ 11.1272 $18+10$ 11.1449 $18+20$ 11.1792			V Q V Q V Q V Q V Q V Q V Q V Q V Q V Q	7
--	--	--	--	---

18+25 18+30	11.1959	2.43 2.40					V V V
18+35 18+40 18+45	11.2288 11.2448 11.2602	2.37 2.33 2.24		100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100			V V
18+50 18+55 19+ 0	11.2747 11.2883 11.3009	2.10 1.98 . 1.84	Q Q Q				V V V
19+ 5 19+10 19+15	11.3125 11.3235 11.3346	1.68 1.60 1.61	Q Q Q				V V V
19+20 19+25	11.3462 11.3583	1.69 1.76	Q Q				V I V I
19+30 19+35 19+40	11.3711 11.3847 11.3988	1.86 1.98 2.04	Q Q Q	eren ander			V V V
19+45 19+50 19+55	11.4126 11.4257 11.4383	2.01 1.91 1.82	Q Q Q				V V V
20+ 0 20+ 5 20+10	11.4500 11.4608 11.4711	1.71 1.57 1.50					V V V
20+15 20+20	11.4816 11.4926	1.52 1.60	Q Q	-			V I V I
20+25 20+30 20+35	11.5040 11.5156 11.5272	1.65 1.68 1.69					V V V
20+40 20+45 20+50	11.5390 11.5507 11.5625	1.70 1.71 1.70		and a state			V V V
20+55 21+ 0 21+ 5	11.5740 11.5850 11.5952	1.68 1.60 1.48	Q Q Q				V V
21+10 21+15 21+20	11.6051 11.6151 11.6258	1.43 1.46 1.55	Q Q				V V V
21+25 21+30	11.6366 11.6471	1.57 1.53					V V
21+35 21+40 21+45	11.6570 11.6665 11.6764	1.43 1.39 1.43	Q Q Q				V V V V
21+50 21+55 22+ 0	11.6869 11.6975 11.7079	1.52 1.55 1.50	Q Q Q				V V V
22+ 5 22+10 22+15	11.7176 11.7270 11.7367	1.40 1.37 1.41				1	V V V
22+20 22+25	11.7471 11.7577	1.50 1.54	Q Q				V V
22+30 22+35 22+40	11.7680 11.7776 11.7867	1.49 1.39 1.33	Q Q Q	8000 			V V V
22+45 22+50 22+55	11.7956 11.8043 11.8128	1.29 1.26 1.24	Q Q Q				V V V
23+ 0 23+ 5 23+10	11.8213 11.8297 11.8380	1.23 1.22 1.21	Q Q Q				V V V
23+10 23+15 23+20	11.8463 11.8545	1.20 1.19			1		V V

23+25	11.8627	1.19 Ç		I	V
23+30	11.8709	1.18 Ç			V V
23+35	11.8790	1.18 Ç			V V
23+40	11.8871	1.18 🤇			V
23+45	11.8952	1.18 0			V V
23+50	11.9033	1.17 🤇			V
23+55	11.9113	1.17 🤇			V
24+ 0	11.9194	1.17 Ç			V
24+ 5	11.9273	1.15 Ç			V
24+10	11.9348	1.09 Ç	2		V
24+15	11.9411	0.92 Q			V
24+20	11.9457	0.67 Q			V
24+25	11.9492	0.50 Q			V
24+30	11.9519	0.40 Q			V
24+35	11.9542	0.33 Q			V
24+40	11.9560	0.27 Q	1		V
24+45	11.9576	0.23 Q			V
24+50	11.9589	0.19 Q			V
24+55	11.9600	0.16 Q			V
25+ 0	11.9610	0.14 Q			V
25+ 5	11.9618	0.12 Q			U V I
25+10	11.9625	0.10 Q			V
25+15	11.9631	0.09 Q			V
25+20	11.9636	0.07 Q			V
25+25	11.9640	0.06 Q	l		V
25+30	11.9644	0.05 Q	1		V
25+35	11.9647	0.04 Q			V
25+40	11.9649	0.04 Q			V I
25+45	11.9651	0.03 Q			V
25+50	11.9653	0.03 Q			V
25+55	11.9655	0.02 Q			· V
26+ 0	11.9656	0.02 Q			V
26+ 5	11.9657	0.02 Q			V
26+10	11.9658	0.01 Q			V
26+15	11.9659	0.01 Q		1	V
26+20	11.9659	0.01 Q			V V
26+25	11.9659	0.00 Q	1		V V
26+30	11.9659	0.00 Q			V

10 YEAR – 1 HOUR OFFSITE F₁

```
Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318offsiteF1110.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
_____
English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
St. Frances of Rome - Wildomar
Offsite (F1)
10-Year 1-Hour
_____
Drainage Area = 274.15(Ac.) = 0.428 Sg. Mi.
Drainage Area for Depth-Area Areal Adjustment = 274.15(Ac.) = 0.428 Sq.
Length along longest watercourse = 8033.90(Ft.)
Length along longest watercourse measured to centroid = 4835.60(Ft.)
Length along longest watercourse = 1.522 Mi.
Length along longest watercourse measured to centroid = 0.916 Mi.
Difference in elevation = 864.00(Ft.)
Slope along watercourse = 567.8338 Ft./Mi.
Average Manning's 'N' = 0.040
Lag time = 0.326 Hr.
Lag time = 19.58 Min.
25% of lag time = 4.90 Min.
40% of lag time = 7.83 Min.
Unit time = 5.00 Min.
Duration of storm = 1 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
Area(Ac.)[1]
              Rainfall(In)[2] Weighting[1*2]
    274.15
                   0.47
                                     128.30
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
274.15 1.30 356.39
                                   356.39
STORM EVENT (YEAR) = 10.00
```

Area Averaged 2-Year Rainfall = 0.468(In) Area Averaged 100-Year Rainfall = 1.300(In) Point rain (area averaged) = 0.810(In) Areal adjustment factor = 99.75 % Adjusted average point rain = 0.808(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 274.150 82.55 0.169 Total Area Entered = 274.15(Ac.) RI Infil. Rate Impervious Adj. Infil. Rate Area% F RI
 AMC2
 AMC-2
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 82.5
 82.5
 0.215
 0.169
 0.182
 1.000
 0.182
 Sum (F) = 0.182Area averaged mean soil loss (F) (In/Hr) = 0.182Minimum soil loss rate ((In/Hr)) = 0.091 (for 24 hour storm duration) Soil low loss rate (decimal) = 0.765 Slope of intensity-duration curve for a 1 hour storm =0.4800 Unit Hydrograph DESERT S-Curve Unit Hydrograph Data _____ Unit time period Time % of lag Distribution Unit Hydrograph (hrs) Graph % (CFS) (hrs)Graph %(CFS)10.08325.5321.3903.84020.16751.0645.43215.00830.25076.59714.01638.72440.333102.12921.84160.34550.417127.66114.44539.90960.500153.1938.75424.18670.583178.7256.24117.24280.667204.2584.72813.06490.750229.7903.69110.197100.833255.3222.9538.159110.917280.8542.5367.008121.000306.3862.1485.935131.083331.9191.6724.619141.167357.4511.4484.001151.250382.9831.2723.514161.333408.5151.1333.131171.417434.0470.9252.555181.500459.5800.8542.359191.583485.1120.7041.944201.667510.6440.6461.785211.750536.1760.4381.210231.917587.2400.2850.788242.000612.7730.2620.723<td

. 2	28	2.333	714.901		0.281		0.777
2	29	2.417	740.434		0.172		0.476
	30	2.500	765.966		0.160		0.441
	31	2.583	791.498		0.168		0.463
				Sum = 10	00.000	Sum=	276.292

(Hr. 1 0.08 2 0.17 3 0.25 4 0.33 5 0.42 6 0.50 7 0.58 8 0.67 9 0.75 10 0.83 11 0.92 12 1.00 Sum = Flood time Total Total Flood Total Flood Total Peak	12.80 25.60 7.90 4.90 (Loss Rate 100.0 volume = Eff s area 27 soil loss = rainfall = volume = soil loss = flow rate of ++++++++++++++++++++++++++++++++++++	<pre>(In/Hr) 0.427 0.436 0.524 0.524 0.553 0.621 0.766 0.883 1.242 2.483 0.766 0.475 Not Used) ective rainf 4.1(Ac.)/[(I 0.18(In 4.163(Ac 0.81(In) 623012.0 C 181357. this hydrog +++++++++++ 1 - H O U a n o f f</pre>	<pre>Max 0.182 0</pre>	Low (0.326) (0.334) (0.401) (0.401) (0.423) (0.475) (0.586) (0.950) (1.900) (0.586) (0.364) Sum = 3(In) 14.3(Ad 2.596(CFS)	(In/Hr) 0.24 0.25 0.34 0.37 0.42 0.56 0.70 1.03 2.30 0.55 0.25 = 7.5 c.Ft)	42 71 39 34 200 59 201 84 93
		graph in 5				
Time(h+m)	Volume Ac.Ft	: Q(CFS) 0	75.0	150.0	225.0	300.0
0+10	0.0065 0.0385 0.1391 0.3529 0.6622 1.0559 1.5229 2.0760 2.7542 3.6396 4.8254 6.3888 8.1285 9.4915	4.65 O		Q Q V V V	 	

1+151+201+251+301+351+401+451+501+552+02+5	10.4676 11.1643 11.6889 12.1034 12.4404 12.7233 12.9607 13.1577 13.3257 13.4706	$141.74 \\ 101.16 \\ 76.16 \\ 60.19 \\ 48.93 \\ 41.07 \\ 34.48 \\ 28.60 \\ 24.39 \\ 21.04 \\ 100 \\ $			Q 	V V V V V V V V V V
2+ 0	13.4706	21.04	Q		-	V
2+ 5	13.5966	18.29	ÌQ	I	1	V
2+10	13.7042	15.63	Q	1		V I
2+15	13.7988	13.73	Q		1	V I
2+20	13.8797	11.75	Q		I	V
2+25	13.9500	10.21	I Q	I		V I
2+30	14.0084	8.48	I Q			V
2+35	14.0585	7.27	Q	al and a second s		V
2+40	14.0988	5.85	Q			I V
2+45	14.1351	5.27	Q			
2+50	14.1698	5.04	Q	1		V I
2+55	14.2024	4.73	Q		1	V I
3+ 0 3+ 5	14.2320	4.30	Q		1	
3+ 5 3+10	14.2569 14.2748	3.62 2.59	Q		1	
3+15	14.2886	2.09	Q Q	1	1	V V
3+13	14.2987	1.46	Q	1		I VI
3+25	14.3015	0.40	Q.			I VI
3+30	14.3024	0.14	Q		-	V V

10 YEAR – 24 HOUR OFFSITE F₁

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Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318offsiteF12410.out
______
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
St. Frances of Rome - Wildomar
Offsite (F1)
10-Year 24-Hour
_____
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Drainage Area for Depth-Area Areal Adjustment = 274.15(Ac.) = 0.428 Sq.
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Length along longest watercourse measured to centroid = 4835.60(Ft.)
Length along longest watercourse = 1.522 Mi.
Length along longest watercourse measured to centroid = 0.916 Mi.
Difference in elevation = 864.00(Ft.)
Slope along watercourse = 567.8338 Ft./Mi.
Average Manning's 'N' = 0.040
Lag time = 0.326 Hr.
Lag time = 19.58 Min.
25% of lag time = 4.90 Min.
40% of lag time = 7.83 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
              Rainfall(In)[2] Weighting[1*2]
Area(Ac.)[1]
     274.15
                   2.23
                                 611.35
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
     274.15
               5.95
                                    1631.19
STORM EVENT (YEAR) = 10.00
```

Area Averaged 2-Year Rainfall = 2.230(In) Area Averaged 100-Year Rainfall = 5.950(In) Point rain (area averaged) = 3.760(In) Areal adjustment factor = 99.95 % Adjusted average point rain = 3.758(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 274.150 82.55 0.169 Total Area Entered = 274.15(Ac.) RI Infil. Rate Impervious Adj. Infil. Rate Area% F RI

 AMC2
 AMC-2
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 82.5
 82.5
 0.215
 0.169
 0.182
 1.000
 0.182

 Sum (F) = 0.182Area averaged mean soil loss (F) (In/Hr) = 0.182Minimum soil loss rate ((In/Hr)) = 0.091 (for 24 hour storm duration) Soil low loss rate (decimal) = 0.765 Unit Hydrograph DESERT S-Curve _____ Unit Hydrograph Data _____ Unit time period Time % of lag Distribution Unit Hydrograph $\begin{array}{c} (hrs) & Graph \ \$ \\ \hline \\ 1 & 0.083 & 25.532 & 1.390 \\ 2 & 0.167 & 51.064 & 5.432 \\ 3 & 0.250 & 76.597 & 14.016 \\ 4 & 0.333 & 102.129 & 21.841 \\ 5 & 0.417 & 127.661 & 14.445 \\ 6 & 0.500 & 153.193 & 8.754 \\ 7 & 0.583 & 178.725 & 6.241 \\ 8 & 0.667 & 204.258 & 4.728 \\ 9 & 0.750 & 229.790 & 3.691 \\ 10 & 0.833 & 255.322 & 2.953 \\ 11 & 0.917 & 280.854 & 2.536 \\ 12 & 1.000 & 306.386 & 2.148 \\ 13 & 1.083 & 331.919 & 1.672 \\ 14 & 1.167 & 357.451 & 1.448 \\ 15 & 1.250 & 382.983 & 1.272 \\ 16 & 1.333 & 408.515 & 1.133 \\ 17 & 1.417 & 434.047 & 0.925 \\ 18 & 1.500 & 459.580 & 0.854 \\ 19 & 1.583 & 485.112 & 0.704 \\ 20 & 1.667 & 510.644 & 0.646 \\ 21 & 1.750 & 536.176 & 0.494 \\ 22 & 1.833 & 561.708 & 0.438 \\ 23 & 1.917 & 587.240 & 0.285 \\ 24 & 2.000 & 612.773 & 0.262 \\ 25 & 2.083 & 638.305 & 0.300 \\ 26 & 2.167 & 663.837 & 0.306 \\ 27 & 2.250 & 689.369 & 0.306 \\ 28 & 2.333 & 714.901 & 0.281 \\ 29 & 2.417 & 740.434 & 0.172 \\ \end{array}$ (hrs) Graph % (CFS) 3.840 15.008 38.724 60.345 39.909 24.186 17.242 13.064 10.197 8.159 7.008 5.935 4.619 4.001 3.514 3.131 2.555 2.359 1.944 1.785 1.366 1.210 0.788 0.723 0.829 0.847 0.847 0.777 0.476

30	2.500	765.966	0.160		0.441
31	2.583	791.498	0.168		0.463
		S	um = 100.000	Sum=	276.292

Unit 1	Time (Hr.) 0.08	Pattern Percent 0.07	Storm Rain (In/Hr) 0.030	Loss rate(Max (0.323)	In./Hr) Low 0.023	Effective (In/Hr) 0.007
2	0.17	0.07	0.030	(0.322)	0.023	0.007
3 4	0.25 0.33	0.07 0.10	0.030 0.045	(0.321) (0.319)	0.023 0.035	0.007
5	0.42	0.10	0.045	(0.318)	0.035	0.011 0.011
6	0.50	0.10	0.045	(0.317)	0.035	0.011
7 8	0.58	0.10 0.10	0.045	(0.316)	0.035	0.011
8 9	0.67 0.75	0.10	0.045 0.045	(0.314) (0.313)	0.035 0.035	0.011 0.011
10	0.83	0.13	0.060	(0.312)	0.046	0.011
11	0.92	0.13	0.060	(0.311)	0.046	0.014
12 13	1.00 1.08	0.13 0.10	0.060 0.045	(0.309) (0.308)	0.046 0.035	0.014
14	1.17	0.10	0.045	(0.307)	0.035	0.011 0.011
15	1.25	0.10	0.045	(0.306)	0.035	0.011
16	1.33	0.10	0.045	(0.305)	0.035	0.011
17 18	1.42 1.50	0.10 0.10	0.045 0.045	(0.303) (0.302)	0.035 0.035	0.011 0.011
19	1.58	0.10	0.045	(0.301)	0.035	0.011
20	1.67	0.10	0.045	(0.300)	0.035	0.011
21 22	1.75 1.83	0.10 0.13	0.045 0.060	(0.299) (0.297)	0.035	0.011
23	1.03	0.13	0.060	(0.297)	0.046 0.046	0.014 0.014
24	2.00	0.13	0.060	(0.295)	0.046	0.014
25	2.08	0.13	0.060	(0.294)	0.046	0.014
26 27	2.17 2.25	0.13 0.13	0.060 0.060	(0.293) (0.291)	0.046 0.046	0.014 0.014
28	2.33	0.13	0.060	(0.291)	0.040	0.014
29	2.42	0.13	0.060	(0.289)	0.046	0.014
30 31	2.50 2.58	0.13 0.17	0.060	(0.288)	0.046	0.014
32	2.50	0.17	0.075 0.075	(0.287) (0.285)	0.058 0.058	0.018 0.018
33	2.75	0.17	0.075	(0.284)	0.058	0.018
34	2.83	0.17	0.075	(0.283)	0.058	0.018
35 36	2.92 3.00	0.17 0.17	0.075 0.075	(0.282) (0.281)	0.058 0.058	0.018
37	3.08	0.17	0.075	(0.281)	0.058	0.018 0.018
38	3.17	0.17	0.075	(0.278)	0.058	0.018
39	3.25	0.17	0.075	(0.277)	0.058	0.018
40 41	3.33 3.42	0.17 0.17	0.075 0.075	(0.276) (0.275)	0.058 0.058	0.018 0.018
42	3.50	0.17	0.075	(0.274)	0.058	0.018
43	3.58	0.17	0.075	(0.273)	0.058	0.018
44	3.67	0.17	0.075	(0.272)	0.058	0.018
45 46	3.75 3.83	0.17 0.20	0.075 0.090	(0.270) (0.269)	0.058 0.069	0.018 0.021
47	3.92	0.20	0.090	(0.268)	0.069	0.021
48	4.00	0.20	0.090	(0.267)	0.069	0.021
49	4.08	0.20	0.090	(0.266)	0.069	0.021

50 52 53 55 55 55 56 66 66 66 66 66 67 77 77 77 77 77 78 88 88 88 88 99 99 99 99 99 99 99 99 99	4.17 4.25 4.33 4.425 4.33 4.425 4.50 4.58 4.50 5.58 5.175 5.325 5.555 5.675 5.820 6.675 5.675 5.675 5.675 5.675 5.820 6.675 5.675 5.675 5.820 7.087 7.587 7.583 8.008 8.58 8.58 8.675 8.808 8.58 8.58 8.675 8.808 8.58 8.675 8.808 8.675 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.5875 8.808 8.58755 8.80855 8.808555 8.8085555555555555555555555555555555555	0.20 0.23 0.23 0.23 0.23 0.23 0.23 0.27 0.27 0.27 0.20 0.20 0.20 0.23 0.23 0.23 0.23 0.23 0.27 0.27 0.27 0.27 0.27 0.27 0.30 0.30 0.30 0.30 0.30 0.33 0.50 0.50 0.50 0.53	0.090 0.090 0.105 0.105 0.105 0.105 0.105 0.105 0.120 0.120 0.090 0.090 0.090 0.090 0.105 0.105 0.105 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.150 0.150 0.150 0.150 0.150 0.150 0.150 0.150 0.150 0.150 0.150 0.165 0.165 0.165 0.165 0.180 0.195 0.195 0.195 0.195 0.226 0.226 0.226 0.226 0.226 0.226 0.226 0.226 0.226 0.221 0.226 0.226 0.226 0.226 0.226 0.226 0.226 0.226 0.226 0.221 0.226 0.226 0.226 0.226 0.226 0.221 0.226 0	$ \left(\begin{array}{c} 0.265 \\ (0.264 \\ (0.262 \\ (0.261 \\ (0.259 \\ (0.259 \\ (0.258 \\) \\ (0.257 \\ (0.256 \\) \\ (0.255 \\ (0.255 \\ (0.251 \\) \\ (0.251 \\ (0.251 \\) \\ (0.251 \\) \\ (0.251 \\) \\ (0.251 \\) \\ (0.249 \\) \\ (0.248 \\) \\ (0.247 \\) \\ (0.246 \\) \\ (0.246 \\) \\ (0.244 \\) \\ (0.243 \\) \\ (0.242 \\) \\ (0.244 \\) \\ (0.243 \\) \\ (0.242 \\) \\ (0.244 \\) \\ (0.243 \\) \\ (0.242 \\) \\ (0.241 \\) \\ (0.239 \\) \\ (0.238 \\) \\ (0.237 \\) \\ (0.236 \\) \\ (0.237 \\) \\ (0.236 \\) \\ (0.237 \\) \\ (0.236 \\) \\ (0.237 \\) \\ (0.236 \\) \\ (0.233 \\) \\ (0.232 \\) \\ (0.233 \\) \\ (0.232 \\) \\ (0.233 \\) \\ (0.232 \\) \\ (0.223 \\) \\ (0.223 \\) \\ (0.222 \\) \\ (0.224 \\) \\ (0.223 \\) \\ (0.222 \\) \\ (0.222 \\) \\ (0.222 \\) \\ (0.222 \\) \\ (0.222 \\) \\ (0.222 \\) \\ (0.222 \\) \\ (0.222 \\) \\ (0.221 \\) \\ (0.210 \\) \\ (0.211 \\) \\ (0.210 \\) \\ (0.209 \\) \\ (0.201 \\) \\ (0.2$	0.069 0.069 0.081 0.081 0.081 0.081 0.081 0.092 0.092 0.092 0.092 0.069 0.069 0.069 0.081 0.081 0.081 0.081 0.092 0.092 0.092 0.092 0.092 0.092 0.092 0.092 0.092 0.092 0.092 0.104 0.104 0.104 0.104 0.104 0.104 0.104 0.104 0.104 0.104 0.104 0.115 0.115 0.115 0.115 0.115 0.115 0.115 0.115 0.115 0.115 0.127 0.128 0.138 0.138 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.173 0.184 0.184 0.196	0.021 0.025 0.025 0.025 0.025 0.025 0.025 0.025 0.028 0.028 0.021 0.021 0.021 0.025 0.025 0.025 0.025 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.032 0.032 0.032 0.032 0.032 0.032 0.035 0.053 0.053 0.057 0.057 0.057 0.057 0.057
104	8.67	0.53	0.241	(0.208)	0.184	0.057

$\begin{array}{c} 110\\ 111\\ 112\\ 113\\ 114\\ 115\\ 116\\ 117\\ 122\\ 123\\ 124\\ 125\\ 126\\ 127\\ 128\\ 130\\ 1312\\ 133\\ 134\\ 135\\ 136\\ 137\\ 138\\ 139\\ 140\\ 141\\ 142\\ 143\\ 144\\ 145\\ 146\\ 147\\ 148\\ 150\\ 151\\ 152\\ 155\\ 156\\ 157\\ 158\end{array}$	9.17 9.25 9.33 9.42 9.50 9.58 9.67 9.75 9.83 9.92 10.00 10.25 10.33 10.42 10.50 10.58 10.67 10.75 10.83 10.92 11.00 11.08 11.17 11.25 11.33 11.42 11.50 11.58 11.67 11.58 11.67 11.58 11.92 12.00 12.08 12.00 12.08 12.17 12.25 12.33 12.42 12.00 12.58 12.75 12.83 12.92 13.00 13.08 13.17	0.63 0.67 0.67 0.67 0.70 0.70 0.70 0.73 0.73 0.73 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.67 0.67 0.67 0.67 0.63 0.93 0.93 0.97 0.970	0.286 0.286 0.301 0.301 0.301 0.316 0.316 0.316 0.316 0.316 0.226 0.226 0.226 0.226 0.226 0.226 0.226 0.226 0.226 0.301 0.301 0.301 0.301 0.301 0.301 0.286 0.271 0.371 0.371 0.371 0.371 0.376 0.376 0.391 0	0.202 0.201 0.200 0.199 0.198 0.197 0.196 0.195 0.194 0.193 0.192 (0.192) (0.192) (0.190) (0.189) (0.187) 0.186 0.185 0.184 0.183 0.182 0.181 0.181 0.181 0.181 0.181 0.177 0.176 0.175 0.174 0.175 0.174 0.175 0.174 0.175 0.174 0.175 0.174 0.175 0.174 0.170 0.168 0.163 0.163 0.163 0.160 0.159	(0.219) (0.219) (0.230) (0.230) (0.230) (0.242) (0.242) (0.242) (0.253) (0.253) (0.253) (0.253) (0.253) (0.253) (0.253) (0.253) (0.253) (0.253) (0.253) (0.230) (0.230) (0.230) (0.230) (0.230) (0.230) (0.230) (0.230) (0.230) (0.220) (0.219) (0.219) (0.219) (0.219) (0.219) (0.219) (0.219) (0.219) (0.219) (0.219) (0.219) (0.219) (0.219) (0.219) (0.227) (0.207) (0.207) (0.207) (0.207) (0.288) (0.288) (0.288) (0.288) (0.288) (0.288) (0.299) (0.299) (0.322) (0.322) (0.334) (0.334) (0.331) (0.391) (0.391)	0.084 0.085 0.101 0.102 0.103 0.119 0.120 0.120 0.136 0.137 0.138 0.053 0.053 0.053 0.053 0.053 0.053 0.053 0.053 0.053 0.053 0.115 0.116 0.117 0.118 0.119 0.105 0.106 0.107 0.108 0.109 0.109 0.109 0.109 0.109 0.109 0.109 0.109 0.109 0.109 0.206 0.207 0.207 0.223 0.224 0.225 0.256 0.257 0.258 0.273 0.274 0.275 0.351 0.352
150	12.50	0.87	0.391	0.166	(0.299)	0.225
151	12.58	0.93	0.421	0.165	(0.322)	0.256
152	12.67	0.93	0.421	0.164	(0.322)	0.257
153	12.75	0.93	0.421	0.163	(0.322)	0.258
154	12.83	0.97	0.436	0.163	(0.334)	0.273
155	12.92	0.97	0.436	0.162	(0.334)	0.274
156	13.00	0.97	0.436	0.161	(0.334)	0.275

$\begin{array}{c} 170\\ 171\\ 172\\ 173\\ 174\\ 175\\ 176\\ 177\\ 178\\ 182\\ 183\\ 184\\ 185\\ 186\\ 187\\ 188\\ 189\\ 190\\ 191\\ 192\\ 193\\ 194\\ 195\\ 196\\ 197\\ 198\\ 200\\ 201\\ 202\\ 203\\ 204\\ 205\\ 206\\ 207\\ 208\\ 209\\ 210\\ 211\\ 212\\ 213\\ 214\\ 215\\ 216\\ 217\\ 218\\ 220\\ 222\\ 223\\ 224\\ 225\\ 226\\ 227\\ 222\\ 223\\ 224\\ 225\\ 226\\ 227\\ 222\\ 223\\ 224\\ 225\\ 227\\ 222\\ 223\\ 224\\ 225\\ 227\\ 225\\ 227\\ 222\\ 223\\ 225\\ 227\\ 225\\ 225$	$\begin{array}{c} 14.17\\ 14.25\\ 14.33\\ 14.42\\ 14.50\\ 14.58\\ 14.67\\ 14.75\\ 14.83\\ 14.92\\ 15.00\\ 15.08\\ 15.17\\ 15.25\\ 15.33\\ 15.42\\ 15.50\\ 15.58\\ 15.67\\ 15.75\\ 15.83\\ 15.92\\ 16.00\\ 16.08\\ 16.17\\ 16.25\\ 16.33\\ 16.42\\ 16.50\\ 16.58\\ 16.67\\ 16.75\\ 16.83\\ 16.92\\ 17.00\\ 17.08\\ 17.17\\ 17.25\\ 17.33\\ 17.42\\ 17.50\\ 17.58\\ 17.75\\ 17.33\\ 17.42\\ 17.50\\ 17.58\\ 17.75\\ 17.83\\ 17.92\\ 18.00\\ 18.08\\ 18.57\\ 18.58\\ 18.57\\ 18.58\\ 18.57\\ 18.58\\ 18.58\\ 18.57\\ 18.58\\ 18.57\\ 18.58\\ 18.58\\ 18.57\\ 18.58\\ 18.57\\ 18.58\\ 18.57\\ 18.58\\ 18.57\\ 18.58\\ 18.57\\ 18.58\\ 18.58\\ 18.57\\ 18.58\\ 18.58\\ 18.58\\ 18.55\\ 18.58\\ 18.58\\ 18.55\\ 18.55\\ 18.58\\ 18.55\\ 18$	0.90 0.90 0.90 0.87 0.87 0.87 0.87 0.83 0.83 0.83 0.83 0.83 0.80 0.77 0.77 0.77 0.63 0.63 0.63 0.63 0.63 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.10 0.10 0.10 0.10 0.10 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.13	0.406 0.406 0.391 0.391 0.391 0.391 0.391 0.376 0.376 0.376 0.376 0.361 0.361 0.346 0.286 0.286 0.286 0.286 0.286 0.286 0.286 0.286 0.286 0.286 0.286 0.286 0.286 0.060 0.060 0.060 0.060 0.045 0.045 0.045 0.045 0.045 0.075 0.060 0.060 0.060 0.060 0.060 0.060 0.045 0.030 0.030	0.150 0.149 0.148 0.148 0.147 0.146 0.145 0.145 0.145 0.142 0.142 0.142 0.142 0.142 0.142 0.139 0.139 0.139 0.139 0.139 0.137 0.136 0.135 0.135 0.135 0.134 (0.132) (0.128) (0.128) (0.127) (0.128) (0.127) (0.126) (0.125) (0.125) (0.125) (0.125) (0.122) (0.122) (0.122) (0.122) (0.122) (0.122) (0.121) (0.120) (0.121) (0.121) (0.120) (0.117) (0.116) (0.113) (0.113) (0.113) (0.113)	(0.311 $)($ 0.299 $)($ 0.299 $)($ 0.299 $)($ 0.299 $)($ 0.299 $)($ 0.299 $)($ 0.288 $)($ 0.288 $)($ 0.288 $)($ 0.288 $)($ 0.288 $)($ 0.276 $)($ 0.276 $)($ 0.276 $)($ 0.265 $)($ 0.265 $)($ 0.265 $)($ 0.219 $)($ 0.046 (0.035 (0.0	0.256 0.257 0.243 0.243 0.244 0.245 0.246 0.232 0.233 0.234 0.219 0.220 0.221 0.206 0.207 0.208 0.148 0.149 0.150 0.150 0.151 0.152 0.014 0.014 0.014 0.014 0.014 0.014 0.011 0.011 0.011 0.011 0.011 0.011 0.018 0.018 0.018 0.018 0.018 0.018 0.018 0.018 0.018 0.018 0.014
226	18.83	0.07	0.030	(0.113)	0.023	0.007
227	18.92	0.07	0.030	(0.112)	0.023	0.007
228	19.00	0.07	0.030	(0.112)	0.023	0.007
229	19.08	0.10	0.045	(0.111)	0.035	0.011

230 19.17 231 19.25 232 19.33 233 19.42 234 19.50 235 19.58 236 19.67 237 19.75 238 19.83 239 19.92 24020.0024120.0824220.1724320.2524420.3324520.4224620.5024720.8325120.9225221.0025321.0825421.1725521.2525621.3325721.4225821.5025921.5826021.6726121.7526221.8326321.9226422.0026522.0826622.1726722.5526822.3326922.4227022.5027122.5827522.6727322.7527422.8327522.9227623.0027723.2528023.3328123.4228223.5028323.5828423.6728523.7528623.83	0.10 0.13 0.13 0.13 0.13 0.10 0.07	0.045 0.045 0.060 0.060 0.045 0.045 0.045 0.030 0.030 0.030 0.045 0.030 0	(0.111) (0.110) (0.109) (0.109) (0.109) (0.108) (0.108) (0.107) (0.107) (0.106) (0.106) (0.105) (0.105) (0.104) (0.104) (0.104) (0.104) (0.103) (0.102) (0.102) (0.101) (0.101) (0.101) (0.101) (0.101) (0.101) (0.101) (0.099) (0.099) (0.099) (0.098) (0.098) (0.098) (0.098) (0.098) (0.097) (0.097) (0.097) (0.097) (0.096) (0.096) (0.096) (0.095) (0.095) (0.095) (0.094) (0.093) (0.092) (0.092) (0.091) (0.091) (0.091)	0.035 0.046 0.046 0.046 0.035 0.035 0.035 0.023 0.023 0.023 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.023 0	0.011 0.011 0.014 0.014 0.014 0.011 0.011 0.011 0.011 0.007 0.007 0.011 0.007 0.
		0.030 0.030 0.030		0.023 0.023 0.023 0.023	0.007 0.007 0.007 0.007

Flood time Total Total Flood Total Peak	<pre>s area 274 soil loss = soil loss = rainfall = volume = soil loss = flow rate of ++++++++++++++++++++++++++++++++++++</pre>	50.339(Ac.Ft) 3.76(In) 1547502.0 Cubic 2192748.7 Cu this hydrograph ++++++++++++++++++++++++++++++++++++	Ft.)] = : Feet bic Feet = 87. ++++++++++ S T O R	In) 35.5(Ac 760(CFS) +++++++++	 	 ++++
		n o f f H aph in 5 Mi				
		Q(CFS) 0		45 0		
 0+ 5	0.0002	0.03 0				
0+25 0+30 0+35 0+40 0+45 0+50 1+5 1+0 1+5 1+10 1+15 1+20 1+25 1+30 1+35	0.0011 0.0039 0.0097 0.0179 0.0281 0.0407 0.0549 0.0702 0.0863 0.1035 0.1222 0.1427 0.1641 0.1856 0.2062 0.2264 0.2264 0.2464 0.2865 0.3065 0.3266 0.3471 0.3686 0.3916 0.4155 0.4401 0.4652 0.5421 0.5686 0.5962 0.5421 0.5686 0.5962 0.6253 0.6556 0.6865 0.7178 0.7496 0.7817 0.8140	1.18 Q 1.49 Q 1.83 Q 2.06 Q 2.22 Q 2.35 VQ 2.50 VQ 2.71 VQ 2.97 VQ 3.11 VQ 3.11 VQ 3.00 VQ 2.93 VQ 2.91 VQ 2.91 VQ 2.91 VQ				

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	3+55	1.0447				Í	1	1
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4+50 1.4823 6.47 VQ $ $ $4+55$ 1.5277 6.59 VQ $ $ $5+$ 1.6226 7.00 VQ $ $ $5+10$ 1.6713 7.07 VQ $ $ $5+110$ 1.6713 7.07 VQ $ $ $5+20$ 1.7643 6.58 VQ $ $ $5+20$ 1.7643 6.58 VQ $ $ $5+25$ 1.8985 6.42 Q $ $ $5+30$ 1.8528 6.44 Q $ $ $5+45$ 1.9922 6.91 VQ $ $ $5+50$ 2.0414 7.15 VQ $ $ $ $ $6+0$ 2.1429 7.41 VQ $ $ $ $ $6+10$ 2.2470 7.61 VQ $ $ $ $ $6+20$ 2.3560 8.04 VQ $ $ $ $ $6+10$ 2.4429 VQ $ $ $ $ $ $ $6+20$ 2.3560								
4+55 1.5277 6.59 $ VQ$ $ VQ$ $ VQ$ $5+0$ 1.5744 6.78 $ VQ$ $ VQ$ $ VQ$ $5+10$ 1.6713 7.07 $ VQ$ $ VQ$ $ VQ$ $5+110$ 1.6713 7.07 $ VQ$ $ VQ$ $ VQ$ $5+15$ 1.7189 6.91 $ VQ$ $ VQ$ $ VQ$ $5+20$ 1.7643 6.58 $ VQ$ $ VQ$ $ VQ$ $5+25$ 1.8085 6.42 $ Q$ $ VQ$ $ VQ$ $5+30$ 1.8528 6.44 $ Q$ $ VQ$ $ VQ $ $ VQ $ $5+45$ 1.9922 6.91 $ VQ $ $ VQ $ $ VQ $ $ VQ $ $5+55$ 2.0414 7.15 $ VQ $ $ VQ $ $ VQ $ $ VQ $ $6+10$ 2.2470 7.61 $ VQ $								
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$5+10$ 1. 6713 7.07 $ v \bar{v}_{Q} $ $5+15$ 1. 7189 6.91 $ v \bar{v}_{Q} $ $5+20$ 1. 7643 6.88 $ vq $ $5+25$ 1. 8085 6.42 $ Q $ $5+30$ 1. 8528 6.44 $ Q $ $5+40$ 1. 9446 6.73 $ Q $ $5+45$ 1. 9922 6.91 $ VQ $ $5+50$ 2.0414 7.15 $ VQ $ $6+5$ 2.1429 7.41 $ VQ $ $6+10$ 2.2470 7.61 $ VQ $ $6+20$ 2.3560 8.04 $ VQ $ $6+20$ 2.3560 8.04 $ VQ $ $6+35$ 2.5279 8.42 $ VQ $ $6+35$ 2.647 8.72 $ VQ $ $6+55$ </td <td></td> <td></td> <td></td> <td>IV Q</td> <td></td> <td></td> <td></td> <td></td>				IV Q				
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5+20 1.7643 6.58 VQ $ $ $ $ $ $ $5+25$ 1.8085 6.42 Q $ $ $ $ $ $ $5+30$ 1.8528 6.44 Q $ $ $ $ $ $ $5+30$ 1.8528 6.44 Q $ $ $ $ $ $ $5+40$ 1.9446 6.73 Q $ $ $ $ $ $ $5+45$ 1.9922 6.91 VQ $ $ $ $ $ $ $5+50$ 2.0414 7.15 VQ $ $ $ $ $ $ $6+0$ 2.1429 7.41 VQ $ $ $ $ $ $ $6+10$ 2.2470 7.61 VQ $ $ $ $ $ $ $6+20$ 2.3560 8.04 VQ $ $ $ $ $ $ $6+21$ 2.4699 8.32 VQ $ $ $ $ $ $ $6+40$ 2.5866 8.53 VQ $ $ $ $ $ $ $6+45$ 2.6467 8.72							1	1
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	5+25	1.8085		I Q		1		1
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5+45 1.9922 6.91 VQ $ $ $ $ $ $ $5+55$ 2.0918 7.31 VQ $ $ $ $ $ $ $5+55$ 2.0918 7.31 VQ $ $ $ $ $ $ $6+0$ 2.1429 7.41 VQ $ $ $ $ $ $ $6+1$ 2.2470 7.61 VQ $ $ $ $ $ $ $6+10$ 2.2470 7.61 VQ $ $ $ $ $ $ $6+20$ 2.3560 8.04 VQ $ $ $ $ $ $ $6+25$ 2.4126 8.21 VQ $ $ $ $ $ $ $6+35$ 2.5279 8.42 VQ $ $ $ $ $ $ $6+40$ 2.5866 8.53 VQ $ $ $ $ $ $ $6+45$ 2.6467 8.72 VQ $ $ $ $ $ $ $6+45$ 2.6467 8.72 VQ $ $ $ $ $ $ $7+0$ 2.8354 $9.$	5+40	1.9446	6.73					
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	5+45	1.9922	6.91					
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23+45.	35.4065	1.98	Q			V I
23+50	35.4201	1.98	Q			VI
23+55	35.4337	1.97	Q			VI
24+ 0	35.4472	1.97	Q			VI
24+ 5	35.4606	1.94	Q			V
24+10	35.4732	1.83	Q			VI
24+15	35.4839	1.56	Q			V
24+20	35.4917	1.13	Q			V I
24+25	35.4975	0.84	Q			V
24+30	35.5021	0.67	Q			VI
24+35	35.5059	0.55	Q	1		V
24+40	35.5091	0.46	Q	-		V
24+45	35.5117	0.38	Q			Vļ
24+50	35.5139	0.32	Q			V I
24+55	35.5158	0.27	Q			V I
25+ 0	35.5174	0.23	Q			V I
25+ 5	35.5187	0.20	Q			V I
25+10	35.5199	0.17	Q			V I
25+15	35.5209	0.15	Q			V I
25+20	35.5218	0.12	Q			V I
25+25	35.5225	0.11	Q			VI
25+30	35.5231	0.09	Q			VI
25+35	35.5236	0.07	Q			V I
25+40	35.5240	0.06	Q			VI
25+45	35.5244	0.05	Q			VI
25+50	35.5247	0.04	Q			VI
25+55	35.5250	0.04	Q			VI
26+ 0	35.5252	0.03	Q			VI
26+ 5	35.5254	0.03	Q			VI
26+10	35.5255	0.02	Q			VI
26+15	35.5256	0.02	Q			VI
26+20	35.5257	0.01	Q			VI
26+25	35.5257	0.01	Q			VI
26+30	35.5258	0.00	Q	l		V

100 YEAR – 1 HOUR OFFSITE F₁

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Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318offsiteF11100.out
_____
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
_____
English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
St. Frances of Rome - Wildomar
Offsite (F1)
100-Year 1-Hour
_____
Drainage Area = 274.15(Ac.) = 0.428 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 274.15(Ac.) = 0.428 Sq.
Length along longest watercourse = 8033.90(Ft.)
Length along longest watercourse measured to centroid = 4835.60(Ft.)
Length along longest watercourse = 1.522 Mi.
Length along longest watercourse measured to centroid = 0.916 Mi.
Difference in elevation = 864.00(Ft.)
Slope along watercourse = 567.8338 Ft./Mi.
Average Manning's 'N' = 0.040
Lag time = 0.326 Hr.
Lag time = 19.58 Min.
25% of lag time = 4.90 Min.
40% of lag time = 7.83 Min.
Unit time = 5.00 Min.
Duration of storm = 1 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
              Rainfall(In)[2] Weighting[1*2]
Area(Ac.)[1]
                                 128.30
     274.15
                    0.47
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
     274.15
                 1.30
                                     356.39
STORM EVENT (YEAR) = 100.00
```

Area Averaged 2-Year Rainfall = 0.468(In) Area Averaged 100-Year Rainfall = 1.300(In) Point rain (area averaged) = 1.300(In) Areal adjustment factor = 99.75 % Adjusted average point rain = 1.297(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 274.150 82.55 0.169 Total Area Entered = 274.15(Ac.) RI RI Infil. Rate Impervious Adj. Infil. Rate Area% F AMC2 AMC-2(In/Hr)(Dec.%)(In/Hr)(Dec.)(In/Hr)82.582.50.2150.1690.1821.0000.182 0.182 Sum (F) = 0.182Area averaged mean soil loss (F) (In/Hr) = 0.182Minimum soil loss rate ((In/Hr)) = 0.091 (for 24 hour storm duration) Soil low loss rate (decimal) = 0.765 _____ Slope of intensity-duration curve for a 1 hour storm =0.4800Unit Hydrograph DESERT S-Curve Unit Hydrograph Data _____ Unit time period Time % of lag Distribution Unit Hydrograph (hrs) Graph % (CFS) (hrs)Graph %10.08325.5321.39020.16751.0645.43230.25076.59714.01640.333102.12921.84150.417127.66114.44560.500153.1938.75470.583178.7256.24180.667204.2584.72890.750229.7903.691100.833255.3222.953110.917280.8542.536121.000306.3862.148131.08331.9191.672141.167357.4511.448151.250382.9831.272161.333408.5151.133171.417434.0470.925181.500459.5800.854191.583485.1120.704201.667510.6440.646211.750536.1760.494221.833561.7080.438231.917587.2400.285242.000612.7730.262252.083638.3050.300262.16763.8370.306272.250689.3690.3063.840 15.008 38.724 60.345 39.909 24.186 17.242 13.064 10.197 8.159 7.008 5.935 4.619 4.001 3.514 3.131 2.555 2.359 1.944 1.785 1.366 1.210 0.788 0.723 0.829 0.847 0.847

28	2.333	714.901	0.281		0.777	
29	2.417	740.434	0.172		0.476	
30	2.500	765.966	0.160		0.441	
31	2.583	791.498	0.168		0.463	
			Sum = 100.000	Sum=	276.292	

(Hr. 1 0.08 2 0.17 3 0.25 4 0.33 5 0.42 6 0.50 7 0.58 8 0.67 9 0.75 10 0.83 11 0.92 12 1.00 Sum = Flood time Total Total Total Flood Total Flood Total Peak	9.10 12.80 25.60 7.90 4.90 (Loss Rate 100.0 volume = Eff s area 27 soil loss = rainfall = volume = soil loss = flow rate of ++++++++++++++++++++++++++++++++++++	<pre>(In/Hr) 0.685 0.700 0.840 0.840 0.996 1.229 1.416 1.992 3.984 1.229 0.763 Not Used) ective rainfa 4.1(Ac.)/[(Ir 0.18(In) 4.163(Ac. 1.30(In) 1109141.7 Cu 181357.9 </pre>	<pre>Max 0.182 0</pre>	Low (0.524) (0.536) (0.643) (0.643) (0.679) (0.762) (0.940) (1.083) (1.524) (3.048) (0.940) (0.583) Sum = (In) 25.5(Ac .186(CFS)	<pre>(In/Hr) 0.50 0.61 0.61 0.70 0.81 1.00 1.22 1.83 3.80 1.00 0.55 = 13.4 :.Ft)</pre>	58 05 14 47 34 10 01 47 80
Time(h+m)	Volume Ac.Ft	Q(CFS) 0			375.0	500.0
0+10 0+15 0+20 0+25	$\begin{array}{c} 0.0133\\ 0.0790\\ 0.2840\\ 0.7166\\ 1.3325\\ 2.1023\\ 3.0029\\ 4.0514\\ 5.3082\\ 6.9038\\ 8.9867\\ 11.6799\\ 14.6564\\ 17.0202 \end{array}$	9.54 Q 29.77 V Q 62.82 V 89.42 V	 		 	

1+15	18.7367	249.23				QI	V	
1+20	19.9615	177.83	1		Q		V	
1+25	20.8817	133.61		Q		1	V	
1+30	21.6088	105.59		QI		1	V	
1+35	22.2000	85.83	l Q			1	V	
1+40	22.6956	71.97	I Q			l	V I	
1+45	23.1118	60.43	I Q			a constru	V	
1+50	23.4577	50.23	I Q	1		a su	V	
1+55	23.7525	42.80	ΙQ	1		1	V	
2+ 0	24.0062	36.84	ΙQ				V	
2+ 5	24.2267	32.01	I Q				V	
2+10	24.4154	27.40	I Q			1	V	
2+15	24.5810	24.05	IQ				V	
2+20	24.7230	20.62	IQ			1	V	
2+25	24.8464	17.92	IQ				V	1
2+30	24.9492	14.92	IQ			1	V	
2+35	25.0375	12.83	Q			1	V	
2+40	25.1089	10.36	Q	I			V	November 1
2+45	25.1728	9.27	Q			1	V	
2+50	25.2331	8.76	Q			1	V	
2+55	25.2895	8.18	Q	1			V	
3+ 0	25.3406	7.42	Q	I			V	1
3+ 5	25.3835	6.23	Q				V	l
3+10	25.4144	4.49	Q	1			V	
3+15	25.4382	3.46	Q	1			V	
3+20	25.4554	2.50	Q			1	V	
3+25	25.4605	0.74	Q				V	
3+30	25.4624	0.27	Q	l		1	7	V

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100 YEAR – 24 HOUR OFFSITE F₁

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Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318offsiteF124100.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
     English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
St. Frances of Rome - Wildomar
Offsite (Fl)
100-Year 24-Hour
Drainage Area = 274.15(Ac.) = 0.428 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 274.15(Ac.) = 0.428 Sq.
Length along longest watercourse = 8033.90(Ft.)
Length along longest watercourse measured to centroid = 4835.60(Ft.)
Length along longest watercourse = 1.522 Mi.
Length along longest watercourse measured to centroid = 0.916 Mi.
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Slope along watercourse = 567.8338 Ft./Mi.
Average Manning's 'N' = 0.040
Lag time = 0.326 Hr.
Lag time = 19.58 Min.
25% of lag time = 4.90 Min.
40% of lag time = 7.83 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
     274.15
                                 611.35
              2.23
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
     274.15
             5.95
                               1631.19
STORM EVENT (YEAR) = 100.00
```

Area Averaged 2-Year Rainfall = 2.230(In) Area Averaged 100-Year Rainfall = 5.950(In) Point rain (area averaged) = 5.950(In) Areal adjustment factor = 99.95 % Adjusted average point rain = 5.947(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 274.150 82.55 0.169 Total Area Entered = 274.15(Ac.)
 RI
 RI
 Infil. Rate
 Impervious
 Adj.
 Infil.
 Rate
 Area%
 F

 AMC2
 AMC-2
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 82.5
 82.5
 0.215
 0.169
 0.182
 1.000
 0.182

 Sum
 (F)
 0.182
 1.000
 0.182
 Sum (F) = 0.182Area averaged mean soil loss (F) (In/Hr) = 0.182Minimum soil loss rate ((In/Hr)) = 0.091 (for 24 hour storm duration) Soil low loss rate (decimal) = 0.765_____ Unit Hydrograph DESERT S-Curve Unit Hydrograph Data _____ Unit time period Time % of lag Distribution Unit Hydrograph Graph % (CFS) (hrs) 3.840 15.008 38.724 60.345 39.909 24.186 17.242 13.064 10.197 8.159 7.008 5.935 4.619 4.001 3.514 3.131 2.555 2.359 1.944 1.785 1.366 1.210 0.788 0.723 0.829 0.847 0.847 0.777 0.476

30	2.500	765.966	0.160		0.441
31	2.583	791.498	0.168		0.463
			Sum = 100.000	Sum=	276.292

Unit 1	Time (Hr.)	Pattern Percent	Storm Rain (In/Hr)	Loss rate(] Max	[n./Hr) Low	Effective (In/Hr)
1 0	80.0	0.07	0.048	(0.323)	0.036	0.011
	0.17 0.25	0.07 0.07	0.048 0.048	(0.322) (0.321)	0.036 0.036	$0.011 \\ 0.011$
	0.23	0.10	0.048	(0.319)	0.055	0.017
5 (0.42	0.10	0.071	(0.318)	0.055	0.017
6 (0.50	0.10	0.071	(0.317)	0.055	0.017
	0.58 0.67	0.10 0.10	0.071 0.071	(0.316) (0.314)	0.055 0.055	0.017 0.017
	0.87	0.10	0.071	(0.314)	0.055	0.017
	0.83	0.13	0.095	(0.312)	0.073	0.022
	0.92	0.13	0.095	(0.311)	0.073	0.022
	1.00 1.08	0.13 0.10	0.095 0.071	(0.309) (0.308)	0.073 0.055	0.022 0.017
	1.08	0.10	0.071	(0.307)	0.055	0.017
	1.25	0.10	0.071	(0.306)	0.055	0.017
	1.33	0.10	0.071	(0.305)	0.055	0.017
	1.42	0.10	0.071	(0.303)	0.055 0.055	0.017 0.017
	1.50 1.58	0.10 0.10	0.071 0.071	(0.302) (0.301)	0.055	0.017
	1.67	0.10	0.071	(0.300)	0.055	0.017
	1.75	0.10	0.071	(0.299)	0.055	0.017
	1.83	0.13	0.095	(0.297)	0.073 0.073	0.022 0.022
	1.92 2.00	0.13 0.13	0.095 0.095	(0.296) (0.295)	0.073	0.022
	2.08	0.13	0.095	(0.294)	0.073	0.022
26	2.17	0.13	0.095	(0.293)	0.073	0.022
	2.25	0.13	0.095	(0.291)	0.073	0.022 0.022
	2.33 2.42	0.13 0.13	0.095 0.095	(0.290) (0.289)	0.073 0.073	0.022
	2.50	0.13	0.095	(0.288)	0.073	0.022
31	2.58	0.17	0.119	(0.287)	0.091	0.028
	2.67	0.17	0.119	(0.285)	0.091	0.028
	2.75 2.83	0.17 0.17	0.119 0.119	(0.284) (0.283)	0.091 0.091	0.028 0.028
	2.92	0.17	0.119	(0.282)	0.091	0.028
36	3.00	0.17	0.119	(0.281)	0.091	0.028
	3.08	0.17	0.119	(0.280)	0.091	0.028
38 39	3.17 3.25	0.17 0.17	0.119 0.119	(0.278) (0.277)	0.091 0.091	0.028 0.028
	3.33	0.17	0.119	(0.276)	0.091	0.028
41	3.42	0.17	0.119	(0.275)	0.091	0.028
42	3.50	0.17	0.119	(0.274)	0.091	0.028
43 44	3.58 3.67	0.17 0.17	0.119 0.119	(0.273) (0.272)	0.091 0.091	0.028 0.028
45	3.75	0.17	0.119	(0.272)	0.091	0.028
46	3.83	0.20	0.143	(0.269)	0.109	0.034
47	3.92	0.20	0.143	(0.268)	0.109	0.034
48 49	4.00 4.08	0.20 0.20	0.143 0.143	(0.267) (0.266)	0.109 0.109	0.034 0.034
	1.00	0.20	0.110	, 0.200,	0.200	

1078.920.570.4040.205(0.309)0.31089.000.570.4040.204(0.309)0.3
--

110 9.17 111 9.25 112 9.33 113 9.42 114 9.50 115 9.58 116 9.67 117 9.75 118 9.83 119 9.92 120 10.00 21 10.08 122 10.17 123 10.25 124 10.33 125 10.42 126 10.50 127 10.58 128 10.67 129 10.75 130 10.83 131 10.92 132 11.00 133 11.08 134 11.17 135 11.25 136 11.33 137 11.42 138 11.50 139 11.58 140 11.67 141 11.75 142 11.83 143 11.92 144 12.00 145 12.08 146 12.17 147 12.25 148 12.33 149 12.42 150 12.50 151 12.58 152 12.67 153 12.75 154 12.83 155 12.92 156 13.00 157 13.08 158 13.17 159 13.25 160 13.33 161 13.42 162 <t< th=""><th>0.63 0.67 0.67 0.67 0.70 0.70 0.70 0.70 0.73 0.73 0.73 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.67 0.67 0.67 0.67 0.67 0.63 0.77 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.93 0.93 0.97 0.77 0.77</th><th>0.452 0.476 0.476 0.476 0.500 0.500 0.523 0.523 0.523 0.523 0.357 0.357 0.357 0.357 0.357 0.357 0.476 0.476 0.476 0.476 0.476 0.476 0.472 0.452 0.595 0.595 0.595 0.595 0.618 0.666 0.666 0.690 0.809 0.809 0.809 0.809 0.809 0.547 0.547</th><th>0.201 0.200 0.199 0.198 0.197 0.196 0.195 0.194 0.193 0.192 0.192 0.191 0.190 0.189 0.188 0.185 0.186 0.185 0.184 0.183 0.182 0.181 0.181 0.180 0.179 0.178 0.177 0.176 0.175 0.174 0.175 0.174 0.175 0.174 0.175 0.174 0.175 0.174 0.175 0.171 0.168 0.168 0.163 0.163 0.163 0.163 0.161 0.159 0.158 0.157 0.156 0.155</th><th>$\left(\begin{array}{c} 0.346 \right) \\ (0.346) \\ (0.364) \\ (0.364) \\ (0.382) \\ (0.382) \\ (0.382) \\ (0.382) \\ (0.400) \\ (0.400) \\ (0.400) \\ (0.273) \\ (0.364) \\ (0.364) \\ (0.364) \\ (0.364) \\ (0.346) \\ (0.328) \\ (0.328) \\ (0.328) \\ (0.328) \\ (0.473) \\ (0.473) \\ (0.473) \\ (0.473) \\ (0.473) \\ (0.510) \\ (0.528) \\ (0.528) \\ (0.528) \\ (0.528) \\ (0.528) \\ (0.619) \\ (0.619) \\ (0.619) \\ (0.619) \\ (0.619) \\ (0.619) \\ (0.4$</th><th>0.250 0.251 0.276 0.277 0.278 0.302 0.303 0.304 0.329 0.330 0.331 0.165 0.166 0.167 0.168 0.169 0.290 0.291 0.292 0.292 0.292 0.292 0.292 0.292 0.292 0.292 0.292 0.292 0.292 0.271 0.272 0.273 0.274 0.275 0.276 0.229 0.231 0.255 0.256 0.257 0.425 0.425 0.425 0.425 0.425 0.425 0.425 0.425 0.425 0.425 0.425 0.425 0.425 0.501 0.502 0.528 0.529 0.649 0.651 0.652 0.392 0.393</th></t<>	0.63 0.67 0.67 0.67 0.70 0.70 0.70 0.70 0.73 0.73 0.73 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.67 0.67 0.67 0.67 0.67 0.63 0.77 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.93 0.93 0.97 0.77 0.77	0.452 0.476 0.476 0.476 0.500 0.500 0.523 0.523 0.523 0.523 0.357 0.357 0.357 0.357 0.357 0.357 0.476 0.476 0.476 0.476 0.476 0.476 0.472 0.452 0.595 0.595 0.595 0.595 0.618 0.666 0.666 0.690 0.809 0.809 0.809 0.809 0.809 0.547 0.547	0.201 0.200 0.199 0.198 0.197 0.196 0.195 0.194 0.193 0.192 0.192 0.191 0.190 0.189 0.188 0.185 0.186 0.185 0.184 0.183 0.182 0.181 0.181 0.180 0.179 0.178 0.177 0.176 0.175 0.174 0.175 0.174 0.175 0.174 0.175 0.174 0.175 0.174 0.175 0.171 0.168 0.168 0.163 0.163 0.163 0.163 0.161 0.159 0.158 0.157 0.156 0.155	$ \left(\begin{array}{c} 0.346 \right) \\ (0.346) \\ (0.364) \\ (0.364) \\ (0.382) \\ (0.382) \\ (0.382) \\ (0.382) \\ (0.400) \\ (0.400) \\ (0.400) \\ (0.273) \\ (0.364) \\ (0.364) \\ (0.364) \\ (0.364) \\ (0.346) \\ (0.328) \\ (0.328) \\ (0.328) \\ (0.328) \\ (0.473) \\ (0.473) \\ (0.473) \\ (0.473) \\ (0.473) \\ (0.510) \\ (0.528) \\ (0.528) \\ (0.528) \\ (0.528) \\ (0.528) \\ (0.619) \\ (0.619) \\ (0.619) \\ (0.619) \\ (0.619) \\ (0.619) \\ (0.4$	0.250 0.251 0.276 0.277 0.278 0.302 0.303 0.304 0.329 0.330 0.331 0.165 0.166 0.167 0.168 0.169 0.290 0.291 0.292 0.292 0.292 0.292 0.292 0.292 0.292 0.292 0.292 0.292 0.292 0.271 0.272 0.273 0.274 0.275 0.276 0.229 0.231 0.255 0.256 0.257 0.425 0.425 0.425 0.425 0.425 0.425 0.425 0.425 0.425 0.425 0.425 0.425 0.425 0.501 0.502 0.528 0.529 0.649 0.651 0.652 0.392 0.393
160 13.33	1.13	0.809	0.158	(0.619)	0.651
161 13.42	1.13	0.809	0.157	(0.619)	0.652
162 13.50	1.13	0.809	0.156	(0.619)	0.653

170 171 172 173 174 175 176 177 180 181 182 183 184 185 186 187 188 190 191 192 193 194 195 196 201 203 204 205 206 207 208 200 211 213 214 216 217 218 219 220 221 223 224 226	$\begin{array}{c} 14.17\\ 14.25\\ 14.33\\ 14.42\\ 14.50\\ 14.58\\ 14.67\\ 14.75\\ 14.83\\ 14.92\\ 15.00\\ 15.08\\ 15.17\\ 15.25\\ 15.33\\ 15.42\\ 15.50\\ 15.58\\ 15.67\\ 15.75\\ 15.83\\ 15.92\\ 16.00\\ 16.08\\ 16.17\\ 16.25\\ 16.33\\ 16.42\\ 16.50\\ 16.58\\ 16.67\\ 16.75\\ 16.83\\ 16.42\\ 16.50\\ 16.58\\ 16.67\\ 16.75\\ 16.83\\ 16.92\\ 17.00\\ 17.08\\ 17.17\\ 17.25\\ 17.33\\ 17.42\\ 17.50\\ 17.58\\ 17.67\\ 17.75\\ 17.83\\ 17.92\\ 18.00\\ 18.08\\ 18.17\\ 18.25\\ 18.33\\ 18.42\\ 18.50\\ 18.58\\ 18.67\\ 18.75\\ 18.83\end{array}$	0.90 0.90 0.87 0.87 0.87 0.87 0.87 0.87 0.83 0.83 0.83 0.83 0.83 0.80 0.80 0.77 0.77 0.77 0.77 0.77 0.77 0.77 0.63 0.63 0.63 0.63 0.63 0.63 0.63 0.13 0.13 0.13 0.10 0.10 0.10 0.10 0.17 0.13 0.13 0.13 0.13 0.10	0.642 0.642 0.618 0.618 0.618 0.618 0.595 0.595 0.595 0.595 0.571 0.571 0.571 0.547 0.547 0.452 0.452 0.452 0.452 0.452 0.955 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.071 0.711 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.095 0.09	0.150 0.149 0.148 0.148 0.147 0.146 0.145 0.145 0.145 0.142 0.142 0.142 0.142 0.141 0.140 0.139 0.139 0.138 0.137 0.136 0.135 0.135 0.131 (0.132) (0.132) (0.132) (0.132) (0.132) (0.132) (0.132) (0.132) (0.132) (0.128) (0.128) (0.128) (0.128) (0.127) (0.128) (0.127) (0.126) (0.125) (0.125) (0.122) (0.122) (0.122) (0.123) (0.122) (0.121) (0.120) (0.121) (0.120) (0.117) (0.116) (0.113) (0.113) (0.113)	(0.491) (0.473) (0.473) (0.473) (0.473) (0.473) (0.473) (0.473) (0.455) (0.455) (0.455) (0.437) (0.437) (0.437) (0.419) (0.419) (0.346) (0.355) 0.073 0.073 0.073 0.073 0.055 0.055 0.055 0.055 0.055 0.091 0.091 0.091 0.091 0.091 0.091 0.091 0.091 0.091 0.091 0.091 0.091 0.091 0.091 0.091 0.073 0.055 0.	0.492 0.493 0.470 0.471 0.472 0.472 0.472 0.473 0.474 0.451 0.452 0.452 0.429 0.430 0.431 0.408 0.409 0.315 0.315 0.316 0.317 0.317 0.318 0.022 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.022 0
224	18.67	0.10	0.071	(0.114)	0.055	0.017

230 19.17 231 19.25 232 19.33 233 19.42 234 19.50 235 19.58 236 19.67 237 19.75 238 19.83 239 19.92 24020.0024120.0824220.1724320.2524420.3324520.4224620.5024720.5824820.6724920.7525020.8325120.9225221.0025321.0825421.1725521.2525621.3325721.4225821.5025921.5826021.6726121.7526221.8326321.9226422.0026522.0826622.1726722.5027122.5827226727322.7527422.8327522.9227623.0027723.0827823.1727923.2528023.3328123.4228223.5828423.6728523.75	0.10 0.13 0.13 0.13 0.10 0.10 0.10 0.07 0.07 0.07 0.07 0.07 0.10 0.07	0.071 0.071 0.095 0.095 0.095 0.071 0.048 0	(0.111) (0.110) (0.109) (0.109) (0.109) (0.108) (0.108) (0.107) (0.107) (0.106) (0.106) (0.105) (0.105) (0.105) (0.104) (0.104) (0.104) (0.104) (0.104) (0.103) (0.102) (0.102) (0.102) (0.101) (0.101) (0.101) (0.101) (0.101) (0.101) (0.099) (0.099) (0.098) (0.098) (0.098) (0.098) (0.098) (0.098) (0.098) (0.097) (0.097) (0.097) (0.097) (0.097) (0.097) (0.095) (0.095) (0.095) (0.095) (0.095) (0.094) (0.094) (0.093) (0.093) (0.092) (0.092) (0.092) (0.091) (0.091)	0.055 0.073 0.073 0.073 0.055 0.055 0.055 0.036 0.036 0.036 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.036 0.03	0.017 0.022 0.022 0.022 0.017 0.017 0.017 0.017 0.011 0.011 0.011 0.017 0.011 0.
281 23.42 282 23.50 283 23.58	0.07 0.07 0.07	0.048 0.048 0.048 0.048 0.048 0.048 0.048 0.048 0.048	(0.092) (0.092) (0.092)	0.036 0.036 0.036	0.011 0.011 0.011

Flood time: Total Total Total Flood Total Peak	s area 274 soil loss = soil loss = rainfall = volume = 3 soil loss =	62.570(Ac.Ft) 5.95(In) 3192518.7 Cubic 2725539.5 Cu this hydrograph	Ft.)] = c Feet abic Feet n = 164.	(In) 73.3(Ac		
++++		++++++++++++++++++++++++++++++++++++++	STOR	М	++++++++++	++++
		aph in 5 Mi			5))	
	Volume Ac.Ft	Q(CFS) 0	50.0	100.0	150.0	200.0
$0+5 \\ 0+10 \\ 0+15 \\ 0+20 \\ 0+25 \\ 0+30 \\ 0+35 \\ 0+40 \\ 0+45 \\ 0+50 \\ 0+55 \\ 1+0 \\ 1+5 \\ 1+10 \\ 1+15 \\ 1+20 \\ 1+25 \\ 1+30 \\ 1+35 \\ 1+3$	0.0003 0.0017 0.0062 0.0154 0.0283 0.0445 0.0644 0.0869 0.1110 0.1366 0.1638 0.1934 0.2258 0.2597 0.2936 0.3262 0.3582 0.3899 0.4216	0.04 Q 0.21 Q 0.64 Q 1.34 Q 1.87 Q 2.36 Q 2.89 Q 3.26 Q 3.51 Q 3.72 Q 3.95 Q 4.29 Q 4.29 Q 4.70 Q 4.93 Q 4.92 Q 4.74 Q				

3+25 3+30 3+35 3+40 3+45	1.3393 1.3910 1.4429 1.4950 1.5472	7.50 7.53 7.56	VQ VQ VQ VQ			
3+50 3+55 4+ 0 4+ 5	1.5998 1.6530 1.7079 1.7652	7.63 7.73 7.97 8.32	VQ VQ VQ VQ	-		
4+10 4+15 4+20 4+25 4+30	1.8241 1.8840 1.9448 2.0067 2.0705	8.70 8.83 8.99 9.27	VQ Q Q Q Q			
4+35 4+40 4+45 4+50 4+55	2.1370 2.2054 2.2749 2.3454 2.4172	9.92 10.09 10.24	Q Q VQ VQ VQ			
5+ 0 5+ 5 5+10 5+15 5+20	2.4910 2.5674 2.6444 2.7198 2.7915	10.72 11.08 11.19 10.94 10.42	VQ VQ VQ VQ VQ			
5+25 5+30 5+35 5+40 5+45	2.8615 2.9317 3.0035 3.0768 3.1521	10.12 10.19 10.43 10.65 10.93	VQ VQ VQ VQ VQ			
5+50 5+55 6+ 0 6+ 5	3.2301 3.3098 3.3906 3.4723	11.32 11.57 11.73 11.87	VQ VQ VQ VQ			
6+10 6+15 6+20 6+25 6+30	3.5553 3.6402 3.7278 3.8173 3.9080	12.04 12.33 12.72 12.99 13.17	VQ VQ Q Q Q			
6+35 6+40 6+45 6+50 6+55	3.9998 4.0927 4.1878 4.2856 4.3853	13.32 13.50 13.80 14.20 14.49	Q Q Q Q			
7+ 0 7+ 5 7+10 7+15 7+20	4.4864 4.5884 4.6910 4.7943 4.8982	14.67 14.81 14.91 14.99 15.08	Q Q Q Q VQ			
7+25 7+30 7+35 7+40 7+45	5.0030 5.1097 5.2191 5.3309 5.4454	15.22 15.49 15.89 16.24 16.62	VQ VQ VQ VQ VQ			
7+50 7+55 8+ 0 8+ 5	5.5637 5.6868 5.8180 5.9621	17.18 17.87 19.05 20.93	Q Q Q VQ			
8+10 8+15 8+20	6.1196 6.2956 6.4961	22.86 25.56 29.12	VQ V Q V Q			

8+25	6.7138	31.61	IVQIIIII
8+30 8+35	6.9430	33.28 34.66	
8+35 8+40	7.1818 7.4303	36.08	
8+45	7.6914	37.91	
8+50	7.9683	40.21	V Q
8+55	8.2591	42.22	V Q I I I
9+ 0	8.5646	44.36	
9+ 5	8.8880 9.2286	46.96 49.45	V Q V Q
9+10 9+15	9.5909	49.4J 52.61	V Q V Q
9+20	9.9809	56.64	
9+25	10.3928	59.81	V Q
9+30	10.8245	62.68	V Q
9+35	11.2771	65.72 68.28	
9+40 9+45	11.7473 12.2352	68.28 70.84	Ι V Q V Q
9+50	12.7427	73.69	
9+55	13.2670	76.12	V Q I I
10+ 0	13.8083	78.60	
10+5	14.3639 14.9162	80.66 80.20	
10+10 10+15	15.4346	75.27	
10+20	15.8923	66.46	V Q I I
10+25	16.3114	60.85	V Q I I
10+30	16.7090	57.73	
10+35 10+40	17.0955 17.4841	56.11 56.43	V Q V Q
10+45	17.8972	59.98	
10+50	18.3543	66.38	V Q I I I
10+55	18.8397	70.48	V Q I I I
11+0	19.3415 19.8547	72.86 74.52	
11+ 5 11+10	20.3746	74.52	
11+15	20.8952	75.59	I V Q I I
11+20	21.4117	75.00	I V Q I I
11+25	21.9268	74.81	
11+30 11+35	22.4426 22.9581	74.88 74.85	
11+40	23.4698	74.30	
11+45	23.9705	72.70	I VQ I I
11+50	24.4539	70.19	I VQ I I
11+55 12+ 0	24.9286 25.4037	68.93 68.99	
12+0 12+5	25.8892	70.50	
12+10	26.3958	73.55	
12+15	26.9487	80.28	I VQ I
12+20	27.5729	90.64	
12+25 12+30	28.2466 28.9554	97.82 102.92	
12+35	29.6964	107.60	
12+40	30.4658	111.71	I V I Q I I
12+45	31.2654	116.10	
12+50 12+55	32.0993 32.9607	121.09 125.07	V Q V Q
12+55 13+ 0	33.8465	123.07	
13+ 5	34.7593	132.54	V V Q I
13+10	35.7022	136.92	
13+15 13+20	36.6906 37.7397	143.51 152.32	
10720	51.1581	T 7 C • 7 C	i i v V l

25 38.8310 158.46 I V IQ I 30 39.904 162.55 I V IQ I 30 39.904 162.55 I V IQ I 10 42.2078 163.16 I V IQ I 15 43.2754 153.02 I V IQ I 15 44.2459 140.91 I V Q I 10 46.0279 126.85 I I Q I I 10 47.7263 122.82 I I Q V I 10 47.7263 122.82 I I Q V I 10 53.091 131.44 I I Q V I 10 53.091 131.27 I I Q V I 10 54.6060 131.06 I Q V I I 10 56.6050 102.74 I Q V <
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$18+25 \\18+30 \\18+35 \\18+40 \\18+45 \\18+50 \\18+55 \\19+0 \\19+5 \\19+10 \\19+15 \\19+20 \\19+25 \\19+20 \\19+35 \\19+40 \\19+35 \\19+40 \\19+45 \\19+55 \\20+0 \\20+5 \\20+10 \\20+15 \\20+20 \\20+25 \\20+30 \\20+35 \\20+40 \\20+45 \\20+50 \\20+55 \\21+0 \\20+55 \\21+0 \\21+5 \\21+10 \\21+15 \\21+20 \\21+25 \\21+30 \\21+25 \\21+25 \\21+30 \\21+25 \\$	71.2349 71.2797 71.3232 71.3660 71.4071 71.4457 71.4457 71.5158 71.5466 71.5760 71.6055 71.6365 71.6688 71.7029 71.7394 71.7768 71.8137 71.8487 71.8822 71.9135 71.9423 71.9423 71.9423 71.9423 71.9423 71.9423 71.9423 72.0576 72.0273 72.0576 72.1509 72.1509 72.1509 72.1509 72.1236 72.2444 72.2738 72.3010 72.3272 72.3541 72.3825 72.4114 72.4395	$ \begin{array}{ccccccccccccccccccccccccccccccccccc$			V V V
19+55 20+ 0 20+ 5	71.8822 71.9135 71.9423	4.85 Q 4.55 Q 4.18 Q			V V V
20+15 20+20	71.9978 72.0273	4.06 Q 4.28 Q			V V
20+30 20+35	72.0885 72.1196	4.47 Q 4.52 Q			V V
20+50 20+55	72.1823 72.2136 72.2444	4.56 Q 4.55 Q 4.47 Q		- La construction de la construc	V V
21+ 5 21+10	72.3010 72.3272	3.95 Q 3.81 Q			V V
21+20 21+25	72.3825 72.4114	4.13 Q 4.20 Q	р 		V V V
21+35 21+40 21+45 21+50	72.4657 72.4913 72.5175	3.81 Q 3.71 Q 3.82 Q			V V V V
21+50 21+55 22+ 0 22+ 5	72.5455 72.5740 72.6016 72.6274	4.06 Q 4.14 Q 4.01 Q 3.75 Q			V V V
22+10 22+15 22+20	72.6525 72.6785 72.7061	3.65 Q 3.77 Q 4.02 Q			V V V
22+25 22+30 22+35 22+40	72.7344 72.7619 72.7875 72.8119	4.11 Q 3.99 Q 3.72 Q 3.54 Q			V V V V
22+45 22+50 22+55	72.8356 72.8588 72.8816	3.44 Q 3.37 Q 3.32 Q			
23+ 0 23+ 5 23+10	72.9042 72.9266 72.9488	3.28 Q 3.25 Q 3.22 Q			V V V
23+15 23+20	72.9709 72.9928	3.20 Q 3.19 Q			V V

23+25	73.0147	3.17 Q	1		l	VI
23+30	73.0364	3.16 Q			1	V
23+35	73.0581	3.15 Q		1		VI
23+40	73.0798	3.14 Q			1	V I
23+45	73.1014	3.14 Q	1	1		VI
23+50	73.1229	3.13 Q				VI
23+55	73.1444	3.12 Q		1	a de la companya de l	VI
24+ 0	73.1659	3.12 Q				VI
24+ 5	73.1870	3.07 Q	I		1	V
24+10	73.2070	2.90 Q		* dependent	****	V
24+15	73.2239	2.46 Q				VI
24+20	73.2362	1.79 Q				VI
24+25	73.2454	1.34 Q				VI
24+30	73.2528	1.06 Q		1	1	V
24+35	73.2588	0.87 Q				V
24+40	73.2637	0.72 Q				VI
24+45	73.2679	0.60 Q		I		VI
24+50	73.2714	0.51 Q				V
24+55	73.2744	0.43 Q	-	1		V
25+ 0	73.2769	0.37 Q			1	V I
25+ 5	73.2790	0.31 Q				VI
25+10	73.2809	0.27 Q		1		V I
25+15	73.2825	0.23 Q				VI
25+20	73.2838	0.19 Q	l			VI
25+25	73.2850	0.17 Q				VI
25+30	73.2859	0.14 Q	1	unnage	-	VI
25+35	73.2867	0.12 Q				VI
25+40	73.2874	0.10 Q				V I
25+45	73.2880	0.08 Q	ł	l	1	VI
25+50	73.2885	0.07 Q			ļ	VI
25+55	73.2889	0.06 Q				VI
26+ 0	73.2892	0.05 Q	1	1	annorm	V
26+ 5	73.2895	0.04 Q				V
26+10	73.2898	0.03 Q		l		V
26+15	73.2899	0.02 Q	Lawrence (Construction)	*******		V
26+20	73.2900	0.02 Q	l	I		V
26+25	73.2901	0.01 Q			ALCONO 1	VI
26+30	73.2901	0.01 Q		l		V

2 YEAR – 1 HOUR OFFSITE F₂

```
Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318offsiteF212.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
     St. Frances of Rome - Wildomar
Offsite (F2)
2-Year 1-Hour
Drainage Area = 31.85(Ac.) = 0.050 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 31.85(Ac.) = 0.050 Sq.
Length along longest watercourse = 1903.90(Ft.)
Length along longest watercourse measured to centroid = 626.80(Ft.)
Length along longest watercourse = 0.361 Mi.
Length along longest watercourse measured to centroid = 0.119 Mi.
Difference in elevation = 91.20(Ft.)
Slope along watercourse = 252.9208 Ft./Mi.
Average Manning's 'N' = 0.020
Lag time = 0.051 Hr.
Lag time = 3.04 Min.
25% of lag time = 0.76 Min.
40% of lag time = 1.22 Min.
Unit time = 5.00 Min.
Duration of storm = 1 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
31.85 0.47 14.91
     31.85
              0.47
                                     14.91
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
     31.85
              1.30
                                   41.41
STORM EVENT (YEAR) = 2.00
```

Area Averaged 2-Year Rainfall = 0.468(In) Area Averaged 100-Year Rainfall = 1.300(In) Point rain (area averaged) = 0.468(In) Areal adjustment factor = 99.97 % Adjusted average point rain = 0.468(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 31.850 62.97 0.457 Total Area Entered = 31.85(Ac.) RI RI Infil. Rate Impervious Adj. Infil. Rate Area% F AMC2AMC-1(In/Hr)(Dec.%)(In/Hr)(Dec.)(In/Hr)63.043.00.6410.4570.3771.0000.377 Sum(F) = 0.377Area averaged mean soil loss (F) (In/Hr) = 0.377Minimum soil loss rate ((In/Hr)) = 0.189(for 24 hour storm duration) Soil low loss rate (decimal) = 0.534_____ Slope of intensity-duration curve for a 1 hour storm =0.4800 Unit Hydrograph DESERT S-Curve Unit Hydrograph Data Unit time period Time % of lag Distribution Unit Hydrograph (hrs) Graph % (CFS)

 1
 0.083
 164.503
 35.078
 11.260

 2
 0.167
 329.005
 48.025
 15.415

 3
 0.250
 493.508
 10.944
 3.513

 4
 0.333
 658.010
 3.952
 1.269

 5
 0.417
 822.513
 2.001
 0.642

 Sum = 100.000

Unit	Time	Pattern	Storm Rain	Loss rate(In./Hr)			Effective	
	(Hr.)	Percent	(In/Hr)]	Max	Lo	W	(In/Hr)
1	0.08	4.40	0.247	(0.377)		0.132	0.115
2	0.17	4.50	0.253	(0.377)		0.135	0.118
3	0.25	5.40	0.303	(0.377)		0.162	0.141
4	0.33	5.40	0.303	(0.377)		0.162	0.141
5	0.42	5.70	0.320	(0.377)		0.171	0.149
6	0.50	6.40	0.359	(0.377)		0.192	0.167
7	0.58	7.90	0.444	(0.377)		0.237	0.207
8	0.67	9.10	0.511	(0.377)		0.273	0.238
9	0.75	12.80	0.719		0.377	(0.384)	0.341
10	0.83	25.60	1.437		0.377	(0.768)	1.060
11	0.92	7.90	0.444	(0.377)		0.237	0.207
12	1.00	4.90	0.275	(0.377)		0.147	0.128
		(Loss Rate	e Not Used)					

Flood times Total Total Total Flood	100.0 volume = Effer s area 31 soil loss = soil loss = rainfall = volume = soil loss =	.9(Ac.)/ 0.22 0.575 0.47(29028.	[(In)/((In) (Ac.Ft) In) 9 Cubic	Ft.)] =			3.0 Ft)	
	flow rate of							
			UR	STO	RМ			
	Hydrogr	aph in	5 Mi	nute in	terval	s ((CFS))	
Time(h+m)	Volume Ac.Ft	Q(CFS)	0	7.5		15.0	22.5	30.0
0+30 0+35 0+40 0+45 0+50 0+55 1+ 0 1+ 5 1+10	0.0565 0.0864 0.1179 0.1519 0.1912 0.2376 0.2965 0.4233 0.5632 0.6248 0.6542 0.6638 0.6658	3.10 3.81 4.33 4.58 4.94 5.71 6.74 8.55 18.41 20.31 8.94 4.27 1.39	VQ VQ Q C C VQ VQ VQ VQ VQ VQ VQ VQ VQ VQ	 V	V V	 		V V V V V V V V V V

2 YEAR – 24 HOUR OFFSITE F₂

```
Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318offsiteF2242.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
   English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
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Lag time = 3.04 Min.
25% of lag time = 0.76 Min.
40% of lag time = 1.22 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
31.85 2.23 71 03
                                      71.03
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
                                189.51
              5.95
     31.85
STORM EVENT (YEAR) = 2.00
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Area Averaged 2-Year Rainfall = 2.230(In) Area Averaged 100-Year Rainfall = 5.950(In) Point rain (area averaged) = 2.230(In) Areal adjustment factor = 99.99 % Adjusted average point rain = 2.230(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 31.850 62.97 0.457 Total Area Entered = 31.85(Ac.) RI Infil. Rate Impervious Adj. Infil. Rate Area% RI F
 AMC2 AMC-1
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 63.0
 43.0
 0.641
 0.457
 0.377
 1.000
 0.377
 Sum (F) = 0.377Area averaged mean soil loss (F) (In/Hr) = 0.377Minimum soil loss rate ((In/Hr)) = 0.189 (for 24 hour storm duration) Soil low loss rate (decimal) = 0.534_____ Unit Hydrograph DESERT S-Curve Unit Hydrograph Data _____ Unit time period Time % of lag Distribution Unit Hydrograph Graph % (CFS) (hrs)

 1
 0.083
 164.503
 35.078
 11.260

 2
 0.167
 329.005
 48.025
 15.415

 3
 0.250
 493.508
 10.944
 3.513

 4
 0.333
 658.010
 3.952
 1.269

 5
 0.417
 822.513
 2.001
 0.642

 Sum = 100.000

Unit	Time	Pattern	Storm Rain	I	Loss rate		Effective
	(Hr.)	Percent	(In/Hr)		Max	Low	(In/Hr)
1	0.08	0.07	0.018	(0.669)	0.010	0.008
2	0.17	0.07	0.018	(0.666)	0.010	0.008
3	0.25	0.07	0.018	(0.664)	0.010	0.008
4	0.33	0.10	0.027	(0.661)	0.014	0.012
5	0.42	0.10	0.027	(0.659)	0.014	0.012
6	0.50	0.10	0.027	(0.656)	0.014	0.012
7	0.58	0.10	0.027	(0.653)	0.014	0.012
8	0.67	0.10	0.027	(0.651)	0.014	0.012
9	0.75	0.10	0.027	(0.648)	0.014	0.012
10	0.83	0.13	0.036	(0.646)	0.019	0.017
11	0.92	0.13	0.036	(0.643)	0.019	0.017
12	1.00	0.13	0.036	(0.641)	0.019	0.017
13	1.08	0.10	0.027	(0.638)	0.014	0.012
14	1.17	0.10	0.027	(0.636)	0.014	0.012
15	1.25	0.10	0.027	(0.633)	0.014	0.012

685.670.270.071(0.507)0.0380.0695.750.270.071(0.505)0.0380.0705.830.270.071(0.502)0.0380.0715.920.270.071(0.500)0.0380.0726.000.270.071(0.498)0.0380.0736.080.300.080(0.496)0.0430.0	68 5.6 69 5.7 70 5.8 71 5.9 72 6.0 73 6.0	0.13 0.13 0.17 0.20 0.20 0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.27 0.27 0.27 0.27 0.27 0.27 0.27 <t< th=""><th>0.071 0.071 0.071 0.071 0.080</th><th>(0.505) (0.502) (0.500) (0.498) (0.496)</th><th>0.038 0.038 0.038 0.038 0.043</th><th>0.012 0.012 0.012 0.017 0.017 0.017 0.017 0.017 0.017 0.017 0.017 0.017 0.017 0.021 0.025 0.025 0.025 0.025 0.025 0.025 0.025 0.029 0.033 0.</th></t<>	0.071 0.071 0.071 0.071 0.080	(0.505) (0.502) (0.500) (0.498) (0.496)	0.038 0.038 0.038 0.038 0.043	0.012 0.012 0.012 0.017 0.017 0.017 0.017 0.017 0.017 0.017 0.017 0.017 0.017 0.021 0.025 0.025 0.025 0.025 0.025 0.025 0.025 0.029 0.033 0.
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76 77 78 90 81 82 83 84 85 86 87 89 90 92 93 94 95 97 99 90 101 102 103 104 105 106 107 108 109 111 112 113 114 115 116 117 122 123 124 125 126 127	6.33 6.42 6.50 6.58 6.75 6.83 6.75 6.75 7.00 7.17 7.25 7.33 7.42 7.50 7.58 7.75 7.83 7.92 8.00 8.17 8.25 8.33 8.42 8.58 8.58 8.58 9.00 9.17 9.25 9.332 9.587 9.587 9.253 10.00 10.250 10.558 1	0.30 0.30 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.37 0.37 0.40 0.40 0.40 0.40 0.43 0.43 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.57 0.63 0.63 0.63 0.63 0.63 0.73 0.73 0.73 0.73 0.73 0.50 0.50 0.50 0.57 0.63 0.63 0.63 0.63 0.63 0.63 0.63 0.63 0.50	0.080 0.080 0.089 0.089 0.089 0.089 0.089 0.089 0.089 0.089 0.089 0.089 0.098 0.098 0.098 0.098 0.098 0.107 0.107 0.107 0.107 0.107 0.116 0.116 0.116 0.134 0.134 0.134 0.134 0.143 0.143 0.152 0.152 0.152 0.169 0.169 0.169 0.169 0.169 0.169 0.169 0.169 0.178 0.178 0.187 0.187 0.187 0.187 0.196 0.196 0.134 0	(0.489) (0.487) (0.485) (0.483) (0.480) (0.478) (0.476) (0.472) (0.472) (0.472) (0.463) (0.465) (0.463) (0.461) (0.461) (0.459) (0.457) (0.455) (0.453) (0.451) (0.449) (0.449) (0.442) (0.442) (0.442) (0.438) (0.436) (0.436) (0.432) (0.428) (0.428) (0.422) (0.423) (0.422) (0.422) (0.400) (0.402) (0.393) (0.395) (0.385) (0.383)	0.043 0.043 0.043 0.048 0.048 0.048 0.048 0.048 0.048 0.048 0.048 0.048 0.048 0.052 0.052 0.052 0.057 0.057 0.057 0.057 0.057 0.057 0.062 0.062 0.062 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.076 0.081 0.081 0.081 0.081 0.090 0.090 0.090 0.095 0.095 0.095 0.095 0.095 0.095 0.071 0.071 0.071 0.071 0.075 0.095 0.095 0.095 0.095 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.095 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.095 0.071 0.071 0.071 0.071 0.071 0.095 0.095 0.095 0.095 0.095 0.071 0	0.037 0.037 0.037 0.042 0.042 0.042 0.042 0.042 0.042 0.042 0.042 0.042 0.042 0.046 0.046 0.046 0.046 0.050 0.050 0.050 0.050 0.050 0.054 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.067 0.071 0.071 0.071 0.071 0.071 0.071 0.079 0.079 0.083 0.083 0.083 0.087 0.091 0.091 0.062 0.062 0.062 0.063 0.083 0.087 0.091 0.091 0.091 0.062 0.062 0.062 0.062 0.083 0
123	10.25	0.50	0.134	(0.393)	0.071	0.062
124	10.33	0.50	0.134	(0.391)	0.071	0.062
125	10.42	0.50	0.134	(0.389)	0.071	0.062
126	10.50	0.50	0.134	(0.387)	0.071	0.062

136 137 138 139 140 141 142 143 144 145 146 147 148 149 150 151 152 153 154 155 156 167 168 169 170 172 173 175 176 177 178 180 181 182 183 184 185 186 187 189 191	$\begin{array}{c} 11.33\\ 11.42\\ 11.50\\ 11.58\\ 11.67\\ 11.75\\ 11.83\\ 11.92\\ 12.00\\ 12.08\\ 12.17\\ 12.25\\ 12.33\\ 12.42\\ 12.50\\ 12.58\\ 12.67\\ 12.75\\ 12.83\\ 12.92\\ 13.00\\ 13.08\\ 13.17\\ 13.25\\ 13.33\\ 13.42\\ 13.50\\ 13.67\\ 13.75\\ 13.83\\ 13.92\\ 14.00\\ 14.08\\ 14.17\\ 14.25\\ 14.33\\ 14.42\\ 14.58\\ 14.67\\ 14.75\\ 14.83\\ 14.92\\ 15.00\\ 15.08\\ 15.17\\ 15.25\\ 15.33\\ 15.42\\ 15.50\\ 15.58\\ 15.67\\ 15.58\\ 15.67\\ 15.58\\ 15.67\\ 15.75\\ 15.83\\ 15.92\end{array}$	0.63 0.63 0.57 0.57 0.57 0.60 0.60 0.83 0.83 0.83 0.87 0.93 0.93 0.93 0.97 0.97 0.97 0.97 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.63 0.97 0.77 0.77 0.77 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.90	0.169 0.169 0.152 0.152 0.152 0.152 0.161 0.223 0.223 0.223 0.232 0.232 0.250 0.250 0.259 0.259 0.259 0.259 0.259 0.259 0.259 0.205 0.225 0.265 0.26	(0.368) (0.367) (0.365) (0.361) (0.359) (0.359) (0.359) (0.359) (0.354) (0.352) (0.350) (0.349) (0.347) (0.347) (0.343) (0.342) (0.342) (0.340) (0.338) (0.337) (0.335) (0.333) (0.331) (0.326) (0.323) (0.326) (0.323) (0.322) (0.323) (0.322) (0.323) (0.317) (0.315) (0.312) (0.312) (0.312) (0.312) (0.312) (0.307) (0.307) (0.302) (0.302) (0.301) (0.299) (0.292) (0.292) (0.292) (0.292) (0.284) (0.281) (0.	0.090 0.090 0.090 0.081 0.081 0.081 0.086 0.086 0.119 0.119 0.124 0.124 0.124 0.124 0.133 0.133 0.133 0.138 0.138 0.138 0.162 0.162 0.162 0.162 0.162 0.162 0.162 0.162 0.162 0.162 0.162 0.162 0.162 0.162 0.162 0.162 0.129 0.129 0.129 0.129 0.129 0.129 0.124 0.129 0.110 0.110 0.110 0.110 0.090 0.090 0.090 0.090 0.090 0.090 0.090	0.079 0.079 0.079 0.071 0.071 0.071 0.071 0.075 0.075 0.075 0.104 0.104 0.104 0.108 0.108 0.108 0.108 0.116 0.116 0.121 0.121 0.121 0.121 0.141 0.141 0.141 0.141 0.141 0.96 0.079 0.079 0.
187	15.58	0.63	0.169	(0.284)	0.090	0.079
188	15.67	0.63	0.169	(0.283)	0.090	0.079
189	15.75	0.63	0.169	(0.281)	0.090	0.079

196 197 198 199 200 201 202 203 204 205 206 207 208 209 210 211 212 213 214 215 216 217 218 219 220 221 222 223 224 225 226 227 228 229 230 231 232 233 234 235 236 237 238 239 240 241 242 243 244 245 246	16.33 16.42 16.50 16.58 16.75 16.75 16.83 16.92 17.00 17.08 17.17 17.25 17.33 17.42 17.50 17.58 17.67 17.75 17.83 17.92 18.00 18.08 18.17 18.25 18.33 18.42 18.50 18.58 18.51 18.75 18.83 18.92 19.00 19.08 19.17 19.25 19.33 19.42 19.50 19.50 19.58 19.75 19.67 19.75 19.7	0.13 0.13 0.13 0.10 0.10 0.10 0.10 0.10 0.10 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.13 0.10 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.00 0.10 0.100	0.036 0.036 0.036 0.027 0.027 0.027 0.027 0.027 0.027 0.045 0.045 0.045 0.045 0.045 0.045 0.045 0.045 0.045 0.045 0.045 0.036 0.027 0	(0.271) (0.269) (0.269) (0.266) (0.265) (0.263) (0.262) (0.261) (0.259) (0.259) (0.253) (0.253) (0.254) (0.253) (0.252) (0.244) (0.247) (0.248) (0.247) (0.244) (0.243) (0.242) (0.242) (0.241) (0.239) (0.236) (0.235) (0.235) (0.236) (0.235) (0.235) (0.236) (0.232) (0.232) (0.232) (0.232) (0.223) (0.223) (0.223) (0.222) (0.224) (0.223) (0.222) (0.221) (0.215) (0.214) (0.213)	0.019 0.019 0.014 0.014 0.014 0.014 0.014 0.014 0.024 0.024 0.024 0.024 0.024 0.024 0.024 0.024 0.024 0.024 0.024 0.024 0.029 0.010 0.014 0.01	0.017 0.017 0.012 0.012 0.012 0.012 0.012 0.012 0.012 0.021 0.017 0.012 0.
240	20.00	0.07	0.018	(0.219)	0.010	0.008
241	20.08	0.10	0.027	(0.218)	0.014	0.012
242	20.17	0.10	0.027	(0.217)	0.014	0.012
243	20.25	0.10	0.027	(0.216)	0.014	0.012
244	20.33	0.10	0.027	(0.215)	0.014	0.012

256 257 258 260 261 262 263 264 265 266 267 273 274 275 277 278 280 281 282 284 285 288 285 288	times Total Total Total Flood	0.07 0.07 0.07 0.10 0.10 0.10 0.07	ective rai: 1.9(Ac.)/[1.19(3.160(2.23(I 120137.8	(In)/(In) Ac.Ft) n) Cubic	<pre>0.199) 0.199) 0.198) 0.197) 0.197) 0.196) 0.195) 0.195) 0.194) 0.194) 0.193) 0.193) 0.193) 0.193) 0.193) 0.192) 0.192) 0.191) 0.191) 0.190) 0.190) 0.190) 0.190) 0.189) 0.189) 0.189) 0.189) 0.189) 0.189) 0.189)</pre>			
		flow rate of				539(CFS)		
			24 - H O n o f f	U R H	STOR ydrog	М	+++++++++++++++++++++++++++++++++++++++	·
Tir		Volume Ac.Ft	Q(CFS)	0	2.5	5.0	7.5	10.0
((()+10)+15)+20	0.0006 0.0022 0.0039 0.0060 0.0086 0.0113	0.22 Q 0.25 V 0.31 V 0.38 V	Q Q				

0+35	0.0141	0.40	VQ			No.	1
0+40	0.0168	0.40	VQ			1	
0+45	0.0196	0.40	VQ			1	ļ
0+50	0.0227	0.45	QV	1			1
0+55 1+ 0	0.0262 0.0298	0.51 0.53	V Q V Q				1
1+ 5	0.0331	0.33	VQ		1	1	1
1+10	0.0361	0.42	VQ			1	1
1+15	0.0389	0.41	VQ				
1+20	0.0417	0.40	VQ		Vermover		
1+25	0.0444	0.40	VQ		1		
1+30 1+35	0.0472 0.0499	0.40	VQ				
1+35	0.0527	0.40 0.40	VQ VQ			1	1
1+45	0.0554	0.40	VQ				Ì
1+50	0.0585	0.45	VQ	-	Ì	1	1
1+55	0.0620	0.51	VQ		-	1	1
2+ 0	0.0657	0.53	VQ				
2+ 5 2+10	0.0693 0.0730	0.53 0.53	VQ	1			
2+10	0.0767	0.53	VQ VQ				1
2+20	0.0804	0.53	IVQ	2			
2+25	0.0840	0.53	ĪVQ				
2+30	0.0877	0.53	VQ	1		1	
2+35	0.0917	0.58	VQ		1		1
2+40 2+45	0.0962 0.1007	0.64 0.66	VQ VQ		1		1
2+40	0.1053	0.66	I VQ				1
2+55	0.1099	0.67	IVQ	l			1
3+ 0	0.1145	0.67	I VQ	l		1	
3+ 5	0.1191	0.67	VQ				l
3+10 3+15	0.1237 0.1283	0.67 0.67	VQ				1
3+20	0.1329	0.67	VQ VQ			1	
3+25	0.1375	0.67	VQ	1			1
3+30	0.1420	0.67	I Q		1	l	
3+35	0.1466	0.67	I Q		1		
3+40	0.1512	0.67	I Q				
3+45 3+50	0.1558 0.1608	0.67 0.71	Q Q			1	
3+55	0.1661	0.78	VQ		1		
4+ 0	0.1716	0.79	I VQ		Ì		I
4 + 5	0.1771	0.80	I VQ				
4+10	0.1826	0.80	VQ				
4+15 4+20	0.1881 0.1939	0.80 0.85	VQ VQ	1	1	-	1
4+25	0.2002	0.03	I VQ		1		1
4+30	0.2066	0.93	VQ				Ì
4+35	0.2130	0.93	I Q	1			I
4+40	0.2195	0.93	I Q	1			
4+45	0.2259	0.93	I Q	1			1
4+50 4+55	0.2327 0.2399	0.98 1.05	IQ IVQ	l.	 		1
4+55 5+ 0	0.2472	1.05	I VQ I VQ	t.			1
5+ 5	0.2538	0.97	I Q	a .	1	-	
5+10	0.2597	0.85	ÎQ			4 1	ļ
5+15	0.2653	0.82	I Q		1		l
5+20	0.2712	0.85	Q I	tennoo et			
5+25 5+30	0.2774 0.2838	0.91 0.93	QV QV			1	1
JT30	0.2030	0.93		1	I	I	I

9+35 0.8489 2.71 Q V $ $ $9+40$ 0.8681 2.78 $ $ $ $ QV $ $ $9+45$ 0.8873 2.80 $ $ QV $ $ $9+50$ 0.9069 2.85 $ $ QV $ $ $9+55$ 0.9270 2.91 $ $ QV $ $ $10+0$ 0.9472 2.93 $ $ QV $ $ $10+5$ 0.9651 2.61 Q V $ $ $10+10$ 0.9800 2.16 Q V $ $ $10+15$ 0.9942 2.06 Q V $ $ $10+20$ 1.0081 2.02 Q V $ $ $10+25$ 1.0219 2.00 Q V $ $	9+40 9+45 9+50 9+55 10+ 0 10+ 5 10+10 10+15 10+20	0.8873 0.9069 0.9270 0.9472 0.9651 0.9800 0.9942 1.0081	2.80 2.85 2.91 2.93 2.61 2.16 2.06 2.02	QV Q V Q V Q V Q V Q V Q V Q V			
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10+35 10+40 10+45 10+50 10+55 11+0 11+5 11+10 11+15 11+20 11+25 11+30 11+35 11+40 11+45 11+55 12+0 12+5 12+10 12+15	1.0511 1.0687 1.0868 1.1051 1.1235 1.1418 1.1599 1.1775 1.2125 1.2300 1.2475 1.2643 1.2802 1.2960 1.3120 1.3284 1.3284 1.3448 1.3636 1.3855 1.4081	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
12+20 $12+25$ $12+30$ $12+35$ $12+40$ $12+45$ $12+50$ $12+55$ $13+0$ $13+5$ $13+10$ $13+15$ $13+20$ $13+25$ $13+20$ $13+25$ $13+40$ $13+45$ $13+50$ $13+55$ $13+40$ $13+55$ $13+40$ $13+55$ $13+40$ $13+55$ $13+40$ $13+55$ $13+40$ $14+55$ $14+10$ $14+25$ $14+20$ $14+25$ $14+40$ $14+35$ $14+40$ $14+55$ $14+40$ $14+55$ $14+40$ $14+55$ $14+50$ $14+55$ $15+0$ $15+5$ $15+10$ $15+15$ $15+20$	1.4313 1.4551 1.4789 1.5034 1.5289 1.5545 1.5805 1.6070 1.6336 1.6619 1.6924 1.7234 1.7545 1.7858 1.8170 1.8447 1.8676 1.8893 1.9107 1.9318 1.9530 1.9754 1.9996 2.0242 2.0486 2.0727 2.0967 2.1206 2.1445 2.1684 2.1920 2.2151 2.2381 2.2608 2.3052 2.3269	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\left \begin{array}{cccccccccccccccccccccccccccccccccccc$	

$15+35 \\ 15+40 \\ 15+45 \\ 15+50 \\ 15+55 \\ 16+0 \\ 16+5 \\ 16+10 \\ 16+25 \\ 16+20 \\ 16+25 \\ 16+30 \\ 16+35 \\ 16+40 \\ 16+45 \\ 16+55 \\ 17+0 \\ 17+5 \\ 17+10 \\ 17+15 \\ 17+20 \\ 17+25 \\ 17+30 \\ 17+25 \\ 17+30 \\ 17+55 \\ 17+40 \\ 17+45 \\ 17+50 \\ 17+55 \\ 18+0 \\ 18+5 \\ 18+10 \\ 18+15 \\ 18+20 \\ 18+55 \\ 18+40 \\ 18+45 \\ 18+55 \\ 19+0 \\ 19+55 \\ 19+0 \\ 19+55 \\ 19+40 \\ 19+45 \\ 19+50 \\ 19+55 \\ 20+10 \\ 20+25 \\ 20+20 \\ 20+20 \\ 20+25 \\ 20+20 \\ 20+2$	2.3893 2.4074 2.4251 2.4426 2.4601 2.4775 2.4902 2.4962 2.5007 2.5046 2.5083 2.5120 2.5153 2.5120 2.5153 2.5211 2.5238 2.5224 2.5238 2.5294 2.5238 2.5294 2.5461 2.5507 2.5553 2.5599 2.5645 2.5691 2.5733 2.5772 2.5809 2.5645 2.5993 2.5645 2.5993 2.5645 2.5993 2.6030 2.6063 2.6093 2.6121 2.6184 2.6206 2.6232 2.6259 2.6259 2.6259 2.6361 2.6325 2.6361 2.6325 2.6361 2.6325 2.6476 2.6424 2.6452 2.6476 2.6452 2.6476 2.6451 2.6537 2.6645 2.6645 2.6645 2.6645	$\begin{array}{c} 2.89\\ 2.63\\ 2.57\\ 2.55\\ 2.54\\ 2.54\\ 1.83\\ 0.87\\ 0.65\\ 0.57\\ 0.53\\ 0.49\\ 0.42\\ 0.41\\ 0.40\\ 0.40\\ 0.40\\ 0.40\\ 0.40\\ 0.40\\ 0.62\\ 0.67\\ 0.67\\ 0.67\\ 0.67\\ 0.67\\ 0.67\\ 0.67\\ 0.67\\ 0.67\\ 0.67\\ 0.67\\ 0.67\\ 0.67\\ 0.67\\ 0.67\\ 0.67\\ 0.62\\ 0.53\\$					V V V V V V V V
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20+35	2.6700	0.40 Q	1	1	I V I
20+30	2.6728		1	1	V I
			1	1	
20+45	2.6755	0.40 IQ	1		V I
20+50	2.6780	0.35 Q			V
20+55	2.6800	0.29 Q			V
21+ 0	2.6819	0.27 Q	1	1	V
21+ 5	2.6840	0.32 Q			I V I
21+10	2.6866	0.38 Q			V I
21+15	2.6893	0.39 0		1	V
21+20	2.6918	0.35 Q	1	1	V
			ł	1	I VI
21+25	2.6937	0.29 Q			
21+30	2.6956	0.27 IQ		1	V V
21+35	2.6978	0.32 Q			V
21+40	2.7004	0.38 Q			V
21+45	2.7031	0.39 Q			V
21+50	2.7055	0.35 Q			I VI
21+55	2.7075	0.29 Q	1		V
22+ 0	2.7094	0.27 Q			V
22+ 5	2.7116	0.32 Q		1	V
22+10	2.7142	0.38 Q	1	1	V V
22+15	2.7169	0.39 Q	1	1	I VI
22+15	2.7193		1]	V V
		. –	1		
22+25	2.7213	0.29 IQ	1		V VI
22+30	2.7232	0.27 Q			V
22+35	2.7251	0.27 Q			I VI
22+40	2.7269	0.27 Q			V
22+45	2.7288	0.27 Q			V
22+50	2.7306	0.27 Q			V
22+55	2.7324	0.27 Q			V
23+ 0	2.7343	0.27 Q		1	V
23+ 5	2.7361	0.27 0	i		V VI
23+10	2.7379	0.27 10	1	1	V
23+15	2.7398	$0.27 _{Q}$	1	1	V V
23+20	2.7416	0.27 Q	1		V V
23+25	2.7435	0.27 Q		1	V V
			1	1	I VI
23+30	2.7453	0.27 Q			1 1
23+35	2.7471	0.27 Q			I VI
23+40	2.7490	0.27 Q			V I
23+45	2.7508	0.27 Q		1	V
23+50	2.7527	0.27 Q			V
23+55	2.7545	0.27 Q			V
24+ 0	2.7563	0.27 Q			V
24+ 5	2.7575	0.17 Q			V V
24+10	2.7578	0.05 Q	-		I VI
24+15	2.7579	0.02 Q			V
24+20	2.7580	0.01 Q	1		I V
		¥			· · · · · · · · · · · · · · · · · · ·

10 YEAR – 1 HOUR OFFSITE F₂

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Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318offsiteF2110.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
St. Frances of Rome - Wildomar
Offsite (F2)
10-Year 1-Hour
_____
Drainage Area = 31.85(Ac.) = 0.050 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 31.85(Ac.) = 0.050 Sq.
Length along longest watercourse = 1903.90(Ft.)
Length along longest watercourse measured to centroid = 626.80(Ft.)
Length along longest watercourse = 0.361 Mi.
Length along longest watercourse measured to centroid = 0.119 Mi.
Difference in elevation = 91.20(Ft.)
Slope along watercourse = 252.9208 Ft./Mi.
Average Manning's 'N' = 0.020
Lag time = 0.051 Hr.
Lag time = 3.04 Min.
25% of lag time = 0.76 Min.
40% of lag time = 1.22 Min.
Unit time = 5.00 Min.
Duration of storm = 1 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
     31.85
               0.47
                                     14.91
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
31.85 1.30 41.41
                                    41.41
STORM EVENT (YEAR) = 10.00
```

Area Averaged 2-Year Rainfall = 0.468(In) Area Averaged 100-Year Rainfall = 1.300(In) Point rain (area averaged) = 0.810(In) Areal adjustment factor = 99.97 % Adjusted average point rain = 0.810(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 31.850 62.97 0.457 Total Area Entered = 31.85(Ac.)
 RI
 RI
 Infil. Rate
 Impervious
 Adj. Infil. Rate
 Area%
 F

 AMC2
 AMC-2
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 63.0
 63.0
 0.438
 0.457
 0.258
 1.000
 0.258
 Sum (F) = 0.258Area averaged mean soil loss (F) (In/Hr) = 0.258Minimum soil loss rate ((In/Hr)) = 0.129 (for 24 hour storm duration) Soil low loss rate (decimal) = 0.534 _____ Slope of intensity-duration curve for a 1 hour storm =0.4800 Unit Hydrograph DESERT S-Curve _____ Unit Hydrograph Data Unit time period Time % of lag Distribution Unit Hydrograph Graph % (CFS) (hrs)

 1
 0.083
 164.503
 35.078
 11.260

 2
 0.167
 329.005
 48.025
 15.415

 3
 0.250
 493.508
 10.944
 3.513

 4
 0.333
 658.010
 3.952
 1.269

 5
 0.417
 822.513
 2.001
 0.642

 Sum = 100.000

Unit	Time	Pattern	Storm Rain	L	oss rate	(In.	/Hr)	Effective
	(Hr.)	Percent	(In/Hr)		Max	Lo	W	(In/Hr)
1	0.08	4.40	0.428	(0.258)		0.228	0.199
2	0.17	4.50	0.437	(0.258)		0.234	0.204
3	0.25	5.40	0.525		0.258	(0.280)	0.267
4	0.33	5.40	0.525		0.258	(0.280)	0.267
5	0.42	5.70	0.554		0.258	(0.296)	0.296
6	0.50	6.40	0.622		0.258	(0.332)	0.364
7	0.58	7.90	0.768		0.258	(0.410)	0.510
8	0.67	9.10	0.885		0.258	(0.472)	0.627
9	0.75	12.80	1.244		0.258	(0.664)	0.986
10	0.83	25.60	2.488		0.258	(1.329)	2.231
11	0.92	7.90	0.768		0.258	(0.410)	0.510
12	1.00	4.90	0.476	(0.258)		0.254	0.222
		(Loss Rate	e Not Used)					

Flood times Total Total	100.0 volume = Effe s area 31 soil loss = soil loss = rainfall = volume = soil loss =	.9(Ac.)/[(0.25(1 0.672(<i>P</i>	(In)/(Ft In) Ac.Ft)	.)] =			
Peak	flow rate of	this hydro	graph =	44.742	2(CFS)		
+++++-	++++++++++++ R u	1 – ноц	JR S			+++++++++	+++
	Hydrogr	aph in 5	5 Minu	te interval	ls ((CFS))		
Time(h+m)	Volume Ac.Ft	Q(CFS)	0	12.5	25.0	37.5	50.0
0+5 0+10 0+15 0+20 0+25 0+30 0+35 0+40 0+45 0+50 0+55 1+0 1+5 1+10 1+15 1+20	0.0525 0.0997 0.1554 0.2159 0.2854 0.3743 0.4897 0.6497	5.37 1 6.85 8.10 8.78 10.08 12.92 16.76 23.23 43.43 44.74 19.86 8.68 2.86 0.61 Q	ν Ω ν Ω ν Ω ν Ω νΩ		 	 VQ VQ 	

10 YEAR – 24 HOUR OFFSITE F₂

```
Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318offsiteF22410.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
     English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
_____
St. Frances of Rome - Wildomar
Offsite (F2)
10-Year 24-Hour
             Drainage Area = 31.85(Ac.) = 0.050 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 31.85(Ac.) = 0.050 Sq.
Length along longest watercourse = 1903.90(Ft.)
Length along longest watercourse measured to centroid = 626.80(Ft.)
Length along longest watercourse = 0.361 Mi.
Length along longest watercourse measured to centroid = 0.119 Mi.
Difference in elevation = 91.20(Ft.)
Slope along watercourse = 252.9208 Ft./Mi.
Average Manning's 'N' = 0.020
Lag time = 0.051 Hr.
Lag time = 3.04 Min.
25% of lag time = 0.76 Min.
40% of lag time = 1.22 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
31.85 2.23 71 03
                                      71.03
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
     31.85
              5.95
                                189.51
STORM EVENT (YEAR) = 10.00
```

Area Averaged 2-Year Rainfall = 2.230(In) Area Averaged 100-Year Rainfall = 5.950(In) Point rain (area averaged) = 3.760(In) Areal adjustment factor = 99.99 % Adjusted average point rain = 3.760(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 31.850 62.97 0.457 Total Area Entered = 31.85(Ac.)

 RI
 RI
 Infil. Rate
 Impervious
 Adj. Infil. Rate
 Area%
 F

 AMC2
 AMC-2
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 63.0
 63.0
 0.438
 0.457
 0.258
 1.000
 0.258

 Sum
 (F)
 =
 0.258
 0.258
 0.258
 0.258

 Sum (F) = 0.258Area averaged mean soil loss (F) (In/Hr) = 0.258Minimum soil loss rate ((In/Hr)) = 0.129(for 24 hour storm duration) Soil low loss rate (decimal) = 0.534 Unit Hydrograph DESERT S-Curve Unit Hydrograph Data _____ Unit time period Time % of lag Distribution Unit Hydrograph Graph % (CFS) (hrs)

 1
 0.083
 164.503
 35.078
 11.260

 2
 0.167
 329.005
 48.025
 15.415

 3
 0.250
 493.508
 10.944
 3.513

 4
 0.333
 658.010
 3.952
 1.269

 5
 0.417
 822.513
 2.001
 0.642

 Sum = 100.000 Sum= 32.099

Unit	Time	Pattern	Storm Rain		loss rate		Effective
	(Hr.)	Percent	(In/Hr)		Max	Low	(In/Hr)
1	0.08	0.07	0.030	(0.457)	0.016	0.014
2	0.17	0.07	0.030	(0.455)	0.016	0.014
3	0.25	0.07	0.030	(0.453)	0.016	0.014
4	0.33	0.10	0.045	(0.452)	0.024	0.021
5	0.42	0.10	0.045	(0.450)	0.024	0.021
6	0.50	0.10	0.045	(0.448)	0.024	0.021
7	0.58	0.10	0.045	(0.446)	0.024	0.021
8	0.67	0.10	0.045	(0.445)	0.024	0.021
9	0.75	0.10	0.045	(0.443)	0.024	0.021
10	0.83	0.13	0.060	(0.441)	0.032	0.028
11	0.92	0.13	0.060	(0.439)	0.032	0.028
12	1.00	0.13	0.060	(0.438)	0.032	0.028
13	1.08	0.10	0.045	(0.436)	0.024	0.021
14	1.17	0.10	0.045	(0.434)	0.024	0.021
15	1.25	0.10	0.045	(0.433)	0.024	0.021

16 17 18 90 12 23 45 67 89 01 23 33 33 33 33 44 44 44 45 55 55 55 55 55 66 66 66 66 67 77 23 73	$\begin{array}{c} 1.33\\ 1.42\\ 1.50\\ 1.58\\ 1.67\\ 1.75\\ 1.83\\ 2.00\\ 2.17\\ 2.23\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 2.58\\ 3.67\\ 3.38\\ 3.58\\ 3.67\\ 3.83\\ 2.58\\ 3.67\\ 5.83\\ 2.00\\ 4.08\\ 4.17\\ 4.25\\ 4.33\\ 4.42\\ 5.08\\ 5.17\\ 5.33\\ 2.58\\ 5.67\\ 5.58\\ 5.67\\ 5.89\\ 2.00\\ 6.08\end{array}$	0.10 0.10 0.10 0.10 0.10 0.10 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.20 0.20 0.20 0.20 0.20 0.23 0.27 0.27 0.27 0.27 0.30	0.045 0.045 0.045 0.045 0.045 0.045 0.060 0.060 0.060 0.060 0.060 0.060 0.060 0.060 0.075 0.020 0.090 0.090 0.105 0.10	(0.431) (0.429) (0.427) (0.426) (0.421) (0.421) (0.412) (0.419) (0.416) (0.414) (0.412) (0.411) (0.409) (0.409) (0.407) (0.402) (0.401) (0.399) (0.397) (0.396) (0.394) (0.392) (0.391) (0.389) (0.384) (0.384) (0.383) (0.381) (0.375) (0.375) (0.375) (0.373) (0.375) (0.375) (0.375) (0.373) (0.375) (0.362) (0.362) (0.352) (0.351) (0.342) (0.340) (0.339)	0.024 0.024 0.024 0.024 0.024 0.024 0.032 0.032 0.032 0.032 0.032 0.032 0.032 0.032 0.032 0.032 0.032 0.032 0.032 0.040 0.056 0.064 0.064 0.064 0.064 0.064 0.064 0.064	0.021 0.021 0.021 0.021 0.021 0.021 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.035 0.042 0.042 0.042 0.042 0.049 0.049 0.049 0.049 0.049 0.049 0.049 0.049 0.049 0.049 0.049 0.049 0.049 0.049 0.049 0.049 0.056 0

118 119 120 121 122	6.33 6.42 6.50 6.58 6.75 6.82 7.00 7.17 7.25 7.42 7.58 7.75 7.32 7.58 7.75 7.92 8.08 8.125 8.420 8.58 8.58 8.58 9.008 7.58 8.3200 9.17 9.259 9.58 9.008 9.175 9.58 9.008 9.175 9.58 9.59 9.50	0.30 0.30 0.30 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.37 0.37 0.37 0.40 0.40 0.40 0.40 0.40 0.43 0.43 0.43 0.50 0.50 0.50 0.50 0.50 0.50 0.55 0.57 0.57 0.63 0.63 0.63 0.63 0.63 0.63 0.63 0.63 0.63 0.63 0.63 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.50 0.50 0.57 0.57 0.63 0.63 0.63 0.63 0.63 0.67 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.50 0.50 0.50 0.50 0.50 0.57 0.57 0.63 0.63 0.67 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.50 0.50 0.50 0.50 0.57 0.57 0.63 0.63 0.63 0.67 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.50 0.50 0.50 0.50 0.50 0.50 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.57 0.50 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.50	0.135 0.135 0.135 0.150 0.150 0.150 0.150 0.150 0.150 0.150 0.150 0.165 0.165 0.165 0.165 0.165 0.165 0.165 0.165 0.1226 0.226 0.226 0.226 0.226 0.226 0.2241 0.226 0.226 0.2241 0.2241 0.241 0.241 0.241 0.256 0.226 0.2256 0.226 0.226 0.226 0.226 0.256 0.256 0.256 0.286 0.286 0.286 0.286 0.301 0.301 0.316 0.316 0.331 0.331 0.331 0.226 0.226	(0.334) (0.333) (0.331) (0.323) (0.328) (0.327) (0.325) (0.324) (0.322) (0.321) (0.319) (0.318) (0.317) (0.315) (0.314) (0.312) (0.312) (0.311) (0.309) (0.309) (0.308) (0.307) (0.305) (0.304) (0.302) (0.301) (0.299) (0.298) (0.294) (0.291) (0.291) (0.291) (0.288) (0.281) (0.284) (0.283) (0.284) (0.283) (0.282) (0.284) (0.283) (0.271) (0.271) (0.270) (0.270) (0.270)	0.072 0.072 0.072 0.080 0.080 0.080 0.080 0.080 0.080 0.080 0.080 0.080 0.080 0.088 0.088 0.096 0.096 0.096 0.096 0.096 0.096 0.096 0.104 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.1210 0.120 0.120 0.120 0.120 0.120 0.1210 0.120 0.120 0.1210 0.1210 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.1210 0.120 0.120 0.120 0.120 0.120 0.121 0.121 0.121 0.153 0.153 0.153 0.153 0.161 0.161 0.161 0.161 0.162 0.177 0.177 0.177 0.120	0.063 0.063 0.070 0.070 0.070 0.070 0.070 0.070 0.070 0.070 0.070 0.070 0.077 0.077 0.077 0.077 0.077 0.077 0.077 0.071 0.091 0.091 0.091 0.091 0.091 0.091 0.091 0.105 0.105 0.105 0.105 0.105 0.105 0.105 0.105 0.105 0.105 0.105 0.105 0.105 0.105 0.112 0.112 0.112 0.112 0.112 0.112 0.112 0.112 0.113 0.133 0.133 0.140 0.147 0.147 0.154 0.105 0.105 0.105
117 118 119 120 121	9.75 9.83 9.92 10.00 10.08	0.70 0.73 0.73 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.67 0.67 0.67 0.67 0.67 0.67 0.63	0.316 0.331 0.331 0.331 0.226	(0.276) (0.275) (0.274) (0.272) (0.271)	0.169 0.177 0.177 0.177 0.177 0.120	0.147 0.154 0.154 0.154 0.155

$\begin{array}{c} 136\\ 137\\ 138\\ 141\\ 142\\ 144\\ 145\\ 152\\ 155\\ 155\\ 155\\ 155\\ 155\\ 155\\ 15$	$\begin{array}{c} 11.33\\ 11.42\\ 11.50\\ 11.58\\ 11.67\\ 11.75\\ 11.83\\ 11.92\\ 12.00\\ 12.08\\ 12.17\\ 12.25\\ 12.33\\ 12.42\\ 12.50\\ 12.58\\ 12.67\\ 12.75\\ 12.83\\ 12.92\\ 13.00\\ 13.08\\ 13.17\\ 13.25\\ 13.33\\ 13.42\\ 13.50\\ 13.58\\ 13.67\\ 13.75\\ 13.83\\ 13.92\\ 14.00\\ 14.08\\ 14.17\\ 14.25\\ 13.83\\ 13.92\\ 14.00\\ 14.08\\ 14.7\\ 14.58\\ 13.67\\ 13.75\\ 13.83\\ 13.92\\ 14.00\\ 14.83\\ 14.92\\ 15.00\\ 15.88\\ 15.17\\ 15.25\\ 15.33\\ 15.42\\ 15.50\\ 15.58\\ 15.67\\ 15.75\\ 15.83\\ 15.92\\ 16.00\\ \end{array}$	0.63 0.63 0.63 0.57 0.57 0.57 0.57 0.60 0.60 0.83 0.83 0.83 0.83 0.93 0.93 0.93 0.97 0.97 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.63 0.87 0.87 0.90 0.90 0.90 0.90 0.90 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.63 0.63 0.63 0.63	0.391 0.391 0.421 0.421 0.421 0.421 0.436 0.436 0.511 0.511 0.511 0.511 0.511 0.346 0.346 0.346 0.346 0.346 0.346 0.346 0.346 0.391 0.391 0.391 0.391 0.391 0.391 0.391 0.391 0.391 0.391 0.391 0.391 0.391 0.391 0.376 0.361 0.346 0.346 0.346 0.346 0.346 0.361 0.346 0.346 0.346 0.346 0.346 0.361 0.346 0.346 0.346 0.346 0.346 0.346 0.346 0.346 0.3286 0.286 0.286 0.286 0.286		0.250) 0.249) 0.248) 0.247) 0.246) 0.244) 0.243) 0.242) 0.241) 0.239) 0.238) 0.237) 0.236) 0.235) 0.235) 0.233) 0.232) 0.231) 0.230 0.231) 0.230 0.229 0.228 0.227 0.225 0.224 0.225 0.224 0.225 0.221 0.220) 0.217) 0.216) 0.217) 0.216) 0.215) 0.214) 0.215) 0.214) 0.215) 0.214) 0.210) 0.215) 0.214) 0.210) 0.200) 0.201) 0.200) 0.201) 0.200) 0.201) 0.200) 0.199) 0.197) 0.196) 0.197) 0.191) 0.190) 0.189)	0.153 0.153 0.153 0.137 0.137 0.137 0.145 0.145 0.145 0.201 0.201 0.209 0.209 0.225 0.225 0.225 0.225 0.225 0.223) (0.233) (0.233) (0.273) (0.227) (0.202) (0.209) (0.209) (0.209) (0.209) (0.209) (0.209) (0.209) (0.201) 0.201 0.153 0	0.133 0.133 0.133 0.119 0.119 0.119 0.126 0.126 0.126 0.126 0.175 0.175 0.175 0.175 0.175 0.182 0.182 0.182 0.196 0.206 0.207 0.209 0.285 0.286 0.287 0.288 0.289 0.291 0.161 0.161 0.161 0.161 0.161 0.161 0.161 0.193 0.194 0.195 0.182 0.182 0.182 0.182 0.182 0.182 0.183 0.194 0.195 0.182 0.182 0.182 0.183 0.194 0.175 0.133 0.133 0.133 0.133 0.133
191	15.92	0.63	0.286	((((0.190)	0.153	0.133

$\begin{array}{c} 196\\ 197\\ 198\\ 199\\ 200\\ 201\\ 202\\ 203\\ 204\\ 205\\ 206\\ 207\\ 208\\ 209\\ 211\\ 212\\ 213\\ 214\\ 215\\ 216\\ 217\\ 218\\ 219\\ 220\\ 221\\ 222\\ 223\\ 226\\ 227\\ 228\\ 230\\ 231\\ 235\\ 236\\ 237\\ 238\\ 239\\ 240\\ 241\\ 242\\ 243\\ 245\\ 246\\ 247\\ 248\\ 249\end{array}$	$16.33 \\ 16.42 \\ 16.50 \\ 16.58 \\ 16.67 \\ 16.75 \\ 16.83 \\ 16.92 \\ 17.00 \\ 17.08 \\ 17.17 \\ 17.25 \\ 17.33 \\ 17.42 \\ 17.50 \\ 17.58 \\ 17.67 \\ 17.58 \\ 17.67 \\ 17.75 \\ 17.83 \\ 17.92 \\ 18.00 \\ 18.08 \\ 18.17 \\ 18.25 \\ 18.33 \\ 18.42 \\ 18.50 \\ 18.58 \\ 18.17 \\ 18.25 \\ 18.33 \\ 18.42 \\ 18.50 \\ 18.58 \\ 18.17 \\ 18.25 \\ 18.33 \\ 18.92 \\ 19.00 \\ 19.08 \\ 19.17 \\ 19.25 \\ 19.33 \\ 19.92 \\ 20.00 \\ 19.58 \\ 19.67 \\ 19.58 \\ 10.5$	0.13 0.13 0.13 0.13 0.10 0.10 0.10 0.10 0.10 0.10 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.13 0.10	0.060 0.060 0.045 0.045 0.045 0.045 0.045 0.045 0.045 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.075 0.060 0.060 0.060 0.060 0.060 0.060 0.060 0.060 0.060 0.060 0.060 0.060 0.045 0	$ \left(\begin{array}{c} 0.185 \right) \\ (0.185) \\ (0.185) \\ (0.181) \\ (0.181) \\ (0.181) \\ (0.180) \\ (0.179) \\ (0.179) \\ (0.177) \\ (0.177) \\ (0.176) \\ (0.175) \\ (0.175) \\ (0.175) \\ (0.175) \\ (0.175) \\ (0.177) \\ (0.173) \\ (0.172) \\ (0.171) \\ (0.173) \\ (0.172) \\ (0.171) \\ (0.173) \\ (0.169) \\ (0.169) \\ (0.169) \\ (0.166) \\ (0.166) \\ (0.166) \\ (0.166) \\ (0.166) \\ (0.166) \\ (0.166) \\ (0.166) \\ (0.166) \\ (0.166) \\ (0.161) \\ (0.164) \\ (0.164) \\ (0.162) \\ (0.161) \\ (0.166) \\ (0.162) \\ (0.161) \\ (0.162) \\ (0.161) \\ (0.162) \\ (0.162) \\ (0.162) \\ (0.162) \\ (0.162) \\ (0.162) \\ (0.162) \\ (0.165) \\ (0.165) \\ (0.157) \\ (0.157) \\ (0.157) \\ (0.155) \\ (0.148) \\ (0.146) \\ (0.146) \\ (0.146) \\ (0.145) \\ (0.144 $	0.032 0.032 0.032 0.024 0.024 0.024 0.024 0.024 0.024 0.024 0.040 0.040 0.040 0.040 0.040 0.040 0.040 0.040 0.040 0.040 0.032 0.024 0	0.028 0.028 0.021 0.021 0.021 0.021 0.021 0.021 0.021 0.035 0.028 0.021 0.
245 246 247	20.42 20.50 20.58 20.67	0.10 0.10 0.10 0.10	0.045 0.045 0.045 0.045	(0.146) (0.146) (0.145) (0.145)	0.024 0.024 0.024 0.024	0.021 0.021 0.021 0.021

256 257 258 260 261 262 263 265 266 270 273 274 275 2778 281 283 284 2867 288 3 3 3 3 3 3 3 3	times Total Total Total Flood	0.07 0.07 0.07 0.10 0.10 0.10 0.07	fective rain 31.9(Ac.)/[(1.98(I 5.255(7 3.76(Ir 205812.3	((Ft.)] =			4 4 1 1 1 4 4 4 4 4 4 4 4 4 4 4 4 4
	Peak	flow rate o	f this hydro	ograph	= 9.2	299(CFS)		
	+++++			UR	STORI		+++++++	-+++
		Hydro	graph in 5	5 Min	nute inter	vals ((CFS))		
Tir	ne(h+m)	Volume Ac.F	't Q(CFS)	0	2.5	5.0	7.5	10.0
(((0.37 V 0.42 V 0.52 V 0.64 V	2 2 Q Q				

$\begin{array}{ccccccccc} 0+35 & 0.0237 \\ 0+40 & 0.0284 \\ 0+45 & 0.0330 \\ 0+50 & 0.0382 \\ 0+55 & 0.0442 \\ 1+ & 0 & 0.0503 \\ 1+ & 5 & 0.0559 \\ 1+10 & 0.0656 \\ 1+20 & 0.0702 \\ 1+25 & 0.0749 \\ 1+30 & 0.0795 \\ 1+35 & 0.0842 \\ 1+40 & 0.0888 \\ 1+45 & 0.0935 \\ 1+50 & 0.0987 \\ 1+55 & 0.1046 \\ 2+ & 0 & 0.1107 \\ 2+ & 5 & 0.1046 \\ 2+ & 0 & 0.1107 \\ 2+ & 5 & 0.169 \\ 2+10 & 0.1231 \\ 2+15 & 0.1293 \\ 2+20 & 0.1355 \\ 2+25 & 0.1417 \\ 2+30 & 0.1479 \\ 2+35 & 0.1547 \\ 2+40 & 0.1621 \\ 2+45 & 0.1698 \\ 2+50 & 0.1775 \\ 2+55 & 0.1853 \\ 3+0 & 0.1930 \\ 3+ & 5 & 0.2008 \\ 3+10 & 0.2085 \\ 3+10 & 0.2085 \\ 3+15 & 0.2163 \\ 3+20 & 0.2240 \\ 3+25 & 0.2318 \\ 3+30 & 0.2395 \\ 3+35 & 0.2473 \\ 3+40 & 0.2550 \\ 3+45 & 0.2628 \\ 3+50 & 0.2711 \\ 3+55 & 0.2801 \\ 4+ & 0 & 0.2893 \\ 4+5 & 0.3923 \\ 4+5 & 0.3923 \\ 4+5 & 0.3923 \\ 4+55 & 0.4045 \\ 5+ & 0 & 0.4168 \\ 5+5 & 0.4281 \\ 5+5 $	$\begin{array}{cccccccccccccccccccccccccccccccccccc$		
4+500.39234+550.40455+00.41685+50.4281	1.65 V Q 1.76 V Q 1.79 V Q 1.64 V Q		

5+35	0.4900	1.65		
5+40 5+45	0.5021 0.5144	1.76 1.79		
5+50	0.5268	1.80		
5+55	0.5392	1.80		
6+ 0	0.5516	1.80		
6+ 5	0.5646	1.88	V Q I I I	
6+10	0.5782	1.99	V Q I I I	
6+15	0.5921	2.01	V Q I I	
6+20	0.6060	2.02		
6+25	0.6200	2.03		
6+30 6+35	0.6339 0.6484	2.03 2.10		
6+35 6+40	0.6637	2.10		
6+45	0.6791	2.24		
6+50	0.6946	2.25		
6+55	0.7101	2.25	V Q	
7+ 0	0.7256	2.25	V Q	
7+ 5	0.7411	2.25		
7+10 7+15	0.7566 0.7721	2.25 2.25		
7+13	0.7881	2.23	V Q V Q	
7+25	0.8049	2.44		
7+30	0.8219	2.46	V QI	
7+35	0.8394	2.55	V Q	
7+40	0.8578	2.66		
7+45 7+50	0.8763 0.8954	2.69 2.78		
7+55	0.9153	2.89		
8+ 0	0.9353	2.91		
8+ 5	0.9566	3.08	V Q I I	
8+10	0.9793	3.30		
8+15 8+20	1.0024 1.0255	3.35 3.37	V Q V Q	
8+25	1.0488	3.38		
8+30	1.0721	3.38	V Q	
8+35	1.0959	3.46	V Q I	
8+40	1.1204	3.56	V Q I	
8+45	1.1451 1.1704	3.59 3.68		
8+50 8+55	1.1965	3.00	V Q V O	
9+ 0	1.2228	3.81		
9+ 5	1.2502	3.98	V Q I I	
9+10	1.2791	4.20	V Q I I	
9+15	1.3084	4.25		
9+20 9+25	1.3383 1.3691	4.35 4.46		
9+30	1.4000	4.49		
9+35	1.4315	4.58	I V Q I I	
9+40	1.4638	4.69	V Q	
9+45	1.4963	4.71	V Q I I	
9+50 9+55	1.5293 1.5632	4.80 4.91		
9+55 10+ 0	1.5972	4.91 4.94	V Q V Q	
10+5 10+5	1.6275	4.39		
10+10	1.6525	3.64	I VQ I I	
10+15	1.6764	3.47	I QV I I	
10+20 10+25	1.6999 1.7232	3.41 3.38		
10+25	1.7464	3.38 3.38	QV QV	
10.00		2.00	· · · · · · · · ·	

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$10+35 \\ 10+40 \\ 10+45 \\ 10+50 \\ 10+55 \\ 11+0 \\ 11+5 \\ 11+10 \\ 11+15 \\ 11+20 \\ 11+25 \\ 11+30 \\ 11+35 \\ 11+40 \\ 11+45 \\ 11+55 \\ 12+0 \\ 12+55 \\ 12+10 \\ 12+15 \\ 12+20 \\ 12+25 \\ 12+30 \\ 12+35 \\ 12+40 \\ 12+45 \\ 12+55 \\ 12+50 \\ 12+55 \\ 12+50 \\ 12+55 \\ 13+0 \\ 13+5 \\ 13+10 \\ 10 \\ 10 \\ 10 \\ 10 \\ 10 \\ 10 \\ 10 \\$	1.7724 1.8021 1.8326 1.8635 1.9255 1.9255 1.9560 1.9857 2.0152 2.0447 2.0742 2.1036 2.1320 2.1589 2.1854 2.2124 2.2400 2.2678 2.2995 2.3364 2.3745 2.4136 2.4537 2.4939 2.5352 2.5781 2.6213 2.6655 2.7108 2.7566 2.8085 2.8687	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	V V 	Q Q Q 7 Q 7 Q	
13+20 13+25 13+30 13+35 13+40 13+45 13+50 13+55 14+0 14+5 14+10 14+15 14+20 14+25 14+20 14+25 14+30 14+35 14+40 14+45 14+50 14+55 15+0 15+5 15+10 15+15 15+20 15+25 15+30	2.9942 3.0580 3.1221 3.1762 3.2541 3.2903 3.3260 3.3616 3.3997 3.4413 3.4839 3.5259 3.5667 3.6072 3.6479 3.6887 3.7298 3.7701 3.8093 3.8482 3.8864 3.9239 3.9612 3.9979 4.0338 4.0696	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$			

$15+35 \\ 15+40 \\ 15+45 \\ 15+50 \\ 15+55 \\ 16+0 \\ 16+10 \\ 16+20 \\ 16+25 \\ 16+30 \\ 16+40 \\ 16+45 \\ 16+55 \\ 17+0 \\ 17+5 \\ 17+10 \\ 17+5 \\ 17+10 \\ 17+20 \\ 17+45 \\ 17+50 \\ 17+55 \\ 17+40 \\ 17+45 \\ 17+50 \\ 17+55 \\ 18+10 \\ 18+15 \\ 18+20 \\ 18+35 \\ 18+40 \\ 18+45 \\ 18+55 \\ 19+0 \\ 19+55 \\ 19+10 \\ 19+25 \\ 19+30 \\ 19+45 \\ 19+50 \\ 19+55 \\ 20+10 \\ 20+15 \\ 20+25 \\ 20+10 \\ 20+25 \\ 20+10 \\ 20+25 \\ 20+10 \\ 20+25 \\ 20+30 \\ 19+55 \\ 20+10 \\ 20+25 \\ 20+10 \\ 20+25 \\ 20+10 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 20+25 \\ 20+30 \\ 2$	4.1031 4.1336 4.1634 4.1930 4.2224 4.2519 4.2732 4.2833 4.2909 4.2976 4.3038 4.3100 4.3156 4.3205 4.3253 4.3253 4.3393 4.3450 4.3522 4.3598 4.3598 4.3675 4.3752 4.3830 4.3907 4.3985 4.4062 4.4134 4.4199 4.4262 4.4324 4.4486 4.4448 4.4499 4.4621 4.4691 4.4740 4.4740 4.4788 4.4829 4.4863 4.4894 4.4975 4.5021 4.5072 4.5132 4.5193 4.5249 4.5249 4.5453 4.5453 4.5579 4.55718	$\begin{array}{c} 4.87\\ 4.43\\ 4.33\\ 4.29\\ 4.28\\ 3.09\\ 1.47\\ 1.10\\ 0.97\\ 0.90\\ 0.90\\ 0.90\\ 0.90\\ 0.90\\ 0.90\\ 0.90\\ 0.68\\ 0.68\\ 0.68\\ 0.68\\ 0.68\\ 0.68\\ 0.68\\ 0.68\\ 0.68\\ 0.68\\ 0.68\\ 0.68\\ 0.91\\ 1.12\\ 1.13\\ 1.13\\ 1.13\\ 1.13\\ 1.13\\ 1.13\\ 1.13\\ 1.13\\ 1.13\\ 1.13\\ 1.13\\ 1.13\\ 1.13\\ 1.13\\ 1.65\\ 0.90\\$				
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00.05				1	,		/
20+35	4.5765		I Q			l	V I
20+40	4.5811		I Q				VI
20+45	4.5858		Q				VI
20+50	4.5899	0.60	Q		-		V
20+55	4.5932		ΙQ				V
21+ 0	4.5964	0.46	I Q			l	V
21+ 5	4.6001	0.53	Q		ł		V
21+10	4.6045	0.64	ΙQ				VI
21+15	4.6090	0.66	I Q				VI
21+20	4.6131	0.59	ΙQ	1	1	ł	VI
21+25	4.6165	0.49	I Q	1	1		VI
21+30	4.6197	0.46	1Q		[and the second se	VI
21+35	4.6233	0.53	I Q			1	V
21+40	4.6277	0.64	Q			1	VI
21+45	4.6323	0.66	Q				V
21+50	4.6364	0.59	ÎQ		1		V
21+55	4.6397	0.49	ÌQ				V
22+ 0	4.6429	0.46	ÍQ			1	V
22+ 5	4.6466	0.53	ΪQ			1	V
22+10	4.6510	0.64	ĮQ				V
22+15	4.6555	0.66	ĮQ		1	1	V
22+20	4.6596	0.59	ĮQ	1		1	V
22+25	4.6630	0.49	IQ		1		V I
22+20	4.6662	0.46	IQ		1	1	V I
22+35	4.6693	0.45	IQ		1		V I
22+30	4.6724	0.45	IQ			1	V I
22+45	4.6755	0.45	10	1	1	I	V
22+50	4.6786	0.45	Q	1	1	1	V
22+55	4.6817	0.45	IQ	1		1	VI
23+ 0	4.6848	0.45	Q		1	1	V
23+ 5	4.6879	0.45	10	1	-	1	VI
23+10	4.6910	0.45		1		1	V
23+15	4.6941	0.45	IQ			1	VI
23+20	4.6972	0.45	Q			Ĩ	V
23+25	4.7003	0.45	IQ	1	1		V
23+30	4.7034	0.45	IQ		1		v
23+35	4.7065	0.45	IQ	1	1	1	vi
23+40	4.7096	0.45	IQ	1	1		V
23+45	4.7127	0.45	IQ	1		1	VI
23+50	4.7158	0.45	IQ	I	1	1	VI
23+55	4.7189	0.45		1	1	l.	VI
23+33 24+ 0	4.7220	0.45	IQ	1	1	1	VI
24 + 0 24 + 5	4.7240	0.45	IQ IO	1	1	1	VI
			IQ	1	1	1	,
24+10	4.7246	0.08	Q	1	1		V
24+15	4.7247	0.03	Q	1		1	V V
24+20	4.7248	0.01	Q	I	I	1	V
were break profe warer break blank minis anna anna, anna anna a							

100 YEAR – 1 HOUR OFFSITE F₂

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Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318offsiteF21100.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
St. Frances of Rome - Wildomar
Offsite (F2)
100-Year 1-Hour
              _____
Drainage Area = 31.85(Ac.) = 0.050 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 31.85(Ac.) = 0.050 Sq.
Length along longest watercourse = 1903.90(Ft.)
Length along longest watercourse measured to centroid = 626.80(Ft.)
Length along longest watercourse = 0.361 Mi.
Length along longest watercourse measured to centroid = 0.119 Mi.
Difference in elevation = 91.20(Ft.)
Slope along watercourse = 252.9208 Ft./Mi.
Average Manning's 'N' = 0.020
Lag time = 0.051 Hr.
Lag time = 3.04 Min.
25% of lag time = 0.76 Min.
40% of lag time = 1.22 Min.
Unit time = 5.00 Min.
Duration of storm = 1 \text{ Hour}(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
31.85 0.47 14 01
                                      14.91
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
     31.85
              1.30
                                  41.41
STORM EVENT (YEAR) = 100.00
```

Area Averaged 2-Year Rainfall = 0.468(In) Area Averaged 100-Year Rainfall = 1.300(In) Point rain (area averaged) = 1.300(In) Areal adjustment factor = 99.97 % Adjusted average point rain = 1.300(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 31.850 62.97 0.457 Total Area Entered = 31.85(Ac.)

 RI
 RI
 Infil. Rate
 Impervious
 Adj. Infil. Rate
 Area%
 F

 AMC2
 AMC-2
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 63.0
 63.0
 0.438
 0.457
 0.258
 1.000
 0.258

 Sum (F) = 0.25

 Sum (F) = 0.258Area averaged mean soil loss (F) (In/Hr) = 0.258Minimum soil loss rate ((In/Hr)) = 0.129 (for 24 hour storm duration) Soil low loss rate (decimal) = 0.534_____ Slope of intensity-duration curve for a 1 hour storm =0.4800 _____ Unit Hydrograph DESERT S-Curve _____ Unit Hydrograph Data Unit time period Time % of lag Distribution Unit Hydrograph (hrs) Graph % (CFS) _____

 1
 0.083
 164.503
 35.078
 11.260

 2
 0.167
 329.005
 48.025
 15.415

 3
 0.250
 493.508
 10.944
 3.513

 4
 0.333
 658.010
 3.952
 1.269

 5
 0.417
 822.513
 2.001
 0.642

 Sum = 100.000

Unit	Time	Pattern	Storm Rain	Loss rate(In./Hr)			Effective
	(Hr.)	Percent	(In/Hr)	Max	Low	ī	(In/Hr)
1	0.08	4.40	0.686	0.258	(0.366)	0.428
2	0.17	4.50	0.702	0.258	(0.375)	0.444
3	0.25	5.40	0.842	0.258	(0.450)	0.584
4	0.33	5.40	0.842	0.258	(0.450)	0.584
5	0.42	5.70	0.889	0.258	(0.475)	0.631
6	0.50	6.40	0.998	0.258	(0.533)	0.740
7	0.58	7.90	1.232	0.258	(0.658)	0.974
8	0.67	9.10	1.419	0.258	(0.758)	1.161
9	0.75	12.80	1.996	0.258	(1.066)	1.738
10	0.83	25.60	3.992	0.258	(2.132)	3.735
11	0.92	7.90	1.232	0.258	(0.658)	0.974
12	1.00	4.90	0.764	0.258	(0.408)	0.506
		(Loss Rate	e Not Used)				

Flood times Total Total Total Flood	100.0 volume = Effe s area 31 soil loss = soil loss = rainfall = volume = soil loss =	.9(Ac.)/[0.26(0.684(1.30(I 120450.4	(In)/(Ft. In) Ac.Ft) n) Cubic Fe)] =	In)	12.5 .Ft)	
Peak	flow rate of	this hydr	ograph =	7.6.	786(CFS)		
+++++	++++++++++++++++++++++++++++++++++++++	1 - H O	+++++++++ URS Hyc	TORM		+++++++++++++++++++++++++++++++++++++++	+++++
	Hydrogr	aph in	5 Minut	e inter	vals ((CFS	;))	
Time(h+m)	Volume Ac.Ft	Q(CFS)	0	20.0	40.0	60.0	80.0
0+40 0+45 0+50 0+55 1+ 0 1+ 5 1+10	0.2161 0.3380 0.4689 0.6146 0.7918 1.0114 1.3025 1.8168 2.3457 2.5992 2.7169 2.7542 2.7629	11.61 14.94 17.70 21.16 25.73 31.89 42.27 74.68 76.79 36.81 17.09 5.42		 2 Q VQ VQ 	V IQ V IQ V I V I V I V I V I V I V I V I V I V I	7 	

100 YEAR – 24 HOUR OFFSITE F₂

```
Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318offsiteF224100.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
   English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
    St. Frances of Rome - Wildomar
Offsite (F2)
100-Year 24-Hour
_____
Drainage Area = 31.85(Ac.) = 0.050 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 31.85(Ac.) = 0.050 Sq.
Length along longest watercourse = 1903.90(Ft.)
Length along longest watercourse measured to centroid = 626.80(Ft.)
Length along longest watercourse = 0.361 Mi.
Length along longest watercourse measured to centroid = 0.119 Mi.
Difference in elevation = 91.20(Ft.)
Slope along watercourse = 252.9208 Ft./Mi.
Average Manning's 'N' = 0.020
Lag time = 0.051 Hr.
Lag time = 3.04 Min.
25% of lag time = 0.76 Min.
40% of lag time = 1.22 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
31.85 2.23 71 03
     31.85
               2.23
                                      71.03
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
     31.85
                                189.51
              5.95
STORM EVENT (YEAR) = 100.00
```

Area Averaged 2-Year Rainfall = 2.230(In) Area Averaged 100-Year Rainfall = 5.950(In) Point rain (area averaged) = 5.950(In) Areal adjustment factor = 99.99 % Adjusted average point rain = 5.950(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 31.850 62.97 0.457 Total Area Entered = 31.85(Ac.)

 RI
 RI
 Infil. Rate
 Impervious
 Adj. Infil. Rate
 Area%
 F

 AMC2
 AMC-2
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 63.0
 63.0
 0.438
 0.457
 0.258
 1.000
 0.258

 Sum
 (F)
 0.258
 0.258
 0.258

 Sum (F) = 0.258Area averaged mean soil loss (F) (In/Hr) = 0.258Minimum soil loss rate ((In/Hr)) = 0.129(for 24 hour storm duration) Soil low loss rate (decimal) = 0.534 Unit Hydrograph DESERT S-Curve Unit Hydrograph Data _____ Unit time period Time % of lag Distribution Unit Hydrograph (hrs) Graph % (CFS)

 1
 0.083
 164.503
 35.078
 11.260

 2
 0.167
 329.005
 48.025
 15.415

 3
 0.250
 493.508
 10.944
 3.513

 4
 0.333
 658.010
 3.952
 1.269

 5
 0.417
 822.513
 2.001
 0.642

 Sum = 100.000

 11.260

Unit	Time	Pattern	Storm Rain	I	loss rate		Effective
	(Hr.)	Percent	(In/Hr)		Max	Low	(In/Hr)
1	0.08	0.07	0.048	(0.457)	0.025	0.022
2	0.17	0.07	0.048	(0.455)	0.025	0.022
3	0.25	0.07	0.048	(0.453)	0.025	0.022
4	0.33	0.10	0.071	(0.452)	0.038	0.033
5	0.42	0.10	0.071	(0.450)	0.038	0.033
6	0.50	0.10	0.071	(0.448)	0.038	0.033
7	0.58	0.10	0.071	(0.446)	0.038	0.033
8	0.67	0.10	0.071	(0.445)	0.038	0.033
9	0.75	0.10	0.071	(0.443)	0.038	0.033
10	0.83	0.13	0.095	(0.441)	0.051	0.044
11	0.92	0.13	0.095	(0.439)	0.051	0.044
12	1.00	0.13	0.095	(0.438)	0.051	0.044
13	1.08	0.10	0.071	(0.436)	0.038	0.033
14	1.17	0.10	0.071	(0.434)	0.038	0.033
15	1.25	0.10	0.071	(0.433)	0.038	0.033

16 17 18 90 12 23 45 67 89 01 23 45 67 89 0 12 34 56 7 89 0 12 34 56 7 89 0 12 34 55 55 55 55 55 55 55 55 55 55 55 55 55	$\begin{array}{c} 1.33\\ 1.42\\ 1.50\\ 1.58\\ 1.67\\ 1.75\\ 1.82\\ 2.08\\ 2.17\\ 2.25\\ 2.33\\ 2.58\\ 7.5\\ 2.58\\ 7.5\\ 2.58\\ 7.5\\ 2.58\\ 7.5\\ 2.58\\ 7.5\\ 3.32\\ 3.58\\ 3.67\\ 3.82\\ 4.08\\ 4.17\\ 4.25\\ 4.33\\ 4.42\\ 5.00\\ 5.17\\ 5.23\\ 5.58\\ 5.67\\ 5.75\\ 5.33\\ 2.58\\ 5.67\\ 5.75\\ 5.33\\ 2.58\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75\\ 5.67\\ 5.75$	0.10 0.10 0.10 0.10 0.10 0.10 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.20 0.20 0.20 0.20 0.20 0.23	0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.119 0.143 0.143 0.167 0.167 0.167 0.167 0.167 0.167 0.190 0.190 0.190 0.190	(0.431) (0.429) (0.427) (0.426) (0.421) (0.422) (0.421) (0.419) (0.419) (0.416) (0.414) (0.412) (0.411) (0.409) (0.407) (0.407) (0.402) (0.401) (0.399) (0.397) (0.396) (0.394) (0.394) (0.384) (0.384) (0.384) (0.375) (0.375) (0.375) (0.375) (0.371) (0.372) (0.375) (0.375) (0.372) (0.371) (0.362) (0.362) (0.351) (0.351) (0.348) (0.346) (0.345)	0.038 0.038 0.038 0.038 0.038 0.038 0.051 0.051 0.051 0.051 0.051 0.051 0.051 0.051 0.051 0.064 0.076 0.076 0.076 0.076 0.076 0.089 0	0.033 0.033 0.033 0.033 0.033 0.033 0.033 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.055 0.057 0.067 0.067 0.067 0.067 0.078 0.089 0
64	5.33	0.23	0.167	(0.353)	0.089	0.078
65	5.42	0.23	0.167	(0.351)	0.089	0.078
66	5.50	0.23	0.167	(0.349)	0.089	0.078
67	5.58	0.27	0.190	(0.348)	0.102	0.089
68	5.67	0.27	0.190	(0.346)	0.102	0.089

76 77 89 80 81 82 83 84 85 87 89 90 92 93 94 95 97 99 90 101 102 103 104 105 107 108 100 111 112 114 115 1167 1121 122 124 125 127 128	6.33 6.42 6.50 6.58 6.75 6.83 6.75 6.83 7.00 7.17 7.23 7.58 7.75 7.83 7.67 7.75 8.00 8.17 8.25 8.42 8.58 8.58 8.58 9.00 9.17 9.23 9.23 9.58 9.23 9.58 9.23 9.58 9.23 9.58 9.23 9.58 9.23 9.58 9.25 9.23 9.58 9.25 9.23 9.58 9.25 9.23 9.58 9.25 9.23 9.58 9.25 9.23 9.58 9.25 9.23 9.58 9.25 9.23 9.58 9.25 10.05 10.5	0.30 0.30 0.30 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.37 0.37 0.40 0.40 0.40 0.40 0.43 0.43 0.50 0.50 0.50 0.50 0.50 0.50 0.57 0.63 0.63 0.67 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.50 0.50 0.57 0.63 0.63 0.63 0.63 0.63 0.63 0.63 0.63 0.50 0.50 0.50 0.50 0.50 0.50 0.57 0.63 0.63 0.63 0.63 0.63 0.67 0.70 0.73 0.73 0.50	0.214 0.214 0.214 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.262 0.262 0.262 0.262 0.262 0.262 0.262 0.309 0.309 0.309 0.357 0.357 0.357 0.357 0.357 0.476 0.476 0.357 0	$ \left(\begin{array}{c} 0.334 \right) \\ (0.333 \right) \\ (0.331 \right) \\ (0.330 \right) \\ (0.321 \right) \\ (0.322 \right) \\ (0.322) \\ (0.322) \\ (0.321) \\ (0.322) \\ (0.321) \\ (0.312) \\ (0.317) \\ (0.315) \\ (0.317) \\ (0.315) \\ (0.314) \\ (0.312) \\ (0.311) \\ (0.309) \\ (0.309) \\ (0.308) \\ (0.307) \\ (0.305) \\ (0.307) \\ (0.305) \\ (0.307) \\ (0.305) \\ (0.307) \\ (0.305) \\ (0.307) \\ (0.305) \\ (0.307) \\ (0.305) \\ (0.307) \\ (0.305) \\ (0.307) \\ (0.305) \\ (0.307) \\ (0.306) \\ (0.307) \\ (0.299) \\ (0.299) \\ (0.297) \\ (0.291) \\ (0.288) \\ (0.287) \\ (0.284) \\ (0.283) \\ (0.284) \\ (0.283) \\ (0.276) \\ (0.276) \\ (0.277) \\ (0.276) \\ (0.277) \\ (0.276) \\ (0.277) \\ (0.276) \\ (0.277) \\ (0.268) \\ (0.267) \\ (0.263) \\ (0.263) \\ (0.262) \\ \end{array} $	0.114 0.114 0.127 0.127 0.127 0.127 0.127 0.127 0.127 0.127 0.127 0.127 0.127 0.127 0.127 0.123 0.153 0.153 0.153 0.165 0.165 0.165 0.165 0.165 0.165 0.165 0.165 0.191 0.191 0.191 0.191 0.203 0.203 0.203 0.203 0.216 0.216 0.216 0.216 0.216 0.216 0.216 0.241 0.241 0.241 0.241 0.241 0.241 0.254 0.267 (0.280) (0.280) (0.280) (0.291 0.191 0.254 0.254	0.100 0.100 0.100 0.111 0.111 0.111 0.111 0.111 0.111 0.111 0.122 0.122 0.122 0.122 0.122 0.133 0.133 0.133 0.144 0.144 0.166 0.166 0.166 0.166 0.166 0.166 0.166 0.177 0.177 0.177 0.177 0.177 0.189 0.211 0.211 0.221 0.222 0.222 0.233 0.233 0.233 0.249 0.250 0.166 0.166 0.166 0.166 0.166 0.166 0.1222 0.222 0.251 0.166 0.166 0.166 0.166 0.166 0.166 0.166 0.166 0.166 0.166 0.166 0.166 0.166 0.166 0.166 0.166 0.166 0.166 0.166 0.222 0.222
123	10.25	0.50	0.357	(0.268)	0.191	0.166
124	10.33	0.50	0.357	(0.267)	0.191	0.166
125	10.42	0.50	0.357	(0.266)	0.191	0.166
126	10.50	0.50	0.357	(0.264)	0.191	0.166

$\begin{array}{c} 136\\ 137\\ 138\\ 139\\ 140\\ 141\\ 142\\ 143\\ 144\\ 145\\ 146\\ 147\\ 148\\ 149\\ 150\\ 151\\ 152\\ 155\\ 1567\\ 1589\\ 161\\ 162\\ 163\\ 166\\ 167\\ 172\\ 173\\ 176\\ 177\\ 178\\ 1881\\ 182\\ 188\\ 188\\ 188\\ 188\\ 188\\ 1$	$\begin{array}{c} 11.33\\ 11.42\\ 11.50\\ 11.58\\ 11.67\\ 11.75\\ 11.83\\ 11.92\\ 12.00\\ 12.08\\ 12.17\\ 12.25\\ 12.33\\ 12.42\\ 12.50\\ 12.58\\ 12.67\\ 12.75\\ 12.83\\ 12.92\\ 13.00\\ 13.08\\ 13.17\\ 13.25\\ 13.33\\ 13.42\\ 13.50\\ 13.58\\ 13.67\\ 13.75\\ 13.83\\ 13.92\\ 14.00\\ 14.08\\ 14.17\\ 14.25\\ 13.83\\ 13.92\\ 14.00\\ 14.08\\ 14.17\\ 14.25\\ 13.83\\ 13.92\\ 14.00\\ 14.08\\ 14.75\\ 13.83\\ 13.92\\ 14.00\\ 14.58\\ 13.67\\ 15.58\\ 15.67\\ 15.58\\ 15.58\\ 15.58\\ 15.58\\ 15.58\\ 15.58\\ 15.58\\ 15.58\\ 15.75\\ 15.83\\ 15.92\\ 15.92\\ 15$	0.63 0.63 0.57 0.57 0.57 0.60 0.60 0.83 0.83 0.83 0.83 0.87 0.93 0.93 0.93 0.97 0.97 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.13 1.677 0.77 0.77 0.77 0.77 0.90 0.90 0.90 0.87 0.83 0.83 0.83 0.83 0.63	0.452 0.452 0.452 0.405 0.405 0.405 0.428 0.428 0.595 0.595 0.595 0.619 0.619 0.666 0.666 0.666 0.666 0.690 0.809 0.809 0.809 0.809 0.809 0.809 0.809 0.547 0.547 0.547 0.547 0.547 0.547 0.547 0.547 0.543 0.619 0.619 0.619 0.619 0.619 0.619 0.595 0.59	(0.252 $)($ 0.250 $)($ 0.249 $)($ 0.248 $)($ 0.247 $)($ 0.246 $)($ 0.244 $)($ 0.243 $)($ 0.242 $)0.2410.2390.2380.2370.2360.2350.2330.2220.2310.2200.2290.2280.2270.2250.2240.2220.2210.2200.2190.2170.2160.2150.2140.2150.2140.2130.2120.2110.2200.2110.2200.2120.2110.2200.2120.2120.2120.2110.2200.2020.2010.2020.2010.2020.2010.2020.1990.1950.1910.190$	0.241 0.241 0.216 0.216 0.229 0.229 0.229 0.229 0.229 (0.318) (0.318) (0.330) (0.330) (0.330) (0.356) (0.356) (0.369) (0.369) (0.432) (0.432) (0.432) (0.432) (0.432) (0.432) (0.292) (0.292) (0.292) (0.292) (0.292) (0.292) (0.292) (0.292) (0.292) (0.343) (0.330) (0.330) (0.330) (0.330) (0.330) (0.330) (0.330) (0.330) (0.330) (0.330) (0.330) (0.330) (0.318) (0.318) (0.318) (0.318) (0.305) (0.292) (0.292) (0.292) (0.292) (0.292) (0.330) (0.330) (0.330) (0.330) (0.318) (0.305) (0.292) (0.292) (0.292) (0.292) (0.292) (0.292) (0.305) (0.292) (0.292) (0.292) (0.292) (0.305) (0.305) (0.292) (0.241) (0.24	0.211 0.211 0.211 0.211 0.189 0.200 0.200 0.200 0.200 0.354 0.356 0.357 0.382 0.383 0.433 0.433 0.433 0.434 0.435 0.461 0.462 0.583 0.584 0.585 0.586 0.587 0.588 0.328 0.329 0.331 0.332 0.331 0.332 0.333 0.430 0.431 0.432 0.409 0.410 0.411 0.412 0.413 0.431 0.332 0.331 0.332 0.331 0.332 0.331 0.332 0.331 0.332 0.331 0.332 0.331 0.332 0.331 0.332 0.331 0.332 0.333 0.430 0.411 0.422 0.393 0.394 0.371 0.352 0.258 0.259 0.260 0.261 0.262
188	15.67	0.63	0.452	0.193	(0.241)	0.259
189	15.75	0.63	0.452	0.192	(0.241)	0.260
190	15.83	0.63	0.452	0.191	(0.241)	0.261

196 197 198 200 202 203 205 207 208 210 212 213 214 216 223 224 226 228 230 233 235 237 238 241 245 245 245 245 245 245 245 245 245 245	16.33 16.42 16.50 16.58 16.75 16.75 16.83 16.92 17.00 17.08 17.17 17.25 17.33 17.42 17.50 17.58 17.67 17.75 17.83 17.92 18.00 18.08 18.17 18.25 18.33 18.42 18.50 18.58 18.75 18.83 18.92 19.00 19.08 19.17 19.25 19.33 19.42 19.50 19.58 19.67 19.75 19.58 19.92 20.00 20.08 20.17 20.25 20.33 20.42 20.50 20.75 20.83 20.67 20.75 20.83 20.67 20.75 20.83 20.67 20.75 20.83 20.67 20.75 20.83 20.67 20.75 20.83 20.67 20.75 20.83 20.67 20.75 20.83 20.67 20.75 20.83 20.67 20.75 20.83 20.67 20.83 20.82 20.83 20.8	0.13 0.13 0.13 0.10 0.10 0.10 0.10 0.10 0.10 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.13 0.10 0.10 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.10 0.07	0.095 0.095 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.119 0.095 0.071 0.07	$ \left(\begin{array}{c} 0.185 \right) \\ (0.185) \\ (0.185) \\ (0.181) \\ (0.182) \\ (0.181) \\ (0.180) \\ (0.179) \\ (0.179) \\ (0.177) \\ (0.176) \\ (0.177) \\ (0.175) \\ (0.175) \\ (0.175) \\ (0.174) \\ (0.173) \\ (0.172) \\ (0.171) \\ (0.172) \\ (0.172) \\ (0.171) \\ (0.170) \\ (0.169) \\ (0.169) \\ (0.166) \\ (0.166) \\ (0.166) \\ (0.166) \\ (0.166) \\ (0.166) \\ (0.166) \\ (0.161) \\ (0.160) \\ (0.160) \\ (0.157) \\ (0.157) \\ (0.157) \\ (0.155) \\ (0.148) \\ (0.147) \\ (0.146) \\ (0.146) \\ (0.144) \\ (0.144) \\ (0.143) \\ (0.1$	0.051 0.051 0.038 0.038 0.038 0.038 0.038 0.038 0.038 0.038 0.038 0.064 0.064 0.064 0.064 0.064 0.064 0.064 0.051 0.051 0.051 0.051 0.051 0.051 0.051 0.051 0.051 0.051 0.051 0.038 0	0.044 0.044 0.033 0.033 0.033 0.033 0.033 0.033 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.044 0.033 0
247	20.58	0.10	0.071	(0.145)	0.038	0.033
248	20.67	0.10	0.071	(0.145)	0.038	0.033
249	20.75	0.10	0.071	(0.144)	0.038	0.033

256 257 258 259 260 261 262 263 264 265 266 267 268 269 270 271 272 273 274 275 276 277 278 279 280 281 282 283 284 285 286 287 288	Flood		0.048 0.071 0.071 0.071 0.048 0.048 0.048 0.071 0.071 0.071 0.048 0.	<pre>(0.136) (0.135) (0.135) (0.134) (0.134) (0.134) (0.133) (0.133) (0.132) (0.132) (0.132) (0.132) (0.131) (0.131) (0.131) (0.131) (0.130) (0.130) (0.130) (0.130) (0.129) (0.129) (0.129) (0.129)</pre>			2 2 3 3 3 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		
	Total	soil loss =	1.9(Ac.)/[(In 2.73(In) 7.236(Ac.))/(Ft.)] =	8.6(AC.E	(t)			
	Total	rainfall =	5.95(In)						
		volume = soil loss =	372685.9 Cul 315183.6	Cubic Feet					
	Peak flow rate of this hydrograph = 18.862(CFS)								
++++++++++++++++++++++++++++++++++++++									
Hydrograph in 5 Minute intervals ((CFS))									
				5.0	10.0	15.0	20.0		
)+ 5)+10)+15)+20)+25	0.0017 0.0058 0.0104 0.0161 0.0230							

0+33 0.0375 1.06 V Image: Constraint of the second	a a -							
$\begin{array}{c c c c c c c c c c c c c c c c c c c $								
$\begin{array}{c c c c c c c c c c c c c c c c c c c $				7 Q	and the second se			
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	0+45				1			
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	0+50	0.0605	1.19 \	7 Q				
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	0+55	0.0699	1.36 \	7 Q				
	1+ 0	0.0795	1.40 \	7 Q		1		
	1+ 5	0.0884	1.29 \	/Q		1		1
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	1+10	0.0962	1.13 \	/Q				1
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	1+15	0.1037	1.09 \	7 Q			1	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	1+20	0.1111	1.08 \	7 Q			Res and a second se	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	1+25	0.1185	1.07 \	7 Q		*		
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	1+30	0.1259				1		1
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	1+35	0.1332	1.07 1	7 Q				1
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	1+40	0.1406	1.07 \	/ Q		l		l
1+55 0.1655 1.36 V 0 1 1 2+0 0.1752 1.40 V 0 1 1 2+10 0.1948 1.42 V 0 1 1 2+15 0.2046 1.42 V 0 1 1 2+20 0.2144 1.42 IVQ 1 1 1 2+30 0.2340 1.42 IVQ 1 1 1 2+30 0.2447 1.55 IVQ 1 1 1 2+45 0.2667 1.76 IVQ 1 1 1 2+55 0.2992 1.78 IVQ 1 1 1 3+10 0.3054 1.78 IVQ 1 1 1 3+20 0.3667 1.78 <t< td=""><td>1+45</td><td>0.1479</td><td>1.07 1</td><td>/ Q</td><td></td><td></td><td></td><td>1</td></t<>	1+45	0.1479	1.07 1	/ Q				1
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	1+50	0.1561	1.19 \	J Q				
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	1+55	0.1655	1.36 \	V Q			1	1
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$\begin{array}{c c c c c c c c c c c c c c c c c c c $	2+ 5	0.1850	1.42	J Q	1	l		
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	2+10	0.1948	1.42	V Q	ł			
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$\begin{array}{c c c c c c c c c c c c c c c c c c c $	2+20	0.2144	1.42	I VQ				
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4+35 0.5684 2.49 V Q 4+40 0.5856 2.49 V Q 4+45 0.6027 2.49 V Q 4+50 0.6208 2.62 V Q 4+55 0.6400 2.79 V Q 5+0 0.6594 2.83 V Q 5+5 0.6773 2.59 V Q 5+10 0.6928 2.26 VQ 5+15 0.7078 2.18 VQ 5+20 0.7235 2.28 VQ 5+25 0.7403 2.43 VQ							- -	ĺ
4+40 0.5856 2.49 V Q 4+45 0.6027 2.49 V Q 4+50 0.6208 2.62 V Q 4+55 0.6400 2.79 V Q 5+0 0.6594 2.83 V Q 5+5 0.6773 2.59 V Q 5+10 0.6928 2.26 VQ 5+15 0.7078 2.18 VQ 5+20 0.7235 2.28 VQ 5+25 0.7403 2.43 VQ							l	1
4+45 0.6027 2.49 VQ 4+50 0.6208 2.62 VQ 4+55 0.6400 2.79 VQ 5+0 0.6594 2.83 VQ 5+5 0.6773 2.59 VQ 5+10 0.6928 2.26 VQ 5+10 0.6928 2.26 VQ 5+15 0.7078 2.18 VQ 5+20 0.7235 2.28 VQ 5+25 0.7403 2.43 VQ	4+40				1			I
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5+ 0 0.6594 2.83 V Q 5+ 5 0.6773 2.59 V Q 5+10 0.6928 2.26 VQ 5+15 0.7078 2.18 VQ 5+20 0.7235 2.28 VQ 5+25 0.7403 2.43 VQ		0.6400			-	l	l	I
5+50.67732.59VVIIII5+100.69282.26VQIIII5+150.70782.18VQIII5+200.72352.28VQIII5+250.74032.43VQIII						l	1	1
5+15 0.7078 2.18 VQ 5+20 0.7235 2.28 VQ 5+25 0.7403 2.43 VQ	5+ 5	0.6773			l	1		
5+15 0.7078 2.18 VQ 5+20 0.7235 2.28 VQ 5+25 0.7403 2.43 VQ	5+10	0.6928	2.26	I VQ	-			
5+25 0.7403 2.43 VQ	5+15	0.7078	2.18					1
	5+20	0.7235	2.28					1
5+30 0.7573 2.47 VQ							1	
	5+30	0.7573	2.47	I VQ		1	1	

5+35 5+40 5+45 5+50 5+55 6+0 6+10 6+25 6+30 6+25 6+35 6+40 6+45 6+55 7+0 7+10 7+25 7+30 7+45 7+55 8+10 8+120 8+25 8+10 8+25 8+30 8+15 8+25 8+30 8+35 8+35 8+45 8+55 8+35 8+55 8+35 8+55 8+35 8+555 8+555 8+555 8+555 8+555 8+555 8+555 8+555 8+555 8+555 8+555 8+555 8+555 8+555 8+555 8+555 8+5555 8+5555 8+5555 8+5555 8+5555 8+5555 8+5555 8+5555 8+5555 8+5555 8+55555 8+55555 8+555555555555555555555555555555555555	0.7753 0.7945 0.8140 0.8335 0.8532 0.8728 0.9728 0.9728 0.9369 0.9369 0.9369 0.9589 0.9810 1.0030 1.0260 1.0501 1.0745 1.0990 1.1235 1.1480 1.1725 1.1971 1.2216 1.2470 1.2736 1.3004 1.3282 1.3572 1.3865 1.4168 1.4482 1.4800 1.5135 1.5495 1.5860 1.6227 1.6595 1.6963 1.7339 1.7728 1.8119 1.8519	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		
7+35		4.04			
7+45	1.3865	4.25	V Q I		
8+ 0	1.4800	4.61	V QI		
			—		
8+20	1.6227	5.33	V Q		
			—		
8+35	1.7339		V Q		
8+50 8+55	1.8519 1.8932	5.82 5.99		1	
9+ 0	1.9348	6.03	V I Q		
9+ 5 9+10	1.9781 2.0239	6.30 6.65	V Q V Q		
9+15	2.0702	6.72	I VI Q		
9+20 9+25	2.1176 2.1662	6.88 7.06	V Q V Q		
9+30	2.2151	7.10	V Q		
9+35 9+40	2.2650 2.3161	7.24 7.42	V Q V Q		
9+45	2.3675	7.46	I IV Q		
9+50 9+55	2.4202 2.4747	7.65 7.92	V Q V Q		
10+ 0 10+ 5	2.5298 2.5787	8.01 7.09	V Q V Q	1	an a
10+10	2.6186	5.80	I IQV		
10+15 10+20	2.6565 2.6937	5.50 5.40			
10+25	2.7305	5.34	Q V		
10+30	2.7673	5.34	I Q V	l	

0+35 0+40 0+45 0+50 0+55 1+0 1+5 1+10 1+25 1+20 1+25 1+30 1+45 1+40 1+45 1+55 2+0 2+55 2+10 2+25 2+30 2+45 2+25 2+35 2+40 2+55 2+45 2+55 2+40 2+55 2+45 2+55 13+0 13+25 13+25 13+25 13+40 13+55 13+40 13+55 13+40 13+55 13+40 13+55 13+40 13+55 13+40 13+55 13+40 13+55 13+40 13+55 13+40 13+55 13+40 13+55 13+40 13+55 13+40 13+55 13+40 13+55 13+40 13+55 13+40 13+55 13+40 13+55 13+55 13+55 13+40 13+55 15+50 15+55 15+10 15+555 15+10 15+555 15+10 15+5555 15+10 15+555555555	2.8084 2.8554 2.9037 2.9525 3.0016 3.0506 3.0988 3.1458 3.1926 3.2393 3.2859 3.325 3.3773 3.4199 3.4619 3.5045 3.5483 3.5923 3.6484 3.7211 3.7977 3.8778 3.9614 4.0458 4.1343 4.2283 4.3237 4.4216 4.5225 4.6243 4.3237 4.4216 4.5225 4.6243 4.7357 4.8602 4.9878 5.1167 5.2463 5.3762 5.4861 5.5683 5.6445 5.7187 5.7919 5.8654 5.9465 6.0380 6.1320 6.2252 6.3166 6.4076 6.4987 6.5899 6.6813 6.7711 6.8586 6.9458 7.0311 7.1966 7.2774 7.3557 7.4337	V V V V V	I Q I Q V Q I Q I V I V I V I V I V I V I V I V I V I Q I <th></th>	
2.8554 2.9037 2.9525 3.0016 3.0506 3.0988 3.1458 3.1926 3.2393 3.2859 3.325 3.3773 3.4199 3.4619 3.5045 3.5483 3.5923 3.6484 3.7211 3.7977 3.8778 3.9614 4.0458 4.0458 4.0458 4.1343 4.2283 4.2283 4.2283 4.2283 4.2283 4.2283 4.2283 4.2283 4.2283 4.2283 4.5225 4.6243 4.7357 4.8602 4.9878 5.1167 5.2463 5.3762 5.4861 5.5683 5.6445 5.7187 5.7919 5.8654 5.9465 6.0380 6.1320 6.2252 6.3166 6.4076 6.4987 6.5899 6.6813 6.7711 6.8586 7.0311 7.1140 7.1967 7.2774 7.3557			Q VQ VQ Q Q QV QV QV Q	$ \begin{bmatrix} 0 \\ 1 \\ 0 \\$
2.8554 6.82 $ $ 2.9037 7.02 $ $ 2.9525 7.09 $ $ 3.0016 7.12 $ $ 3.0988 7.00 $ $ 3.1458 6.83 $ $ 3.1458 6.83 $ $ 3.1458 6.83 $ $ 3.2393 6.77 $ $ 3.2393 6.77 $ $ 3.2359 6.77 $ $ 3.325 6.77 $ $ 3.3773 6.52 $ $ 3.4199 6.18 $ $ 3.4419 6.10 $ $ 3.5045 6.19 $ $ 3.5483 6.35 $ $ 3.5923 6.39 $ $ 3.6484 8.15 3.7211 10.55 3.7977 11.13 3.8778 11.63 3.9614 12.13 4.0458 12.26 4.1343 12.86 4.2283 13.65 4.3237 13.85 4.4216 14.22 4.5225 14.65 4.6243 14.77 4.7357 16.18 4.8602 18.07 4.9878 18.52 5.1167 18.71 5.2463 18.83 5.3762 18.86 5.4861 15.95 5.683 11.95 5.6445 11.06 5.7187 10.77 5.7919 10.63 5.8654 10.67 5.9465 11.78 6.0380 13.28 <td>6.82 7.02 7.09 7.12 7.12 7.00 6.83 6.77 6.77 6.77 6.77 6.52 6.10 6.35 6.39 8.15 10.55 11.13 12.26 13.65 13.65 13.65 14.22 14.65 14.77 16.18 18.07 18.52 11.95 11.06 15.95 11.06 10.77 10.63 11.78 13.22 13.</td> <td></td> <td> Q VQ Q Q Q QV QV QV QV</td> <td>$\begin{bmatrix} 0 & & & \\ VQ & & \\ VQ & & \\ Q & \\ Q & & \\ Q & \\$</td>	6.82 7.02 7.09 7.12 7.12 7.00 6.83 6.77 6.77 6.77 6.77 6.52 6.10 6.35 6.39 8.15 10.55 11.13 12.26 13.65 13.65 13.65 14.22 14.65 14.77 16.18 18.07 18.52 11.95 11.06 15.95 11.06 10.77 10.63 11.78 13.22 13.		Q VQ Q Q Q QV QV QV QV	$ \begin{bmatrix} 0 & & & \\ VQ & & \\ VQ & & \\ Q & \\ Q & & \\ Q & \\$
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	6.82 Q 7.02 VQ 7.02 VQ 7.09 VQ 7.12 Q 7.00 QV 6.83 QV 6.77 QV 6.77 QV 6.77 QV 6.18 QV 6.19 QV 6.35 QV 6.39 Q V 10.55 V Q 11.63 V Q 12.26 V Q 12.86 V Q 12.86 V Q 14.65 V Q 14.65 V Q 16.18 V	$\left[\begin{array}{c} Q \\ & Q \\ $		

15+35 7.5044 10.25 15+40 7.6635 8.80 15+55 7.6815 8.41 15+55 7.7392 8.38 16+ 7.7971 8.41 16+ 7.8383 5.97 16+ 7.8383 5.97 16+15 7.8689 1.42 16+20 7.8797 1.57 16+30 7.8993 1.42 16+45 7.9235 1.09 16+50 7.9383 1.07 17+0 7.9457 1.07 17+10 7.9662 1.66 17+20 7.9393 1.77 17+35 8.0271 1.78 </th <th>Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V I V V I V V I V V I V V I V V I V V I V V I V V I V V I V V I V V</th>	Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V Q I V I V V I V V I V V I V V I V V I V V I V V I V V I V V I V V I V V
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20+35	8.3210	1.07	I Q		Vooren		V I
20+40	8.3283	1.07	I Q				V
20+45	8.3357	1.07	ΙQ	1	1	1	V I
20+50	8.3422	0.94	Q	l			VI
20+55	8.3475	0.77	I Q				VI
21+ 0	8.3526	0.73	I Q		Yuuuu 40.	1	VI
21+ 5	8.3584	0.84	I Q	1		1	VI
21+10	8.3653	1.01	Q				VI
21+15	8.3725	1.05	Q				V
21+20	8.3790	0.94	IQ		I		V
21+25	8.3843	0.77	Q				VI
·21+30	8.3893	0.73	10				V
21+35	8.3952	0.84	ÍQ		-		VI
21+40	8.4021	1.01	I Q			1	VI
21+45	8.4093	1.05	ÌQ	1			V
21+50	8.4158	0.94	ÌQ	1	l		VI
21+55	8.4211	0.77	Q		1		VI
22+ 0	8.4261	0.73	ÌQ				V
22+ 5	8.4320	0.84	IQ		1		V
22+10	8.4389	1.01	ΙQ		1		VI
22+15	8.4461	1.05	ÌQ		1		VI
22+20	8.4526	0.94	Q				V
22+25	8.4579	0.77	I Q	1		!	VI
22+30	8.4629	0.73	I Q		l	1	VI
22+35	8.4679	0.72	I Q		l		VI
22+40	8.4728	0.71	Q		1		VI
22+45	8.4777	0.71	Q		1		VI
22+50	8.4826	0.71	I Q		l	I	VI
22+55	8.4875	0.71	Q		1		VI
23+ 0	8.4924	0.71	I Q				VI
23+ 5	8.4973	0.71	Q				VI
23+10	8.5022	0.71	Q		l		VI
23+15	8.5071	0.71	I Q				VI
23+20	8.5120	0.71	IQ	l			VI
23+25	8.5169	0.71	IQ				V I
23+30	8.5219	0.71	Q		-		VI
23+35	8.5268	0.71	IQ				VI
23+40	8.5317	0.71	ΙQ				VI
23+45	8.5366	0.71	Q				V
23+50	8.5415	0.71	IQ				VI
23+55	8.5464	0.71	IQ	I			VI
24+ 0	8.5513	0.71	lQ	1			VI
24+ 5	8.5545	0.46	Q				VI
24+10	8.5553	0.12	Q				VI
24+15	8.5556	0.04	Q				VI
24+20	8.5557	0.01	Q		1		V

100 YEAR – 1 HOUR OFFSITE F₄

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Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318offsiteF41100.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
St. Frances of Rome - Wildomar
Offsite (F4)
100-Year 1-Hour
Drainage Area = 4.79(Ac.) = 0.007 Sg. Mi.
Drainage Area for Depth-Area Areal Adjustment = 4.79(Ac.) = 0.007 Sq.
Length along longest watercourse = 1644.50(Ft.)
Length along longest watercourse measured to centroid = 913.10(Ft.)
Length along longest watercourse = 0.311 Mi.
Length along longest watercourse measured to centroid = 0.173 Mi.
Difference in elevation = 76.00(Ft.)
Slope along watercourse = 244.0134 Ft./Mi.
Average Manning's 'N' = 0.020
Lag time = 0.056 Hr.
Lag time = 3.34 Min.
25% of lag time = 0.83 Min.
40% of lag time = 1.34 Min.
Unit time = 5.00 Min.
Duration of storm = 1 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
      4.79
               0.47
                                      2.24
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
4.79 1.30 6.23
                                   6.23
STORM EVENT (YEAR) = 100.00
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Mi.

Area Averaged 2-Year Rainfall = 0.468(In) Area Averaged 100-Year Rainfall = 1.300(In) Point rain (area averaged) = 1.300(In) Areal adjustment factor = 100.00 % Adjusted average point rain = 1.300(In) Sub-Area Data: Area(Ac.)Runoff IndexImpervious %4.79056.610.496 Total Area Entered = 4.79(Ac.) RI RI Infil. Rate Impervious Adj. Infil. Rate Area% F AMC2AMC-2(In/Hr)(Dec.%)(In/Hr)(Dec.)(In/Hr)56.656.60.5050.4960.2791.0000.279 Sum (F) = 0.279Area averaged mean soil loss (F) (In/Hr) = 0.279Minimum soil loss rate ((In/Hr)) = 0.140 (for 24 hour storm duration) Soil low loss rate (decimal) = 0.503 Slope of intensity-duration curve for a 1 hour storm =0.4800Unit Hydrograph DESERT S-Curve Unit Hydrograph Data Unit time period Time % of lag Distribution Unit Hydrograph (hrs) Graph % (CFS) 1.526 2.363 0.578 0.225 0.094 0.042 Sum = 100.000 Sum = 4.827

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit	Time	Pattern	Storm Rain	Loss rate(Loss rate(In./Hr)		
	(Hr.)	Percent	(In/Hr)	Max	Low	(In/Hr)	
1	0.08	4.40	0.686	0.279	(0.345)	0.407	
2	0.17	4.50	0.702	0.279	(0.353)	0.423	
3	0.25	5.40	0.842	0.279	(0.424)	0.563	
4	0.33	5.40	0.842	0.279	(0.424)	0.563	
5	0.42	5.70	0.889	0.279	(0.447)	0.610	
6	0.50	6.40	0.998	0.279	(0.502)	0.719	
7	0.58	7.90	1.232	0.279	(0.620)	0.953	
8	0.67	9.10	1.420	0.279	(0.714)	1.140	
9	0.75	12.80	1.997	0.279	(1.004)	1.717	
10	0.83	25.60	3.993	0.279	(2.009)	3.714	
11	0.92	7.90	1.232	0.279	(0.620)	0.953	
12	1.00	4.90	0.764	0.279	(0.384)	0.485	

Flood times Total Total Total Flood	(Loss Rate No 100.0 volume = Effect s area 4 soil loss = soil loss = rainfall = volume = soil loss =	ctive rainfa .8(Ac.)/[(In 0.28(In 0.112(Ac 1.30(In) 17746.0 C	n)/(Ft.)) .Ft) ubic Fee] = t			
	flow rate of 	 ++++++++++++++++++++++++++++++++		+++++++++++++++++++++++++++++++++++++++		· + + + + + + + + + + + + + + + + + + +	 ++++
	Ru	1 – H O U n o f f			a p h		
	Hydrogr	aph in 5	Minute	interva	ls ((CFS))		
Time(h+m)	Volume Ac.Ft	Q(CFS) 0		5.0	10.0	15.0	20.0
0+ 5 0+10 0+15 0+20 0+25 0+30 0+35 0+40 0+45 0+50 0+55 1+ 0 1+ 5 1+10 1+15 1+20 1+25	0.0659 0.0869 0.1124 0.1443 0.1864 0.2600 0.3400 0.3790 0.3979	1.61 V 2.09 V 2.53			 		

100 YEAR – 24 HOUR OFFSITE F₄

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Unit Hydrograph Analysis
    Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
         Study date 07/23/19 File: c318offsiteF424100.out
Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978
Program License Serial Number 6222
_____
English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used
English Units used in output format
_____
St. Frances of Rome - Wildomar
Offsite (F4)
100-Year 24-Hour
Drainage Area = 4.79(Ac.) = 0.007 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 4.79(Ac.) = 0.007 Sq.
Length along longest watercourse = 1644.50(Ft.)
Length along longest watercourse measured to centroid = 913.10(Ft.)
Length along longest watercourse = 0.311 Mi.
Length along longest watercourse measured to centroid = 0.173 Mi.
Difference in elevation = 76.00(Ft.)
Slope along watercourse = 244.0134 Ft./Mi.
Average Manning's 'N' = 0.020
Lag time = 0.056 Hr.
Lag time = 3.34 Min.
25% of lag time = 0.83 Min.
40% of lag time = 1.34 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)
2 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
      4.79
              2.23
                                 10.68
100 YEAR Area rainfall data:
Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
      4.79
              5.95
                                     28.50
STORM EVENT (YEAR) = 100.00
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Mi.

Area Averaged 2-Year Rainfall = 2.230(In) Area Averaged 100-Year Rainfall = 5.950(In) Point rain (area averaged) = 5.950(In) Areal adjustment factor = 100.00 % Adjusted average point rain = 5.950(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 4.790 56.61 0.496 Total Area Entered = 4.79(Ac.) RI RI Infil. Rate Impervious Adj. Infil. Rate Area% F AMC2AMC-2(In/Hr)(Dec.%)(In/Hr)(Dec.)(In/Hr)56.656.60.5050.4960.2791.0000.279 Sum (F) = 0.279 Area averaged mean soil loss (F) (In/Hr) = 0.279Minimum soil loss rate ((In/Hr)) = 0.140(for 24 hour storm duration) Soil low loss rate (decimal) = 0.503 _____ Unit Hydrograph DESERT S-Curve _____ Unit Hydrograph Data _____ Unit time period Time % of lag Distribution Unit Hydrograph (hrs) Graph % (CFS) _ _ _ _ _ _ _ _ _ _ _ 1.526 2.363 0.578 0.225 0.094 0.042 Sum = 100.000 Sum= 4.827 _____

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit	Time	Pattern	Storm Rain]	Loss rate		Effective
	(Hr.)	Percent	(In/Hr)		Max	Low	(In/Hr)
1	0.08	0.07	0.048	(0.495)	0.024	0.024
2	0.17	0.07	0.048	(0.493)	0.024	0.024
3	0.25	0.07	0.048	(0.491)	0.024	0.024
4	0.33	0.10	0.071	(0.489)	0.036	0.035
5	0.42	0.10	0.071	(0.488)	0.036	0.035
6	0.50	0.10	0.071	(0.486)	0.036	0.035
7	0.58	0.10	0.071	(0.484)	0.036	0.035
8	0.67	0.10	0.071	(0.482)	0.036	0.035
9	0.75	0.10	0.071	(0.480)	0.036	0.035
10	0.83	0.13	0.095	(0.478)	0.048	0.047
11	0.92	0.13	0.095	(0.476)	0.048	0.047
12	1.00	0.13	0.095	(0.474)	0.048	0.047
13	1.08	0.10	0.071	(0.472)	0.036	0.035
14	1.17	0.10	0.071	(0.471)	0.036	0.035

16 17 18 9 21 22 34 56 7 8 9 0 12 33 35 37 8 9 0 12 34 56 7 8 9 0 12 34 55 55 55 55 55 55 55 55 55 55 55 55 55	$\begin{array}{c} 1.25\\ 1.33\\ 1.42\\ 1.58\\ 1.67\\ 1.58\\ 1.67\\ 1.82\\ 2.08\\ 2.23\\ 2.42\\ 2.58\\ 7.53\\ 2.67\\ 2.34\\ 2.58\\ 7.53\\ 2.67\\ 2.34\\ 2.58\\ 7.53\\ 2.67\\ 2.34\\ 2.58\\ 7.53\\ 2.67\\ 3.67\\ 3.32\\ 3.58\\ 3.67\\ 3.75\\ 3.892\\ 4.08\\ 4.17\\ 4.33\\ 4.92\\ 0.87\\ 5.33\\ 2.58\\ 7.5\\ 5.58\\$	0.10 0.10 0.10 0.10 0.10 0.10 0.10 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.20 0.27 0.27 0.27 0.27	0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.071 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.095 0.119 0.190 0.19	$ \left(\begin{array}{c} 0.469 \right) \\ (0.467) \\ (0.465) \\ (0.463) \\ (0.461) \\ (0.459) \\ (0.458) \\ (0.456) \\ (0.456) \\ (0.452) \\ (0.450) \\ (0.445) \\ (0.445) \\ (0.445) \\ (0.447) \\ (0.445) \\ (0.443) \\ (0.443) \\ (0.439) \\ (0.438) \\ (0.436) \\ (0.434) \\ (0.432) \\ (0.432) \\ (0.432) \\ (0.422) \\ (0.422) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.420) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.420) \\ (0.422) \\ (0.420) \\ (0.422) \\ (0.422) \\ (0.420) \\ (0.422) \\ (0.422) \\ (0.420) \\ (0.422) \\ (0.420) \\ (0.422) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.420) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.423) \\ (0.422) \\ (0.404) \\ (0.404) \\ (0.404) \\ (0.404) \\ (0.402) \\ (0.399) \\ (0.397) \\ (0.397) \\ (0.395) \\ (0.384) \\ (0.382) \\ (0.382) \\ (0.374) \\ (0.372) \\ (0.370) \\ \end{array} $	0.036 0.036 0.036 0.036 0.036 0.036 0.036 0.036 0.048 0.048 0.048 0.048 0.048 0.048 0.048 0.048 0.048 0.048 0.048 0.048 0.048 0.048 0.060 0.072 0.096 0.096 0.096 0.096 0.096 0.096 0.096	0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.047 0.059 0.095 0.095 0.095 0.095 0.095 0.095

75 76 77 80 81 82 83 84 85 86 87 89 90 92 93 94 95 97 99 90 101 102 103 104 105 106 107 108 109 110 112 113 114 115 116 127 128 120 121 122 124 126 127 128 129 131	6.25 6.33 6.42 6.50 6.58 6.75 6.75 6.83 6.75 7.00 7.17 7.25 7.32 7.58 7.75 7.83 7.92 8.00 8.17 8.25 8.33 8.42 8.58 8.58 8.58 8.58 9.00 9.17 9.25 9.23 9.25 9.587 9.23 9.25 10.00 10.25 10.25 10.58 10.587 10.75 1	0.30 0.30 0.30 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.33 0.37 0.40 0.40 0.40 0.40 0.43 0.43 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.57 0.63 0.67 0.70 0.73 0.73 0.73 0.73 0.73 0.63 0.67 0.50 0.67 0.50 0.50 0.50 0.70 0.73 0.73 0.73 0.50 0.50 0.50 0.50 0.67	0.214 0.214 0.214 0.214 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.238 0.262 0.262 0.262 0.262 0.262 0.286 0.286 0.309 0.309 0.357 0.357 0.357 0.357 0.357 0.357 0.476 0	(0.364) (0.362) (0.361) (0.359) (0.357) (0.356) (0.354) (0.353) (0.351) (0.349) (0.348) (0.346) (0.346) (0.343) (0.341) (0.340) (0.338) (0.337) (0.337) (0.335) (0.334) (0.329) (0.328) (0.324) (0.323) (0.321) (0.322) (0.324) (0.323) (0.321) (0.322) (0.311) (0.312) (0.315) (0.312) (0.312) (0.312) (0.312) (0.312) (0.312) (0.309) (0.309) (0.302) (0.301) (0.299) (0.292) (0.294) (0.292) (0.280) (0.281) (0.	0.108 0.108 0.108 0.108 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.120 0.132 0.132 0.132 0.132 0.132 0.132 0.144 0.144 0.144 0.144 0.156 0.156 0.156 0.156 0.156 0.180 0.180 0.180 0.192 0.204 0.204 0.204 0.204 0.227 0.227 0.227 0.227 0.2239 0.2	0.106 0.106 0.106 0.106 0.106 0.118 0.118 0.118 0.118 0.118 0.118 0.118 0.118 0.118 0.130 0.130 0.130 0.130 0.142 0.142 0.142 0.142 0.142 0.154 0.154 0.154 0.177 0.177 0.177 0.177 0.177 0.177 0.177 0.201 0.201 0.201 0.201 0.201 0.201 0.225 0.237 0.237 0.237 0.237 0.237 0.237 0.237 0.237 0.237
127 128 129	10.67 10.75	0.67 0.67	0.476 0.476	(0.285) (0.284) (0.282)	0.239 0.239	0.237 0.237

$\begin{array}{c} 135\\ 136\\ 137\\ 138\\ 139\\ 140\\ 141\\ 142\\ 143\\ 144\\ 145\\ 146\\ 147\\ 148\\ 150\\ 151\\ 152\\ 153\\ 156\\ 157\\ 158\\ 160\\ 161\\ 162\\ 163\\ 166\\ 167\\ 168\\ 169\\ 177\\ 178\\ 176\\ 177\\ 178\\ 179\\ 180 \end{array}$	$11.25 \\ 11.33 \\ 11.42 \\ 11.50 \\ 11.58 \\ 11.67 \\ 11.75 \\ 11.83 \\ 11.92 \\ 12.00 \\ 12.08 \\ 12.17 \\ 12.25 \\ 12.33 \\ 12.42 \\ 12.50 \\ 12.58 \\ 12.67 \\ 12.75 \\ 12.83 \\ 12.42 \\ 12.50 \\ 12.58 \\ 12.67 \\ 12.75 \\ 12.83 \\ 12.92 \\ 13.00 \\ 13.08 \\ 13.17 \\ 13.25 \\ 13.33 \\ 13.42 \\ 13.50 \\ 13.58 \\ 13.67 \\ 13.75 \\ 13.83 \\ 13.92 \\ 14.00 \\ 14.08 \\ 14.17 \\ 14.25 \\ 14.33 \\ 14.42 \\ 14.50 \\ 14.58 \\ 14.67 \\ 14.55 \\ 14.83 \\ 14.92 \\ 15.00 \\ 15.0$	0.63 0.63 0.63 0.57 0.57 0.57 0.57 0.60 0.60 0.83 0.83 0.83 0.83 0.87 0.93 0.93 0.93 0.93 0.97 0.97 0.97 1.13 0.77 0.77 0.77 0.77 0.77 0.90 0.90 0.87 0.83 0.93	0.452 0.452 0.452 0.452 0.405 0.405 0.405 0.428 0.428 0.595 0.595 0.595 0.619 0.619 0.666 0.666 0.666 0.666 0.666 0.690 0.690 0.809 0.809 0.809 0.809 0.809 0.809 0.809 0.547 0.595 0.595 0.595	(0.274) (0.273) (0.271) (0.270) (0.269) (0.266) (0.265) (0.263) (0.262) 0.261 0.259 0.258 0.257 0.256 0.254 0.252 0.250 0.249 0.248 0.247 0.248 0.247 0.245 0.244 0.243 0.242 0.242 0.240 0.238 0.237 0.238 0.231 0.230 0.231 0.230 0.223 0.231 0.222 0.225 0.224 0.231 0.223 0.222 0.225 0.224	0.227 0.227 0.227 0.227 0.227 0.204 0.204 0.204 0.215 0.215 0.215 (0.299) (0.299) (0.311) (0.311) (0.311) (0.311) (0.335) (0.335) (0.335) (0.337) (0.347) (0.347) (0.347) (0.347) (0.347) (0.407) (0.407) (0.407) (0.407) (0.407) (0.407) (0.407) (0.407) (0.275) (0.323) (0.311) (0.311) (0.311) (0.311) (0.299) (0.299) (0.299) (0.299)	0.225 0.225 0.225 0.225 0.201 0.201 0.201 0.213 0.213 0.334 0.336 0.337 0.362 0.363 0.413 0.415 0.416 0.441 0.442 0.444 0.564 0.565 0.566 0.567 0.569 0.566 0.567 0.569 0.565 0.566 0.567 0.569 0.567 0.569 0.570 0.309 0.311 0.312 0.313 0.314 0.315 0.412 0.413 0.315 0.412 0.313 0.314 0.315 0.412 0.313 0.314 0.393 0.394 0.395 0.376 0.377
172 173 174 175 176 177 178	14.33 14.42 14.50 14.58 14.67 14.75 14.83	0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.83	0.619 0.619 0.619 0.619 0.619 0.619 0.619	0.227 0.226 0.225 0.224 0.223 0.222 0.222	(0.311) (0.311) (0.311) (0.311) (0.311) (0.311) (0.299)	0.391 0.393 0.394 0.395 0.396 0.397 0.375

195 16.25 196 16.33 197 16.42 198 16.50	0.13 0.13 0.13 0.13	0.095 0.095 0.095 0.095	(0.202) (0.201) (0.200) (0.199)	0.048 0.048 0.048 0.048 0.048	0.047 0.047 0.047 0.047
199 16.58 200 16.67 201 16.75	0.10 0.10 0.10 0.10 0.10	0.071 0.071 0.071 0.071 0.071	(0.198) (0.197) (0.196)	0.036 0.036 0.036	0.035 0.035 0.035
203 16.92 204 17.00 205 17.08	0.10 0.10 0.17	0.071 0.071 0.119	(0.195) (0.194) (0.193) (0.192)	0.036 0.036 0.036 0.060	0.035 0.035 0.035 0.059
206 17.17 207 17.25 208 17.33 209 17.42	0.17 0.17 0.17 0.17	0.119 0.119 0.119 0.119 0.119	(0.191) (0.190) (0.189) (0.188)	0.060 0.060 0.060 0.060	0.059 0.059 0.059 0.059
210 17.50	0.17	0.119	(0.187)	0.060	0.059
211 17.58	0.17	0.119	(0.186)	0.060	0.059
212 17.67	0.17	0.119	(0.185)	0.060	0.059
213 17.75	0.17	0.119	(0.184)	0.060	0.059
214 17.83	0.13	0.095	(0.184)	0.048	0.047
215 17.92	0.13	0.095	(0.183)	0.048	0.047
216 18.00	0.13	0.095	(0.182)	0.048	0.047
217 18.08	0.13	0.095	(0.181)	0.048	0.047
218 18.17	0.13	0.095	(0.180)	0.048	0.047
219 18.25	0.13	0.095	(0.179)	0.048	0.047
220 18.33	0.13	0.095	(0.178)	0.048	0.047
221 18.42 222 18.50 223 18.58 224 18.67	0.13 0.13 0.10 0.10	0.095 0.095 0.071 0.071	(0.177) (0.176) (0.176) (0.176) (0.175)	0.048 0.048 0.036 0.036	0.047 0.047 0.035 0.035
225 18.75	0.10	0.071	(0.174)	0.036	0.035
226 18.83	0.07	0.048	(0.173)	0.024	0.024
227 18.92	0.07	0.048	(0.172)	0.024	0.024
228 19.00	0.07	0.048	(0.171)	0.024	0.024
229 19.08	0.10	0.071	(0.171)	0.036	0.035
230 19.17	0.10	0.071	(0.170)	0.036	0.035
231 19.25	0.10	0.071	(0.169)	0.036	0.035
232 19.33	0.13	0.095	(0.168)	0.048	0.047
233 19.42	0.13	0.095	(0.167)	0.048	0.047
234 19.50	0.13	0.095	(0.167)	0.048	0.047
235 19.58	0.10	0.071	(0.166)	0.036	0.035
236 19.67 237 19.75 238 19.83 239 19.92	0.10 0.10 0.07 0.07	0.071 0.071 0.048 0.048	(0.165) (0.164) (0.164)	0.036 0.036 0.024 0.024	0.035 0.035 0.024 0.024
240 20.00	0.07	0.048	(0.162)	0.024	0.024
241 20.08	0.10	0.071	(0.161)	0.036	0.035
242 20.17	0.10	0.071	(0.161)	0.036	0.035
243 20.25	0.10	0.071	(0.160)	0.036	0.035
244 20.33	0.10	0.071	(0.159)	0.036	0.035
245 20.42	0.10	0.071	(0.159)	0.036	0.035
246 20.50	0.10	0.071	(0.158)	0.036	0.035
247 20.58	0.10	0.071	(0.157)	0.036	0.035
248 20.67	0.10	0.071	(0.157)	0.036	0.035
249 20.75	0.10	0.071	(0.156)	0.036	0.035
250 20.83	0.07	0.048	(0.155)	0.024	0.024
251 20.92	0.07	0.048	(0.155)	0.024	0.024
252 21.00	0.07	0.048	(0.154)	0.024	0.024
253 21.08	0.10	0.071	(0.154)	0.036	0.035
254 21.17	0.10	0.071	(0.153)	0.036	0.035
			· · · · · · · · · · · · · · · · · · ·		

$\begin{array}{c} 255\\ 256\\ 257\\ 258\\ 259\\ 260\\ 261\\ 262\\ 263\\ 265\\ 266\\ 270\\ 271\\ 272\\ 273\\ 275\\ 277\\ 278\\ 281\\ 282\\ 283\\ 284\\ 285\\ 286\\ 287\\ 288\end{array}$	times Total Total Total Flood Total Peak	<pre>soil loss = soil loss = rainfall = volume = soil loss = flow rate of ++++++++++ R u</pre>	ective rain 2.69(I 1.074(A 5.95(In 56672.3 46783 this hydro this hydro n o f f	In)/(n) c.Ft)) Cubic .5 Cu graph graph +++++ U R H 	0.144) 0.143) 0.143) 0.142) 0.142) 0.142) 0.142) 0.141) 0.141) 0.141) 0.140) 0.140) 0.140) 0.140) 0.140) 0.140) 0.140) 0.140) 0.140) 0.140) 1.40) 0.140) 0.140) 0.140) 0.140) 0.140) 0.141) 0.141) 0.141) 0.141) 0.141) 0.141) 0.142) 0.142) 0.142) 0.142) 0.142) 0.142) 0.142) 0.142) 0.142) 0.142) 0.142) 0.142) 0.142) 0.141) 0.141) 0.140)	1.3(Ac.F 747(CFS) +++++++++++ 4 c a p h	t) +++++++++++	
Tir		Hyarogi Volume Ac.Ft				7als ((CFS)) 5.0		10.0
	 0+ 5	0.0002	0.04 Q					-
1	0+20	0.0009 0.0016 0.0025 0.0036	0.13 Q					

0+30 0+35 0+40 0+45 0+50 0+55 1+0 1+5 1+10 1+15 1+20 1+25 1+30 1+35 1+40 1+45 1+55 2+0 2+55 2+10 2+15 2+20 2+25 2+30 2+35 2+40 2+45 2+50 2+55 3+0 3+5	0.0047 0.0059 0.0071 0.0083 0.0096 0.0111 0.0126 0.0141 0.0153 0.0165 0.0177 0.0189 0.0201 0.0213 0.0224 0.0224 0.0224 0.0224 0.0224 0.0224 0.0224 0.0224 0.0224 0.0224 0.0224 0.0225 0.0213 0.0224 0.0224 0.0225 0.0249 0.0280 0.0295 0.0311 0.0327 0.0342 0.0358 0.0374 0.0391 0.0410 0.0429 0.0449 0.0488 0.0488 0.0488 0.0508	0.17 0.17 0.17 0.19 0.22 0.22 0.21 0.18 0.17 0.17 0.17 0.17 0.17 0.17 0.17 0.22 0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.22 0.22 0.23 0.29 0.29 0.29	$ \begin{array}{c} Q\\ $		
3+35 3+40 3+45 3+50 3+55 4+0 4+5 4+10 4+15 4+20 4+25 4+30 4+35 4+40 4+45 4+50 4+55 5+0 5+15 5+20 5+25	0.0626 0.0645 0.0665 0.0709 0.0732 0.0756 0.0779 0.0803 0.0828 0.0825 0.0855 0.0882 0.0909 0.0937 0.0964 0.0993 0.1024 0.1055 0.1084 0.1158 0.1185	0.29 0.29 0.30 0.33 0.34 0.34 0.34 0.34 0.36 0.39 0.40 0.40 0.40 0.40 0.40 0.42 0.45 0.45 0.36 0.35 0.39	Q Q Q Q Q Q Q Q Q Q		

6+10 0.1464 0.50 0 V $ $ $ $ $ $ $ $ $6+20$ 0.1535 0.511 0 V $ $ $ $ $ $ $6+25$ 0.1570 0.511 0 V $ $ $ $ $ $ $6+35$ 0.1642 0.53 0 V $ $ $ $ $ $ $6+40$ 0.1642 0.57 0 V $ $ $ $ $ $ $6+45$ 0.1720 0.57 0 V $ $ $ $ $ $ $ $ $6+55$ 0.1799 0.57 0 V $ $ $ $ $ $ $ $ $7+0$ 0.1838 0.57 0 V $ <th></th>	
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$\begin{array}{c} 10+30\\ 10+35\\ 10+40\\ 10+45\\ 10+50\\ 10+55\\ 11+0\\ 11+5\\ 11+10\\ 11+15\\ 11+20\\ 11+25\\ 11+30\\ 11+35\\ 11+40\\ 11+45\\ 11+55\\ 12+20\\ 12+15\\ 12+10\\ 12+15\\ 12+20\\ 12+25\\ 12+30\\ 12+45\\ 12+50\\ 12+55\\ 13+0\\ 13+5\\ 13+10\\ 13+15\\ 13+20\\ 13+55\\ 13+10\\ 13+55\\ 13+10\\ 13+55\\ 13+10\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 13+50\\ 13+55\\ 14+0\\ 14+55\\ 14+10\\ 14+55\\ 14+10\\ 14+55\\ 14+55\\ 14+40\\ 14+55\\ 14+55\\ 15+50\\ $	0.4427 0.4492 0.4567 0.4644 0.4723 0.4801 0.4880 0.4957 0.5033 0.5108 0.5183 0.5257 0.5332 0.5404 0.5473 0.5540 0.5609 0.5679 0.5749 0.5833 0.5936 0.6045 0.6158 0.6277 0.6323 0.6523 0.6658 0.7780 0.7226 0.7386 0.7751 0.7938 0.8127 0.8316 0.8478 0.8598 0.8777 0.8814 0.9023 0.9269 0.9404 0.9538 0.9023 0.9269 0.9404 0.9538 0.9023 0.9269 0.9404 0.9538 0.9023 0.9269 0.9404 0.9538 0.9670 0.9801 0.9932 1.0063 1.0195 1.0251 1.0451 1.0576 1.0701 1.0701 1.0703	0.86 0.95 1.09 1.12 1.13 1.14 1.14 1.14 1.09 0.99 0.21 1.58 1.65 1.72 1.58 1.95 2.04 2.72 2.75 2.35 1.74 1.52 1.52 1.52 1.52 1.52 1.91 1.91 1.91 1.91 1.91 1.91 1.72 1.72 1.91	V V	

15+30 $15+35$ $15+40$ $15+45$ $15+50$ $15+55$ $16+0$ $16+10$ $16+15$ $16+20$ $16+25$ $16+30$ $16+35$ $16+40$ $16+45$ $16+50$ $16+55$ $17+0$ $17+5$ $17+10$ $17+25$ $17+20$ $17+25$ $17+30$ $17+35$ $17+40$ $17+45$ $17+50$ $18+5$ $18+10$ $18+25$ $18+10$ $18+25$ $18+20$ $18+25$ $18+40$ $18+45$ $18+50$ $18+55$ $19+0$ $19+55$ $19+10$ $19+15$ $19+20$ $19+55$ $19+40$ $19+45$ $19+50$ $19+55$ $20+10$ $20+15$ $20+10$ $20+25$			
	1.63 Q 1.26 Q 1.21 Q 1.19 Q 1.19 Q 0.89 Q 0.42 Q 0.30 Q 0.24 Q 0.23 Q 0.21 Q 0.17 Q 0.17 Q 0.17 Q 0.17 Q 0.21 Q 0.21 Q 0.28 Q 0.28 Q 0.29 Q 0.29 Q 0.29 Q 0.29 Q 0.23 Q 0.13 Q 0.13 Q 0.14 Q	1.48 Q 1.26 Q 1.19 Q 1.19 Q 1.19 Q 1.19 Q 0.89 Q 0.89 Q 0.42 $ Q$ 0.23 Q 0.24 Q 0.23 Q 0.21 Q 0.18 Q 0.17 Q 0.17 Q 0.21 Q 0.23 Q 0.24 Q 0.28 $ Q$ 0.29 $ Q$ 0.29 $ Q$ 0.29 $ Q$ 0.23 Q 0.23 Q 0.23 Q 0.18 Q	1.48 Q 1.26 Q 1.21 Q 1.19 Q 1.19 Q 1.19 Q 1.19 Q 0.89 Q 0.30 Q 0.24 Q 0.23 Q 0.23 Q 0.17 Q 0.17 Q 0.17 Q 0.17 Q 0.21 Q 0.221 Q 0.28 $ Q$ 0.29 $ Q$ 0.29 $ Q$ 0.29 $ Q$
1.1279 1.1380 1.1467 1.1550 1.1632 1.1714 1.1796 1.1857 1.1886 1.1906 1.1924 1.1940 1.1956 1.1970 1.1983 1.1995 1.2007 1.2019 1.2031 1.2045 1.2063 1.2082 1.2102 1.2121 1.2141 1.2161 1.2180 1.2200 1.2218 1.2200 1.2218 1.2251 1.2267 1.2282 1.2282 1.2298 1.2314 1.2330 1.2345 1.2360 1.2372 1.2385 1.2404 1.2412 1.2442 1.2442 1.2442 1.2442 1.2442 1.2442 1.2442 1.2444 1.2457 1.2502 1.2514 1.2502 1.2514 1.2563 1.2574 1.2586 1.2574 1.2586 1.2597 1.2609			

20+30 1.2621 0.17 Q I I I 20+35 1.2633 0.17 Q I I I	V V
20+40 1.2644 0.17 Q	V I
20+45 1.2656 0.17 Q	V
20+50 1.2667 0.15 Q	V
20+55 1.2675 0.13 Q	V
21+ 0 1.2684 0.12 Q	V
21+ 5 1.2693 0.13 Q	V
21+10 1.2704 0.16 Q	V
21+15 1.2715 0.17 Q	V
21+20 1.2726 0.15 Q	V
21+25 1.2734 0.12 Q	V
21+30 1.2743 0.12 Q	V
21+35 1.2752 0.13 Q	V
21+40 1.2763 0.16 Q	VI
21+45 1.2774 0.17 Q	V
21+50 1.2785 0.15 Q	V I
21+55 1.2793 0.12 Q	V I
22+ 0 1.2802 0.12 Q	V I
22+ 5 1.2811 0.13 Q	V I
22+10 1.2822 0.16 Q	VI
22+15 1.2833 0.17 Q	VI
22+20 1.2844 0.15 Q	VI
22+25 1.2852 0.12 Q	VI
22+30 1.2861 0.12 Q	VI
22+35 1.2869 0.12 Q	V
22+40 1.2876 0.11 Q	VI
22+45 1.2884 0.11 Q	VI
22+50 1.2892 0.11 Q	VI
22+55 1.2900 0.11 Q	V
23+ 0 1.2908 0.11 Q 23+ 5 1.2916 0.11 Q	V
	VI
	VI
23+15 1.2932 0.11 Q 23+20 1.2939 0.11 Q	V I V I
23+25 1.2947 0.11 Q	VI
23+30 1.2955 0.11 Q	VI
23+35 1.2963 0.11 Q	VI
23+40 1.2971 0.11 Q	V
23+45 1.2979 0.11 Q	V
23+50 1.2987 0.11 Q	V
$23+55$ 1.2995 0.11 \tilde{Q}	V
$24+0$ 1.3002 0.11 \tilde{Q}	V
24+5 1.3008 0.08 Q	V
24+10 1.3009 0.02 Q	V
24+15 1.3010 0.01 Q	V
24+20 1.3010 0.00 Q	V
24+25 1.3010 0.00 Q	V

APPENDIX "H"

ROUTING ANALYSIS

DEVELOPED 100 YEAR – 24 HOUR AREA C₁ & F₃

FLOOD HYDROGRAPH ROUTING PROGRAM Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2005 Study date: 07/23/19

```
Program License Serial Number 6222
 St. Frances of Rome - Wildomar
Developed C1 & F3
100-Year 24-Hour
From study/file name: c318devC1F324100.rte
Number of intervals = 290
       Time interval = 5.0 (Min.)
       Maximum/Peak flow rate =
                         2.480 (CFS)
       Total volume = 1.059 (Ac.Ft)
   Status of hydrographs being held in storage
         Stream 1 Stream 2 Stream 3 Stream 4 Stream 5
   Peak (CFS)0.0000.0000.0000.0000.000Vol (Ac.Ft)0.0000.0000.0000.0000.000
6.000 to Point/Station 7.000
Process from Point/Station
**** RETARDING BASIN ROUTING ****
User entry of depth-outflow-storage data
Total number of inflow hydrograph intervals = 290
Hydrograph time unit = 5.000 (Min.)
Initial depth in storage basin = 0.00(Ft.)
Initial basin depth = 0.00 (Ft.)
Initial basin storage = 0.00 (Ac.Ft)
Initial basin outflow = 0.00 (CFS)
Depth vs. Storage and Depth vs. Discharge data:
Basin DepthStorageOutflow(S-0*dt/2)(S+0*dt/2)(Ft.)(Ac.Ft)(CFS)(Ac.Ft)(Ac.Ft)
0.0000.0000.0000.0001.0000.1410.7940.1382.0000.3161.2510.3123.0000.5261.5840.5214.0000.7751.8640.769
                            0.000
                            0.144
                             0.320
                             0.531
                             0.781
```

	5.000	1.063	2.113	3 1		
		Hydrogra	ph Detentio	on Basin		
Gr	aph valu	es: 'I'= u				
 Time (Hours) 0.083 0.167		Outflow (CFS) 0.00 0.01 0.01 0.01 0.02 0.02 0.02 0.02	Storage (Ac.Ft) 0.000 0.001 0.002 0.002 0.003 0.004 0.004 0.004 0.004 0.005 0.006 0.006 0.006 0.007 0.008 0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.010 0.011 0.011 0.011 0.011 0.011 0.012 0.012 0.012 0.012 0.013 0.013 0.013 0.013 0.014 0.015 0.015 0.015 0.016 0.017 0.018 0.017 0.018 0.017 0.018 0.019 0.020 0.021 0.022 0.022 0.022 0.022 0.022 0.025 0.025 0.026 0.026 0.026 0.027		 	 Depth 2.48 (Ft.) 0.00 0.00 0.01 0.01 0.02 0.02 0.03 0.03 0.03 0.03 0.04 0.05 0.06 0.06 0.06 0.06 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.08 0.08 0.08 0.08 0.08 0.08 0.08 0.08 0.09 0.09 0.09 0.09 0.09 0.09 0.09 0.00 0.10 0.10 0.10 0.11 0.11 0.12 0.12 0.12 0.13 0.13 0.14 0.14 0.15 0.15 0.15 0.16 0.16 0.16 0.17 0.17 0.17 0.17 0.17 0.19

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9.333 9.417 9.500 9.583 9.667 9.750 9.833 9.917 10.000 10.083 10.167 10.250 10.333 10.417 10.500 10.583 10.667 11.000 11.083 11.167 11.250 11.333 11.417 11.500 11.583 11.67 11.750 11.833 12.417 12.000 12.083 12.167 12.500 12.333 12.417 12.500 12.583 12.167 12.500 12.583 12.417 12.500 12.583 12.417 12.500 12.583 12.417 12.500 12.583 12.417 13.000 13.083 13.167 13.250 13.333 13.417 13.500 13.583 13.667 13.750 13.750 13.833 13.917	0.81 0.85 0.92 0.97 0.98 1.04 1.08 1.09 0.62 0.59 0.59 0.59 0.59 0.91 0.94 0.95 0.95 0.91 0.87 0.88 0.89 0.71 0.88 0.89 0.71 0.71 0.71 0.71 0.71 0.71 0.71 0.71 0.71 0.71 0.72 1.58 1.62 1.58 1.95 1.951	0.48 0.50 0.51 0.52 0.54 0.56 0.57 0.59 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.63 0.64 0.66 0.67 0.68 0.69 0.71 0.71 0.72 0.73 0.74 0.882 0.885 0.88 0.992 0.94 0.981 1.04 1.09 1.11 1.12 1.12 1.13 1.13	0.086 0.088 0.090 0.093 0.096 0.099 0.102 0.105 0.108 0.111 0.112 0.123 0.125 0.126 0.123 0.125 0.126 0.127 0.128 0.128 0.129 0.120 0 I I 0 I I			0.61 0.62 0.64 0.68 0.70 0.72 0.77 0.79 0.92 0.91 0.92	
13.583 13.667 13.750 13.833	1.91 1.47 1.41 1.41	1.11 1.12 1.12 1.13	0.261 0.265 0.267 0.269		I I I I I I I I I I I I I I I I I I	I 	1.6 1.7 1.7 1.7

19.333 19.417 19.500 19.583 19.667 19.750 19.750 19.833 19.917 20.000 20.083 20.167 20.250 20.333 20.417 20.500 20.583 20.667 20.750 20.833 20.917 21.000 21.083 21.167 21.250 21.333 21.417 21.500 21.833 21.417 22.000 22.083 22.167 22.250 22.333 22.167 22.250 22.833 22.167 22.250 22.333 22.167 22.500 22.833 22.667 22.750 22.833 22.167 22.500 23.333 23.167 23.2500 23.333 23.417 23.000 23.083 23.167 23.2500 23.333 23.417 23.000 23.083 23.167 23.2500 23.333 23.417 23.000 23.083 23.167 23.2500 23.333 23.417 23.000 23.083 23.167 23.2500 23.333 23.417 23.2500 23.333 23.417 23.2500 23.333 23.417 23.2500 23.333 23.417 23.2500 23.683 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.2833 23.417 23.2500 23.8337 23.417 23.2500 23.8337 23.417 23.2500 23.8337 23.417 23.2500 23.8337 23.417 23.2500 23.8337 23.417 23.2500 23.8337 23.417 23.2500 23.8337 23.6700 23.837 23.6700 23.837 23.6700 23.837 23.837 23.917 23.837	0.14 0.15 0.16 0.14 0.12 0.10 0.08 0.08 0.08 0.08 0.10 0.11 0.12 0.10 0.08 0.09 0.0000 0.0000 0.0000 0.000000 0.00000 0.000000	0.62 0.60 0.58 0.55 0.53 0.52 0.50 0.48 0.47 0.45 0.44 0.43 0.42 0.41 0.39 0.38 0.37 0.36 0.32 0.32 0.32 0.32 0.32 0.28 0.225 0.24 0.25 0.225 0.221 0.22 0.22 0.21 0.22 0.21 0.22	0.109 0.103 0.103 0.100 0.097 0.094 0.091 0.089 0.086 0.083 0.081 0.076 0.074 0.072 0.070 0.068 0.065 0.065 0.063 0.061 0.059 0.050 0.050 0.052 0.050 0.052 0.050 0.049 0.048 0.047 0.046 0.042 0.041 0.042 0.041 0.042 0.041 0.030 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.032 0.031 0.030 0.029 0.028 0.027 0.026 0.025 0.024	I 0 I			$ 0.78 \\ 0.75 \\ 0.73 \\ 0.71 \\ 0.69 \\ 0.67 \\ 0.65 \\ 0.63 \\ 0.61 \\ 0.59 \\ 0.57 \\ 0.56 \\ 0.54 \\ 0.53 \\ 0.51 \\ 0.50 \\ 0.54 \\ 0.53 \\ 0.51 \\ 0.50 \\ 0.48 \\ 0.47 \\ 0.46 \\ 0.45 \\ 0.43 \\ 0.42 \\ 0.41 \\ 0.42 \\ 0.41 \\ 0.40 \\ 0.39 \\ 0.38 \\ 0.37 \\ 0.36 \\ 0.35 \\ 0.31 \\ 0.33 \\ 0.32 \\ 0.31 \\ 0.33 \\ 0.32 \\ 0.31 \\ 0.30 \\ 0.29 \\ 0.28 \\ 0.27 \\ 0.26 \\ 0.25 \\ 0.25 \\ 0.25 \\ 0.22 \\ 0.21 \\ 0.21 \\ 0.20 \\ 0.19 \\ 0.19 \\ 0.19 \\ 0.19 \\ 0.19 \\ 0.18 \\ 0.17 \\ 0.17 \\ 0.11 \\ 0.17 \\ 0.11 $
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29.333 29.417 29.500 29.583 29.667 29.750 29.833 29.917 30.000 30.083 30.167 30.250 30.333 30.417 30.500 30.583 30.667 30.750 30.833 30.917 31.000 31.083 31.167 31.250 31.333 31.417 31.500 31.583 31.667 31.750 31.833 31.417 32.000 32.083 32.167 32.500 32.333 32.417 32.500 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 32.667 32.750 33.667 33.677 33.000 3.673 33.417 3.000 3.683 33.417 3.500 3.673 3.750 3.673 3.750 3.673 3.750 3.675 3.750 3.677 3.750 3.675 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.677 3.750 3.750 3.677 3.750	$\begin{array}{cccccccccccccccccccccccccccccccccccc$				0.02 0.01 0.00 0.00 0.00 0.00
	0.01 0.00 0.00	0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.00 0.001 0 0.00 0.001 0 0.00 0.001 0 0.00 0.001 0 0.00 0.001 0 0.00 0.001 0 0.00 0.001 0 0.00 0.001 0 0.00 0.001 0 0.00 0.001 0 0.00 0.000 0.000 0.00 0.000 0.000 0.00 0.000 0.000 </td <td>$\begin{array}{c ccccccccccccccccccccccccccccccccccc$</td> <td>0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0.01 <</td> <td>0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0.01 0.001 0.01 0.001 0.01 0.001 0.01 0.001 <!--</td--></td>	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0.01 <	0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.002 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0 0.01 0.001 0.01 0.001 0.01 0.001 0.01 0.001 0.01 0.001 </td

34.333	0.00	0.00	0.000	0	1	1	1	1	0.00
34.417	0.00	0.00	0.000	0				1	0.00
34.500	0.00	0.00	0.000	0	-				0.00
34.583	0.00	0.00	0.000	0	1				0.00
34.667	0.00	0.00	0.000	0	1				0.00
34.750	0.00	0.00	0.000	0	1		1		0.00
34.833	0.00	0.00	0.000	0		1	1	1	0.00
Number of intervals = 418 Time interval = 5.0 (Min.) Maximum/Peak flow rate = 1.273 (CFS) Total volume = 1.059 (Ac.Ft) Status of hydrographs being held in storage Stream 1 Stream 2 Stream 3 Stream 4 Stream 5 Peak (CFS) 0.000 0.000 0.000 0.000 Vol (Ac.Ft) 0.000 0.000 0.000 0.000 *********************************									

DEVELOPED 100 YEAR – 24 HOUR AREA D₁

FLOOD HYDROGRAPH ROUTING PROGRAM Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2005 Study date: 07/23/19

Program License Serial Number 6222 St. Frances of Rome - Wildomar Developed D1 100-Year 24-Hour From study/file name: c318devD124100.rte Number of intervals = 291 Time interval = 5.0 (Min.) Maximum/Peak flow rate = 3.150 (CFS) Total volume = 1.498 (Ac.Ft) Status of hydrographs being held in storage Stream 1 Stream 2 Stream 3 Stream 4 Stream 5 Peak (CFS)0.0000.0000.0000.0000.000Vol (Ac.Ft)0.0000.0000.0000.0000.000 Process from Point/Station 8.000 to Point/Station 9.000 **** RETARDING BASIN ROUTING **** User entry of depth-outflow-storage data _____ Total number of inflow hydrograph intervals = 291 Hydrograph time unit = 5.000 (Min.) Initial depth in storage basin = 0.00(Ft.) _____ Initial basin depth = 0.00 (Ft.) Initial basin storage = 0.00 (Ac.Ft) Initial basin outflow = 0.00 (CFS) Depth vs. Storage and Depth vs. Discharge data: Basin Depth Storage Outflow (S-O*dt/2) (S+O*dt/2) (Ft.) (Ac.Ft) (CFS) (Ac.Ft) (Ac.Ft) 0.0000.0000.0000.0000.7500.1891.1600.1851.7500.4822.3510.4742.7500.8223.1050.8113.7501.2123.7131.199 0.193 0.000 0.490 0.833 1.225

Hydrograph Detention Basin Routing

Gra	apn valu	es: ·r=	UNIC INFLOV	v; ·O·=	FOULTION	at time	snown	
Time	Inflow	Outflow	Storage					Depth
(Hours)	(CFS)	(CFS)	(Ac.Ft)	0	0.8	1.57	2 36	
0.083	0.06	0.00	0.000	0	1	1	1	0.00
0.167	0.12	0.00	0.001	OI	-			0.00
0.250	0.13	0.01	0.002	OI	1	1		0.01
0.333	0.16	0.02	0.002	OI	1			0.01
0.417	0.19	0.02	0.004	OI	1	1	1	0.01
0.500	0.19	0.03	0.005	OI	1	1	-	0.02
0.583	0.19	0.04	0.006	OI	1	4	1	0.02
0.667	0.19	0.04	0.007	OI		1	1	0.03
0.750	0.19	0.05	0.008	OI		l	i	0.03
0.833	0.22	0.05	0.009	ΟI		l		0.04
0.917	0.25	0.06	0.010	ΟI			Í	0.04
1.000	0.25	0.07	0.011	ΟI		1		0.05
1.083	0.22	0.08	0.013	ΟI		l	1	0.05
1.167	0.20	0.08	0.013	ΟI	1	1		0.05
1.250	0.19	0.09	0.014	OI	ł			0.06
1.333	0.19	0.09	0.015	OI	l	.	l	0.06
1.417	0.19	0.10	0.016	OI				0.06
1.500	0.19	0.10	0.016	0		1		0.06
1.583	0.19	0.10	0.017	10				0.07
1.667	0.19	0.11	0.017	0		1		0.07
1.750	0.19	0.11	0.018	10				0.07
1.833	0.22	0.11	0.019	OI				0.07
1.917	0.25	0.12	0.019	IOI				0.08
2.000 2.083	0.25 0.26	0.13 0.13	0.020 0.021	OI OI	1			0.08
2.083	0.26	0.13	0.021	01 01	-		1	0.08
2.250	0.20	0.14	0.023	IOI	1	1	1	0.09
2.333	0.26	0.15	0.023	01 01	l	1	f	0.09
2.333	0.26	0.15	0.024	01 01	I	1		0.10
2.500	0.26	0.15	0.025	OI	and out of the second se	1	1	0.10
2.583	0.29	0.16	0.026	OI	Ì	1		0.10
2.667	0.31	0.16	0.027	0 I	i			0.11
2.750	0.32	0.17	0.028	I O	l			0.11
2.833	0.32	0.18	0.029	I O	1			0.11
2.917	0.32	0.18	0.030	O I	l	1	-	0.12
3.000	0.32	0.19	0.031	0 I	I	I	1	0.12
3.083	0.32	0.19	0.032	0 I	l	1	1	0.13
3.167	0.32	0.20	0.032	OI	-	l		0.13
3.250	0.32	0.20	0.033	OI		1		0.13
3.333	0.32	0.21	0.034	OI			ale and a second se	0.14
3.417	0.32	0.21	0.035	OI				0.14
3.500	0.32	0.22	0.036	OI				1 0.14
3.583	0.32	0.22	0.036	OI				0.14
3.667	0.32	0.23	0.037	OI				0.15
3.750	0.32	0.23	0.037	I OI		I		0.15
3.833	0.35	0.23	0.038	OI	1		1	0.15
3.917	0.38	0.24	0.039	I OI	1			0.16
4.000 4.083	0.38 0.38	0.25 0.25	0.040 0.041	I O I	1	I	1	0.16
4.083 4.167	0.38	0.25	0.041	OI OI	1	1	1	0.16 0.17
4.167	0.38	0.26	0.042	01 01			1	0.17
4.230	0.38	0.20	0.043	0 I		1	l	0.17
4.000	0.11	0.27	0.011	1 U I	l	ł	ą	, 0.17

Graph values: 'I'= unit inflow; 'O'=outflow at time shown

4.417 4.500 4.583 4.667 4.750 4.833 4.917 5.000 5.083 5.167 5.250 5.333 5.417 5.500 5.837 5.750 5.833 5.917 6.000 6.083 6.167 6.250 6.333 6.667 6.250 6.333 6.667 6.750 6.833 6.667 7.750 7.250 7.333 7.167 7.250 7.333 7.167 7.583 7.500 7.333 7.167 7.583 7.500 7.333 7.167 7.583 7.500 7.333 7.167 7.583 7.500 7.503 7.500 7.503 7.500 7.583 7.500 7.503 7.500 7.503 7.500 7.503 7.500 7.503 7.500 7.500 7.503 7.500 7.500 7.503 7.500 7.500 7.503 7.500 7.500 7.503 7.500 7	0.44 0.45 0.45 0.45 0.45 0.45 0.45 0.51 0.51 0.45 0.39 0.41 0.45 0.51 0.51 0.51 0.51 0.51 0.57 0.57 0.57 0.57 0.57 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.62 0.70 0.77 0.77 0.80 0.82 0.83 0.96 0.96 0.96 0.96 0.96 0.96 0.99 1.02 1.02 1.05 1.05 1.02 1.021	0.27 0.28 0.29 0.30 0.31 0.32 0.33 0.33 0.34 0.34 0.34 0.35 0.36 0.35 0.36 0.37 0.38 0.39 0.40 0.41 0.42 0.42 0.43 0.44 0.42 0.43 0.44 0.42 0.43 0.55 0.51 0.52 0.53 0.55 0.55 0.57 0.58 0.60 10.55 0.57 0.58 0.61 0.63 0.63 0.61 0.63 0.61 0.63 0.61 0.77 0.77 0.77 0.77 0.79	0.045 0.046 0.047 0.048 0.049 0.050 0.051 0.053 0.054 0.055 0.056 0.056 0.056 0.057 0.058 0.059 0.060 0.061 0.062 0.063 0.064 0.065 0.067 0.068 0.069 0.071 0.072 0.074 0.075 0.076 0.071 0.072 0.074 0.075 0.076 0.071 0.072 0.074 0.075 0.076 0.071 0.072 0.074 0.075 0.076 0.071 0.072 0.074 0.075 0.076 0.071 0.072 0.074 0.075 0.076 0.077 0.078 0.080 0.081 0.082 0.083 0.085 0.086 0.089 0.091 0.093 0.095 0.097 0.100 0.102 0.104 0.111 0.113 0.123 0.126 0.129	0 I 0 I	$ 0.18 \\ 0.19 \\ 0.19 \\ 0.19 \\ 0.19 \\ 0.20 \\ 0.20 \\ 0.20 \\ 0.20 \\ 0.21 \\ 0.21 \\ 0.21 \\ 0.22 \\ 0.22 \\ 0.22 \\ 0.22 \\ 0.22 \\ 0.22 \\ 0.23 \\ 0.23 \\ 0.23 \\ 0.23 \\ 0.23 \\ 0.23 \\ 0.24 \\ 0.24 \\ 0.25 \\ 0.25 \\ 0.25 \\ 0.25 \\ 0.25 \\ 0.25 \\ 0.25 \\ 0.26 \\ 0.27 \\ 0.27 \\ 0.27 \\ 0.27 \\ 0.28 \\ 0.29 \\ 0.29 \\ 0.29 \\ 0.29 \\ 0.29 \\ 0.30 \\ 0.31 \\ 0.31 \\ 0.31 \\ 0.31 \\ 0.32 \\ 0.33 \\ 0.34 \\ 0.34 \\ 0.35 \\ 0.35 \\ 0.35 \\ 0.35 \\ 0.36 \\ 0.37 \\ 0.38 \\ 0.39 \\ 0.40 \\ 0.40 \\ 0.41 \\ 0.42 \\ 0.43 \\ 0.44 \\ 0.45 \\ 0.44 \\ 0.45 \\ 0.46 \\ 0.47 \\ 0.48 \\ 0.49 \\ 0.50 \\ 0.51 \\ 0.51 \\ 0.51 \\ 0.51 \\ 0.51 \\ 0.19 \\ 0.19 \\ 0.19 \\ 0.19 \\ 0.19 \\ 0.19 \\ 0.19 \\ 0.10 $
9.250 9.333	1.20 1.21 1.24	0.79 0.81			

9.417 9.500 9.583 9.667 9.750 9.833 9.917 10.000 10.083 10.167 10.250 10.333 10.417 10.500 10.583 10.667 10.750 10.833 10.917 11.000 11.083 11.167 11.250 11.333 11.417 11.500 11.583 11.677 12.500 12.833 12.417 12.000 12.083 12.167 12.503 12.417 12.500 12.833 12.417 12.500 12.833 12.417 12.500 12.833 12.417 12.500 12.833 12.917 13.000 13.083 13.167 13.250 13.333 13.417 13.500 13.583 13.667 13.750 13.833 13.917 14.000 14.083 13.917 13.000 13.917 13.917 13.917 1	$\begin{array}{c} 1.27\\ 1.28\\ 1.31\\ 1.34\\ 1.34\\ 1.40\\ 1.45\\ 1.47\\ 1.22\\ 1.00\\ 0.96\\ 0.96\\ 0.96\\ 1.11\\ 1.25\\ 1.27\\ 1.28\\ 1.29\\ 1.30\\ 1.22\\ 1.21\\ 1.21\\ 1.21\\ 1.21\\ 1.21\\ 1.21\\ 1.21\\ 1.25\\ 1.20\\ 1.22\\ 1.22\\ 1.21\\ 1.21\\ 1.25\\ 1.90\\ 1.22\\ 1.22\\ 1.21\\ 1.21\\ 1.55\\ 1.90\\ 1.97\\ 2.05\\ 2.10\\ 2.12\\ 2.35\\ 2.37\\ 2.44\\ 2.55\\ 1.95\\ 1.84\\ 3.12\\ 3.14\\ 3.15\\ 1.84\\ 1.85\\ 2.08\\ 1.84\\ 1.85\\ 2.08\\ 1.84\\ 1.85\\ 2.08\\ 1.84\\ 1.85\\ 2.08\\ 1.84\\ 1.85\\ 2.08\\ 1.84\\ 1.85\\ 2.08\\ 1.84\\ 1.85\\ 1.85\\$	0.83 0.84 0.86 0.90 0.92 0.94 0.96 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.98 0.99 1.00 1.01 1.03 1.04 1.05 1.06 1.07 1.08 1.09 1.091	0.135 0.138 0.141 0.144 0.147 0.150 0.153 0.157 0.159 0.160 0.177 0.220 0.201 0.201 0.201 0.201 0.201 0.220 0.228 0.220 0.228 0.220 0.229 0.322 0.322 0.322 0.324 0.325 0.327 0.327 0.329 0 I	
13.833	1.84	1.72	0.326	0I
13.917	1.84	1.72	0.327	0I

14.417 14.500 14.583 14.667 14.750 14.750 14.833 14.917 15.000 15.083 15.167 15.250 15.333 15.417 15.500 15.750 15.750 15.750 15.833 15.917 16.000 16.333 16.417 16.500 16.583 16.417 16.500 16.833 16.417 16.500 16.833 16.417 17.000 17.083 17.167 17.250 17.333 17.417 17.500 17.833 17.417 17.500 17.833 17.417 17.500 17.833 17.917 18.000 18.250 18.333 18.417 18.250 18.333 18.417 18.500 18.583 18.677 18.750 18.750 18.750 18.333 18.167 18.250 18.333 18.167 18.250 18.333 18.417 18.500 18.583 18.917 19.0083 19.167 19.250 19.250	2.24 2.24 2.25 2.25 2.20 2.15 2.09 2.04 2.04 1.98 1.94 1.93 1.70 1.47 1.47 0.89 0.26 0.22 0.20 0.19 0.19 0.25 0.32 0.32 0.32 0.32 0.32 0.32 0.32 0.26 0.26 0.26 0.22 0.32 0.32 0.32 0.32 0.32 0.32 0.26 0.26 0.26 0.26 0.26 0.26 0.22 0.32 0.32 0.32 0.32 0.32 0.32 0.26 0.219 0.19 0.19 0.19 0.19 0.19 0.19	1.79 1.80 1.81 1.82 1.84 1.85 1.86 1.87 1.88 1.88 1.88 1.88 1.88 1.88 1.88 1.88 1.88 1.88 1.88 1.88 1.88 1.88 1.87 1.88 1.88 1.87 1.88 1.88 1.87 1.88 1.87 1.88 1.87 1.88 1.87 1.88 1.87 1.88 1.87 1.88 1.87 1.88 1.74 1.662 1.551 1.47 1.31 1.22 1.21 1.12 1.09 0.990 0.930 0.852 0.870 0.852 0.870 0.852 0.990 0.930 0.774 0.700 0.675 0.651	0.343 0.346 0.349 0.352 0.355 0.358 0.360 0.362 0.363 0.366 0.367 0.368 0.368 0.367 0.368 0.363 0.360 0.357 0.355 0.350 0.322 0.322 0.313 0.294 0.241 0.241 0.227 0.249 0.241 0.224 0.227 0.220 0.213 0.201 0.195 0.189 0.166 0.151 0.147 0.143 0.138 0.134 0.126 0.151 0.147 0.126 0.1217 0.121 0.107			0 0 0 0 0 0 0 0 0 0 0 0	$\begin{array}{c} 1.28\\ 1.29\\ 1.30\\ 1.31\\ 1.32\\ 1.33\\ 1.33\\ 1.33\\ 1.34\\ 1.35\\ 1.35\\ 1.35\\ 1.36\\ 1.36\\ 1.36\\ 1.36\\ 1.36\\ 1.36\\ 1.36\\ 1.36\\ 1.36\\ 1.36\\ 1.36\\ 1.36\\ 1.36\\ 1.36\\ 1.36\\ 1.36\\ 1.32\\$
19.333	0.22	0.64	0.104	II	0		0.41

19.417 19.500 19.583 19.667 19.750 19.750 19.833 19.917 20.000 20.083 20.167 20.250 20.333 20.417 20.500 20.583 20.667 20.750 20.833 21.167 21.250 21.333 21.417 21.500 21.583 21.417 21.500 21.583 21.417 22.000 22.083 22.167 22.503 22.417 22.500 22.833 22.417 22.500 22.833 22.417 22.500 22.833 22.417 22.500 22.833 22.417 23.000 23.083 23.167 23.250 23.333 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.333 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.333 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.417 23.500 23.583 23.667 23.750 23.583 23.67 23.583 23.5917 23.500 23.583 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.500 23.583 23.5917 23.5917 23.500 23.583 23.5917 23.5917 23.5917 23.5917 23.5917 23.5917 23.5917 23.5917 23.5917 23.5917 23.5917 23.5917 23.5917 23.5917 23	0.25 0.22 0.20 0.19 0.16 0.13 0.19 0.13	0.62 0.60 0.59 0.57 0.56 0.54 0.53 0.51 0.49 0.48 0.47 0.46 0.42 0.43 0.42 0.41 0.40 0.39 0.38 0.37 0.36 0.32 0.31 0.30 0.29 0.28 0.22 0.21 0.21 0.21 0.20 0.20 0.20 0.20 0.20 0.21 0.22 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.21 0.221 0.21	0.101 0.098 0.096 0.093 0.091 0.088 0.086 0.083 0.070 0.073 0.074 0.073 0.071 0.069 0.065 0.065 0.063 0.061 0.060 0.058 0.057 0.056 0.055 0.053 0.052 0.051 0.050 0.049 0.048 0.047 0.046 0.042 0.044 0.042 0.042 0.044 0.042 0.042 0.044 0.045 0.042 0.044 0.042 0.042 0.044 0.038 0.037 0.036 0.031 0.029 0.028 0.029 0.028 0.029 0	I I O I I O I I O I I O I O I		0.40 0.39 0.38 0.37 0.36 0.35 0.32 0.32 0.29 0.27 0.26 0.27 0.26 0.22 0.22 0.22 0.22 0.22 0.22 0.22 0.22 0.22 0.22 0.22 0.22 0.22 0.22 0.22 0.22 0.22 0.21 0.20 0.19 0.18 0.18 0.15 0.14 0.14 0.13 0.13 0.13 0.14 0.14 0.14 0.13 0.12
24.333	0.00	0.17	0.027	IO		0.11

28.750 0.00 0.02 0.003 0 0.01 28.833 0.00 0.02 0.003 0 0.01 28.917 0.00 0.02 0.003 0 0.01 29.000 0.00 0.02 0.003 0 0.01 29.083 0.00 0.01 0.002 0 0.01 29.167 0.00 0.01 0.002 0 0.01 29.250 0.00 0.01 0.002 0 0.01 29.333 0.00 0.01 0.002 0 0.01

29.417 29.500 29.583 29.667 29.750 29.833 29.917 30.000 30.083 30.167 30.250 30.333 30.417 30.500 30.583 30.667 30.750 30.833 30.917 31.000 31.083 31.167 31.250 31.333 31.417 31.500 31.583 31.667 31.750 31.333 31.417 32.000 32.083 32.167 32.250 32.333 32.417 32.000 32.667 32.750 32.833 32.417 32.500 32.833 32.917 33.000 33.083 33.167 33.250 33.333 33.417 33.250 33.333 33.417 33.500			0.002 0.002 0.002 0.002 0.002 0.002 0.002 0.002 0.002 0.001 0.000 0	000000000000000000000000000000000000000				0.01 0.00 0.00	
33.500 33.583	0.00 0.00	0.00 0.00	$0.000 \\ 0.000$	0 0				0.00	
			*******	YDROG	RAPH DAT	`A******	* * * * * * * * *		
<pre>************************************</pre>									

Vol (Ac.Ft)	0.000	0.000	0.000	0.000	0.000
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DEVELOPED 100 YEAR – 24 HOUR AREA E₁

FLOOD HYDROGRAPH ROUTING PROGRAM Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2005 Study date: 07/23/19

Program License Serial Number 6222 St. Frances of Rome - Wildomar Developed E1 100-Year 24-Hour From study/file name: c318devE124100.rte Number of intervals = 289 Time interval = 5.0 (Min.) Maximum/Peak flow rate = 1.692 (CFS) Total volume = 0.909 (Ac.Ft) Status of hydrographs being held in storage Stream 1 Stream 2 Stream 3 Stream 4 Stream 5 Peak (CFS)0.0000.0000.0000.0000.000Vol (Ac.Ft)0.0000.0000.0000.0000.000 Process from Point/Station 10.000 to Point/Station 11.000 **** RETARDING BASIN ROUTING **** User entry of depth-outflow-storage data _____ Total number of inflow hydrograph intervals = 289 Hydrograph time unit = 5.000 (Min.) Initial depth in storage basin = 0.00(Ft.) Initial basin depth = 0.00 (Ft.) Initial basin storage = 0.00 (Ac.Ft) Initial basin outflow = 0.00 (CFS) Depth vs. Storage and Depth vs. Discharge data: Basin Depth Storage Outflow (S-O*dt/2) (S+O*dt/2) (Ft.) (Ac.Ft) (CFS) (Ac.Ft) (Ac.Ft) ____ 0.0000.0000.0000.0000.0000.5000.0850.6790.0830.0871.5000.2951.7960.2890.3012.5000.5602.5040.5510.569 ------

Hydrograph Detention Basin Routing

Graph values: '1'= unit inflow; '0'=outflow at time shown Time Inflow; CFS) (Ac. 72).0 0.4 0.85 1.27 1.68 Popth COM 0.01 1 1 1 1.68 Popth 0.16 0.16 0.16 0.16 0.00 0.16 0.00 0.16 0.00 0.16 0.00 0.16 0.00 0.11 0.00 0.01 0.00 0.12 0.01 0.01 0.02 0.667 0.12 0.02 0.667 0.12 0.03 0.667 0.12 0.03 0.66 0.01 0.03 0.66 0.11 <th colspan="2"

Graph values: 'I'= unit inflow; 'O'=outflow at time shown

9.500 9.583 9.667 9.750 9.833 9.917 10.000 10.083 10.167 10.250 10.333 10.417 10.500 10.583 10.667 10.750 10.833 11.167 11.250 11.333 11.417 11.500 11.583 11.667 11.750 11.833 12.167 12.250 12.333 12.417 12.500 12.833 12.417 12.500 12.833 12.417 12.500 12.833 12.417 12.500 12.833 12.417 12.500 12.833 12.417 12.500 12.833 12.417 13.000 13.083 13.167 13.250 13.333 13.417 13.500 13.583 13.417 13.500 13.583 13.417 13.500 13.837 13.917 14.008 13.837 13.917 14.008 13.837 13.917 14.008 13.837 13.917 14.008 14.083 13.917 14.008 14.083 14.083 15.917 13.917	0.86 0.90 0.92 0.92 0.92 0.96 0.98 0.98 0.67 0.60 0.60 0.60 0.60 0.82 0.88 0.88 0.88 0.88 0.83 0.83 0.83 0.74 0.72 0.77 0.77 0.78 1.10 1.17 1.18 1.22 1.35 1.35 1.35 1.35 1.69 1.90	0.59 0.61 0.62 0.64 0.65 0.67 0.68 0.69 0.68 0.69 0.68 0.69 0.68 0.68 0.69 0.70 0.71 0.72 0.72 0.72 0.73 0.73 0.74 0.74 0.74 0.74 0.74 0.74 0.74 0.75 0.79 0.82 0.89 0.93 0.93 0.95 1.00 1.09 1.091	0.074 0.076 0.076 0.078 0.078 0.080 0.082 0.084 0.086 0.087 0.087 0.086 0.085 0.085 0.085 0.085 0.085 0.085 0.086 0.086 0.086 0.090 0.091 0.092 0.093 0.093 0.093 0.093 0.093 0.094 0.095 0.096 0.09	0 I 0 0 0 I 0 0 0 I 0 0 0 I 0 0 0 I 0 0 10 I 0 0 10 I I 0 0 10 I I 0 0 10 I I 0 0 10 I I 0 0 0 I I 0 0 0 I I 0 0 0 I I 0 0 0 I I 0 0 0 I I 0 0 0 I I 0 0 0 I I<	445678901100000122334445555555555567901346679124691356777777
13.833 13.917 14.000	1.08 1.08 1.09	1.09 1.09 1.09	0.162 0.162 0.162	0 0 0 0 0 0 0 0 0 0 I 0 0 0 0 0 0 0 0 0 0	.87 .87 .87

16.500 0.16 0.93 0.132 I 0 16.583 0.13 0.90 0.126 I 0 16.583 0.12 0.87 0.121 I 0 16.667 0.12 0.87 0.121 I 0 16.750 0.12 0.84 0.116 I 0 16.833 0.12 0.82 0.111 I 0 16.917 0.12 0.79 0.106 I 0 17.000 0.12 0.77 0.102 I 0 17.083 0.18 0.75 0.098 I 0 17.167 0.20 0.73 0.094 I 0 17.250 0.20 0.71 0.090 I 0 <	0.90 0.91 0.91 0.92 0.90 0.89 0.88 0.77 0.65 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.63 0.51 0.49 0.44 0.39 0.38 0.32 0.22 0.22 0.22 0.22 0.21 0.21
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29.583 0.00 0.00 0.00 0 0.000 0 0 0 0 0 0 0 0	29.500	0.00	0.00	0.000	0		1			0.00
29.667 0.00 0.00 0.000 0 0.00 29.750 0.00 0.00 0.000 0 0.000 29.833 0.00 0.00 0.000 0 0.000 29.917 0.00 0.00 0.000 0 0.000 30.000 0.00 0.00 0.000 0 0.000 30.000 0.00 0.00 0.000 0 0.000 30.083 0.00 0.00 0.000 0 0.000 30.167 0.00 0.00 0.000 0 0.000 30.250 0.00 0.00 0.000 0 0.000 30.250 0.00 0.00 0.000 0 0.000 30.333 0.00 0.00 0.000 0 0.000 30.417 0.00 0.00 0.000 0 0.000 30.500 0.00 0.00 0.000 0 0.000 30.583 0.00 0.00 0.000 0 0.000 *********************************	29.583	0.00	0.00	0.000	0	i		s and a second se	1	0.00
29.750 0.00 0.00 0.000 0 0.00 29.833 0.00 0.00 0.000 0 0.00 29.917 0.00 0.00 0.000 0 0.000 30.000 0.00 0.00 0.000 0 0.000 30.083 0.00 0.00 0.000 0 0.000 30.167 0.00 0.00 0.000 0 0.000 30.250 0.00 0.00 0.000 0 0.000 30.333 0.00 0.00 0.000 0 0.000 30.333 0.00 0.00 0.000 0 0.000 30.583 0.00 0.000 0.000 0.000 30.583 0.00 0.000 0.000 0.000 30.583 0.00 0.000 0.000 0.000 0.000 30.583 0.000 0.000 0.000 0.000 0.000 0.000 0.000 30.583 0.000 0	29.667			0.000	0		Ì		Ì	0.00
29.917 0.00 0.00 0.000 0 0.00 30.000 0.00 0.00 0.000 0 0.00 30.083 0.00 0.00 0.000 0 0.00 30.167 0.00 0.00 0.000 0 0.00 30.250 0.00 0.00 0.000 0 0.000 30.333 0.00 0.00 0.000 0 0 0.000 30.417 0.00 0.00 0.000 0 0 0.000 30.500 0.00 0.00 0.000 0 0 0.000 30.583 0.00 0.00 0.000 0 0 0 0.000 *********************************	29.750	0.00	0.00	0.000	0			1	Ì	0.00
30.000 0.00 0.00 0.000 0 0 0 0 0 0 0 0 0	29.833	0.00	0.00	0.000	0		1		1	0.00
30.083 0.00 0.00 0.000 0 0.00 30.167 0.00 0.00 0.000 0 0.00 30.250 0.00 0.00 0.000 0 0.000 30.333 0.00 0.00 0.000 0 0.000 30.417 0.00 0.00 0.000 0 0 0.000 30.500 0.00 0.00 0.000 0 0 0.000 30.583 0.00 0.00 0.000 0 0 0 0.000 *********************************	29.917	0.00	0.00	0.000	0		1			0.00
30.167 0.00 0.00 0.000 0 0.00 30.250 0.00 0.00 0.000 0 0.00 30.333 0.00 0.00 0.000 0 0.00 30.417 0.00 0.00 0.000 0 0 0.00 30.500 0.00 0.00 0.000 0 0 0 0.00 30.583 0.00 0.00 0.00 0 0 0 0 0.00 **********************************	30.000	0.00	0.00	0.000	0					0.00
30.250 0.00 0.00 0.00 0.000 0 0.00 30.333 0.00 0.00 0.00 0.000 0 0.00 30.417 0.00 0.00 0.00 0.000 0 0.00 30.500 0.00 0.00 0.000 0 0 0.00 30.583 0.00 0.00 0.00 0 0 0.00 **********************************	30.083	0.00	0.00	0.000	0	1	·	1		0.00
30.333 0.00 0.00 0.000 0 0.000 0 0 0.000 30.417 0.00 0.00 0.000 0 0 0.000 0 0 0.000 30.500 0.00 0.00 0.000 0 0 0 0 0 0 0 0 0	30.167	0.00	0.00	0.000	0		1		1	0.00
30.417 0.00 0.00 0.000 0 0.00 30.500 0.00 0.00 0.000 0 0 0.00 30.583 0.00 0.00 0.000 0 0 0.000 *********************************	30.250	0.00	0.00	0.000	0			1		0.00
30.500 0.00 0.00 0.000 0 0.000 0 0 0.000 30.583 0.00 0.00 0.000 0 0.000 0 0 0.000 *********************************	30.333	0.00	0.00	0.000	0			1		0.00
30.583 0.00 0.00 0.000 0 0.00 **********************************	30.417	0.00	0.00	0.000	0	1	1			0.00
<pre>************************************</pre>	30.500	0.00	0.00	0.000	0					0.00
Number of intervals = 367 Time interval = 5.0 (Min.) Maximum/Peak flow rate = 1.148 (CFS) Total volume = 0.909 (Ac.Ft) Status of hydrographs being held in storage Stream 1 Stream 2 Stream 3 Stream 4 Stream 5 Peak (CFS) 0.000 0.000 0.000 Vol (Ac.Ft) 0.000 0.000 0.000	30.583	0.00	0.00	0.000	0			1		0.00
	Number of intervals = 367 Time interval = 5.0 (Min.) Maximum/Peak flow rate = 1.148 (CFS) Total volume = 0.909 (Ac.Ft) Status of hydrographs being held in storage Stream 1 Stream 2 Stream 3 Stream 4 Stream 5									
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	* * * *		'							

APPENDIX "I"

BASIN, STORM DRAIN, & CHANNEL CALCULATIONS

BASIN C₁ VOLUME

Basin C_1

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Depth (Feet)	Area (Feet ²)	Volume (Feet ³)	Σ Volume (Feet ³)	Σ Volume (Acre-Ft)
0.0	5457	0	0	0.000
1.0	6859	6158	6158	0.141
2.0	8362	7610	13768	0.316
3.0	9965	9163	22931	0.526
4.0	11669	10817	33748	0.775
5.0	13473	12571	46319	1.063

BASIN C₁ OUTLET

Basin C_1

	Outlet	Infiltration	Total Outflow
Depth (Feet)	Flow (CFS)	Flow (CFS)	Flow (CFS)
0	0	0	0.1
1	0.666	0.128	0.794
2	1.095	0.156	1.251
3	1.398	0.186	1.584
4	1.647	0.217	1.864
5	1.862	0.251	2.113

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INPUT INFORMATION

This is a Round Culvert

Pipe diameter = 0.417 ft

Entrance Shape:

Sharp Flush

Culvert Length = 0.667 ft Culvert Slope = 5.00E-03 ft/ft Roughness Coef. = 0.0150 Orifice Coef. of Discharge = 0.700 Entry Loss Coef. 'Ke' = 0.500

Water Head above bottom of Culv. at entrance = 1.000 ft

Output:

Flow Capacity 'Q' = 0.666 cfs Flow Velocity 'V' = 4.876 fps

INPUT INFORMATION

This is a Round Culvert

Pipe diameter = 0.417 ft

Entrance Shape:

Sharp Flush

Culvert Length = 0.667 ft Culvert Slope = 5.00E-03 ft/ft Roughness Coef. = 0.0150 Orifice Coef. of Discharge = 0.700 Entry Loss Coef. 'Ke' = 0.500

Water Head above bottom of Culv. at entrance = 2.000 ft

Output:

Flow Capacity 'Q' = 1.095 cfs Flow Velocity 'V' = 8.020 fps

INPUT INFORMATION

This is a Round Culvert

Pipe diameter = 0.417 ft

Entrance Shape:

Sharp Flush

Culvert Length = 0.667 ft Culvert Slope = 5.00E-03 ft/ft Roughness Coef. = 0.0150 Orifice Coef. of Discharge = 0.700 Entry Loss Coef. 'Ke' = 0.500

Water Head above bottom of Culv. at entrance = 3.000 ft

Output:

Flow Capacity 'Q' = 1.398 cfs Flow Velocity 'V' = 10.24 fps

INPUT INFORMATION

This is a Round Culvert

Pipe diameter = 0.417 ft

Entrance Shape:

Sharp Flush

Culvert Length = 0.667 ft Culvert Slope = 5.00E-03 ft/ft Roughness Coef. = 0.0150 Orifice Coef. of Discharge = 0.700 Entry Loss Coef. 'Ke' = 0.500

Water Head above bottom of Culv. at entrance = 4.000 ft

Output:

Flow Capacity 'Q' = 1.647 cfs Flow Velocity 'V' = 12.06 fps

INPUT INFORMATION

This is a Round Culvert

Pipe diameter = 0.417 ft

Entrance Shape:

Sharp Flush

Culvert Length = 0.667 ft Culvert Slope = 5.00E-03 ft/ft Roughness Coef. = 0.0150 Orifice Coef. of Discharge = 0.700 Entry Loss Coef. 'Ke' = 0.500

Water Head above bottom of Culv. at entrance = 5.000 ft

Output:

Flow Capacity 'Q' = 1.862 cfs Flow Velocity 'V' = 13.64 fps

BASIN D₁ VOLUME

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Basin D_1

Depth (Feet)	Area (Feet ²)	Volume (Feet ³)	Σ Volume (Feet ³)	Σ Volume (Acre-Ft)
0.00	10254	0	0	0.000
0.75	11717	8239	8239	0.189
1.75	13755	12736	20975	0.482
2.75	15894	14824	35799	0.822
3.75	18133	17013	52813	1.212

BASIN D₁ OUTLET

Basin D_1

	Outlet	Infiltration	Total Outflow
Depth (Feet)	Flow (CFS)	Flow (CFS)	Flow (CFS)
0	0	0	0.2
0.75	0.942	0.218	1.160
1.75	2.095	0.256	2.351
2.75	2.809	0.296	3.105
3.75	3.375	0.338	3.713

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INPUT INFORMATION

This is a Box Culvert!

Width = 0.583 ft Height = 0.500 ft

Entrance Shape:

Sharp Flush

Culvert Length = 0.667 ft Culvert Slope = 5.00E-03 ft/ft Roughness Coef. = 0.0150 Orifice Coef. of Discharge = 0.660 Entry Loss Coef. 'Ke' = 0.500

Water Head above bottom of Culv. at entrance = 0.750 ft

Output:

Flow Capacity 'Q' = 0.942 cfs Flow Velocity 'V' = 3.231 fps

INPUT INFORMATION

This is a Box Culvert!

Width = 0.583 ft Height = 0.500 ft

Entrance Shape:

Sharp Flush

Culvert Length = 0.667 ft Culvert Slope = 5.00E-03 ft/ft Roughness Coef. = 0.0150 Orifice Coef. of Discharge = 0.660 Entry Loss Coef. 'Ke' = 0.500

Water Head above bottom of Culv. at entrance = 1.750 ft

Output:

Flow Capacity 'Q' = 2.095 cfs Flow Velocity 'V' = 7.186 fps

INPUT INFORMATION

This is a Box Culvert!

Width = 0.583 ft Height = 0.500 ft

Entrance Shape:

Sharp Flush

Culvert Length = 0.667 ft Culvert Slope = 5.00E-03 ft/ft Roughness Coef. = 0.0150 Orifice Coef. of Discharge = 0.660 Entry Loss Coef. 'Ke' = 0.500

Water Head above bottom of Culv. at entrance = 2.750 ft

Output:

Flow Capacity 'Q' = 2.809 cfs Flow Velocity 'V' = 9.636 fps

INPUT INFORMATION

This is a Box Culvert!

Width = 0.583 ft Height = 0.500 ft

Entrance Shape:

Sharp Flush

Culvert Length = 0.667 ft Culvert Slope = 5.00E-03 ft/ft Roughness Coef. = 0.0150 Orifice Coef. of Discharge = 0.660 Entry Loss Coef. 'Ke' = 0.500

Water Head above bottom of Culv. at entrance = 3.750 ft

<u>Output:</u>

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Flow Capacity 'Q' = 3.375 cfs Flow Velocity 'V' = 11.58 fps

BASIN E₁ VOLUME

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Basin E_1

Depth (Feet)	Area (Feet ²)	Volume (Feet ³)	Σ Volume (Feet ³)	Σ Volume (Acre-Ft)
0.0	6863	0	0	0.000
0.5	7988	3713	3713	0.085
1.5	10313	9150	12863	0.295
2.5	. 12738	11525	24388	0.560

BASIN E₁ OUTLET

 $\mathsf{Basin}\ \mathsf{E_1}$

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	Outlet	Infiltration	Total Outflow
Depth (Feet)	Flow (CFS)	Flow (CFS)	Flow (CFS) [.]
0	0	0	0.1
0.5	0.530	0.149	0.679
1.5	1.604	0.192	1.796
2.5	2.267	0.237	2.504

Basin E1 Outlet - 6" x 6"

OUTPUT INFORMATION

This report is for a channel running full.

The Flow Capacity is 0.530 cfs The flow velocity is 2.121 fps

CHANNEL PROPERTIES

The friction factor 'n' = 0.0150The channel slope = 5.00E-03 ft/ft

'Rect.' Shaped Channel:

Width = 0.500ft Height = 0.500ft Flow Area = 0.250 sq-ft Wetted perimiter = 1.500 ft Hydraulic radius = 0.167 ft Basin E1 Outlet - 6" x 6"

INPUT INFORMATION

This is a Box Culvert!

Width = 0.500 ft Height = 0.500 ft

Entrance Shape:

Sharp Flush

Culvert Length = 0.667 ft Culvert Slope = 5.00E-03 ft/ft Roughness Coef. = 0.0150 Orifice Coef. of Discharge = 0.660 Entry Loss Coef. 'Ke' = 0.500

Water Head above bottom of Culv. at entrance = 1.500 ft

Output:

Flow Capacity 'Q' = 1.604 cfs Flow Velocity 'V' = 6.416 fps

Under Pressure

Basin E1 Outlet - 6" x 6"

INPUT INFORMATION

This is a Box Culvert!

Width = 0.500 ft Height = 0.500 ft

Entrance Shape:

Sharp Flush

Culvert Length = 0.667 ft Culvert Slope = 5.00E-03 ft/ft Roughness Coef. = 0.0150 Orifice Coef. of Discharge = 0.660 Entry Loss Coef. 'Ke' = 0.500

Water Head above bottom of Culv. at entrance = 2.500 ft

Output:

Flow Capacity 'Q' = 2.267 cfs Flow Velocity 'V' = 9.067 fps

Under Pressure

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OFFSITE STORM DRAIN CAPACITIES

Freeway Crossing Existing 24" CSP Pipe Capacity

OUTPUT INFORMATION

This report is for a channel running full.

The Flow Capacity is 14.59 cfs The flow velocity is 4.643 fps

CHANNEL PROPERTIES

The friction factor 'n' = 0.0255The channel slope = 0.0160 ft/ft

Round Channel:

Diameter = 2.000 ft Flow Area = 3.142 sq-ft Wetted perimiter = 6.283 ft Hydraulic radius = 0.500 ft Freeway Crossing Existing 30" CSP Pipe Capacity

OUTPUT INFORMATION

This report is for a channel running full.

The Flow Capacity is 26.45 cfs The flow velocity is 5.388 fps

CHANNEL PROPERTIES

The friction factor 'n' = 0.0255The channel slope = 0.0160 ft/ft

Round Channel:

Diameter = 2.500 ft Flow Area = 4.909 sq-ft Wetted perimiter = 7.854 ft Hydraulic radius = 0.625 ft Freeway Crossing Existing 66" CSP Pipe Capacity

OUTPUT INFORMATION

This report is for a channel running full.

The Flow Capacity is 216.6 cfs The flow velocity is 9.115 fps

CHANNEL PROPERTIES

The friction factor 'n' = 0.0255The channel slope = 0.0160 ft/ft

Round Channel:

Diameter = 5.500 ft Flow Area = 23.76 sq-ft Wetted perimiter = 17.28 ft Hydraulic radius = 1.375 ft

OVERFLOW PARKING LOT CHANNEL C-C FLOW CAPACITY

Overflow Parking Lot Channel Channel C-C: Runoff From Offsite Area F4 Rectuangular Channel: 0.76' Deep x 3' Wide

OUTPUT INFORMATION

This report is for a channel running full.

The Flow Capacity is 11.97 cfs The flow velocity is 5.252 fps

CHANNEL PROPERTIES

The friction factor 'n' = 0.0150The channel slope = 7.00E-03 ft/ft

'Rect.' Shaped Channel:

Width = 3.000ft Height = 0.760ft Flow Area = 2.280 sq-ft Wetted perimiter = 4.520 ft Hydraulic radius = 0.504 ft

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APPENDIX "J"

LEMON STREET OVERTOPPING CALCULATIONS

OFFSITE & LEMON STREET DRAINAGE SUMMARY

Lemon Street Flow Calculations

Storm Drain	Flow (CFS)
24" CSP	14.59
30" CSP	26.45
66" CSP	216.6

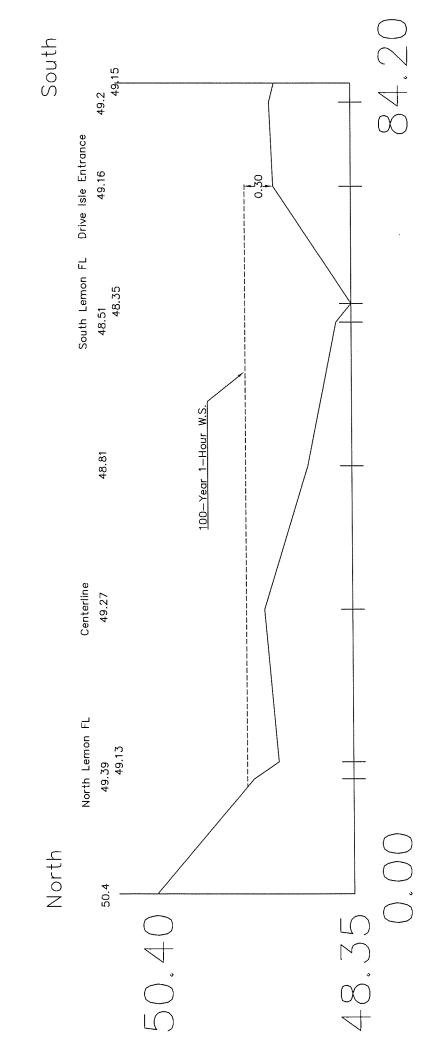
Flow (CFS)
43.77
26.45
216.6
286.82

Storm Event	F ₁ Flow (CFS)	F ₂ Flow (CFS)	F1 Flow Reaching Lemon (CFS)	Total Flow Reaching Lemon (CFS)
2 Year - 1 Hour	95.798	20.312	95.798	116.11
2 Year - 24 Hour	18.394	4.539	18.394	22.933
10 Year - 1 Hour	252.596	44.742	252.596	297.338
10 Year - 24 Hour	87.8	9.299	87.76	97.059
100 Year - 1 Hour	432.2	76.786	286.82	363.606
100 Year - 24 Hour	164.6	18.862	164.612	183.474

Overflow	Calculat	$Q = 0.049 W^{1.4} (\frac{D}{S})^{0.155}$	
Section A-A 100-Year 1-Hour		Width (Feet) Depth (Feet) Slope (Ft/Ft)	50 0.30 0.0295
		Q (CFS)	16.8
Section A-A 10-Year 1-Hour		Width (Feet) Depth (Feet) Slope (Ft/Ft) Q (CFS)	50 0.24 0.0295 16.2
Section B-B 100-Year 1-Hour		Width (Feet) Depth (Feet) Slope (Ft/Ft) Q (CFS)	50 0.05 0.0409 12.1

LEMON STREET CAPACITY A-A EAST DRIVE ISLE ENTRANCE





Vertical Scale 1'=1" Horizontal Scale 1'=10" Lemon Street Capacity Section A-A: East Drive Isle Entrance North Lemon Capacity

OUTPUT INFORMATION

This report is for a channel running full.

The Flow Capacity is 3.399 cfs The flow velocity is 2.889 fps

CHANNEL PROPERTIES

The friction factor 'n' = 0.0150The channel slope = 0.0295 ft/ft

Custom Channel:

Geom. Information:

SIDE 1,(0,0)-(0.950,0.140) LEN = 0.960 SIDE 2,(0.950,0.140)-(16.81,0) LEN = 15.86

Flow Area = 1.177 sq-ft Wetted perimiter = 16.82 ft Hydraulic radius = 0.0700 ft Lemon Street Capacity Section A-A: East Drive Isle Entrance South Lemon Capacity

OUTPUT INFORMATION

This report is for a channel running full.

The Flow Capacity is 143.5 cfs The flow velocity is 9.090 fps

CHANNEL PROPERTIES

The friction factor 'n' = 0.0150The channel slope = 0.0295 ft/ft

Custom Channel:

Geom. Information:

SIDE 1,(0,0)-(11.40,0.350) LEN = 11.41 SIDE 2,(11.40,0.350)-(26.34,0.650) LEN = 14.94 SIDE 3,(26.34,0.650)-(28.29,0.810) LEN = 1.957 SIDE 4,(28.29,0.810)-(40.38,0) LEN = 12.12

Flow Area = 15.78 sq-ft Wetted perimiter = 40.42 ft Hydraulic radius = 0.391 ft Lemon Street Capacity Section A-A: East Drive Isle Entrance 100-Year 1-Hour Storm Depth - 0.30'

OUTPUT INFORMATION

This report is for a channel running full.

The Flow Capacity is 370.9 cfs The flow velocity is 11.15 fps

CHANNEL PROPERTIES

The friction factor 'n' = 0.0150The channel slope = 0.0295 ft/ft

Custom Channel:

Geom. Information:

SIDE 1,(0,0)-(0.820,0.0700) LEN = 0.823 SIDE 2,(0.820,0.0700)-(2.580,0.330) LEN = 1.779 SIDE 3,(2.580,0.330)-(18.44,0.190) LEN = 15.86 SIDE 4,(18.44,0.190)-(33.42,0.650) LEN = 14.99 SIDE 5,(33.42,0.650)-(48.36,0.950) LEN = 14.94 SIDE 6,(48.36,0.950)-(50.31,1.110) LEN = 1.957 SIDE 7,(50.31,1.110)-(62.40,0.300) LEN = 12.12 SIDE 8,(62.40,0.300)-(62.40,0) LEN = 0.300

Flow Area = 33.28 sq-ft Wetted perimiter = 62.77 ft Hydraulic radius = 0.530 ft Lemon Street Capacity Section A-A: East Drive Isle Entrance 10-Year 1-Hour Storm Depth - 0.24'

OUTPUT INFORMATION

This report is for a channel with a specified flow rate.

The input flow rate is 297.3 cfs The flow velocity is 10.38 fps The flow depth is 1.050 ft

CHANNEL PROPERTIES

The friction factor 'n' = 0.0150The channel slope = 0.0295 ft/ft

Custom Channel:

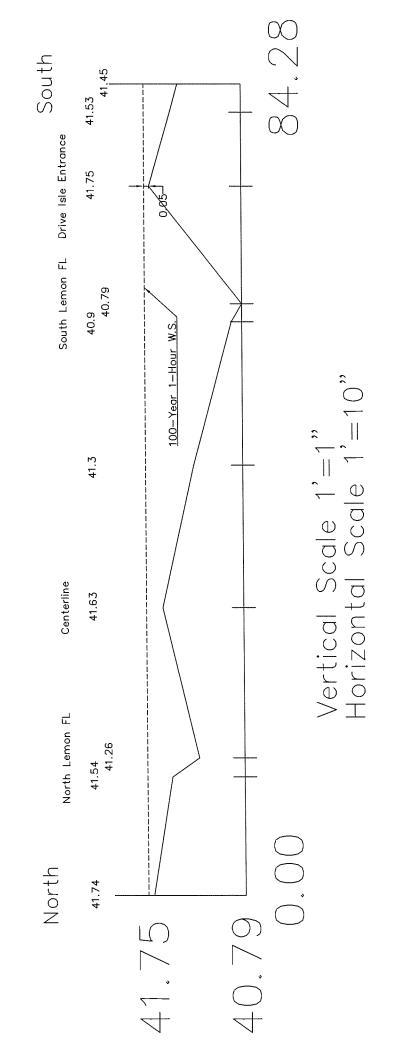
Geom. Information:

SIDE 1,(0,0)-(0.820,0.0700) LEN = 0.823 SIDE 2,(0.820,0.0700)-(2.580,0.330) LEN = 1.779 SIDE 3,(2.580,0.330)-(18.44,0.190) LEN = 15.86 SIDE 4,(18.44,0.190)-(33.42,0.650) LEN = 14.99 SIDE 5,(33.42,0.650)-(48.36,0.950) LEN = 14.94 SIDE 6,(48.36,0.950)-(50.31,1.110) LEN = 1.957 SIDE 7,(50.31,1.110)-(62.40,0.300) LEN = 12.12 SIDE 8,(62.40,0.300)-(62.40,0) LEN = 0.300

Flow Area = 29.56 sq-ft Wetted perimiter = 62.00 ft Hydraulic radius = 0.477 ft

LEMON STREET CAPACITY SECTION B-B WEST DRIVE ISLE ENTRANCE

Cross Section B-B West Drive Isle



Lemon Street Capacity Section B-B: West Drive Isle Entrance Lemon Street Capacity

OUTPUT INFORMATION

This report is for a channel running full.

The Flow Capacity is 298.4 cfs The flow velocity is 10.57 fps

CHANNEL PROPERTIES

The friction factor 'n' = 0.0150The channel slope = 0.0409 ft/ft

Custom Channel:

Geom. Information:

SIDE 1,(0,0)-(0,0.0100) LEN = 0.0100 SIDE 2,(0,0.0100)-(12.26,0.210) LEN = 12.26 SIDE 3,(12.26,0.210)-(14.22,0.490) LEN = 1.980 SIDE 4,(14.22,0.490)-(29.80,0.120) LEN = 15.58 SIDE 5,(29.80,0.120)-(44.70,0.450) LEN = 14.90 SIDE 6,(44.70,0.450)-(59.63,0.850) LEN = 14.94 SIDE 7,(59.63,0.850)-(61.48,0.960) LEN = 1.853 SIDE 8,(61.48,0.960)-(73.62,0) LEN = 12.18

Flow Area = 28.24 sq-ft Wetted perimiter = 73.71 ft Hydraulic radius = 0.383 ft Lemon Street Capacity Section B-B: West Drive Isle Entrance 100-Year 1-Hour Depth - 0.05'

OUTPUT INFORMATION

This report is for a channel running full.

The Flow Capacity is 365.7 cfs The flow velocity is 11.46 fps

CHANNEL PROPERTIES

The friction factor 'n' = 0.0150The channel slope = 0.0409 ft/ft

Custom Channel:

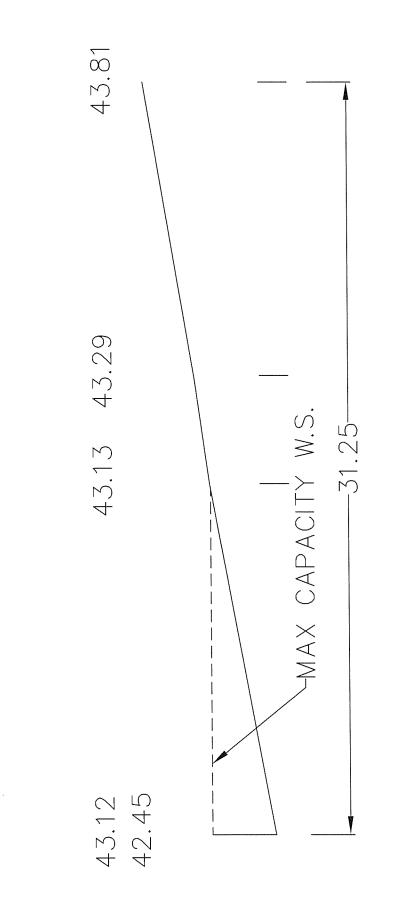
Geom. Information:

SIDE 1,(0,0)-(0,0.0600) LEN = 0.0600 SIDE 2,(0,0.0600)-(12.26,0.260) LEN = 12.26 SIDE 3,(12.26,0.260)-(14.22,0.540) LEN = 1.980 SIDE 4,(14.22,0.540)-(29.80,0.170) LEN = 15.58 SIDE 5,(29.80,0.170)-(44.70,0.500) LEN = 14.90 SIDE 6,(44.70,0.500)-(59.63,0.900) LEN = 14.94 SIDE 7,(59.63,0.900)-(61.48,1.010) LEN = 1.853 SIDE 8,(61.48,1.010)-(73.62,0.0500) LEN = 12.18 SIDE 9,(73.62,0.0500)-(73.62,0) LEN = 0.0500

Flow Area = 31.92 sq-ft Wetted perimiter = 73.81 ft Hydraulic radius = 0.432 ft

PARKING LOT FLOW CAPACITIES





Vertical Scale 1'=1"

Parking Lot Capacity Cross Section C-C 16.8 CFS Overflow From Lemon

OUTPUT INFORMATION

This report is for a channel running full.

The Flow Capacity is 32.75 cfs The flow velocity is 6.808 fps

CHANNEL PROPERTIES

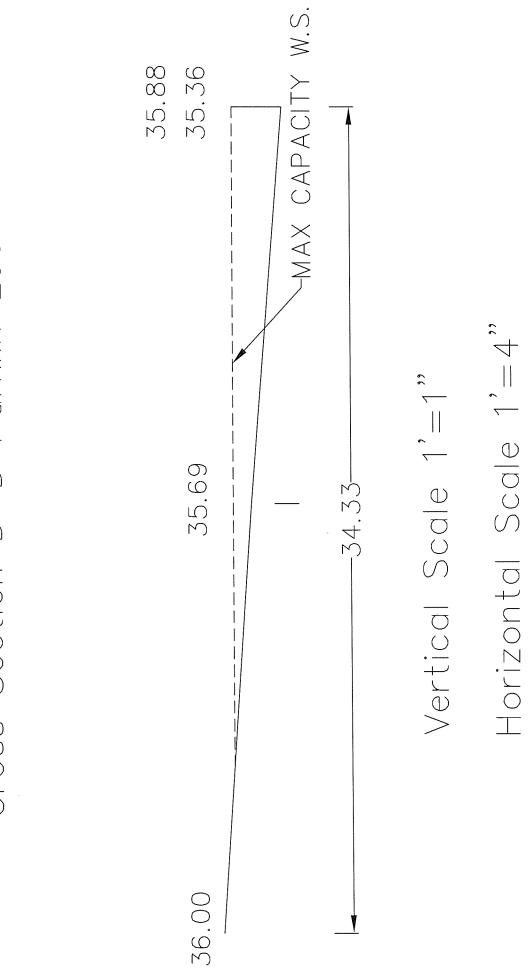
The friction factor 'n' = 0.0150The channel slope = 0.0216 ft/ft

Custom Channel:

Geom. Information:

SIDE 1,(0,0)-(0,0.670) LEN = 0.670 SIDE 2,(0,0.670)-(14.36,0) LEN = 14.38

Flow Area = 4.811 sq-ft Wetted perimiter = 15.05 ft Hydraulic radius = 0.320 ft



Section D-D Parklin Lot Cross

Parking Lot Capacity Cross Section D-D 16.8 CFS Overflow From Lemon

OUTPUT INFORMATION

This report is for a channel running full.

The Flow Capacity is 28.03 cfs The flow velocity is 4.068 fps

CHANNEL PROPERTIES

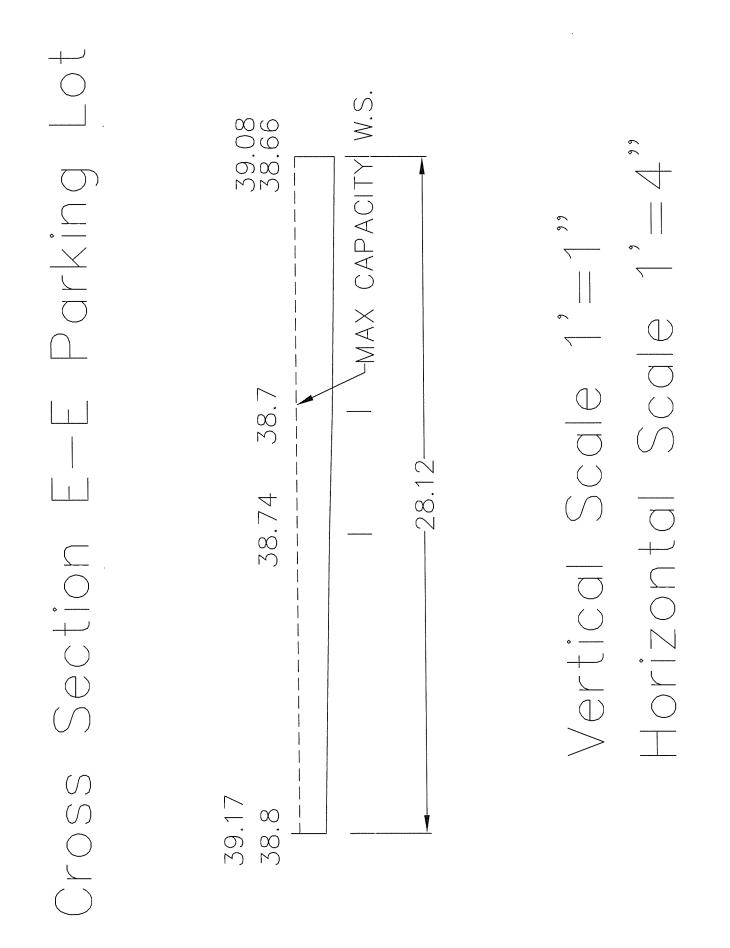
The friction factor 'n' = 0.0150The channel slope = 0.0109 ft/ft

Custom Channel:

Geom. Information:

SIDE 1,(0,0)-(10.93,0.190) LEN = 10.93 SIDE 2,(10.93,0.190)-(27.42,0.520) LEN = 16.49 SIDE 3,(27.42,0.520)-(27.42,0) LEN = 0.520

Flow Area = 6.892 sq-ft Wetted perimiter = 27.94 ft Hydraulic radius = 0.247 ft



Parking Lot Capacity Cross Section E-E 12.1 CFS Overflow From Lemon

OUTPUT INFORMATION

This report is for a channel running full.

The Flow Capacity is 81.69 cfs The flow velocity is 8.231 fps

CHANNEL PROPERTIES

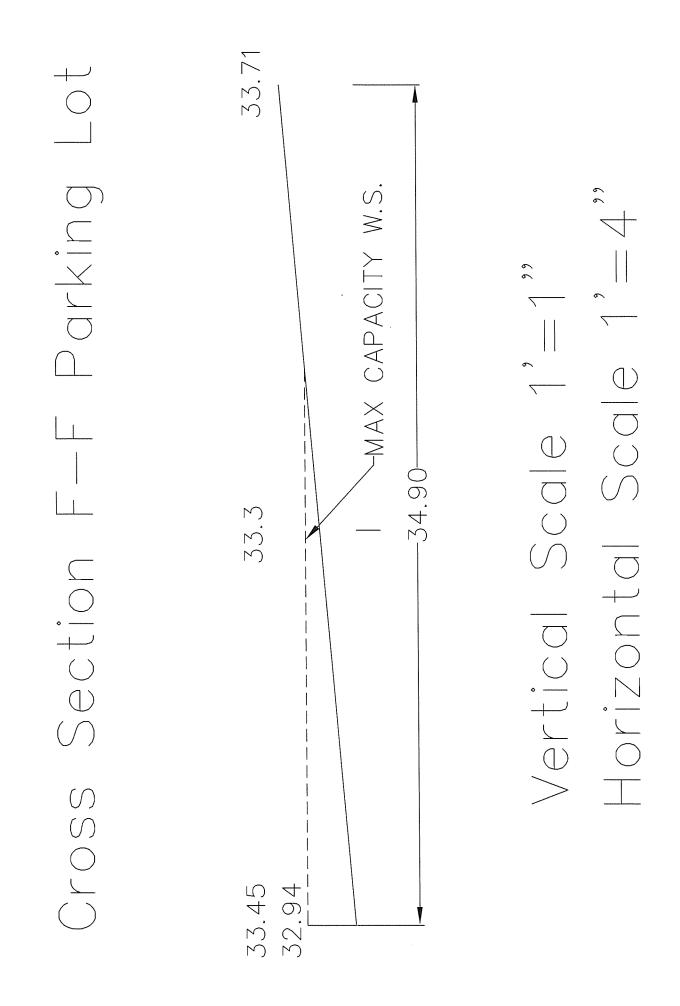
The friction factor 'n' = 0.0150The channel slope = 0.0286 ft/ft

Custom Channel:

Geom. Information:

SIDE 1,(0,0)-(0,0.280) LEN = 0.280 SIDE 2,(0,0.280)-(12.42,0.340) LEN = 12.42 SIDE 3,(12.42,0.340)-(17.56,0.380) LEN = 5.140 SIDE 4,(17.56,0.380)-(28.12,0.420) LEN = 10.56 SIDE 5,(28.12,0.420)-(28.12,0) LEN = 0.420

Flow Area = 9.925 sq-ft Wetted perimiter = 28.82 ft Hydraulic radius = 0.344 ft



Parking Lot Capacity Cross Section F-F 12.1 CFS Overflow From Lemon

OUTPUT INFORMATION

This report is for a channel running full.

The Flow Capacity is 16.44 cfs The flow velocity is 2.779 fps

CHANNEL PROPERTIES

The friction factor 'n' = 0.0150The channel slope = 5.00E-03 ft/ft

Custom Channel:

Geom. Information:

SIDE 1,(0,0)-(0,0.510) LEN = 0.510 SIDE 2,(0,0.510)-(16.39,0.150) LEN = 16.39 SIDE 3,(16.39,0.150)-(23.16,0) LEN = 6.772

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Flow Area = 5.916 sq-ft Wetted perimiter = 23.68 ft Hydraulic radius = 0.250 ft

APPENDIX "K" GEOTECHNICAL REPORT

New Church @ St. Frances of Rome

Wildomar, California

Prepared for: **Diocese of San Bernardino** 1201 E Highland Avenue San Bernardino, CA 92404





Prepared by:

LandMark Consultants, Inc. 77-948 Wildcat Drive Palm Desert, CA 92211 (760) 360-0665

April 2016

a MBE Company

Geo-Engineers and Geologists

April 25, 2016

Mr. David E. Meir Diocese of San Bernardino 1201 E. Highland Avenue San Bernardino, CA 92404 780 N. 4th Street El Centro, CA 92243 (760) 370-3000 (760) 337-8900 fax

77-948 Wildcat Drive Palm Desert, CA 92211 (760) 360-0665 (760) 360-0521 fax

Geotechnical Report New Church @ St Frances of Rome Wildomar, California LCI Report No. LP16027

Dear Mr. Meir:

The attached geotechnical report is provided for design and construction of the proposed new church at St Frances of Rome, 21591 Lemon Street, Wildomar, California. Our geotechnical investigation was conducted in response to your request for our services. The enclosed report describes our soil engineering investigation and presents our professional opinions regarding geotechnical conditions at the site.

The findings of this study indicate the site is underlain by interbedded silty sands with traces of gravels and silty sands, with near surface silty sands with traces of gravels. The near surface, silty sands are expected to be low to non-expansive. The subsurface soils are loose to medium dense in nature. Groundwater was not encountered in the borings (51.5 feet) during the time of exploration.

Elevated sulfate and chloride levels were not encountered in the soil samples tested for this study. However, the soil is severely corrosive to metal. We recommend a minimum of 2,500 psi concrete of Type II Portland Cement with a maximum water/cement ratio of 0.60 (by weight) should be used for concrete placed in contact with native soils of this project.

Evaluation of liquefaction potential at the site indicates that it is unlikely that the subsurface soil will liquefy under seismically induced ground shaking since groundwater is believed to be deeper than 50 feet. No mitigation is required for liquefaction effects at this site.

Seismic settlements of the dry sands have been calculated to be approximately $\frac{1}{2}$ to 1 inch based on the field exploration data. Total seismic settlements are not expected to exceed an inch with differential settlements approximately $\frac{1}{4}$ to $\frac{1}{2}$ inch.

We did not encounter soil conditions that would preclude developing the new church at the site provided the professional opinions contained in this report are implemented in the design and construction of this project. Our findings, professional opinions, and application options are related **only through reading the full report**, and are best evaluated with the active participation of the engineer of record who developed them.

We appreciate the opportunity to provide our findings and professional opinions regarding geotechnical conditions at the site. If you have any questions or comments regarding our findings, please call our office at (760) 360-0665.



Distribution:

Client (electronic copy)

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Section 1 INTRODUCTION

1.1 Project Description

This report presents the findings of our geotechnical exploration and laboratory evaluation of recovered soils for the proposed new church building located in northern portion of St Frances of Rome, 21591 Lemon Street, Wildomar, California (See Vicinity Map, Plate A-1). The proposed development will consist of 1,200 seats new church building, additional car parking areas and other on-site improvements on the existing complex. A site plan for the proposed development was provided by W.J. Mckeever Inc.

The structure is planned to consist of wood and metal frame construction founded on shallow concrete footings and concrete slabs-on-grade. Footing loads at exterior bearing walls are estimated at 2 to 10 kips per lineal foot. Column loads are estimated to range from 5 to 60 kips. If structural loads exceed those stated above, we should be notified so we may evaluate their impact on foundation settlement and bearing capacity. Site development will include mass grading, building pad preparation, underground utility installation, parking lots construction, sidewalk placement, landscape areas and retention basins.

1.2 Purpose and Scope of Work

The purpose of this geotechnical study was to investigate the upper 11.5 to 51.5 feet of subsurface soil at selected locations within the site for evaluation of in-situ soil strength and physical/engineering properties. Professional opinions report regarding geotechnical conditions at this site and the effect on design and construction were developed from field exploration and laboratory evaluation of recovered soils. The scope of our services consisted of the following:

- < Field exploration and in-situ testing of the site soils at selected locations and depths.
- < Laboratory testing for physical and/or chemical properties of selected recovered soil samples.
- < Review of literature and publications pertaining to local geology, faulting, and seismicity.
- < Engineering analysis and evaluation of the data collected.

< Preparation of this report presenting our findings and professional opinion regarding the geotechnical aspects of project design and construction.

This report addresses the following geotechnical parameters:

- < Subsurface soil and groundwater conditions
- < Site geology, regional faulting and seismicity, near-source seismic factors, and site seismic accelerations
- < Liquefaction potential
- < Hydro-Collapse potential
- < Expansive soil and methods of mitigation
- < Aggressive soil conditions to metals and concrete
- < Soil percolation rates of the native soil for retention basin areas

Professional opinions with regard to the above parameters are presented for the following:

- < Mass grading and earthwork
- < Building pad and foundation subgrade preparation
- < Allowable soil bearing pressures and expected settlements
- < Deep Foundations (drilled piers)
- < Concrete slabs-on-grade
- < Mitigation of the potential effects of salt concentrations in native soil to concrete mixes and steel reinforcement
- < Excavation conditions and buried utility installations
- < Lateral earth pressures
- < Seismic design parameters
- < Preliminary Pavement structural sections

Our scope of work for this report did not include an evaluation of the site for the presence of environmentally hazardous materials or conditions.

1.3 Authorization

Mr. David E. Meir of the Diocese of San Bernardino provided authorization by written agreement to proceed with our work on February 19, 2016. We conducted our work according to our written proposal dated January 27, 2016.

Section 2 METHODS OF INVESTIGATION

2.1 Field Exploration

Subsurface exploration was performed on March 15, 2016 using 2R Drilling of Ontario California to advance five (5) borings to depths of 11.5 to 51.5 feet below existing ground surface. The borings were advanced with a truck-mounted, CME 75 drill rig using 8-inch diameter, hollow-stem, continuous-flight augers. The approximate boring locations were established in the field and plotted on the site map by sighting to discernable site features. The boring locations are shown on the Site and Exploration Plan (Plate A-2).

A staff engineer observed the drilling operations and maintained a log of the soil encountered and sampling depths, visually classified the soil encountered during drilling in accordance with the Unified Soil Classification System, and obtained drive tube and bulk samples of the subsurface materials at selected intervals. Relatively undisturbed soil samples were retrieved using a 2-inch outside diameter (OD) split-spoon sampler or a 3-inch OD Modified California Split-Barrel (ring) sampler. The samples were obtained by driving the sampler ahead of the auger tip at selected depths.

The drill rig was equipped with a 140-pound CME automatic hammer with a 30-inch drop for conducting Standard Penetration Tests (SPT) in accordance with ASTM D1586. The number of blows required to drive the samplers the last 12 inches of an 18 inch drive length into the soil is recorded on the boring logs as "blows per foot". Blow count reported on the boring logs represent the field blow counts. No corrections have been applied for effects of overburden pressure, automatic hammer drive energy, drill rod lengths, liners, and sampler diameter.

After logging and sampling the soil, the exploratory borings were backfilled with the excavated material. The backfill was loosely placed and was not compacted to the requirements specified for engineered fill.

The subsurface logs are presented on Plates B-1 through B-6 in Appendix B. A key to the log symbols is presented on Plate B-7. The stratification lines shown on the subsurface logs represent the approximate boundaries between the various strata. However, the transition from one stratum to another may be gradual over some range of depth.

2.2 Laboratory Testing

Laboratory tests were conducted on selected bulk and relatively undisturbed soil samples to aid in classification and evaluation of selected engineering properties of the site soils. The tests were conducted in general conformance to the procedures of the American Society for Testing and Materials (ASTM) or other standardized methods as referenced below. The laboratory testing program consisted of the following tests:

- < Particle Size Analyses (ASTM D422) used for soil classification and liquefaction evaluation.
- < Unit Dry Densities (ASTM D2937) and Moisture Contents (ASTM D2216) used for insitu soil parameters.
- < Moisture-Density Relationship (ASTM D1557) used for soil compaction determinations.
- < Direct Shear (ASTM D3080) used for soil strength determination.
- < Chemical Analyses (soluble sulfates & chlorides, pH, and resistivity) (Caltrans Methods) used for concrete mix evaluations and corrosion protection requirements.

The laboratory test results are presented on the subsurface logs and on Plates C-1 through C-4 in Appendix C.

Engineering parameters of soil strength, compressibility and relative density utilized for developing design criteria provided within this report were either extrapolated from data obtained from the field and laboratory testing program.

Section 3 DISCUSSION

3.1 Site Conditions

The project site is rectangular-shaped in plan view, is relatively flat-lying slopes gently to the north, and consists of approximately 9.5 acres of existing St Frances of Rome wordship complex. The site is bounded by Lemon Street to the north and Orchird Street to the west. Residential homes are surrounding the complex and these properties are flat-lying and are approximately at the same elevation with this site.

The project site lies at an elevation between approximately 1,330 and 1,345 feet above mean sea level (MSL) in the French Valley of Southern California. Annual average rainfall in this region is approximately 11 inches with average summertime temperature highs above 90°F and lows in the mid 50's to low 60's. Average winter temperature highs are in the high 60's with lows in mid 30's to low 40's.

3.2 Geologic Setting

The project site is located within the French Valley, which is located to the east/northeast of the Elsinore-Temecula Trough and to the south of the Perris Plain within the Peninsular Ranges geomorphic province. The Peninsular Ranges are one of the largest geologic units in western North America. They extend 200 kilometers (125 miles) from the Transverse Ranges and the Los Angeles Basin south to the Mexican border and beyond another 1,250 kilometers (775 miles) to the tip of Baja California. The total province varies in width from 48 to 160 Kilometers (30-100 miles) (Norris & Webb, 1976).

The Peninsular Ranges are a northwest-southeast oriented complex of blocks separated by similarly trending faults (Norris & Webb, 1976). Major faults of the Peninsular Ranges are the San Jacinto and related branches within the San Jacinto zone and the Elsinore and associated faults within the Elsinore zone.

The Elsinore-Temecula trough, located to the west/southwest of the project site, is a linear, low-lying block northeast of the Santa Ana Mountains and southwest of the Perris Plain. It extends from

Corona on the northwest about 30 miles (48 km) southeast and has a maximum width of 3 miles (4.8 km). The Perris Plain, located to the north of the project site, is a major topographic feature between the San Jacinto (northeast) and Elsinore (southwest) fault zones. The plain is a broad, nearly flat surface dotted with bedrock hills extending from near Corona southeasterly to Hemet. The average elevation of the Perris Plain is 520 meters (1,700 feet) (Norris & Webb, 1976). The nearby hills to the project site are composed of Mesozoic granitic rocks, Mesozoic intrusive rocks, and upper Jurassic marine rocks. Figure 1 shows the location of the site in relation to regional faults and physiographic features.

The surrounding regional geology includes the San Jacinto and Santa Rosa Mountains to the east/southeast, the Santa Ana Mountains to the west/northwest, the Elsinore Fault zone to the southwest, and the San Jacinto Fault zone to the northeast. Lake Elsinore is located to the west of the project site.

3.3 Subsurface Soil

Subsurface soils encountered during the field exploration conducted on March 15, 2016 consist of dominantly medium dense to dense, silty sands (SM) to a depth of 51.5 feet, the maximum depth of exploration. The near surface soils are granular and non-expansive in nature. The subsurface logs (Plates B-1 through B-6) depict the stratigraphic relationships of the various soil types.

3.4 Groundwater

Groundwater was not encountered in the borings during the time of exploration. Groundwater levels may fluctuate with precipitation, irrigation of adjacent properties, drainage, and site grading. The groundwater level noted should not be interpreted to represent an accurate or permanent condition. Based on the regional topography, groundwater flow is assumed to be generally towards the east to southeast within the site area. Flow directions may vary locally in the vicinity of the site.

Historic groundwater records in the vicinity of the project site indicate that groundwater has fluctuated between 10 to 31 feet below the ground surface within the past 40 years according to The California Department of Water Resources, Division of Planning and Local Assistance web site.

3.5 Faulting

The project site is located in the seismically active French Valley of southern California with numerous mapped faults of the Elsinore Fault Zone traversing the region. We have performed a computer-aided search of known faults or seismic zones that lie within a 62 mile (100 kilometer) radius of the project site (Table 1).

A fault map illustrating known active faults relative to the site is presented on Figure 1, *Regional Fault Map.* Figure 2 shows the project site in relation to local faults. The criterion for fault classification adopted by the California Geological Survey defines Earthquake Fault Zones along active or potentially active faults. An active fault is one that has ruptured during Holocene time (roughly within the last 11,000 years). A fault that has ruptured during the last 1.8 million years (Quaternary time), but has not been proven by direct evidence to have not moved within Holocene time is considered to be potentially active. A fault that has not moved during Quaternary time is considered to be inactive.

Review of the current Alquist-Priolo Earthquake Fault Zone maps (CGS, 2000a) indicates that the nearest mapped Earthquake Fault Zone is the Elsinore-Temecula fault located approximately 1.5 miles southwest of the project site. Riverside County fault maps indicate that the nearest Riverside County mapped fault is the Glen Ivy segment of the Elsinore Fault Zone located approximately 0.2 miles southwest of the project site. A portion of the project site lies within the County Fault Zone boundary and may require additional evaluation.

3.6 General Ground Motion Analysis

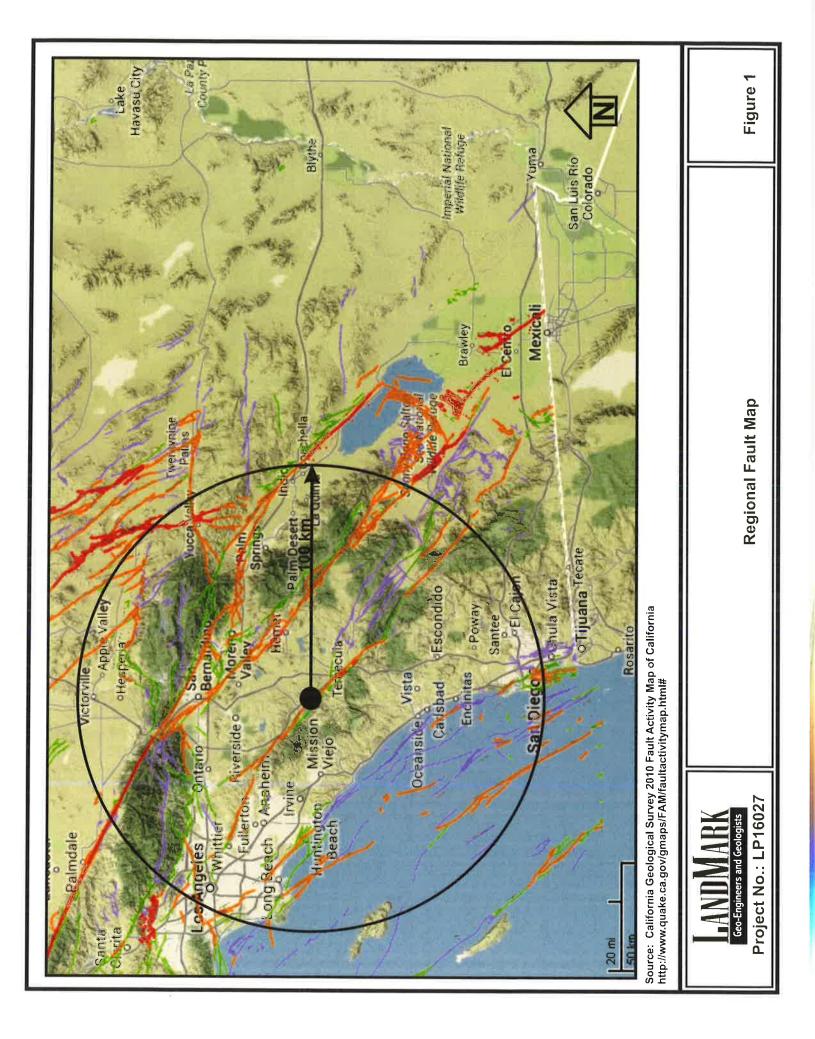
The project site is considered likely to be subjected to moderate to strong ground motion from earthquakes in the region. Ground motions are dependent primarily on the earthquake magnitude and distance to the seismogenic (rupture) zone. Acceleration magnitudes also are dependent upon attenuation by rock and soil deposits, direction of rupture and type of fault; therefore, ground motions may vary considerably in the same general area.

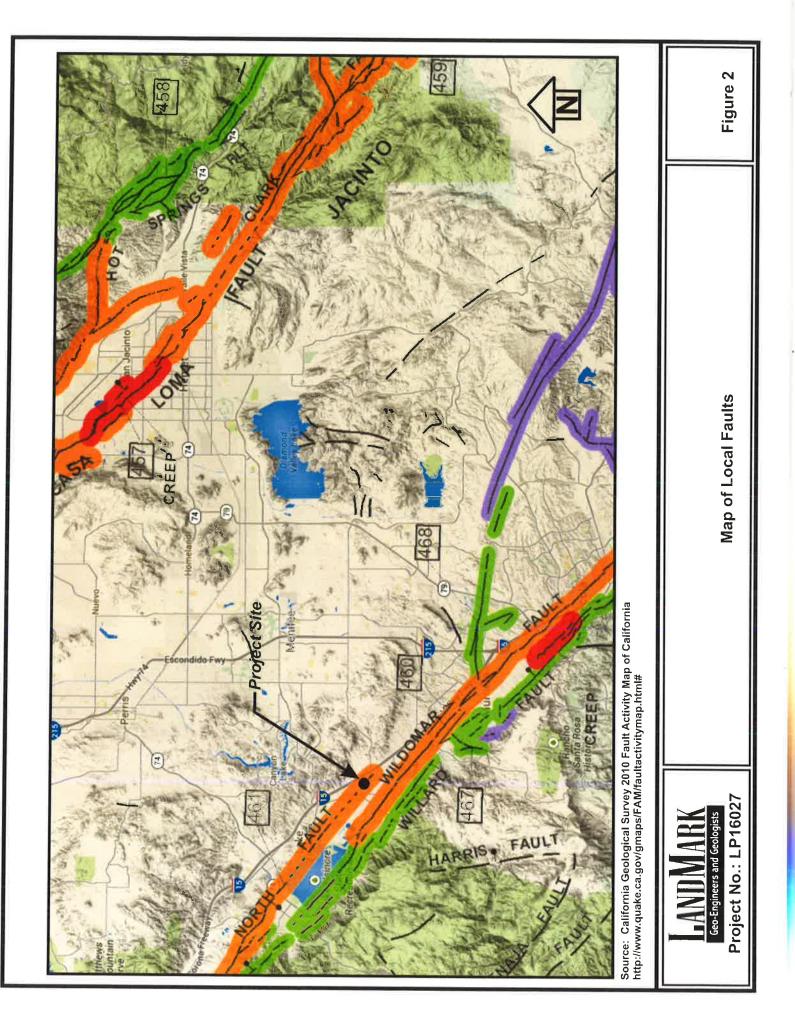
Fault Name	Approximate Distance (miles)	Approximate Distance (km)	Maximum Moment Magnitude (Mw)	Fault Length (km)	Slip Rate (mm/yr)
Elsinore - Glen Ivy	0.2	0.3	6.8	36 ± 4	5 ± 2
Elsinore - Temecula	1.5	2.4	6.8	43 ± 4	5 ± 2
Whittier	16.8	26.9	6.8	38 ± 4	2.5 ± 1
Chino Avenue	18.5	29.6	6.7	28 ± 3	1 ± 1
San Jacinto - San Jacinto Valley	22.0	35.1	6.9	43 ± 4	12 ± 6
San Jacinto - Anza	22.1	35.4	7.2	91 ± 9	12 ± 6
San Joaquin Hills	22.4	35.8	6.6	28 ± 3	0.5 ± 0.2
Elsinore - Julian	24.0	38.5	7.1	76 ± 8	5 ± 2
San Jacinto - San Bernardino	26.8	42.9	6.7	36 ± 4	12 ± 6
Newport-Inglewood (offshore)	28.8	46.0	7.1	66 ± 7	1.5 ± 0.5
San Andreas - San Bernardino (South)	34.1	54.6	7.4	103 ± 10	30 ± 7
Rose Canyon	35.1	56.1	7.2	70 ± 7	1.5 ± 0.5
Newport-Inglewood	36.7	58.7	7.1	66 ± 7	1 ± 0.5
Cucamonga	39.0	62.4	6.9	28 ± 3	5 ± 2
Puente Hills Blind Thrust	39.4	63.1	7.1	44 ± 4	0.7 ± 0.4
Garnet Hill *	40.0	64.1			
San Jose	40.4	64.6	6.4	20 ± 2	0.5 ± 0.5
Sierra Madre	42.7	68.4	7.2	57 ± 6	2 ± 1
Pinto Mtn.	43.7	69.9	7.2	74 ± 7	2.5 ± 2
Cleghorn	44.4	71.0	6.5	25 ± 3	3 ± 2
Coronado Bank	44.9	71.9	7.6	185 ± 19	3 ± 1
Palos Verdes	45.0	72.1	7.3	96 ± 10	3 ± 1

 Table 1

 Summary of Characteristics of Closest Known Active Faults

* Note: Faults not included in CGS database.





								EXPLANATIO	N	
			located are que maps o trend or	or inferred, a ried where co f selected su ily All offshore	ind li ontin ibsu re fa	by doth luation Inface I Iults ba	ed lir or e noriz ased	es where concealed by young xislence is uncertain Conceale ons, so locations shown are a	d, by dashed lines where approxim er rocks or by lakes or bays. Fault lu ed faults in the Great Valley are bass approximate and may indicate stru- cords are shown as solid lines where	aces ed on clural
						F		T CLASSIFICATION COL Indicating Recency of Move		
	-		Fault at of the fo	ong which his	stori	c (last			rred and is associated with one or	more
			(a) a re caused fault, ca indicate	corded earth by ground s iused by the d. Where rep	haki An beal	ing du vin-Tel ed sur	ring hach face	earthquakes, e.g. extensive g api earthquake of 1952). The ruptures on the same fault ha	are some well-defined surface br round breakage, not on the White a date of the associated aarthqua ave occurred, only the date of the l well documented as to location of gr	Wolf ke is atest
				creep slippa aced survey			rour	d displacement usually withou	t accompanying earthquakes	
•	4	14.16	red trian	e to the right gle indicates d location of	kno	wn loc	atior	of rupture termination point C	of observed surface displacement Open black triangle indicates uncerta	Solid aín or
P 10		_	Date bra	icketed by tri	ang	les ind	icale	s local fault break		
197	2	-	No trian	gle by date in	ndica	ales ar	inte	rmediate point along fault brea	ak	
412	-	-	Fault that with lead	at exhibits la ler) indicates	ult o s rep	reep s resent	slippa lative	age. Hachures indicate linear locations where fault creep ha	extent of fault creep. Annotation (c as been observed and recorded	reep
	-	1968	on some nal point	other fault [Date vhicl	of cau h trigg	isativ	e earthquake indicated. Squar	hat has been toggered by an earthq res to right and left of date indicate to (creep either continuous or interm	ermi-
-	-	4,	Holocen age dep	e faulting inc osits: offset	lude stre	am con	pono	is, scarps showing little erosic	istoric record. Geomorphic evidence n, or the following features in Holo , and triangular faceted spurs. Rec , strata displaced by faulting	cene
		de-	Late Qu describe	aternary fau d for Holoce	lt di ene	splace faults	men exce	t (during past 700,000 years)) Geomorphic evidence similar to Faulting may be younger, but lac	lhat k of
-	-	3	Quatema time duri ated Plic	ary fault (age ng the past	uno 16 ago	differer million e_Unn	ntiate year umb	d) Most faults of this category s; possible exceptions are fau ered Quatemary faults were b	y show evidence of displacement so ilts which displace rocks of undiffer pased on Fault Map of California, 1	enti-
		4	Pre-Qua displace of recom	ternary fault ment. Some	(old fauli ure,	er that Is are s or was	16і show	nillion years) or fault without ro n in this calegory because the done with the object of dating	source of mapping used was	
								DDITIONAL FAULT SYMI	BOLS	
			Bar and	ball on down	lhro	wn sid	e (re	lative or apparent)		
			Arrows a	long fault ind	lical	e relat	ive o	r apparent direction of lateral r	novement	
		2	Arrow on	fault indicate	es d	irectio	n of c	lip		
•		• 2	Low angl subseque of dip	e fault (barbs ently steeper	on ned	upper On off	plate Ishor	 Fault surface generally dips e faults, barbs simply indicate 	less lhan 45° but locally may have t a reverse fault regardless of steep	ness
								OTHER SYMBOLS		
<u>15</u>	7		name, ag fault has gist to de	je of fault dis been zoned i lineale zone:	plac by tř s to	ement te Alqu encon	; and Jist-F apass	d pertinent references including Priolo Earthquake Fault Zoning s faults with Holocene displace		ere a eolo-
	-			l discontinuit tween baser				parating differing Neogene str	uctural domains. May indicate disc	onli-
m	////	///	Brawley step betw	Seismic Zoni veen the Imp	e, a eria	linear I and S	zone San A	e of seismicity locally up to 10 Indreas faults	km wide associated with the relea	sing
Ge	ologi	c	Years Before	Fault	P	lecen	.v	DESCR	IPTION	
1	Time Scale		Present (Approx_)	Symbol		of	1	ON LAND	OFFSHORE	
		Historic						Displacement during historic time (Includes areas of known fault creep	e g. San Andreas fault 1906) 5	
	alemary.	Holocens	200	-		1	-	Displacement during Holocens Ime	Fault offsets seafloor sediments	
7	Late Quatemary	H	- 11,700					Faults showing avidance of displacement during inte	Fault cuts strate of Late Pointocare age	
Quaternary		;uc				Î	Ĩ	Ousternary line		
Qua	čarly Quatemary	Plaintocene	- 700,000				-2-	Undivided Quaternary faults - most faults in Uhis category show avidence of displacement during the last I,600,000 years, possible exceptions are faults which displace rocks of undifferentiated Pilo Pleistocene	Paul cuts stols of Quaternary age	

* Qualernary now recognized as extending to 2.6 Ma (Walker and Geissman, 2009) Quaternary faults in this map were established using the previous 1.6 Ma criterion.

1

Faults without recognized Quaternary displacement or showing evidence of no displacement during Quaternary time. Not necessarily inactive Fault cuts strata of Plocene or older age.

-1.600.000 -

4.5 billion (Age of Earth)

Pre-Quatemary

<u>CBC General Ground Motion Parameters:</u> The 2013 CBC general ground motion parameters are based on the Risk-Targeted Maximum Considered Earthquake (MCE_R). The U.S. Geological Survey "U.S. Seismic Design Maps Web Application" (USGS, 2014) was used to obtain the site coefficients and adjusted maximum considered earthquake spectral response acceleration parameters. The site soils have been classified as Site Class D (stiff soil profile). Design spectral response acceleration parameters are defined as the earthquake ground motions that are two-thirds (2/3) of the corresponding MCE_R ground motions. Design earthquake ground motion parameters are provided in Table 2. *A Risk Category II was determined using Table 1604.5 and the Seismic Design Category is E since* S_1 *is greater than 0.75.*

The Maximum Considered Earthquake Geometric Mean (MCE_G) peak ground acceleration (PGA_M) value was determined from the "U.S. Seismic Design Maps Web Application" (USGS, 2013) for liquefaction and seismic settlement analysis in accordance with 2013 CBC Section 1803.5.12 and CGS Note 48 (PGA_M = $F_{PGA}*PGA$). *A PGA_M value of 0.94g has been determined for the project site*.

3.7 Seismic and Other Hazards

► **Groundshaking.** The primary seismic hazard at the project site is the potential for strong groundshaking during earthquakes along the Temecula Segment of the Elsinore Fault Zone. A further discussion of groundshaking follows in Section 3.4.

► Surface Rupture. The project site does not lie within a State of California, Alquist-Priolo Earthquake Fault Zone. The project site lies within the Riverside County designated fault zone for the Glen Ivy fault segment of the Elsinore Fault. Surface fault rupture is considered to be unlikely at the project site because of the well-delineated fault lines through the French Valley as shown on USGS, CDMG, and Riverside County maps. However, because of the high tectonic activity and deep alluvium of the region, we cannot preclude the potential for surface rupture on undiscovered or new faults that may underlie the site.

► Liquefaction. Liquefaction is unlikely to be a potential hazard at the site, due to groundwater deeper than 50 feet (the maximum depth that liquefaction is known to occur).

20	ן 13 California Building Code (CI	'able 2 BC) and A	ASCE 7-10) Seismic Para	meters	
				CBC Reference		
	Soil Site Class:	D		Table 20.3-1		
	Latitude:	33.6333	Ν	14010 2010 1		
	Longitude:		W			
	Risk Category:	II				
	Seismic Design Category:	Е				
	Maximum Considered Earthqua	ke (MCE)	Ground Mo	tion		
Mappe	d MCE _o Short Period Spectral Response	Ss	2.351 g	Figure 1613.3.1((1)	
Ma	pped MCE _R 1 second Spectral Response	\mathbf{S}_1	0.946 g	Figure 1613.3.1(
	Short Period (0.2 s) Site Coefficient	Fa	1.00	Table 1613.3.3(
	Long Period (1.0 s) Site Coefficient	F _v	1.50	Table 1613.3.3(2	-	
MCE _o Spectral	Response Acceleration Parameter (0.2 s)	S _{MS}	2.351 g	$= F_a * S_s$	Equation 10	6-37
-	Response Acceleration Parameter (1.0 s)	S_{MS} S_{M1}	1.419 g	$= F_a + S_s$ = $F_v + S_1$	Equation 10	
meng opeend		S _{M1}	1.419 g	$= \Gamma_v \cdot S_1$		0-36
	Design Earthquake Groun	d Motion				
Design Spectral	Response Acceleration Parameter (0.2 s)	S _{DS}	1.567 g	$= 2/3 * S_{MS}$	Equation 1	5-39
Design Spectral	Response Acceleration Parameter (1.0 s)	S _{D1}	0.946 g	$= 2/3 * S_{M1}$	Equation 10	6-40
		TL	8.00 sec		ASCE Figu	
		To		$=0.2*S_{DI}/S_{DS}$	8	
		T _s	0.60 sec			
	Peak Ground Acceleration	PGA _M	0.94 g	-0D[(0DS	ASCE Equa	ation 11.8
	1		-	Perioc	Sa	MCEB
	Generalized Design Response S			T (sec) (g)	(g)
	(ASCE 7-10 Section 11.4.5)		0.00	0.63	0.94
2.5				0.12	1.57	2.35
				0.60	1.57	2.35
				0.70	1.35	2.03
						1 1 77
⇒ ²⁰	┼┼┼╲┼┼┼┼┼┼┼┼┼┼┼┼┼			0.80	1.18	1.77
				0.90	1.05	1.58
Sa (g				0.90	1.05 0.95	1.58 1.42
Sa (g	·			0.90 1.00 1.10	1.05 0.95 0.86	1.58 1.42 1.29
Sa (g	·· \ \ \ \			0.90 1.00 1.10 1.20	1.05 0.95 0.86 0.79	1.58 1.42 1.29 1.18
Sa (g				0.90 1.00 1.10 1.20 1.20	1.05 0.95 0.86 0.79 0.79	1.58 1.42 1.29 1.18 1.18
Sa (g				0.90 1.00 1.10 1.20 1.20 1.40	1.05 0.95 0.86 0.79 0.79 0.68	1.58 1.42 1.29 1.18 1.18 1.01
Sa (g				0.90 1.00 1.10 1.20 1.20 1.40 1.50	1.05 0.95 0.86 0.79 0.79 0.68 0.63	1.58 1.42 1.29 1.18 1.18 1.01 0.95
Sa (g				0.90 1.00 1.10 1.20 1.20 1.40 1.50 1.75	1.05 0.95 0.86 0.79 0.79 0.68 0.63 0.54	1.58 1.42 1.29 1.18 1.18 1.01 0.95 0.81
cceleration, Sa (g				0.90 1.00 1.10 1.20 1.20 1.40 1.50 1.75 2.00	1.05 0.95 0.86 0.79 0.79 0.68 0.63 0.54 0.47	1.58 1.42 1.29 1.18 1.18 1.01 0.95 0.81 0.71
Spectral Acceleration, Sa (9				0.90 1.00 1.10 1.20 1.20 1.40 1.50 1.75 2.00 2.20	1.05 0.95 0.86 0.79 0.79 0.68 0.63 0.54 0.47 0.43	1.58 1.42 1.29 1.18 1.18 1.01 0.95 0.81 0.71 0.65
Spectral Acceleration, Sa (g				0.90 1.00 1.10 1.20 1.20 1.40 1.50 1.75 2.00 2.20 2.40	1.05 0.95 0.86 0.79 0.79 0.68 0.63 0.54 0.47 0.43 0.39	1.58 1.42 1.29 1.18 1.18 1.01 0.95 0.81 0.71 0.65 0.59
Spectral Acceleration, Sa (9				0.90 1.00 1.10 1.20 1.20 1.40 1.50 1.75 2.00 2.20 2.40 2.60	1.05 0.95 0.86 0.79 0.79 0.68 0.63 0.54 0.47 0.43 0.39 0.36	1.58 1.42 1.29 1.18 1.18 1.01 0.95 0.81 0.71 0.65 0.59 0.55
Spectral Acceleration, Sa (9		.5 3.0	3.5	0.90 1.00 1.10 1.20 1.20 1.40 1.50 1.75 2.00 2.20 2.40 2.60 2.80	1.05 0.95 0.86 0.79 0.79 0.68 0.63 0.54 0.47 0.43 0.39 0.36 0.34	1.58 1.42 1.29 1.18 1.01 0.95 0.81 0.71 0.65 0.59 0.55 0.51
Spectral Acceleration, Sa (g		.5 3.0	3.5	0.90 1.00 1.10 1.20 1.20 1.40 1.50 1.75 2.00 2.20 2.40 2.60 2.80	1.05 0.95 0.86 0.79 0.79 0.68 0.63 0.54 0.47 0.43 0.39 0.36	1.58 1.42 1.29 1.18 1.01 0.95 0.81 0.71 0.65 0.59 0.55

 MCE_R Response Spectra

Other Potential Geologic Hazards.

► Landsliding. The hazard of landsliding is unlikely due to the regional planar topography. No ancient landslides are shown on geologic maps of the region and no indications of landslides were observed during our site investigation.

► Volcanic hazards. The site is not located in proximity to any known volcanically active area and the risk of volcanic hazards is considered very low.

► **Tsunamis, sieches, and flooding.** The site does not lie near any large bodies of water, so the threat of tsunami, sieches, or other seismically-induced flooding is unlikely.

► Expansive soil. The near surface soils at the project site consist of silty sands which are non-expansive.

3.8 Seismic Settlement

An evaluation of the non-liquefaction seismic settlement potential was performed using the relationships developed by Tokimatsu and Seed (1984, 1987) for dry sands. This method is an empirical approach to quantify seismic settlement using SPT blow counts and PGA estimates from the probabilistic seismic hazard analysis.

The soils beneath the site consist primarily of loose to medium dense silty sands to maximum penetrated. Based on the empirical relationships, total induced settlements are estimated to be on the order or $\frac{1}{2}$ to1 inch in the event of a MCE_G earthquake (0.94g peak ground acceleration). Should settlement occur, buried utility lines and the buildings may not settle equally. Therefore we recommend that utilities, especially at the points of entry to the buildings, be designed to accommodate differential movement.

The computer printouts for the estimates of induced settlement are included in Appendix D.

3.9 Hydroconsolidation

In arid climatic regions, granular soils have a potential to collapse upon wetting. This collapse (hydroconsolidation) phenomena is the result of the lubrication of soluble cements (carbonates) in the soil matrix causing the soil to densify from its loose configuration during deposition.

Based on our experience in the vicinity of the project site, there is a slight risk of collapse upon inundation from at the site. Therefore, development of building foundation is not required to include provisions for mitigating the hydroconsolidation caused by soil saturation from landscape irrigation or broken utility lines.

3.10 Soil Infiltration Rate

A total of four (4) infiltration tests were conducted on March 18, 2016 at the proposed location for the on-site storm-water retention basins as shown on the Site and Exploration Plan (Plate A-2). The infiltration tests were performed to the guideline from Design Handbook for Low Impact Development Best Management Practices, prepared by Riverside County Flood Control and Water Conservation District, Appendix A, Section 2.3, dated September 2011.

The tests were performed using perforated pipes inside an 8-inch diameter flight auger borehole made to depths of approximately 5.0 feet below the existing ground surface, corresponding to the anticipated bottom depth of the stormwater retention basin. The pipes were filled with water and successive readings of drop in water levels were made every 10 minutes for a total elapsed time of 60 minutes, until a stabilization drop was recorded.

The test results indicate that the stabilized soil infiltration rate for the soil ranges from 1.61 to 1.98 inches per hour. A maximum soil infiltration rate of 1.61 inches per hour may be used for the on-site storm-water retention basin design. An oil/water separator should be installed at inlets to the stormwater retention basin to prevent sealing of the basin bottom with silt and oil residues. The field and conversion calculation worksheets are included in Appendix E.

We recommend additional testing should be performed after the completion of rough grading operations, to verify the soil infiltration rate.

Section 4 **DESIGN CRITERIA**

4.1 Site Preparation

<u>Pre-grade Meeting</u>: Prior to site preparation, a meeting should be held at the site with as a minimum, the owner's representative, grading contractor and geotechnical engineer in attendance.

<u>Clearing and Grubbing</u>: All surface improvements, debris and/or vegetation including grass, trees, and weeds on the site at the time of construction should be removed from the construction area. Root balls should be completely excavated. Organic stripping should be hauled from the site and not used as fill. Any trash, construction debris, concrete slabs, old pavement, landfill, and buried obstructions such as old foundations and utility lines exposed during rough grading should be traced to the limits of the foreign materials and removed. Any excavations resulting from site clearing and grubbing should be dish-shaped to the lowest depth of disturbance and backfilled with engineered fill.

<u>Mass Grading</u>: Prior to placing any fills, the surface 12 inches of soil should be removed, the exposed surface uniformly moisture conditioned to a depth of 8 inches by discing and wetting to $\pm 2\%$ of optimum moisture, and re-compacted to at least 90% of ASTM D1557 maximum density. Native soils may be used for mass grading, placed in 6 inch maximum lifts, uniformly moisture conditioned to a depth of 8 inches by discing and wetting to $\pm 2\%$ of optimum moisture, and re-compacted in 6 inch maximum lifts, uniformly moisture conditioned to a depth of 8 inches by discing and wetting to $\pm 2\%$ of optimum moisture, and re-compacted to at least 90% of ASTM D1557 maximum density.

<u>Building Pad Preparation</u>: The exposed surface soil within the proposed building pad areas should be removed to 30 inches below the lowest foundation grades, or 60 inches below the original grade (whichever is deeper), extending five feet beyond all exterior wall/column lines (including adjacent concrete areas). The exposed sub-grade shall be saturated to a minimum depth of 5 feet and compacted with a vibratory steel drum roller to achieve a minimum compaction of 95% of the maximum dry density. Moisture penetration and compaction should be verified prior to construction of the engineered fill pad. After achieving the recommended compaction, the engineered building pad may be constructed by placing the removed soils in uniformly moisture conditioned to $\pm 2\%$ of optimum moisture, and recompacted to at least 90% of ASTM D1557 maximum density.

The on-site soils are suitable for use as compacted fill and utility trench backfill. Imported fill soil (if required) should similar to onsite soil or non-expansive, granular soil meeting the USCS classifications of SM, SP-SM, or SW-SM with a maximum rock size of 3 inches. *The geotechnical engineer should approve imported fill soil sources before hauling material to the site*. Native and imported materials should be placed in lifts no greater than 8 inches in loose thickness, uniformly moisture conditioned to $\pm 2\%$ of optimum moisture, and re-compacted to at least 90% of ASTM D1557 maximum density.

In areas other than the building pad which are to receive concrete slabs and asphalt concrete pavement, the ground surface should be over-excavated to a depth of 12 inches, uniformly moisture conditioned to $\pm 2\%$ of optimum moisture, and re-compacted to at least 90% of ASTM D1557 maximum density.

<u>Trench Backfill</u>: On-site soil free of debris, vegetation, and other deleterious matter may be suitable for use as utility trench backfill. Backfill within roadways should be placed in layers not more that 6 inches in thickness, uniformly moisture conditioned to $\pm 2\%$ of optimum moisture and mechanically compacted to a minimum of 90% of the ASTM D1557 maximum dry density except for the top 12 inches of the trench which shall be compacted to at least 95%. Native backfill should only be placed and compacted after encapsulating buried pipes with suitable bedding and pipe envelope material.

Pipe envelope/bedding should either be clean sand (Sand Equivalent SE>30) or crushed rock when encountering groundwater. A geotextile filter fabric (Mirafi 140N or equivalent) should be used to encapsulate the crushed rock to reduce the potential for in-washing of fines into the gravel void space. Precautions should be taken in the compaction of the backfill to avoid damage to the pipes and structures.

Adequate site drainage is essential to future performance of the project. Infiltration of excess irrigation water and stormwaters can adversely affect the performance of the subsurface soil at the site. Positive drainage should be maintained away from all structures (5% for 5 feet minimum across unpaved areas) to prevent ponding and subsequent saturation of the native soil. Gutters and

downspouts may be considered as a means to convey water away from foundations. If landscape irrigation is allowed next to the building, drip irrigation systems or lined planter boxes should be used. The subgrade soil should be maintained in a moist, but not saturated state, and not allowed to dry out. Drainage should be maintained without ponding.

Observation and Density Testing: All site preparation and fill placement should be continuously observed and tested by a representative of a qualified geotechnical engineering firm. Full-time observation services during the excavation and scarification process is necessary to detect undesirable materials or conditions and soft areas that may be encountered in the construction area. The geotechnical firm that provides observation and testing during construction shall assume the responsibility of "*geotechnical engineer of record*" and, as such, shall perform additional tests and investigation as necessary to satisfy themselves as to the site conditions and the recommendations for site development.

<u>Auxiliary Structures Foundation Preparation:</u> Auxiliary structures such as free standing or retaining walls should have the existing soil beneath the structure foundation prepared in the manner recommended for the building pad except the preparation needed only to extend 30 inches below and beyond the footing.

4.2 Foundations and Settlements

Shallow column footings and continuous wall footings are suitable to support the structures provided they are founded on a layer of properly prepared and compacted soil as described in Section 4.1. The foundations may be designed using an allowable soil bearing pressure of 2,000 psf. The allowable soil pressure may be increased by 20% for each foot of embedment depth in excess of 18 inches and by one-third for short term loads induced by winds or seismic events. The maximum allowable soil pressure at increased embedment depths shall not exceed 2,800 psf.

All exterior and interior] foundations should be embedded a minimum of 18 inches below the building support pad or lowest adjacent final grade, whichever is deeper. Continuous wall footings should have a minimum width of 12 inches. Isolated column footings should have a minimum width of 24 inches. *Recommended concrete reinforcement and sizing for all footings should be provided by the structural engineer.*

Resistance to horizontal loads will be developed by passive earth pressure on the sides of footings and frictional resistance developed along the bases of footings and concrete slabs. Passive resistance to lateral earth pressure may be calculated using an equivalent fluid pressure of 300 pcf to resist lateral loadings. The top one foot of embedment should not be considered in computing passive resistance unless the adjacent area is confined by a slab or pavement. An allowable friction coefficient of 0.35 may also be used at the base of the footings to resist lateral loading.

Foundation movement under the estimated static loadings and seismic site conditions are estimated to not exceed ³/₄ inch with differential movement of about two-thirds of total movement for the loading assumptions stated above when the subgrade preparation guidelines given above are followed. Foundation movements under the seismic loading due to dry settlement are provided in Section 3.8 of this report.

4.3 Slabs-On-Grade

Concrete slabs and flatwork should be a minimum of 5 inches thick. Concrete floor slabs may either be monolithically placed with the foundation or dowelled after footing placement. The concrete slabs may be placed on granular subgrade that has been compacted at least 90% relative compaction (ASTM D1557).

American Concrete Institute (ACI) guidelines (ACI 302.1R-04 Chapter 3, Section 3.2.3) provide recommendations regarding the use of moisture barriers beneath concrete slabs. The concrete floor slabs should be underlain by a 10-mil polyethylene vapor retarder that works as a capillary break to reduce moisture migration into the slab section. All laps and seams should be overlapped 6-inches or as recommended by the manufacturer. The vapor retarder should be protected from puncture. The joints and penetrations should be sealed with the manufacturer's recommended adhesive, pressure-sensitive tape, or both. The vapor retarder should extend a minimum of 12 inches into the footing excavations. The vapor retarder should be covered by 4 inches of clean sand (Sand Equivalent SE>30) unless placed on 2.5 feet of granular fill, in which case, the vapor retarder may lie directly on the granular fill with 2 inches of clean sand cover.

Placing sand over the vapor retarder may increase moisture transmission through the slab, because it provides a reservoir for bleed water from the concrete to collect. The sand placed over the vapor

retarder may also move and mound prior to concrete placement, resulting in an irregular slab thickness. For areas with moisture sensitive flooring materials, ACI recommends that concrete slabs be placed without a sand cover directly over the vapor retarder, provided that the concrete mix uses a low-water cement ratio and concrete curing methods are employed to compensate for release of bleed water through the top of the slab. The vapor retarder should have a minimum thickness of 15-mil (Stego-Wrap or equivalent).

Concrete slab and flatwork reinforcement should consist of chaired rebar slab reinforcement (minimum of No. 4 bars at 18-inch centers, both horizontal directions) placed at slab mid-height to resist potential swell forces and cracking. *Slab thickness and steel reinforcement are minimums only and should be verified by the structural engineer/designer knowing the actual project loadings.* The construction joint between the foundation and any mowstrips/sidewalks placed adjacent to foundations should be sealed with a polyurethane based non-hardening sealant to prevent moisture migration between the joint.

Control joints should be provided in all concrete slabs-on-grade at a maximum spacing (in feet) of 2 to 3 times the slab thickness (in inches) as recommended by American Concrete Institute (ACI) guidelines. All joints should form approximately square patterns to reduce randomly oriented contraction cracks. Contraction joints in the slabs should be tooled at the time of the pour or sawcut (¼ of slab depth) within 6 to 8 hours of concrete placement. Construction (cold) joints in foundations and area flatwork should either be thickened butt-joints with dowels or a thickened keyed-joint designed to resist vertical deflection at the joint. All joints in flatwork should be sealed to prevent moisture, vermin, or foreign material intrusion. Precautions should be taken to prevent curling of slabs in this arid desert region (refer to ACI guidelines).

All independent concrete flatworks should be underlain by 12 inches of moisture conditioned and compacted soils. All flatwork should be jointed in square patterns and at irregularities in shape at a maximum spacing of 10 feet or the least width of the sidewalk.

4.4 Concrete Mixes and Corrosivity

Selected chemical analyses for corrosivity were conducted on bulk samples of the near surface soil from the project site (Plate C-4). The native soils tested were shown to have low levels of sulfate

and chloride ion concentrations. Resistivity determinations on the soil indicate severely potential for metal loss because of electrochemical corrosion processes.

A minimum of 2,500 psi concrete of Type II Portland Cement with a maximum water/cement ratio of 0.60 (by weight) should be used for concrete placed in contact with native soil on this project (sitework including streets, sidewalks, driveways, patios, and foundations).

A minimum concrete cover of three (3) inches is recommended around steel reinforcing or embedded components (anchor bolts, hold-downs, etc.) exposed to native soil or landscape water (to 18 inches above grade). The concrete should also be thoroughly vibrated during placement.

Landmark does not practice corrosion engineering. We recommend that a qualified corrosion engineer evaluate the corrosion potential on metal construction materials and concrete at the site.

4.5 Excavations

All trench excavations should conform to CalOSHA requirements for Type C soil. The contractor is solely responsible for the safety of workers entering trenches. Temporary excavations with depths of 4 feet or less may be cut nearly vertical for short duration. Temporary slopes should be no steeper than 1.5:1 (horizontal:vertical). Sandy soil slopes should be kept moist, but not saturated, to reduce the potential of raveling or sloughing.

Trench excavations deeper than 4 feet will require shoring or slope inclinations in conformance to CAL/OSHA regulations for Type C soil. Surcharge loads of stockpiled soil or construction materials should be set back from the top of the slope a minimum distance equal to the height of the slope. All permanent slopes should not be steeper than 3:1 to reduce wind and rain erosion. Protected slopes with ground cover may be as steep as 2:1. However, maintenance with motorized equipment may not be possible at this inclination.

4.6 Lateral Earth Pressures

Earth retaining structures, such as retaining walls, should be designed to resist the soil pressure imposed by the retained soil mass. Walls with granular drained backfill may be designed for an assumed static earth pressure equivalent to that exerted by a fluid weighing 38 pcf for unrestrained (active) conditions (able to rotate 0.1% of wall height), and 52 pcf for restrained (at-rest) conditions. These values should be verified at the actual wall locations during construction.

4.7 Seismic Design

This site is located in the seismically active southern California area and the site structures are subject to strong ground shaking due to potential fault movements along the San Andreas Fault. Engineered design and earthquake-resistant construction are the common solutions to increase safety and development of seismic areas. Designs should comply with the latest edition of the CBC for Site Class D using the seismic coefficients given in Section 3.6 of this report.

4.8 Pavements

Pavements should be designed according to CALTRANS or other acceptable methods. Traffic indices were not provided by the project engineer or owner; therefore, we have provided structural sections for several traffic indices for comparative evaluation. The public agency or design engineer should determine the appropriate traffic index for the site. Maintenance of proper drainage is necessary to prolong the service life of the pavements. Based on the current State of California CALTRANS method, an estimated R-value of 30 for the subgrade soil and assumed traffic indices, the following table provides structure thicknesses for asphaltic concrete (AC) pavement sections.

R-value of Subgrade	Soll - 30 (estimated)	Design Method - CALTRANS 200							
	Flexible	Pavements							
Traffic Index (assumed)	Asphaltic Concrete Thickness (in.)	Aggregate Base Thickness (in.)							
5.0	3.0	6.0							
6.0	3.5	8.5							
7.0	4.5	9.5							
8.0	5.0	11.5							

PAVEMENT STUCTURAL SECTIONS

R-Value of Subgrade Soil 30 (actimated)

D. I. M. dl CALED AND OCC

Notes:

- Asphaltic concrete shall be Caltrans, Type B, 3/4 inch maximum medium grading, (1/2 inch for 1) parking areas) compacted to a minimum of 95% of the 50-blow Marshall density (ASTM D1559).
- Aggregate base shall conform to Caltrans Class 2 (3/4 in. maximum), compacted to a 2) minimum of 95% of ASTM D1557 maximum dry density.
- Place pavements on 12 inches of moisture conditioned (at least 2% of over optimum) native 3) soil compacted to a minimum of 95% of the maximum dry density determined by ASTM D1557, or the governing agency requirements.

Final pavement sections may need to be determined by sampling and R-Value testing during grading operations when actual subgrade soils are exposed.

Section 5 LIMITATIONS AND ADDITIONAL SERVICES

5.1 Limitations

The findings and professional opinions within this report are based on current information regarding the proposed new church at St Frances of Rome, 21591 Lemon Street, Wildomar, California. The conclusions and professional opinions of this report are invalid if:

- < Proposed building(s) location and size are changed from those shown in this report
- < Structural loads change from those stated or the structures are relocated.
- < The Additional Services section of this report is not followed.
- < This report is used for adjacent or other property.
- < Changes of grade or groundwater occur between the issuance of this report and construction other than those anticipated in this report.
- < Any other change that materially alters the project from that proposed at the time this report was prepared.

Findings and professional opinions in this report are based on selected points of field exploration, geologic literature, laboratory testing, and our understanding of the proposed project. Our analysis of data and professional opinions presented herein are based on the assumption that soil conditions do not vary significantly from those found at specific exploratory locations. Variations in soil conditions can exist between and beyond the exploration points or groundwater elevations may change. If detected, these conditions may require additional studies, consultation, and possible design revisions.

This report contains information that may be useful in the preparation of contract specifications. However, the report is not worded is such a manner that we recommend its use as a construction specification document without proper modification. The use of information contained in this report for bidding purposes should be done at the contractor's option and risk.

This report was prepared according to the generally accepted *geotechnical engineering standards of practice* that existed in Riverside County at the time the report was prepared. No express or implied warranties are made in connection with our services. This report should be considered invalid for periods after two years from the report date without a review of the validity of the findings and

professional opinions by our firm, because of potential changes in the Geotechnical Engineering Standards of Practice.

The client has responsibility to see that all parties to the project including, designer, contractor, and subcontractor are made aware of this entire report. The use of information contained in this report for bidding purposes should be done at the contractor's option and risk.

5.2 Additional Services

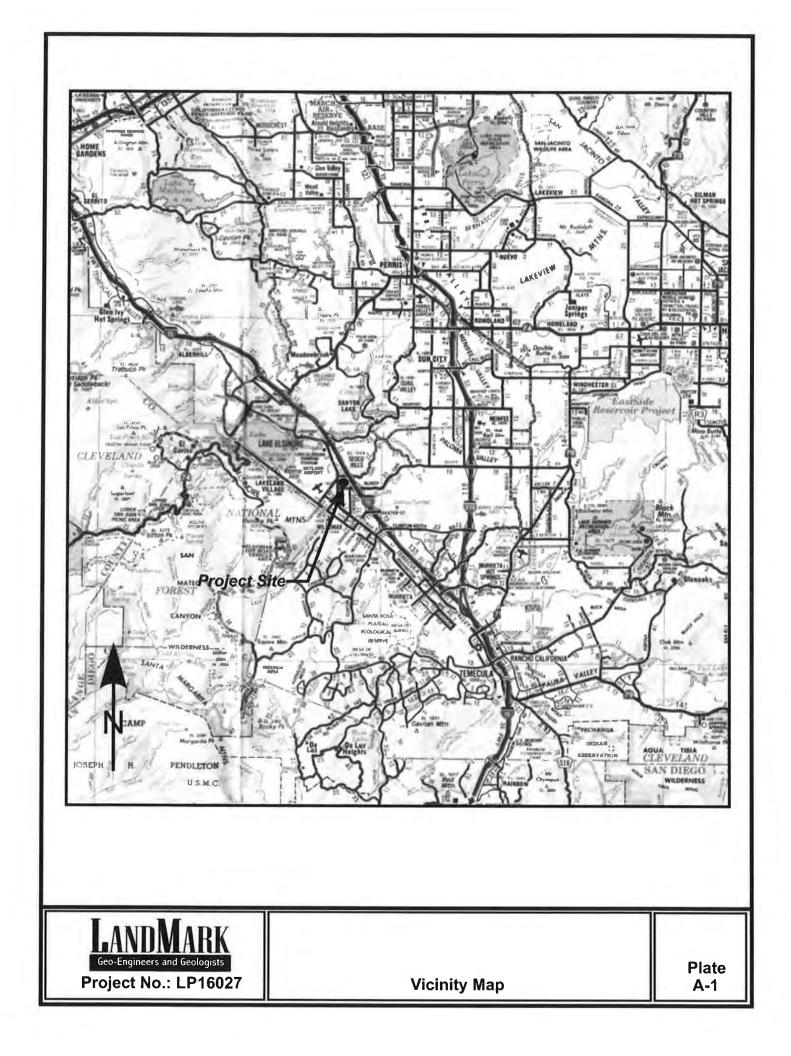
We recommend that a qualified geotechnical consultant be retained to provide the tests and observations services during construction. *The geotechnical engineering firm providing such tests and observations shall become the geotechnical engineer of record and assume responsibility for the project.*

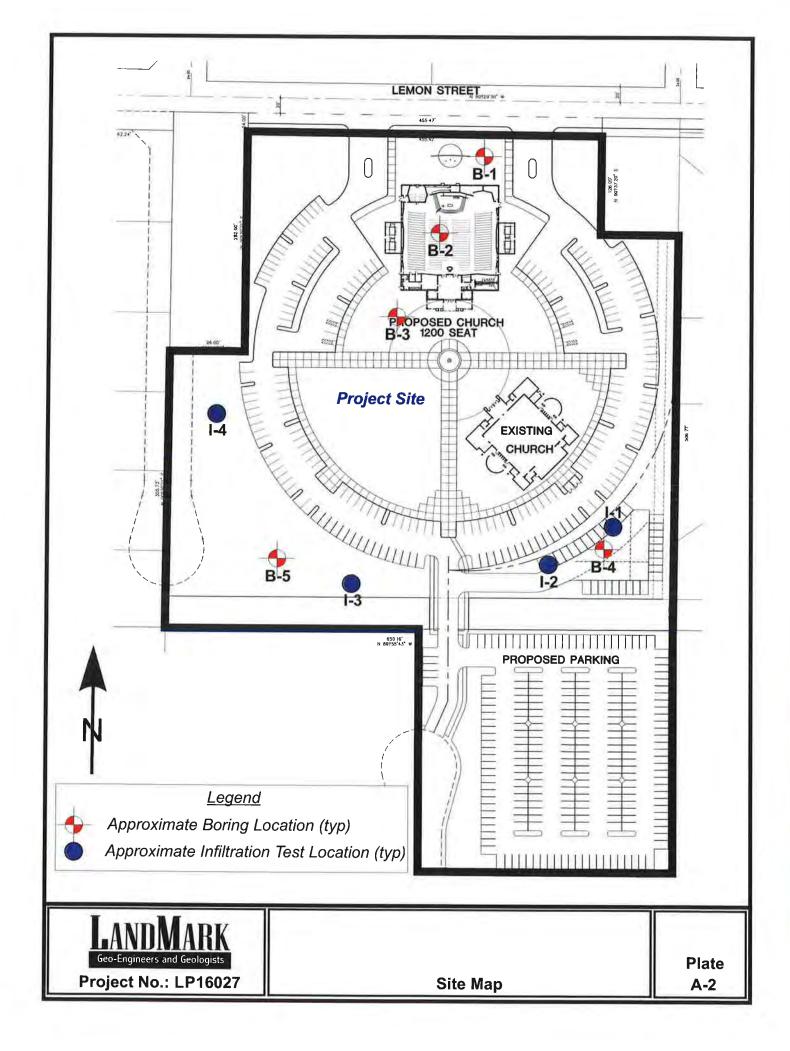
The professional opinions presented in this report are based on the assumption that:

- < Consultation during development of design and construction documents to check that the geotechnical professional opinions are appropriate for the proposed project and that the geotechnical professional opinions are properly interpreted and incorporated into the documents.
- < **LandMark Consultants, Inc.** will have the opportunity to review and comment on the plans and specifications for the project prior to the issuance of such for bidding.
- < Continuous observation, inspection, and testing by the geotechnical consultant of record during site clearing, grading, excavation, placement of fills, building pad and subgrade preparation, and backfilling of utility trenches.
- < Observation of foundation excavations and reinforcing steel before concrete placement.
- < Other consultation as necessary during design and construction.

We emphasize our review of the project plans and specifications to check for compatibility with our professional opinions and conclusions. Additional information concerning the scope and cost of these services can be obtained from our office.

APPENDIX A







Soil Map-Western Riverside Area, California

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AP LEGEND MAP INFORMATION	Spoil Area The soil surveys that comprise your AOI were mapped at 1:15,800.	OI) Stony Spot Varning: Soil Map may not be valid at this scale.	C Very Stony Spot	Wet Spot	∧ Other	3	Special Line Features Please rely on the har scale on each map sheet for map		Streams and Canals Source of Map: Natural Resources Conservation Service	Web Soil Survey L	Rails Coordinate System: Web Mercator (EPSG:3857)	Interstate Highways	US Routes Drojection, which preserves direction and shape but distorts	uistance and area. A projection mar preserves area, such as me Albers equal-area conic projection, should be used if more accurate		Background the USDA-NRCS certified data as of the use o		Soll Survey Area: Western Kiverside Area, California Survey Area Data: Version 8, Sep 22, 2015	Soil map units are lab	or larger.	Date(s) aerial images were photographed: Feb 24, 2015—Feb	6102,022	The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background		of map unit boundaries may be evident.		
MAPL	Area of Interest (AOI)	Area of Interest (AOI)	Soil Man Hait Dolyana	Soil Man Hait Lince		Soil Map Unit Points	Special Point Features	Blowout	Borrow Pit	Clav Spot		Closed Depression	Gravel Pit	Gravelly Spot	Landfill	Lava Flow	Marsh or swamp	Mine or Quarry	Miscellaneous Water	Perenniał Water	Rock Outcrop	Saline Spot	Sandy Spot	Severely Eroded Spot	Sinkhole	Slide or Slip	
	Area of Int		Soils		2		Special F	9)	×	0	*	**	0	×	4	¢	0	0	>	+	 	0	¢	A	. 8

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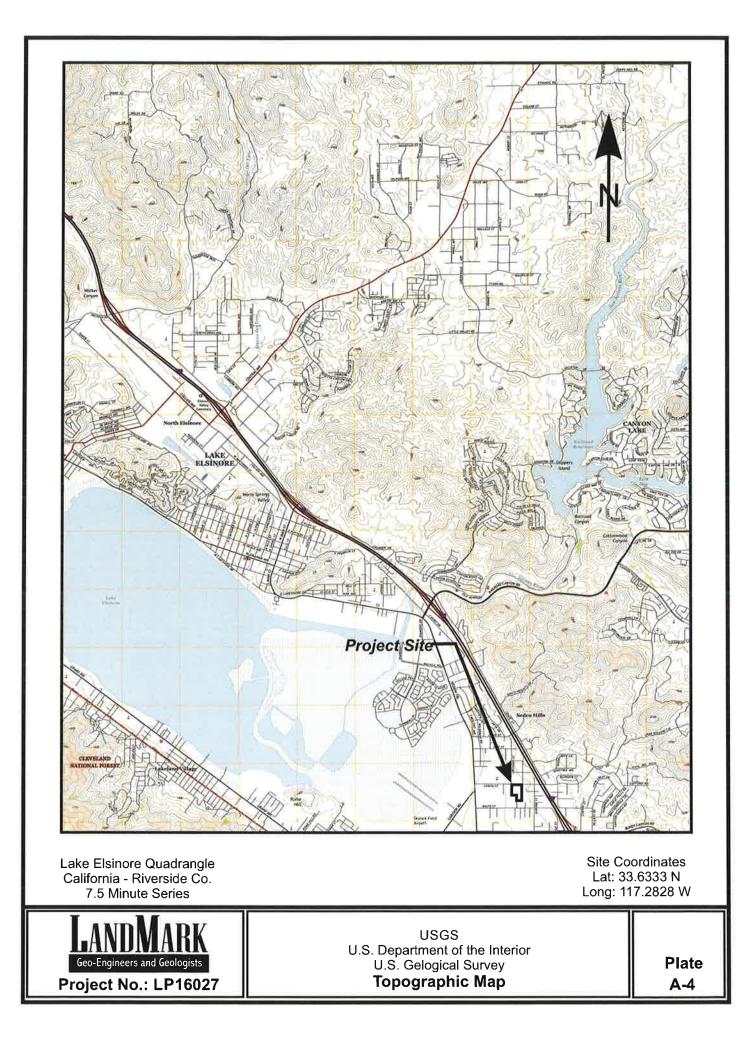
USDA Natural Resources Conservation Service

Web Soil Survey National Cooperative Soil Survey

Map Unit Legend

Western Riverside Area, California (CA679)											
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI								
GyC2	Greenfield sandy loam, 2 to 8 percent slopes, eroded	77.5	79.8%								
GyD2	Greenfield sandy loam, 8 to 15 percent slopes, eroded	0.3	0.3%								
HcC	Hanford coarse sandy loam, 2 to 8 percent slopes	16.8	17.2%								
ReC2	Ramona very fine sandy loam, 0 to 8 percent slopes, ero ded	0.9	1.0%								
TeG	Terrace escarpments	1.7	1.7%								
Totals for Area of Interest		97.2	100.0%								







Flood Insurance Rate Map (FIRM)

Project No.: LP16027

Plate A-5

LEGEND



SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

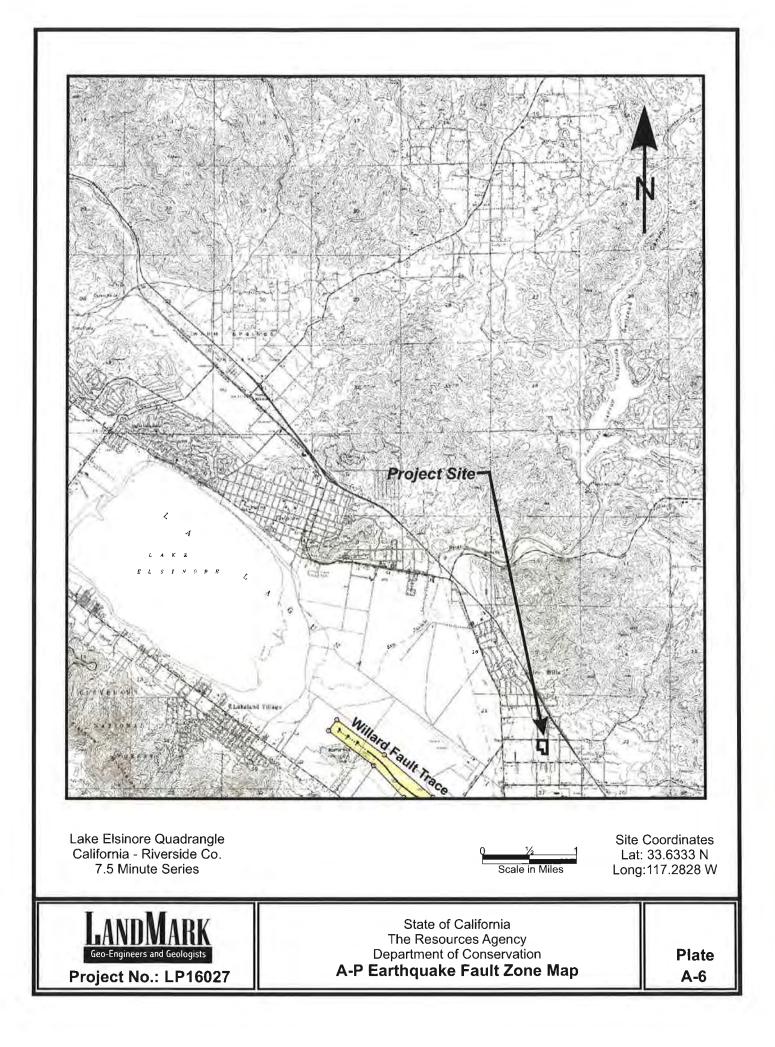
The 1% annual flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Bevation is the water-surface elevation of the 1% annual chance flood.

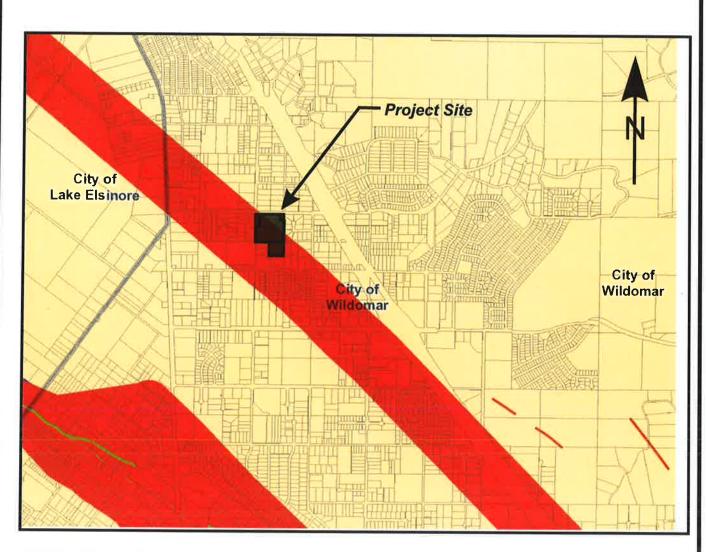
ZONE A	No Base Flood Elevations determined.
ZONE AE	Base Flood Elevations determined.
ZONE AH	Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
ZONE AO	Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
ZONE AR	Special Flood Hazard Area formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
ZONE A99	Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
ZONE V	Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VE	Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

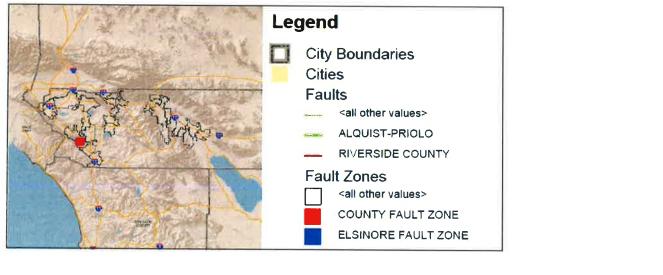
The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

FLOODWAY AREAS IN ZONE AE

OTHER FLOOD AREAS ZONE X Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood. OTHER AREAS ZONE X Areas determined to be outside the 0.2% annual chance floodplain. ZONE D Areas in which flood hazards are undetermined, but possible, COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS OTHERWISE PROTECTED AREAS (OPAs) CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas. 1% annual chance floodplain boundary 0.2% annual chance floodplain boundary Floodway boundary Zone D boundary CBRS and OPA boundary Boundary dividing Special Flood Hazard Area Zones and boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities. Base Flood Elevation line and value; elevation in feet* ~ 513 ~~~~ Base Flood Elevation value where uniform within zone; elevation (EL 987) in feet* * Referenced to the North American Vertical Datum of 1988 Cross section line (A) $\langle A \rangle$ 23-----23 Transect line 87°07'45", 32°22'30" Geographic coordinates referenced to the North American Datum of 1983 (NAD 83), Western Hemisphere 2476000mN 1000-meter Universal Transverse Mercator grid values, zone 11N 600000 FT 5000-foot grid ticks: California State Plane coordinate system, zone VI (FIPSZONE 0406), Lambert Conformal Conic projection Bench mark (see explanation in Notes to Users section of this DX5510 x FIRM panel) River Mile •M1.5







Geo-Engineers and Geologists

Project No.: LP16027

Riverside County Information Technology (RCIT) Geographic Information Services

Fault Map

APPENDIX B

CL	IENT:	Dios	sis o	f San Bernardino	METHOD OF DF	RILLIN	IG: C	ME 7	5 w/au	toham	mer
			_	ces of Rome Catholic Church	DATE OBS		_				_
LOCA		_	91 L	emon Street, Wildomar, CA	LOGO	GEDE	BY: G.	_		_	_
-	FIE	LD	1 2	LOG OF BORING: B-	1		LA		RATOR	RY	-
DEPTH (FT) CLASSIFICATION	SAMPLE TYPE	BLOW COUNT	POCKET PEN (TSF) PI			UHE	DRY UNIT WT. (PCF)	UNCONFINED		PLASTICITY INDEX	PASSING #200
DEPTI	SAMP	BLOW	POCK	DESCRIPTION OF MATERIAL		MOIS TUHE CONTENT	DRY (PCF)	UNCO	non	PLAS.	PASS
5	•	23		SILTYSAND (SM): Brown, with traces of gravel moist and dense with depth		3.5	118.9				40
10		55				8.5	131.3				
15		47		SILTY SAND (SM): Dark brown. moist and medium dense		10.3	134.5				40
20		24				9.2					
25		20		SILTY SAND (SM): Brown. moist and medium dense		11.0					34
30		22				8.1					24
35 40											
SUF	RFACE E	LEVA	TION:	1334 ftTOTAL DEPTH:31.5 ft	DEPTH TO W	ATER:	<u> </u>	I/A	Ľ.	0.0	
	JECT P1602		:	LANDMAI Geo-Engineers and Geolo	RK austs				I	PLATE B-1	

				cis of Rome Catholic Church emon Street, Wildomar, CA	DATE OB	SERVI GED I	_				-
	_	ELD					_	ABOR	_		-
			SF) Pr	LOG OF BORING: B-2		1.000		1	1	1	
ICATION	TYPE	OUNT	PEN (TS	PAGE 1 OF 2		ш.	TW	NED SSION (T	MIT	TY INDEX	#200
CLASSIFICATION	SAMPLE TYPE	BLOW COUNT	POCKET PEN (TSF) PI	DESCRIPTION OF MATERIAL		MOISTURE	DRY UNIT WT (PCF)	UNCONFINED COMPRESSION (TSF)	τιαυίρ μιμιτ	PLASTICITY INDEX	DASSING #200
		15 21 29		SILTY SAND (SM): Brown, with traces of gravel.		7.3 4.9 3.0	123.5 113.0 110.9				3
		21		SILTY SAND (SM): Brown.		9.3					24
111111		25		SILTY SAND (SM): Dark brown		10.2					3.
		25				8.1					2'
		22									41
		34									27
SUR	FACE EL	EVATI	ON:	1331 ft TOTAL DEPTH: 51.5 ft	DEPTH TO W	ATER:	N	/A			-
	JECT 21602			I.ANDMARK						LATE B-2	

	ECT:	St. F	rand	San Bernardino ces of Rome Catholic Church emon Street, Wildomar, CA	METHOD OF DF DATE OBS LOG	ERV	ED: 3	/15	/201	6	_	_
	FIE	_							_	ATOF	RY	-
			Id (J:	LOG OF BORING: B-	2			T				
CATION	TYPE	DUNT	PEN. (TS	PAGE 2 OF 2		w _	- MT	NED	L) NOISS	MIT	TY INDE	#200
CLASSIFICATION	SAMPLE TYPE	BLOW COUNT	POCKET PEN. (TSF) PI	DESCRIPTION OF MATERIAL		MOISTURE	DRY UNIT WT.	UNCONFINED	COMPRESSION (TSF)	LIQUID LIMIT	PLASTICITY INDEX	PASSING #200
11111				SILTY SAND (SM): Brown.								
5		47		dense with depth								15
		41										13
SUR	ACE E	LEVAT	ION:	<u>1331 ft</u> TOTAL DEPTH: <u>51.5 ft</u>	DEPTH TO W	ATER		N/A	T			
PRO. LF	ECT 1602			LANDMAN Geo-Engineers and Geolo	RK aists					P	B-3	

CLI	ENT:	Dios	sis o	f San Bernardino	METHOD OF D	RILLI	NG: C	ME 75	w/aut	toham	mer
				ces of Rome Catholic Church	DATE OB						
LOCAT	_		91 L	emon Street, Wildomar, CA	LOG	GED					-
-	FIE	LD	1.5	LOG OF BORING: B-	2		/	ABOR/	TOR	Y	_
DEPTH (FT) CLASSIFICATION	SAMPLE TYPE	BLOW COUNT	POCKET PEN. (TSF) PI		5	MOISTURE	DRY UNIT WT. (PCF)	UNCONFINED COMPRESSION (TSF)	רוסחום רושוב	PLASTICITY INDEX	PASSING #200
DEPT	SAMP	BLOW	POCK	DESCRIPTION OF MATERIAL		MOISTURE	DRY L	UNCC	רומחו	PLAS	PASS
mm				SILTY SAND (SM): Brown, with traces of gravel.							
5		23		moist and medium dense		5.5	123,0				3
10		15				5.2	119.5				
15		49		SILTY SAND (SM): Brown. moist and dense		7.0	135 5				2
20		23		SILTY SAND (SM): Dark brown. moist and medium dense		11.0					
25		22				9,0					2
30		21				11.1		_			
30											
	ACE E	LEVAT	TION:	<u>1307 ft</u> TOTAL DEPTH: <u>31.5 ft</u>	DEPTH TO V	WATER:	N	I/A	-		
PROJ LP	ECT 1602			LANDNAI Geo-Engineers and Geole	RK gisis				P	LATE B-4	

PRO					OF DRILLIN OBSERVI	ED: 3/	15/201	6		_
LOCA	TION:	2159	91 Le	emon Street, Wildomar, CA	LOGGED	BY: G	. Chan	dra		
	FIE	LD				L	BORA	TOR	Y	
DEPTH (FT) CLASSIFICATION	SAMPLE TYPE	COUNT	POCKET PEN. (TSF) PI	LOG OF BORING: B-4	JRE NT	UT WT	UNCONFINED COMPRESSION (TSF)	LIMIT	PLASTICITY INDEX	
DEPTH (FT)	SAMPL	BLOW COUNT	POCKE	DESCRIPTION OF MATERIAL	MOISTURE	DRY UNIT WT. (PCF)	UNCONFINED	LIQUID LIMIT	PLASTI	ļ
				SILTY SAND (SM): Brown, with traces of gravel.						
5		6		moist and loose	10_1					
10		10			10.1					
25 30 35										
	RFACE E	LEVAT	ION:	<u>1329 ft</u> TOTAL DEPTH: <u>11.5 ft</u> DEPTH	H TO WATER:		J/A	-	LATE	

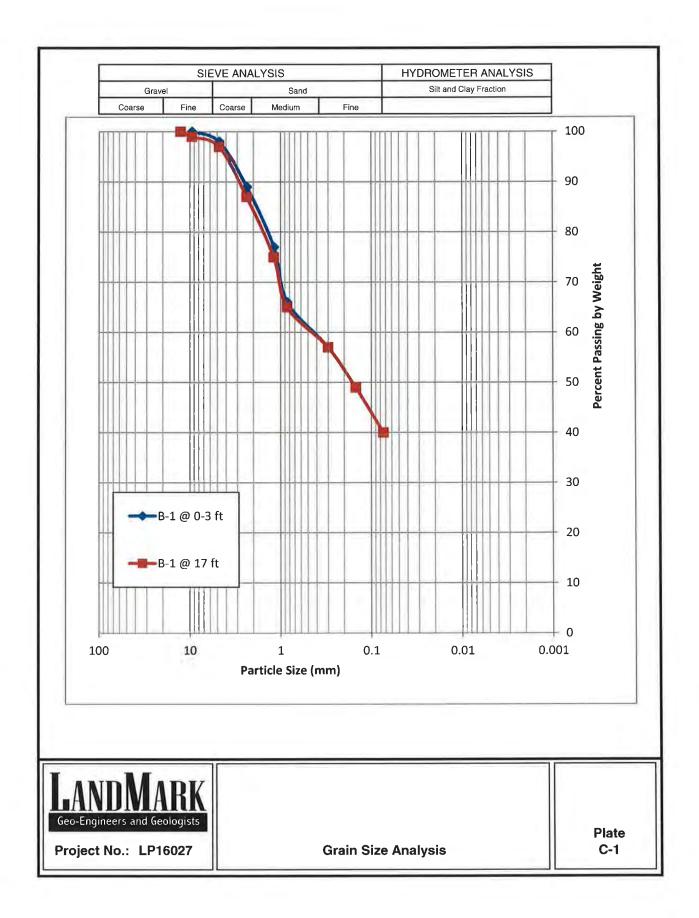
ROJE	CT:	St. I	Fran	ces of Rome Catholic Church	METHOD OF DRILLING: CME 75 w/autohammer DATE OBSERVED: 3/15/2016 LOGGED BY: G. Chandra								
CAT	ON:	215	91 L	emon Street, Wildomar, CA	LOG	GE) B	Y : G	i. Ch	an	dra	_	
	FIE	LD	1.5				-	L	ABC	RA	TOR	Y	
CLASSIFICATION	SAMPLE TYPE	BLOW COUNT	POCKET PEN. (TSF) PI	LOG OF BORING: B-5		TURE	ENT	DRY UNIT WT.	UNCONFINED	COMPRESSION (TSF)	רוסחום רואוב	PLASTICITY INDEX	PASSING #200
CLAS	SAMP	BLOW	POCK	DESCRIPTION OF MATERIAL		MOISTURE	CONTENT	(PCF)	UNCO	COMF	rioni	PLAS	PASS
				SILTY SAND (SM): Brown, with traces of gravel,									
		14		moist and medium dense		9.8	3						í.
		15				11.4	4						32
	ACE E		-	<u>1328 ft</u> TOTAL DEPTH: <u>11.5 ft</u>	DEPTH TO V	VATE	R: _		N/A		_		-
	ECT 1602			Geo-Engineers and Geologi	K						Р	LATE B-6	

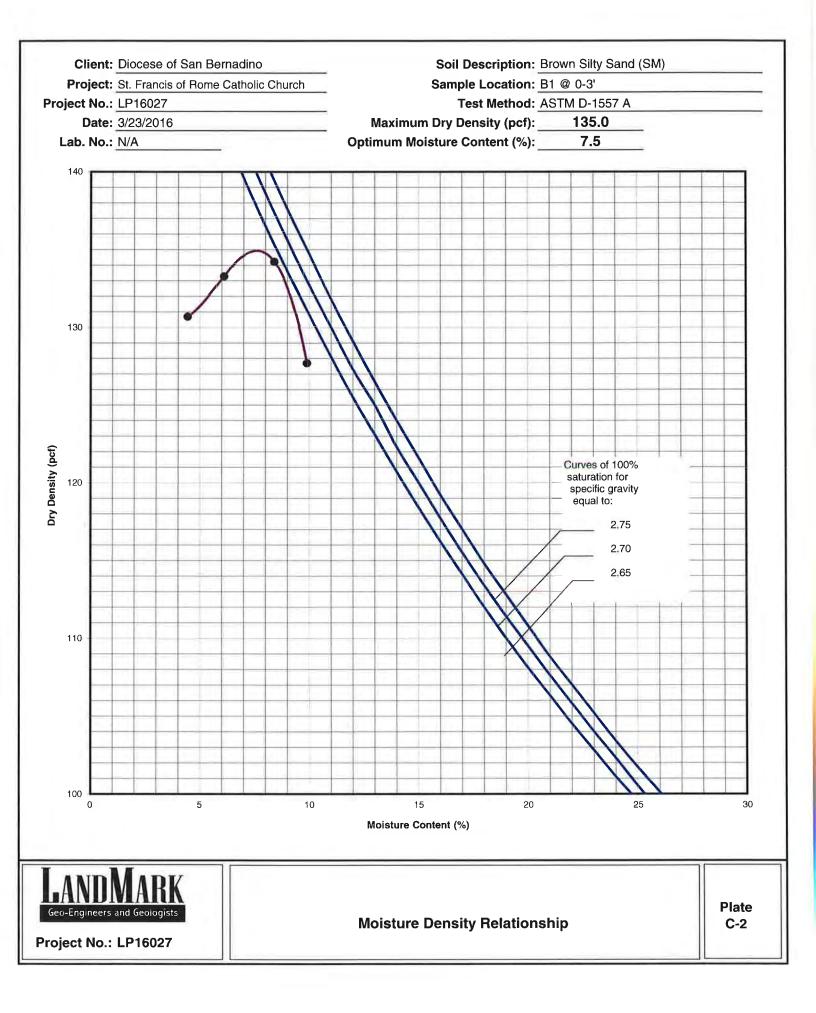
Coarse grained soils More than half of material is larger that No, 200 sieve	ARY DIVISIONS Gravels More than half of coarse fraction is larger than No. 4 sieve Sands	Clean gravels (less than 5% fines) Gravel with fines	SYMB	GW GP	Well graded gravels, gravel- Poorly graded gravels, or gra	SECONDARY I			
than half of material is	coarse fraction is larger than No_4 sieve	than 5% fines) Gravel with fines	A . M . M		Poorly graded gravels, or gra				
than half of material is	coarse fraction is larger than No_4 sieve					vel-sand mixtures, I	ittle or no fines		
than half of material is	sieve		H 141 H	Gravel with fines					
than half of material is	Sands		644	GC	Clayey gravels, gravel-sand-				
	Sanus		13393	-					
		Clean sands (less than 5% fines)		SW	Well graded sands, gravelly				
	More than half of coarse fraction is		100 A	SP	Poorly graded sands or grave		olines		
	smaller than No. 4	Sands with fines		SM	Silty sands, sand-silt mixture	s, non-plastic fines			
· .	516110		14	SC	Clayey sands, sand-clay mix	tures, plastic fines			
	Silts an	d clays		ML	Inorganic silts, clayey silts w	th slight plasticity			
	Liquid limit is l		CL	Inorganic clays of low to mee	lium plasticily, grave	ly, sandy, or lean clay	/S		
Fine grained soils More than half of material is			OL	Organic silts and organic cla	ys of low plasticity				
maller than No. 200 sieve	Silts an	d clays		мн	Inorganic silts, micaceous or	diatomaceous silty :	soils, elastic silts		
			111	СН	Inorganic clays of high plasti	city, fat clays			
	Liquid limit is n	nore Ihan 50%	<u>66</u>	он	Organic clays of medium to h	igh plasticily, organ	ic silts		
Highly organic soils			1222	РТ	Peat and other highly organi	c soils			
		-							
		San		GRAI	N SIZES Gravel			1	
Silts and Cl	ays	Fine Mediur		arse		Coarse	Cobbles	Boulders	
	20	0 40	10	4	3/4*	3*	12"		
		US Standard Seri	ies Sieve			Clear Square (Openings		
				lī	Clays & Plastic Silts	Strength **	Blows/ft. *	1	
Sands, Gravels, etc.	Blows/ft.*			l	Very Soft	0-0.25	0-2		
Very Loose	0-4				Soft	0 25-0 5	2-4		
Loose	4-10				Firm	0 5-1 0	4-8		
Medium Dense	10-30				Stiff	1.0-2.0	8-16		
Dense	30-50				Very Stiff	2.0-4.0	16-32		
Very Dense	Over 50			Į	Hard	Over 4.0	Over 32		
Number of blows of 140	lb. hammer falling	30 inches to drive	a 2 inch	O.D. (1 3/8 in LD) split spoon (/	ASTM D1586).			
					sting or approximated by the	ne Standard			
Penetration Test (ASTM	ID1586), Pocket P	enetrometer, Torv	ane, or v	isual o	bservation.				
ne of Samples:	1920	ple 🛛 Stai	ndard Pe	netratio	on Test Shelby	Tube 💿 E	Bulk (Bag) Sample		
pe of Samples:	Ring Sam								
	N Ring Sam								
illing Notes:		ow Counts							
illing Notes:	. Sampling and Bl		umberof	blowe	per foot of a 140 lb. hamn	her falling 30 inch	es.		
illing Notes:	. Sampling and Bl	Ring Sampler - N			per foot of a 140 lb, hamn ber of blows per foot.	ner falling 30 inch	es.		
illing Notes:	. Sampling and Bl	Ring Sampler - N Standard Penetra	tion Test	- Num	per foot of a 140 lb, hamn ber of blows per foot. hinal diameter tube hydrau	-	es.		
illing Notes: 1	. Sampling and Bl	Ring Sampler - N Standard Penetra Shelby Tube - Th	ition Test ree (3) ind	- Num	ber of blows per foot.	-	es.		
illing Notes: 1 2 3	. Sampling and Bl 2. P. P. = Pocket P 3. NR = No recove	Ring Sampler - N Standard Penetra Shelby Tube - Th enetrometer (tons ry.	ition Test ree (3) ind s/s.f.).	- Num ch nom	ber of blows per foot. hinal diameter tube hydrau	-	85.		
illing Notes: 1 2 3	. Sampling and Bl	Ring Sampler - N Standard Penetra Shelby Tube - Th enetrometer (tons ry.	ition Test ree (3) ind s/s.f.).	- Num ch nom	ber of blows per foot. hinal diameter tube hydrau	-	əs.		
illing Notes: 1 2 3	. Sampling and Bl 2. P. P. = Pocket P 3. NR = No recove	Ring Sampler - N Standard Penetra Shelby Tube - Th enetrometer (tons ry.	ition Test ree (3) ind s/s.f.).	- Num ch nom	ber of blows per foot. hinal diameter tube hydrau	-	es.	_	
illing Notes: 1 2 3	. Sampling and Bl 2. P. P. = Pocket P 3. NR = No recove	Ring Sampler - N Standard Penetra Shelby Tube - Th enetrometer (tons ry.	ition Test ree (3) ind s/s.f.).	- Num ch nom	ber of blows per foot. hinal diameter tube hydrau	-	85.		
illing Notes: 1 2 3	. Sampling and Bl P. P. = Pocket P NR = No recove GWT ¥ = Gr ARK	Ring Sampler - N Standard Penetra Shelby Tube - Th enetrometer (tons ry.	ition Test ree (3) ind s/s.f.).	- Num ch nom	ber of blows per foot. hinal diarneter tube hydrau	-	es.	Plat	

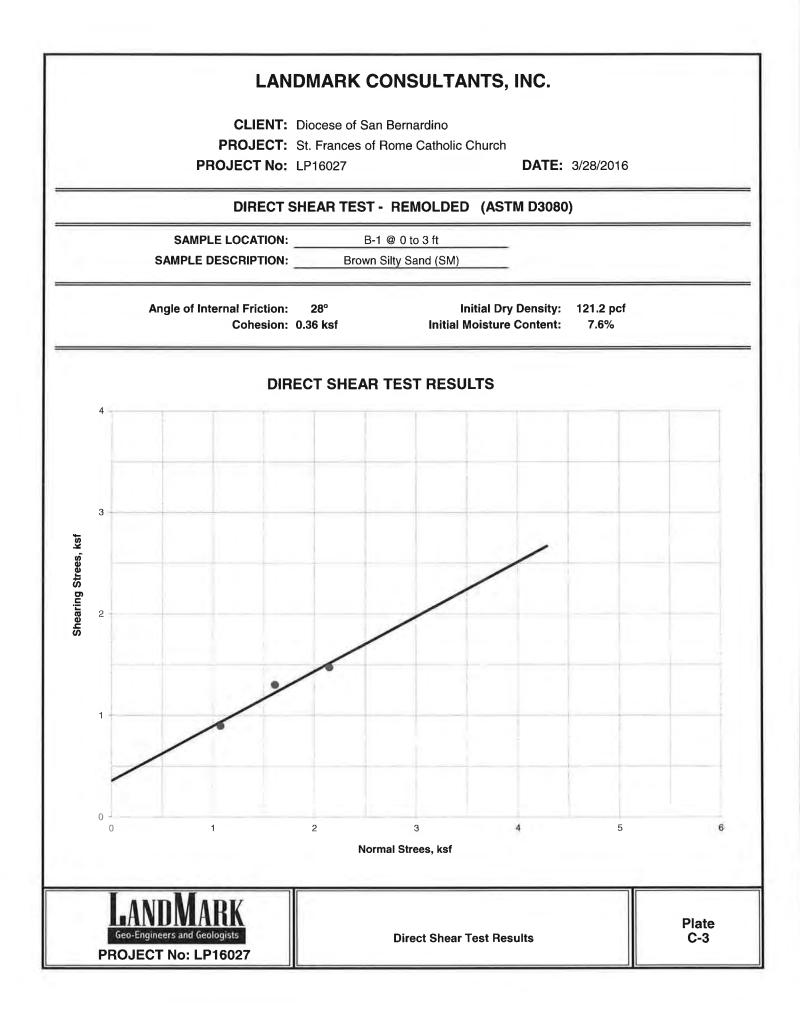
Project No.: LP16027

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APPENDIX C







LANDMARK CONSULTANTS, INC.

CLIENT: Diocese Of San Bernardino PROJECT: St. Frances of Rome Catholic Church JOB No.: LP16027 DATE: 04/16/16

P٢

CHEMICAL ANALYSIS	**********
B-1	Caltrans
0-3	Method
7.25	643
	424
1500	643
130	422
126	417
	B-1 0-3 7.25 1500 130

Material Affected	Chemical	Amount in	Degree of Corrosivity	
Concrete	Agent Soluble Sulfates	Soil (ppm) 0 - 1,000 1,000 - 2,000 2,000 - 20,000 > 20,000	Low Moderate Severe Very Severe	
Normal Grade Steel	Soluble Chlorides	0 - 200 200 - 700 700 - 1,500 > 1,500	Low Moderate Severe Very Severe	
Normal Grade Steel	Resistivity	1 - 1,000 1,000 - 2,000 2,000 - 10,000 > 10,000	Very Severe Severe Moderate Low	
NDNAR igineers and Geologi No.: LP16027			cted Chemical est Results	Plate C-4

APPENDIX D

Seismic Settlement Calculation

Project Name: St Frances of Rome Project No.: LP16027 Location: B-1

	Design Ground Motion 0.94 g Total Unit Weight, 120 pcf Water Unit Weight, 62.4 pcf Depth to Groundwater 60 ft Hammer Effenciency 90 Rod Length 3	Maximum Credible Earthquake	6.8
		Water Unit Weight, Depth to Groundwater	62.4 pcf 60 ft
ter	tod Length 3	ammer Effenciency	06
ter .y		od Length	3

170 /001	62.4 pcf	60 ft	06	m

	-	-	-	-	-	-	-	-	-	-	<u> </u>	۰
	Enc			4.97E-04	7.86E-04	1.41E-03	I.62E-03				Ĭ	
	E15			6.15E-04	9.73E-04	1.75E-03	2.01E-03					
Shear Strain Gam-	eff			2.07E-03	2.69E-03	3.69E-03	3.93E-03					
	Gmax	675	1155	1320	1443	1498	1606	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	
	d	0.201	0.402	0.603	0.804	1.005	1.206	0.000	0.000	0.000	0.000	
	N10601CS	38	68	55	47	37	35	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	
	Fine Content	40	40	40	40	34	24					
	N1(60)	27.7	52.3	41.6	34.7	27.3	27.8	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	
	O-PRESS	0.30	0.60	0.00	1.20	1.50	1.80	0.00	0.00	0.00	0.00	
	Susceptible O-PRESS	0	0	1		1	1					
THICKNESS	(ft.)	5	5	5	5	5	5					
DEPTH	(ft.)	5	10	15	20	25	30					
	SPT				24	20	22					Î
	Mod. Cal	23	55	47								

REFERENCES

Tokimatsu and Seed, 1984. Simplified Procedures for the Evaluation of Settlements in Clean Sands.
 Seed and Idriss, 1982. Ground Motion and Soil Liquefaction During Earthquakes, EERI Monograph.
 Youd, Leslie, 1997. Proceeding of the NCEER Workshop on Evaluation of Liquefaction Resistance of Soils
 Pradel, Daniel, 1998. JGEE, Vol. 124, No. 4, ASCE
 Seed, et.al., 2003, Recent Advances in Soil Liquefaction Engineering: A Unified and Consistent Framework. University of California, Earthquake Engineering Research Center Report 2003-06, 71 p.

Nc 9.3

TOTAL (in.)

Settlement (in.)

0.06 0.09 0.17 0.19 0.52

Seismic Settlement Calculation

Project Name: St Frances of Rome Project No.: LP16027 Location: B-2

Maximum Credible Earthquake	6.8
Design Ground Motion	0.94 g
Total Unit Weight,	120 pcf
Water Unit Weight,	62 4 pcf
Depth to Groundwater	60 ft
Hammer Effenciency	06
Rod Length	3

Nc 9.3

TOTAL (in.)											1.01
Settlement (in.)			0.20	0.16	0.11	0.16	0.17	0.08	0.05	0.07	
Enc			1.71E-03	1.35E-03	9.07E-04	1.30E-03	1.42E-03	6.87E-04	4.50E-04	6.11E-04	
E15			2.11E-03	1.67E-03	1.12E-03	1.61E-03	1.75E-03	8.50E-04	5.57E-04	7.56E-04	
Shear Strain Gam- eff			4.04E-03	3.60E-03	2.92E-03	3.50E-03	3.54E-03	2.34E-03	1.81E-03	1.92E-03	
Gmax	590	858	1128	1345	1587	1652	1749	2039	2264	2230	
đ	0.201	0.402	0.603 -	0.804	1.005	1.206	1.407	1.608	1.809	2.010	
N _{1(60)CS}	26	28	34	38	44	38	36	47	53	44	
Fine Content	30	30	30	24	31	21	41	27	15	13	
N1(60)	18.1	20.0	25.7	30.4	34.1	31.6	25.8	37.2	48.5	40.2	
O-PRESS	0.30	0.60	0.90	1.20	1.50	1.80	2.10	2.40	2.70	3.00	
Susceptible	0	0	1	1		1		1	1	1	
THICKNESS (ft.)	5	5	5	5	5	5	5	5	5	5	
DEPTH (ft.)	5	10	15	20	25	30	35	40	45	50	
SPT				21	25	25	22	34	47	41	1
Mod. Cal	15	21	29								

REFERENCES

Torimment and Seed, 1984. Simplified Procedures for the Evaluation of Settlements in Clean Sands.
 Tokimatu and Seed, 1984. Simplified Procedures for the Evaluation of Settlements in Clean Sands.
 Seed and Idriss, 1982. Ground Motion and Soil Liquefaction During Earthquakes, EERI Monograph.
 Youd, Leslie, 1997. Proceeding of the NCEER Workshop on Evaluation of Liquefaction Resistance of Soils
 Pradel, Daniel, 1998..JGEE, Vol. 124, No. 4, ASCE
 Seed, et.al., 2003, Recent Advances in Soil Liquefaction Engineering: A Unified and Consistent Framework. University of California, Earthquake Engineering Research Center Report 2003-06, 71 p.

Seismic Settlement Calculation

Project Name: St Frances of Rome Project No.: LP16027 Location: B-3

6.8	0.94 g	120 pcf	62.4 pcf	60 ft	06	3	
Maximum Credible Earthquake	Design Ground Motion	Total Unit Weight,	Water Unit Weight,	Depth to Groundwater	Hammer Effenciency	Rod Length	

9.3 9.3

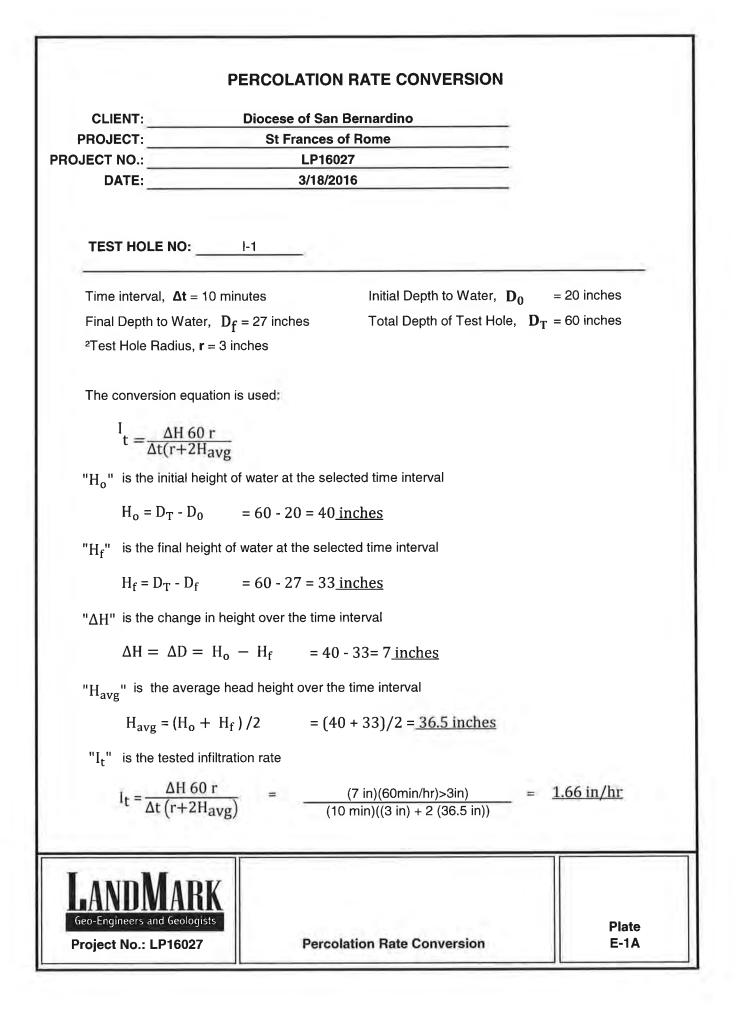
TOTAL	(in.)											0.60
Settlement	(in.)			0.07	0.13	0.17	0.22					
	Enc			6.16E-04	1.12E-03	1.44E-03	1.85E-03					
	E15			7.63E-04	1.38E-03	1.78E-03	2,28E-03					
Shear Strain Gam-	eff			2.32E-03	3.25E-03	3.73E-03	4.21E-03					
	Gmax	699	788	1283	1378	1494	1579	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	
	d	0.201	0.402	0.603	0.804	1.005	1.206	0.000	0.000	0.000	0.000	
	N _{1(60)CS}	37	22	50	41	37	33	#DIV/0!	#DIV/0	#DIV/0!	#DIV/0;	
	Fine Content	32	32	20	23	23	23					
	N1(60)	27.7	14.3	43.4	33.3	30.0	26.6	#DIV/0!	#DIV/0!	#DIV/0!	#DIV/0!	
	O-PRESS	0.30	0.60	0.90	1.20	1.50	1.80	0.00	0.00	0.00	0.00	
	Susceptible O-PRESS	0	0	1	1	1	I					
THICKNESS	(ft.)	5	5	5	5	5	5					
DEPTH	(ft.)	5	10	15	20	25	30					
	SPT				23	22	21					
	Mod. Cal	23	15	49								

REFERENCES

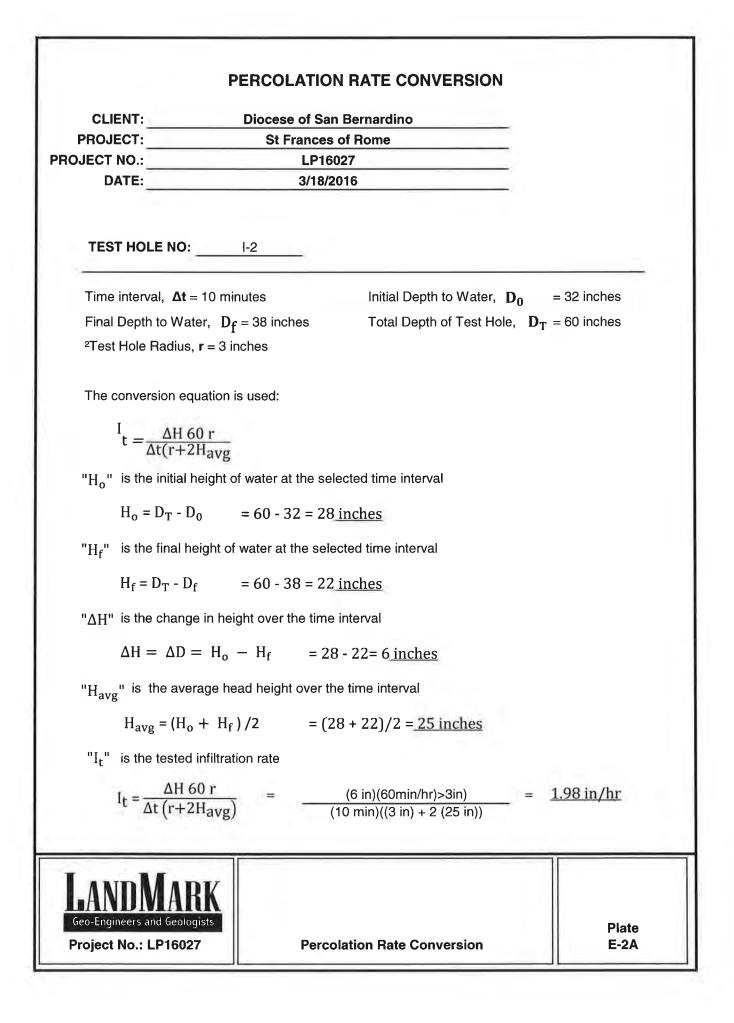
Tokimatsu and Seed. 1984. Simplified Procedures for the Evaluation of Settlements in Clean Sands.
 Seed and Idriss, 1982. Ground Motion and Soil Liquefaction During Earthquakes, EERI Monograph.
 Youd, Leslie, 1997. Proceeding of the NCEER Workshop on Evaluation of Liquefaction Resistance of Soils
 Pradel, Daniel, 1998. JGEE, Vol. 124, No. 4, ASCE
 Seed, et.al., 2003, Recent Advances in Soil Liquefaction Engineering: A Unified and Consistent Framework. University of California, Earthquake Engineering Research Center Report 2003-06, 71 p.

APPENDIX E

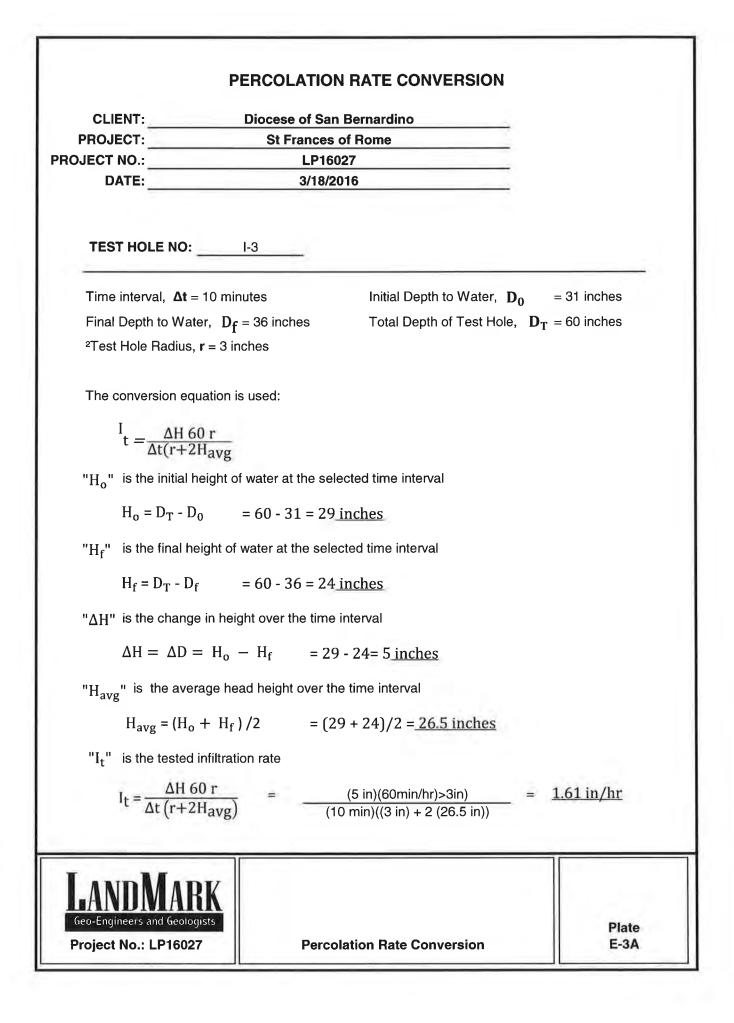
Project:	St Francis o	of Rome	Project No:	LP16	6027	Date:	3/18/16
Test Hole N	0:	I-1	Tested By:		Ale	хА	
Depth of Te	est Hole, D _T :	5'	USCS Soil Cl	assification:			
	Test Hole	Dimension	s (inches)		Length	Width	
Diamete	r (if round)=	1		ctangular)=			
Sandy Soil (Criteria Test*	8					
							Greater
			Time	Initial	Final	Change in	than or
			interval,	Depth to	Depth to	Water	Equal to 6"?
Trial No.	Start Time	Stop Time	(min.)	Water (in.)	Water (in.)	Level (in.)	(y/n)
1	8:50	9:15	25.00	29.00	55.00	26.00	у
2	9:15	9:40	25.00	30.00	50.00	20.00	у
six hours (a	pproximatel	y 30 minute	intervals) wi ∆t	ith a precisio D _o	n of at least D _f	ΔD	
			Time	Initial	Final	Change in	Percolation
			Interval	Depth to	Depth to	Water	Rate
Trial No.	Start Time	Stop Time	(min.)		Water (in.)		
1	0.10	9:52	10.00	18.00	27.00	9.00	1.11
2	9:52	10:02	10.00	27.00	35.00	8.00	1.25
3		10:12	10.00	35.00	43.00	8.00	1.25
4		10:22	10.00	19.00	27.00	8.00	1.25
5	10:22	10:32	10.00	27.00	35.00	8.00	1.25
6	10:32	10:42	10.00	20.00	27.00	7.00	1.43
7							
8							
9			-				
10							
11							
12							
10 11							



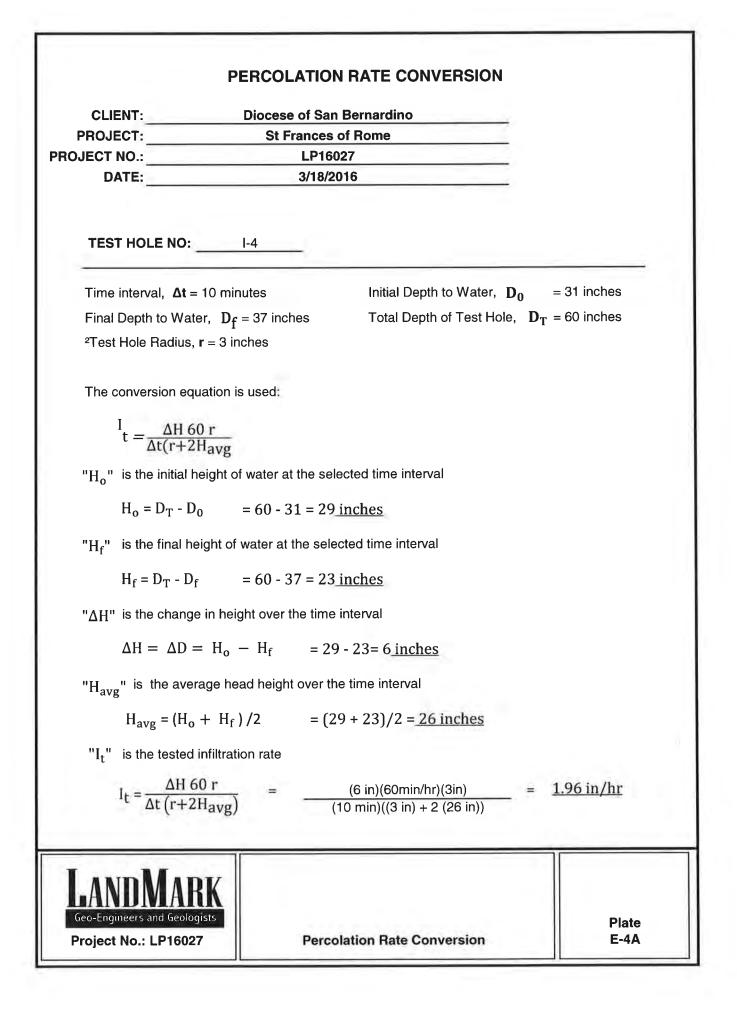
Test Hole No: Depth of Test I Diameter (if Sandy Soil Crit Trial No. Si 1 2	Test Hole f round)=	I-2 5' Dimension 6"	s (inches)	assification: ctangular)=	Ale Length	x A Width	
Diameter (if Sandy Soil Crit Trial No. Si 1 2	Test Hole f round)=	Dimension: 6"	s (inches)		Length	Width	
Trial No. 51 2	f round)=	6"	-	ctangular)=	Length	Width	
Trial No. S1			Sides (if re	ctangular)=			
Trial No. S1 1 2	eria Test*						
1 2							
1							Greater
1			Time	Initial	Final	Change in	than or
1			Interval,	Depth to	Depth to	Water	Equal to 6"?
	tart Time	Stop Time	(min.)	Water (in.)	Water (in.)	Level (in.)	(y/n)
	8:51	9:16	25.00	20.00	45.00	25.00	у
	9:16	9:41	25.00	20.00	44.00	24.00	у
six hours (appl	roximately	y 30 minute	Δt	Do	D _f	۵D	
			Time	Initial	Final	Change in	Percolation
			Interval	Depth to	Depth to	Water	Rate
Trial No. St	tart Time	Stop Time	(min.)		Water (in.)		(min./in.)
1	9:43	9:53	10.00	18.00	25.00	7.00	1.43
2	9:53	10:03	10.00	25.00	33.00	8.00	1.25
	10:03	10:13	10.00	16.00	23.00	7.00	1.43
	10:13	10:23	10.00	19.00	25.00	6.00	1.67
	10:23	10:33	10.00	25.00	32.00	7.00	1.43
6	10:33	10:43	10.00	32.00	38.00	6.00	1.67
7							
8							
9							
10							
11							
12							
11							



Project: Test Hole No	St Francis o	f Rome	Project No:	LP1	6027	Date:	3/18/16
Death CT	0:	I-3	Tested By:		Ale	хА	
uepth of Te	st Hole, D _T :	5'	USCS Soil Cl	assification:			
		Dimension	s (inches)		Length	Width	
Diameter	(if round)=		1	ctangular)=			
	riteria Test*						
							Greater
			Time	Initial	Final	Change in	than or
			Interval,	Depth to	Depth to	Water	Equal to 6"?
Trial No.	Start Time	Stop Time	(min.)	Water (in.)	Water (in.)	Level (in.)	(y/n)
1	10:51	11:16	25.00	25.00	40.00	15.00	У
2	11:16	11:41	25.00	22.00	34.00	12.00	у
Other wise,	pre-soak (fi	ll) overnight		east twelve r	neasuremer	nts per hole	10 minutes. over at least
			Δt	Do	Df	ΔD	
			Time	Initial	Final	Change in	Percolation
			Interval	Depth to	Depth to	Water	Rate
Trial No.	Start Time	Stop Time	(min.)	Water (in.)	Water (in.)	Level (in.)	(min./in.)
1	11:43	11:53	10.00	7.00	13.00	6.00	1.67
2	11:53	12:03	10.00	13.00	18.00	6.00	1.67
3	12:03	12:13	10.00	18.00	23.00	5.00	2.00
4	12:13	12:23	10.00	19.00	25.00	6.00	1.67
5	12:23	12:33	10.00	25.00	31.00	6.00	1.67
6	12:33	12:43	10.00	31.00	36.00	5.00	2.00
7							
8							
9							
10							
10							
10							



Depth of Test Hole, D,:5'USCS Soil Classification:Test Hole Dimensions (inches)LengthWidthDiameter (if round)=6"Sides (if rectangular)=TimeInitialFinalChange in WaterTrial No.Start TimeStop TimeGreater than orTrial No.Start TimeStop TimeInterval, Depth toDepth to Water (in.)Greater than orEqual to 6"?Trial No.Start TimeStop TimeInitial InitialFinal Depth to Water (in.)Greater than orEqual to 6"?Trial No.Start TimeStop TimeInitial InitialFinal Depth toOn YaterAtt To 0Do D DOn YAD DAtt TimeDo InitialPrecolationInterval Do 0On POn ADAtt Do D DDo P Change in WaterPercolationInterval Do 0Depth to Depth toMater TimeInterval Do DOn P ADAtt Time Interval Do D <th col<="" th=""><th>Depth of Test Hole, D₁:5'USCS Soil Classification:Test Hole Dimensions (inches)LengthWidthDiameter (if round)=6"Sides (if rectangular)=ImitialFinalChange in WaterGreater than orEqual to 6"?Trial No.Start TimeStop TimeInitial (min.)Final Water (in.)Change in Water (in.)110:5411:1925:0028:0038:0010:00y211:1911:4425:0025:0037:0012:00y110:5411:1925:0025:0037:0012:00y110:5411:1925:0025:0037:0012:00y111:4511:50Otain at least twelve measurements taken every 10 minutes.0orneroidsMater (in.)Water (in.)WaterPercolation111:4511:5510:0023:0038:007:001:43111:4511:5510:0030:0038:007:001:43211:5512:0510:0030:0038:007:001:43312:0512:1510:0038:0045:007:001:43412:1512:2510:0031:006:001:67512:2512:3510:0025:0031:006:001:67612:3512:4510:0031:0</th><th>Project:</th><th>St Francis c</th><th>f Rome</th><th>Project No:</th><th>LP10</th><th>6027</th><th>Date:</th><th>3/18/16</th></th>	<th>Depth of Test Hole, D₁:5'USCS Soil Classification:Test Hole Dimensions (inches)LengthWidthDiameter (if round)=6"Sides (if rectangular)=ImitialFinalChange in WaterGreater than orEqual to 6"?Trial No.Start TimeStop TimeInitial (min.)Final Water (in.)Change in Water (in.)110:5411:1925:0028:0038:0010:00y211:1911:4425:0025:0037:0012:00y110:5411:1925:0025:0037:0012:00y110:5411:1925:0025:0037:0012:00y111:4511:50Otain at least twelve measurements taken every 10 minutes.0orneroidsMater (in.)Water (in.)WaterPercolation111:4511:5510:0023:0038:007:001:43111:4511:5510:0030:0038:007:001:43211:5512:0510:0030:0038:007:001:43312:0512:1510:0038:0045:007:001:43412:1512:2510:0031:006:001:67512:2512:3510:0025:0031:006:001:67612:3512:4510:0031:0</th> <th>Project:</th> <th>St Francis c</th> <th>f Rome</th> <th>Project No:</th> <th>LP10</th> <th>6027</th> <th>Date:</th> <th>3/18/16</th>	Depth of Test Hole, D ₁ :5'USCS Soil Classification:Test Hole Dimensions (inches)LengthWidthDiameter (if round)=6"Sides (if rectangular)=ImitialFinalChange in WaterGreater than orEqual to 6"?Trial No.Start TimeStop TimeInitial (min.)Final Water (in.)Change in Water (in.)110:5411:1925:0028:0038:0010:00y211:1911:4425:0025:0037:0012:00y110:5411:1925:0025:0037:0012:00y110:5411:1925:0025:0037:0012:00y111:4511:50Otain at least twelve measurements taken every 10 minutes.0orneroidsMater (in.)Water (in.)WaterPercolation111:4511:5510:0023:0038:007:001:43111:4511:5510:0030:0038:007:001:43211:5512:0510:0030:0038:007:001:43312:0512:1510:0038:0045:007:001:43412:1512:2510:0031:006:001:67512:2512:3510:0025:0031:006:001:67612:3512:4510:0031:0	Project:	St Francis c	f Rome	Project No:	LP10	6027	Date:	3/18/16
Test Hole Dimensions (inches)LengthWidthDiameter (if round)=6"Sides (if rectangular)=Image: Sides (if rectangular)=Image: Sides (if rectangular)=Sandy Soil Criteria Test*TimeInitialFinalChange in Water (in.)Greater than orTrial No.Start TimeStop Time(min.)Water (in.)Water (in.)Level (in.)(y/n)110:5411:1925:0028:0038:0010:00y211:1911:4425:0025:0037:0012:00yIf two consecutive measurements show that six inches of water seeps away in less than 25minutes, the test shall be run for an additional hour with measurements taken every 10 minutes.Dther wise, pre-soak (fill) overnight. Obtain at least twelve measurements per hole over at leastintervalDepth toDepth toYange Start TimeStart TimeStart TimeInitialFinalChange in Percolation111:4511:5510:0023:0030:007:001:43211:5512:0510:0038:0045:007:001:43312:0512:1510:0025:0031:006:001:67412:1512:2510:0031:0037:006:001:67512:2512:3510:0025:0031:006:001:67612:3512:4510:0031:0037:006:001:677Image: Size Size Size Size Size Size Size Si	Test Hole Dimensions (inches)LengthWidthDiameter (if round)=6"Sides (if rectangular)=Image: Sides (if rectangular)=Sandy Soil Criteria Test*TimeInitialFinalChange in WaterTrial No.Start TimeStop Time(min.)Water (in.)Water (in.)Level (in.)110:5411:1925:0028:0038:0010:00y211:1911:4425:0025:0037:0012:00yIf two consecutive measurements show that six inches of water seeps away in less than 25minutes, the test shall be run for an additional hour with measurements taken every 10 minutes.Other wise, pre-soak (fill) overnight. Obtain at least twelve measurements per hole over at least ix hours (approximately 30 minute intervals) with a precision of at least 0.25".Trial No.Start TimeStop Time(min.)Water (in.)Water (in.)PercolationIntervalDepth toDepth toDepth toMater (in.)Image: Nater	Test Hole N	0:	1-4	Tested By:		Ale	хA		
Diameter (if round)= 6" Sides (if rectangular)= Image: Charage in the standy Soil Criteria Test* Sindy Soil Criteria Test* Time Initial Final Depth to Greater than or Equal to 6"? Trial No. Start Time Stop Time (min.) Water (in.) Water (in.) Level (in.) (y/n) 1 10:54 11:19 25:00 28:00 38:00 10:00 y 2 11:19 11:44 25:00 25:00 37:00 12:00 y If two consecutive measurements show that six inches of water seeps away in less than 25 ninutes, the test shall be run for an additional hour with measurements per hole over at least ix hours (approximately 30 minute intervals) with a precision of at least 0.25". Percolation Trial No. Start Time Stop Time (min.) Water (in.) Water (in.) Level (in.) (min./in.) 1 11:45 11:55 10:00 23:00 30:00 7.00 1.43 2 11:55 12:05 10:00 30:00 30:00 7.00 1.43 1 11:45 11:5	Diameter (if round)= 6" Sides (if rectangular)= Image: Construct of the state of the s	Depth of Te	st Hole, D _T :	5'	USCS Soil Cl	assification:				
Andy Soil Criteria Test* Time Interval, Depth to Initial Depth to Final Depth to Greater Water Water 1 10:54 11:19 25:00 28:00 38:00 10:00 y 2 11:19 11:44 25:00 28:00 38:00 10:00 y 2 11:19 11:44 25:00 25:00 37:00 12:00 y 1f two consecutive measurements show that six inches of water seeps away in less than 25 minutes, the test shall be run for an additional hour with measurements taken every 10 minutes. 10 minutes. 25:00 0ther wise, pre-soak (fill) overnight. Obtain at least twelve measurements per hole over at least ix hours (approximately 30 minute intervals) with a precision of at least 0.25". Percolation Trial No. Start Time Stop Time (min.) Water (in.) Water (in.) Percolation 1 11:45 11:55 10:00 23:00 30:00 7:00 1:43 2 11:55 12:05 10:00 38:00 45:00 7:00 1:43 3 12:05 12:15 10:00 25:00 6:00	Sandy Soil Criteria Test* Time Interval, Depth to Initial Depth to Final Depth to Greater Water (mailed the construction of the constructin on the construction of the construction of the construc		Test Hole	Dimension	s (inches)		Length	Width		
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1 10:54 11:19 25.00 28.00 38.00 10.00 y 2 11:19 11:44 25.00 25.00 37.00 12.00 y If two consecutive measurements show that six inches of water seeps away in less than 25 ninutes, the test shall be run for an additional hour with measurements taken every 10 minutes. Other wise, pre-soak (fill) overnight. Obtain at least twelve measurements per hole over at least ix hours (approximately 30 minute intervals) with a precision of at least 0.25". ΔD Trial No. Start Time Stop Time (min.) Water (in.) Depth to Water (in.) Water (in.) Rate 1 11:45 11:55 10:00 23:00 30:00 7:00 1:43 2 11:55 12:05 10:00 23:00 30:00 7:00 1:43 3 12:05 12:05 10:00 38:00 45:00 7:00 1:43 4 12:15 12:25 10:00 38:00 45:00 7:00 1:43 4 12:15 12:25 10:00 31:00	1 10:54 11:19 25.00 28.00 38.00 10.00 y 2 11:19 11:44 25.00 25.00 37.00 12.00 y If two consecutive measurements show that six inches of water seeps away in less than 25 ninutes, the test shall be run for an additional hour with measurements taken every 10 minutes. Other wise, pre-soak (fill) overnight. Obtain at least twelve measurements per hole over at least ix hours (approximately 30 minute intervals) with a precision of at least 0.25". ΔD Initial X hours (approximately 30 minute intervals) with a precision of at least 0.25". ΔD Percolation Rate Trial No. Start Time Stop Time (min.) Water (in.) Water (in.) Level (in.) (min./in.) 1 11:45 11:55 10.00 23.00 30.00 7.00 1.43 2 11:55 12:05 10.00 38.00 45.00 7.00 1.43 3 12:05 12:05 10.00 38.00 45.00 7.00 1.43 4 12:15 12:25 10.00 31.00 6.00 1.67 <td></td> <td></td> <td></td> <td>Interval,</td> <td>Depth to</td> <td>Depth to</td> <td>Water</td> <td>Equal to 6"?</td>				Interval,	Depth to	Depth to	Water	Equal to 6"?	
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APPENDIX F

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