

# **SCHEU INDUSTRIAL PARK**

Rancho Cucamonga, CA

## **PRELIMINARY DRAINAGE STUDY**

March 22, 2018



Reference 149-215.022

**PREPARED BY:**

**Encompass Associates, Inc.**

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# CONTENTS

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SECTION	TITLE
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---

- |     |   |
|-----|---|
| □ A | <b><i>DISCUSSION</i></b> <ul style="list-style-type: none"><li>□ <i>Vicinity Map</i></li><li>□ <i>Index Map</i></li></ul> |
| □ Q | <b><i>Q100 HYDROLOGY</i></b>  |

- |     |  |
|-----|--|
| □ R | <b><i>REFERENCES &amp; MAPS</i></b> <ul style="list-style-type: none"><li>□ <i>Soils Map (from County Hydrology Manual)</i></li><li>□ <i>Isohyetal Map (from County Hydrology Manual)</i></li><li>□ <i>Hydrology Map</i></li></ul> |
|-----|--|
-

## ***DISCUSSION***

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The purpose of this preliminary drainage study is to determine the drainage facility requirements for Scheu Industrial Park in Rancho Cucamonga. Specifically, the subject project is located south of Acacia Street, west of industrial businesses, north of 7th Street, and east of Archibald Avenue, in the City of Rancho Cucamonga, County of San Bernardino, California.

The subject site will consist of 4 warehouse buildings, with an approximate area of 16 acres. Proposed drainage is overland and by sheet flow generally in a southwesterly direction. The site is not subject to off-site runoff.

The project is tributary to an existing storm drain in Archibald Avenue. A storm drain in 7<sup>th</sup> Street will be constructed to collect runoff from multiple on-site subareas.

The 100-year storm event was modeled in the rational method hydrology calculations in this study.

The rational method hydrologic model, as defined by Flood Control for San Bernardino County, was followed in the determination of storm runoff. AES software was utilized for hydrology calculations and some street flow depth analysis.

# **RATIONAL METHOD**

## **HYDROLOGY**

\*\*\*\*\*  
RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
(Reference: 1986 SAN BERNARDINO CO. HYDROLOGY CRITERION)  
(c) Copyright 1983-2012 Advanced Engineering Software (aes)  
Ver. 19.0 Release Date: 06/01/2012 License ID 1584

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\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
\* SCHEU PROPERTY - NEC 7TH AND ARCHIBALD \*  
\* DEVELOPED HYDROLOGY \*  
\* 100-YEAR \*

FILE NAME: Z:\FTP\AES\SCHUED00.DAT  
TIME/DATE OF STUDY: 09:34 03/22/2018

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--\*TIME-OF-CONCENTRATION MODEL\*--

USER SPECIFIED STORM EVENT(YEAR) = 100.00  
SPECIFIED MINIMUM PIPE SIZE(INCH) = 8.00  
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95  
\*USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL\*

SLOPE OF INTENSITY DURATION CURVE(LOG(I;IN/HR) vs. LOG(Tc;MIN)) = 0.6000  
USER SPECIFIED 1-HOUR INTENSITY(INCH/HOUR) = 1.4000

\*ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD\*

\*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL\*

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET  
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)\*(Velocity) Constraint = 6.0 (FT\*FT/S)

\*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN  
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*

\*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

\*\*\*\*\*  
FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

```
=====
INITIAL SUBAREA FLOW-LENGTH( FEET) = 660.00
ELEVATION DATA: UPSTREAM( FEET) = 97.00 DOWNSTREAM( FEET) = 87.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.432
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.249
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 0.90 0.75 0.100 56 9.43
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 3.38
TOTAL AREA(ACRES) = 0.90 PEAK FLOW RATE(CFS) = 3.38

*****
FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM( FEET) = 82.00 DOWNSTREAM( FEET) = 78.00
FLOW LENGTH( FEET) = 715.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 15.0 INCH PIPE IS 9.4 INCHES
PIPE-FLOW VELOCITY( FEET/SEC.) = 4.18
ESTIMATED PIPE DIAMETER( INCH) = 15.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.38
PIPE TRAVEL TIME(MIN.) = 2.85 Tc(MIN.) = 12.28
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 1375.00 FEET.

*****
FLOW PROCESS FROM NODE 3.00 TO NODE 3.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE 3.00 TO NODE 3.00 IS CODE = 13
-----
>>>>CLEAR THE MAIN-STREAM MEMORY<<<<
=====

*****
FLOW PROCESS FROM NODE 1.00 TO NODE 3.10 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH( FEET) = 904.00
ELEVATION DATA: UPSTREAM( FEET) = 97.00 DOWNSTREAM( FEET) = 86.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.176
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.837
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
```

LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN	(MIN.)
COMMERCIAL	B	1.20	0.75	0.100	56	11.18
SUBAREA AVERAGE PERVIOUS LOSS RATE, $F_p$ (INCH/HR) = 0.75						
SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p$ = 0.100						
SUBAREA RUNOFF (CFS) = 4.06						
TOTAL AREA (ACRES) = 1.20 PEAK FLOW RATE (CFS) = 4.06						

\*\*\*\*\*  
FLOW PROCESS FROM NODE 3.10 TO NODE 3.10 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE $T_c$ (MIN.) = 11.18						
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.837						
SUBAREA LOSS RATE DATA (AMC II):						
DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN	
COMMERCIAL	B	1.70	0.75	0.100	56	
SUBAREA AVERAGE PERVIOUS LOSS RATE, $F_p$ (INCH/HR) = 0.75						
SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p$ = 0.100						
SUBAREA AREA (ACRES) = 1.70 SUBAREA RUNOFF (CFS) = 5.76						
EFFECTIVE AREA (ACRES) = 2.90 AREA-AVERAGED $F_m$ (INCH/HR) = 0.07						
AREA-AVERAGED $F_p$ (INCH/HR) = 0.75 AREA-AVERAGED $A_p$ = 0.10						
TOTAL AREA (ACRES) = 2.9 PEAK FLOW RATE (CFS) = 9.82						

\*\*\*\*\*  
FLOW PROCESS FROM NODE 3.10 TO NODE 3.20 IS CODE = 91  
-----

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

UPSTREAM NODE ELEVATION (FEET) = 86.00					
DOWNSTREAM NODE ELEVATION (FEET) = 85.00					
CHANNEL LENGTH THRU SUBAREA (FEET) = 230.00					
"V" GUTTER WIDTH (FEET) = 3.00	GUTTER HIKE (FEET) = 0.200				
PAVEMENT LIP (FEET) = 0.030 MANNING'S N = .0150					
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.02000					
MAXIMUM DEPTH (FEET) = 0.50					
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.512					
SUBAREA LOSS RATE DATA (AMC II):					
DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN
COMMERCIAL	B	0.40	0.75	0.100	56
SUBAREA AVERAGE PERVIOUS LOSS RATE, $F_p$ (INCH/HR) = 0.75					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p$ = 0.100					
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 10.45					
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 2.16					
AVERAGE FLOW DEPTH (FEET) = 0.50 FLOOD WIDTH (FEET) = 30.00					
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 1.78 $T_c$ (MIN.) = 12.95					
SUBAREA AREA (ACRES) = 0.40 SUBAREA RUNOFF (CFS) = 1.24					
EFFECTIVE AREA (ACRES) = 3.30 AREA-AVERAGED $F_m$ (INCH/HR) = 0.07					
AREA-AVERAGED $F_p$ (INCH/HR) = 0.75 AREA-AVERAGED $A_p$ = 0.10					
TOTAL AREA (ACRES) = 3.3 PEAK FLOW RATE (CFS) = 10.21					

END OF SUBAREA "V" GUTTER HYDRAULICS:

DEPTH (FEET) = 0.50	FLOOD WIDTH (FEET) = 29.74
FLOW VELOCITY (FEET/SEC.) = 2.14	DEPTH*VELOCITY (FT*FT/SEC) = 1.07
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.20 = 1134.00 FEET.	



```

*****
FLOW PROCESS FROM NODE      3.20 TO NODE      3.20 IS CODE =   1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.95
RAINFALL INTENSITY(INCH/HR) = 3.51
AREA-AVERAGED Fm(INCH/HR) = 0.07
AREA-AVERAGED Fp(INCH/HR) = 0.75
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 3.30
TOTAL STREAM AREA(ACRES) = 3.30
PEAK FLOW RATE(CFS) AT CONFLUENCE = 10.21

*****
FLOW PROCESS FROM NODE      3.21 TO NODE      3.22 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 154.00
ELEVATION DATA: UPSTREAM(FEET) = 89.00 DOWNSTREAM(FEET) = 87.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 5.435
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.914
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
LAND USE              GROUP  (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
COMMERCIAL            B      0.20    0.75    0.100    56   5.43
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 1.05
TOTAL AREA(ACRES) = 0.20 PEAK FLOW RATE(CFS) = 1.05

*****
FLOW PROCESS FROM NODE      3.22 TO NODE      3.23 IS CODE =  31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 85.00 DOWNSTREAM(FEET) = 84.00
FLOW LENGTH(FEET) = 60.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 9.0 INCH PIPE IS 4.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.74
ESTIMATED PIPE DIAMETER(INCH) = 9.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.05
PIPE TRAVEL TIME(MIN.) = 0.21 Tc(MIN.) = 5.65
LONGEST FLOWPATH FROM NODE 3.21 TO NODE 3.23 = 214.00 FEET.

*****
FLOW PROCESS FROM NODE      3.23 TO NODE      3.23 IS CODE =  81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

```

```

=====
MAINLINE Tc(MIN.) = 5.65
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.781
SUBAREA LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp      Ap      SCS
    LAND USE          GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL              B        0.30    0.75    0.100    56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 0.30      SUBAREA RUNOFF(CFS) = 1.54
EFFECTIVE AREA(ACRES) = 0.50    AREA-AVERAGED Fm(INCH/HR) = 0.07
AREA-AVERAGED Fp(INCH/HR) = 0.75 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 0.5        PEAK FLOW RATE(CFS) = 2.57

*****
FLOW PROCESS FROM NODE 3.23 TO NODE 3.24 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 84.00 DOWNSTREAM(FEET) = 82.00
FLOW LENGTH(FEET) = 239.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 8.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.51
ESTIMATED PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.57
PIPE TRAVEL TIME(MIN.) = 0.88 Tc(MIN.) = 6.53
LONGEST FLOWPATH FROM NODE 3.21 TO NODE 3.24 = 453.00 FEET.

*****
FLOW PROCESS FROM NODE 3.24 TO NODE 3.24 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 6.53
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.298
SUBAREA LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp      Ap      SCS
    LAND USE          GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL              B        0.70    0.75    0.100    56
COMMERCIAL              B        0.20    0.75    0.100    56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 0.90      SUBAREA RUNOFF(CFS) = 4.23
EFFECTIVE AREA(ACRES) = 1.40    AREA-AVERAGED Fm(INCH/HR) = 0.07
AREA-AVERAGED Fp(INCH/HR) = 0.75 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 1.4        PEAK FLOW RATE(CFS) = 6.58

*****
FLOW PROCESS FROM NODE 3.24 TO NODE 3.25 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 82.00 DOWNSTREAM(FEET) = 80.00
FLOW LENGTH(FEET) = 200.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.5 INCHES

```

PIPE-FLOW VELOCITY (FEET/SEC.) = 6.15  
ESTIMATED PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 6.58  
PIPE TRAVEL TIME (MIN.) = 0.54 Tc (MIN.) = 7.07  
LONGEST FLOWPATH FROM NODE 3.21 TO NODE 3.25 = 653.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 3.25 TO NODE 3.25 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc (MIN.) = 7.07  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 5.050  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.60	0.75	0.100	56
COMMERCIAL	B	0.20	0.75	0.100	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.75  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100  
SUBAREA AREA (ACRES) = 0.80 SUBAREA RUNOFF (CFS) = 3.58  
EFFECTIVE AREA (ACRES) = 2.20 AREA-AVERAGED Fm (INCH/HR) = 0.07  
AREA-AVERAGED Fp (INCH/HR) = 0.75 AREA-AVERAGED Ap = 0.10  
TOTAL AREA (ACRES) = 2.2 PEAK FLOW RATE (CFS) = 9.85

\*\*\*\*\*

FLOW PROCESS FROM NODE 3.25 TO NODE 3.26 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 80.00 DOWNSTREAM (FEET) = 79.00  
FLOW LENGTH (FEET) = 113.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 21.0 INCH PIPE IS 12.7 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.49  
ESTIMATED PIPE DIAMETER (INCH) = 21.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 9.85  
PIPE TRAVEL TIME (MIN.) = 0.29 Tc (MIN.) = 7.36  
LONGEST FLOWPATH FROM NODE 3.21 TO NODE 3.26 = 766.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 3.26 TO NODE 3.26 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc (MIN.) = 7.36  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.930  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.40	0.75	0.100	56
COMMERCIAL	B	0.10	0.75	0.100	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.75  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100  
SUBAREA AREA (ACRES) = 0.50 SUBAREA RUNOFF (CFS) = 2.18  
EFFECTIVE AREA (ACRES) = 2.70 AREA-AVERAGED Fm (INCH/HR) = 0.07  
AREA-AVERAGED Fp (INCH/HR) = 0.75 AREA-AVERAGED Ap = 0.10

TOTAL AREA (ACRES) = 2.7 PEAK FLOW RATE (CFS) = 11.80

\*\*\*\*\*  
FLOW PROCESS FROM NODE 3.26 TO NODE 3.20 IS CODE = 31

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<  
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) =	79.00	DOWNSTREAM (FEET) =	77.00
FLOW LENGTH (FEET) =	230.00	MANNING'S N =	0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS	14.5 INCHES		
PIPE-FLOW VELOCITY (FEET/SEC.) =	6.68		
ESTIMATED PIPE DIAMETER (INCH) =	21.00	NUMBER OF PIPES =	1
PIPE-FLOW (CFS) =	11.80		
PIPE TRAVEL TIME (MIN.) =	0.57	Tc (MIN.) =	7.94
LONGEST FLOWPATH FROM NODE 3.21 TO NODE 3.20 =	996.00 FEET.		

\*\*\*\*\*  
FLOW PROCESS FROM NODE 3.20 TO NODE 3.20 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====

TOTAL NUMBER OF STREAMS =	2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:	
TIME OF CONCENTRATION (MIN.) =	7.94
RAINFALL INTENSITY (INCH/HR) =	4.71
AREA-AVERAGED Fm (INCH/HR) =	0.07
AREA-AVERAGED Fp (INCH/HR) =	0.75
AREA-AVERAGED Ap =	0.10
EFFECTIVE STREAM AREA (ACRES) =	2.70
TOTAL STREAM AREA (ACRES) =	2.70
PEAK FLOW RATE (CFS) AT CONFLUENCE =	11.80

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	10.21	12.95	3.512	0.75 ( 0.07)	0.10	3.3	1.00
2	11.80	7.94	4.713	0.75 ( 0.07)	0.10	2.7	3.21

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	20.24	7.94	4.713	0.75 ( 0.07)	0.10	4.7	3.21
2	18.95	12.95	3.512	0.75 ( 0.07)	0.10	6.0	1.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) =	20.24	Tc (MIN.) =	7.94
EFFECTIVE AREA (ACRES) =	4.72	AREA-AVERAGED Fm (INCH/HR) =	0.07
AREA-AVERAGED Fp (INCH/HR) =	0.75	AREA-AVERAGED Ap =	0.10
TOTAL AREA (ACRES) =	6.0		
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.20 =	1134.00 FEET.		

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```

FLOW PROCESS FROM NODE      3.20 TO NODE      3.30 IS CODE =  31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    77.00  DOWNSTREAM(FEET) =    76.00
FLOW LENGTH(FEET) =    81.00  MANNING'S N =  0.013
DEPTH OF FLOW IN  24.0 INCH PIPE IS  16.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    8.71
ESTIMATED PIPE DIAMETER(INCH) =  24.00  NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =    20.24
PIPE TRAVEL TIME(MIN.) =    0.15  Tc(MIN.) =    8.09
LONGEST FLOWPATH FROM NODE      1.00 TO NODE      3.30 =    1215.00 FEET.

*****
FLOW PROCESS FROM NODE      3.30 TO NODE      3.30 IS CODE =  81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) =    8.09
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =  4.658
SUBAREA LOSS RATE DATA(AMC  II):
  DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp      Ap      SCS
    LAND USE          GROUP  (ACRES)  (INCH/HR)  (DECIMAL)  CN
COMMERCIAL              B        2.80    0.75    0.100    56
COMMERCIAL              B        1.00    0.75    0.100    56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.75
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
SUBAREA AREA(ACRES) =    3.80  SUBAREA RUNOFF(CFS) =    15.68
EFFECTIVE AREA(ACRES) =    8.52  AREA-AVERAGED Fm(INCH/HR) =  0.07
AREA-AVERAGED Fp(INCH/HR) =  0.75  AREA-AVERAGED Ap =  0.10
TOTAL AREA(ACRES) =    9.8  PEAK FLOW RATE(CFS) =    35.15

*****
FLOW PROCESS FROM NODE      3.30 TO NODE      3.00 IS CODE =  31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    76.00  DOWNSTREAM(FEET) =    73.00
FLOW LENGTH(FEET) =    46.00  MANNING'S N =  0.013
DEPTH OF FLOW IN  21.0 INCH PIPE IS  15.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =   18.54
ESTIMATED PIPE DIAMETER(INCH) =  21.00  NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =    35.15
PIPE TRAVEL TIME(MIN.) =    0.04  Tc(MIN.) =    8.13
LONGEST FLOWPATH FROM NODE      1.00 TO NODE      3.00 =    1261.00 FEET.

*****
FLOW PROCESS FROM NODE      3.00 TO NODE      3.00 IS CODE =  11
-----
>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
  STREAM      Q      Tc  Intensity  Fp(Fm)      Ap      Ae  HEADWATER
  NUMBER      (CFS)  (MIN.) (INCH/HR) (INCH/HR)      (ACRES)  NODE

```

1	35.15	8.13	4.644	0.75 ( 0.07)	0.10	8.5	3.21
2	30.10	13.15	3.480	0.75 ( 0.07)	0.10	9.8	1.00

LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 1261.00 FEET.

\*\* MEMORY BANK # 1 CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.38	12.28	3.626	0.75 ( 0.07)	0.10	0.9	1.00

LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 1375.00 FEET.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	38.03	8.13	4.644	0.75 ( 0.07)	0.10	9.1	3.21
2	34.35	12.28	3.626	0.75 ( 0.07)	0.10	10.5	1.00
3	33.34	13.15	3.480	0.75 ( 0.07)	0.10	10.7	1.00

TOTAL AREA (ACRES) = 10.7

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 38.03 Tc (MIN.) = 8.132  
EFFECTIVE AREA (ACRES) = 9.12 AREA-AVERAGED Fm (INCH/HR) = 0.07  
AREA-AVERAGED Fp (INCH/HR) = 0.75 AREA-AVERAGED Ap = 0.10  
TOTAL AREA (ACRES) = 10.7  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 1375.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 3.00 TO NODE 4.00 IS CODE = 31  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 73.00 DOWNSTREAM (FEET) = 72.50  
FLOW LENGTH (FEET) = 64.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 33.0 INCH PIPE IS 23.0 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 8.59  
ESTIMATED PIPE DIAMETER (INCH) = 33.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 38.03  
PIPE TRAVEL TIME (MIN.) = 0.12 Tc (MIN.) = 8.26  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4.00 = 1439.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 4.00 TO NODE 4.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 8.26  
RAINFALL INTENSITY (INCH/HR) = 4.60  
AREA-AVERAGED Fm (INCH/HR) = 0.07  
AREA-AVERAGED Fp (INCH/HR) = 0.75  
AREA-AVERAGED Ap = 0.10  
EFFECTIVE STREAM AREA (ACRES) = 9.12  
TOTAL STREAM AREA (ACRES) = 10.70  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 38.03

\*\*\*\*\*

FLOW PROCESS FROM NODE 4.10 TO NODE 4.20 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 714.00  
ELEVATION DATA: UPSTREAM(FEET) = 92.00 DOWNSTREAM(FEET) = 86.00

$T_c = K * [(LENGTH^{**} 3.00) / (ELEVATION\ CHANGE)]^{**0.20}$

SUBAREA ANALYSIS USED MINIMUM  $T_c$ (MIN.) = 10.951

\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.885

SUBAREA  $T_c$  AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN	$T_c$ (MIN.)
COMMERCIAL	B	3.20	0.75	0.100	56	10.95

SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.75

SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.100

SUBAREA RUNOFF(CFS) = 10.97

TOTAL AREA(ACRES) = 3.20 PEAK FLOW RATE(CFS) = 10.97

\*\*\*\*\*

FLOW PROCESS FROM NODE 4.20 TO NODE 4.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 81.00 DOWNSTREAM(FEET) = 72.50  
FLOW LENGTH(FEET) = 222.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 15.0 INCH PIPE IS 11.0 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.34  
ESTIMATED PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 10.97  
PIPE TRAVEL TIME(MIN.) = 0.33  $T_c$ (MIN.) = 11.28  
LONGEST FLOWPATH FROM NODE 4.10 TO NODE 4.00 = 936.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 4.00 TO NODE 4.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 11.28  
RAINFALL INTENSITY(INCH/HR) = 3.82  
AREA-AVERAGED  $F_m$ (INCH/HR) = 0.07  
AREA-AVERAGED  $F_p$ (INCH/HR) = 0.75  
AREA-AVERAGED  $A_p$  = 0.10  
EFFECTIVE STREAM AREA(ACRES) = 3.20  
TOTAL STREAM AREA(ACRES) = 3.20  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 10.97

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	$Q$ (CFS)	$T_c$ (MIN.)	Intensity (INCH/HR)	$F_p$ ( $F_m$ ) (INCH/HR)	$A_p$	$A_e$ (ACRES)	HEADWATER NODE
1	38.03	8.26	4.602	0.75( 0.07)	0.10	9.1	3.21
1	34.35	12.41	3.603	0.75( 0.07)	0.10	10.5	1.00

1	33.34	13.28	3.460	0.75 ( 0.07)	0.10	10.7	1.00
2	10.97	11.28	3.817	0.75 ( 0.07)	0.10	3.2	4.10

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	47.75	8.26	4.602	0.75 ( 0.07)	0.10	11.5	3.21
2	46.33	11.28	3.817	0.75 ( 0.07)	0.10	13.3	4.10
3	44.70	12.41	3.603	0.75 ( 0.07)	0.10	13.7	1.00
4	43.26	13.28	3.460	0.75 ( 0.07)	0.10	13.9	1.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 47.75 Tc(MIN.) = 8.26  
EFFECTIVE AREA(ACRES) = 11.46 AREA-AVERAGED Fm(INCH/HR) = 0.07  
AREA-AVERAGED Fp(INCH/HR) = 0.75 AREA-AVERAGED Ap = 0.10  
TOTAL AREA(ACRES) = 13.9  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4.00 = 1439.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 72.50 DOWNSTREAM(FEET) = 72.00  
FLOW LENGTH(FEET) = 278.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 45.0 INCH PIPE IS 35.2 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.15  
ESTIMATED PIPE DIAMETER(INCH) = 45.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 47.75  
PIPE TRAVEL TIME(MIN.) = 0.90 Tc(MIN.) = 9.16  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 5.00 = 1717.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 5.00 TO NODE 5.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 9.16  
RAINFALL INTENSITY(INCH/HR) = 4.33  
AREA-AVERAGED Fm(INCH/HR) = 0.07  
AREA-AVERAGED Fp(INCH/HR) = 0.75  
AREA-AVERAGED Ap = 0.10  
EFFECTIVE STREAM AREA(ACRES) = 11.46  
TOTAL STREAM AREA(ACRES) = 13.90  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 47.75

\*\*\*\*\*  
FLOW PROCESS FROM NODE 5.10 TO NODE 5.20 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<



```
=====
INITIAL SUBAREA FLOW-LENGTH( FEET) = 267.00
ELEVATION DATA: UPSTREAM( FEET) = 88.00 DOWNSTREAM( FEET) = 85.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 6.972
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.093
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 2.00 0.75 0.100 56 6.97
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 9.03
TOTAL AREA(ACRES) = 2.00 PEAK FLOW RATE(CFS) = 9.03

*****
FLOW PROCESS FROM NODE 5.20 TO NODE 5.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM( FEET) = 80.00 DOWNSTREAM( FEET) = 72.00
FLOW LENGTH( FEET) = 51.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.1 INCHES
PIPE-FLOW VELOCITY( FEET/SEC.) = 18.68
ESTIMATED PIPE DIAMETER( INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 9.03
PIPE TRAVEL TIME(MIN.) = 0.05 Tc(MIN.) = 7.02
LONGEST FLOWPATH FROM NODE 5.10 TO NODE 5.00 = 318.00 FEET.

*****
FLOW PROCESS FROM NODE 5.00 TO NODE 5.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 7.02
RAINFALL INTENSITY(INCH/HR) = 5.07
AREA-AVERAGED Fm(INCH/HR) = 0.07
AREA-AVERAGED Fp(INCH/HR) = 0.75
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 2.00
TOTAL STREAM AREA(ACRES) = 2.00
PEAK FLOW RATE(CFS) AT CONFLUENCE = 9.03

** CONFLUENCE DATA **
STREAM Q Tc Intensity Fp(Fm) Ap Ae HEADWATER
NUMBER (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES) NODE
1 47.75 9.16 4.325 0.75( 0.07) 0.10 11.5 3.21
1 46.33 12.18 3.645 0.75( 0.07) 0.10 13.3 4.10
1 44.70 13.32 3.454 0.75( 0.07) 0.10 13.7 1.00
1 43.26 14.19 3.325 0.75( 0.07) 0.10 13.9 1.00
2 9.03 7.02 5.074 0.75( 0.07) 0.10 2.0 5.10
```

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	52.08	7.02	5.074	0.75( 0.07)	0.10	10.8	5.10
2	55.43	9.16	4.325	0.75( 0.07)	0.10	13.5	3.21
3	52.78	12.18	3.645	0.75( 0.07)	0.10	15.3	4.10
4	50.80	13.32	3.454	0.75( 0.07)	0.10	15.7	1.00
5	49.14	14.19	3.325	0.75( 0.07)	0.10	15.9	1.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 55.43 Tc(MIN.) = 9.16  
EFFECTIVE AREA(ACRES) = 13.46 AREA-AVERAGED Fm(INCH/HR) = 0.07  
AREA-AVERAGED Fp(INCH/HR) = 0.75 AREA-AVERAGED Ap = 0.10  
TOTAL AREA(ACRES) = 15.9  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 5.00 = 1717.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 72.00 DOWNSTREAM(FEET) = 71.00  
FLOW LENGTH(FEET) = 215.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 42.0 INCH PIPE IS 29.2 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.77  
ESTIMATED PIPE DIAMETER(INCH) = 42.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 55.43  
PIPE TRAVEL TIME(MIN.) = 0.46 Tc(MIN.) = 9.62  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 6.00 = 1932.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 6.00 TO NODE 6.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 9.62  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.200  
SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
COMMERCIAL B 1.00 0.75 0.100 56  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100  
SUBAREA AREA(ACRES) = 1.00 SUBAREA RUNOFF(CFS) = 3.71  
EFFECTIVE AREA(ACRES) = 14.46 AREA-AVERAGED Fm(INCH/HR) = 0.07  
AREA-AVERAGED Fp(INCH/HR) = 0.75 AREA-AVERAGED Ap = 0.10  
TOTAL AREA(ACRES) = 16.9 PEAK FLOW RATE(CFS) = 55.43  
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 16.9 TC(MIN.) = 9.62  
EFFECTIVE AREA(ACRES) = 14.46 AREA-AVERAGED Fm(INCH/HR) = 0.07  
AREA-AVERAGED Fp(INCH/HR) = 0.75 AREA-AVERAGED Ap = 0.100

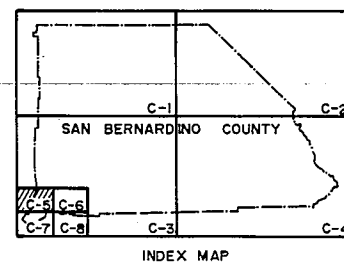
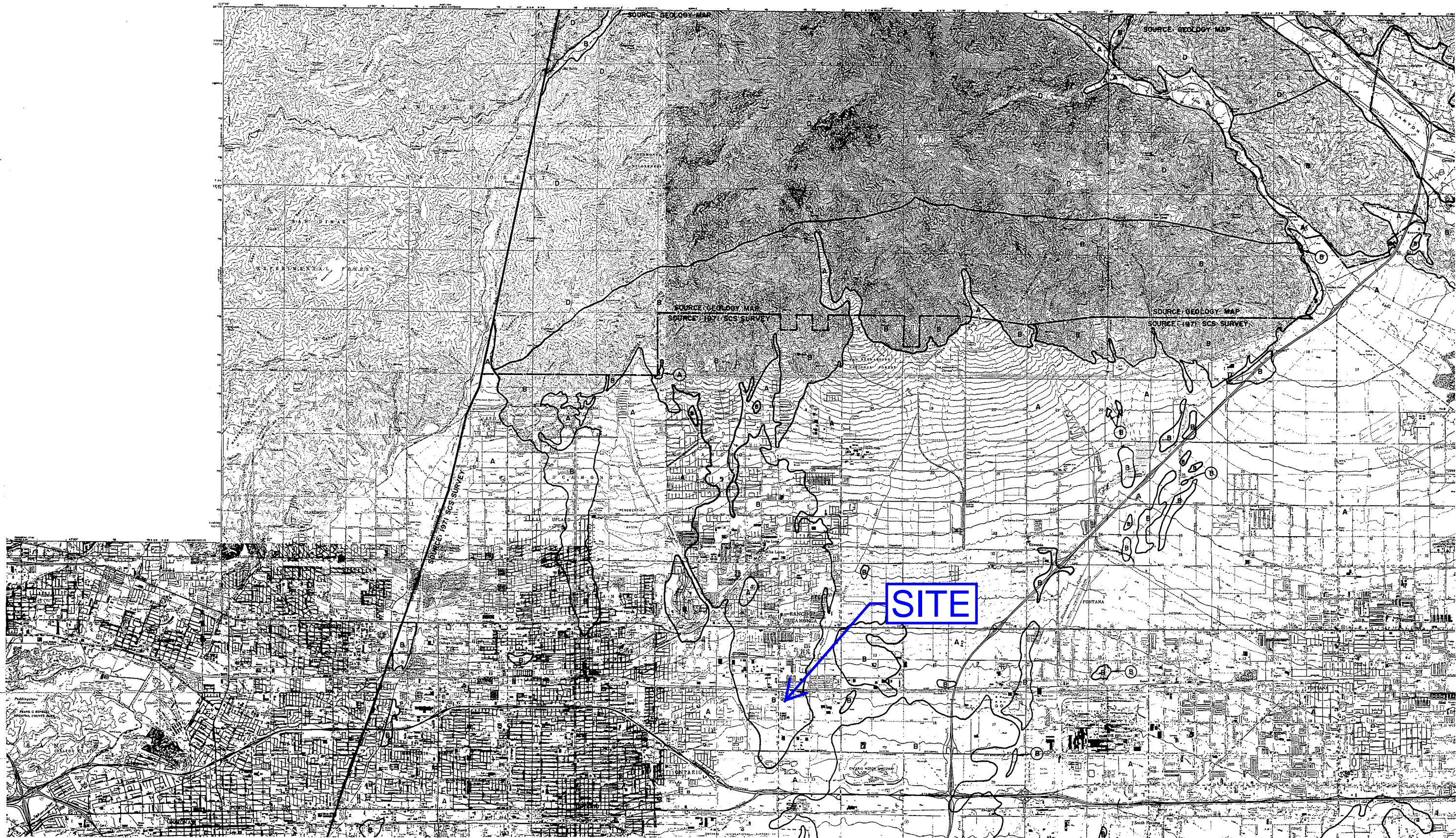
PEAK FLOW RATE (CFS) = 55.43

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	52.08	7.49	4.878	0.75 ( 0.07)	0.10	11.8	5.10
2	55.43	9.62	4.200	0.75 ( 0.07)	0.10	14.5	3.21
3	52.78	12.65	3.562	0.75 ( 0.07)	0.10	16.3	4.10
4	50.80	13.80	3.382	0.75 ( 0.07)	0.10	16.7	1.00
5	49.14	14.67	3.259	0.75 ( 0.07)	0.10	16.9	1.00
=====							
=====							
END OF RATIONAL METHOD ANALYSIS							

## **REFERENCES & MAPS**





- LEGEND
- SOIL GROUP BOUNDARY
  - A SOIL GROUP DESIGNATION
  - BOUNDARY OF INDICATED SOURCE



**SCALE REDUCED BY 1/2**

**SAN BERNARDINO COUNTY**  
HYDROLOGY MANUAL

**HYDROLOGIC SOILS GROUP MAP  
FOR  
SOUTHWEST-A AREA**



