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December 27, 2018

Pacific West Communities, Inc. c/o TK Development Attention: Ken Koss 6891 East Dorado Court Tucson, AZ 85715-4755 sylken1@sbcglobal.net

Supplemental Recommendations Lime Stabilization and Pavement Design 3422 Santa Rosa Avenue Santa Rosa, California Project Number: 4307.01.04.1

As requested, this letter presents supplemental recommendations for lime stabilization of onsite soil and pavement design for the property located at 3422 Santa Rosa Avenue in Santa Rosa, California. We performed a geotechnical study for the project and presented the results in a letter dated December 21, 2018. That report did not provide recommendations for lime stabilization or pavement design.

Excavations

The excavation of weak, compressible, surface soil should extend at least 12 inches below pavement subgrade, and at least 3 feet beyond the edge of pavement (where planned excavations do not completely remove the weak soil) to allow space for the installation of a select fill blanket 12 inches thick. Lime stabilized native soil may be used in lieu of select fill.

Fill Placement

The surface exposed by stripping and removal of weak, compressible surface soil should be scarified to a depth of at least 6 inches, uniformly moisture-conditioned to near optimum and compacted to at least 90 percent of the maximum dry density of the materials as determined by ASTM Test Method D-1557. Approved fill material should then be spread in thin lifts, uniformly moisture-conditioned to near optimum and properly compacted. All fill materials used in the upper 12 inches of pavement subgrade must be select. The select fill can consist of approved on-site soil or import materials with a low expansion potential, or lime stabilized on-site clayey soil. Select fill specifications are presented in our geotechnical study for the project. The upper 6 inches of pavement subgrade, and the aggregate base course for pavement should be compacted to a minimum of 95% relative compaction.

Lime Stabilization

For preliminary planning purposes, we estimate that high calcium lime mixed at a minimum of $5\frac{1}{2}$ percent (dry weight) will stabilize the moderately expansive site soil. This percentage of lime needs to be verified prior to construction with engineering analysis and laboratory Atterberg Limits and/or pH testing using lime from the same source as that planned for use on the project, and a sample of the soil to be treated. Laboratory test results and engineering analysis may indicate that a higher percentage of lime is required. The contractor should allow a minimum of 5 business days for the laboratory tests to be completed.



The lime stabilization should be performed in accordance with Section 24 of the Caltrans Standard Specifications except that a curing seal will not be required, provided the moisture content of the lime-stabilized material is maintained at or above optimum moisture content until it is permanently covered with subsequent construction. Lime stabilized materials are generally not suitable for reuse as general fill, select fill or backfill after compaction has taken place.

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Pavements

Provided the site grading is performed as recommended herein, the uppermost 12-inches of pavement subgrade soil will be either imported select fill with a minimum R-value of 20 or lime stabilized site soil that generally has an R-value of at least 50. Based on those R-values we recommend the pavement sections listed in the tables below be used.

PAVEMENT SECTIONS WITH IMPORTED SELECT FILL SUBGRADE				
ті	ASPHALT CONCRETE (feet)	CLASS 2 AGGREGATE BASE (feet)	IMPORTED SELECT FILL* (feet)	
7.0	0.30	1.15	1.0	
6.0	0.25	1.05	1.0	
5.0	0.20	0.90	1.0	

* R-value ≥ 20

PAVEMENT SECTIONS WITH LIME STABILIZED SELECT FILL SUBGRADE				
ті	ASPHALT CONCRETE (feet)	CLASS 2 AGGREGATE BASE (feet)	LIME STABILIZED SELECT FILL* (feet)	
7.0	0.35	0.50	1.0	
6.0	0.30	0.50	1.0	
5.0	0.20	0.50	1.0	

* R-value ≥ 50

Pavement thicknesses were computed using Caltrans CalFP v1.5 design software and are based on a pavement life of 20 years. These recommendations are intended to provide support for traffic represented by the indicated Traffic Indices. They are not intended to provide pavement sections for heavy concentrated construction storage or wheel loads such as forklifts, parked truck-trailers and concrete trucks (or for post-construction concentrated wheel loads such as self-loading dumpster trucks).

In areas where heavy construction storage and wheel loads are anticipated, the pavements should be designed to support these loads. Support could be provided by increasing pavement sections or by providing reinforced concrete slabs. Alternatively, paving can be deferred until heavy construction storage and wheel loads are no longer present. Loading areas for self-loading dumpster trucks should be provided with reinforced concrete slabs at least 6 inches thick, and reinforced with No. 4

bars at 12-inch centers each way.

Prior to placement of aggregate base, the upper 6 inches of the pavement subgrade soil (excluding lime stabilized soil) should be scarified, uniformly moisture-conditioned to near optimum, and compacted to at least 95 percent relative compaction to form a firm, non-yielding surface. Lime stabilized select fill subgrade soil should be compacted as specified in Section 24 of the Caltrans Standard Specifications.

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Aggregate base materials should be spread in thin layers, uniformly moisture-conditioned, and compacted to at least 95 percent relative compaction to form a firm, non-yielding surface. The materials and methods used should conform to the requirements of the City of Santa Rosa and the current edition of the Caltrans Standard Specifications, except that compaction requirements should be based on ASTM Test Method D-1557. Aggregate used for the base course should comply with the minimum requirements specified in Caltrans Standard Specifications, Section 26 for Class 2 Aggregate Base.

Parking Lot Drainage

Water tends to migrate under pavements and collect in the aggregate courses at low areas on parking lot subgrade soil, such as around storm drain inlets and the thread of paved swales leading to inlets. The ponded water will soften subgrade soil and, under repetitive heavy-wheel loads, will induce inordinately high stresses on the subgrade and pavement components that could result in untimely maintenance. Under-pavement drainage can be improved and maintenance reduced by replacing a 12-inch wide strip (extending at least 15 feet on either side of the inlet) of the select subbase layer or subgrade soil with a subdrain consisting of ³/₄-inch or 1¹/₂-inch free-draining Class 1 Permeable Material. The drain rock should be outletted into the storm drain inlet. Storm drain trenches can be made to serve as pavement subdrains. We should be consulted to verify the suitability of storm drain trenches as pavement subdrains in a case-specific basis.

Where pavements will abut landscaped areas, the pavement baserock layer and subgrade soil should be protected against saturation from irrigation and rainwater with a subdrain, similar to that previously discussed. The subdrain should extend to a depth of at least 12 inches below the bottom of the baserock layer. Alternatively, a grouted moisture cut-off that extends 12 inches below the bottom of the baserock layer should be provided below or immediately behind the curb and gutter.

The recommendations presented herein are subject to the limitations set forth in our referenced report. We trust this provides the information you require at this time. If you have questions please call.

Verv trulv vours. **RGH** Consultants

Ryan E. Padgett Project Manager

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