Appendices

G. Traffic and Parking Technical Memorandum



DATE April 25, 2019

TO Ana Gonzalez

Director, Planning and Development Riverside Unified School District 3070 Washington Street

Riverside, CA 92504

FROM Fernando Sotelo, PE, PTP

Senior Associate

SUBJECT Traffic and Parking Technical Memorandum for the Longfellow Elementary School

Expansion Project

PROJECT NUMBER RIV-25

Introduction and Project Description

The Riverside Unified School District plans to renovate and expand the existing school by acquiring two residential properties adjacent to the school, 2210 and 2226 Seventh Street. Longfellow Elementary School is at 3610 Eucalyptus Avenue in the City of Riverside, CA 92507 (see Figure 1, *Local Vicinity*). This technical memorandum assesses potential traffic impacts to the circulation system associated with the proposed expansion.

The project site is in a residential area and surrounded by residences to the west, east, south, and north (see Figure 2, *Aerial Photograph*). The proposed improvements include: a new drop-off location, parking lot, and access driveway off Franklin Avenue, shown on Figure 3, *Site Plan*. The school currently has an enrollment of about 741 students in grades K to 6. The project would modernize buildings and provide new facilities—such as additional parking spaces and new buildings. However, the project would not result in an increase in student capacity. The overall construction would be completed by the summer of 2021.

This Technical Memorandum analyzes site access and the transportation network in the vicinity of the school; describes student drop-off and pick-up procedures; provides a detailed review of the project components that would affect traffic and parking; identifies issues related to access, traffic congestion, and pedestrian/bike travel; and evaluates parking conditions. Existing traffic conditions are used as the "baseline" for the analysis and to evaluate the potential impacts of the proposed project. The overall purpose of this report is to inform decision makers and the general public whether the proposed project would result in any significant impacts.

Methodology

This section of the report sets forth guidelines for analyzing traffic impacts from projects on the roadway network and thresholds of significance. The methodologies described are generally consistent with City of Riverside requirements for the preparation of traffic impact analyses.

DEFINITION OF LEVEL OF SERVICE

Roadway capacity is generally limited by the ability to move vehicles through intersections. A level of service (LOS) is a standard performance measurement to describe the operating characteristics of a street



system in terms of the level of congestion or delay experienced by motorists. Service levels range from A through F, which relate to traffic conditions from best (uncongested, free-flowing conditions) to worst (total breakdown with stop-and-go operation).

The methodology used to assess the operation of a signalized intersection is based on the Highway Capacity Manual (HCM). The intersection LOS analysis is based on the traffic volumes observed during the peak hour conditions. The peak hours selected for analysis are the highest volumes that occur in four consecutive 15-minute periods from 7 to 9 AM and from 2 to 4 PM on weekdays. The HCM 6th edition signalized intersection methodology presents LOS in terms of control delay (in seconds per vehicle). Table 1 describes the level of service concept and the operating conditions expected under each level of service for signalized and unsignalized intersections. The software PTV Vistro 7 was used to determine the LOS at the study area intersections.

Table 1 Intersection Level of Service Descriptions

| | | Average Delay Per | Vehicle (seconds) |
|-----|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|-------------------|-------------------|
| LOS | Description | Signalized | Unsignalized |
| А | Free-flow operation. Vehicles are completely unimpeded in their ability to maneuver within the traffic stream. | 0 to 10.00 | 0 to 10.00 |
| В | Reasonable unimpeded operation. The ability to maneuver within the traffic stream is only slightly restricted, and control delay at the boundary intersections is not significant. | 10.01 to 20.00 | 10.01 to 15.00 |
| С | Stable operation. The ability to maneuver and change lanes at midsegment locations may be more restricted than at LOS B. | 20.01 to 35.00 | 15.01 to 25.00 |
| D | Less stable condition in which small increases in flow may cause substantial increases in delay and decreases in travel speeds. | 35.01 to 55.00 | 25.01 to 35.00 |
| E | Unstable operation and significant delay. Such operations may be due to some combination of adverse progression, high volume, and inappropriate signal timing at the boundary intersections. | 55.01 to 80.00 | 35.01 to 50.00 |
| F | Flow at extremely low speed. Congestion is likely occurring at the boundary intersections, as indicated by high delay and extensive queuing. | 80.01 and up | 50.01 and up |

Source: Highway Capacity Manual, 6th edition, Transportation Research Board, 2016.

DETERMINATION OF IMPACTS

The City of Riverside General Plan Circulation Element includes LOS standards. According to Policy CCM-2.3, the City shall maintain LOS D or better on Arterial Streets whenever possible.

Signalized Intersections: According to the City of Riverside 2012 Traffic Impact Study Guidelines, an impact would occur at an intersection when project trips would cause:



- » an intersection to degrade from an acceptable LOS A to D to an unacceptable LOS E or F,
- » an increase in delay during the peak hour as follows:
 - LOS A/B by 10 or more seconds
 - LOS C by 8 or more seconds
 - LOS D by 5 or more seconds
 - LOS E by 2 or more seconds
 - LOS F by 1 or more seconds

Unsignalized Intersections: An impact is considered significant if the study determines if project traffic results in the intersection to go from LOS D to LOS E or worse and the peak hour traffic signal warrant for the intersection is met with the additional project traffic.

Existing Circulation and Parking Conditions

The key roadways in the vicinity of the school are Eucalyptus Avenue, Franklin Avenue, Seventh Street, Sixth Street, and University Avenue.

- » Eucalyptus Avenue is a two-lane north-south local street. Curbside parking and paved sidewalks are available on both sides. The curb on the east side of the street adjacent to the school property is designated as a loading zone.
- » Franklin Avenue is a two-lane north-south local street. Curbside parking and paved sidewalks are available on both sides. Adjacent to the school side, there is a posted signage for no parking on Monday to Friday from 7:50 to 8:20 AM.
- » Seventh Street is a two-lane east-west local street west of the school and a cul-de-sac east of the school. On both sides there are curbside parking and paved sidewalks. West of Franklin Avenue and on the northern portion of Mission Inn Avenue, parking is limited to 10 minutes.
- » Sixth Street is a two-lane east-west local street. Paved sidewalks and curbside parking are available on both sides, with the exception of no curbside parking west of Eucalyptus Avenue and northern portion of Sixth Street (adjacent to the staff parking lot). This roadway runs along the north side of the school.
- » University Avenue is a four-lane divided roadway that passes about 150 feet south of the campus. No on-street parking is permitted. There are paved sidewalks and striped (Class II) bicycle lanes on each side of the street.

PARKING

The school currently has two parking lots for teachers and staff. Curbside parking is permitted on public streets in the vicinity of the school, including: both sides of Eucalyptus Avenue, Franklin Avenue, Sixth Street, and Mission Inn Avenue. During a site visit at approximately 8:00 AM, vehicles that parked on Eucalyptus Avenue, Franklin Avenue, Sixth Street, and Mission Inn Avenue belonged to parents, staff members, or residents.

PUBLIC TRANSIT

The Riverside Transit Authority (RTA) provides public transit bus service to the City of Riverside, including the project vicinity. Table 2 shows nearby RTA routes.



Table 2 Public Transit Routes near the Project Site

| | | | Operation | |
|-------------------|--------------------------------------------------------------------|------------------------------------------|---------------|----------------------------------------|
| Route | General Direction of Travel | Roadways near Project Site | Days per week | Frequency, weekday peak hours, minutes |
| 1 | East-West from Riverside to Corona | University Avenue | 7 | 15 |
| 10 | East-west in Riverside | Mission Inn Avenue, Eucalyptus Avenue | 7 | 70 |
| 14 | North-South, Loma Linda to Riverside | University Avenue | 7 | 75 |
| 22 | Northwest-Southeast, Riverside to Perris | University Avenue | 7 | 40 |
| 204 | East-West, Riverside to Montclair (in San Bernardino County) | University Avenue | Mon-Fri | Approx. 60 |
| Gold Line | East-West from Riverside to Corona | University Avenue | Mon-Fri | 15 |
| Source: RTA 2019. | · | - | | • |

VEHICULAR CIRCULATION

PlaceWorks reviewed the current traffic and parking operations as well as pedestrian activities during student drop-off in the morning on Tuesday, November 13, 2018, between 7:55 and 8:40 AM. The following observations relate to vehicular circulation, parking, and pedestrian activity.

The school starts at 8:20 AM, and the afternoon dismissal is at 2:40 PM except on early dismissal days on Wednesdays. There are two designated passenger/student loading areas. One is in on the western frontage of the school on Eucalyptus Avenue, and other is one the eastern frontage of school on Franklin Avenue.

The western drop-off/pick-up on Eucalyptus Avenue and adjacent to the school has a designated loading zone north of Mission Inn Avenue, extending for almost 210 feet and marked with white curbside marking. The posted sign states that it is a loading zone on Monday to Friday from 7:50 to 8:20 AM, Monday to Tuesday and Thursday to Friday from 2:30 to 3:00 PM, and Wednesday from 1:00 to 1:45 PM. Following the passenger loading zone is 100 feet of yellow curbside dedicated for a bus loading zone.

The eastern drop-off/pick-up on Franklin Avenue and adjacent to the school has a designated loading zone located south of Sixth Street, extending for almost 180 feet and marked with white curbside marking. The posted sign states it is a loading zone Monday to Friday from 7:50 to 8:20 AM. It was observed that parents also parked their cars on both sides of Eucalyptus Avenue and Franklin Avenue for a short time and walked their children to the school building.

Traffic counts were taken on Franklin Avenue at Seventh Street on Tuesday, November 27, 2018, while school was in session. The existing AM peak hour and PM student dismissal count worksheets and figures showing turn-movement volumes are provided in Attachment A. The intersection operations analysis results are summarized in Table 3. The study intersection of Franklin Avenue at Seventh Street currently operates at acceptable LOS A during the peak hours. LOS worksheets for existing conditions are provided in Attachment B.



Table 3 Existing Peak Hour Intersection Levels of Service

| | AM Peak F | lour | PM Peak I | Hour |
|---------------------------------------|-------------------------|------|----------------------------|------|
| Intersection | Average Delay (sec/veh) | LOS | Average Delay (sec/veh) | LOS |
| Franklin Avenue at Mission Inn Avenue | 8.2 | А | 7.4 | А |

Notes: LOS calculation worksheets in Attachment B. AM peak hour is from 7 to 9 AM and the PM peak hour from 2 to 4 PM.

FUTURE CONDITIONS INTERSECTION ANALYSIS

Figure 4, *Paving Plan*, shows the off-street parking lot, the drop-off area, and proposed circulation plan for the school. A two-lane loop driveway would guide student drop-off/pick-up circulation on a one-way southbound path from Franklin Avenue, entering at Seventh Street and exiting about 190 feet to south. A 40-space parking lot would provide additional guest and staff parking. As discussed above, the project would not result in an increase in student capacity. However, the proposed drop-off area would modify traffic patterns on Franklin Avenue because the new drop-off area would direct parents to drop off students in the proposed off-street location rather than at the curbside locations along Franklin Avenue. To identify potential issues with queuing and vehicular conflicts at the proposed egress driveway on Franklin Avenue south of Seventh Street, future traffic volumes and intersection levels of service and queueing calculations were performed. The future traffic volumes were predicted by reviewing existing traffic patterns and manually adjusting traffic to the proposed drop-off area. The intersection LOS with project are summarized in Table 4. Table 4 shows that the study intersections would continue to operate at acceptable LOS A during the peak hours. LOS worksheets for existing conditions are provided in Attachment B. Attachment C shows the manual adjustments to calculate future traffic with the project.

Table 4 Future Peak Hour Intersection LOS with Project

| | | AM Peak I | Hour | PM Peak I | Hour |
|----|---------------------------------------|----------------------------|------|----------------------------|------|
| | Intersection | Average Delay (sec/veh) | LOS | Average Delay (sec/veh) | LOS |
| 1. | Franklin Avenue at Mission Inn Avenue | 8.2 | А | 7.3 | А |
| 2. | Franklin Avenue at Egress Driveway | 9.0 | А | 8.8 | А |

Notes: LOS calculation worksheets in Attachment B. AM peak hour is from 7 to 9 AM and the PM peak hour from 2 to 4 PM.

Signal warrants are a set of criteria used to evaluate the potential need for the installation of a traffic signal at an unsignalized or stop-controlled intersection. The methodology for the signal warrant analysis is in the 2014 California Manual on Uniform Traffic Control Devices. The manual states that if one or more of the criteria for signal warrants is met, an engineering study is required to evaluate other factors to determine if an intersection must be signalized. This analysis uses Warrant 3 criteria, which are based on peak hour traffic volumes. The signal warrant calculations are included in Attachment D. No unsignalized study intersections would meet the peak hour warrants with project traffic. Although the project would result in modified traffic on Franklin Avenue, the drop-off would mostly move off-street and improve traffic conditions. Impacts would be less than significant. According to the impact criteria provided on page 3, there would be no impacts to the study intersections and no mitigation would be required.



CRASH DATA

A 10-year crash history of roadways and intersections on the immediate vicinity of the school including Franklin Avenue, Sixth Street, Seventh Street, (January 2008 through December 2017) was obtained from the Transportation Injury Mapping System (TIMS) website to identify potential safety issues in the vicinity of the school. Crash variables (type, severity, etc.) were reviewed at in study area to assess if any potential crash patterns might be identifiable. In the last 10 years, all the reported collisions in the study area consisted of vehicle to vehicle, vehicle to bicycle, vehicle to pedestrian, and vehicle to object. The only reported collision at an intersection in the vicinity of the school occurred at the intersection of Franklin Avenue / University Avenue. The accident was a vehicle-to-vehicle rear-end collision on University Avenue in the westbound lanes due to unsafe speeds; it resulted in injuries but no deaths. No crashes have been reported on Franklin Avenue north of University Avenue. This accident occurred in September 2007 outside school hours. Other accidents occurred on University Avenue west and east of Franklin Avenue. The 10-year collision history in the area indicates that accidents are not frequent and are typically vehicle to vehicle and not related to school activity.

Conclusion and Recommendations

This memorandum evaluates potential impacts of the project-related reconfigured drop-off/pick-up zone. The new south parking lot would alter traffic patterns on Franklin Avenue because vehicles would enter the drop-off/pick-up area at the ingress driveway at the intersection of Franklin Avenue at Seventh Street and egress via a new driveway on Franklin Avenue approximately 190 feet to the south. No impacts were identified, and all intersections would continue to operate at acceptable LOS. The reconfigured drop-off/pick-up zone would allow more vehicles to queue on the school property while reducing student drop-off and vehicle maneuvers to park and stop to drop off students along curbs on public streets. Therefore, the modified drop-off area and parking lot would improve traffic conditions on Franklin Avenue. It is recommended that the parking be prohibited for a length of approximately 20 feet north and south along the curbside area immediately adjacent to the egress driveways. The prohibition should be placed at a minimum during the student drop-off and pick-up periods to allow better traffic flow and improve line of sight.

To ensure proper circulation and efficient use of the new parking lot and drop-off/pick-up areas, the school should educate staff and visitors about the use of the new lot and provide monitors to assist students getting in and out of vehicles and to ensure that vehicles pull forward at the drop-off area.



FIGURES

Figure 1 - Local Vicinity

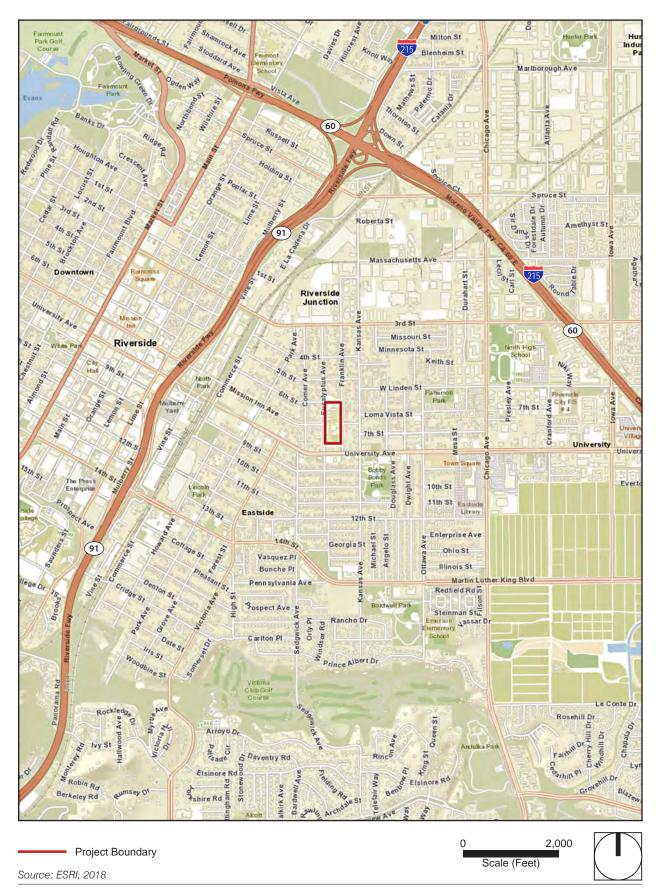


Figure 2 - Aerial Photograph

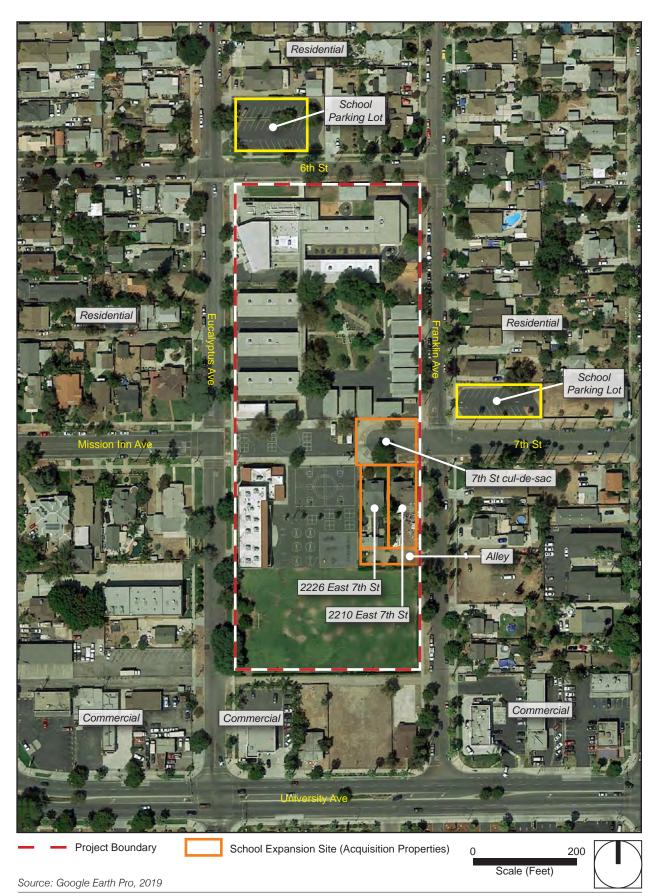
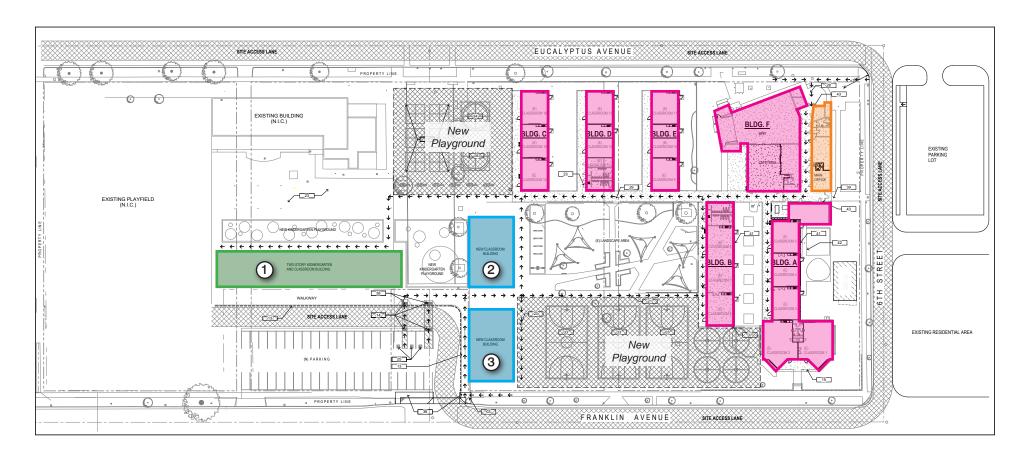


Figure 3 - Site Plan





New 2-Story Classroom Building #1



Buildings to be Renovated



New 1-Story Classroom Building #2



Administration Building to be Converted to Parent Center/Classroom and Day Care

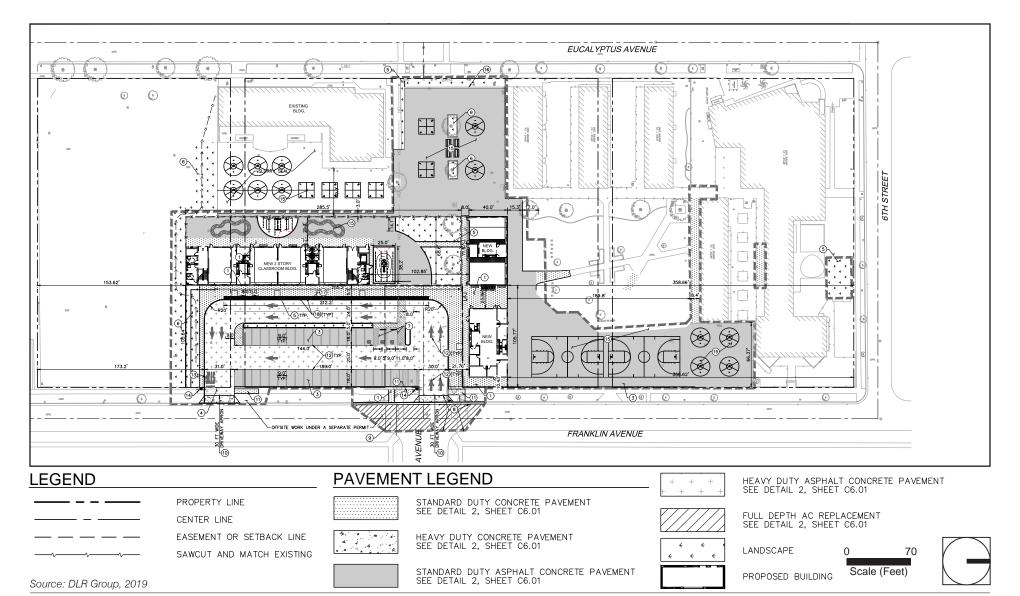


New 1-Story Administration Building #3





Figure 4 - Paving Plan





ATTACHMENT A: Traffic Counts

National Data & Surveying Services

Intersection Turning Movement Count

Location: Franklin Ave & 7th Street City: Riverside Control: 4-Way Stop Project ID: 18-06147-001 Date: 11/27/2018

| 2011.011 | , 5.0, | | | | | | | To | tal | | | | | | 11, 27, 2010 | | |
|-----------------|--------|------------|----------|-------|--------|----------|--------|-------|--------|--------|--------|-------|--------|--------|--------------|-------|-------|
| NS/EW Streets: | | Frankli | n Ave | | | Franklir | n Ave | | | 7th St | reet | | | 7th St | reet | | |
| | | NORTH | BOUND | | | SOUTH | BOUND | | | EASTB | OUND | | | WESTE | BOUND | | |
| AM | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | |
| /AIVI | NL | NT | NR | NU | SL | ST | SR | SU | EL | ET | ER | EU | WL | WT | WR | WU | TOTAL |
| 7:00 AM | 0 | 1 | 1 | 0 | 6 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 3 | 0 | 16 |
| 7:15 AM | 0 | 2 | 1 | 0 | 4 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 4 | 0 | 1 | 0 | 15 |
| 7:30 AM | 0 | 2 | 0 | 0 | 7 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 1 | 8 | 0 | 30 |
| 7:45 AM | 1 | 8 | 0 | 0 | 4 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 1 | 12 | 1 | 33 |
| 8:00 AM | 0 | 4 | 5 | 0 | 33 | 18 | 0 | 0 | 0 | 1 | 0 | 0 | 8 | 0 | 18 | 1 | 88 |
| 8:15 AM | 0 | 3 | 2 | 0 | 25 | 18 | 0 | 1 | 0 | 0 | 0 | 0 | 3 | 0 | 7 | 0 | 59 |
| 8:30 AM | 0 | 0 | 0 | 0 | 6 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 4 | 0 | 14 |
| 8:45 AM | 0 | 2 | 0 | 0 | 2 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 0 | 12 |
| | NL | NT | NR | NU | SL | ST | SR | SU | EL | ET | ER | EU | WL | WT | WR | WU | TOTAL |
| TOTAL VOLUMES: | 1 | 22 | 9 | 0 | 87 | 55 | 0 | 1 | 1 | 2 | 0 | 0 | 29 | 3 | 55 | 2 | 267 |
| APPROACH %'s: | 3.13% | 68.75% | 28.13% | 0.00% | 60.84% | 38.46% | 0.00% | 0.70% | 33.33% | 66.67% | 0.00% | 0.00% | 32.58% | 3.37% | 61.80% | 2.25% | |
| PEAK HR : | | 07:30 AM - | | | | | | | | | | | | | | | TOTAL |
| PEAK HR VOL: | 1 | 17 | 7 | 0 | 69 | 44 | 0 | 1 | 0 | 1 | 0 | 0 | 21 | 2 | 45 | 2 | 210 |
| PEAK HR FACTOR: | 0.250 | 0.531 | 0.350 | 0.000 | 0.523 | 0.611 | 0.000 | 0.250 | 0.000 | 0.250 | 0.000 | 0.000 | 0.656 | 0.500 | 0.625 | 0.500 | 0.597 |
| | | 0.6 | 94 | | | 0.55 | 59 | | | 0.2 | 50 | | | 0.6 | 48 | | 0.557 |
| | | NORTH | IROLIND. | | | SOUTH | BOLIND | | | EASTB | OUIND | | | WESTE | SOLIND | | 1 |
| PM | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | |
| I IVI | NL | ΝŢ | NR | NU | SL | ST | SR | SU | ĔĹ | ĒT | ER | EU | WL | wT | WR | WU | TOTAL |
| 2:00 PM | 0 | 2 | 4 | 0 | 7 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 1 | 7 | 0 | 28 |
| 2:15 PM | Õ | 4 | 2 | ŏ | 5 | 3 | 1 | Ö | ő | Õ | Ö | Õ | 4 | ō | 5 | Ö | 24 |
| 2:30 PM | i | 6 | 5 | ō | 8 | 6 | ō | ō | ō | i | 2 | ō | 4 | 2 | 13 | i | 49 |
| 2:45 PM | 0 | 1 | 4 | 0 | 16 | 10 | 0 | 0 | 0 | 2 | 0 | 0 | 5 | 0 | 10 | 0 | 48 |
| 3:00 PM | 1 | 1 | 3 | 0 | 9 | 7 | 1 | 0 | 1 | 0 | 0 | 0 | 2 | 0 | 8 | 0 | 33 |
| 3:15 PM | 0 | 2 | 1 | 0 | 7 | 3 | 0 | 1 | 0 | 0 | 0 | 0 | 3 | 0 | 6 | 0 | 23 |
| 3:30 PM | 0 | 1 | 2 | 0 | 8 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 5 | 0 | 22 |
| 3:45 PM | 0 | 4 | 5 | 0 | 2 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 6 | 0 | 2 | 0 | 23 |
| | NL | NT | NR | NU | SL | ST | SR | SU | EL | ET | ER | EU | WL | WT | WR | WU | TOTAL |
| TOTAL VOLUMES: | 2 | 21 | 26 | 0 | 62 | 35 | 2 | 1 | 1 | 5 | 2 | 0 | 33 | 3 | 56 | 1 | 250 |
| APPROACH %'s: | 4.08% | 42.86% | 53.06% | 0.00% | 62.00% | 35.00% | 2.00% | 1.00% | 12.50% | 62.50% | 25.00% | 0.00% | 35.48% | 3.23% | 60.22% | 1.08% | |
| PEAK HR : | | 02:15 PM - | 03:15 PM | | | | | | | | | | | | | | TOTAL |
| PEAK HR VOL: | 2 | 12 | 14 | 0 | 38 | 26 | 2 | 0 | 1 | 3 | 2 | 0 | 15 | 2 | 36 | 1 | 154 |
| PEAK HR FACTOR: | 0.500 | 0.500 | 0.700 | 0.000 | 0.594 | 0.650 | 0.500 | 0.000 | 0.250 | 0.375 | 0.250 | 0.000 | 0.750 | 0.250 | 0.692 | 0.250 | 0.786 |
| | | 0.5 | 02 | | | 0.63 | DE . | | | 0.50 | nn | | | 0.6 | 70 | | 0.700 |

National Data & Surveying Services

Intersection Turning Movement Count

Location: Franklin Ave & 7th Street City: Riverside Control: 4-Way Stop

Project ID: 18-06147-001 Date: 11/27/2018

| Control | 4-way St | υp | | | | | | | | | | | | Date: | 11/2//2010 | , | |
|-----------------|----------|----------|------------|-------|-------|-------|---------|-------|------------|-------|--------|-------|-------|-------|------------|-------|-------|
| | | | | | | | | Bil | kes | | | | | | | | _ |
| NS/EW Streets: | | Frank | lin Ave | | | Frank | lin Ave | | | 7th S | Street | | | 7th S | Street | | |
| | | NORTI | HBOUND | | | SOUTH | HBOUND | | | EAST | BOUND | | | WEST | BOUND | | |
| AM | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | |
| | NL | NT | NR | NU | SL | ST | SR | SU | EL | ET | ER | EU | WL | WT | WR | WU | TOTAL |
| 7:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 |
| 8:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | | |
| | NL | NT | NR | NU | SL | ST | SR | SU | EL | ET | ER | EU | WL | WT | WR | WU | TOTAL |
| TOTAL VOLUMES: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 |
| APPROACH %'s: | | | | | | | | | | | | | 0.00% | 0.00% | 100.00% | 0.00% | |
| PEAK HR: | | 07:30 AM | - 08:30 AM | | | | | · | | · | | · | | | | | TOTAL |
| PEAK HR VOL: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 |
| PEAK HR FACTOR: | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.250 | 0.000 | 0.250 |
| | | | | | | | | | | | | | | | | | |

| PM | TOTAL 0 1 0 0 2 |
|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------|
| NL NT NR NU SL ST SR SU EL ET ER EU WL WT WR WU | 0 1 0 |
| 2:15 PM 0 | 1 0 |
| 2:30 PM | 1 0 0 2 0 |
| 2:45 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 </th <th>0 0 2 0</th> | 0 0 2 0 |
| 3:00 PM | 0 2 0 |
| 3:15 PM 0 0 0 0 0 0 0 0 0 | 2 |
| 3:30 PM 0 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | 0 |
| 3:45 PM 0 0 0 0 0 0 0 0 0 0 0 0 1 0 0 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | |
| NL NT NR NU SL ST SR SU EL ET ER EU WL WT WR WU | 2 |
| | 1 |
| | |
| | TOTAL |
| TOTAL VOLUMES: 0 | 6 |
| APPROACH %'s: 100.00% 0.00% 0.00% 0.00% 100.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% | |
| PEAK HR: 02:15 PM - 03:15 PM | TOTAL |
| PEAK HR VOL: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 <t< th=""><th>3</th></t<> | 3 |
| PEAK HR FACTOR: 0.00 0.000 0.000 0.000 0.250 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.250 0.000 0.000 | 0.375 |
| 0.250 0.250 | |

National Data & Surveying Services

Intersection Turning Movement Count City: Riverside Turning Movement Count Date: 11/27/2018

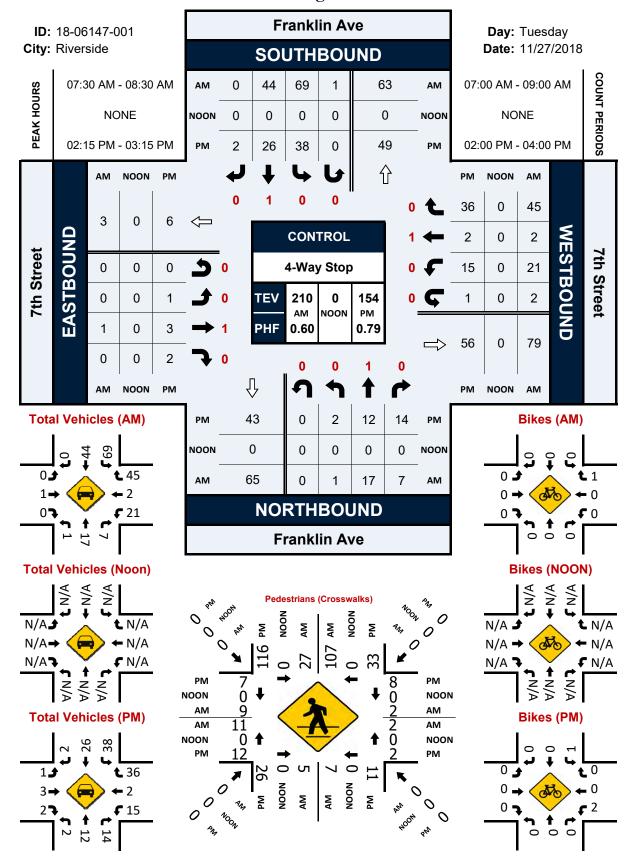
Pedestrians (Crosswalks)

| NS/EW Streets: | | | Frankl | in Ave | 7th 9 | Street | 7th S | | |
|----------------------------------------------------------------|----------------------------------|-------------------------------|-----------------------|-----------------------|-------------------|-----------------------|----------------------------|----------------------------|--------------------------------|
| AM | NORT EB | H LEG WB | SOUT EB | H LEG WB | EAST NB | r LEG SB | WEST NB | Γ LEG SB | TOTAL |
| 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM | 0 0 1 0 22 | 1 1 6 26 64 11 | 0 0 0 0 0 | 0 0 0 0 2 | 0 1 0 2 | 1 0 0 0 2 | 0 0 0 0 2 8 | 1 0 1 0 4 4 | 3 2 8 32 107 23 |
| 8:30 AM 8:45 AM | 2 | 0 | 0 | 0 | 0 | 0 | 0 0 | 0 1 | 2 2 |
| TOTAL VOLUMES : APPROACH %'s : | EB 30 21.58% | WB 109 78.42% | EB 5 41.67% | WB 7 58.33% | NB 3 50.00% | SB 3 50.00% | NB 11 50.00% | SB 11 50.00% | TOTAL 179 |
| PEAK HR : PEAK HR VOL : PEAK HR FACTOR : | 07:30 AM - 27 0.307 0.3 | 107 0.418 | 5 0.417 0.4 | 7 0.350 129 | 2 0.250 0.! | 2 0.250 500 | 11 0.344 0.4 | 9 0.563 117 | TOTAL 170 0.397 |

| PM | NORTH LEG | | SOUT | H LEG | EAS | Γ LEG | WEST | Γ LEG | |
|-----------------|-----------|------------|--------|--------|--------|----------|--------|--------|-------|
| PIVI | EB | WB | EB | WB | NB | SB | NB | SB | TOTAL |
| 2:00 PM | 2 | 1 | 0 | 1 | 0 | 1 | 0 | 0 | 5 |
| 2:15 PM | 0 | 4 | 0 | 3 | 0 | 1 | 0 | 0 | 8 |
| 2:30 PM | 79 | 26 | 17 | 8 | 2 | 0 | 9 | 5 | 146 |
| 2:45 PM | 32 | 2 | 9 | 0 | 0 | 6 | 1 | 0 | 50 |
| 3:00 PM | 5 | 1 | 0 | 0 | 0 | 1 | 2 | 2 | 11 |
| 3:15 PM | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 |
| 3:30 PM | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| 3:45 PM | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |
| | | | | | | | | | |
| | EB | WB | EB | WB | NB | SB | NB | SB | TOTAL |
| TOTAL VOLUMES : | 125 | 34 | 26 | 12 | 2 | 9 | 12 | 7 | 227 |
| APPROACH %'s: | 78.62% | 21.38% | 68.42% | 31.58% | 18.18% | 81.82% | 63.16% | 36.84% | |
| PEAK HR: | 02:15 PM | - 03:15 PM | | | | <u> </u> | | | TOTAL |
| PEAK HR VOL : | 116 | 33 | 26 | 11 | 2 | 8 | 12 | 7 | 215 |
| PEAK HR FACTOR: | 0.367 | 0.317 | 0.382 | 0.344 | 0.250 | 0.333 | 0.333 | 0.350 | 0.200 |
| | 0.3 | 355 | 0.3 | 370 | 0.4 | 417 | 0.3 | 39 | 0.368 |

Franklin Ave & 7th Street

Peak Hour Turning Movement Count



Prepared by NDS/ATD

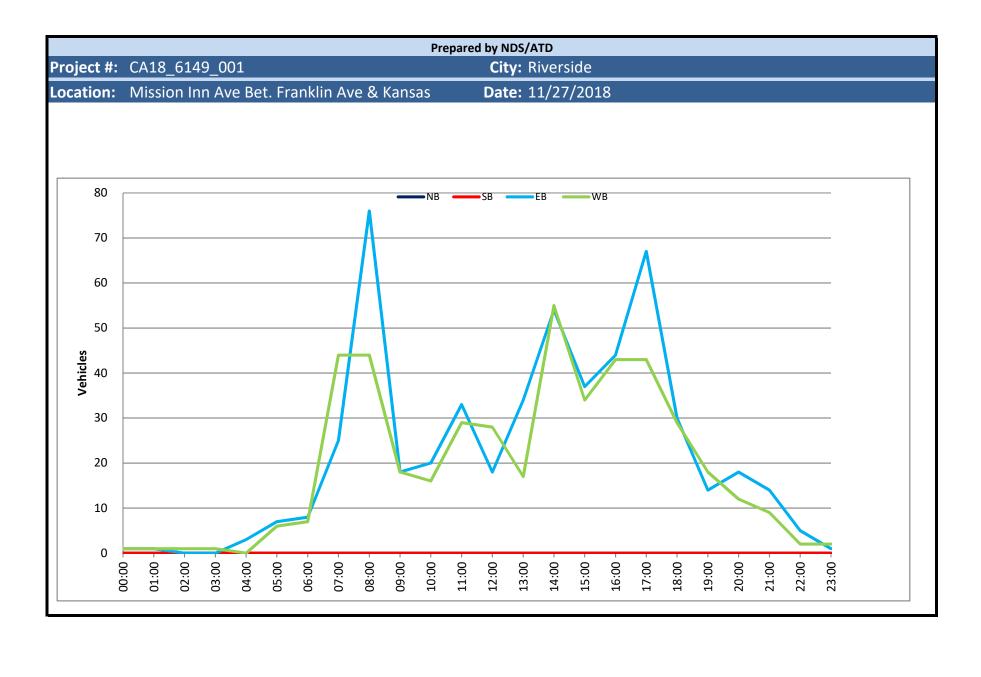
VOLUME

Mission Inn Ave Bet. Franklin Ave & Kansas Ave

Day: Tuesday **Date:** 11/27/2018

City: Riverside
Project #: CA18_6149_001

| | DAILY TOTALS | c | | NB | | SB | | EB | | WB | | | | | | To | tal |
|---------------------------------|--------------|---------------|--------------|---------------|-------------|---------------|--------------|---------------------------------|----|-------|----|---------------|--------------|---------------|-------------|---------------|--------------|
| | DAILT TOTAL | , | | 0 | | 0 | | 528 | | 460 | | | | | | 98 | 88 |
| AM Period | NB SB | ЕВ | | WB | | _ | TAL | PM Period | NB | | SB | ЕВ | | WB | | | TAL |
| 00:00 00:15 | | 0 0 | | 0 1 | | 0 1 | | 12:00 12:15 | | | | 8 3 | | 13 8 | | 21 11 | |
| 00:30 | | 1 | | 0 | | 1 | | 12:30 | | | | 1 | | 4 | | 5 | |
| 00:45 01:00 | | 0 | 1 | 0 | 1 | 0 | 2 | 12:45 13:00 | | | | <u>6</u> | 18 | <u>3</u> 5 | 28 | 9 | 46 |
| 01:00 | | 1 | | 1 | | 2 | | 13:15 | | | | 11 | | 2 | | 13 | |
| 01:30 | | 0 | _ | 0 | | 0 | • | 13:30 | | | | 12 | | 5 | | 17 | |
| 01:45 02:00 | | 0 | 1 | 0 | 1 | 0 | 2 | 13:45 14:00 | | | | 5 12 | 34 | 5 12 | 17 | 10 24 | 51 |
| 02:15 | | 0 | | 0 | | 0 | | 14:15 | | | | 7 | | 9 | | 16 | |
| 02:30 02:45 | | 0 0 | | 0 1 | 1 | 0 1 | 1 | 14:30 14:45 | | | | 14 21 | 54 | 20 14 | 55 | 34 35 | 109 |
| 03:00 | | 0 | | 0 | | 0 | | 15:00 | | | | 11 | <u> </u> | 10 | 33 | 21 | 103 |
| 03:15 | | 0 | | 1 | | 1 | | 15:15 | | | | 8 | | 9 | | 17 | |
| 03:30 03:45 | | 0 0 | | 0 | 1 | 0 | 1 | 15:30 15:45 | | | | 9 9 | 37 | 7 8 | 34 | 16 17 | 71 |
| 04:00 | | 0 | | 0 | _ | 0 | _ | 16:00 | | | | 13 | | 12 | | 25 | |
| 04:15 04:30 | | 1 1 | | 0 0 | | 1 1 | | 16:15 16:30 | | | | 13 13 | | 9 14 | | 22 27 | |
| 04:45 | | 1 | 3 | 0 | | 1 | 3 | 16:45 | | | | 5 | 44 | 8 | 43 | 13 | 87 |
| 05:00 | | 0 | | 0 | | 0 | | 17:00 | | | | 12 | | 12 | | 24 | |
| 05:15 05:30 | | 0 4 | | 2 1 | | 2 5 | | 17:15 17:30 | | | | 11 28 | | 11 13 | | 22 41 | |
| 05:45 | | 3 | 7 | 3 | 6 | 6 | 13 | 17:45 | | | | 16 | 67 | 7 | 43 | 23 | 110 |
| 06:00 | | 0 | | 0 | | 0 | | 18:00 18:15 | | | | 11 | | 8 | | 19 | |
| 06:15 06:30 | | 2 1 | | 1 1 | | 3 | | 18:30 | | | | 5 7 | | 4 5 | | 9 12 | |
| 06:45 | | 5 | 8 | 5 | 7 | 10 | 15 | 18:45 | | | | 7 | 30 | 12 | 29 | 19 | 59 |
| 07:00 07:15 | | 7 6 | | 5 6 | | 12 12 | | 19:00 19:15 | | | | 6 3 | | 1 6 | | 7 9 | |
| 07:30 | | 8 | | 15 | | 23 | | 19:30 | | | | 4 | | 5 | | 9 | |
| 07:45 | | 4 | 25 | 18 | 44 | 22 | 69 | 19:45 | | | | 1 | 14 | 6 | 18 | 7 | 32 |
| 08:00 08:15 | | 39 28 | | 26 10 | | 65 38 | | 20:00 20:15 | | | | 4 5 | | 4 4 | | 8 9 | |
| 08:30 | | 7 | | 5 | | 12 | | 20:30 | | | | 5 | | 2 | | 7 | |
| 08:45 09:00 | | <u>2</u> 6 | 76 | <u>3</u> 5 | 44 | 5 11 | 120 | 20:45 21:00 | | | | <u>4</u> 5 | 18 | <u>2</u> 4 | 12 | <u>6</u> 9 | 30 |
| 09:15 | | 6 | | 6 | | 12 | | 21:15 | | | | 5 | | 1 | | 6 | |
| 09:30 | | 2 | 4.0 | 5 | 4.0 | 7 | 2.0 | 21:30 | | | | 1 | | 2 | | 3 | |
| 09:45 10:00 | | 4 | 18 | 3 | 18 | <u>6</u> 7 | 36 | 21:45 22:00 | | | | 3 1 | 14 | 0 | 9 | 5 1 | 23 |
| 10:15 | | 2 | | 4 | | 6 | | 22:15 | | | | 2 | | 1 | | 3 | |
| 10:30 | | 7 7 | 20 | 8 | 16 | 15 | 26 | 22:30 22:45 | | | | 2 0 | _ | 1 0 | 2 | 3 0 | 7 |
| 10:45 11:00 | | 10 | 20 | 6 | 16 | 8 16 | 36 | 23:00 | | | | 0 | 5 | 1 | 2 | 1 | , |
| 11:15 | | 6 | | 7 | | 13 | | 23:15 | | | | 1 | | 0 | | 1 | |
| 11:30 11:45 | | 5 12 | 33 | 9 7 | 29 | 14 19 | 62 | 23:30 23:45 | | | | 0 0 | 1 | 0 1 | 2 | 0 1 | 3 |
| TOTALS | | 12 | 192 | | 168 | | 360 | TOTALS | | | | | 336 | | 292 | | 628 |
| SPLIT % | | | 53.3% | | 46.7% | | 36.4% | SPLIT % | | | | | 53.5% | | 46.5% | | 63.6% |
| | | | | NB | | SB | | EB | | WB | | | | | | To | tal |
| | DAILY TOTALS | S | | 0 | | 0 | | 528 | | 460 | | | | | | | 88 |
| AM Peak Hour | | | 07:30 | | 07:30 | | 07:30 | PM Peak Hour | | | | | 17:00 | | 14:00 | | 17:00 |
| AM Pk Volume | | | 79 | | 69 | | 148 | PM Pk Volume | | | | | 67 | | 55 | | 110 |
| Pk Hr Factor | | | 0.506 | | 0.663 | | 0.569 | Pk Hr Factor | | | | | 0.598 | | 0.688 | | 0.671 |
| 7 - 9 Volume 7 - 9 Peak Hour | | | 101 07:30 | | 88 07:30 | | 189 07:30 | 4 - 6 Volume 4 - 6 Peak Hour | | | | | 111 17:00 | | 86 16:30 | | 197 17:00 |
| 7 - 9 Pk Volume | | | 79 | | 69 | | 148 | 4 - 6 Pk Volume | | | | | 67 | | 45 | | 110 |
| Pk Hr Factor | 0.000 | 0.000 | 0.506 | | 0.663 | | 0.569 | Pk Hr Factor | | 0.000 | C | 0.000 | 0.598 | | 0.804 | | 0.671 |
| | | | | | | | | | | | | | | | | | |



Prepared by NDS/ATD

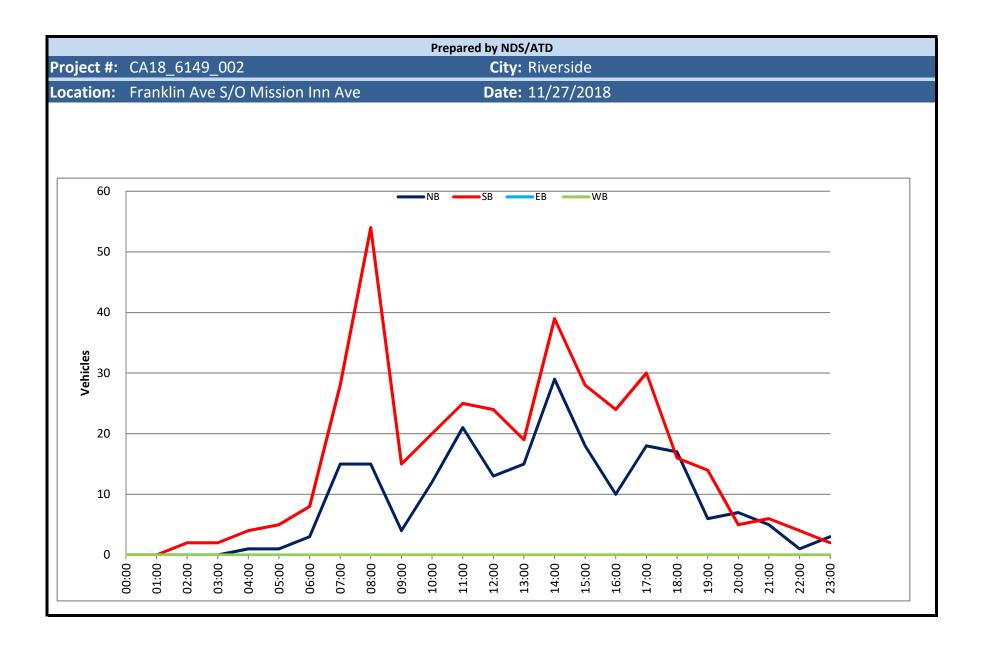
VOLUME

Franklin Ave S/O Mission Inn Ave

Day: Tuesday **Date:** 11/27/2018

City: Riverside
Project #: CA18_6149_002

| | DAII | V TO | TALS | | NB | SB | | EB | | WB | | | | | То | otal |
|---------------------------------|----------|-----------|-------------|-------|-------|----------|-------------|---------------------------------|---------------|-------------|----------|----------|-------|-------|----------|-------------|
| | DAIL | .1 10 | TALS | | 214 | 374 | | 0 | | 0 | | | | | 58 | 88 |
| AM Period | NB | | SB | EB | WB | ТО | TAL | PM Period | NB | | SB | EB | | WB | TO | TAL |
| 00:00 | 0 | | 0 | | | 0 | | 12:00 | 3 | | 7 | | | | 10 | |
| 00:15 | 0 | | 0 | | | 0 | | 12:15 12:30 | 3 | | 4 | | | | 7 | |
| 00:30 00:45 | 0 0 | | 0 | | | 0 | | 12:30 | 4 3 | 13 | 5 8 2 | 4 | | | 9 11 | 37 |
| 01:00 | 0 | | 0 | | | 0 | | 13:00 | 3 | 13 | 2 | · | | | 5 | |
| 01:15 | 0 | | 0 | | | 0 | | 13:15 | 3 | | 5 | | | | 8 | |
| 01:30 | 0 0 | | 0 | | | 0 | | 13:30 | 5 | 15 | 8 | 0 | | | 13 8 | 24 |
| 01:45 02:00 | 0 | | 1 | | | 1 | | 13:45 14:00 | <u>4</u> 5 | 15 | 4 1 6 | 9 | | | 11 | 34 |
| 02:15 | Ö | | 0 | | | 0 | | 14:15 | 7 | | 7 | | | | 14 | |
| 02:30 | 0 | | 1 | | | 1 | | 14:30 | 12 | | 9 | | | | 21 | |
| 02:45 | 0 | | 0 2 | | | 0 | 2 | 14:45 | 5 | 29 | 17 3 | 9 | | | 22 | 68 |
| 03:00 03:15 | 0 0 | | 0 | | | 0 | | 15:00 15:15 | 5 3 | | 9 4 | | | | 14 7 | |
| 03:30 | 0 | | 1 | | | 1 | | 15:30 | 2 | | 7 | | | | 9 | |
| 03:45 | 0 | | 1 2 | | | 1 | 2 | 15:45 | 8 | 18 | 8 2 | 8 | | | 16 | 46 |
| 04:00 | 0 | | 0 | | | 0 | | 16:00 | 3 | | 5 | | | | 8 | |
| 04:15 04:30 | 0 1 | | 0 | | | 0 | | 16:15 16:30 | 2 4 | | 6 10 | | | | 8 14 | |
| 04:45 | | | 1 4 | | | 1 | 5 | 16:45 | 1 | 10 | 3 2 | 4 | | | 4 | 34 |
| 05:00 | 0 | | 1 | | | 1 | | 17:00 | 2 | | 6 | | | | 8 | |
| 05:15 | 1 | | 0 | | | 1 | | 17:15 | 5 | | 9 | | | | 14 | |
| 05:30 05:45 | 0 | | 4 0 5 | | | 4 | 6 | 17:30 17:45 | 7 4 | 18 | 7 8 3 | 0 | | | 14 12 | 48 |
| 06:00 | 0 . | | 3 | | | 3 | U | 18:00 | 8 | 10 | 5 | 0 | | | 13 | 40 |
| 06:15 | 0 | | 1 | | | 1 | | 18:15 | 4 | | 3 | | | | 7 | |
| 06:30 | 1 | | 0 | | | 1 | | 18:30 | 2 | | 0 | | | | 2 | |
| 06:45 | | | 4 8 | | | 6 | 11 | 18:45 19:00 | <u>3</u> | 17 | 8 1 3 | 6 | | | 11 4 | 33 |
| 07:00 07:15 | 2 | | 4 | | | 7 | | 19:15 | 3 | | 2 | | | | 5 | |
| 07:30 | 2 | | 12 | | | 14 | | 19:30 | 2 | | 6 | | | | 8 | |
| 07:45 | | | 8 28 | | | 16 | 43 | 19:45 | 0 | 6 | 3 1 | 4 | | | 3 | 20 |
| 08:00 | 9 | | 25 | | | 34 | | 20:00 | 1 | | 1 | | | | 2 | |
| 08:15 08:30 | 4 0 | | 20 4 | | | 24 4 | | 20:15 20:30 | 3 1 | | 0 1 | | | | 3 2 | |
| 08:45 | | | 5 54 | | | 7 | 69 | 20:45 | 2 | 7 | 3 5 | <u>;</u> | | | 5 | 12 |
| 09:00 | 1 | | 5 | | | 6 | | 21:00 | 3 | | 1 | | | | 4 | |
| 09:15 | 0 | | 2 | | | 2 | | 21:15 | 0 | | 2 | | | | 2 | |
| 09:30 09:45 | 2 1 | | 3 5 15 | | | 5 6 | 19 | 21:30 21:45 | 1 1 | 5 | 2 1 6 | : | | | 3 2 | 11 |
| 10:00 | 0 | | 9 | | | 9 | 19 | 22:00 | 0 | <u> </u> | 1 | , | | | 1 | - 11 |
| 10:15 | 5 | | 1 | | | 6 | | 22:15 | 0 | | 0 | | | | 0 | |
| 10:30 | 3 | | 3 | | | 6 | | 22:30 | 0 | | 1 | | | | 1 | |
| 10:45 11:00 | 4 1 6 | | 7 20 8 | | | 11 14 | 32 | 22:45 23:00 | 0 | 1 | 2 2 | | | | 3 | 5 |
| 11:00 | 6 | | 8 3 | | | 9 | | 23:15 | 2 | | 0 | | | | 2 | |
| 11:30 | 7 | | 5 | | | 12 | | 23:30 | 0 | | 0 | | | | 0 | |
| 11:45 | 2 2 | 1 | 9 25 | | | 11 | 46 | 23:45 | 1 | 3 | 0 2 | <u> </u> | | | 1 | 5 |
| TOTALS | 7 | '2 | 163 | | | | 235 | TOTALS | | 142 | 21 | .1 | | | | 353 |
| SPLIT % | 30 | .6% | 69.4% | | | | 40.0% | SPLIT % | | 40.2% | 59. | 8% | | | | 60.0% |
| | DAH | V Te | TALC | | NB | SB | | EB | | WB | | | | | To | otal |
| | DAIL | .7 10 | TALS | | 214 | 374 | | 0 | | 0 | | | | | | 88 |
| AM Peak Hour | 07 | 7:30 | 07:30 | | | | 07:30 | PM Peak Hour | | 14:00 | 14 | :15 | | | | 14:15 |
| AM Pk Volume | 2 | !3 | 65 | | | | 88 | PM Pk Volume | | 29 | 4 | 2 | | | | 71 |
| Pk Hr Factor | | 639 | 0.650 | | | | 0.647 | Pk Hr Factor | | 0.604 | | 518 | | | | 0.807 |
| 7 - 9 Volume | | 30 | 82 | | | | 112 | 4 - 6 Volume | | 28 | 5 | | | | | 82 |
| 7 - 9 Peak Hour | | 7:30 | 07:30 | | | | 07:30 | 4 - 6 Peak Hour | | 17:00 | | :00 | | | | 17:00 |
| 7 - 9 Pk Volume Pk Hr Factor | | 23 639 | 65 0.650 | | | | 88 0.647 | 4 - 6 Pk Volume Pk Hr Factor | | 18 0.643 | | 0 333 | | | | 48 0.857 |
| FR III FACLUF | 0.0 | 033 | 0.650 | 0.000 | 0.000 | | 0.047 | - K III Factor | | 0.043 | 0.8 | | 0.000 | 0.000 | | 0.037 |





ATTACHMENT B: LOS Worksheets







Intersection Level Of Service Report Intersection 1: Franklin Avenue at Seventh Street

Control Type:All-way stopDelay (sec / veh):8.2Analysis Method:HCM 6th EditionLevel Of Service:AAnalysis Period:15 minutesVolume to Capacity (v/c):0.230

Intersection Setup

| Name | | | | | | | | | | | | |
|------------------------|--------|------------|--------|--------|------------|--------|--------|-----------|--------|--------|-----------|--------|
| Approach | ١ | Northbound | | | Southbound | | | Eastbound | I | V | Vestbound | t |
| Lane Configuration | | + | | | + | | | + | | + | | |
| Turning Movement | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| Speed [mph] | | 30.00 | | | 30.00 | | | 30.00 | | 30.00 | | |
| Grade [%] | 0.00 | | | | 0.00 | | | 0.00 | | 0.00 | | |
| Crosswalk | Yes | | | Yes | | | | Yes | | Yes | | |

Volumes

| Name | | • | | | | • | | | | | | |
|-----------------------------------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| Base Volume Input [veh/h] | 1 | 17 | 7 | 70 | 44 | 0 | 0 | 1 | 0 | 23 | 2 | 45 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [%] | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 |
| Growth Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 1 | 17 | 7 | 70 | 44 | 0 | 0 | 1 | 0 | 23 | 2 | 45 |
| Peak Hour Factor | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 0 | 7 | 3 | 29 | 18 | 0 | 0 | 0 | 0 | 10 | 1 | 19 |
| Total Analysis Volume [veh/h] | 2 | 28 | 12 | 117 | 73 | 0 | 0 | 2 | 0 | 38 | 3 | 75 |
| Pedestrian Volume [ped/h] | | 0 | | | 0 | | | 0 | | | 0 | |





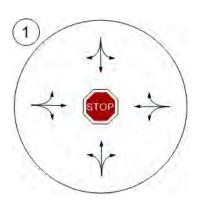
Intersection Settings

| intersection settings | | | | | | | | | |
|-------------------------------------------|------|-------|------|-------|--|--|--|--|--|
| Lanes | | | | | | | | | |
| Capacity per Entry Lane [veh/h] | 850 | 825 | 783 | 868 | | | | | |
| Degree of Utilization, x | 0.05 | 0.23 | 0.00 | 0.13 | | | | | |
| Movement, Approach, & Intersection Result | s | | | | | | | | |
| 95th-Percentile Queue Length [veh] | 0.16 | 0.89 | 0.01 | 0.46 | | | | | |
| 95th-Percentile Queue Length [ft] | 3.90 | 22.19 | 0.19 | 11.51 | | | | | |
| Approach Delay [s/veh] | 7.46 | 8.66 | 7.61 | 7.78 | | | | | |
| Approach LOS | Α | A | A | A | | | | | |
| Intersection Delay [s/veh] | 8.22 | | | | | | | | |
| Intersection LOS | | , | ٩ | | | | | | |



Lane Configuration and Traffic Control

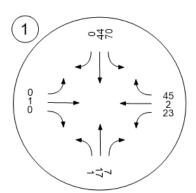






Traffic Volume - Future Total Volume







Intersection Level Of Service Report Intersection 1: Franklin Avenue at Seventh Street

Control Type:All-way stopDelay (sec / veh):7.4Analysis Method:HCM 6th EditionLevel Of Service:AAnalysis Period:15 minutesVolume to Capacity (v/c):0.098

Intersection Setup

| Name | | | | | | | | | | | | |
|------------------------|--------|------------|--------|--------|------------|--------|--------|-----------|--------|-----------|--------|--------|
| Approach | ١ | Northbound | | | Southbound | | l | Eastbound | | Westbound | | d |
| Lane Configuration | | + | | | + | | | + | | | + | |
| Turning Movement | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| Speed [mph] | | 30.00 | | | 30.00 | | 30.00 | | | 30.00 | | |
| Grade [%] | | 0.00 | | | 0.00 | | 0.00 | | | 0.00 | | |
| Crosswalk | | Yes | | | Yes | | Yes | | | Yes | | |

Volumes

| Name | | | | | | | | | | | | |
|-----------------------------------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| Base Volume Input [veh/h] | 2 | 12 | 14 | 38 | 26 | 2 | 1 | 3 | 2 | 16 | 2 | 36 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [%] | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 |
| Growth Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 2 | 12 | 14 | 38 | 26 | 2 | 1 | 3 | 2 | 16 | 2 | 36 |
| Peak Hour Factor | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 1 | 4 | 4 | 12 | 8 | 1 | 0 | 1 | 1 | 5 | 1 | 11 |
| Total Analysis Volume [veh/h] | 3 | 15 | 18 | 48 | 33 | 3 | 1 | 4 | 3 | 20 | 3 | 46 |
| Pedestrian Volume [ped/h] | | 0 | | | 0 | | | 0 | | | 0 | |



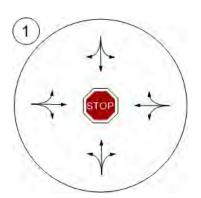
Intersection Settings

| microcolon octango | | | | | | | | | | |
|-----------------------------------------|------|------|------|------|--|--|--|--|--|--|
| Lanes | | | | | | | | | | |
| Capacity per Entry Lane [veh/h] | 925 | 854 | 887 | 933 | | | | | | |
| Degree of Utilization, x | 0.04 | 0.10 | 0.01 | 0.07 | | | | | | |
| Movement, Approach, & Intersection Resu | Its | | | | | | | | | |
| 95th-Percentile Queue Length [veh] | 0.12 | 0.33 | 0.03 | 0.24 | | | | | | |
| 95th-Percentile Queue Length [ft] | 3.03 | 8.16 | 0.68 | 5.98 | | | | | | |
| Approach Delay [s/veh] | 7.05 | 7.68 | 7.10 | 7.17 | | | | | | |
| Approach LOS | Α | А | A | A | | | | | | |
| Intersection Delay [s/veh] | 7.36 | | | | | | | | | |
| Intersection LOS | | | A | | | | | | | |



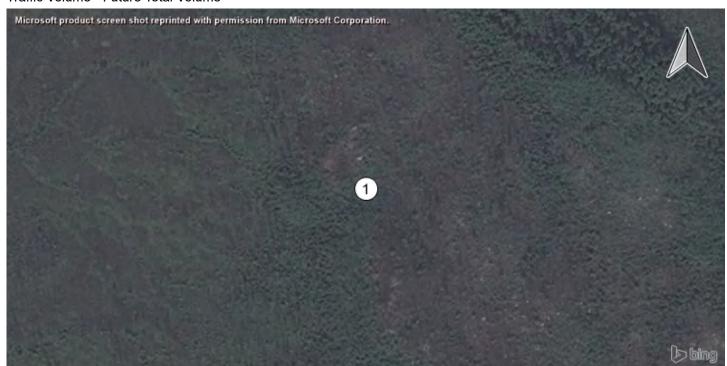
Lane Configuration and Traffic Control

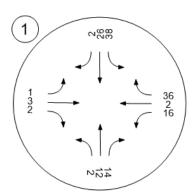






Traffic Volume - Future Total Volume







Intersection Level Of Service Report Intersection 1: Franklin Avenue at Seventh Street

Control Type:All-way stopDelay (sec / veh):8.2Analysis Method:HCM 6th EditionLevel Of Service:AAnalysis Period:15 minutesVolume to Capacity (v/c):0.261

Intersection Setup

| Name | | | | | | | | | | | | | |
|------------------------|--------|------------|--------|--------|----------|--------|--------|-----------|--------|-----------|--------|--------|--|
| Approach | ١ | Northbound | | | outhboun | d | I | Eastbound | ł | Westbound | | d | |
| Lane Configuration | | + | | | + | | | + | | | + | | |
| Turning Movement | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | |
| No. of Lanes in Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | |
| Speed [mph] | | 30.00 | | | 30.00 | | 30.00 | | | 30.00 | | | |
| Grade [%] | | 0.00 | | | 0.00 | | 0.00 | | | 0.00 | | | |
| Crosswalk | | Yes | | | Yes | | Yes | | | Yes | | | |

Volumes

| Name | | | | | | | | | | | | |
|-----------------------------------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| Base Volume Input [veh/h] | 25 | 49 | 61 | 1 | 0 | 113 | 2 | 23 | 45 | 0 | 0 | 0 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [%] | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 |
| Growth Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 25 | 49 | 61 | 1 | 0 | 113 | 2 | 23 | 45 | 0 | 0 | 0 |
| Peak Hour Factor | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 10 | 20 | 25 | 0 | 0 | 47 | 1 | 10 | 19 | 0 | 0 | 0 |
| Total Analysis Volume [veh/h] | 42 | 82 | 102 | 2 | 0 | 188 | 3 | 38 | 75 | 0 | 0 | 0 |
| Pedestrian Volume [ped/h] | | 0 | | | 0 | | | 0 | | | 0 | |



Intersection Settings

| L | а | n | e: | S |
|---|---|---|----|---|

| Capacity per Entry Lane [veh/h] | 866 | 936 | 817 | 728 |
|---------------------------------|------|------|------|------|
| Degree of Utilization, x | 0.26 | 0.20 | 0.14 | 0.00 |

Movement, Approach, & Intersection Results

| 95th-Percentile Queue Length [veh] | 1.05 | 0.76 | 0.49 | 0.00 |
|------------------------------------|-------|-------|-------|------|
| 95th-Percentile Queue Length [ft] | 26.14 | 18.96 | 12.34 | 0.00 |
| Approach Delay [s/veh] | 8.62 | 7.83 | 8.13 | 0.00 |
| Approach LOS | А | A | Α | A |
| Intersection Delay [s/veh] | | 8. | 23 | |
| Intersection LOS | | , | 4 | |



Intersection Level Of Service Report Intersection 3: Franklin @ Egress Driveway

Control Type:Two-way stopDelay (sec / veh):9.3Analysis Method:HCM 6th EditionLevel Of Service:AAnalysis Period:15 minutesVolume to Capacity (v/c):0.142

Intersection Setup

| Name | | | | | | |
|------------------------|--------|--------|--------|----------|--------|--------|
| Approach | North | nbound | South | bound | East | bound |
| Lane Configuration | • | 1 | 1 | → | ٦ | Γ |
| Turning Movement | Left | Thru | Thru | Right | Left | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Pocket | 0 | 0 | 0 | 0 | 0 | 0 |
| Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| Speed [mph] | 30 | 0.00 | 30 | 0.00 | 30.00 | |
| Grade [%] | 0 | 0.00 | | .00 | 0.00 | |
| Crosswalk | Y | ′es | Y | es es | Y | es es |

Volumes

| Name | | | | | | |
|-----------------------------------------|--------|--------|--------|--------|--------|--------|
| Base Volume Input [veh/h] | 0 | 25 | 0 | 0 | 110 | 51 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [%] | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 |
| Growth Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 0 | 25 | 0 | 0 | 110 | 51 |
| Peak Hour Factor | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 0 | 8 | 0 | 0 | 35 | 16 |
| Total Analysis Volume [veh/h] | 0 | 32 | 0 | 0 | 139 | 65 |
| Pedestrian Volume [ped/h] | (|) | (| 0 | (|) |



Intersection Settings

| Priority Scheme | Free | Free | Stop | |
|------------------------------------|------|------|------|--|
| Flared Lane | | | | |
| Storage Area [veh] | 0 | 0 | 0 | |
| Two-Stage Gap Acceptance | | | No | |
| Number of Storage Spaces in Median | 0 | 0 | 0 | |

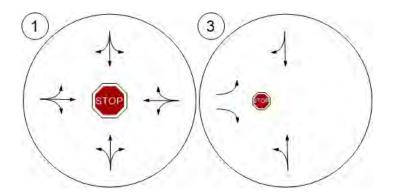
Movement, Approach, & Intersection Results

| V/C, Movement V/C Ratio | 0.00 | 0.00 | 0.00 | 0.00 | 0.14 | 0.06 | | |
|---------------------------------------|------|------|------|------|-------|------|--|--|
| d_M, Delay for Movement [s/veh] | 7.22 | 0.00 | 0.00 | 0.00 | 9.27 | 8.53 | | |
| Movement LOS | А | А | А | А | Α | А | | |
| 95th-Percentile Queue Length [veh/ln] | 0.00 | 0.00 | 0.00 | 0.00 | 0.49 | 0.19 | | |
| 95th-Percentile Queue Length [ft/ln] | 0.00 | 0.00 | 0.00 | 0.00 | 12.31 | 4.77 | | |
| d_A, Approach Delay [s/veh] | 0.00 | | 0.00 | | 9.03 | | | |
| Approach LOS | A | | А | | A | | | |
| d_I, Intersection Delay [s/veh] | 7.81 | | | | | | | |
| Intersection LOS | А | | | | | | | |



Lane Configuration and Traffic Control

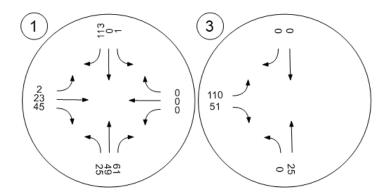






Traffic Volume - Future Total Volume







Intersection Level Of Service Report Intersection 1: Franklin Avenue at Seventh Street

Control Type:All-way stopDelay (sec / veh):7.3Analysis Method:HCM 6th EditionLevel Of Service:AAnalysis Period:15 minutesVolume to Capacity (v/c):0.143

Intersection Setup

| Name | | | | | | | | | | | | |
|------------------------|--------|------------|--------|------------|--------|-----------|--------|-----------|--------|--------|--------|--------|
| Approach | ١ | Northbound | | Southbound | | Eastbound | | Westbound | | d | | |
| Lane Configuration | + | | + | | + | | | + | | | | |
| Turning Movement | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| Speed [mph] | 30.00 | | | 30.00 | | 30.00 | | 30.00 | | | | |
| Grade [%] | 0.00 | | 0.00 | | 0.00 | | | 0.00 | | | | |
| Crosswalk | | Yes | | | Yes | | Yes | | Yes | | | |

Volumes

| Name | | | | | | | | | | | | |
|-----------------------------------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| Base Volume Input [veh/h] | 28 | 35 | 40 | 0 | 0 | 66 | 0 | 0 | 0 | 1 | 17 | 36 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [%] | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 |
| Growth Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 28 | 35 | 40 | 0 | 0 | 66 | 0 | 0 | 0 | 1 | 17 | 36 |
| Peak Hour Factor | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 9 | 11 | 13 | 0 | 0 | 21 | 0 | 0 | 0 | 0 | 5 | 11 |
| Total Analysis Volume [veh/h] | 35 | 44 | 51 | 0 | 0 | 84 | 0 | 0 | 0 | 1 | 22 | 46 |
| Pedestrian Volume [ped/h] | | 0 | | | 0 | | | 0 | | | 0 | |



Intersection Settings

| Intersection Settings | | | | | | | |
|--------------------------------------------|-------|------|------|------|--|--|--|
| Lanes | | | | | | | |
| Capacity per Entry Lane [veh/h] | 907 | 1000 | 813 | 908 | | | |
| Degree of Utilization, x | 0.14 | 0.08 | 0.00 | 0.08 | | | |
| Movement, Approach, & Intersection Results | s | | | | | | |
| 95th-Percentile Queue Length [veh] | 0.50 | 0.27 | 0.00 | 0.25 | | | |
| 95th-Percentile Queue Length [ft] | 12.49 | 6.86 | 0.00 | 6.15 | | | |
| Approach Delay [s/veh] | 7.63 | 6.93 | 0.00 | 7.29 | | | |
| Approach LOS | Α | A | A | A | | | |
| Intersection Delay [s/veh] | 7.34 | | | | | | |
| Intersection LOS | Δ | | | | | | |



Intersection Level Of Service Report Intersection 3: Franklin @ Egress Driveway

Control Type:Two-way stopDelay (sec / veh):9.1Analysis Method:HCM 6th EditionLevel Of Service:AAnalysis Period:15 minutesVolume to Capacity (v/c):0.080

Intersection Setup

| Crosswalk | Y | es | Yes | | Yes | |
|------------------------|----------|--------|------------|--------|-----------|--------|
| Grade [%] | 0.00 | | 0.00 | | 0.00 | |
| Speed [mph] | 30 | 30.00 | | 30.00 | | 0.00 |
| Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Pocket | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| Turning Movement | Left | Thru | Thru | Right | Left | Right |
| Lane Configuration | H | | | | יור | |
| Approach | North | bound | Southbound | | Eastbound | |
| Name | | | | | | |

Volumes

| Name | | | | | | |
|-----------------------------------------|--------|--------|--------|--------|--------|--------|
| Base Volume Input [veh/h] | 0 | 28 | 0 | 0 | 46 | 35 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [%] | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 | 2.00 |
| Growth Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 0 | 28 | 0 | 0 | 46 | 35 |
| Peak Hour Factor | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 | 0.6000 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 0 | 12 | 0 | 0 | 19 | 15 |
| Total Analysis Volume [veh/h] | 0 | 47 | 0 | 0 | 77 | 58 |
| Pedestrian Volume [ped/h] | 0 | | 0 | | 0 | |



Intersection Settings

| Priority Scheme | Free | Free | Stop |
|------------------------------------|------|------|------|
| Flared Lane | | | |
| Storage Area [veh] | 0 | 0 | 0 |
| Two-Stage Gap Acceptance | | | No |
| Number of Storage Spaces in Median | 0 | 0 | 0 |

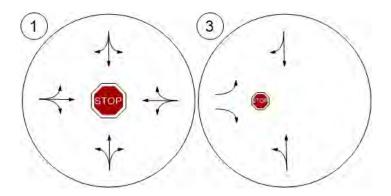
Movement, Approach, & Intersection Results

| V/C, Movement V/C Ratio | 0.00 | 0.00 | 0.00 | 0.00 | 0.08 | 0.05 | | |
|---------------------------------------|------|------|------|------|------|------|--|--|
| d_M, Delay for Movement [s/veh] | 7.22 | 0.00 | 0.00 | 0.00 | 9.06 | 8.51 | | |
| Movement LOS | А | А | А | А | А | А | | |
| 95th-Percentile Queue Length [veh/ln] | 0.00 | 0.00 | 0.00 | 0.00 | 0.26 | 0.17 | | |
| 95th-Percentile Queue Length [ft/ln] | 0.00 | 0.00 | 0.00 | 0.00 | 6.50 | 4.23 | | |
| d_A, Approach Delay [s/veh] | 0.00 | | 0.00 | | 8. | 82 | | |
| Approach LOS | , | 4 | А | | A | | | |
| d_I, Intersection Delay [s/veh] | 6.55 | | | | | | | |
| Intersection LOS | | A | | | | | | |



Lane Configuration and Traffic Control

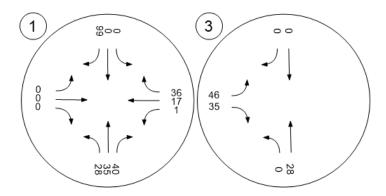






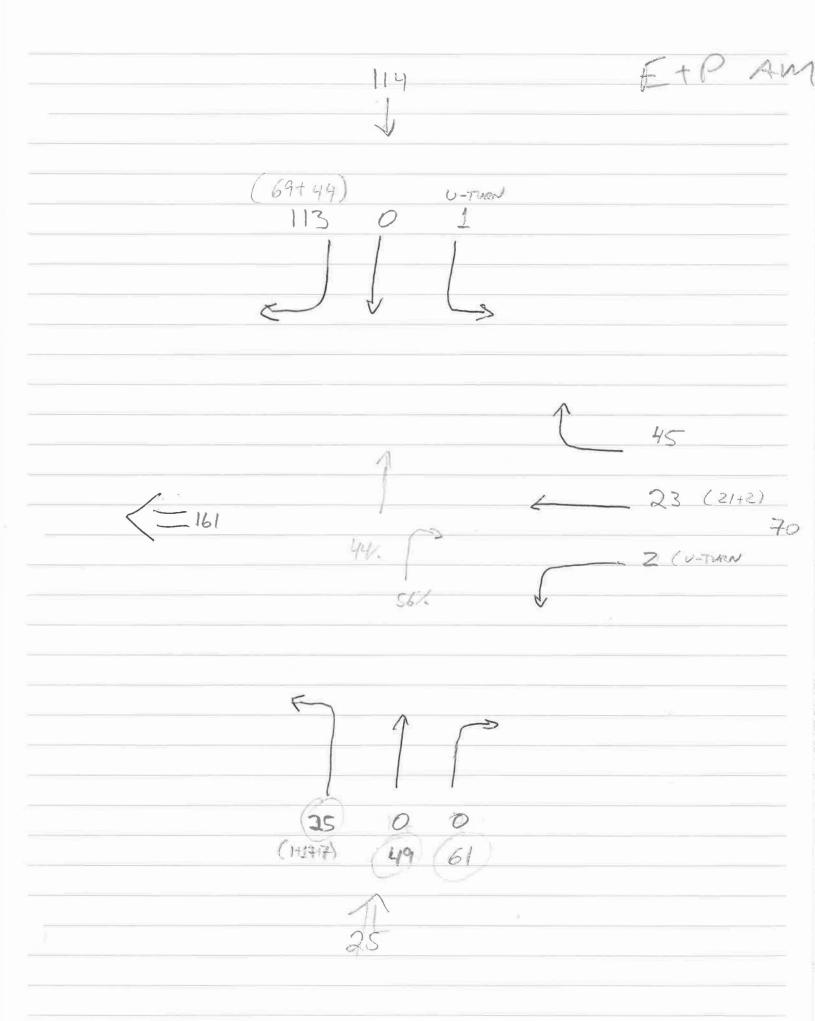
Traffic Volume - Future Total Volume

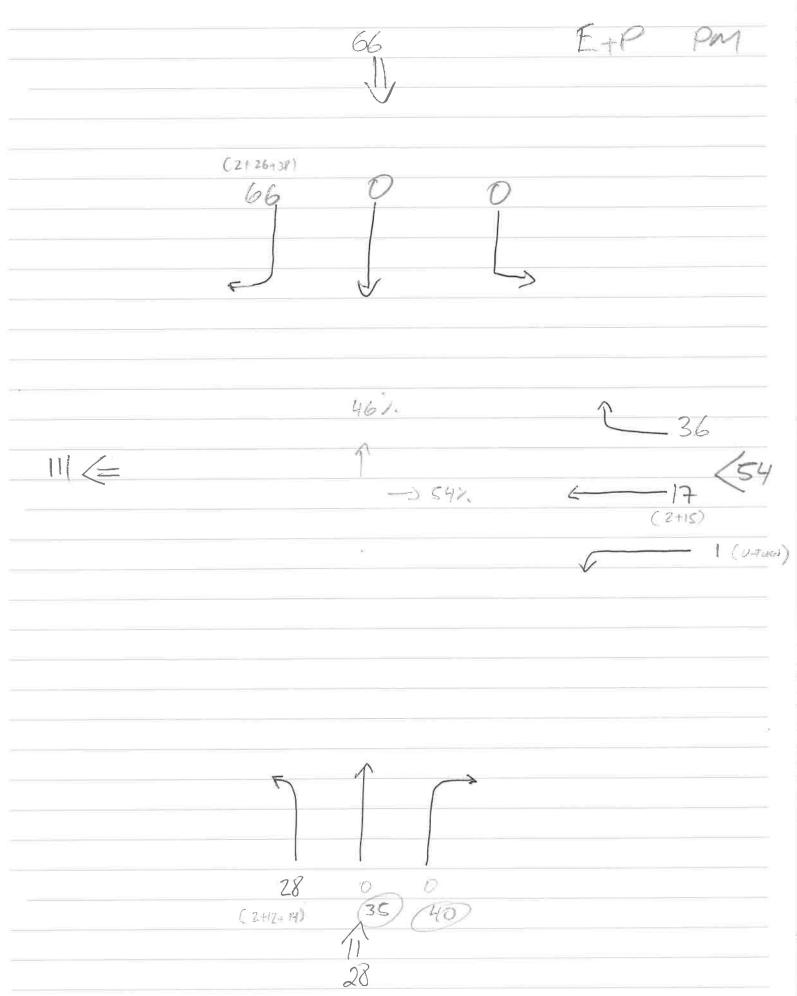






ATTACHMENT C: Project Traffic





Maker of the Genuine Swiss Army Knife



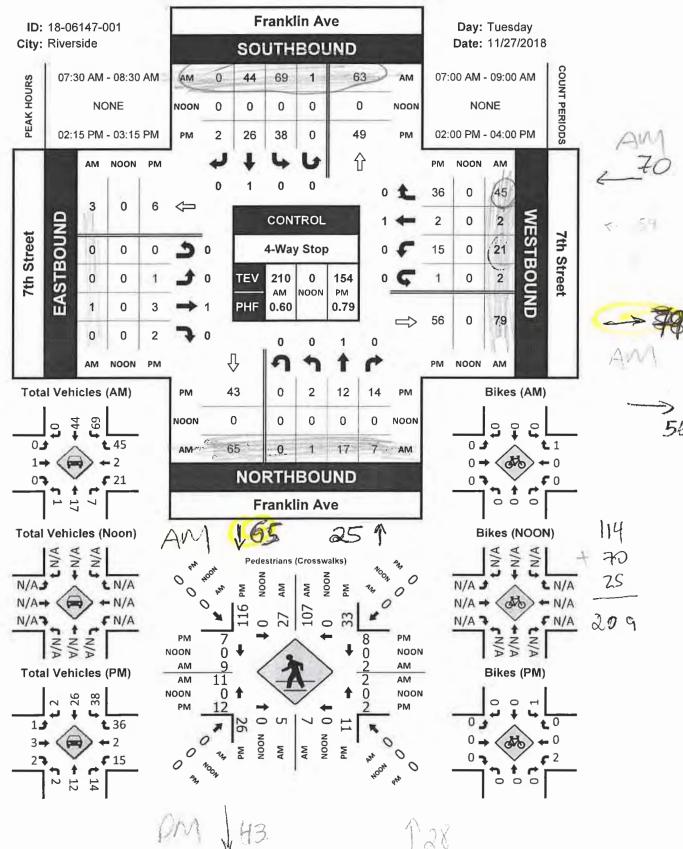


Prepared by National Data & Surveying Services

Franklin Ave & 7th Street

44

Peak Hour Turning Movement Count



SBN SBT + SBR + WOT + WBL + NBL+NBT+NBR= 167 69 + 44 + 0 + 2 + 21 + 1 + 17+7 +21 + 0 + AM= 69 +44 2 + 12+14 = 111 2 LONGFELLOW ELEMENTARY SCHOOL EXPANSION DRAFT EIR RIVERSIDE UNIFIED SCHOOL DISTRICT Figure 3-6 - Paving Plan 3. Project Description 9 T33ATS HT8 EUCAL YPTUS AVENUE FRANKLIN AVENUE STANDARD DUTY ASPHALT CONCRETE PAVEMENT STANDARD DUTY CONCRETE PAVEMENT HEAVY DUTY CONCRETE PAVEMENT PAVEMENT LEGEND HEAVY DUTY CENTER LINE

79

Source: DLR Group, 2018

LEGEND

PlaceWorks

Scale (Feet)

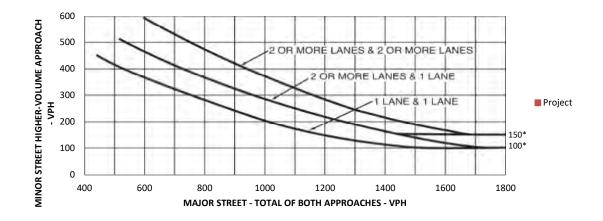


ATTACHMENT D: Signal Warrants

Traffic Conditions: Existing AM

| Major Street Name: Franklin Avenue | Total of Both Approaches (VPH) = | 140 |
|------------------------------------|----------------------------------|-----|
| | Number of Apporach Lanes = | 1 |

| Minor Street Name: Seventh Street | High Volume Approach (VPH) = | 70 |
|-----------------------------------|------------------------------|----|
| | Number of Apporach Lanes = | 1 |

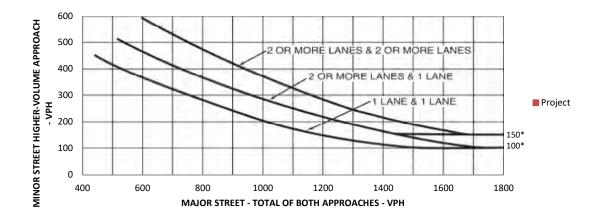


*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Traffic Conditions: Existing PM

| Major Street Name: Franklin Avenue | Total of Both Approaches (VPH) = | 94 |
|------------------------------------|----------------------------------|----|
| | Number of Apporach Lanes = | 1 |
| | | |

| Minor Street Name: Seventh Street | High Volume Approach (VPH) = | 54 |
|-----------------------------------|------------------------------|----|
| | Number of Apporach Lanes = | 1 |

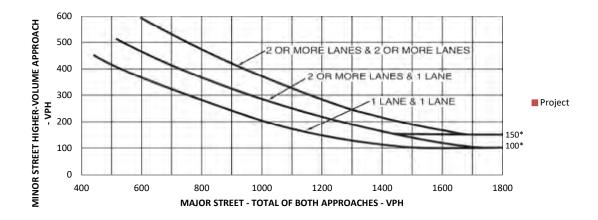


*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Traffic Conditions: Future AM

| Major Street Name: Franklin Avenue | Total of Both Approaches (VPH) = | 249 |
|------------------------------------|----------------------------------|-----|
| | Number of Apporach Lanes = | 1 |
| | | |

| Minor Street Name: Seventh Street | | High Volume Approach (VPH) = | |
|-----------------------------------|--|------------------------------|---|
| | | Number of Apporach Lanes = | 1 |

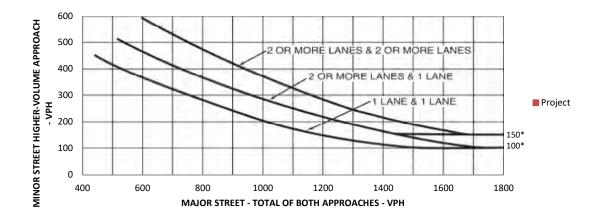


*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Traffic Conditions: Future PM

| Major Street Name: Franklin Avenue | Total of Both Approaches (VPH) = | 169 |
|------------------------------------|----------------------------------|-----|
| | Number of Apporach Lanes = | 1 |

| Minor Street Name: Seventh Street | High Volume Approach (VPH) = | 54 |
|-----------------------------------|------------------------------|----|
| | Number of Apporach Lanes = | 1 |



*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

| Appendices |
|------------|
|------------|

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