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Dist.-Co.-Rte.

P.M. / P.M.

E.A. / Project No.

State Route 1 (Pacific Coast Highway) Drainage Rehabilitation and Bridge Replacement at Solstice Canyon Creek | Project Addendum/Environmental Reevaluation to the Previously Approved Mitigated Negative Declaration (MND)/Finding of No Significant Impact (FONSI)

## **19 November 2022**

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# 2. ABSTRACT

The purpose of this Addendum/Reevaluation is to address design changes to the State Route 1 (Pacific Coast Highway) Drainage Rehabilitation and Bridge Replacement at Solstice Canyon Creek Project since the MND/FONSI was approved March 19, 2019 (SCH No. 2018111004). This Addendum/Reevaluation will discuss changes in the scope of work of the aforementioned project [addition of a pedestrian undercrossing structure at Project Location No. 10 (Solstice Canyon Creek)] and capture any updates to environmental commitments as a result of related environmental reevaluations.

## 3. REGULATORY MANDATE

This Environmental Addendum/Reevaluation will identify project changes and reexamine topical categories found in the previous Mitigated Negative Declaration/Finding of No Significant Impact (MND/FONSI) that are relevant to the addition of a pedestrian undercrossing at Project Location No. 10 (Solstice Canyon Creek). The relevant/identified topical categories analyzed and discussed in the Addendum/Reevaluation are Biology, Cultural/Archaeological Resources, Hazardous Waste, Geology, Hydraulics, and Noise, and this Environmental Addendum/Reevaluation will ascertain if there are any changes that may affect the environmental setting or any related regulatory changes.

Under NEPA, an Environmental Reevaluation (ER) is a reconsideration of the adequacy or validity of a Categorical Exclusion (CE) determination, a Finding of No Significant Impact (FONSI), or an Environmental Impact Statement (EIS). The purpose is to assess whether any factors would affect the validity of the CE determination or FONSI/EIS environmental document.



An addendum serves a similar purpose under CEQA. In accordance with Section 15164 of the CEQA guidelines, the Lead Agency or Responsible Agency must prepare an addendum to a previously certified Initial Study (IS) if some changes or additions are necessary.

An ER considers such factors as whether pursuant to 23 CFR 771.129:

- There have been changes in the project design or its surroundings and impacts
- Any new right-of-way issues have been identified
- There is new information in laws or regulations that apply to the project

## 4. EXISTING CONDITIONS

State Route 1 (SR-1), or Pacific Coast Highway (SR-1/PCH), is a major north-south state highway that runs along most of the California-Pacific coastline and originates at Interstate 5 (I-5) near Dana Point in Orange County, with the most northerly terminus at U.S. Highway 101 (US-101) near Leggett in Mendocino County. Through Los Angeles and Ventura Counties, SR-1/PCH serves the City of Long Beach on the south, and traverses the Los Angeles Harbor Region, South Bay Cities, Los Angeles International Airport, Venice/Santa Monica (intermittently as Lincoln Boulevard), and Pacific Palisades/Malibu as it approaches the Ventura County line and Point Mugu/Oxnard at the north.

Within project limits, the SR-1/PCH highway facility lays between the Pacific coastline and the Santa Monica Mountains, which are roughly 45 miles long and form an east-west range of low mountains along the coast from the City of Los Angeles to the Oxnard Plane. They are particularly characterized by long, south-draining canyons on their south flank, and north-draining canyons to U.S. Route 101 on their north flank. State Route 27 (SR-27/Topanga Canyon Boulevard), State Route 23 (SR-23), Malibu Canyon Road, and Kanan Dume Road are the main north-south passes through the Santa Monica Mountains between U.S. 101 and SR-1/PCH within project limits. The SR-1/PCH highway facility provides interregional, recreational, and local commuter service through a semi-urban, partly rural corridor, and consists of four lanes (two in each direction) within the proposed project limits. From Santa Monica, SR-1/PCH curves west through the Pacific Palisades neighborhood of Los Angeles before becoming Malibu's main thoroughfare to the Ventura County Line. The following table captures the scope of work for the proposed project as outlined in a previous Environmental Reevaluation/Revalidation approved August 21, 2021 under the same SCH Number for the reduction in the original scope of work (removal of proposed work at Project Locations No. 7-9 and 11/12).

Table 1. Proposed Project Scope of Work as of August 21, 2021

Project Location No.	Post Mile	Activity	
1	LA 37.67	Remove debris from corrugated steel drainage pipe, replace cured-in place pipe lining	
2	LA 39.08	Replace existing pipe with 24" RCP	
3	LA 40.16	Replace 36" CMP	
4	LA 40.18	Install culvert barrel lining (CIP) in upstream section of pipe, replace in-kind 24" RCP middle section of downstream pipe using Cut-and-Cover method, install culvert barrel lining downstream (CIP) section of pipe	
5	LA 40.23	Remove debris from corrugated steel drainage pipe, replace cured-in place pipe lining	
6	LA 40.24	Replace 36" RCP and 18" CMP sections	
10	LA 50.36	Replace bridge/culvert with new bridge with an underlying natural slope creek bottom	
13	LA 61.29	Replace 30" RCP	
14	LA 61.35	Replace 24" RCP	
15	LA 61.68	Replace 24" RCP	
16	LA 62.51	Replace 24" RCP on upstream section, joint seal manhole	
17	LA 62.55	Install culvert barrel lining (CIP), remove debris and clear manhole, and replace 18" CMP on downstream section	
18	VEN 0.67	Install culvert barrel lining (CIP), replace lid/grate for upstream drop inlet	
19	VEN 0.92	Install culvert barrel lining (CIP)	



Figure 1a Proposed Project Location and Vicinity





Figure 1b Locations of Construction | Western Segment – Locations No. 1-6



Location No.	Post Mile	Activity	
1	LA 37.67	Remove debris from corrugated steel drainage pipe, replace cured-in place pipe lining	
2	LA 39.08	Replace existing pipe with 24" RCP	
3	LA 40.16	teplace 36" CMP	
4	LA 40.18	Install culvert barrel lining (CIP) in upstream section of pipe, replace in-kind 24" RCP middle section of downstream pipe using Cut-and-Cover method, install culvert barrel lining	
		downstream (CIP) section of pipe	
5	LA 40.23	Remove debris from corrugated steel drainage pipe, replace cured-in place pipe lining	
6	LA 40.24	Replace 36" RCP and 18" CMP sections	

CMP = Corrugated Metal Pipe CSP = Corrugated Steel Pipe RCP = Reinforced Concrete Pipe CIP = Cured-In-Place pipe lining



Figure 1c Locations of Construction | Central Segment – Location No. 10



Location No.	Post Mile	Activity
10	LA 50.36	Replace bridge/culvert with new bridge with an underlying natural slope creek bottom



Figure 1d Locations of Construction | Eastern Segment – Locations No. 13-19



Location No.	Post Mile	Activity	
13	LA 61.29	Replace 30" RCP	
14	LA 61.35	Replace 24" RCP	
15	LA 61.68	Replace 24" RCP	
16	LA 62.51	Replace 24" RCP on upstream section, joint seal manhole	
17	LA 62.55	Install culvert barrel lining (CIP), remove debris and clear manhole, and replace 18" CMP on downstream section	
18	VEN 0.67	Install culvert barrel lining (CIP), replace lid/grate for upstream drop inlet	
19	VFN 0 92	Install culvert harrel lining (CIP)	

CMP = Corrugated Metal Pipe CSP = Corrugated Steel Pipe RCP = Reinforced Concrete Pipe CIP = Cured-In-Place pipe lining



#### 5. PREVIOUS ENVIRONMENTAL DOCUMENTATION

Initial Study/Environmental Assessment (October 6, 2018) with Mitigated Negative Declaration/Finding of No Significant Impact (March 25, 2019). The Initial Study/Environmental Assessment (IS/EA) for the State Route 1 (Pacific Coast Highway) Drainage Rehabilitation and Bridge Replacement at Solstice Canyon Creek project was approved by Caltrans on October 26, 2018, and the Mitigated Negative Declaration/Finding of No Significant Impact (MND/FONSI) was approved by Caltrans, as assigned by the Federal Highway Administration (FHWA) on March 25, 1019, SCH No. 2018111004. The environmental review, consultation, and any other action required in accordance with applicable Federal laws for this project is being, or has been, carried out by the State of California Department of Transportation under its assumption of responsibility pursuant to 23 U.S.C. 327.

Revalidation for Planned Relocation of Southern California Edison Utilities (August 24, 2021). Continued development of design at 60% PS&E presented minor changes in design at Project Location No. 10 (Solstice Canyon Creek as they pertain to the MND/FONSI for the State Route 1 (Pacific Coast Highway) Drainage Rehabilitation and Bridge Replacement at Solstice Canyon Creek project as approved March 25, 2019 (SCH No. 2018111004). Construction of the proposed bridge structure at Project Location No. 10 (Solstice Canyon Creek) was found to require relocation of Southern California Edison utilities, and the changes were captured in a revalidation approved August 24, 2021. Because the proposed utility relocation did not pose any potential impacts to environmentally sensitive areas and the improvements would occur completely within the prism of the roadway and previously studied area, no updates to environmental studies were warranted.

Revalidation for Reduction in Scope of Work (March 29, 2022). An additional revalidation was completed and approved on March 29, 2022, to capture a reduction in the scope of work (removal of improvements at Project Locations Nos. 7-9 and 11/12). Inclement storm events prompted urgent need of drainage rehabilitation at Project Locations Nos. 7-9 and 11/12 that were completed through Director's Order/Emergency response under separate project EAs, which ultimately prompted the removal of improvements at these locations from the proposed project scope of work. The total number of proposed project locations was thus reduced from nineteen (19) locations to fourteen (14), and the project locations removed from the scope of work are highlighted in the following table.

Table 2. Proposed Project Locations Removed from Scope of Work as of March 29, 2022

Location	Post	Activity	
<b>No.</b>	Mile LA 37.67	Remove debris from corrugated steel drainage pipe, replace cured-in place pipe lining	
2	LA 37.07 LA 39.08	Replace existing pipe with 24" RCP	
3	LA 40.16	Replace 36" CMP	
4	LA 40.18	Install culvert barrel lining (CIP) in upstream section of pipe, replace in-kind 24" RCP middle section of	
		downstream pipe using Cut-and-Cover method, install culvert barrel lining downstream (CIP) section of pipe	
5	LA 40.23	Remove debris from corrugated steel drainage pipe, replace cured-in place pipe lining	
6	LA 40.24	Replace 36" RCP and 18" CMP sections	
7	LA 50.05	Replace 18" RCP	
8	LA 50.08	Replace 24" CMP	
9	LA 50.28	Install culvert barrel lining (CIP), repair joint seals at headwall and pipe, regrade channel and remove debris	
		and vegetation at outlet	
10	LA 50.36	Replace bridge/culvert with new bridge with an underlying natural slope creek bottom	
11	LA 50.39	Remove debris from drainage pipe, replace cured-in place pipe lining	
12	LA 50.42	Install culvert barrel lining (CIP) in upstream section of existing pipe, replace 20" RCP on downstream end	
13	LA 61.29	Replace 30" RCP	
14	LA 61.35	Replace 24" RCP	
15	LA 61.68	Replace 24" RCP	
16	LA 62.51	Replace 24" RCP on upstream section, joint seal manhole	
17	LA 62.55	Install culvert barrel lining (CIP), remove debris and clear manhole, and replace 18" CMP on downstream	
		section	
18	VEN 0.67	Install culvert barrel lining (CIP), replace lid/grate for upstream drop inlet	
19	VEN 0.92	Install culvert barrel lining (CIP)	



Project location numbers were not changed to ensure consistency, and the current scope of work and revised list of fourteen (14) project locations is summarized in the following table.

Table 3. Revised Scope of Work/List of Project Locations as of March 29, 2022

Location No.	Post Mile	Activity	
1	LA 37.67	Remove debris from corrugated steel drainage pipe, replace cured-in place pipe lining	
2	LA 39.08	Replace existing pipe with 24" RCP	
3	LA 40.16	Replace 36" CMP	
4	LA 40.18	Install culvert barrel lining (CIP) in upstream section of pipe, replace in-kind 24" RCP middle section of downstream pipe using Cut-and-Cover method, install culvert barrel lining downstream (CIP) section of pipe	
5	LA 40.23	Remove debris from corrugated steel drainage pipe, replace cured-in place pipe lining	
6	LA 40.24	Replace 36" RCP and 18" CMP sections	
10	LA 50.36	Replace bridge/culvert with new bridge with an underlying natural slope creek bottom	
13	LA 61.29	Replace 30" RCP	
14	LA 61.35	Replace 24" RCP	
15	LA 61.68	Replace 24" RCP	
16	LA 62.51	Replace 24" RCP on upstream section, joint seal manhole	
17	LA 62.55	Install culvert barrel lining (CIP), remove debris and clear manhole, and replace 18" CMP on downstream	
		section	
18	VEN 0.67	Install culvert barrel lining (CIP), replace lid/grate for upstream drop inlet	
19	VEN 0.92	Install culvert barrel lining (CIP)	

The reduction in scope of work and number of project locations presented no change to the environmental impacts of the project, therefore, no updates to environmental studies were warranted at that time. Concurrently, a preliminary proposal for the addition of a pedestrian undercrossing at Project Location No. 10 (Solstice Canyon Creek) was presented to the Division of Environmental Planning, though design on the proposed structure was not yet available for review/evaluation. The revalidation documented this preliminary proposal nevertheless and outlined the scope of accompanying environmental reevaluation that would be required when design for the proposed pedestrian undercrossing was available.

# 6. CHANGES IN THE PROJECT SETTING

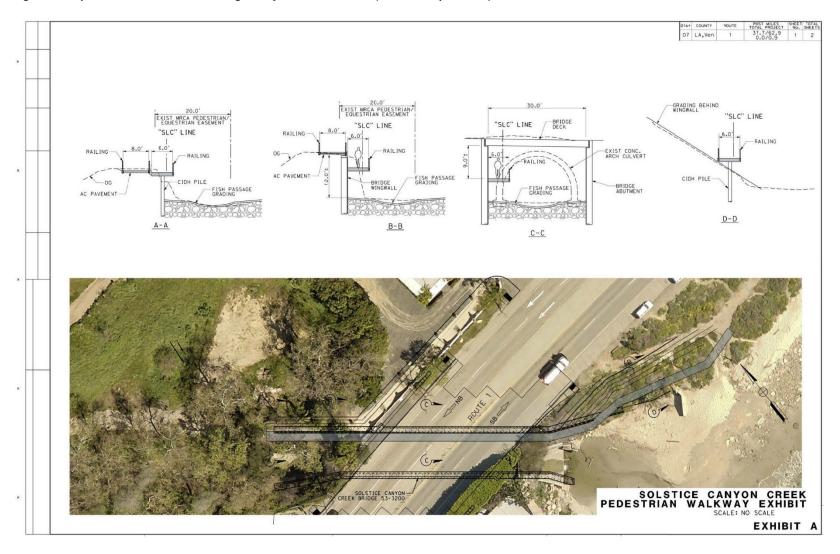
There are no substantial changes in the project environmental setting that would affect the previous analyses prepared for the MND/FONSI approved March 25, 2019, nor the previous environmental revalidations approved on August 24, 2021, and March 29, 2022.

## 7. PROJECT CHANGES AND EFFECTS

Addition of Pedestrian Undercrossing at Project Location No. 10 (Solstice Canyon Creek). In May of 2021, Caltrans Design began preliminary work on plans for a pedestrian undercrossing at Project Location No. 10 (Solstice Canyon Creek) in response to a request for public beach access from property owners adjacent to the project area. The property owners of the Calamigos Beach Club Restaurant at 26025 Pacific Coast Highway, Malibu, CA 90265 (which is part of the Calamigos Guest Ranch located at 327 Latigo Canyon Road, Malibu, CA 90265), requested that Caltrans consider providing walkway access underneath the newly proposed bridge structure at Solstice Canyon Creek from the restaurant to the beach as part of the proposed undertaking. While the intention of the request was to provide an undercrossing walkway to allow restaurant patrons access to the beach without having to cross the Pacific Coast Highway (PCH) roadway, its use was expanded to allow general public access (not specific to the adjacent property), which is more consistent with City of Malibu and California Coastal Commission goals of providing greater beach access to the general public. The following Figure 2 presents the general design of the proposed pedestrian undercrossing at Project Location No. 10 (Solstice Canyon Creek).



Figure 2. Proposed Pedestrian Undercrossing at Project Location No. 10 (Solstice Canyon Creek)





By July 2022, final design, and accompanying hydraulic analysis, and grading plans were achieved and plans/layouts specific to this proposed project location are appended to this addendum/reevaluation as Attachment A. In general, PCH runs north-to-south, but in the area of Project Location No. 10 (Solstice Canyon Creek), the roadway traverses the coastline in an east-to-west direction. The proposed pedestrian undercrossing is designed as a cantilevered concrete walkway so as not to impede hydraulic flow of the creek and would be attached to the eastern wing wall structure, perpendicular and beneath the newly proposed bridge.

Proposed pedestrian access to the undercrossing on the north side of the proposed bridge structure would originate at the roadway and be located approximately 70 feet north-west of the existing Calamigos Restaurant driveway. Southern access would start at the flat area on the embankment of the roadway just southeast of the proposed bridge structure. The walkway would be constructed parallel to the embankment [supported on evenly spaced Cast-In-Drilled-Hole (CIDH) piles] before connecting with the southern end of the cantilevered pedestrian undercrossing structure. In a review of the proposed pedestrian undercrossing plans/layouts, the following additional elements were implemented:

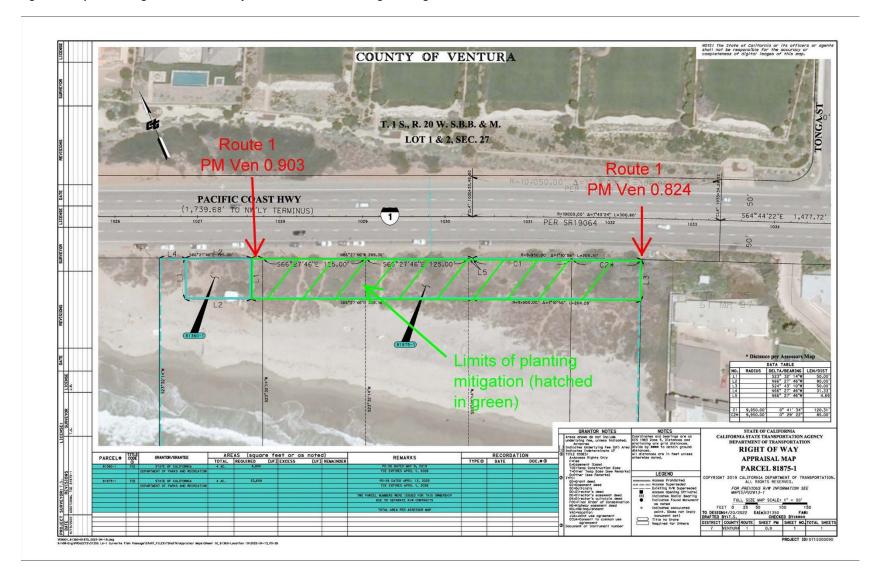
- To comply with ADA standards, the proposed pedestrian undercrossing structure would have a minimum width of 6 feet and a maximum slope grade of 8.33%.
- The proposed pedestrian undercrossing structure would be constructed above the 50-year flood zone line to ensure flood events do not cause damage to the facility.
- The proposed pedestrian undercrossing structure would be attached via cantilever method to the eastern wingwall
  and bridge abutments. Along the bridge structure, the walkway would be constructed approximately 10 feet above
  the channel bottom. Cable railings and lighting would be installed for pedestrian safety.

Final Proposed Creek Grading and Water Diversion Plan at Project Location No. 10 (Solstice Canyon Creek). In addition to the structural excavation work previously evaluated at Project Location No. 10 (Solstice Canyon Creek), grading plans for fish passage within Solstice Canyon Creek were finalized within the footprint of the proposed new bridge structure and the beachside area just south of the project area. Final proposed grading plans include construction of an engineered stream bed along the creek that would be composed of a 6-inch-thick top sand layer and an 8-inch-thick Class II Rock Slope Protection (RSP) bottom layer. The graded limits would extend approximately 50 feet downstream of the southern proposed bridge structure limits, and 125 feet upstream from the northern proposed bridge structure limits. In order to protect upstream embankments against erosion, Class XI RSP would be installed to extend approximately 40 feet upstream of the proposed bridge structure wingwalls. During construction, a water diversion plan would be implemented to temporarily redirect any flow within Solstice Canyon Creek via a pipe bypass method. During redirection of creek flow, the pipe on the southern portion would be placed on top of plywood sheets to minimize disturbance of the sand surface on the beach side and within an Environmentally Sensitive Area (ESA) in terms of Archaeology. Full proposed construction details can be referenced in the plans/layouts specific to this proposed project location as appended to this addendum/reevaluation as Attachment A.

Supplemental/Proposed Revegetation at Project Location No. 19 – Biological Mitigation. As part of biological mitigation for the proposed undertaking, a total of 8 acres of revegetation with native plantings is proposed at Project Location No. 19 (VEN PM 0.92). Adjacent parcels 81360-1 and 81875-1 will be granted for these purposes by the State of California – Department of Parks and Recreation, located along PCH/SR-1 between VEN PM 0.903 and 0.824. Planting will be implemented south of the PCH roadway on the beach side and will consist of brush removal (by hand) and the manual excavation of several holes (12-inches-deep) to accommodate 1-gallon size native plantings. The general extent of these parcels is delineated in the following figure.



Figure 3. Proposed Revegetation Area at Project Location No. 19 – Biological Mitigation





## 8. ENVIRONMENTAL REEVALUATION

Relevant environmental analyses were conducted to evaluation the proposed changes to the project. Environmental reevaluation was limited to the following topics deemed relevant in consideration of the aforementioned scope of work:

- Biology
- Cultural/Archaeological Resources
- Geology
- Hazardous Waste
- Hydrology
- Noise

Technical studies were conducted to determine the type and degree of impacts associated with the aforementioned project changes. These studies are list as follows and are available for review at the Caltrans District 7 Offices, Division of Environmental Planning, located at 100 S. Main Street, Los Angeles, California 90012:

- NES Amendment for the SR-1 Solstice Creek Culvert Retrofit and Bridge Replacement Project, February 23, 2022
- Supplemental Finding of No Adverse Effect without Standard Conditions for the State Route 1 Solstice Creek Fish
  Passage and Bridge Replacement Project [Focused Studies at Locations 10-12 (LA-001 PM 50.36 to 50.42], August 1,
  2022
- Foundation Report for Solstice Canyon Creek Bridge and Pedestrian Walkway, July 18, 2022
- Hazardous Waste Assessment for PS&E Package, September 1, 2022
- Structures Final Hydraulic Report for Solstice Canyon Creek Bridge, February 20, 2021
- Technical Addendum to the Structures Final Hydraulic Report for the Solstice Creek Bridge Replacement Project, March 11, 2021
- Technical Construction Nosie Memorandum for the Pacific Coast Highway Drainage Restoration Project, March 2022

## 8.1 Biological/Natural Environment

Reference NES Amendment for the SR-1 Solstice Creek Culvert Retrofit and Bridge Replacement Project, February 23, 2022

Wildlife. Reevaluation of impacts to the biological/natural environment showed that only minimal impacts to wildlife species are anticipated, so long as all avoidance and minimization measures are implemented. Potential impacts include disturbance of foraging, roosting, and nesting due to construction activity, temporary loss of habitat, and potential relocation of individuals that are within the construction footprint. Additionally, there will be disturbance from noise, dust, and other construction activity, including de-watering a small amount of Solstice Creek. Many of these locations already have substantial human presence, primarily including typical beach activity and car/truck traffic on SR-1. The sites with the most potential for impacts to wildlife species are Project Location Nos. 10, 13, 14, and 15. These sites have the most potential habitat and/or the least amount of human disturbance. Biological monitors shall be on-site at all times during construction work at these locations and any work that is adjacent to a beach environment.

**Updates to Presence of Regional Species and Habitats of Concern.** Updated species lists were obtained from the U.S. Fish and Wildlife Service Information for Planning and Consultation (IPaC) system, as well as an updated search of the California Natural Diversity Database (CNDDB). Five new listed species were identified in the updated IPaC/CNDDB lists and are presented in the following table.



Table 4. New Listed Species – Post IPaC/CNDDB Update

Common Name	Scientific Name	Status	General Habitat Description	Habitat Present/Absent within Project Site	Rationale
Southern California legless lizard	Anniella stebbinsi	SSC	Variety of habitats with moist, sandy, loose soil and sparse vegetation.	Present	General habitat ispresent, however roadway fill or other heavily modified areas are likely to be unappealing to this species as it requires loose soil that it can burrow through and heavy compaction of soil precludes their presence. Pre-construction surveys will be conducted prior to the start of construction activities.
Crotch bumble bee	Bombus crotchii	CSE	Generalist foragers in grassland and scrub habitat.	Present	Coastal scrub habitat is present at several project locations.
Quino checkerspot butterfly	Euphydryas editha quino	FE	Sunny openings within chaparral and coastal sage shrublands in Riverside and San Diego Counties	Absent	Substantially outside of the species range. Preconstruction surveys will be conducted prior to the start of construction activities.
Nuttall's scrub oak	Quercus dumosa	1B.1	Closed cone coniferous forest, chaparral, coastal scrub.	Present	Coastal Scrub habitat is present within the project site. However, no scrub oaks are present within the project footprint or will be affected by the project.
Chaparral ragwort	Senecio aphanactis	2B.2	Dry alkaline flats in Chaparral, cismontane woodland, and coastal scrub.	Absent	Alkaline soils not present within the project footprint.

**Discussion of Impacts to Biological Resources.** Reevaluation of impacts to the biological/natural environment showed that the addition of Rock Slope Protection (RSP) at Project Location No. 10 (Solstice Canyon Creek) present approximately 0.04 acres of impacts to riparian woodlands, to be converted from temporary impacts to permanent impacts. This will require additional offsite mitigation. Proposed mitigation ratios are being increased to 3:1 for impacts to riparian woodlands, and 1.5:1 for all other long-term temporary impacts to native habitats. This is consistent with the Local Coastal Programs (LCPs) and the California Coastal Commission's mitigation requirements for impacts within Environmentally Sensitive Habitat Areas (ESHAs).

With installation of the RSP and the addition of a pedestrian undercrossing at Project Location No. 10 (Solstice Canyon Creek), it is anticipated that two additional native trees, a heritage coast live oak (*Quercus agrifolia*), and a mature western sycamore tree (*Platanus recemosa*) will require removal. An additional 0.11 acres of impacts to disturbed ruderal vegetation along the roadway support slope (beachside of SR-1) is also anticipated. The following tables summarize revised impacts to Native Trees and Habitat and corresponding mitigation ratios.

**Table 5. Revised Native Tree Impacts and Mitigation Ratios** 

Native Tree Species Impacts	Original Impacts	Revised Impacts	Mitigation Ratio:	Total to be Mitigated
Platanus racemosa	2 Total; 1 Heritage, 1	3 Total; 1 Heritage, 2	10:1	30 Trees.
(Western sycamore)	Mature	Mature, root impacts to a third mature.		
Quercus agrifolia (Coast Live Oak)	1 Juvenile	2 Total; 1 Heritage and 1 Juvenile	10:1 and 5:1	15
Populus fremontii (Cottonwood)	1 Mature in an ornamental setting.	1 Mature	5:1	5



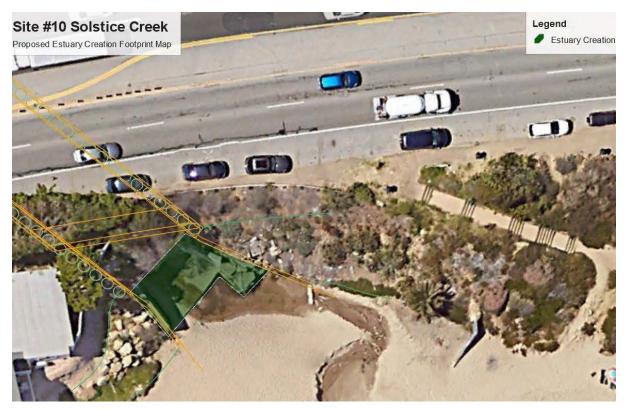
Table 6. Revised Habitat Impacts and Mitigation Ratios

Habitat Type	Amount of Habitat Present	Proposed Mitigation Ratios
Sycamore Riparian Woodland	Approx. Total: 0.08 Acres	Mitigation Ratio 3:1
	Approx. Temp. Impacts: 0.03 Acres	Total: 0.30 Acres
	Approx. Perm. Impacts: 0.05 Acres	On-site Restoration: 0.03 Acres
		Off-site Restoration: 0.27 Acres with MRCA
Unvegetated Estuary Lagoon	Approx. Temp. Impacts: 0.06 Acres.	Mitigation Ratio: 1.5:1
	Approx. On-site Creation: 0.02 Acres	Total: 0.09 Acres
		On-site Creation: 0.02 Acres
		On-site Restoration: 0.06 Acres
		Off-Site Restoration: 0.01 Acres.
Coastal Scrub Ephemeral Stream Channel	Approx. Temp. 0.13 Acres (WOUS)	Mitigation Ratio: 1.5:1
	Approx. Temp. 0.08 Acres CDFW Only	Total: Approx. 0.32 Acres.
	Total: Approx. Temp. 0.21 Acres	Approx. 0.21 Acres On-site
		Approx. 0.11 Acres Off-Site
Coastal Scrub Uplands	Approx. Temp. 0.39 Acres	Mitigation Ratio 1.5:1
		Total 0.58 Acres
		Approx. 0.39 Acres On-Site
		Approx. 0.19 Acres Off-Site

Creation of Unvegetated Estuary Habitat as a Result of Improvements at Project Location No. 10 (Solstice Canyon Creek).

One additional impact that was not documented in the original Natural Environment Study (NES) is the on-site creation of unvegetated estuary habitat as a result of removal of the existing concrete culvert bottom at Project Location No. 10 (Solstice Canyon Creek). This activity will generate excess debris piles at the culvert outlet (to be retained on site) and will create approximately 0.02 acres of unvegetated estuary habitat at the mouth of Solstice Canyon Creek. The following figure presents the extent of this new estuary habitat creation.

Figure 4. Proposed Estuary Creation Footprint at Project Location No. 10 (Solstice Canyon Creek)





## Updates to Avoidance, Minimization, and/or Mitigation Measures

- Rock Slope Protection (RSP) within Solstice Canyon Creek shall be vegetated with willow cuttings (*Salix ssp.*) in order to provide some habitat benefits within the armored portion of the creek.
- All re-vegetation shall be done with native species mixes appropriate for the location and known to occur within the
  project vicinity and local seeds shall be used if feasible to help maintain the genetic integrity of the area.
- All native trees will be mitigated at a 10:1 ratio. If there is not enough space on-site to mitigate the trees, additional off-site mitigation will have to be purchased in order to plant the required trees.

**Conclusions and Regulatory Determination.** Due to the changes in project scope within Waters of the United States, the following permits will require updating:

- Army Corps Nationwide Permit 404 (Pre-Construction notification)
- Regional Water Quality Control Board 401 Water Quality Certification
- California Department of Fish and Wildlife 1600 Streambed Alteration Agreement

Any additional avoidance, minimization, and or mitigation measures as a result of updates to the aforementioned permits shall be adopted when issued and incorporated into the project's Environmental Commitments Record (ECR) and PS&E package accordingly.

**Federal Endangered Species Act Consultation Summary.** Under provision of Section 7(a)(2) of the ESA, a federal agency (e.g. FHWA) that permits, licenses, funds, and otherwise authorizes a project activity must consult with the USFWS to ensure that its actions would not jeopardize the continued existence of any listed species or destroy or adversely modify critical habitat. Caltrans originally initiated Section 7 consultation with USFWS Ventura Office on October 22, 2018 requesting concurrence with Caltrans' determination of **May Affect, Not Likely to Adversely Affect**: California Red-legged Frog (*Rana Draytonii*). The USFWS Concurred in a letter dated November 20, 2018.

Caltrans has determined that the proposed change to the project scope will have no effect on southern Steelhead trout (*Onchorhyncus mykiss*).

**Conclusion.** In conclusion, the new scope of work will not change the project's overall construction impacts. However, dur to the increased permanent footprint and permanent loss of additional trees, including a heritage coast live oak (*Quercus agrifolia*) and an additional western sycamore (*Platanus racemosa*), additional off-site mitigation is proposed.

As this project currently stands, there is no further need for biological review and this project may proceed. If there should be any additional changes in scope of work, further reevaluation will be required, and the Division of Environmental Planning shall be notified accordingly.

## 8.2. Cultural Resources

Reference Supplemental Finding of No Adverse Effect without Standard Conditions for the State Route 1 Solstice Creek Fish Passage and Bridge Replacement Project [Focused Studies at Locations 10-12 (LA-001 PM 50.36 to 50.42], August 1, 2022

Revisions to Area of Direct Impact (ADI)/Area of Potential Effects (APE). Based on the changes made to the scope of work and in accordance with Section 106 Programmatic Agreement Stipulation VII.A, the ADI/APE for the proposed project was modified to include the proposed walkway at Project Location No. 10 (Solstice Canyon Creek), which would add 0.072 acres to the originally proposed Area of Direct Impacts (ADI). No changes were made to the ADI/APE for Project Location No. 19 that was originally included as Attachment A of the 2018 HPSR. Changes in scope of work at Project Location No. 10 (Solstice Canyon Creek) and the associated changes in the ADI/APE for Project Locations no. 10, 11, and 12 in the vicinity of Solstice Canyon Creek are reflected in the following figure.



Figure 5. Revised Project ADI/APE Map for Project Locations No. 10, 11, and 12 in the Vicinity of Solstice Canyon Creek



Areas of Direct Impact and ESA Locations



**Public Participation and Requirements of Assembly Bill 52 (AB52).** The previous Native American consultation parties (listed below) that had previously shown interest in the project were contacted to inform them of the project changes on July 13, 2022.

- Patrick Tumamait -Barbareño/Ventureño Band of Mission Indians
- Mia Lopez, Chairperson Coastal Band of the Chumash Nation
- Jairo Avila, Tribal Historic and Cultural Preservation Officer Fernandeño Tataviam Band of Mission Indians
- Beverly Salazar Folkes, Elders Council Fernandeño Tataviam Band of Mission Indians
- Robert Dorame, Chairperson Gabrielino Tongva Indians of California Tribal Council

Acknowledgements of the receipt of project changes were received from the following parties on July 14, 2022:

- Mr. Patrick Tumamait
- Ms. Beverly Salazar Folkes
- Ms. Christina Conley-Haddock on behalf of Mr. Robert Dorame

Supplemental Finding of No Adverse Effect (FNAE) for Project Changes in Scope of Work. A supplemental Finding of No Adverse Effect (FNAE) was prepared for changes to the project scope of work and was concurred with/approved by the State Historic Preservation Officer (SHPO) on October 15, 2022. In addition to the previously proposed 2018 construction activities included in the initial/original 2018 FNAE, the Supplemental FNAE included the construction of a pedestrian undercrossing and walkway to allow access to the beach front; and final grading and activities associated with the construction of the fish passage and temporary diversion of the creek via a pipeline, which will be placed on plywood to protect the archaeological site of concern.

Assessment of Effects to Archaeological Site CA-LAN-210. Sixty-five percent (65%) of the proposed walkway for the pedestrian undercrossing would be located within the previously defined ADI for the bridge structure at Solstice Canyon Creek and would be built as part of the proposed undertaking. The ADI was extended to cover the land needed for the walkway exit to the southwest of the bridge structure and ending at a flat area on the embankment of the PCH roadway where users will be able to access the beach.

The required maximum excavation depth for the portion of the walkway path on the north side of the bridge structure at Corral Canyon Road is specified at 9-feet (maximum). The southern portion of the elevated walkway will be placed on top of Cast-In-Drilled-Hole (CIDH) piles with a diameter of 2-feet. The required excavation depth for the CIDH piles will be to bedrock, circa between 40-and-50-feet (maximum).

Additionally, the grading of Solstice Canyon Creek on the south side of the culvert/new bridge will extend roughly 50-feet downstream and 125-feet upstream from the proposed bridge limits and will follow the structure excavation within the proposed bridge improvements. The temporary redirection of the creek will be achieved via pipeline and will have no effect on the immediate surface as it will be placed on top of plywood to be placed on the ground surface to protect disturbances to positive Shovel Test Pit (STP) locations of archaeological site CA-LAN-210.

The impact of construction of the walkway to connect to the pedestrian undercrossing and the grading of the creek would, at a minimum, affect only a combined total of 0.5 acres of additional land for both the northside and southside of the walkway. Overall, the diversion of the creek for construction purposes would have a minimal impact as Environmentally Sensitive Area (ESA) fencing would be installed to minimize pedestrian traffic, as well as a plywood bed to protect the creek bed where the positive STPs were located.

As previously stated in the 2018 Caltrans FNAE, prehistoric site CA-LAN-210 is assumed eligible under Criterion D for the purposes of this undertaking only. The previously recorded site boundary encompasses a total of 11 acres and the total acreage of the site that may be affected by the work proposed at Project Location No. 10 (Solstice Canyon Creek), including the newly proposed walkway construction and the grading of the creek on both upstream/downstream areas is approximately 1.26 acres. Project Location No. 10 (Solstice Canyon Creek) is located on the southeastern portion of the site and the majority of the proposed project ground disturbance will occur in previously disturbed areas – where the existing culvert location is located. Construction work will occur nearly/entirely within the previous footprint of the previous feature. An additional 125 feet upstream and 50-feet downstream as well as along the fill in a southwest direction, and 4-feet on either side of the previous culvert construction footprint will be added to the current project and this work will occur within the 10-feet of documented fill. While the proposed project excavations will have a maximum depth of 15-feet, it is likely that the soils below the 10-feet of



fill are also disturbed. Therefore, a minimal percentage of potentially native soils may be disturbed in comparison to the total site acreage.

Based on the above, there is limited-to-no potential to affect CA-LAN-210, which is assumed NRHP-eligible under Criterion D for the purposes of this project only; however, a requirement for archaeological and Native American monitoring is proposed as a condition of the Undertaking to further avoid any potential adverse effects. The previously prepared 2018 Post-Review Discovery and Monitoring Plan (PRDMP) is still valid and shall be utilized during construction as stated in the 2018 FNAE. Additionally, the 2018 Environmentally Sensitive Area (ESA) Action Plan will still be used to ensure avoidance and protection.

Assessment of Supplemental Proposed Revegetation at Project Location No. 19 (Post Mile VEN 0.92). In addition to the previously proposed 2018 construction activities at Solstice Canyon Creek, as well as the new proposed walkway and temporary creek diversion efforts during construction; supplemental revegetation has also been proposed at Project Location No. 19 to consist of native plants for a total of 8-acres as part of biological mitigation associated with the project. Parcels 81360-1 and 81875-1 will be granted by the State of California – Department of Parks of Recreation for these purposes along PCH/SR-1 between VEN Post Mile 0.90 and 0.82. During the identification efforts in support of the proposed undertaking, no previously recorded cultural resources were found at this location. Nonetheless, due to the sensitivity of the route, both archaeological and Native American monitoring will occur during all ground disturbing activities in the area as per procedures as outlined in the 2018 PRDMP.

**Project Conditions and Post-Review Discovery and Monitoring Plan.** Through background research and Native American consultation, the portion of the APE located at Project Location No. 10 (Solstice Canyon Creek) has been identified as the previously recorded location of archaeological site CA-LAN-210. Because of this, and the fact that undisturbed sediments may occur at depths below 10 feet at this location and Project Locations 11, 12, and 19, archaeological and Native American monitoring is recommended. The previously submitted and approved ESA, PRDMP, and PCRIP are still valid, and the newly expanded Supplemental APE map will be utilized during construction to determine ESA locations as well as PRDMP areas.

**Conclusions.** The results of the identification efforts in support of the proposed Undertaking found one previously recorded prehistoric archaeological site within the APE: CA-LAN-210. CSO approved the assumption of eligibility for the site on November 14, 2018, and it is still deemed valid for the proposed changes and expansion of the ADI. CA-LAN-210 will be protected from inadvertent project effects through the establishment of an ESA and implementation of the procedures outlined in the 2018 ESA Action Plan and 2018 PRDMP.

No previously recorded archaeological sites were found at Location 19 (located between VEN-001 PM 0.903 and PM 0.824), where the proposed biological mitigation efforts will occur. Nonetheless due to the sensitivity of the area and the possibility of inadvertent archaeological discoveries, the implementation of the 2018 PRDMP will also take place at this location.

In 2018 Caltrans District 7 applied the Criteria of Adverse Effect per 36 CFR 800.5(a)(1) for CA-LAN-210 and determined that the Finding of No Adverse Effect (FNAE) is appropriate for this undertaking per Stipulation X.B.2 of the Section 106 Programmatic Agreement, which are deemed valid for the proposed changes to the scope of work. Should intact portions of the site be encountered during construction of the undertaking, the information potential of the site will be captured by the imposed construction monitoring, as described in the approved 2018 PRDMP.

#### 8.3 Geoloav

Reference Foundation Report for Solstice Canyon Creek Bridge and Pedestrian Walkway, July 18, 2022

Pursuant to a request by the Office of Bridge Design South (OBDS), the Division of Engineering Services – Geotechnical Services, prepared a Foundation Report for the proposed replacement of the Solstice Canyon Creek bridge (Caltrans Bridge No. 53-0030) at Project Location No. 10 and the addition of a pedestrian undercrossing and walkway to the project scope of work. In April 2020, 10 geotechnical boring samples were tested for uniaxial compression of rock and four rock samples were submitted for point load testing.

**Surface Conditions.** Solstice Canyon is a steep sided coastal canyon that runs northwest from the Pacific Ocean. The west side of the canyon is labeled as Solstice Hill, which is a moderately flat area, at approximately 500 feet elevation. The east side of the canyon is a ridge that forms a drainage divide between Corral Canyon to the east, and trends north, at approximately 600 feet elevation. There are existing ripraps protections and concrete waste slabs in the existing embankments adjacent to bridge wingwalls.



**Subsurface Conditions.** Based on a 2004 Log of Test Borings (LOTB), the site consists of loose to medium dense artificial fill to approximate elevation 18-to-13 feet underlain by very loose to very dense alluvium, all underlain by igneous rock (andesitic breccia) on the east side and sedimentary rock (shale/siltstone) on the west side. Bedrock surface varies from approximate elevation 4-feet at Abutment 1 and elevation 0.0 at Abutment 2 of the existing bridge/culvert structure. The shale/siltstone is described as medium gray, intensely weathered, and soft to moderately soft. The andesitic breccia is medium dark gray, slightly to moderately weathered, moderately hard to hard, and slightly to moderately fractured.

**Groundwater.** Groundwater data includes a measurement from a 2004 geotechnical investigation and former environmental monitoring wells at the Union 76 Gas Station and former Chevron site adjacent to Project Location No. 10.A summary of groundwater data is summarized in the following table:

Table 7. Summary of Groundwater Data

Boring/Well No.	Distance/Direction from Center of Bridge	Ground Surface Elevation (feet)	Depth to Groundwater (feet)	Groundwater Elevation (feet)
Boring No. 04-1 (2004 LOTB)	7 ft / West (S 80° W)	33.90	23.00	10.90
Well No. MW-2 (former Chevron site)	6 ft / North (N 2° W)	35.15	24.65	10.50
Well No. MW-8 (Union 76 gas station)	186 ft / Northwest (N 68° W)	31.56	15.81	15.75

The design groundwater elevation used for analysis was 10.9 feet.

**As-Built and Scour Data.** The existing bridge is a single span, cast-in-place (CIP), reinforced concrete (RC) arch culvert, originally built in 1947. The bridge is 21-feet-long with a 41° skew. The width of the bridge is approximately 162 feet. The headwalls transition to cantilevered retaining walls that angle away from the inlet and outlet of the culvert. The bottom of footing elevation varies along the culvert and wingwalls from approximately 11.0-ft to 8.75-ft. Scour data for the existing bridge structure is summarized in the following table.

Table 8. Scour Data for Existing Solstice Canyon Creek Bridge

Support Location	Long Term (Degradation and Contraction) Scour Elevation (feet)	Short Term Local Scour Depth (feet)
Abutment 1R	24.2	0.0
Abutment 2R	24.2	0.0
Bridge Abutment 1	8.0	7.4
Bridge Abutment 2	10.0	7.4
Abutment 1L	12.2	0.0
Abutment 2L	13.3	3.0

**Soil Corrosion Evaluation.** Corrosion test results show the project site is corrosive. The following table presents a summary of the corrosion test results.

**Table 9. Soil Corrosion Test Summary** 

			Resistivity	Sulfate (PPM)	Chloride (PPM)	Corrosive?
04-1	17.4 to 8.9	8.39	970	260	21	No
04-1	8.9 to 3.9	7.61	740	676	20	No
04-2	27.6 to 23.6	7.95	330	1,116	477	No
04-2	23.6 to 18.6	7.37	470	853	163	No
04-2	18.6 to 8.6	8.20	470	624	568	Yes
04-3	35.2 to 18.7	7.75	390	5,073	215g	Yes
04-3	18.7 to 15.2	8.07	730	685	77	No
04-3	15.2 to 10.2	7.95	950	163	26	No



#### SEISMIC INFORMATION

**Ground Motion Hazard.** The peak ground acceleration (PGA) for the site is 0.58g. The design magnitude (M) is 6.64 and the site-to-fault distance is 10.75 miles (17.3 km). The site is underlain by approximately 30 feet of soil that is classified as Type S2, according to the Seismic Design Criteria, Version 2.0, dated April 2019.

**Surface Fault Rupture.** Based on data, there is no calculated potential rupture at the bridge site although the site is located within the AP zone. The bridge can be designed with a calculated potential surface rupture hazard of 0.

**Liquefaction.** According to the map of the Earthquake Fault Zones and Seismic Hazard Zones of Malibu Beach 7.5 Minute Quadrangles, dated August 16, 2007, the bridge is within a liquefaction zone. Liquefaction analysis was performed for the site utilizing borings 04-1, 04-2, and 04-4 (2004 LOTB) with design groundwater table elevation of 10.9 feet. for borings 04-3 and 04-5, SPT blow counts are higher than 30 below design groundwater table, therefore, there is no potential for liquefaction in these borings. Downdrag load was conservatively calculated considering full embedment for the CIDH piles in the top 34 feet depth (abutment wall section). Potential liquefaction is summarized in the following table.

Table 10. Liquefaction Potential at Solstice Canyon Creek Bridge

Support	Liquefaction Elevation (feet)	Estimated Seismic Induced Settlement (inches)	Downdrag Zone Bottom Elevation (feet)	Estimated Downdrag Load (kips/pile)	
Abutment 1	Boring 04-2	10.9 to 5.4 (3.31m to 1.64m)			
Al. 1 2	Boring 04-1	10.9 to 7.8 (3.31m to 2m)	-	200	
Abutment 2	Boring 04-4	10.9 to 7.8 (3.31m to 2.38m)	<del>-</del> 7.0		
Pedestrian Walkway	Boring 04-2	10.9 to 5.4 (3.31m to 1.64m)		100	

Lateral spreading is limited by foundations for the bridge replacement, which are shafts that will be excavated into bedrock. However, the post liquefaction load against the abutment walls should be checked by OBDS utilizing a friction angle of  $\phi$ =3 degrees (ka=0.9 and kp=1.11) conservatively estimated for the liquefiable soil layer for the post liquefaction load case.

**Tsunami Risk.** According to the Tsunami Inundation Map for Emergency Planning, Malibu Beach Quadrangle, the project is not located within a Tsunami inundation area.

# 8.4 Hazardous Waste/Materials

Reference Hazardous Waste Assessment for PS&E Package, September 1, 2022

A Hazardous Waste Assessment (HWA) was prepared and approved on September 1, 2022, in response to a request to update the HWA dated January 26, 2022. The HWA accounts for the reduction in scope of work as captured in the March 2022 environmental revalidation and permanent easements, property acquisition, and temporary construction easements and property acquisition in-fee as required for the proposed undertaking.

Caltrans conducted five Site Investigations (SIs) in support of the HWA which have sampled various media and the Solstice Canyon Creek Bridge (e.g., groundwater, soils, surface water) at required parcels.

## SITE INVESTIGATION (SI) RESULTS

Soils. A total of 82 soil samples were collected from 24 borings along the project corridor. A summary of findings follows:

- Total lead (TTLC) was detected in 81 of 82 samples analyzed at concentrations ranging from 0.87 to 1000 mg/kg.
  - 14 samples had TTLC concentrations greater than or equal to 80mg/kg which is the threshold for regulated material under the Caltrans Department of Toxic Substances Control (DTSC) Aerially Deposited Lead (ADL) Agreement.



- Soluble Lead was detected in 55 of 70 samples analyzed at concentrations ranging from 0.15 to 55 mg/l.
  - 7 samples had Soluble lead concentrations greater than or equal to 5 mg/kg which is the threshold for classification as non-RCRA California Hazardous Waste
- Soluble Lead by the TCLP method was detected in 8 out of 20 samples analyzed at concentrations ranging from 0.051
   NJ to 2.1 mg/l.
- Petroleum hydrocarbons were detected in 65 out of 74 samples analyzed.
  - Petroleum hydrocarbons as Gasoline range organics was not detected above laboratory reporting limits.
  - Petroleum hydrocarbons as Diesel Range Organics was detected in 59 samples analyzed at concentrations ranging from 1.1 to 190 mg/kg
  - Petroleum hydrocarbons as Oil Range Organics was detect in 65 samples analyzed at concentrations ranging from 1.1 to 1700 mg/kg.

A total of 51 soil samples were analyzed for Title-22 Metals. Metals were below environmental screening levels except for arsenic. Arsenic was detected in 51 samples at concentrations ranging from 0.71 NJ to 9.8 mg/kg. Arsenic concentrations were above EPA Regional Screening Levels for Arsenic (3 mg/kg) but are within the background concentration for Los Angeles (12 mg/kg) as established by DTSC (DTSC 2008).

A total of 74 soil samples were analyzed for VOCs and SVOCs. VOCs and/or SVOCs were detected in 58 samples analyzed at concentrations below environmental screening levels:

- 4-Chlorotoluene was detected in 1 sample at a concentration of 0.59 μg/kg.
- 4-Isopropyltoluene was detected in 2 samples at concentrations ranging from 0.97 to 1.3 μg/kg.
- Benzene was detected in 39 samples at concentrations ranging from 0.55 to 53 μg/kg.
- Carbon disulfide was detected in 21 samples at concentrations ranging from 0.95 to 42 μg/kg.
- Ethylbenzene was detected in 7 samples at concentrations ranging from 0.57 to 2.7 μg/kg.
- Toluene was detected in 41 samples at concentrations ranging from 0.42 NJ to 19 μg/kg.
- Trans-1,2-Dichloroethene was detected in 1 sample at a concentration of 1.4  $\mu$ g/kg.
- Bis(2-ethylhexyl)phthalate was detected in 3 samples at concentrations ranging from 63 to 470 μg/kg.

A total of 4 samples were analyzed for organochlorine pesticides. Organochlorine pesticides were detected in two samples at concentrations below environmental screening levels. The following organochlorine pesticides were detected.

- 4,4'-DDD was detected in 2 samples analyzed at concentrations ranging from 2.0 to 5.2 μg/kg.
- 4,4´-DDT was detected in 2 samples analyzed at concentrations ranging from 6.5 to 31 μg/kg.
- Apha-BHC was detected in 2 samples analyzed at concentrations ranging from 1.2 to 2.9 μg/kg.
- ullet Beta-BHC was detected in 1 sample analyzed at a concentration of 4.6  $\mu g/kg$

**Water and Groundwater Samples.** A total of 12 water samples, 9 groundwater, and 3 surface water samples were collected for the project. The groundwater grab samples, which included a duplicate sample, were collected from 8 borings along Solstice Canyon Creek between 2006 and 2020, which 3 surface water samples, including a duplicate, were collected from the creek from 2006 to 2007.

Surface water and Groundwater contained detectable concentrations of Metals, petroleum hydrocarbons, VOCs, organochlorine pesticides, and other NPDES permit constituents. Various Metals, Petroleum Hydrocarbons, and sulfates have exceeded NPDES Construction Dewatering General Permit Screening Levels.

## **SOIL MANAGEMENT**

Soils along the project corridor have Aerially Deposited Lead (ADL) and petroleum hydrocarbon impacts requiring special management, handling, and disposal. The Office of Environmental Engineering (OEE) has drafted the Non- Standard Special (NSSP) 14-11.11 "Department Generated Contaminated Material" for the PS&E package and will submit the NSSP to HQ-DEA for approval.

Structure excavation for bridge removal at Solstice Canyon Creek (Project Location No. 10) is not considered to be impacted by petroleum hydrocarbons as the material being removed was placed during construction of the bridge and culvert and is above



the historic high-water table. Bridge removal should not be impacted by petroleum hydrocarbon releases in groundwater associated with adjacent and closed Leaking Underground Storage Tank (LUST) hazardous waste sites.

OEE has defined the contamination and classified soils into categories for management:

**Contaminated soil:** Material containing petroleum hydrocarbons, lead, volatile organic compounds, semi-volatile organic compounds.

Petroleum Contaminated Soil (PC): Material containing total petroleum hydrocarbons (TPH) with diesel range organics and/or oil range organics above the maximum soil screening level for soils less than 20 feet above groundwater according to Calif. Regional Water Quality Control Board Interim Site Assessment & Cleanup Guidebook, Los Angeles and Ventura Counties, Region 4, May 1996 OR above United States Environmental Protection Agency (EPA) Regional Screening Levels (RSLs) for Residential soil, November 2021.

Petroleum Impacted Soil (PI): Material containing TPH with diesel range organics and/or oil range organics below the maximum soil screening level for soils less than 20 feet above groundwater according to Calif. Regional Water Quality Control Board Interim Site Assessment & Cleanup Guidebook, Los Angeles and Ventura Counties, Region 4, May 1996 and below United States Environmental Protection Agency (EPA) Regional Screening Levels (RSLs) for Residential soil, November 2021.

**Type PC-1:** Petroleum contaminated soil containing lead concentrations greater than or equal to 1,000 mg/kg total lead or 5.0 mg/L soluble lead as tested using the CA-WET. Material is California hazardous waste that must be disposed of at an appropriately permitted California Class I disposal facility. Type PC-1 material exists at the locations listed in the following table:

Table 11. Existence of Type PC-1 Material within Project Study Area

<b>Project Location</b>	Elements of Work	Depth Below Ground Surface (BGS)
14	Roadway Excavation (Type PC-1)	Surface-1 feet BGS

**Type PI-1:** Petroleum impacted soil containing lead concentrations greater than or equal to 1,000 mg/kg total lead or 5.0 mg/L soluble lead as tested using the CA-WET. Material is California hazardous waste that must be disposed of at an appropriately permitted California Class I disposal facility.

Table 12. Existence of Type PI-1 Material within Project Study Area

<b>Project Location</b>	Elements of Work	Depth Below Ground Surface (BGS)
10 (Solstice Canyon Creek)	Roadway Excavation (Type PI-1)	O-to-5 BGS
14	Roadway Excavation (Type PI-1)	1-3 feet BGS

**Type PI-2:** Petroleum impacted soil containing lead concentrations below 320 mg/kg total lead and below 5 mg/L soluble. Must be disposed of at an appropriately permitted California Class III or Class II facility, or an appropriately permitted California Soil treatment/recycling facility.

Table 13. Existence of Type PI-2 Material within Project Study Area

<b>Project Location</b>	Elements of Work	Depth Below Ground Surface (BGS)
10 (Solstice Canyon Creek)	Roadway Excavation (Type PI-2)	O-to-14 BGS
13	Roadway Excavation (Type PI-2)	Surface to total depth
14	Roadway Excavation (Type PI-2)	3 feet BGS to total depth

**Unregulated Backfill:** Backfill material exposed to aerially deposited lead deposition at concentrations below regulatory screening levels. Material contains average lead concentrations less than 80 mg/kg total lead and below 5 mg/L soluble lead and is not regulated by DTSC as a hazardous substance or a hazardous waste. This material does not require disposal at a permitted landfill or solid waste disposal facility. The RWQCB has jurisdiction over reuse of this material at locations outside the job site limits.



Table 14. Existence of Unregulated Backfill Material within Project Study Area

<b>Project Location</b>	Elements of Work	Depth Below Ground Surface (BGS)
10 (Solstice	Bridge Removal	As shown on plans
Canyon Creek)		

## **CAST-IN-DRILLED HOLE (CIDH) PILES**

Cast-In-Drilled-Hole (CIDH) piles for the proposed bridge structure at Project Location No. 10 (Solstice Canyon Creek) are 48-inch diameter borings, which are anticipated to extend to 40-feet below average sea level. Based on discussion and input from the Project Development Team (PDT), it is not feasible to separate impacted materials during boring. The mass of a single pile was examined, and using the highest concentration of total lead, Total Petroleum Hydrocarbons-Diesel Range (TPH-D), and Total Petroleum Hydrocarbons-Oil Range (TPH-O) for each target depth of contamination were calculated, and the concentrations are presented in the following table.

Table 15. Total Lead/TPH-D/TPH-O Contaminant Concentrations at Project Location No. 10 (Solstice Canyon Creek)

Contaminant	Concentration (mg/kg)
Total Lead (mg/kg)	34.64
TPH-D	4.86
TPH-O	26.63

Taken as a whole, the resulting cuttings from the CIDH piles are considered non-hazardous waste for disposal. CIDH piles can be managed under structures procedures for non-hazardous contaminated soil waste for disposal.

## MINIMIAL DISTURBANCE OF CONTAMINATED SOILS

The following construction activities which will disturb contaminated soils are considered minimal disturbance activities:

- 1. Temporary construction area signposts
- 2. Planting
- 3. Removal of Hot Mix Asphalt (HMA) Dike
- 4. Installation of HMA Dike Type D
- 5. Concrete Rock Slope Protection (RSP)
- 6. Conduit installation
- 7. Roadway excavation waste, concrete slabs

These activities will disturb soils only in the immediate area of the activity. Contaminated material subject to minimal disturbance must remain in the immediate area of disturbance and shall not be transported elsewhere or disposed of outside of the highway. The requirements for soil with minimal disturbance are contained in NSSP 14-11.11. If excess soils are generated, they must be disposed of in a Class II landfill.

## HEALTH AND SAFETY PLAN FOR MANAGEMENT OF CONTAMINATED AND IMPACTED SOILS

The contractor will be required to prepare a project-specific Health and Safety Plan (HASP) to protect workers from exposure to contaminated and impacted soils. The HASP must be signed and sealed by a Certified Industrial Hygienist (CIH), and the funds shall be allocated accordingly.

## **GROUNDWATER MANAGEMENT**

Construction dewatering will be required for the construction activities at Project Location No. 10 (Solstice Canyon Creek). Groundwater beneath the project site is contaminated with metals and sulfates above the discharge limits of a NPDES permit. Unfiltered groundwater samples collected in June 2020 detected arsenic, cadmium, copper, lead, mercury, nickel, silver, zinc,



and sulfates at concentrations above NPDES permit limits. Treatment options for groundwater may be limited as the filtered groundwater samples collected in June 2020 detected lead and nickel at concentrations above NPDES permit limits.

Groundwater samples from June 2020 detected volatile organic compounds and organochlorine pesticides at concentrations below NPDES permit limits. Previous groundwater samples detected additional metals constituents above NPDES permit limits and VOCs and petroleum hydrocarbons at concentrations below NPDES permit limits.

OEE has crafted NSSP 14-11.17 "DEWATERING AND MANAGEMENT OF CONTAMINATED LIQUID" for management of contaminated groundwater on the project and has submitted the specification to HQ Division of Environmental Analysis for approval.

**Dewatering, Collecting, Containerizing and Disposing Plan.** The Contractor will be required to prepare a project specific Dewatering, Collecting, Containerizing and Disposing Plan to document how the contractor will manage construction dewatering. This cost must be incorporated into in BEES either as a separate item or included in construction dewatering costs.

**Health and Safety Plan for management of Construction Dewatering.** The Contractor will be required to prepare a project specific Health and Safety Plan (HASP) to protect workers from exposure to contaminated groundwater. The HASP must be signed and sealed by a Certified Industrial Hygienist (CIH) and the appropriate funds shall be allocated appropriately.

## **NEARBY HAZARDOUS WASTE SITES**

A review of environmental databases, California Water Quality Control Board's Geotracker database, and the Department of Toxic Substances Control's Envirostor database identified hazardous waste sites along the project corridor. Given the construction activities, OEE has reviewed these sites within for potential impacts to the project which are presented in the following table.



Table 16. Hazardous Waste Sites Identified within 1000 feet of the Project Corridor

Project Location No.	Hazardous Waste Site within 1000 feet	Status	Risk Analysis
1	POTRERO CANYON PARK (T10000013337) 15101 PACIFIC COAST HWY PACIFIC PALISADES, CA 90272	OPEN - ASSESSMENT & INTERIM REMEDIAL ACTION	No Risk - TPH Release. Site Investigations related to Caltrans Permit (07-18-N-RD-0716) and Site remediation covered work area and no impacts to work area are present from release
2	SURFSIDE CLEANERS (SL0603738960) 17340 SUNSET BLVD. PACIFIC PALISADES, CA 90272	OPEN - ASSESSMENT & INTERIM REMEDIAL ACTION	Low Risk - PCE release associated with Dry Cleaning Operations. Given Construction activities, distance from construction area to the release, contaminate transport is away from construction area, the site is not anticipated to pose a risk to the project.
10 & 11	FORMER CHEVRON / BEAU RIVAGE/ FORMER RESTAURANT (T0603770976) 26025 PACIFIC COAST HWY MALIBU, CA 90265-4519	CLOSED - LUST CLEANUP SITE	Low Risk - Construction area in exposed soils investigated thoroughly with Site Investigations. Low risk remains for areas Northwest of the bridge under paved right. Given impacts identified the HW Site is considered a low risk.
10 & 11	GAS S/S (T0603700047) 26201 PACIFIC COAST HWY MALIBU, CA 90265	CLOSED - LUST CLEANUP SITE	Low Risk - Construction area in exposed soils investigated thoroughly with Site Investigations. Low risk remains for areas Northwest of the bridge under paved right. Given impacts identified the HW Site is considered a low risk.
10 & 11	76 PRODUCTS STATION #5331 (T0603703141) 26101 PACIFIC COAST HWY MALIBU, CA 90265	CLOSED - LUST CLEANUP SITE	Low Risk - Construction area in exposed soils investigated thoroughly with Site Investigations. Low risk remains for areas Northwest of the bridge under paved right. Given impacts identified the HW Site is considered a low risk.
10 & 11	TOSCO/UNOCAL #30856 (T10000000536) 26101 PACIFIC COAST HWY. MALIBU, CA 90265	CLOSED - LUST CLEANUP SITE	Low Risk - Construction area in exposed soils investigated thoroughly with Site Investigations. Low risk remains for areas Northwest of the bridge under paved right. Given impacts identified the HW Site is considered a low risk.
18	ZUMA BEACH SERVICE YARD (T0603704865) 30100 PACIFIC COAST HWY MALIBU, CA 90265	CLOSED - LUST CLEANUP SITE	Low Risk - Diesel Release documented in 1996, no cleanup documents exist. Given distance from construction area and construction activities no risk is anticipated.
19	ZUMA BEACH SERVICE YARD (T0603704865) 30100 PACIFIC COAST HWY MALIBU, CA 90265	CLOSED - LUST CLEANUP SITE	Low Risk - Diesel Release documented in 1996, no cleanup documents exist. Given distance from construction area and construction activities no risk is anticipated.



## **ASBESTOS CONTAINING MATERIAL (ACM)**

Bridges are considered regulated structures by the United State Environmental Protection Agency (USEPA) and require compliance with National Emission Standards for Hazardous Air Pollutants (NESHAP) including notification to the delegated air district. The delegated air district for Los Angeles County is the South Coast Air Quality Management District (SCAQMD). SCAQMD requires an asbestos survey to accompany the required notification of proposed work on structures.

No ACM was identified in the bridge materials sampled; however, ACM may still be present in internal components that were inaccessible during sampling. Should unanticipated ACM, or potential ACM be discovered during construction, OEE shall be contacted, and protocols shall be followed according to Standard Provision 14-11.02, "Discovery of Unanticipated Asbestos and Hazardous Substances."

## **NESHAP NOTIFICATION**

Bridge structures are subject to the National Emissions Standards for Hazardous Air Pollutants (NESHAP) regulations. The regulations require notification to the delegated air district prior to renovation regardless of whether asbestos was detected. The delegated air district is the South Coast Air Quality Management District (SCAQMD). The asbestos survey must accompany the notification. The contractor is required to submit notification to the SCAQMD at least 15 days prior to work on the bridge structure.

#### TREATED WOOD WASTE

Removal of the wood posts from the roadway signs and MBGRs will generate Treated Wood Waste (TWW) which requires disposal as a hazardous waste. The wood posts used in the guardrails and roadways signs have been treated with chemical preservatives that are hazardous (e.g., arsenic, chromium, copper, creosote, and pentachlorophenol). TWW is a non-RCRCA (California) hazardous waste, and the handling and the handling, storage, transportation, and disposal is subject to California hazardous waste regulations. SSP 14-11.14 "Treated Wood Waste" shall be utilized in the project PS&E package.

## THERMOPLASTIC AND PAINTED STRIPES AND PAVEMENT MARKING

The project scope includes removal of pavement delineation, currently the method of removal is unspecified.

Yellow traffic stripes contain lead and chromium at concentrations that exceed hazardous waste threshold levels established by the California Health and Safety Code and Title 22 of the California Code of regulations. The waste generated by the removal of yellow thermoplastic and yellow paint traffic stripes by-itself require disposal at a Class I facility. The contractor is required to prepare an LCP to protect workers from exposure to the hazards from lead per Cal-OSHA Title 8 California Code of Regulations and a Work Plan for management, testing, transport, and disposal of the hazardous waste. The SSP 14-11.12 "Remove Yellow Traffic Stripe with Hazardous Waste Residue" shall be utilized in the project PS&E package.

White, non-yellow, thermoplastic, paint stripes, and pavement markings contain lead at a concentration that is not hazardous. Yellow thermoplastic traffic stripe and pavement markings installed after 2006 and yellow traffic paint used after 1997, contain low concentrations of lead and are classified as non-hazardous waste. Removal of white traffic stripes and pavement markings will be performed during construction. Residue from removing white traffic stripes by-itself will not contain hazardous levels of lead. The contractor is required to prepare an LCP to protect workers from exposure to the hazards from lead per Cal-OSHA Title 8 California Code of Regulations. The SSP 84-9.03C "Remove Traffic Stripes and Pavement Markings Containing Lead" shall be utilized in the project PS&E package.

All thermoplastic, paint stripes, and pavement markings contain lead and an LCP will be required to protect workers, as management of these materials exposes workers to health hazards.



# 8.5 Hydrology

Reference Structures Final Hydraulic Report for Solstice Canyon Creek Bridge, February 20, 2021; Technical Addendum to the Structures Final Hydraulic Report for the Solstice Creek Bridge Replacement Project, March 11, 2021

A Structures Hydraulic Report was prepared for the proposed bridge replacement project at Project Location No. 10 (Solstice Canyon Creek) and a Technical Addendum to that report was prepared for the addition of a pedestrian undercrossing and walkway to the project scope of work.

Project Watershed. The Solstice Canyon Creek watershed upstream of the existing culvert under SR-1 drains approximately 4.7 square miles. Solstice Canyon Creek is a small perennial, spring-fed creek that drains directly into the Pacific Ocean approximately 2.3 miles east from the City of Malibu. Solstice Canyon Creek is a rural watershed with very little development except for a small housing development (approximately 10% of the watershed) in the lower easterly side of the watershed. The watershed is covered by 31% forest. Solstice Canyon Creek begins in its headwaters as a small spring at an approximate elevation of 2,100-feet. The watershed lays completely within the Santa Monica Mountains National Recreation Area, part of the larger coastal mountain range of the Transverse Ranges. Solstice Canyon Creek flows in a southeasterly direction; flowing approximately 5 miles before reaching the Pacific Ocean at the Dan Blocker State Beach. The watershed has a mean basin elevation of 1,331-feet with a maximum basin elevation of 2,785-feet. Solstice Canyon Creek is a high gradient mountain stream that has an averaged slope of approximately 3% at the project location. The average annual precipitation is approximately 19.9 inches.

**Streambed.** The existing culvert has no hydraulic skew. Field observations and photographic logs find the channel bed has some channel armoring such as heavy grained soils or larger material such as river gravel, cobbles, etc., which could potentially help protect the channel from scouring, but those will be deemed negligible for this analysis. Overall, the channel and side slopes within the project area appear to be both vertically and horizontally stable.

**Project Design Flood Discharges.** The current FEMA Flood Insurance Study (FIS) used for this report is 06079CV001C, effective May 2018. The areas of minimal flood hazard, which are the areas outside the SFHA and higher than the elevation of the 0.2-percent-annual-chance flood, are labeled Zone C or Zone X (unshaded). The bridge replacement and culvert modification proposed is not located within a 100-year base floodplain and exists within a FEMA Zone X (unshaded) area, which is considered to be of minimal flood hazard.

Solstice Canyon Creek is an ungaged/unregulated watershed that makes predicting peak flood magnitudes problematic. However, three nearby watersheds with stream gaging stations were used to predict flood magnitudes at Solstice Canyon Creek. There are two U.S. Geological Survey (USGS) stream gages and one stream gage operated by the Los Angeles County Flood Control District (LACFCD). There is the USGS stream gage on Malibu Creek (USGS gage station 11105500) approximately 3.9-miles away, the USGS stream gage on the Topanga Creek watershed (USGS gage station 11104000) approximately 9.2-miles away, and the LACFCD stream gage at Zuma Canyon Creek (Station F53-R) approximately 4.4 miles away. Using the annual peak streamflows, (following the Bulletin 17-B guidelines for a log-Pearson Type III distribution) was calculated at each of the aforementioned gages. The results from this analysis were then compared to the peak flood flow analysis of the regional regression flood-frequency equations for rural ungagged streams developed by the USGS in their publication Methods for Determining Magnitude and Frequency of Flood in California 2006. According to the FEMA flood insurance study, and proved through in the above analysis, a comparison of the results obtained from the log-Pearson Type III analysis of the three stream gages produced higher peak flow rates as compared to the results from the regional regression equations from the USGS publication.

Reviewing the hydrological (watershed) characteristics for each of the stream-gaged watersheds, Zuma Canyon Creek and Solstice Canyon Creek shared many approximate hydrological results such as the mean annual precipitation, drainage area, and mean basing elevation. Therefore, it was determined that the peak flood flows from Zuma Canyon Creek would be basin transferred to the Solstice Canyon Creek watershed using the analyses from the log-Pearson Type III distribution were used for the determination of flood flows at Solstice Canyon Creek.

A comparison was made using the National Streamflow Statistics program (NSS) using the WMS software and the Q100 for this watershed to verify the basin transfer method. Based on NSS, the 100-year peak discharge (Q100) value was approximately 3300 cfs. The design flood discharge (cubic feet/second) is expressed in the following table. The results of the flow analysis are shown in the following table.



**Table 17. Solstice Canyon Creek Watershed Discharge Values** 

Flood Frequency	Design Flood Discharge (cfs)
$Q^{200}$	4180
$Q^{100}$	3360
Q <sup>50</sup>	2650
Q <sup>10</sup>	1225
$Q^2$	217

There are no reports of recent flooding in this area from the BIRIS database. At this site, the channel appears to be very stable with no erosion on the banks and exposed substructure visible.

#### **DESIGN OBJECTIVES**

The hydraulic report addresses the scour and hydraulic impacts of the proposed work on the Solstice Canyon Creek Bridge and the impacts on the floodplain. Hydraulic analyses were calculated using the Two-Dimensional (SHR-2D) hydraulic modeling software (v. 13.0.12) developed by the U.S. Bureau of Reclamation/Aquaveo. To determine if the proposed project would have any adverse effects on the floodplain of Solstice Canyon Creek, the design flood discharges were modeled through the existing and proposed structures utilizing the SRH-2D hydraulic model. One model was made for the existing conditions and other models for the proposed conditions, tides, sea-level rise, and other impacts utilizing the new structure.

#### SURFACE WATER MODELING SYSTEM (SMS) SRH-2D MODELING APPROACH AND METHODOLOGY

Information used for modeling includes topographical land surveys collected by aerial LiDAR, in-stream surveys collected by bathymetric methods by Caltrans' Structure Maintenance and Investigations' Hydraulic Inspection Branch, Microstation drawing from Preliminary Investigations, and the latest Federal Emergency Management Agency's (FEMA) Flood Insurance Study (FIS) and Flood Insurance Rate Maps (FIRM) for Los Angeles County.

**Tidal Impacts on the Cordilleras Creek Floodplain.** The nearest station with the required amount of data available for tidal observations and predictions was Datums for 9410840, Santa Monica Marina CA from the NOAA Tides and Currents. The tidal impacts and their effect on this project are presented in the following table.

Table 18. Tidal Effects at Project Location No. 10 (Solstice Canyon Creek)

Datum	Elevation (feet)	Description
HAT	7.08	Highest Astronomical Tide
MHHW	5.24	Mean Higher-High Water
MLLW	-0.19	Mean Lower-Low Water
NAVD88	0.00	North American Vertical Datum of 1988

The Highest Astronomical Tide is an extreme event that would happen once or twice a year. For the Maximum tide observed for this station the depth of flow in the bridge would be below the soffit. For the average daily high tide, the Mean Higher High Water flow is below the Q100 elevations and will not add any additional conditions to alter the Q100 parameters.

Sea Level Rise (SLR) Considerations at Project Location No. 10 (Solstice Canyon Creek). The following table presents the SLR projections for the Solstice Canyon Creek Bridge replacement project and revised per the 2018 Sea Level Guidance.



Table 19. Sea Level Rise Projections at Project Location No. 10 (Solstice Canyon Creek)

			Probabilistic Pro	iections (in feet)		
		MEDIAN	LIKELY RANGE	1 IN 20 CHANCE	1 IN 200 CHANCE	
						H++ Scenario
	2020	0.4	02.05	0.0	0.0	1
High Emissions	2030	0.4	0.3 - 0.5 0.4 - 0.8	0.6 0.9	0.8 1.2	1 1.7
riigii Liilissioiis	2050	0.8	0.4 - 0.8	1.3	1.9	2.6
Low Emissions	2060	0.9	0.6 – 1.2	1.5	2.3	
High Emissions	2060	1.1	0.8 – 1.4	1.8	2.6	3.8
Low Emissions	2070	1.0	0.7 – 1.4	1.9	3.0	
High Emissions	2070	1.3	1.0 – 1.8	2.3	3.4	5.1
Low Emissions	2080	1.2	0.8 – 1.7	2.3	3.8	6.5
High Emissions	2080	1.7	1.1 – 2.3	2.9	4.4	6.5
Low Emissions	2090	1.3	0.8 - 2.0	2.7	4.6	0.1
High Emissions	2090	2.0	1.3 – 2.8	3.5	5.5	8.1
Low Emissions	2100	1.5	0.9 – 2.3	3.1	5.5	10.0
High Emissions	2100	2.3	1.5 – 3.3	4.3	6.8	10.0

Even higher values could occur with one or more combinations of strong storms, high tide events, wind waves, and high flow events on the rivers.

**Tsunami Effects at Project Location No. 10 (Solstice Canyon Creek).** Caltrans requires that the design of all new bridges within five miles of the coast (and in bays) must include an evaluation for tsunami loads. Caltrans evaluates bridges for the tsunami hazard consistent with a 5% probability of being exceeded in 50 years. A tsunami can damage a bridge if the waves are high enough to strike the deck. However, the design tsunami at most locations along California's coast should be below the superstructure. Therefore, wherever possible, new bridges should be designed so the tsunami flows below the soffit (or bottom girder flange). The maximum wave height elevation is 8.85 ft Mean Sea Level (11.47 ft). Maximum velocity is 2.62 ft/s adding 3.5 ft for sea-level rise the estimated tsunami wave height is 14.97 ft.

Fish Passage Requirements at Project Location No. 10 (Solstice Canyon Creek). The proposed design of a single-span bridge structure spanning 30-feet is sufficient to meet analysis requirements set forth by the California Department of Fish and Wildlife (CDFW) culvert/bridge criteria for the design method – Stream Simulation Option. According to the CDFW, the Stream Simulation Option is a design process that is intended to mimic the natural stream processes within a culvert/bridge. Determination of the high and low fish passage design flows, water velocity, and water depth is not required for this option since the stream hydraulic characteristics within the bridge are designed to mimic the stream conditions upstream and downstream of the bridge crossing. CDFW design requirements for the Stream Simulation Option are 1) expand the bridge crossing to be as wide, or wider than, the bankfull channel and 2) grade/slope the channel bed inside the bridge at a gradient similar to that of the adjacent stream reach. With the design of the proposed bridge, Caltrans has met the design requirements for the Stream Simulation Option. The proposed 30-foot bridge opening is as wide as the bankfull channel. Modeling and analyses show that all flood flows will be contained within the natural upstream creek channel and flows through the proposed bridge structure are not anticipated to raise the floodwater elevations above the bankfull channel elevation. Finally, the channel bed slope will be regraded to match the upstream and downstream natural slope after removal of the existing culvert and its concrete apron.

Ocean Waves and Total Water Levels. To calculate oceanic effects the Preliminary Hydraulic Report (PHR) that was completed for this project used the Total Water Level (TWL) from the 2016 FEMA FIS Study for this county. The TWL is the sum of the Stillwater Elevation (SWEL), the wave setup, and wave runup. The current FEMA FIS Study does not show the TWL but does indicate the wave runup as shown in Graphic 3. Table 5 from FEMA will be used instead of the TWL for the calculation of oceanic effects for this project. The following table summarizes coastal transect mapping considerations.



**Table 20. Summary of Coastal Transect Mapping Considerations** 

	Wave Runup Elevation1 (feet)			Wave Setup Elevation1 (feet)		
Flooding Source and Location	10% Annual Chance	1% Annual Chance	0.2% Annual Chance	10% Annual Chance	1% Annual Chance	0.2% Annual Chance
At Escondido Beach, at Escondido Canyon Mouth	10.7	12.9	15.5	*	*	*
At Escondido Beach, approximately 200 feet East of the Intersection of Latigo Shore Place and Latigo Shore Drive	11.5	14.3	16.9	*	*	*

Zone VE on the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) is subdivided into elevation zones and Base Flood Elevations (BFEs). The limit of Zone VE shown on the FIRM is defined as the farthest inland extent of any of these criteria (determined for the 1% annual chance flood condition):

- The primary frontal dune zone is defined in 44 CFR Section 59.1 of the NFIP regulations. The primary frontal dune represents a continuous or nearly continuous mound or ridge of sand with relatively steep seaward and landward slopes that occur immediately landward and adjacent to the beach. The primary frontal dune zone is subject to erosion and overtopping from high tides and waves during major coastal storms. The inland limit of the primary frontal dune zone occurs at the point where there is a distinct change from a relatively steep slope to a relatively mild slope.
- The wave runup zone occurs where the (eroded) ground profile is 3.0 feet or more below the 1-percent annual chance TWL.
- The wave overtopping splash zone is the area landward of the crest of an overtopped barrier, in cases where the potential 1-percent annual chance TWL.
- The breaking wave height zone occurs where 3-foot or greater wave heights could occur (this is the area where the wave crest profile is 2.1 feet or more above the total Stillwater elevation).
- The high-velocity flow zone is landward of the overtopping splash zone (or area on a sloping beach or other shore type), where the product of depth of flow times the flow velocity squared (hv2) is greater than or equal to 200 ft3/sec2. This zone may only be used on the Pacific Coast.

## **HYDRAULIC MODELING RESULTS**

Analyzing the Solstice Canyon Creek Bridge at existing conditions, all floodwaters from the design flood discharges are not contained within the main channel throughout the hydraulic study area of the project site. Overflow from the creek spread out to the shallow depression at the north-west corner of the existing entrance headwall. The proposed structure contains all flows in the main channel.

Stage, Velocity, and Freeboard at Project Location No. 10 (Solstice Canyon Creek). Tables 21 through 24 summarize the hydraulic results at the water surface elevations at the lowest chord of the existing and proposed structures. Using the existing culvert headwall/bridge soffit elevation and the calculated water surface elevations, the available freeboard at the culvert and the proposed bridge was calculated. Note that the modeling indicates that all the channel flows at the entrance do not go directly into the channel. Calculations indicate a maximum flow of 2500 cfs. For study purposes, the existing culvert shall be analyzed as if all flow goes to the culvert. Also, the results shown in Tables 21 and 22 are at the beginning and end of the existing culvert which is not at the same locations as the deck edges of the proposed bridge.

Table 21. Hydraulic Results under Existing Conditions upstream at Project Location No. 10 (Solstice Canyon Creek)

Design Flood Discharge	Soffit Elevation (feet)	Water Surface Elevation	Maximum Channel	Available Freeboard
(cfs)		(feet)	Velocity (fps)	(feet)
Q <sup>100</sup> 3360	29.7	36	13.4	0
Q <sup>50</sup> 2650	29.7	27.6	13.2	0



Table 22. Hydraulic Results under Existing Conditions downstream at Project Location No. 10 (Solstice Canyon Creek)

Design Flood Discharge (cfs)	Soffit Elevation (feet)	Water Surface Elevation (feet)	Maximum Channel Velocity (fps)	Available Freeboard (feet)
Q <sup>100</sup> 3360	29.7	18.7	25	0
Q <sup>50</sup> 2650	29.7	17.3	23.7	0

Table 23. Hydraulic Results under Proposed Conditions at Project Location No. 10 (Solstice Canyon Creek), Upstream Edge of Bridge Deck

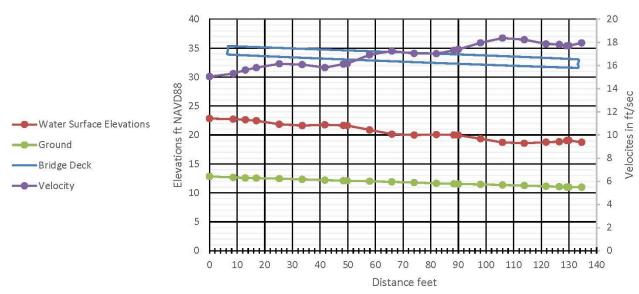
Design Flood Discharge (cfs)	Soffit Elevation (feet)	Water Surface Elevation (feet)	Maximum Channel Velocity (fps)	Available Freeboard (feet)
Q <sup>100</sup> 3360	33.2	22.7	15.2	10.5
Q <sup>50</sup> 2650	33.2	22.3	15.2	10.9

Table 24. Hydraulic Results under Proposed Conditions at Project Location No. 10 (Solstice Canyon Creek), Downstream Edge of Deck

Design Flood Discharge (cfs)	Soffit Elevation (feet)	Water Surface Elevation (feet)	Maximum Channel Velocity (fps)	Available Freeboard (feet)
Q <sup>100</sup> 3360	30.9	19.2	18.8	11.7
Q <sup>50</sup> 2650	30.9	18.5	15.7	12.4

Waterway Impacts/Coastal Assessment at Project Location No. 10 (Solstice Canyon Creek)/Floodplain. A profile of the proposed water surface elevations at the new structure is shown in the following figure for a cross section located along the centerline of the stream. The water surface elevations for the existing and proposed conditions are within 2 tenths of a foot. The proposed structures have minor changes in the characteristics of the floodplain. There is a very slight increase in the lateral extents of the floodplain, but those extents are within the FEMA established Boundaries.

Figure 6. Profile of Proposed Water Surface Elevations at Bride Deck, Project Location No. 10 (Solstice Canyon Creek)



With the proximity of the Solstice Canyon Creek culvert to the coast of the Pacific Ocean, the proposed replacement bridge must take into account the impacts of Sea-Level Rise (SLR). Using the guidance of SLR projections from the National Research Council's 2018 report titled "State of California Sea-Level Rise Guidance" and adopted by the California Coastal Commission, the preliminary hydraulic analysis for the proposed Solstice Canyon Creek Bridge will account for the SLR projections for the



projected year of 2100. Table 19 provides the SLR projections (depth above existing ocean sea levels) for the Pacific Ocean near Los Angeles, California for the projected year of 2100.

In addition to the effects of SLR on the project location, the effects of coastal flooding from the Pacific Ocean for various ocean storm frequencies will be analyzed and inserted into the hydraulic model as downstream boundary conditions. Water Elevations for the 1% annual chance 500 feet west of the mouth of Solstice Canyon Creek shall be used for calculation purposes. All calculations are based on the Q100 flow of 3360 cfs.

**Table 24. Coastal Assessment Impacts Matrix** 

	Tidal	Water surface elevations at downstream edge of the proposed deck	Freeboard	
Datum	Description	Elevation (ft)		
НАТ	Highest Astronomical Tide	7.08	19.2	13.9
MLLVV	Mean Lower-Low Water	-0.19	19.2	13.9
9	Sea Level rise		_	
Projected Year	Scenario	Sea Level Rise Projection (feet)		
	Likely Range	3.3	19.2	13.9
2100	1 in 200 Chance	6.8	19.2	13.9
	H+++	10	19.2	13.9
- 100	sunami Effects			
Maximum Wave Height Elevation	8.85 ft Mean sea level 11.47 ft NAVD88			
Tsunami Effects requirements	Add 3.5 feet for sea-level rise	14.97 feet	19.2	13.9
Wav	e Runup Elevation			
1% Annual Chance		18.3 feet	19.3	13.8

The probability of maximum wave tsunami event coinciding with a 1% annual exceedance storm event and 2100 SLR is relatively small, the tsunami wave height is excluded as part of coastal hazard assessment in this report. Table 24 indicates that none of the oceanic effects will cause any flooding of the proposed structure.

Through hydraulic modeling of the Solstice Canyon Creek Bridges/Floodplain, it was determined that the proposed work will have minor changes in the floodplain. The proposed structure will lower the water surface elevations, decrease the flow velocities, and slightly increase the wetted area of the channel that was previously covered in a culvert. The proposed structure will not impede flows that pass the design-year flood events.

The proposed pedestrian walkway may have flooding due to the wave runup conditions. Wave runup may cause shallow flooding due to small indeterminate water surface elevation increases that are greater than the 100-year water surface



elevations. The depth from the proposed walkway to the proposed bridge invert is approximately 10 feet. Periodic inspections or maintenance may be required for the invert of the bridge to keep any reductions of the waterway depth from occurring.

## STREAMBED AND CHANNEL SLOPES

For the natural proposed channel bottom, there is a geologic report and Log of Test Borings dated February 3, and 4, 2005. Based on the geologic report the bed is composed of:

- Artificial fill material is found to overlay most of the natural site. The fill material generally consists of medium dense to loose silty gravel with sand, clayey gravel with sand, cobbles, rootlets, and concrete debris. Cobbles encountered during the drilling ranged in size from 0.3 ft to 0.6 ft and are hard, moderately weathered, and subrounded. Multiple cobbles and boulders are visible within the slopes of the wash (from the slopes to the bottom of the wash). They are also observed scattered at the bottom of the wash. The boulders were hard, moderately weathered, subrounded to round, and ranged from approximately 10 ft to 3 ft in size. The fill material extends to a minimum elevation of approximately 18.7 ft in all borings except boring 04-2, where the fill material extends to a minimum elevation of 23.6 ft
- Marine Deposits are found throughout the site and consist of loose organic-rich black fine-grained silty sand with an
  abundance of shell fragments. The marine deposits are found to be generally consistent in thickness in all of the
  borings except boring 04-2. Across the site, the general thickness of this layer is approximately 3.3 ft and in boring 042, the thickness of this layer is approximately 7.5 ft. The marine deposits extend to a minimum elevation of 16.1 ft
- Alluvial Deposits are encountered across the site and consist primarily of medium dense to very dense silty sand with gravel, silty gravel with sand, clayey gravel with sand, and sandy lean clay with gravel. Also incorporated with the alluvial/fan deposits are cobbles and boulders. Cobbles encountered during the drilling ranged in size from 0.3 ft to 1.0 ft and are hard, moderately weathered, subangular to subrounded, and consist primarily of andesite and sandstone. Boulders encountered during the drilling range in size from approximately 1.0 ft to 1.5 ft are hard, moderately to slightly weathered, subangular to subrounded, and consist primarily of andesite. The alluvial deposits extend to a minimum elevation of 0.9 ft.
- Bedrock encountered during the drilling operation consists of volcanic rock, andesitic breccia, and sedimentary rock, shale/siltstone. The andesitic breccia is encountered in all borings, except 04-1. The minimum elevation encountered is 1.3 ft. The andesitic breccia is interpreted as belonging to the Zuma Volcanics that includes basaltic and andesitic flows, breccias, pillow lavas, mudflow breccias, and local interlayers of siltstone and mudstone. The shale/siltstone is encountered in borings 04-1 and 04-4. The minimum elevation encountered is -40.0 feet, the maximum depth explored during the subsurface investigation. The shale/siltstone is interpreted as belonging to either the Trancas or the Monterey Formation.

It is estimated the bed is composed of sand, sandy silt, gravel, and cobbles similar to that of the 2007 structure at Corral Canyon, located 800 feet upstream. It is assumed that this combination of materials would provide some armoring for scour protection. Overall, the channel and side slopes within the project area appear to be both vertically and horizontally stable.

#### **DRIFT/FLOATING DEBRIS**

Caltrans's Bridge Inspection Reports did not indicate occasional large drift accumulations. In general, historical and current site-specific documents tend to indicate that floating drift/debris should not be expected to be a significant issue at this bridge during typical high flood conditions. Therefore, a floating drift/debris load will not be added as a component of the local pier and abutment scour analysis.

## SCOUR AND CHANNEL DEGRADATION

A scour analysis was calculated following the guidelines set forth by the FHWA's Hydraulic Engineering Circular Number 18 (HEC-18) - Evaluating Scour at Bridges, 5th Edition, and the National Cooperative Highway Research Program's NCHRP Report 24-20 - Estimation of Scour at Bridge Abutments. HEC-18 defines total scour as a summation of three components: 1) long-term degradation of the riverbed, 2) contraction scour at the bridge, and 3) local scour at the piers and abutments.

**Long-Term Degradation Changes.** The long-term streambed elevation changes at the existing bridge were calculated using a 75-year (assumed serviceable life of the structure) and projected degradation trends using historical channel cross sections



collected at the upstream face of the bridge. At the project location, there is no long-term streambed elevation history so no calculations for this parameter of scour are available. However, 800 feet upstream at the Coral Canyon structure constructed in 2007, there has been approximately 0.5 feet of aggradation based on survey and construction records.

**Contraction Scour.** Contraction scour is a lowering of the streambed across the stream or waterway bed at the bridge caused when the flow area of a stream at the flood stage is reduced, either by a natural constriction of the stream channel or by a bridge structure. Contraction scour can occur through the constriction of the channel caused by the bridge in either the horizontal or vertical direction (pressure-flow contraction scour). At the project location, contraction scour will be accounted for in the NCHRP Report 24-20 -Estimation of Scour at Bridge Abutments.

**Local Pier and Abutment Scour.** Local scour involves the removal of bed material around piers, abutments, and embankments. It is caused by an acceleration of flow and resulting vortices induced by obstructions to the flow. For the local scour conditions for the structure, both local pier and local abutment scour were analyzed. There is local scour along the toe of the entrance embankments and along the abutments due to the velocity of the flows.

## **CONCLUSIONS AND RECCOMENDATIONS**

- The proposed replacement work was determined not to cause any significant or immediate hydraulic or scour-related issues provided recommendations for scour depth in this report are used.
- There will be ample freeboard as designed at the entrance and exit of the structure during the peak of the Q<sup>100</sup> event.
- The entrance headwall should resemble something like the following graphic: the scour will be much less than with the planned headwall that has the 90-degree" lip" on the ends of the wingwalls. The maximum scours on the 45 degree opening wing walls are 11.8 ft. The maximum scour depending on grading, is approximately 20 ft.

## 8.6 Noise

Reference Technical Construction Nosie Memorandum for the Pacific Coast Highway Drainage Restoration Project, March 2022

The Caltrans District 7 Office of Environmental Engineering, Noise and Vibration Branch, prepared a Technical Construction Noise Memorandum in March 2022 to evaluate potential noise impacts and any corresponding construction noise abatement measures that may result from proposed night construction work at 5 of 14 project locations. The analyses were completed under the requirements of Title 23, Part 772 of the Code of Federal Regulations (23 CFR 772.19) "Construction Noise," which requires the identification of sensitive land uses and measures to minimize construction noise impacts.

The proposed drainage restoration project, as a whole, does not meet the criteria to be classified as either Type I or Type II project categories. Therefore, it falls under the Type III category. Accordingly, since the project will not increase traffic volume capacity, speed, roadway alignment, or alter existing traffic noise shielding features within the project limits; preparation of a detailed Noise Study Report (NSR) was not required to evaluate potential traffic noise impacts and abatement in accordance with Caltrans Traffic Noise Protocol for New Highway Construction, Reconstruction, and Retrofit Barrier Projects. The following five project locations evaluated where night construction work is proposed are presented in the following table.

Table 25. Project Locations Analyzed where Night Construction Work is Proposed

Project Location No.	Post Mile	Description of Work
2	39.08	Replace existing pipe with 24-inch Reinforced Concrete Pipe (RCP)
3	40.16	Replace in-kind 36-inch Corrugated Metal Pipe (CMP)
4	40.18	Install culvert barrel lining (CIP) in upstream section of pipe, replace in-kind 24" RCP middle section of downstream pipe using Cut-and-Cover method, install culvert barrel lining downstream (CIP) section of pipe
6	40.24	Replace 36" RCP and 18" CMP sections
16	62.51	Replace 24" RCP on upstream section, joint seal manhole

The project site is located on SR-1/Pacific Coast Highway, from 0.4 miles south of Temescal Canyon Road to 0.1 miles north of Tonga Street. Only the drainage restoration sites where sensitive land uses have been identified and which could have potential construction noise impacts have been included in the construction noise impact analysis. The locations that could be



subject to construction noise impacts are mainly comprised of residential land uses, however, the northernmost site (Project Location No. 16) near Leo Carillo Beach is an outdoor recreational facility.

Ambient Sound Levels. Sound emitted by vehicular traffic traveling on PCH is the predominant and highest noise source in the project area. Vehicles traveling on PCH generally include automobiles, trucks, buses, and motorcycles. In the evening and early morning hours as traffic volumes decrease, ocean wave action can become the main source of noise. Existing sound levels were measured at noise-sensitive receptors closest to the proposed drainage restoration sites. Measurement site locations are shown in the images and in Figure 3 following this section. The measurements consisted of short-term readings of approximately 30-minute duration. The six site measurement locations were chosen to be representative of the existing land uses in the vicinity of the project. Long-term 24-hour measurements were not possible due to unavailability of appropriate locations for setting up a sound level meter (SLM). Due to the Covid-19 epidemic conditions, contacting residents for permission to enter private property areas for SLM placement was not possible. However, recent noise long-term measurements conducted in July 2021 for another project at 19812 Pacific Coast Hwy, a location that is acoustically representative of the southern portion of this project's limits can be used to estimate existing worst-hour traffic noise conditions for the analysis locations for this project. The adjustment for the measured noise levels derived from that location's noisiest hour is +1 dBA, with worst-hour noise conditions occurring from 3:23 to 4:23 PM. Therefore, a +1 dBA adjustment will be used to approximate noisiest hour for the analysis locations for this project. The measured sound levels are presented in the following table.

**Table 26. Existing Ambient Sound Levels** 

Receiver/ Measurement Site	Post mile	Project Location No.	Land Use	Existing Measure Noise Level Leq <sub>avg</sub>	Adjusted to Noisiest Hour Level Leq <sub>avg</sub>	Measurement Time/Date	Duration
R1	39.08	2	Residential, Outdoors	76.0	77	11:01AM 1/5/2022	30 minutes
R2	-	18034 W. Coastline Dr.	Residential, Outdoors	61.4	63	1:07PM 1/5/2022	15 minutes
R3	40.16	3	Residential, Outdoors	73.5	75	11:57AM 1/5/2022	30 minutes
R4	40.18 40.24	4/6	Recreational/Residential, Outdoors	69.8	71	1:45PM 1/5/2022	30 minutes/
R5	62.51	16	Recreational, Outdoors	70.6	72	11:15AM 1/5/2022	11 minutes
R6	62.51	16	Recreational, Outdoors	54.2	55	11:11AM 1/5/2022	35 minutes

**Construction Noise.** 23 CFR 772 requires that construction noise impacts be identified but does not specify specific methods or abatement criteria for evaluating construction noise. However, the FHWA Roadway Construction Noise Model (Federal Highway Administration 2006) can be used to determine if construction would result in adverse construction noise impacts on land uses or activities in the project area. Construction noise is regulated by Caltrans Standard Specifications, Section 14-8, Noise and Vibration which sets forth requirements that noise levels generated during construction shall comply with applicable local, state, and federal regulations.

Construction noise may also be governed by the City of Malibu Municipal Code Chapter 8-24 Noise, specifically Section 8.24.050(G), which prohibits the use of construction tools, equipment, impact devices, derricks or hoists on weekdays between the hours of 7:00 PM and 7:00 AM, before 8:00 AM or after 5:00 PM on Saturday, or at any time on Sundays or holidays, unless the City Manager grants expressed written permission pursuant to Section 8.24.060(D).

Construction work for the proposed project will most likely not include any stationary sources, construction equipment or activities that could produce ground-borne vibration levels that would exceed those which are considered to cause damage to buildings. Construction equipment may produce temporary and infrequent ground-borne vibration, however if it is expected that heavy duty impact-type construction equipment will be used, provisions should be put in place for minimizing disturbances to the nearby residential areas. Typically, equipment used for trenching or hand-held jackhammers do not generate levels of vibration that could cause any sort of architectural or structural damage to standard residential structures. For purposes of evaluating the significance of the vibration impacts, the following numeric thresholds can be used:



- Vibration levels that exceed approximately 85 VdB or 0.075 in/sec at sensitive land uses, which is the vibration level
  that is considered by the Federal Transit Administration (FTA) to be the threshold for human annoyance for
  infrequent ground-borne vibration; or
- Vibration levels that exceed approximately 102 VdB or 0.5 in/sec for new residential structures and approximately 97
   VdB or 0.3 in/sec for older residential structures.

The construction noise impacts were assessed by Caltrans using the FHWA's Roadway Construction Noise Model version 1.00 (RCNM) and the FHWA Construction Noise Handbook. The RCNM was used for calculating construction equipment noise level emissions from the jobsites. In order to develop the analytical model, all relevant parameters, including construction equipment, receiver locations, existing shielding and existing terrain in the area of potential impact, were used in order to predict expected construction noise levels. It is important to note that at the time this report was prepared, the exact inventory of equipment to be used during construction was not known and assumptions were made in order to calculate predicted construction noise emissions. Predicted construction airborne noise emissions are presented in the following table.

**Table 27. Estimated Maximum Construction Noise Levels and Impacts** 

Receiver/ Measurement Site	Project Location No.	Distance from Site Boundary (feet)	Daytime Existing Worst Hour Noise Level Leq <sub>avg</sub>	Nighttime Existing Worst Hour Noise Level Leq <sub>avg</sub>	Predicted Construction Noise Level Leq <sub>max</sub>	Daytime Existing Maximum Noise Level Leq <sub>max</sub>	Nighttime Existing Maximum Noise Level Leq <sub>max</sub>	Predicted Construction Maximum Noise Level Leq <sub>max</sub>	Potential Caltrans Noise Exceedance without Mitigation?
R1	2	25	76 dBA	72 dBA	87 dBA	92 dBA	90 dBA	90 dBA	Yes
R2	18034 W. Coastline Drive	200	61 dBA	57 dBA	74 dBA	77 dBA	73 dBA	70 dBA	No
R3	3	25	74 dBA	72 dBA	86 dBA	90 dBA	88 dBA	90 dBA	Yes
R4	4/6	25	70 dBA	65 dBA	86 dBA	86 dBA	84 dBA	90 dBA	Yes
R5	16	25	71 dBA	67 dBA	87 dBA	87 dBA	86 dBA	90 dBA	Yes
R6	16	185	55 dBA	50 dBA	70 dBA	68 dBA	68 dBA	73 dBA	No

The FHWA has compiled data on the noise-generating characteristics of specific types of construction equipment. Noise levels generated by heavy equipment can range from approximately 70 dBA to noise levels of up to 101 dBA when measured at a distance of 50-feet from the noise source. The noise levels diminish rapidly with distance at a rate of approximately 6 to up to 9 dBA per doubling of distance for acoustically hard and soft sites, respectively. An example of an acoustically hard site would be a parking lot while an acoustically soft site would be a park or any area with lawn or grassy surface. Assuming an acoustically hard site, a noise level of 75 dBA measured at 50 feet from the noise source would be reduced to 69 dBA at 100 feet and to 63 dBA at 200 feet. Construction noise levels at receptors would tend to vary based on the location of construction activity and the amount of equipment in operation. The project would involve the use of multiple pieces of construction equipment. However, the equipment would not all be in use at the same location because of physical space and safety considerations. For the purposes of this analysis, the maximum and average construction noise levels were estimated and used for determining the expected ambient noise levels during construction.

**Short-Term Construction Impacts.** Construction of the project would require site preparation, trenching, breaking and removal of existing roadway sections, compaction and roadway paving, material loading, unloading and hauling. These activities typically involve the use of heavy equipment, such as dump trucks, excavators, backhoes and loaders as well as smaller but noisy gear such as concrete saws and jackhammers. While construction would be temporary, the use of these types of equipment could generate noise that would be heard in the nearby residential areas adjacent to the project locations close to residential areas. Once the project is complete, ambient noise levels will return to existing conditions since the project does not include any capacity or speed increasing design features.

Construction noise levels at receptors would tend to vary based on the location of construction activity and the number of equipment being simultaneously used. The construction process requires the simultaneous use of several pieces of equipment, many of which move around the site and change their operating conditions on a day-by-day and hour-by-hour basis. The typical site-wide sound levels for the construction phases are difficult to estimate since they depend on factors such as number and type of equipment being used and construction phase being performed.



As shown in Table 27, the loudest construction activities are predicted to generate noise levels of up to 86 dBA  $Leq_{avg}$  and 90 dBA  $L_{max}$  at the sensitive land uses. In order to estimate construction related noise levels at the sensitive receptors located in the vicinity of the various proposed drainage restoration locations, FHWA's RCNM was used to perform the calculations. As previously stated, noise levels diminish rapidly with distance at a rate of approximately 6 to up to 9 dBA per doubling of distance for acoustically hard and soft sites, respectively. This attenuation is taken into account by the RCNM software. For the purposes of determining the maximum construction noise levels, the distance is based on the locations of the noise-sensitive receptors and the nearest estimated location where heavy equipment will be operating.

As shown in Table 27, construction would have a potentially temporary adverse construction noise impact at nearby noise sensitive receptors. Noise control measures are recommended to require the use of a feasible sound control plan and to schedule construction activities consistent with Section 8.24.050 of the City of Malibu Municipal Code. Compliance and implementation of the recommended noise control measures discussed later would reduce short-term construction noise levels.

**Long-Term Operational Impacts.** There are no expected operational noise impacts from the proposed project. As previously stated, the project will not add traffic volume capacity, increase speed, or remove any existing shielding from the roadway. However, since is expected that traffic noise will decrease due to reduced speeds for the construction zone, residents within the construction zone area may perceive that noise has increased after project completion when traffic volume and speed return to before-project conditions.

**Predicted Construction Noise**. As estimated for this project, equipment involved in construction is expected to generate noise levels ranging from 86 to 90 dBA at a distance of 50 feet, for each single piece of equipment. Noise emissions from multiple construction equipment would increase by approximately 3 dBA for each additional piece of equipment that generates the same noise. The increase would be 2 dBA if the sources differ by 2 or 3 dBA, and 1 dBA increase would result when the equipment producing noise differs by 4 to 9 dBA. The construction equipment noise would decrease a rate of about 6 dBA per doubling of distance. The predicted construction activity noise levels have been calculated using assumptions in regards to type, quantity and location of equipment. Actual noise levels could differ from predictions. Normally, construction noise levels should not exceed 86 dBA (L<sub>max</sub>) at a distance of 50 feet and vibration levels should remain below 0.2 in/sec at 25 feet. Implementing the following measures would minimize temporary construction noise impacts:

- 1. Provide public outreach and project schedule information:
  - a. Prior to construction, all residences within 1,000 feet of the site shall be individually notified of the project's construction schedule.
  - b. Prior to construction, a sign should be posted on the site that is legible from at least 50 feet off-site. The sign should include a telephone number that residents can call to inquire about the construction process and to register complaints. The contractor or Caltrans Resident Engineer in charge should designate a "noise control coordinator" who will reply to all construction noise-related questions and complaints.
- Equipment noise control is needed to reduce the noise emissions from construction sites by mandating a specified noise level for design of new equipment, and updating old equipment with new noise control devices and techniques presented below:
  - a. Mufflers are very effective devices which reduce the noise emanating from the intake or exhaust of an engine, compressor, or pump. The fitting of effective mufflers on all new equipment and retrofitting of mufflers on existing equipment is necessary to yield an immediate noise reduction at all types of road construction sites.
  - b. Sealed and lubricated tracks for crawler mounted equipment will lessen the sound radiated from the track assembly resulting from metal to soil and metal to metal contact. Contractors, site engineers, and inspectors should ensure that the tracks are kept in excellent condition by periodic maintenance and lubrication
  - c. Lowering exhaust pipe exit height closer to the ground can result in an off-site noise reduction. Barriers are more effective in attenuating noise when the noise source is closer to ground level.
  - d. General noise control technology can have substantially quieter construction equipment when manufacturers apply state-of-the-art technology to new equipment or repair old equipment to maintain original equipment noise levels.



- 3. In—use site noise control is necessary to prevent existing equipment from producing noise levels in excess of specified limits. Any equipment that produces noise levels less than the specified limits would not be affected. However, those exceeding the limit would be required to meet compliance by repair, retrofit, or replacement. New equipment with the latest noise sensitive components and noise control devices are generally quieter than older equipment, if properly maintained and inspected regularly. They should be repaired or replaced if necessary, to maintain the in-use noise limit. All equipment applying the in-use noise limit would achieve an immediate noise reduction if properly enforced.
- 4. Site restrictions should be applied to achieve noise reduction through different methods, resulting in an immediate reduction of noise emitted to the community without requiring any modification to the source noise emissions. The methods include shielding with barriers for equipment and site, truck rerouting and traffic control, time scheduling, and equipment relocation. The effectiveness of each method depends on the type of construction involved and the site characteristics.
  - a. Shielding with temporary barriers or noise blankets should be implemented at an early stage of a project to reduce construction equipment noise. The placement of barriers must be carefully considered to reduce limitation of site access. Barriers may be natural or man-made, such as excess land fill used as a temporary berm strategically placed to act as a barrier or any of the commercially available temporary construction barriers or noise blankets.
  - b. Efficient rerouting of trucks and control of traffic activity on construction site will reduce noise due to vehicle idling, gear shifting and accelerating under load. Planning proper traffic control will result in efficient workflow and reduce noise levels. In addition, rerouting trucks does not reduce noise levels but transfers noise to other areas that are less sensitive to noise.
  - c. Time scheduling of activities should be implemented to minimize noise impact on exposed areas. Local activity patterns and surrounding land uses must be considered in establishing site curfews. However, limiting working hours can decrease productivity. Sequencing the use of equipment with relatively low noise levels versus equipment with relatively high noise levels during noise sensitive periods is an effective noise control measure.
  - d. Equipment location should be as far from noise sensitive land use areas as possible. The contractor should substitute quieter equipment or use quieter construction processes at or near noise sensitive areas.
- 5. Educating contractors and their employees to be sensitive to noise impact problems and noise control methods. The contractor and all subcontractors shall be knowledgeable about the details of Chapter 8.24 'Noise" of the Malibu Municipal Code and the noise control requirements included in the project's Standard Specifications and Special Provisions, and shall conform to its requirements at all times. This may be one of the most cost-effective ways to help operators and supervisors become more aware of the construction site noise problem and to implement the various methods of improving the conditions. A training program for equipment operators is recommended to instruct them in methods of operating their equipment to minimize environmental noise. Many training programs are presently given on the subject of job safety. This can be extended to include the impact due to noise and methods of abatement.

**Summary and Conclusion.** Construction noise and vibration impact analysis for the sensitive land uses in the vicinity of the project has determined that construction activities for the proposed drainage restoration project will cause noticeable ambient noise level increases at the sensitive land uses adjacent to the various site locations. Construction noise may also sporadically and frequently exceed established local jurisdiction noise policy criteria and Caltrans construction noise thresholds.

#### 9. ENVIRONMENTAL DETERMINATION

Analysis of the proposed project's relationship to the surrounding environment, the final approved environmental document and its impacts, and the ensuing environmental reevaluation provide the basis for the following determinations:

- a. The circumstances surrounding the project remain essentially the same as they were when the final IS/EA was considered and approved.
- b. The area's social, economic, and environmental setting remains essentially the same as when the IS/EA for the State Route 1 (Pacific Coast Highway) Drainage Rehabilitation and Bridge Replacement at Solstice Canyon Creek Project was written.



In addition, the environmental review, consultation, and any other action required in accordance with applicable Federal Laws for this project is being, or has been, carried out by the State of California Department of Transportation under its assumption of responsibility pursuant to 23 U.S.C. 327.

11/30/2022

Date

This Addendum/Environmental Reevaluation to the Mitigated Negative Declaration is hereby approved pursuant to the California Environmental Quality Act (CEQA) Guidelines Section 15164 which limits its use to minor technical changes or additions in the project scope, impacts, and mitigation measures identified in the preceding pages.

EDUARDO AGUILAR
Senior Environmental Planner/Branch Chief

Senior Environmental Planner/Branch Chief Caltrans District 7 Division of Environmental Planning



#### **NEPA/CEQA RE-VALIDATION FORM**

<b>DIST-CO-RTE</b> : 07-LA-001/07-VEN-001	
<b>PM/PM:</b> LA PM 37.6/62.86 / VEN PM 0.0/0.92	
EA or Fed-Aid Project No.: EA 07-31350	
Other Project No. (specify): E-FIS 0715000090	
Project Title: State Route 1 (Pacific Coast Highway) Drainage Rehabilitation	and Bridge Replacement at Solstice
Canyon Creek	
Environmental Approval Type: IS/EA w/MND/FONSI	
<b>Date Approved:</b> 3/29/2019	
Reason for Consultation (23 CFR 771.129), check one:	
□ Project proceeding to next major federal approval	
☐ Change in scope, setting, effects, mitigation measures, requirements	
☐ 3-year timeline (EIS only)	
□ <b>N/A</b> (Re-Validation for CEQA only)	
Description of Changed Conditions:	
Revalidation for change in scope of work (addition of pedestrian undercross	sina at Proiect Location No. 10 (Solstice
Canyon Creek) and for Environmental Certification of 100% PS&E review.	
NEPA CONCLUSION - VALIDITY	
Based on an examination of the changed conditions and supporting inform	ation: (Check ONE of the three
statements below, regarding the validity of the original document/determine	nation (23 CFR 771.129). If document is
no longer valid, indicate whether additional public review is warranted and	whether the type of environmental
document will be elevated.)	
$\hfill\square$ The original environmental document or CE remains valid. No further	documentation will be prepared.
☑ The original environmental document or CE is in need of updating; furt	her documentation has been prepared
and $\square$ is included on the continuation sheet(s) or $\boxtimes$ is attached. Wit	h this additional documentation, the
original ED or CE remains valid.	
Additional public review is warranted (23 CFR 771.111(h)(3))	⊠ No
☐ The original document or CE is no longer valid.	
Additional public review is warranted (23 CFR 771.111(h)(3))	□No
Supplemental environmental document is needed. ☐ Yes ☐ No	
New environmental document is needed. ☐ Yes ☐ No (If "Yes," spec	ify type: )
CONCURRENCE WITH NEPA CONCLUSION	
I concur with the NEPA conclusion above.	
4	
$\mathcal{M}$	11/30/2022
	11/30/2022
Signature: Eduardo Aguilar, Environmental Branch Chief	Date
$O$ $A$ $\cdot$ $A$ $\cdot$	
Calvin Liu	11/30/2022
Signature: Calvin Liu, Project Manager	Date
Signature. Culvin Liu, i roject ivianagei	Date



#### **CEQA CONCLUSION** (Only mandated for projects on the State Highway System.)

Based on an examination of the changed conditions and supporting information, the following conclusion has been reached regarding appropriate CEQA documentation: (Check ONE of the five statements below, indicating whether any additional documentation will be prepared, and if so, what kind. If additional documentation is prepared, attach a copy of this signed form and any continuation sheets.)

attach a copy of this signed form and any continuation sheets.)	
$\hfill\square$ Original document remains valid. No further documentation is	necessary.
☑ Only minor technical changes or additions to the previous docur been or will be prepared and is ☐ included on the continuation circulated for public review. (CEQA Guidelines, §15164)	•
☐ Changes are substantial, but only minor additions or changes are adequate. A Supplemental environmental document will be p review. (CEQA Guidelines, §15163)	
☐ Changes are substantial, and major revisions to the current documentionmental document will be prepared, and it will be circul §15162)  (Specify type of subsequent document, e.g., Subsequent FEIR):	
☐ The CE is no longer valid. New CE is needed. ☐ Yes ☐ No	
· ·	
CONCURRENCE WITH CEQA CONCLUSION	
I concur with the CEQA conclusion above.	
E Agrifar	11/30/2022
Signature: Eduardo Aguilar, Environmental Branch Chief	Date
Calvin Liu	11/30/2022
Signature: Calvin Liu, Project Manager	Date



#### CONTINUATION SHEET(S)

#### Changes in project design, e.g., scope change; a new alternative; change in project alignment.

In May of 2021, Caltrans Design began preliminary work on plans for a pedestrian undercrossing at Project Location No. 10 (Solstice Canyon Creek) in response to a request for public beach access from property owners adjacent to the project area. The property owners of the Calamigos Beach Club Restaurant at 26025 Pacific Coast Highway, Malibu, CA 90265 (which is part of the Calamigos Guest Ranch located at 327 Latigo Canyon Road, Malibu, CA 90265), requested that Caltrans consider providing walkway access underneath the newly proposed bridge structure at Solstice Canyon Creek from the restaurant to the beach as part of the proposed undertaking. While the intention of the request was to provide an undercrossing walkway to allow restaurant patrons access to the beach without having to cross the Pacific Coast Highway (PCH) roadway, its use was expanded to allow general public access (not specific to the adjacent property), which is more consistent with City of Malibu and California Coastal Commission goals of providing greater beach access to the general public.

By July 2022, final design, and accompanying hydraulic analysis, and grading plans were achieved and plans/layouts specific to this proposed project location are appended to this addendum/reevaluation as Attachment A. In general, PCH runs north-to-south, but in the area of Project Location No. 10 (Solstice Canyon Creek), the roadway traverses the coastline in an east-to-west direction. The proposed pedestrian undercrossing is designed as a cantilevered concrete walkway so as not to impede hydraulic flow of the creek and would be attached to the eastern wing wall structure, perpendicular and beneath the newly proposed bridge.

Proposed pedestrian access to the undercrossing on the north side of the proposed bridge structure would originate at the roadway and be located approximately 70 feet north-west of the existing Calamigos Restaurant driveway. Southern access would start at the flat area on the embankment of the roadway just southeast of the proposed bridge structure. The walkway would be constructed parallel to the embankment [supported on evenly spaced Cast-In-Drilled-Hole (CIDH) piles] before connecting with the southern end of the cantilevered pedestrian undercrossing structure. In a review of the proposed pedestrian undercrossing plans/layouts, the following additional elements were implemented:

- To comply with ADA standards, the proposed pedestrian undercrossing structure would have a minimum width of 6 feet and a maximum slope grade of 8.33%.
- The proposed pedestrian undercrossing structure would be constructed above the 50-year flood zone line to ensure flood events do not cause damage to the facility.
- The proposed pedestrian undercrossing structure would be attached via cantilever method to the eastern wingwall and bridge abutments. Along the bridge structure, the walkway would be constructed approximately 10 feet above the channel bottom. Cable railings and lighting would be installed for pedestrian safety.

Changes in environmental setting, e.g., new development affecting traffic or air quality.  $\ensuremath{\text{N/A}}$ 

Changes in environmental circumstances, e.g., a new law or regulation; change in the status of a listed species. N/A

Changes to environmental impacts of the project, e.g., a new type of impact, or a change in the magnitude of an existing impact.

In addition to the structural excavation work previously evaluated at Project Location No. 10 (Solstice Canyon Creek), grading plans for fish passage within Solstice Canyon Creek were finalized within the footprint of the proposed new bridge structure and the beachside area just south of the project area. Final proposed grading plans



include construction of an engineered stream bed along the creek that would be composed of a 6-inch-thick top sand layer and an 8-inch-thick Class II Rock Slope Protection (RSP) bottom layer. The graded limits would extend approximately 50 feet downstream of the southern proposed bridge structure limits, and 125 feet upstream from the northern proposed bridge structure limits. In order to protect upstream embankments against erosion, Class XI RSP would be installed to extend approximately 40 feet upstream of the proposed bridge structure wingwalls. During construction, a water diversion plan would be implemented to temporarily redirect any flow within Solstice Canyon Creek via a pipe bypass method. During redirection of creek flow, the pipe on the southern portion would be placed on top of plywood sheets to minimize disturbance of the sand surface on the beach side and within an Environmentally Sensitive Area (ESA) in terms of Archaeology. Full proposed construction details can be referenced in the plans/layouts specific to this proposed project location as appended to this addendum/reevaluation as Attachment A.

Changes to avoidance, minimization, and/or mitigation measures since the environmental document was approved.

As part of biological mitigation for the proposed undertaking, a total of 8 acres of revegetation with native plantings is proposed at Project Location No. 19 (VEN PM 0.92). Adjacent parcels 81360-1 and 81875-1 will be granted for these purposes by the State of California – Department of Parks and Recreation, located along PCH/SR-1 between VEN PM 0.903 and 0.824. Planting will be implemented south of the PCH roadway on the beach side and will consist of brush removal (by hand) and the manual excavation of several holes (12-inches-deep) to accommodate 1-gallon size native plantings.

Changes to environmental commitments since the environmental document was approved, e.g., the addition of new conditions in permits or approvals. When this applies, append a revised Environmental Commitments Record (ECR) as one of the Continuation Sheets.

Minor changes in environmental commitments as they pertain to Biology, Cultural/Archaeological Resources, Hazardous Waste, Geology, Hydraulics, and Noise. Reference Environmental Commitments Record (ECR) in Attachment B for details.

STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION

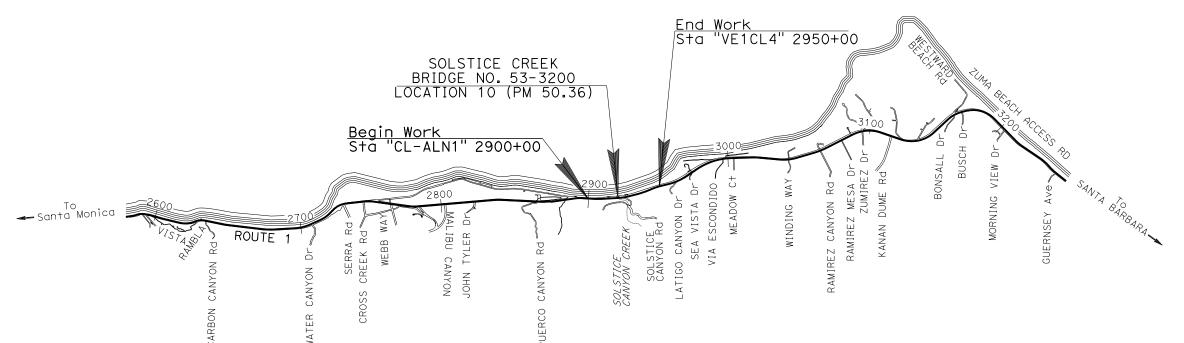
# PROJECT PLANS FOR BUILDING CONSTRUCTION STATE HIGHWAY

IN LOS ANGELES COUNTY AT SOLSTICE CREEK BRIDGE

TO BE SUPPLEMENTED BY STANDARD PLANS DATED 2018

PACIFIC OCEAN

# ATTACHMENT A - PLANS/LAYOUTS FOR PROJECT LOCATION NO. 10 (SOLSTICE CANYON CREEK)



MALIBU

THE CONTRACTOR SHALL POSSESS THE CLASS (OR CLASSES) OF LICENSE AS SPECIFIED IN THE "NOTICE TO BIDDERS."

INDEX OF PLANS

LAYOUTS

PROFILE

REVISED STANDARD PLANS

STRUCTURE PLANS

UTILITY PLANS

TITLE AND LOCATION MAP TYPICAL CROSS SECTIONS

KEY MAP LINE INDEX PROJECT CONTROL

CONSTRUCTION DETAILS

DRAINAGE PLANS, PROFILES,

CONSTRUCTION AREA SIGNS

SUMMARY OF QUANTITIES

STAGE CONSTRUCTION PLANS, DETAILS, QUANTITIES

PAVEMENT DELINEATION AND SIGN PLAN,

EROSION CONTROL PLANS, QUANTITIES

ELECTRICAL PLANS AND QUANTITIES

SOLSTICE CREEK BRIDGE PLANS

THE STANDARD PLANS LIST APPLICABLE TO THIS CONTRACT IS INCLUDED IN THE NOTICE TO BIDDERS AND SPECIAL PROVISIONS BOOK

DETAILS AND QUANTITIES

SHEET No. DESCRIPTION

8-14

15-29

30-44

46-65 66-68

69-70

71-73 74-79

80-124

45

Madhan Cin PROJECT ENGINEER REGISTERED CIVIL ENGINEER PLANS APPROVAL DATE

CIVIL THE STATE OF CALIFORNIA OR ITS
OFFICERS OR AGENTS SHALL NOT BE
RESPONSIBLE FOR THE ACCURACY OR
COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET.

07-313504 CONTRACT No.

05-28-21

37.7/62.9, 0.0/0.9

07 LA, Ven

SISKIYOU

LOCATION MAP

LASSEN

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SAN BERNARDING

RIVERSIDE

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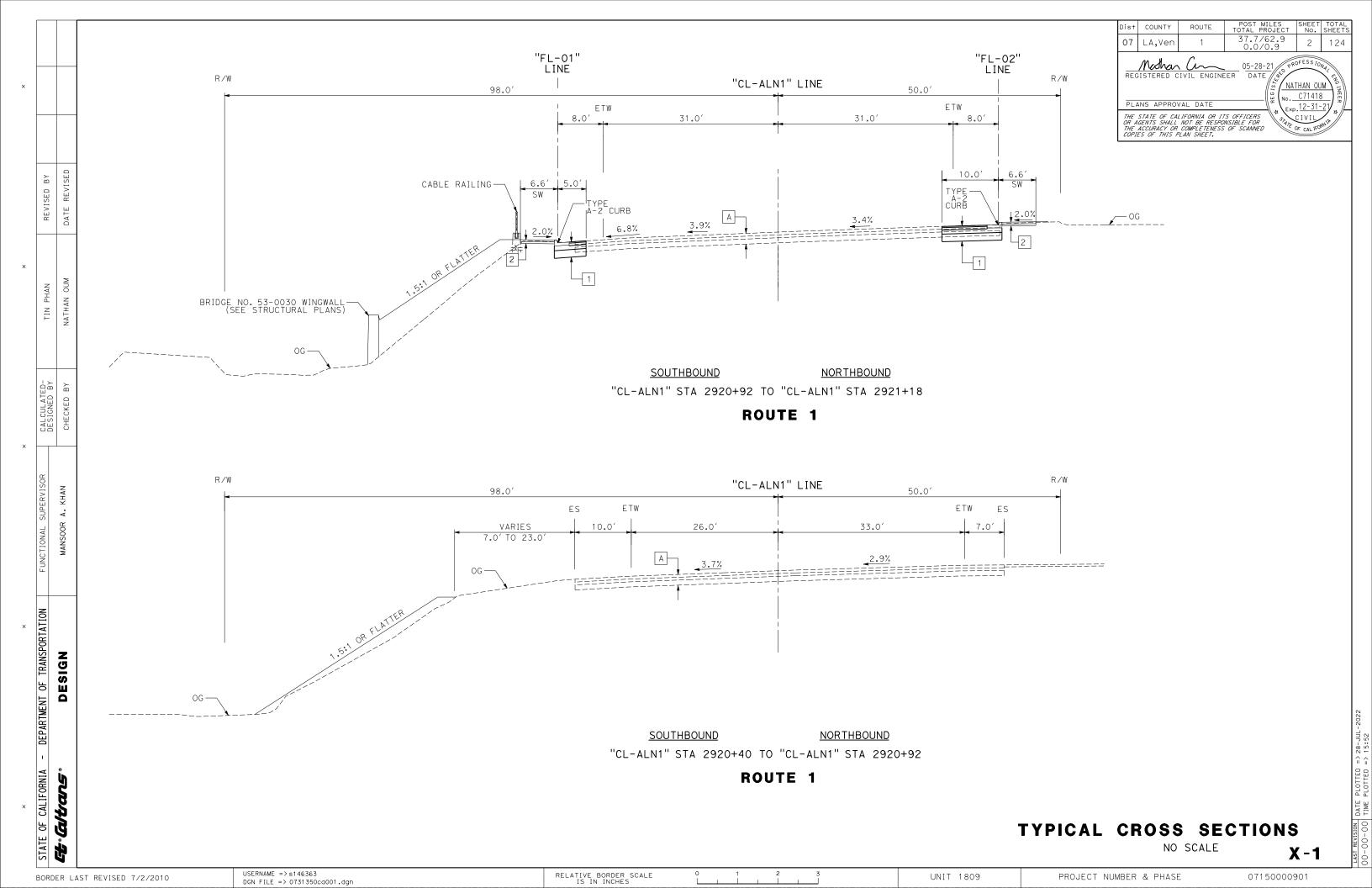
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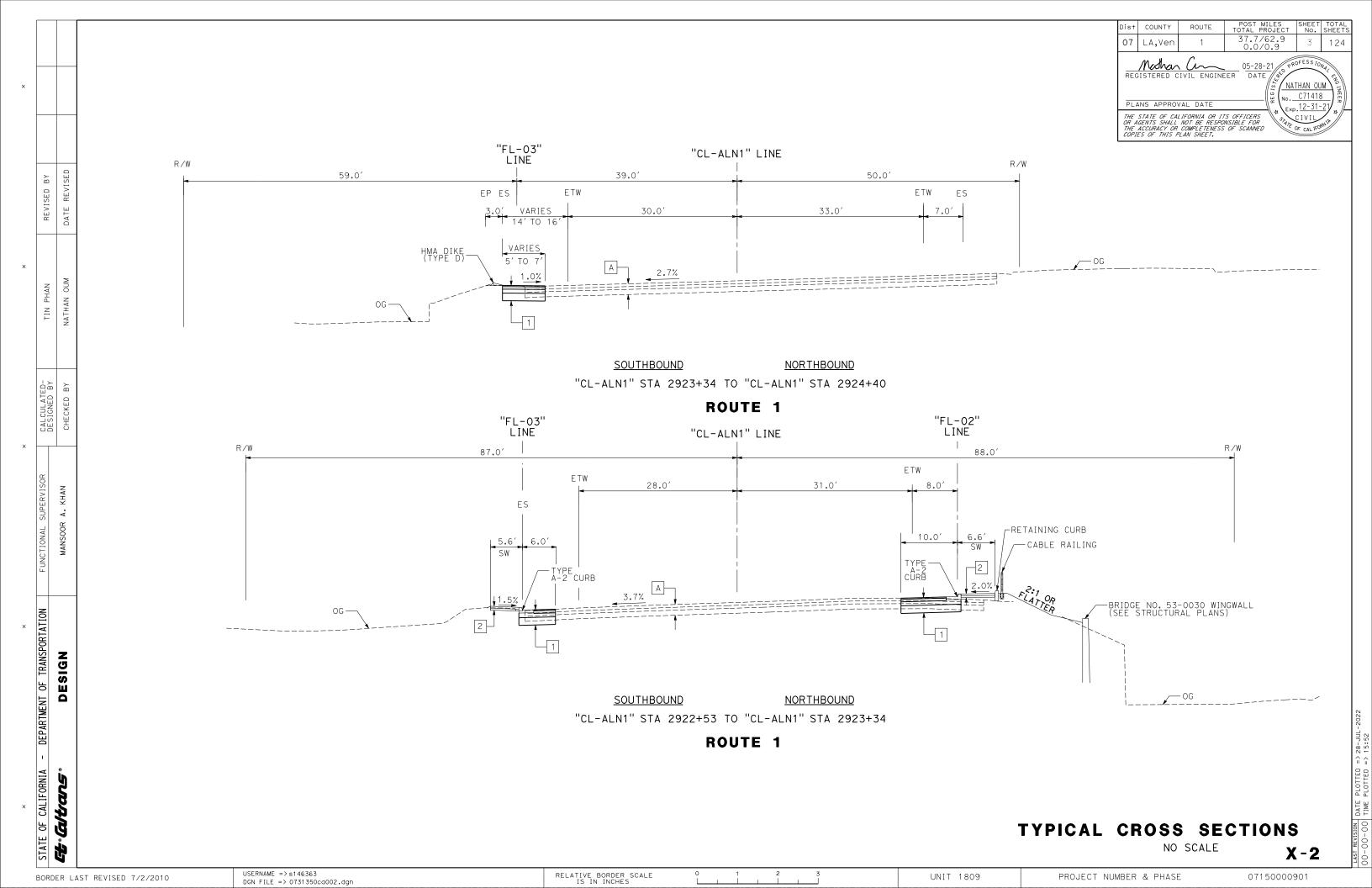
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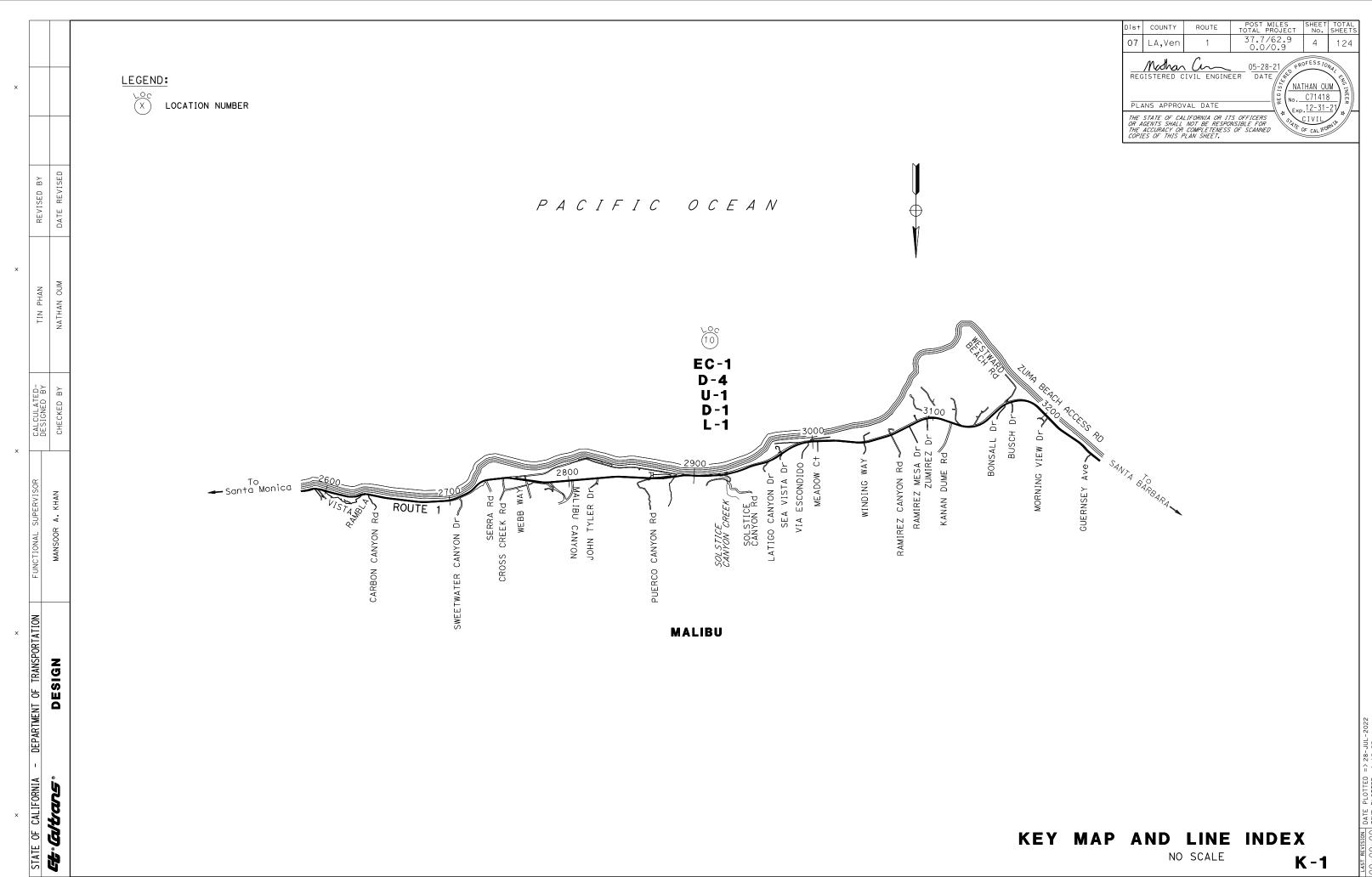
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UNIT 1809 | PROJECT NUMBER & PHASE 07150000901

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RELATIVE BORDER SCALE
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UNIT 1809
PROJECT NUMBER & PHASE

ASE 07150000901

NOTES: Dist COUNTY ABBREVIATIONS: 37.7/62.9 0.0/0.9 07 LA,Ven FOR COMPLETE PROJECT CONTROL DATA, SEE THE SURVEY RECORDS (SR18-142, SR18-143, AND SR18-144) PΚ PARKER-KALON SURVEY NAIL ON THE FILE IN THE SURVEYS DEPARTMENT AT THE DISTRICT OFFICE. AC ShId ASPHALT CONCRETE SHOULDER TOP OF CURB TC BASIS OF BEARINGS AND COORDINATES: REGISTERED CIVIL ENGINEER DATE 2. FD FOUND BEARINGS AND COORDINATES FOR THIS PROJECT ARE BASED ON THE CALIFORNIA COORDINATE SYSTEM OF 1983, HPGN EPOCH ADJUSTMENT [CCS 83 (1991.35)], ZONE 5, U.S. SURVEY FOOT. XX/XX/XX PLANS APPROVAL DATE LEGEND: BASIS OF ELEVATIONS: THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET. 3. ELEVATION FOR THIS PROJECT ARE BASED ON THE NATIONAL GEODETIC VERTICAL DATUM STATION IN THE NATIONAL Δ OF 1929 (NGVD 29). SPATIAL REFERENCE SYSTEM IN THE EVENT GPS MACHINE CONTROL/GUIDANCE IS USED FOR THIS PROJECT, THE CONTRACTOR SHALL CONTACT AND MEET WITH THE SURVEYS DEPARTMENT AT THE DISTRICT CONTROL FOR DESIGN AND CONSTRUCTION OFFICE TO OBTAIN THE CONTROL NECESSARY TO ESTABLISH A GPS SITE CALIBRATION IN COMPLIANCE WITH THIRD-ORDER SURVEY SPECIFICATION AS DEFINED IN CHAPTER 6, STATION DESIGNATION NORTHING EASTING ELEVATION DESCRIPTION TABLES 6B-1 AND 6B-2 OF THE CALTRANS SURVEY MANUAL. PMHV4130 6,334,600.92 1,834,687.14 46.72 REV] PRHV4131 1,834,597.92 6,334,601.51 48.79 1,834,683.40 6,334,598.98 46.28 1,834,697.89 6,334,740.47 1,835,081.62 6,335,822.02 45.06 34.59 CLHV301 REFR305 1,835,080.69 6,335,822.24 1,835,165.75 6,336,316.74 1,835,285.76 6,336,664.12 REFR304 34.64 SUHV410 PMHV6945 CLHV761 1,835,321.85 6,336,982.27 1,835,382.11 6,337,335.94 1,835,358.10 6,337,594.12 SUHV401 40.50 CLHV308 42.32 1,835,386.71 6,338,075.17 1,835,386.85 6,338,668.71 36.61 21.02 CLHV307 CLHV306 PMHV9542 1,835,393.68 6,339,196.14 18.98 ÆEFR\$05 REFR304-CLHV308 291824. SUHV401 8 CLHV761 ♥PMHV9542 2 3 4 2905 6 LINE /E RIGHT) PRHV4131 CLHV301 2945 6 - PMHV6946 DEPARTMENT OF TRANSPORTATION PMHV4130 CALIFORNIA **Gltans** PROJECT CONTROL 능 NO SCALE

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RELATIVE BORDER SCALE
IS IN INCHES

UNIT 1809

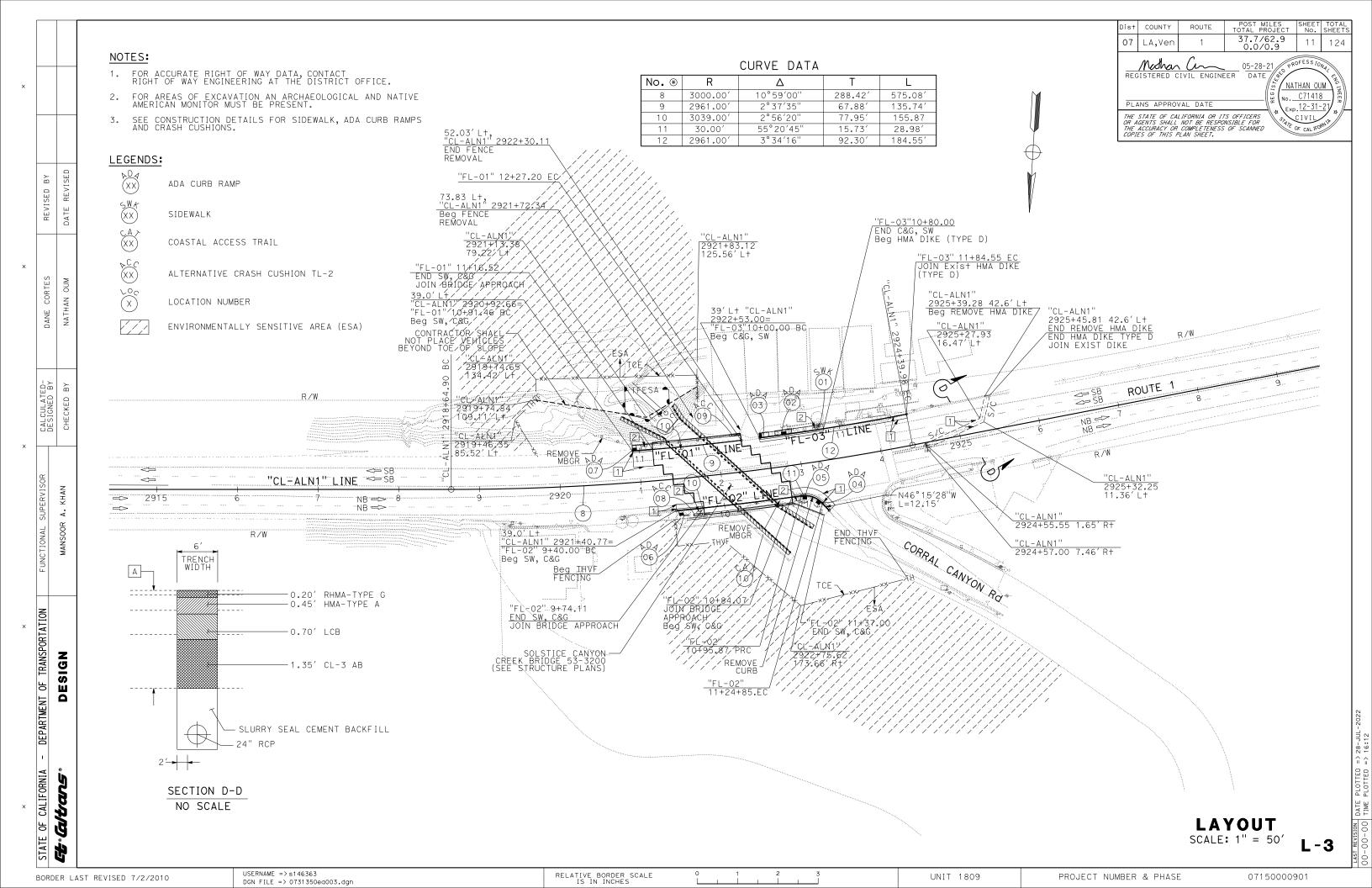
PROJECT NUMBER & PHASE

07150000901

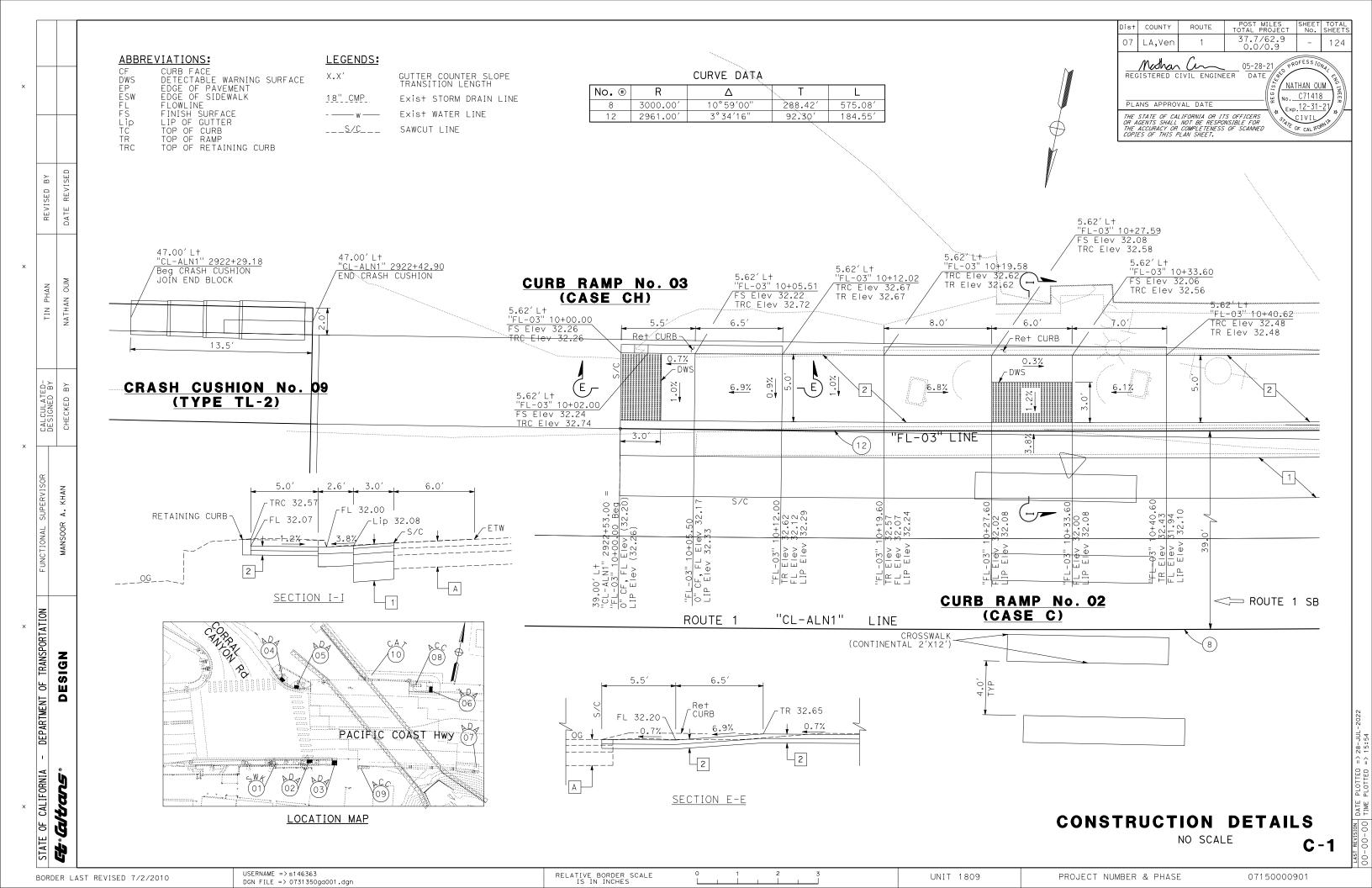
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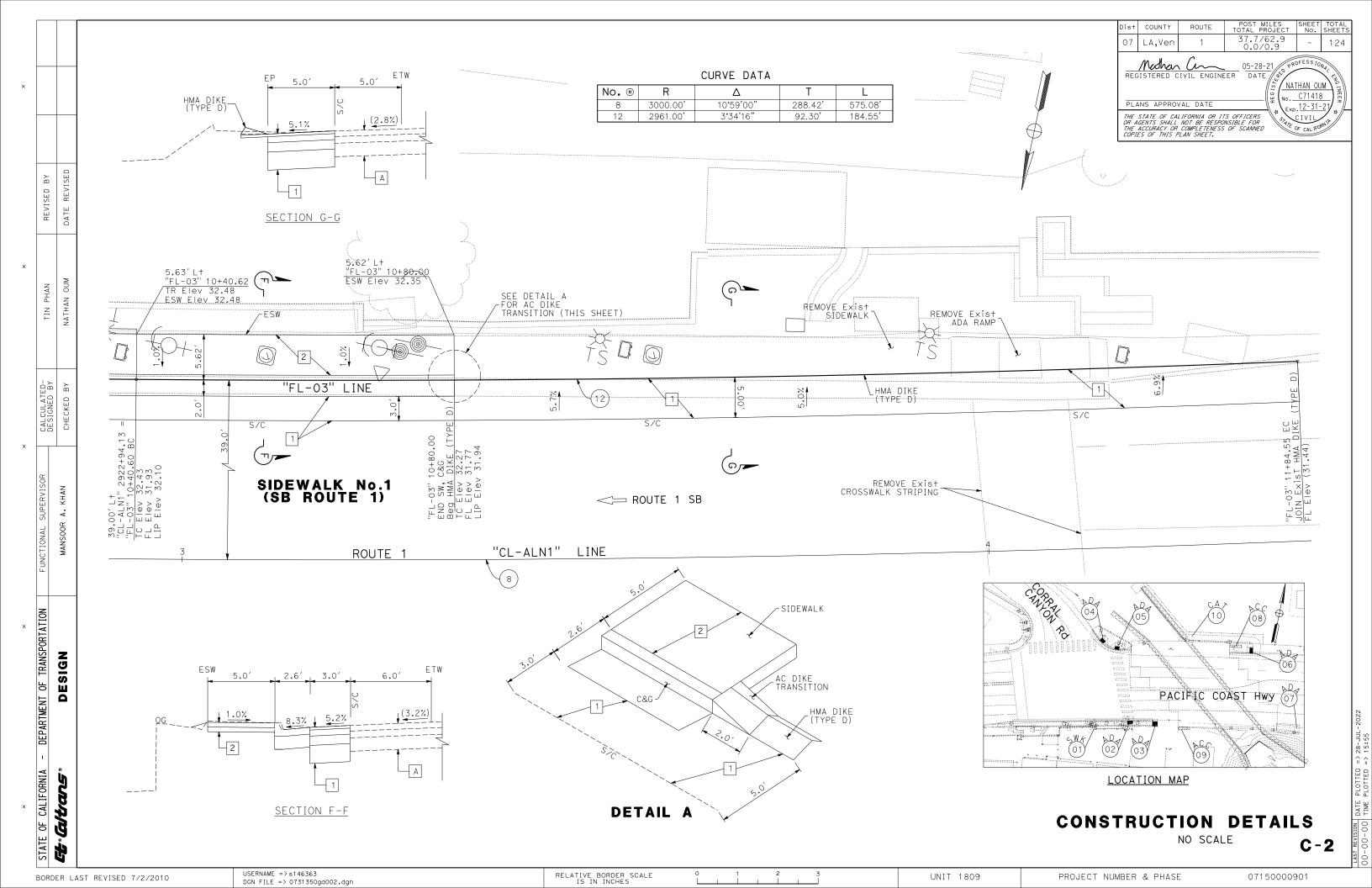
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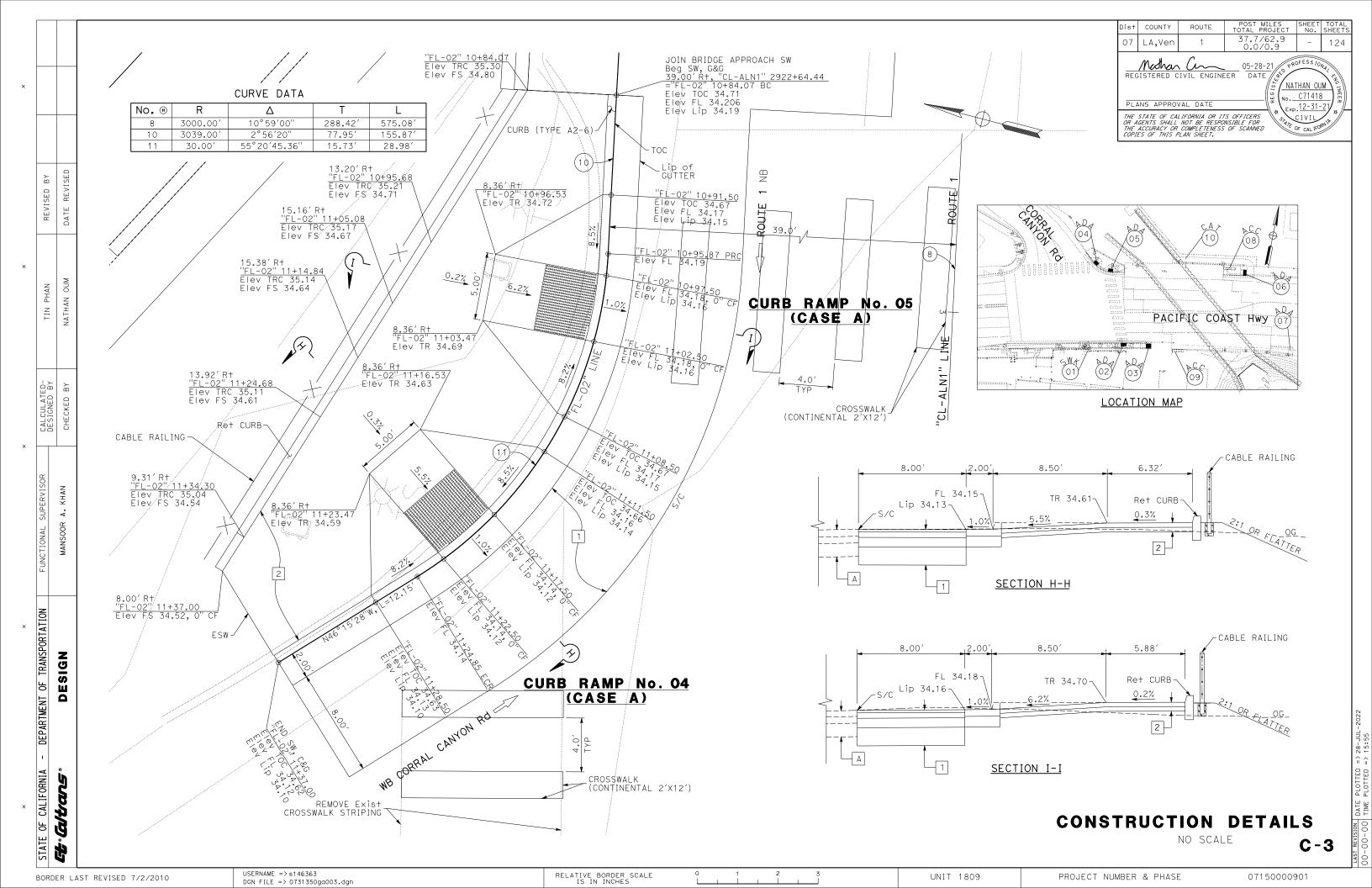
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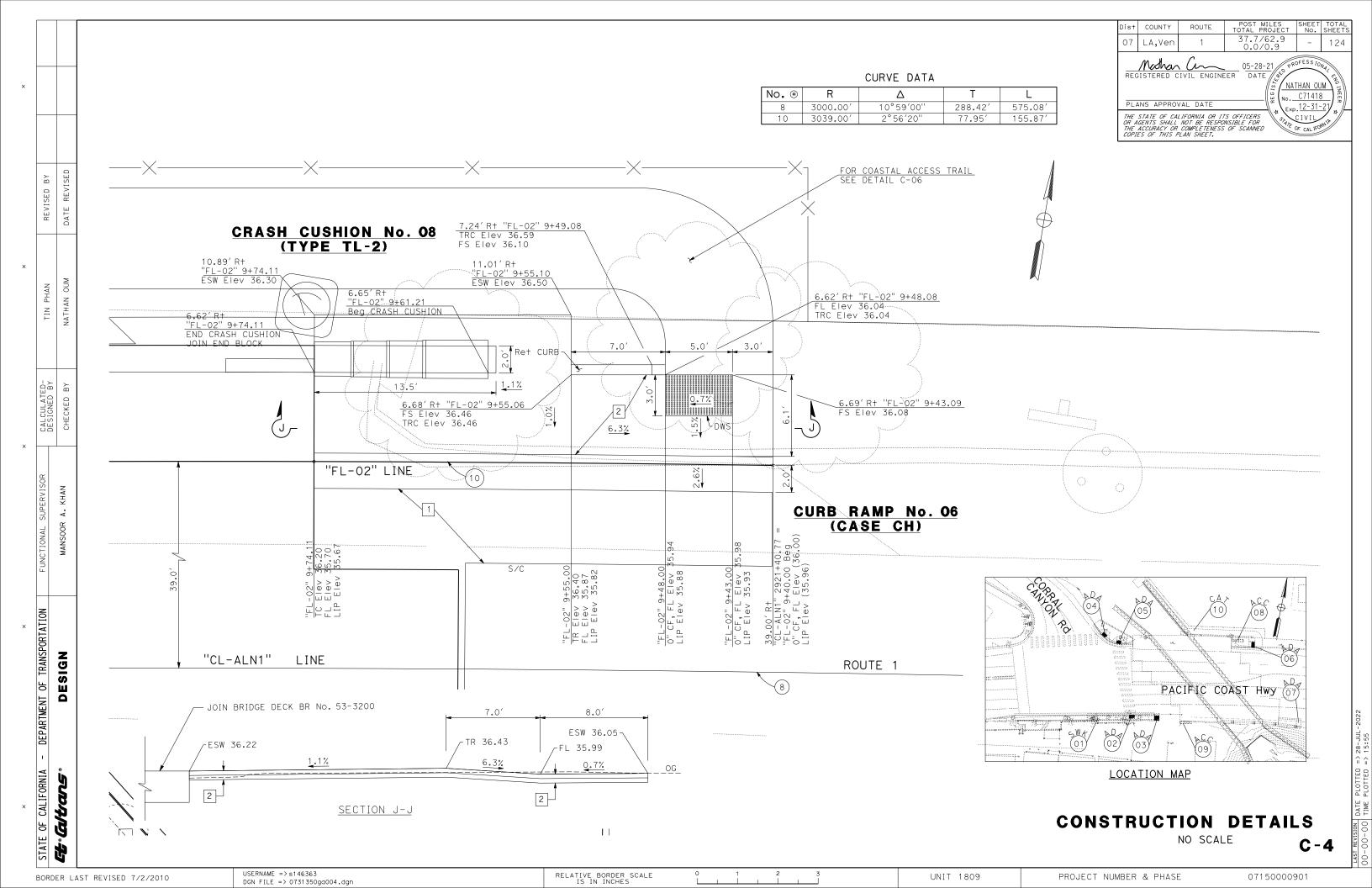


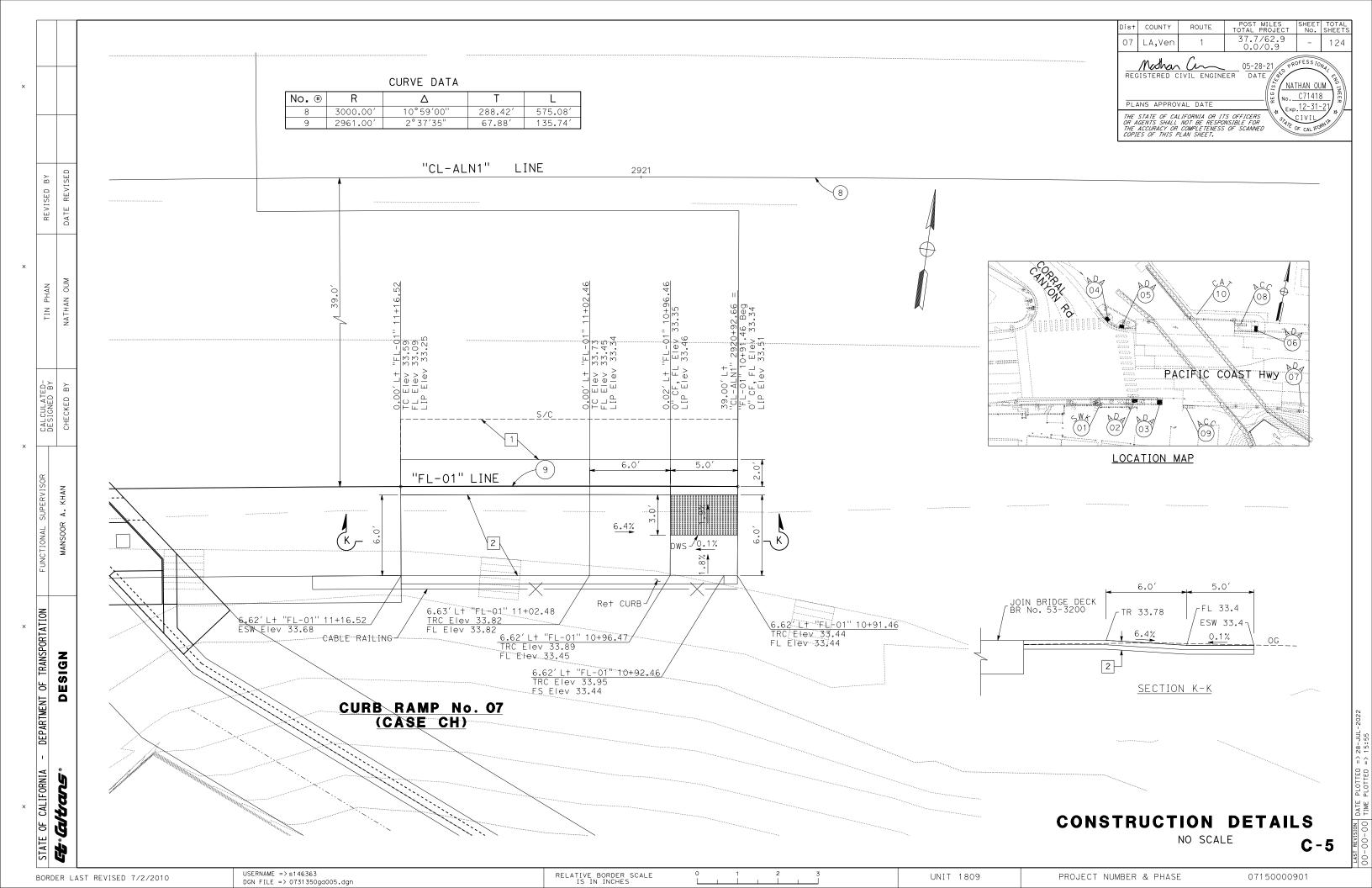


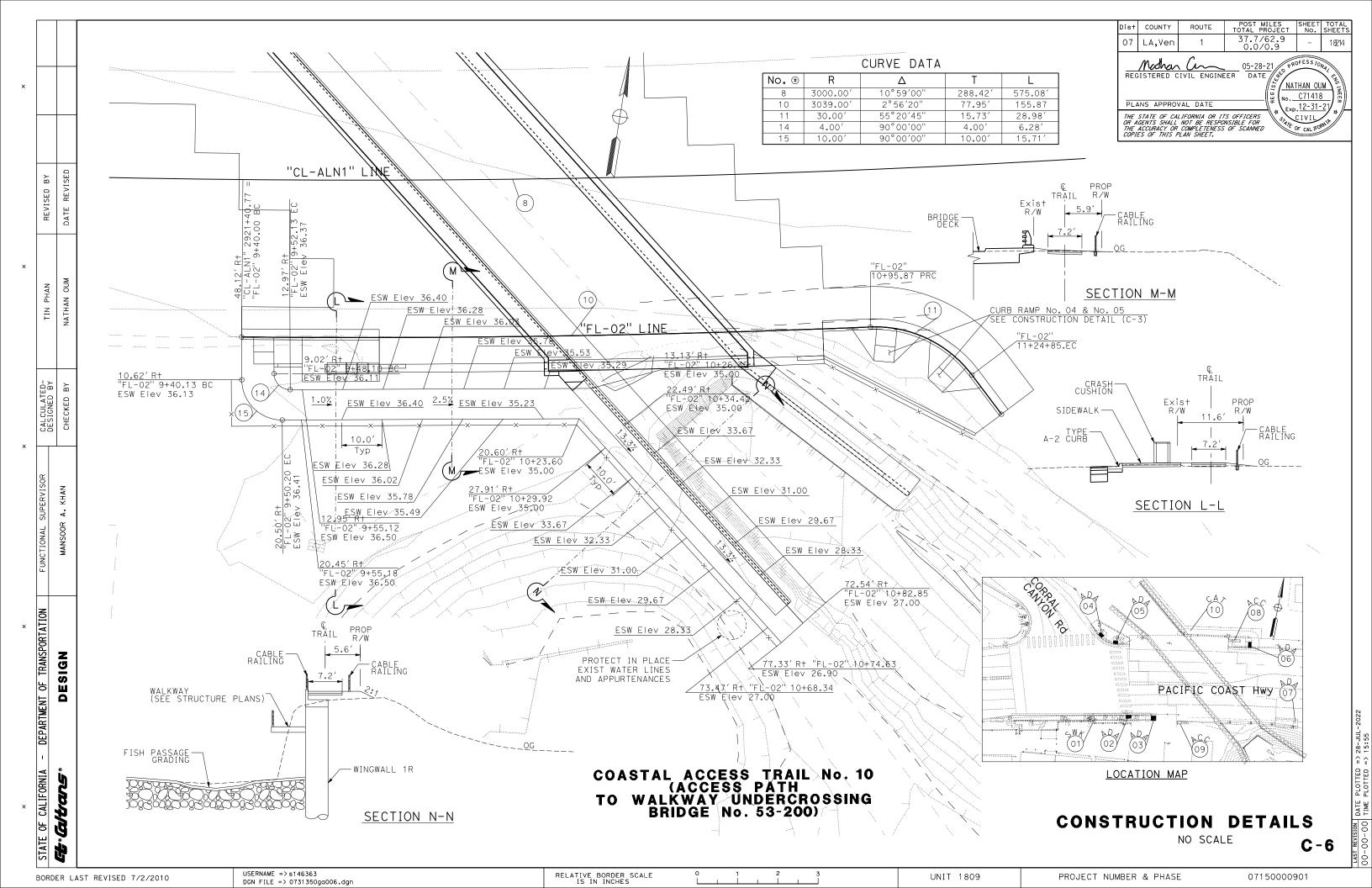


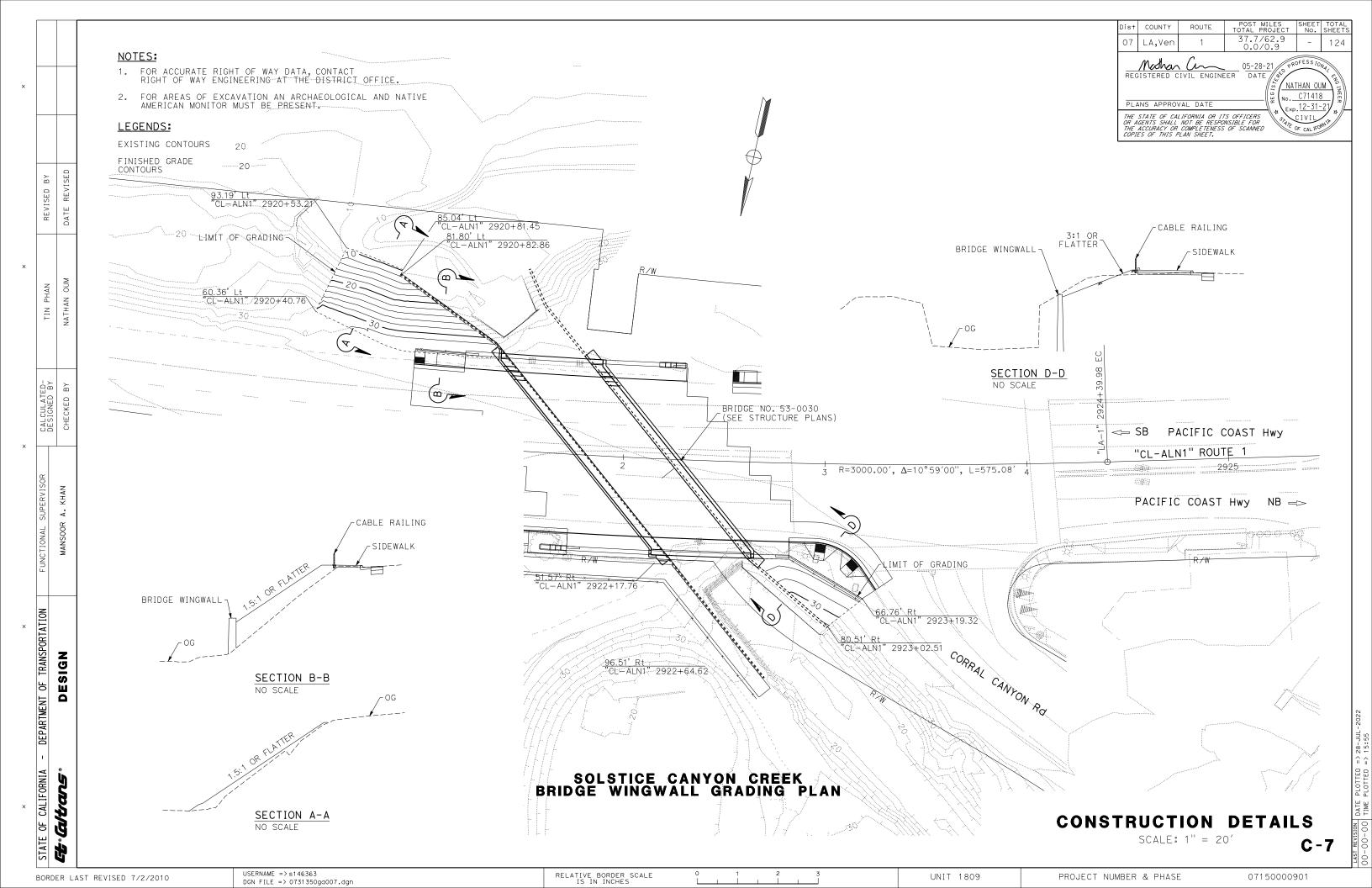


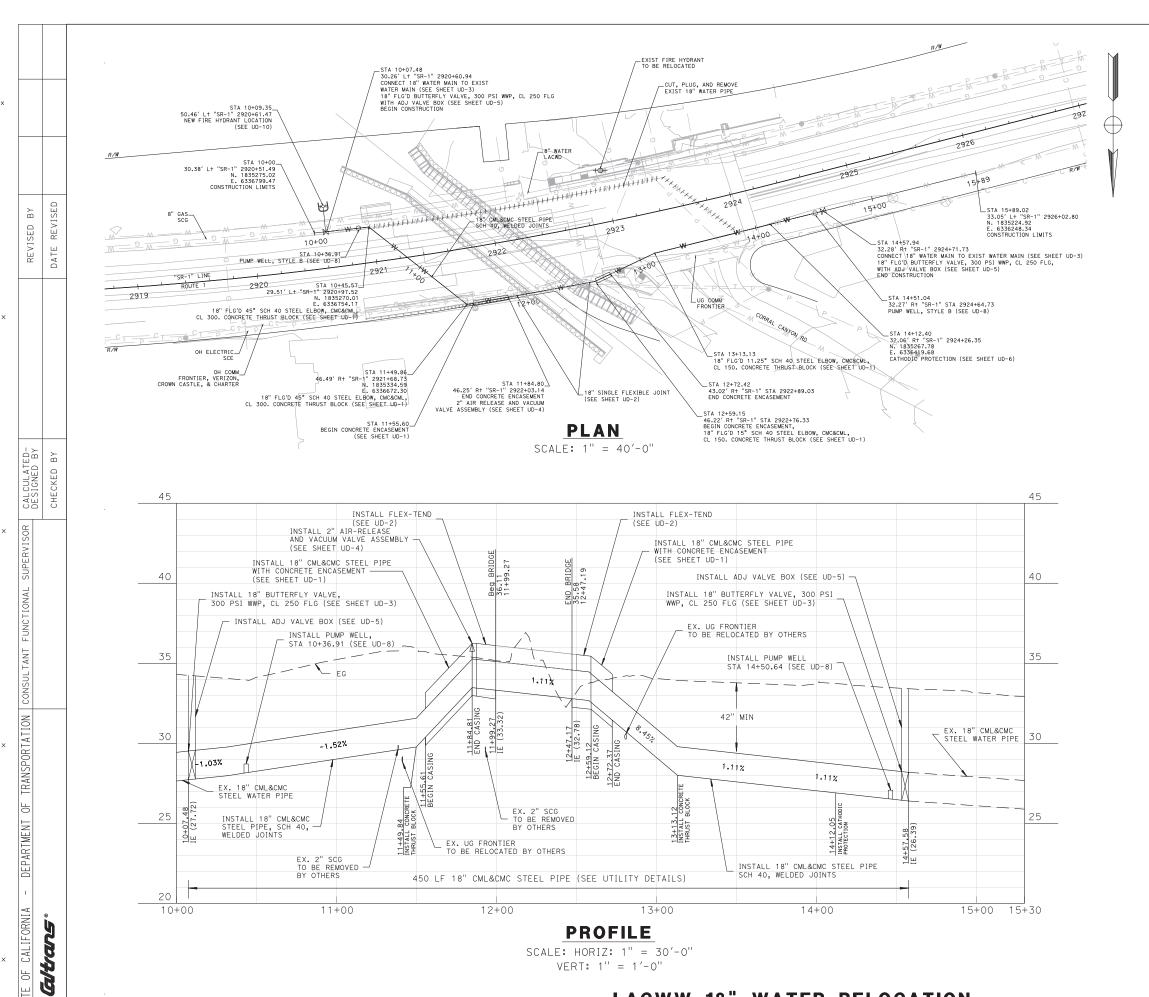












POST MILES SHEET TOTAL TOTAL PROJECT No. SHEETS Dist COUNTY 07 37.7/62.9 01 LΑ REGISTERED CIVIL ENGINEER DATE PLANS APPROVAL DATE THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET, CIVIL

#### NOTES:

- 1. FOR ACCURATE RIGHT OF WAY DATA, CONTACT RIGHT OF WAY ENGINEERING AT THE DISTRICT OFFICE.
- 2. LOCATION OF UTILITY LINES SHOWN ON THESE PLANS ARE APPROXIMATE. THE CONTRACTOR IS TO FIELD VERIFY OR POTHOLE IN ORDER TO DETERMINE EXACT LOCATION.
- 3. REFER TO BRIDGE PLANS FOR UTILITY HANGER DETAILS.
- 4. MAXIMUM HANGER SPACING SHALL BE 15'. HANGERS SHALL BE A MINIMUM OF 2' AWAY FROM PIPE

SOLSTICE CANYON BRIDGE

# UTILITY PLAN

SCALE AS SHOWN

**U-8** 

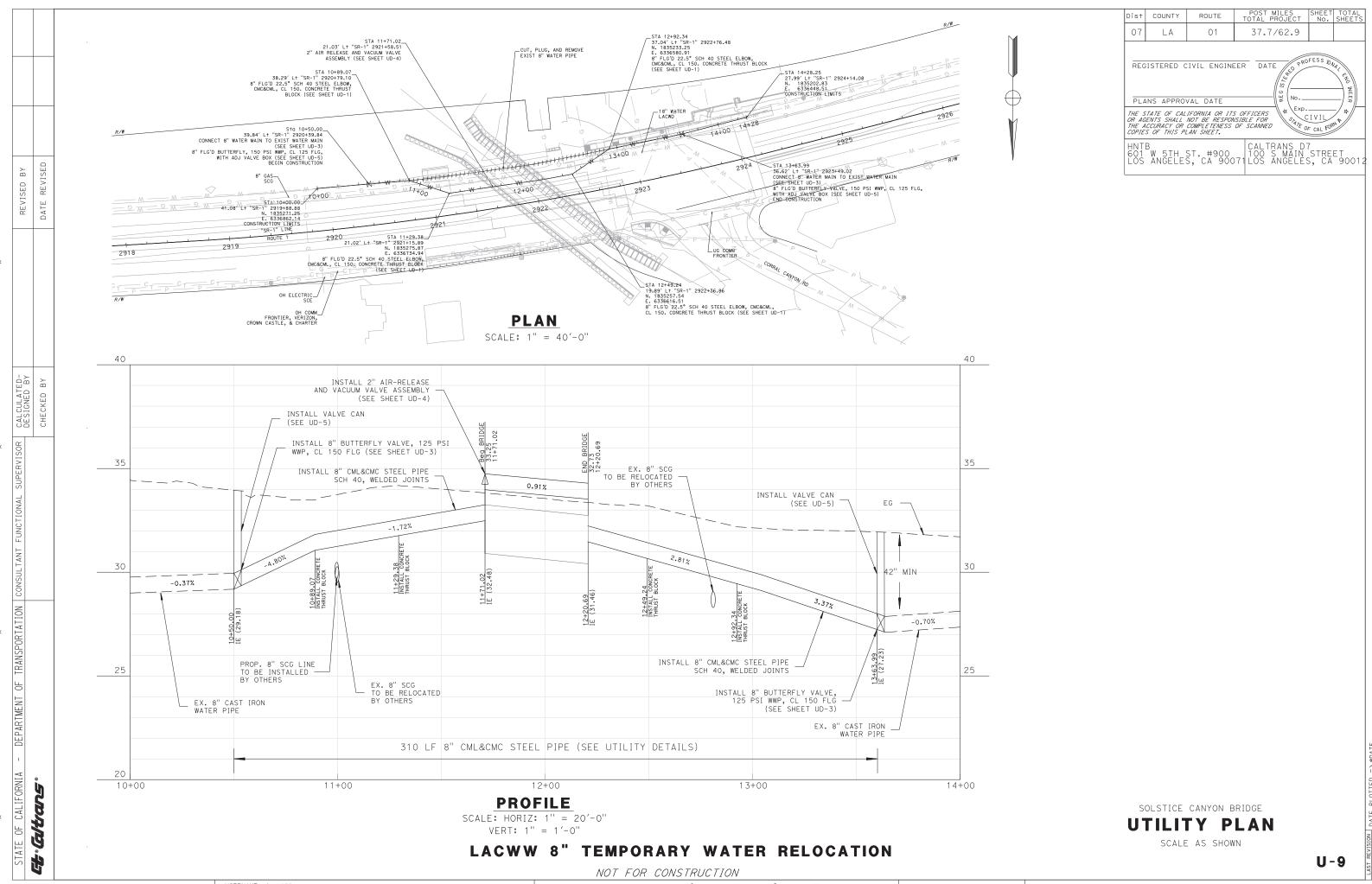
NOT FOR CONSTRUCTION

LACWW 18" WATER RELOCATION

RELATIVE BORDER SCALE IS IN INCHES

UNIT 0000

PROJECT NUMBER & PHASE



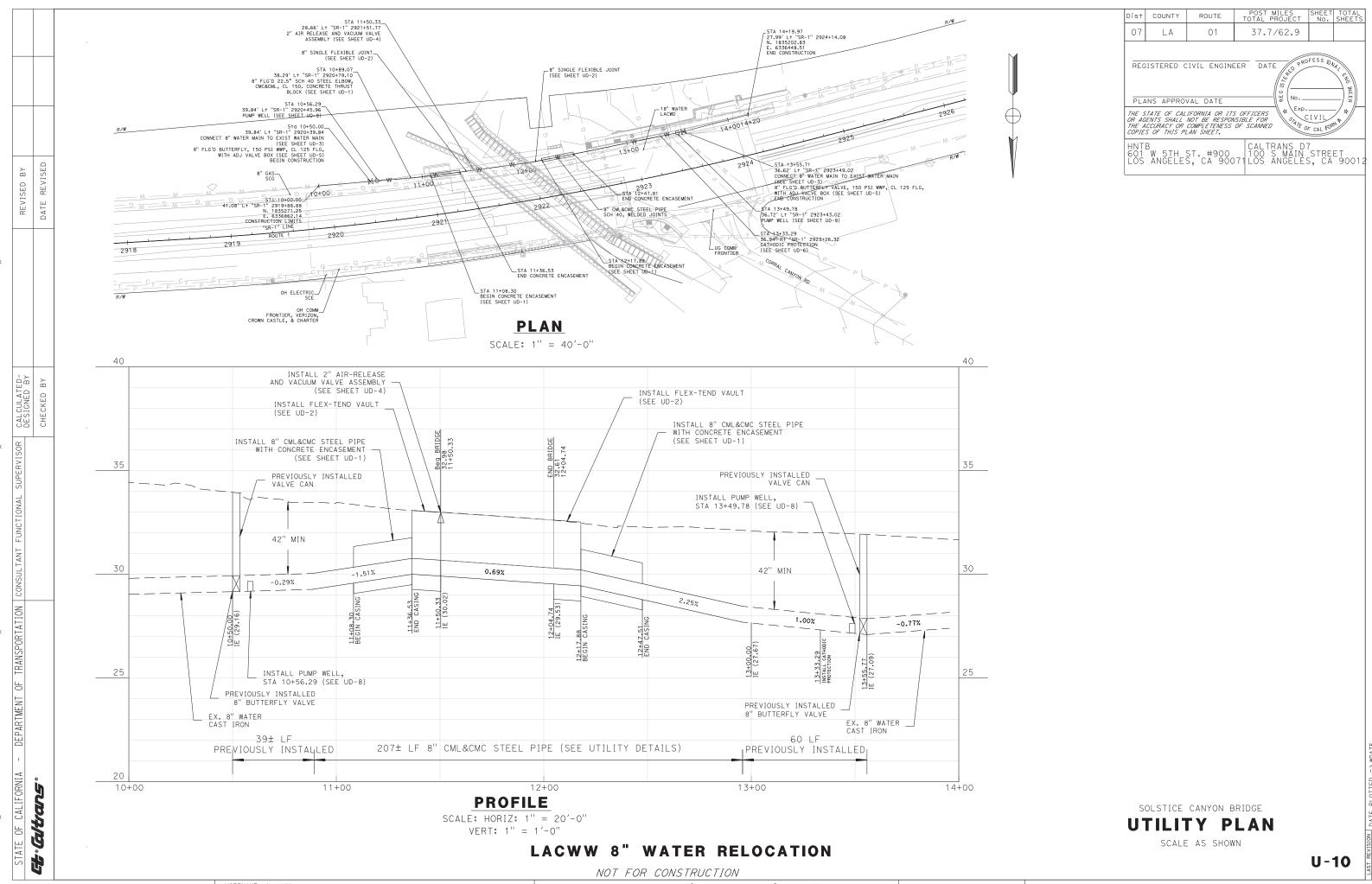
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PROJECT NUMBER & PHASE

BORDER LAST REVISED 7/2/2010

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UNIT 0000



00715000090

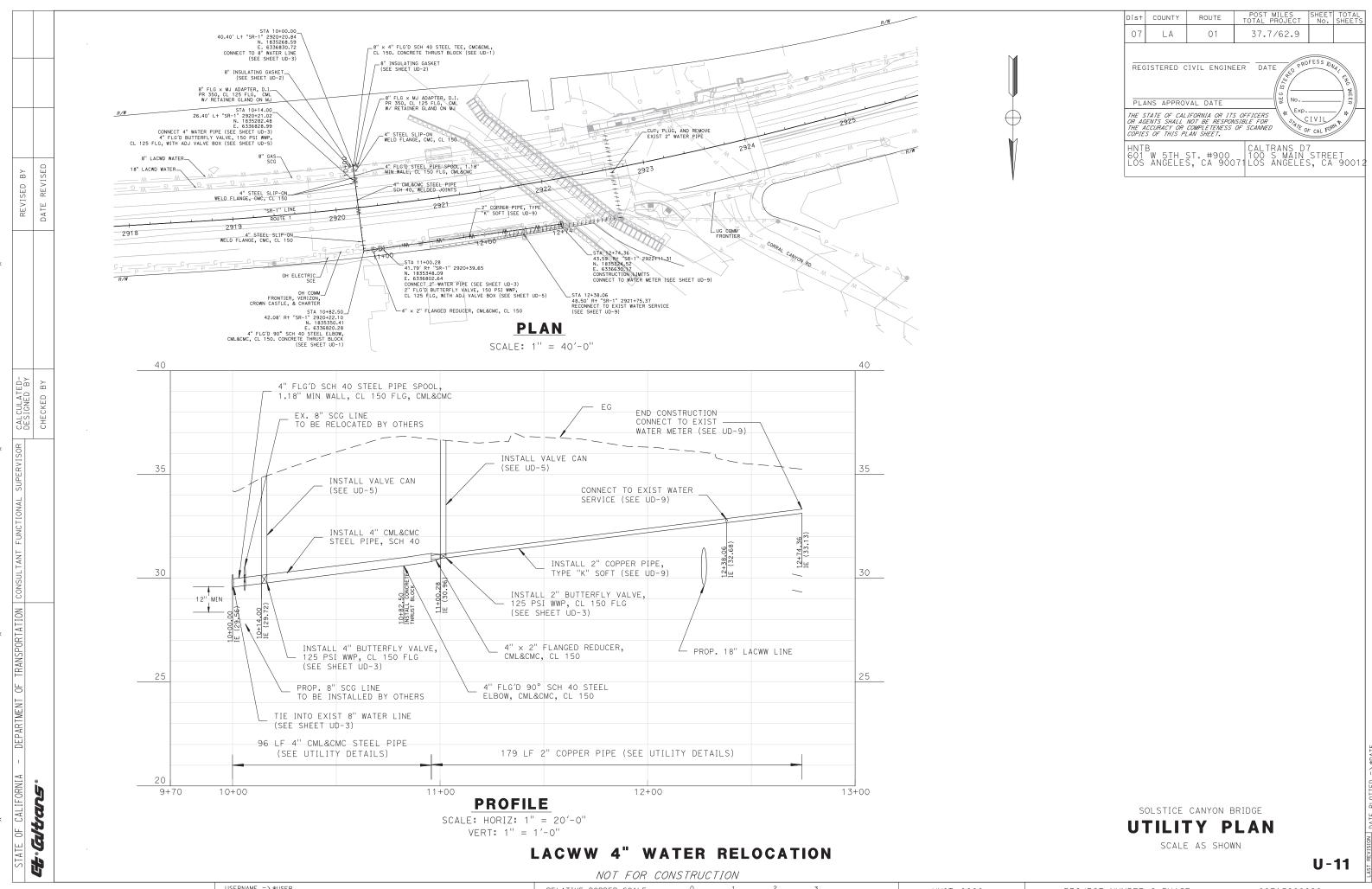
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UNIT 0000

PROJECT NUMBER & PHASE



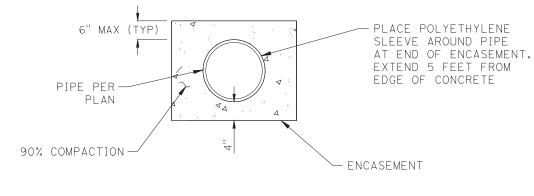
BORDER LAST REVISED 7/2/2010

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#### GENERAL NOTES

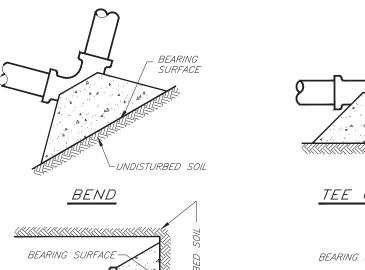
- 1. ALL ANCHOR AND THRUST BLOCKS SHALL BEAR AGAINST UNDISTURBED
- 2. MINIMUM ALLOWABLE WATER PRESSURE FOR DESIGN OF THRUST BLOCKS IS 150 PSI. BEARING AREA INCREASES DIRECTLY WITH INCREASE IN
- 3. ALL CONCRETE USED IN THRUST BLOCKS SHALL ATTAIN 2000 PSI
- 4. ALL ANCHOR RODS SHALL BE REINFORCING STEEL AND A MINIMUM OF  $\frac{1}{2}$ INCH IN DIAMETER.
- 5. USE ANCHOR BLOCKS AT VERTICAL BENDS WHEN PIPE IS ABOVE OR BELOW GROUND. SIZE OF BLOCK AND ROD SHALL BE AS SHOWN ON THE PLANS OR AS DETERMINED BY THE DISTRICT.
- 6. USE 30 POUND FELT TO INSURE COLD JOINT.
- 7. CONCRETE SHALL NOT COME INTO DIRECT CONTACT WITH ASBESTOS-CEMENT PIPE.
- 8. FOR PIPE GREATER THAN 12" IN DIAMETER, ENGINEER IS TO SUBMIT CALCULATIONS FOR APPROVAL.

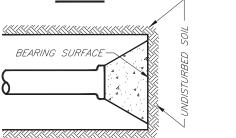


Dist	COUNTY	ROUTE	POST MILES TOTAL PROJECT	SHEET No.	TOTAL SHEETS
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OR AG	ENTS SHALL	IFORNIA OR ITS NOT BE RESPON COMPLETENESS LAN SHEET.	) UFF1UENO \\ \ ~	F CAL FOR	/ '//
HNT 601 LOS		ST. #900 S, CA 900	CALTRANS D 100 S MAIN 071 LOS ANGELES		ET 90012

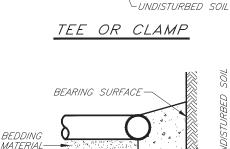
#### FLEX-TEND ENCASEMENT DETAIL

NOT TO SCALE





PLUGGED/CAPPED END



TYPICAL CROSS SECTION

	PROP PAVEN	
		2.5 MIN
95% SUBGRADE COMPACTION	NIW WIN	TRENCH WALLS
90% COMPACTION  6" MIN		CONTRACTOR SHALL ENSURE PROPER INSTALLATION OF PIPE INVERTS PER PLAN WITHIN SLURRY ENCASEMENT LIMITS
2 SACK — SLURRY MIX	NIW9	PROP CML&CMC STEEL PIPE

#### TABLE /

MINIM	MUM BEA	ring ar	EAS IN	SQ FT*
MAIN SIZE	TEE**	90° BEND	45° BEND	22 ½° BEND
6''	4	4	4	3
8''	5	7	4	3
10"	9	12	6	4
12"	12	16	9	6
18"	19	27	15	8

\* BASED ON 150 PSI WWP PRESSURE & SOIL BEARING LOADS OF 2000 PSF. THE RATIO OF WIDTH TO HEIGHT SHALL NOT EXCEED 1  $\frac{1}{2}$  TO 1 \*\* TEES, PLUGS, CAPS, AND HYDRANTS

TABLE //

BEARING **SURFACE** 

***SOIL TYPE	**** MAX ALLOWABLE SOIL BEARING VALUES	
LOOSE SAND	500 PSF	4
SOFT SANDY CLAY	1000 PSF	2
ADOBE	1000 PSF	2
COMPACT FINE SAND	2000 PSF	1
COMPACT COARSE SAND	2000 PSF	1
MEDIUM STIFF CLAY	2000 PSF	1

\*\*\*THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE SAFE SOIL BEARING VALUES AND THE POSITION AND SIZE OF BEARING AREAS

\*\*\* BASED ON 2 FEET MINIMUM DEPTH OF COVER OVER THE PIPE

NOT TO SCALE

## **CONCRETE THRUST BLOCK**

NOT FOR CONSTRUCTION

NOT TO SCALE 18" PIPE: STA 11+55.61 TO STA 11+84.80

PIPE ENCASEMENT DETAIL

AND STA 12+59.14 TO STA 12+71.94 8" PIPE: STA 11+18.85 TO STA 11+50.88

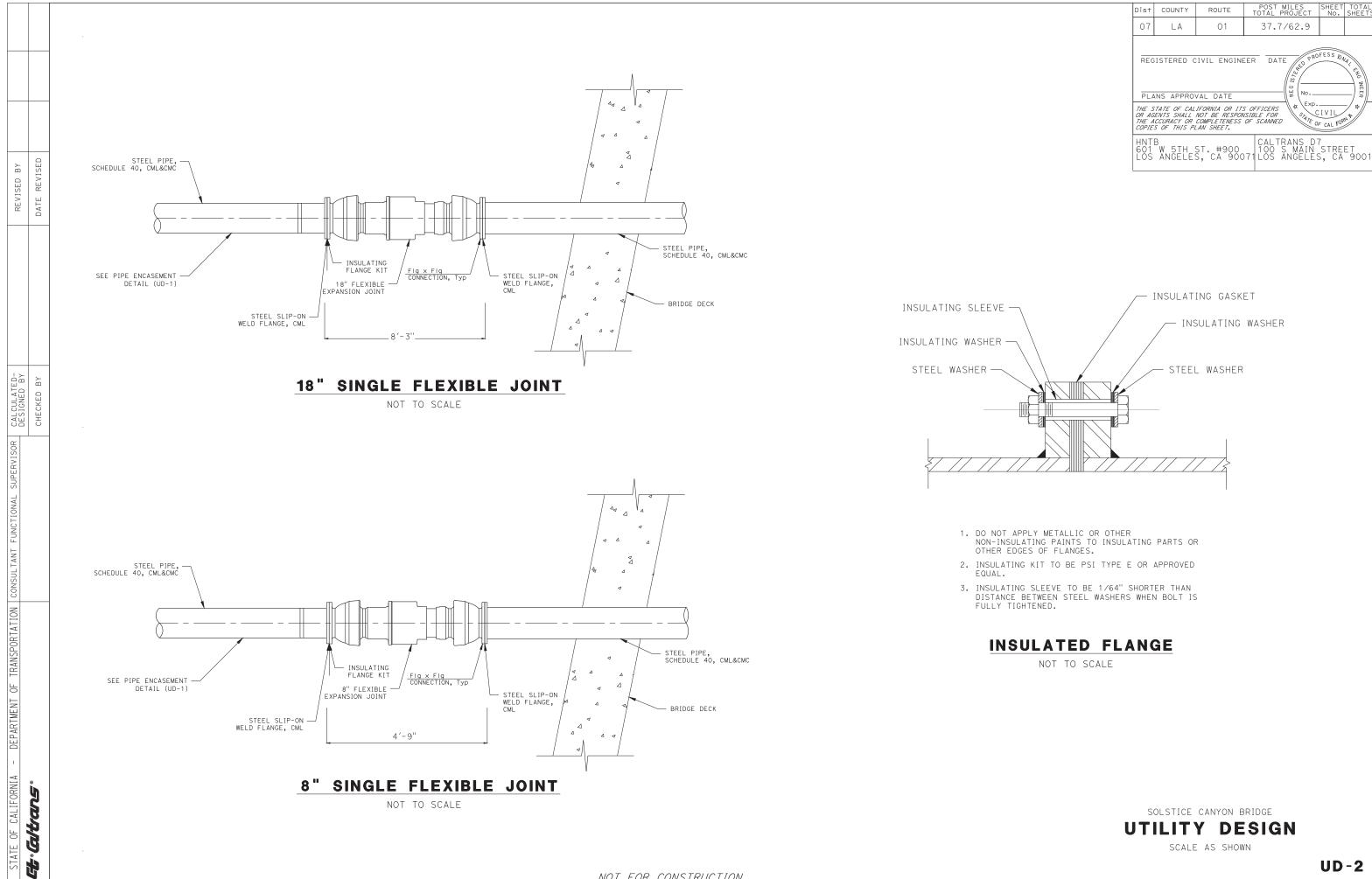
AND STA 12+24.36 TO STA 12+66.15

SOLSTICE CANYON BRIDGE

# UTILITY DESIGN

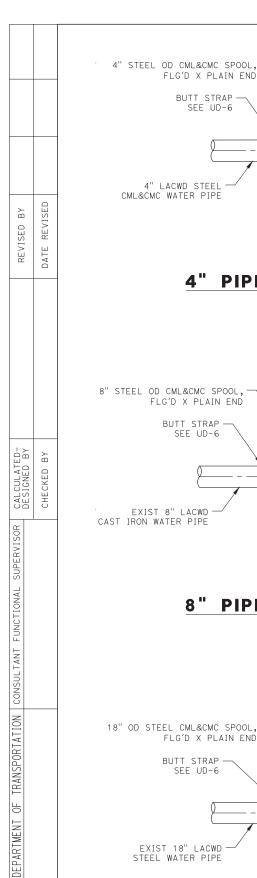
SCALE AS SHOWN

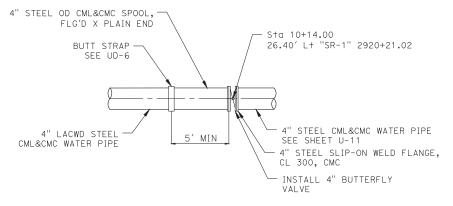
UD-1



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**UD-2** 



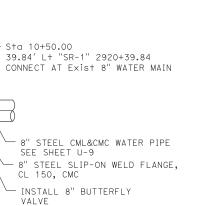


#### 4" x 2" FLANGED REDUCER, CML&CMC, CL 300 4" STEEL OD CML&CMC SPOOL, FLG'D X PLAIN END - Sta 11+00.28 41.79' Rt "SR-1" 2920+39.65 BUTT STRAP -SEE UD-6 2" COPPER PIPE, TYPE "K" SOFT 4" LACWD STEEL CML&CMC WATER PIPE 5′ MIN SEE SHEET U-11 INSTALL 2" BUTTERFLY VALVE

#### POST MILES SHEET TOTAL TOTAL PROJECT No. SHEET: Dist COUNTY ROUTE 07 37.7/62.9 01 LΑ REGISTERED CIVIL ENGINEER DATE PLANS APPROVAL DATE THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET, CIVIL HNTB 601 W 5TH ST. #900 LOS ANGELES, CA 9007 LOS ANGELES, CA 9001

#### PIPE CONNECTION DETAIL

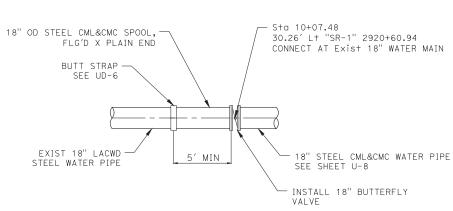
NOT TO SCALE STA 10+14.00



# 8" PIPE CONNECTION DETAIL

5′ MIN

NOT TO SCALE STA 10+50.00

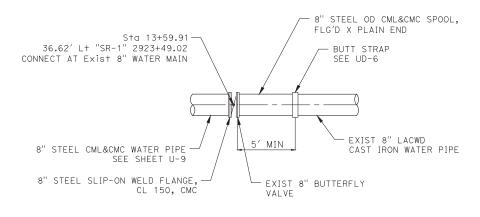


# 18" PIPE CONNECTION DETAIL

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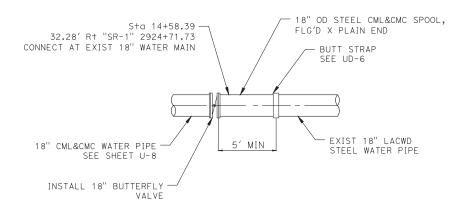
## 4" PIPE CONNECTION DETAIL

NOT TO SCALE STA 11+00.28



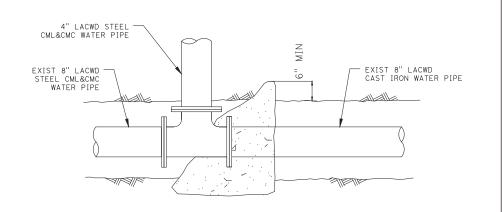
## 8" PIPE CONNECTION DETAIL

NOT TO SCALE STA 13+59.91



# 18" PIPE CONNECTION DETAIL

NOT TO SCALE STA 14+58.39



# 4" PIPE CONNECTION DETAIL

NOT TO SCALE STA 10+00.00

SOLSTICE CANYON BRIDGE

# UTILITY DESIGN

SCALE AS SHOWN

NOT FOR CONSTRUCTION

BORDER LAST REVISED 7/2/2010

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USERNAME => \$USER DGN FILE => \$REQUEST RELATIVE BORDER SCALE IS IN INCHES

UNIT 0000

PROJECT NUMBER & PHASE

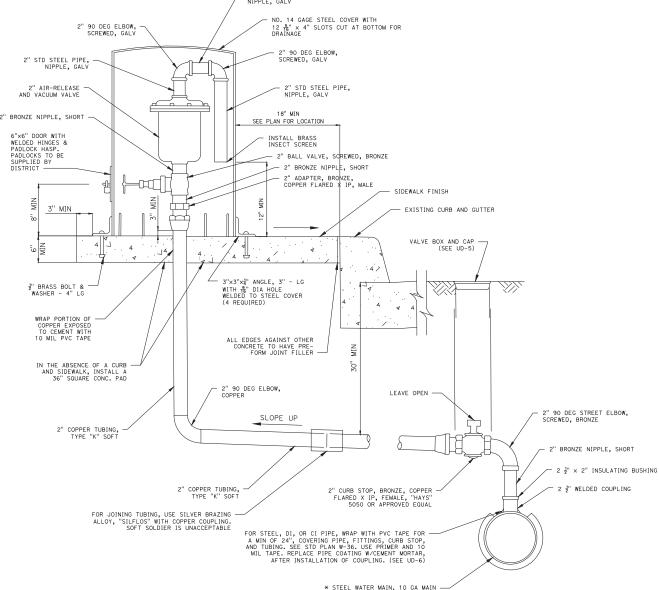
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**UD-3** 

TATION CONSULTANT FUNCTIONAL	ONAL SUPERVISOR	CALCULATED- DESIGNED BY	REVISED BY		
		CHECKED BY	DATE REVISED		

#### GENERAL NOTES

- 1. THE HEIGHT AND DIAMETER OF THE STEEL COVER SHALL PROVIDE A 2" MINIMUM CLEARANCE AROUND THE VALVE ASSEMBLY.
- 2. IN AREAS SUBJECT TO FREEZING, ALL VALVES AND PIPING ABOVE GROUND SHALL BE INSULATED.
- 3. PAINT VALVE ASSEMBLY ABOVE GROUND, AND STEEL COVER, WITH TWO COATS OF DISTRICT APPROVED RED PRIMER AND TWO COATS OF DISTRICT APPROVED FOREST GREEN OR DUNES TAN.
- 4. USE PROPER CLASS FITTINGS FOR WORKING WATER PRESSURE. (CLASS 150 MIN).
- 5. IF BRONZE NIPPLE (ITEM NO. 3) IS OVER 12" LONG, ADD CORPORATION STOP NEXT TO MAIN. (LEAVE OPEN).
- 6. SEE PLANS FOR VALVE SIZES AND USE SAME SIZE FITTINGS, AND NIPPLE LENGTHS TO SUIT. (NO CLOSE NIPPLES).



07 37.7/62.9 01 LΑ REGISTERED CIVIL ENGINEER DATE

ROUTE

POST MILES SHEET TOTAL TOTAL PROJECT No. SHEETS

CIVIL

OF CAL FORT

PLANS APPROVAL DATE THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET.

Dist COUNTY

# AIR-RELEASE AND VACUUM VALVE ASSEMBLY

NOT TO SCALE

SOLSTICE CANYON BRIDGE

# **UTILITY DESIGN**

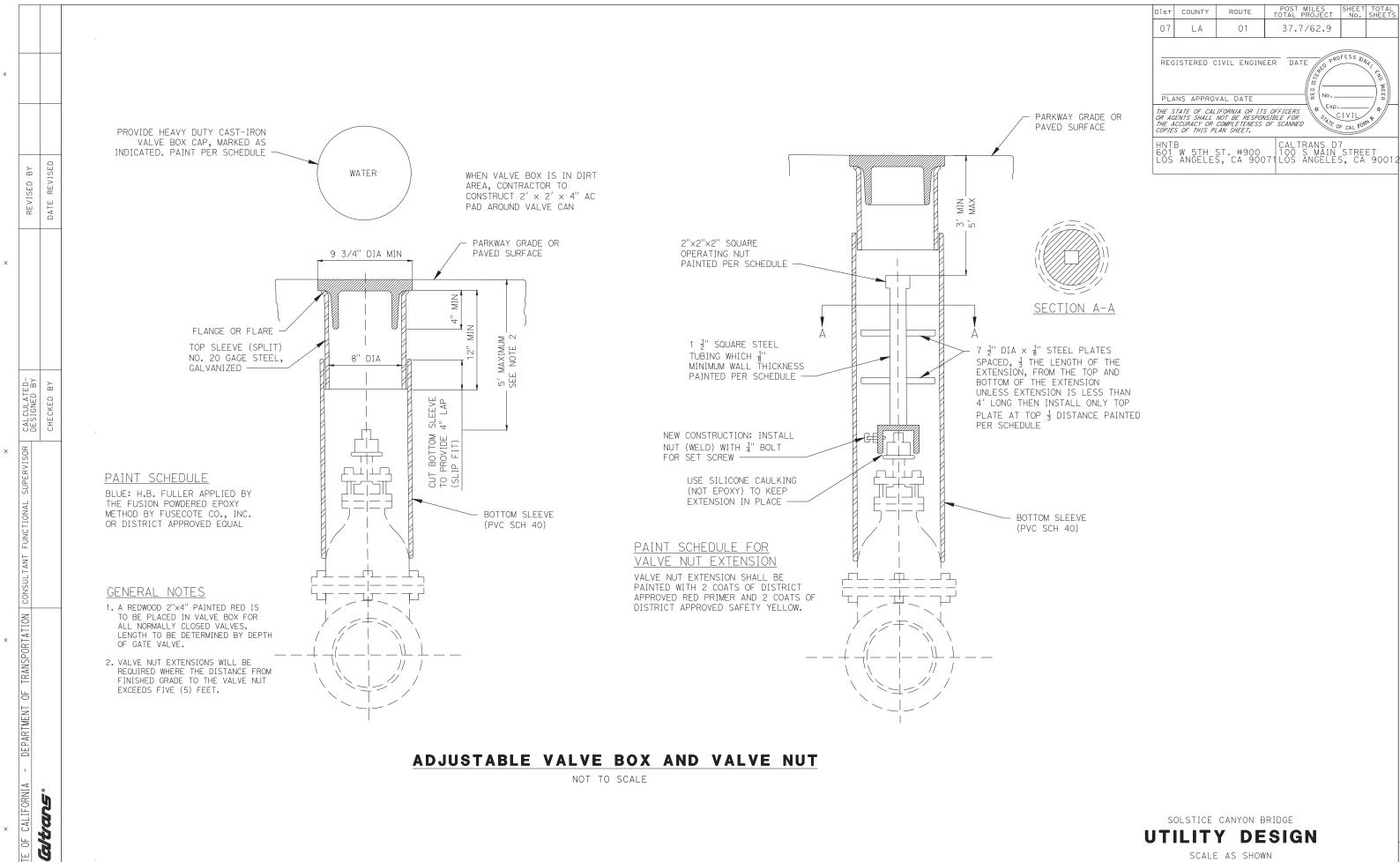
SCALE AS SHOWN

**UD-4** 

NOT FOR CONSTRUCTION

\* FOR CONNECTION TO ACP, DI, CI OR STEEL (LESS THAN 10 GA) PIPE, SEE LACWW STD PLAN W-5, NOTE 3 FOR DOUBLE-STRAP CLAMP TO BE USED FOR ITEM 12. CLAMP OUTLET SHALL BE SET VERTICALLY FACING UPWARD

USERNAME => \$USER RELATIVE BORDER SCALE IS IN INCHES UNIT 0000 PROJECT NUMBER & PHASE 00715000090 BORDER LAST REVISED 7/2/2010



**UD-5** 

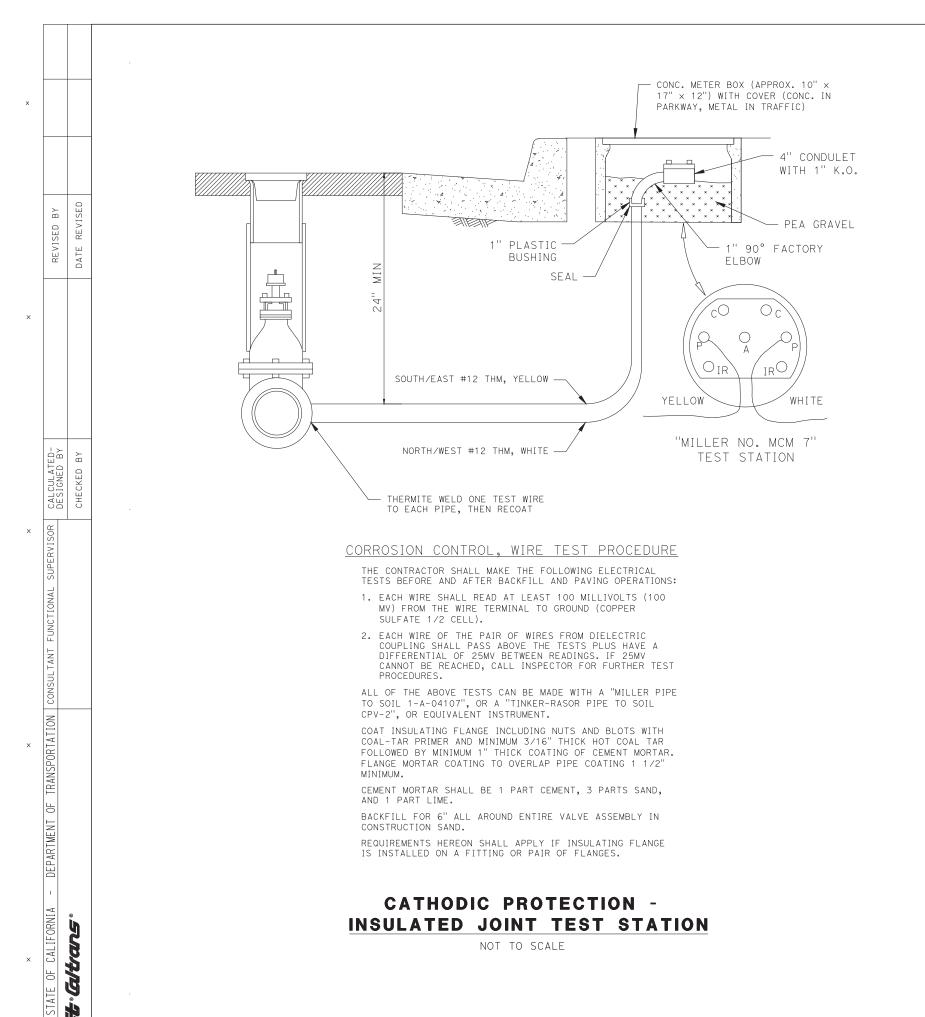
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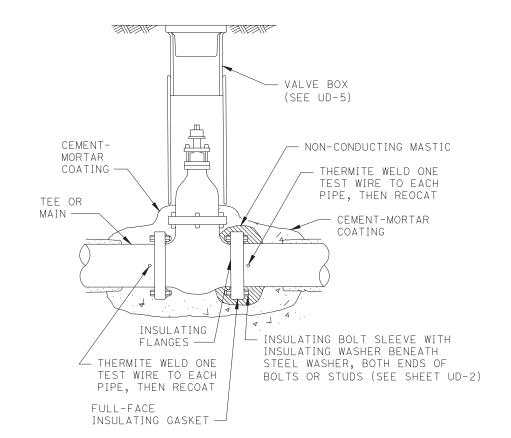
RELATIVE BORDER SCALE IS IN INCHES

UNIT 0000

PROJECT NUMBER & PHASE



POST MILES SHEET TOTAL TOTAL PROJECT No. SHEETS Dist COUNTY 07 01 37.7/62.9 LΑ REGISTERED CIVIL ENGINEER DATE PLANS APPROVAL DATE THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET, HNTB 601 W 5TH ST. #900 LOS ANGELES, CA 9007 LOS ANGELES, CA 9001



SOLSTICE CANYON BRIDGE

# UTILITY DESIGN

SCALE AS SHOWN

UD-6

NOT FOR CONSTRUCTION

RELATIVE BORDER SCALE IS IN INCHES

UNIT 0000

PROJECT NUMBER & PHASE

BORDER LAST REVISED 7/2/2010

USERNAME => \$USER

#### GENERAL NOTES

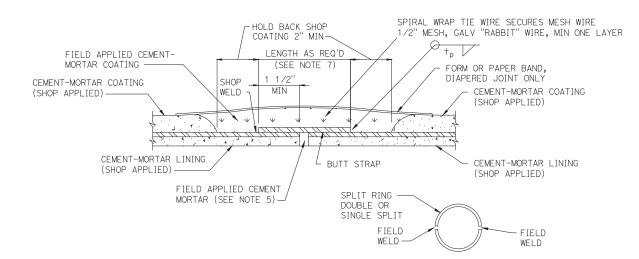
- 1. CEMENT MORTAR SHALL BE APPLIED TO WELDED JOINTS ONLY AFTER THE HEAT OF WELDING HAS DISSIPATED. JOINT WELDS SHALL NOT BE COOLED BY QUENCHING.
- 2. THE INTERIOR SURFACE OF JOINTS TO BE LINED WITH CEMENT MORTAR SHALL BE CLEANED, AND BRUSHED WITH APPROVED CEMENT ADHESIVE, IMMEDIATELY BEFORE THE MORTAR IS APPLIED.
- 3. CEMENT MORTAR FOR THE INTERIOR OF JOINTS SHALL CONSIST OF ONE PART CEMENT, ONE PART SAND, WATER, AND AN APPROVED CEMENT ADHESIVE ADDED ACCORDING TO MANUFACTURER'S RECOMMENDATIONS.
- 4. CEMENT MORTAR FOR THE EXTERIOR OF JOINTS SHALL CONSIST OF ONE PART CEMENT, ONE PART SAND, AND WATER, AND SHALL BE POURED INTO ONE SIDE OF FORM ONLY.
- 5. THE INTERIOR OF ALL JOINTS SHALL BE SWABBED BY MEANS OF A BALL AND ROD.
- 6. THE POINTING HANDHOLE SHALL BE INSTALLED ADJACENT TO A RUBBER GASKET JOINT, OR CENTERED OVER A BUTT-STRAP JOINT, AND SHALL BE USED AS NOTED ON PLANS OR WHERE A BALL AND ROD SWAB CANNOT BE USED.
- 7. FOR POINTING HANDHOLE, THE MINIMUM LENGTH OF THE BUTT STRAP SHALL BE 9 INCHES FOR ALL PIPE SIZES LISTED IN THE TABLE BELOW. WITHOUT HAND HOLE, THE MINIMUM LENGTH OF STRAP SHALL BE AS SHOWN IN THE FOLLOWING TABLE:

PIPE SIZES IN INCHES

MIN LENGTH OF BUTT STRAP REQ'D IN INCHES

6 THRU 18 20 THRU 36

8. A BOLTED FLANGED JOINT MAY BE USED AS AN ACCEPTABLE ALTERNATE TO THE RUBBER GASKET OR THE BUTT-STRAP JOINT



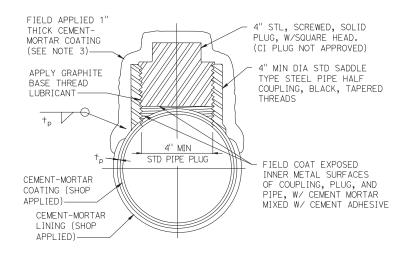
#### DETAIL B - FIELD WELDED BUTT STRAP JOINT

NOT TO SCALE

#### FIELD APPLIED CEMENT-- FORM OR PAPER BAND MORTAR COATING CEMENT-MORTAR COATING CEMENT-MORTAR COATING (SHOP APPLIED) (SHOP APPLIED) CEMENT-MORTAR COATING CEMENT-MORTAR COATING (SHOP APPLIED) (SHOP APPLIED) LINING SHALL BE BEVELED 3/8" MIN DIA X 6" LONG BONDING JUMPER, PRE-CURVED TO FIT CONTOUR OF PIPE; NO SHAPING OF BONDING JUMPER ON PIPE WILL FIELD APPLIED CEMENT MORTAR (SEE NOTE 5) BE ALLOWED. WELD 1" AT EACH END OF BAR. RUBBER GASKET (SEE DETAIL "D") — ROLLED SPIGOT

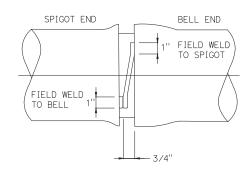
# STEEL PIPE JOINT - RUBBER GASKET JOINT

NOT TO SCALE



#### DETAIL C - POINTING HANDHOLE

NOT TO SCALE



#### **DETAIL D - BONDING JUMPER**

NOT TO SCALE

SOLSTICE CANYON BRIDGE

# UTILITY DESIGN

SCALE AS SHOWN

**UD-7** 

NOT FOR CONSTRUCTION

RELATIVE BORDER SCALE IS IN INCHES

PROJECT NUMBER & PHASE

BORDER LAST REVISED 7/2/2010

USERNAME => \$USER

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POST MILES SHEET TOTAL TOTAL PROJECT No. SHEET

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37.7/62.9

Dist COUNTY

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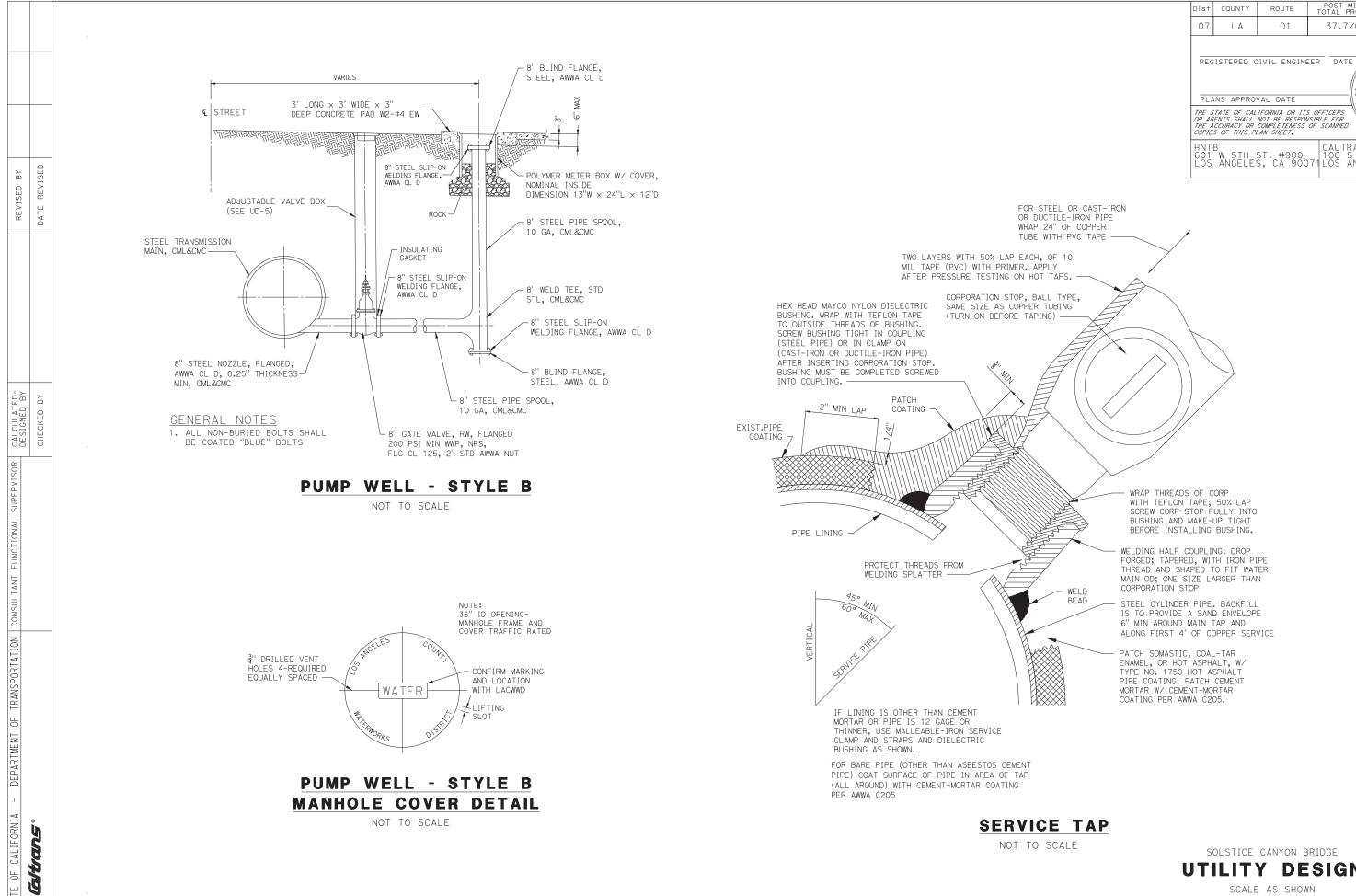
PLANS APPROVAL DATE

07

ROUTE 01

REGISTERED CIVIL ENGINEER DATE

THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET.



**UD-8** 

THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET.

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37.7/62.9

POST MILES SHEET TOTAL TOTAL PROJECT No. SHEET:

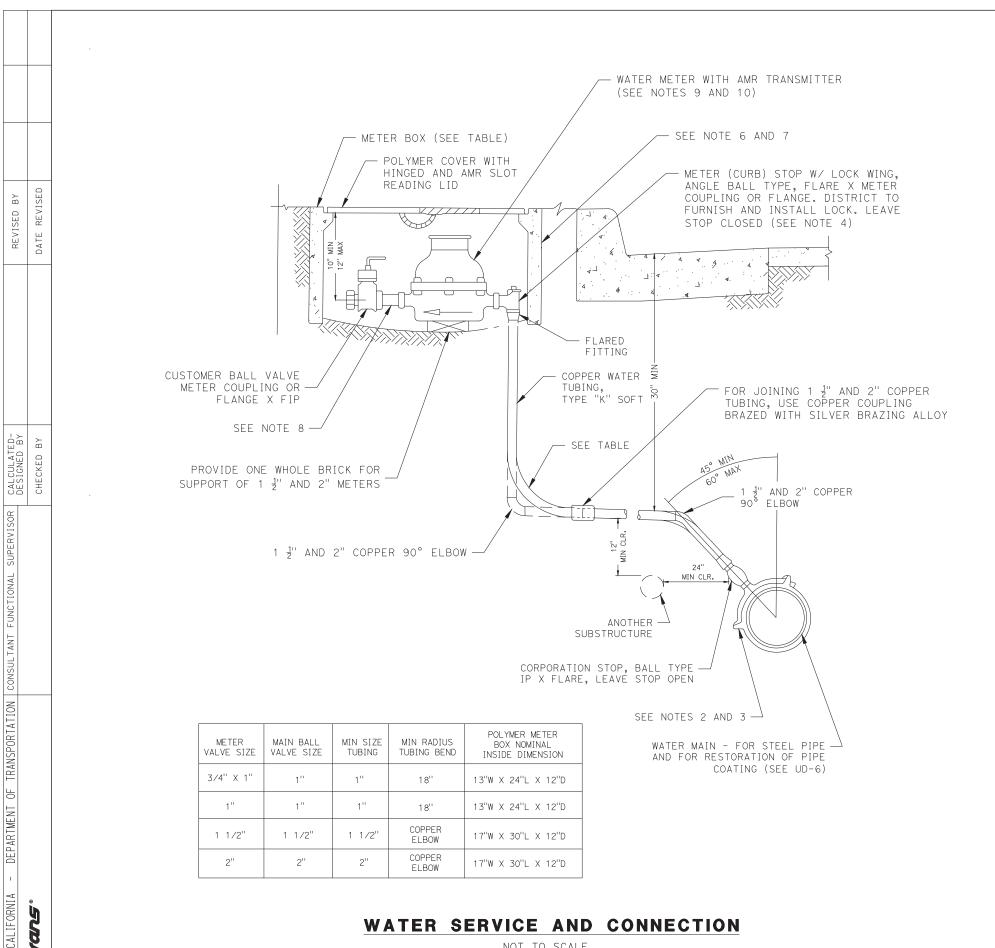
ALONG FIRST 4' OF COPPER SERVICE PATCH SOMASTIC, COAL-TAR ENAMEL, OR HOT ASPHALT, W/

TYPE NÓ. 1750 HOT ASPHÁLT PIPE COATING. PATCH CEMENT MORTAR W/ CEMENT-MORTAR COATING PER AWWA C205.

SOLSTICE CANYON BRIDGE

UTILITY DESIGN

SCALE AS SHOWN



POST MILES SHEET TOTAL TOTAL PROJECT No. SHEET: Dist COUNTY ROUTE 37.7/62.9 07 01 LΑ REGISTERED CIVIL ENGINEER DATE PLANS APPROVAL DATE THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET. CIVIL HNTB 601 W 5TH ST. #900 | CALTRANS D7 100 S MAIN STREET LOS ANGELES, CA 9007 | LOS ANGELES, CA 9001

#### GENERAL NOTES

1. NO METER BOX SHALL BE INSTALLED CLOSER THAN TEN (10) FEET FROM EDGE OF DRIVEWAY APRON (TOP OF X), OR TEN (10) FEET FROM THE PROJECTION OF THE EDGE OF THE GARAGE OPENING, WHICHEVER PROVIDES THE LARGEST SEPARATION BETWEEN THE METER BOX AND THE DRIVEWAY.

NO METER BOX SHALL BE INSTALLED IN A LOCATION WHERE VEHICLE LOADING MAY DAMAGE THE METER BOX AND/OR METER

NO METER BOX SHALL BE INSTALLED ADJACENT TO SIDEWALK TRANSITIONS WHERE WALKWAY IS NOT PARALLEL TO STREET.

- 2. MINIMUM DISTANCE BETWEEN SERVICE TAPS ON MAIN TO A BELL, COUPLING, JOINT, OR FITTING IS 36".
- 3. USE MALLEABLE-IRON OR DUCTILE-IRON DOUBLE STRAP CLAMPS ON CAST IRON, DUCTILE-IRON, AND STEEL PIPE (LESS THAN 10 GA WALL THICKNESS). USE BRONZE DOUBLE STRAP CLAMPS ON ACP. USE A WELDED THREADED OUTLET ON STEEL PIPE (WALL THICKNESS 10 GA AND GREATER). ON ALL METALLIC MAINS, INSTALL AN INSULATING BUSHING BETWEEN CLAMP OR WELDED THREADED OUTLET AND STOP. CLAMP OR WELDED OUTLET SHALL HAVE OUTLET ONE SIZE LARGER THAN STOP TO ALLOW FOR BUSHING. (SEE
- 4. TEST AT SYSTEM PRESSURE AND FLUSH SERVICE LINE BEFORE LOCKING.
- 5. ONLY EXCAVATED SOIL OR BACKFILL MATERIAL APPROVED BY DISTRICT IS TO BE USED TO BACKFILL TRENCH. NO TRASH IS TO BE LEFT IN TRENCH.
- 6. FRONT EDGE OF METER BOX TO BE PLACED AGAINST REAR OF CURB EXCEPT WHEN THERE IS A SIDEWALK ADJACENT TO REAR OF CURB. THEN, FRONT EDGE OF METER BOX TO BE PLACED AGAINST REAR OF SIDEWALK.
- 7. ALL SERVICE CONNECTIONS SHALL BE INSTALLED FROM THE MAIN IN THE STREET FROM WHICH THE SERVICE IS ADDRESSED, AT RIGHT ANGLES TO THE WATER MAIN, LOCATED AS SHOWN ON PLANS OR DIRECTED BY THE DISTRICT, AND NOT CLOSER THAN TEN (10) FEET TO ANY DRIVEWAY (TOP OF X), WALKWAY, CURB RETURN, OR OTHER UTILITY UNLESS OTHERWISE NOTED ON PLAN.
- 8. ALL 1 1/2-INCH AND 2-INCH METERS SHALL HAVE FLANGE CONNECTIONS ON THE MAIN CASE, AND ALL NECESSARY BOLTS, NUTS, AND RUBBER GASKETS.
- 9. METERED WATER SERVICE CONNECTIONS MAY ONLY BE INSTALLED UNDER DISTRICT SUPERVISION. AFTER RECEIPT AND PROCESSING OF WATER SERVICE APPLICATION BY THE DISTRICT AND PAYMENT OF ALL APPLICABLE CHARGES.
- 10. THE WATER METER MUST BE COMPATIBLE WITH THE DISTRICT'S AUTOMATED METER READING (AMR) SYSTEM. FOR DETAILS, SEE METER SPECIFICATIONS ISSUED AT THE TIME OF APPLICATION FOR WATER SERVICE.

WATER SERVICE AND CONNECTION

NOT TO SCALE

SOLSTICE CANYON BRIDGE

# UTILITY DESIGN

SCALE AS SHOWN

**UD-9** 

NOT FOR CONSTRUCTION

RELATIVE BORDER SCALE IS IN INCHES

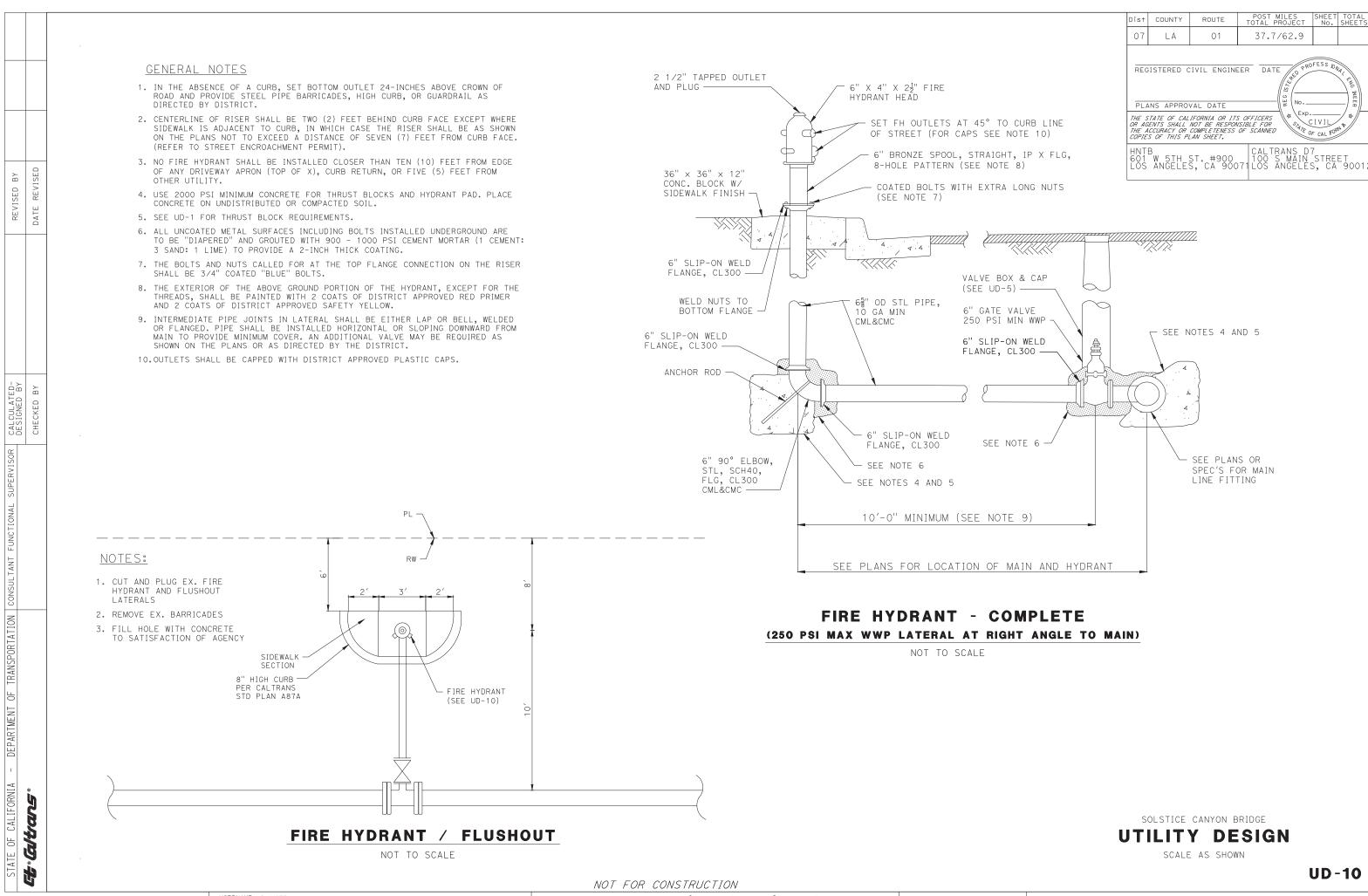
PROJECT NUMBER & PHASE

BORDER LAST REVISED 7/2/2010

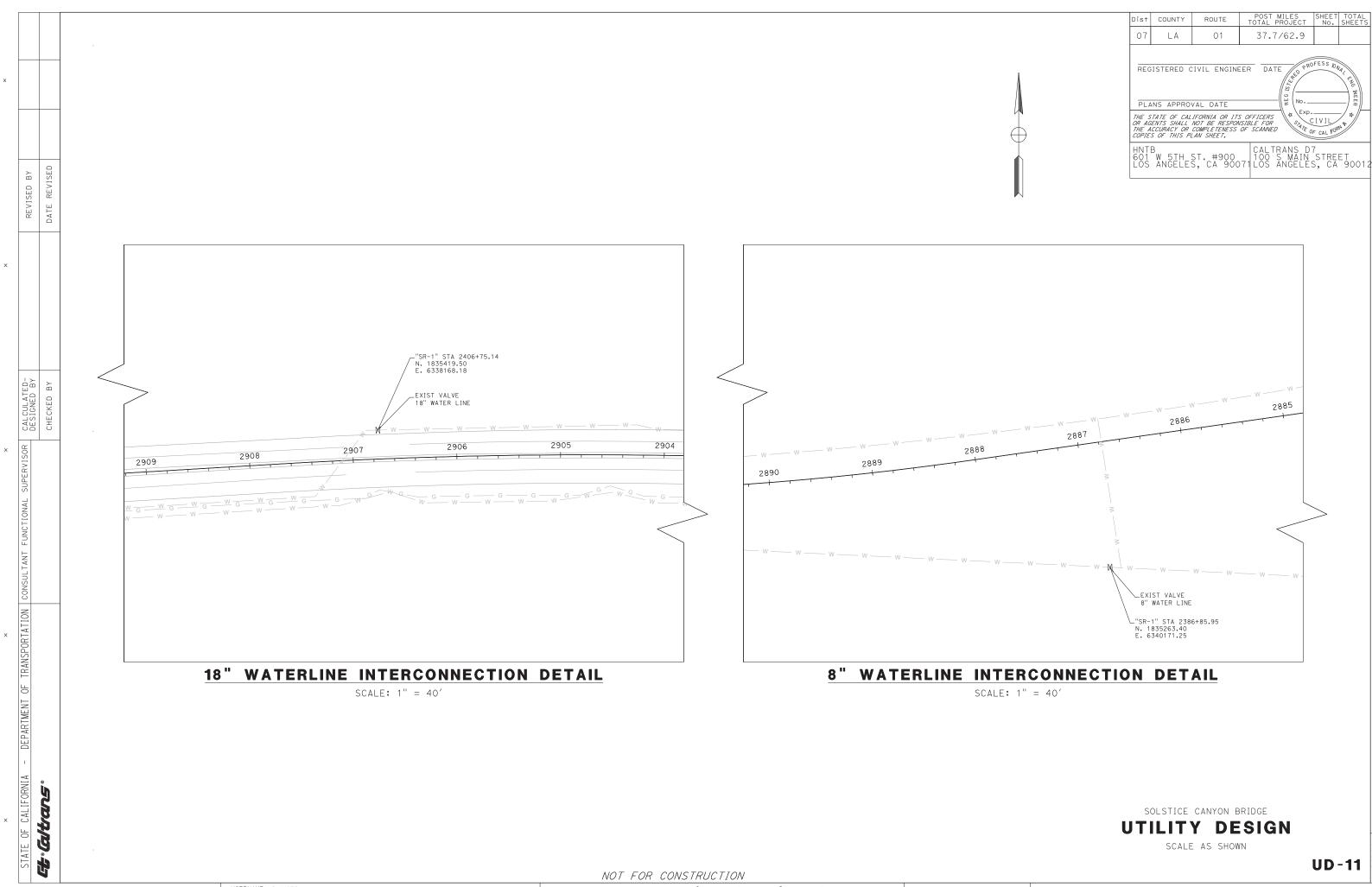
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UNIT 0000



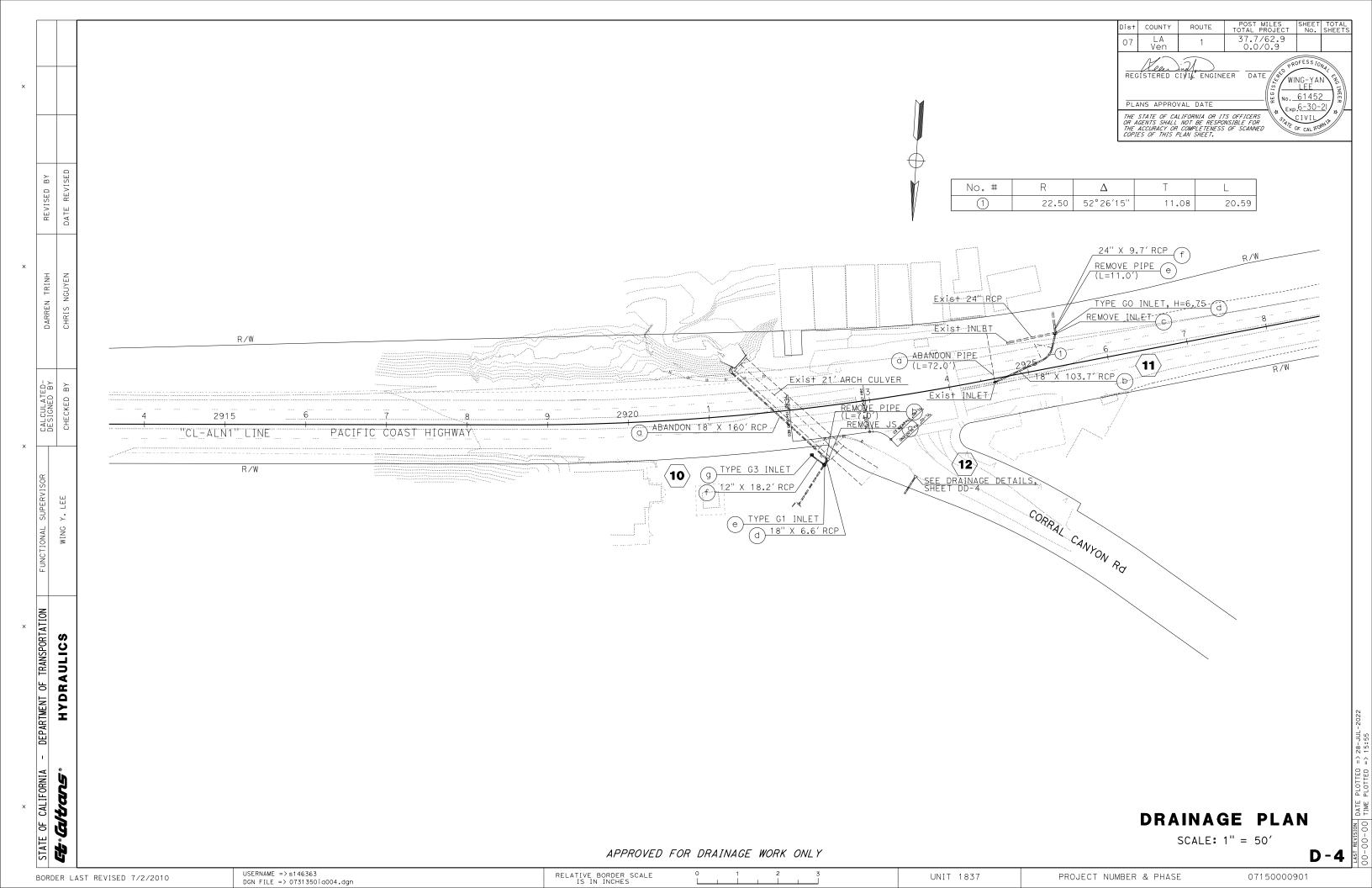
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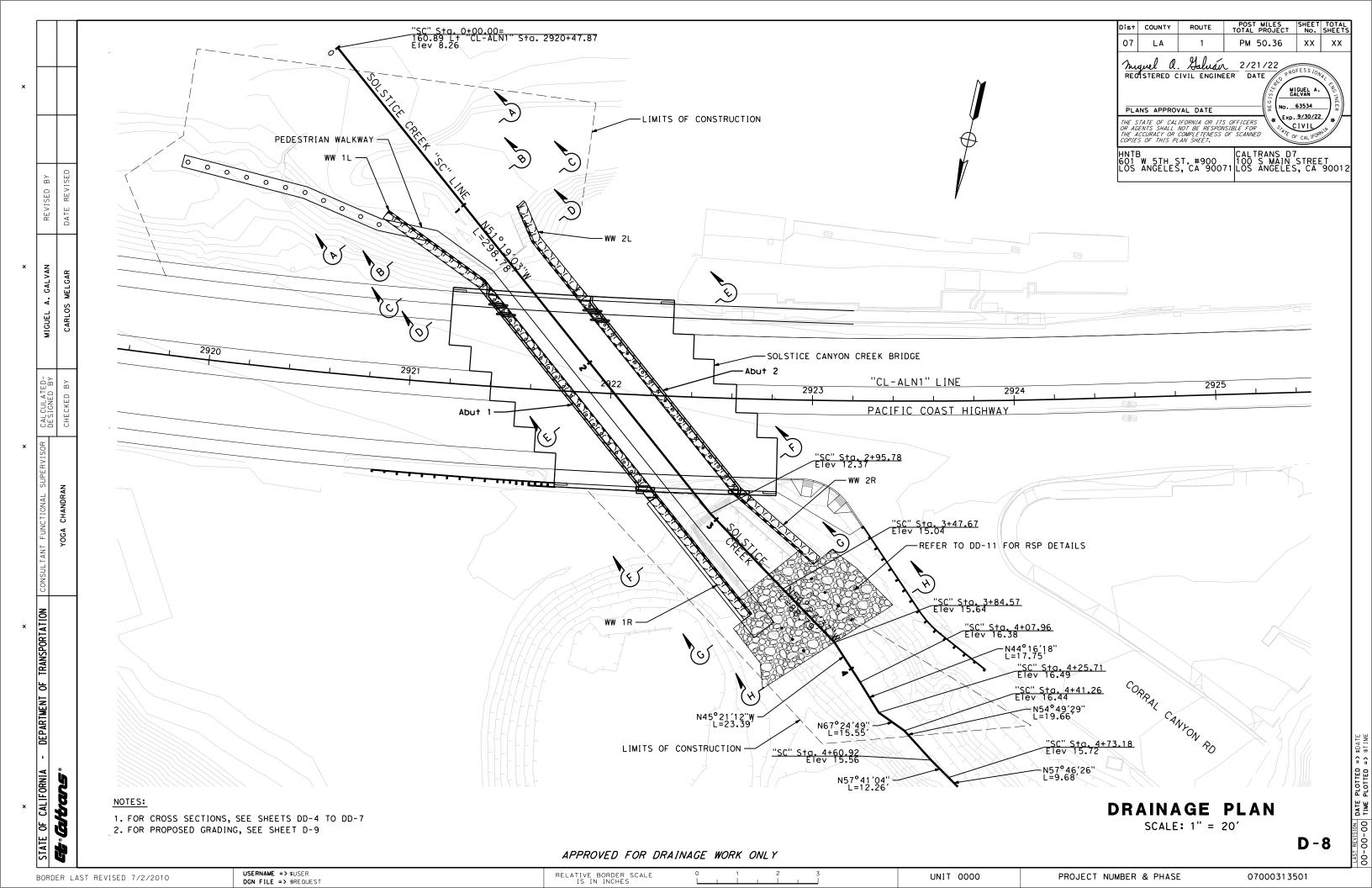


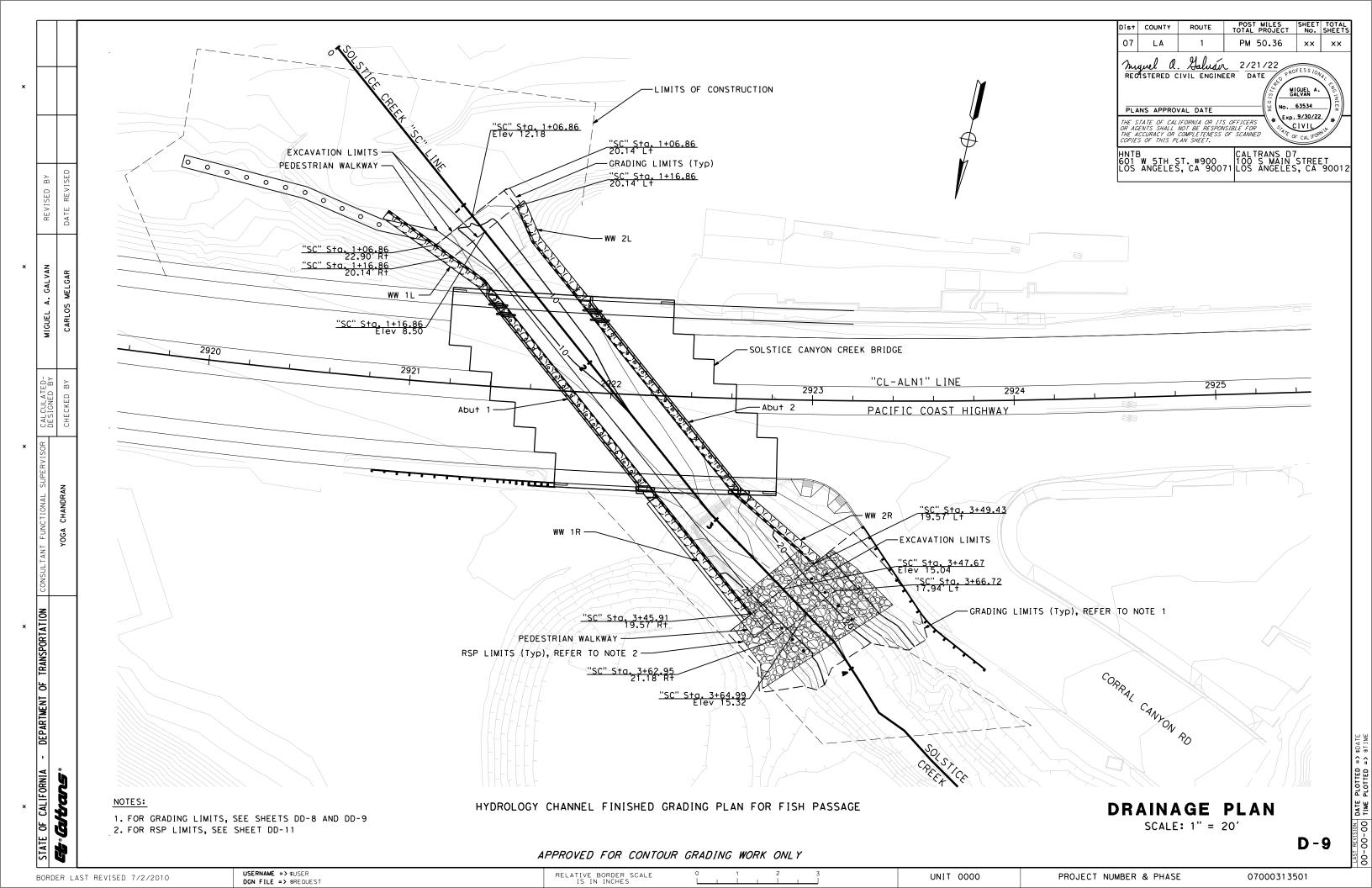
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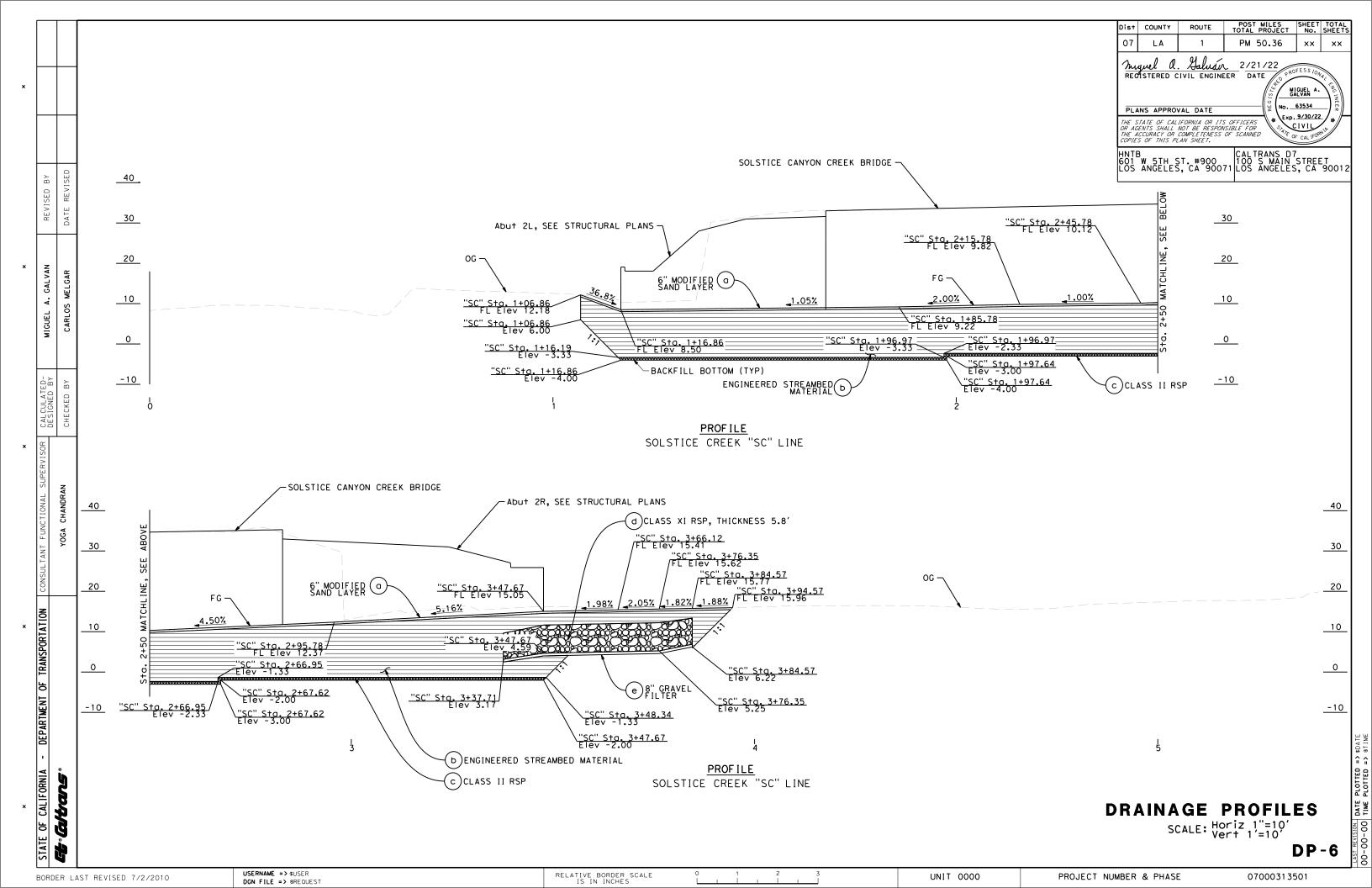
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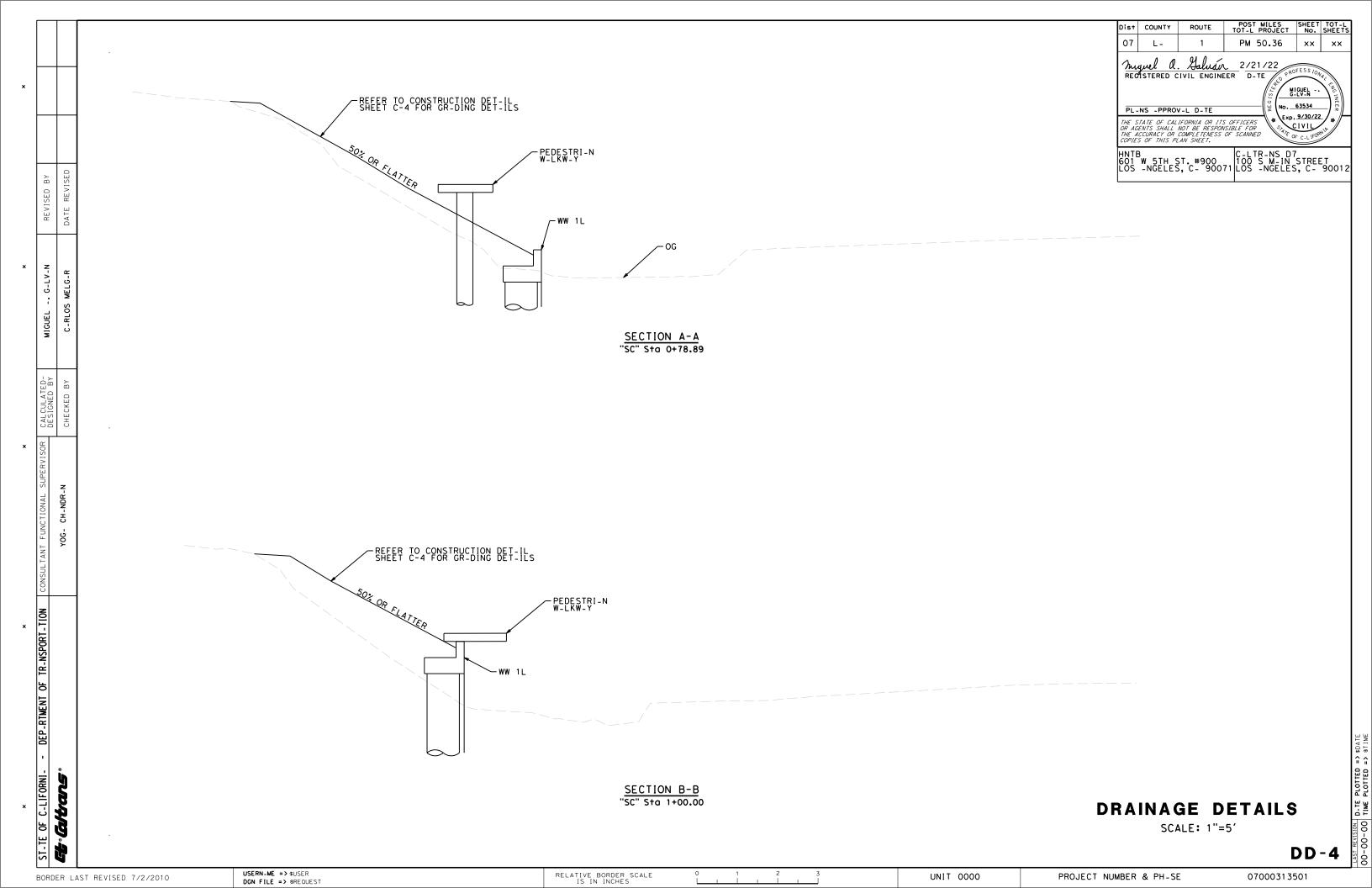
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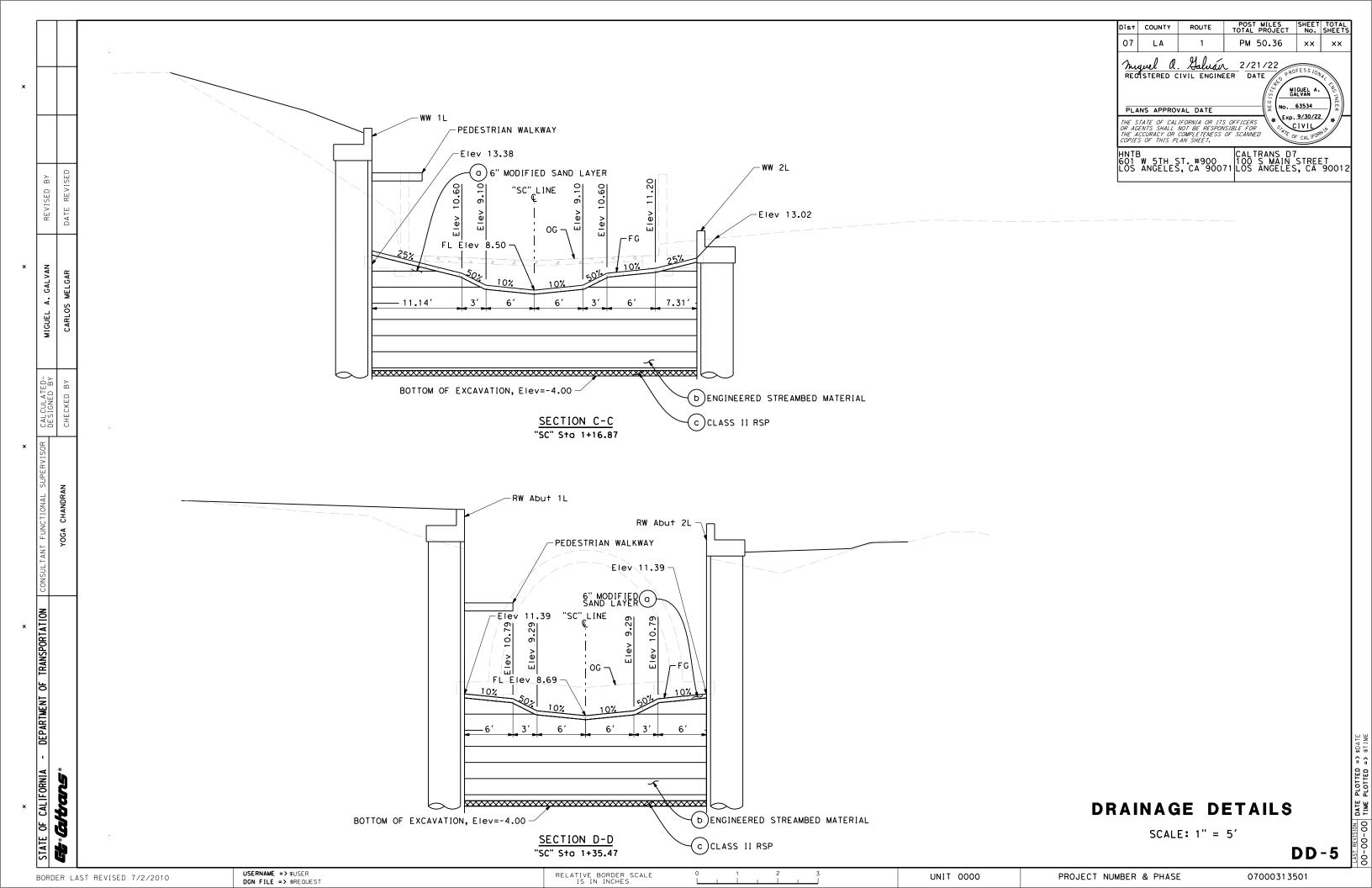


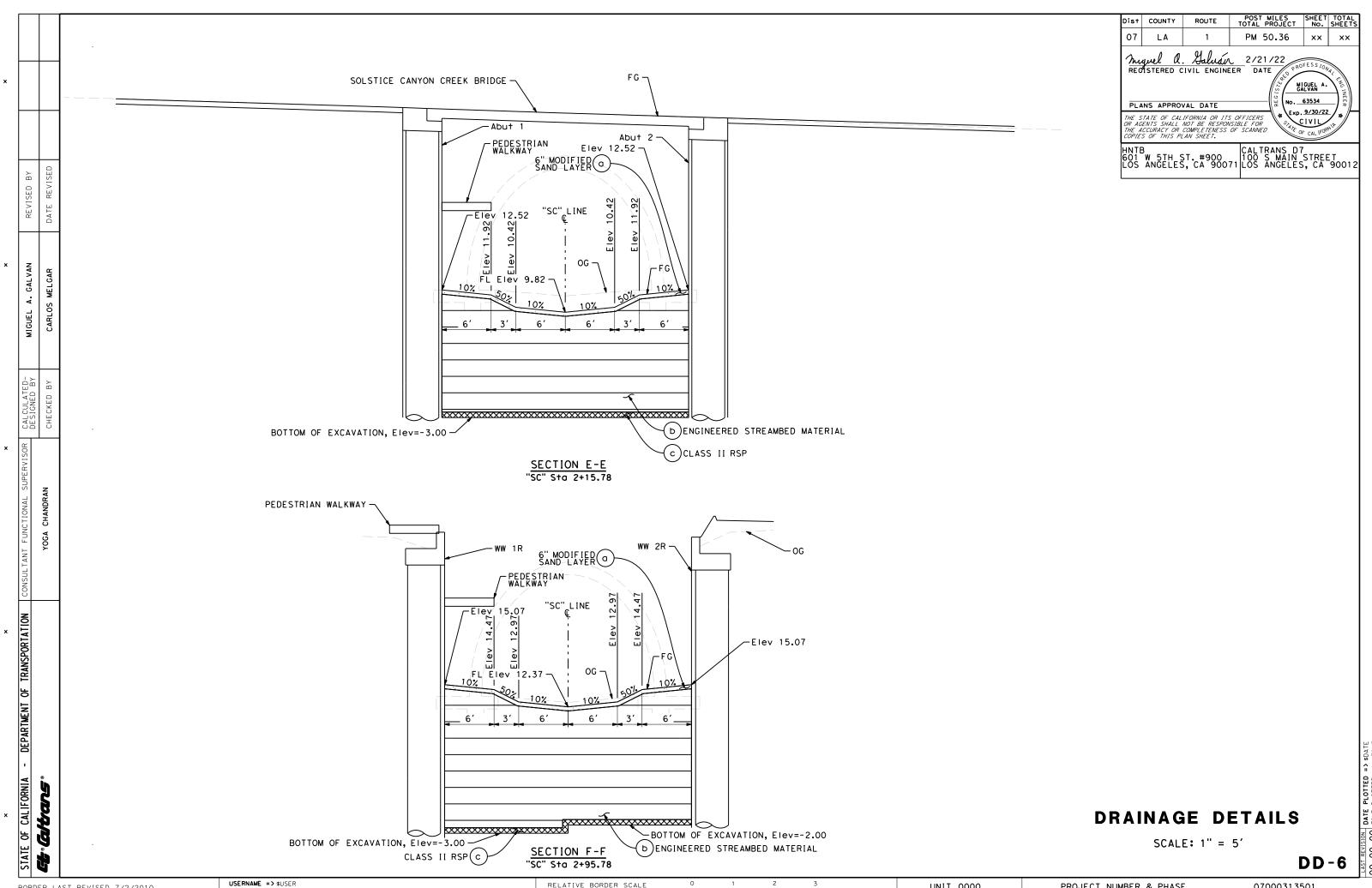




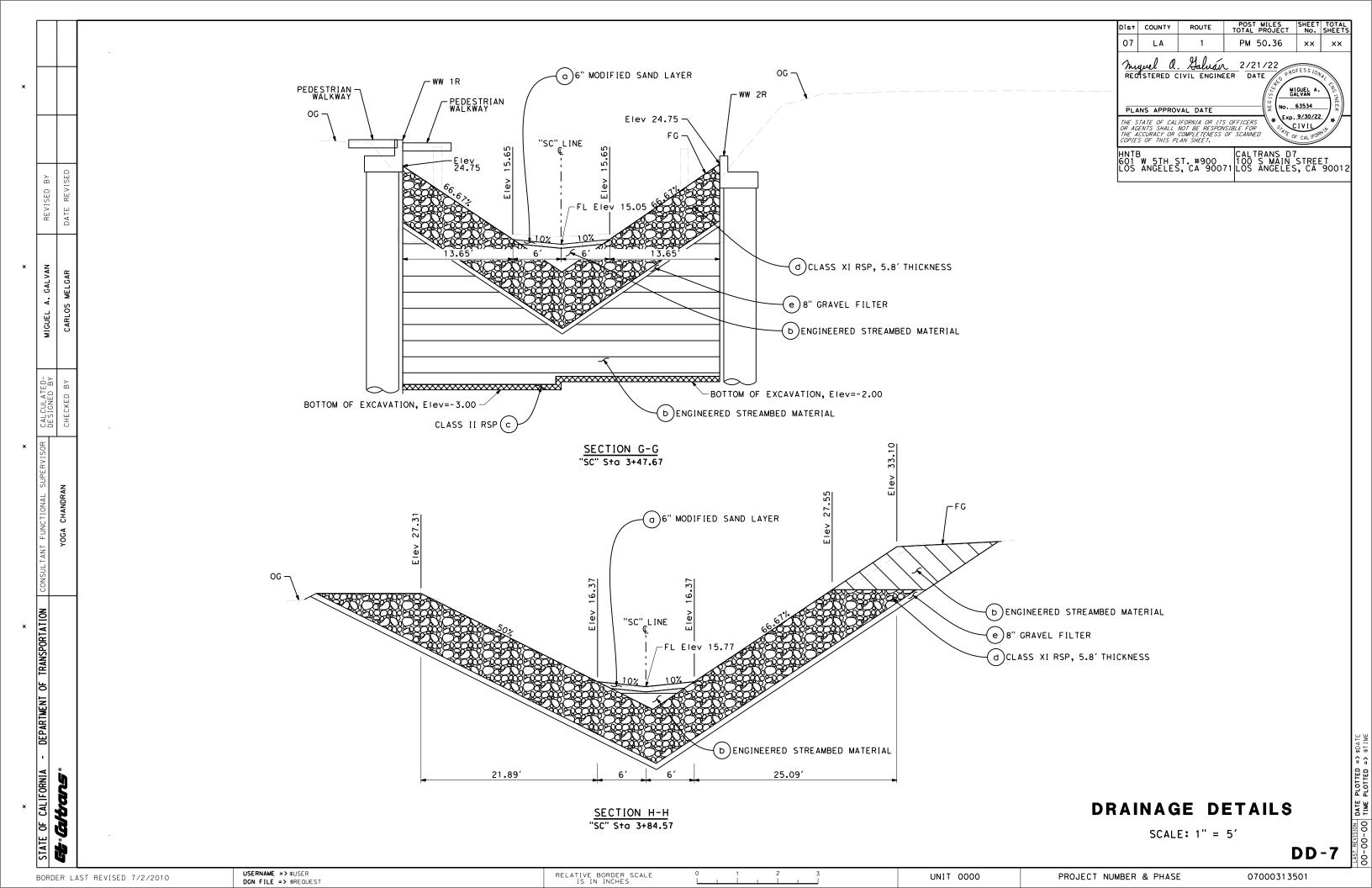


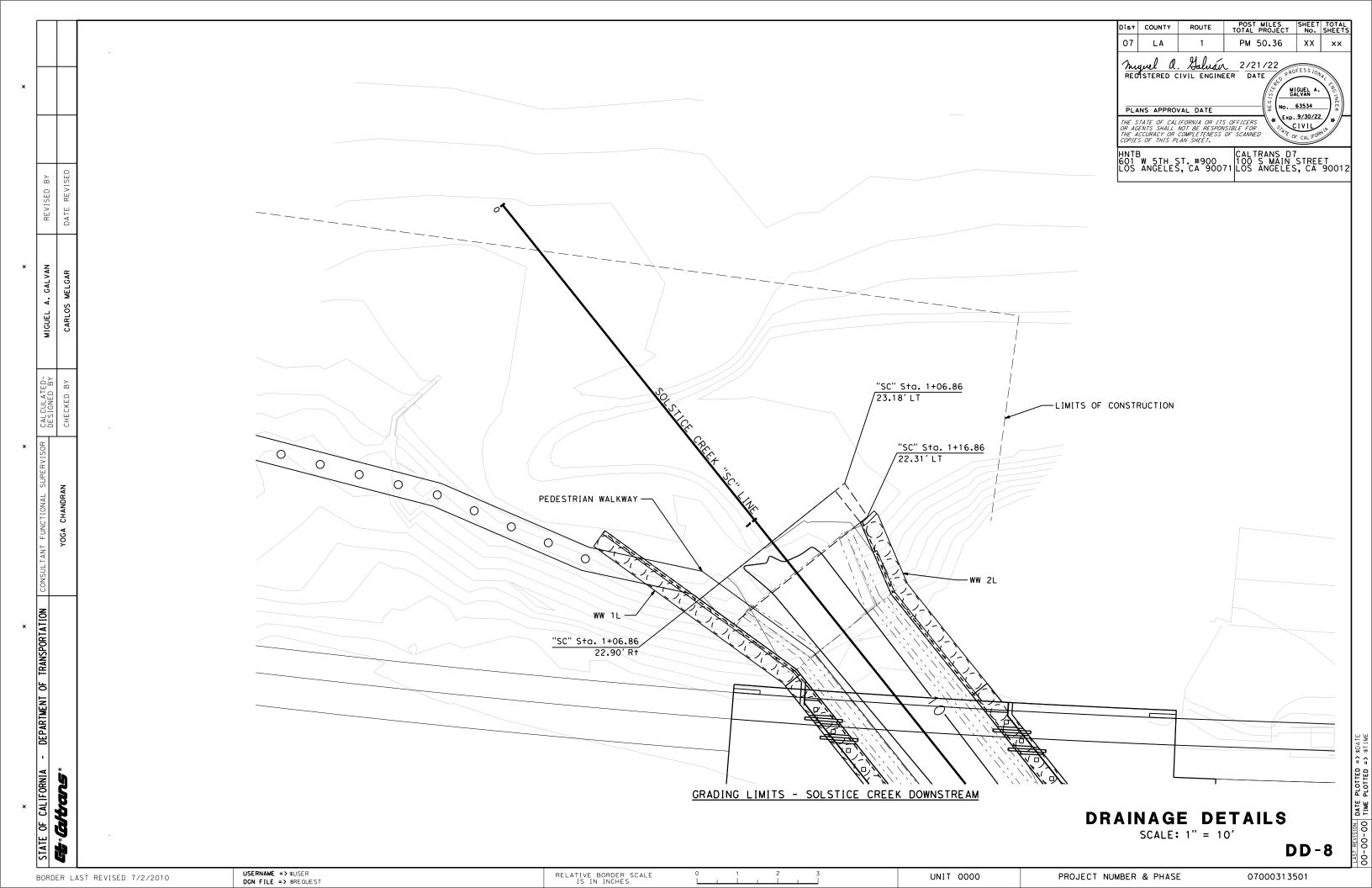


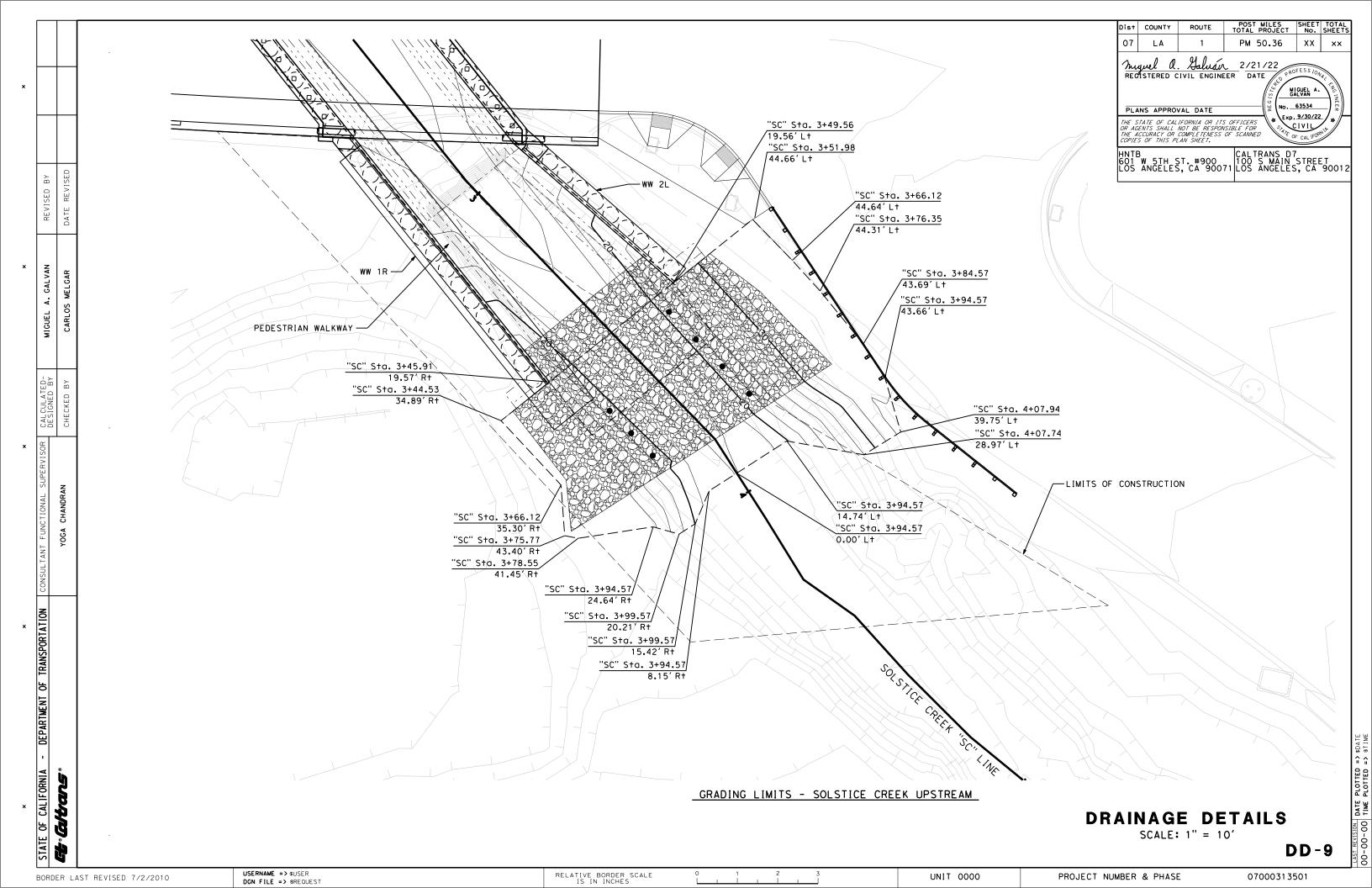


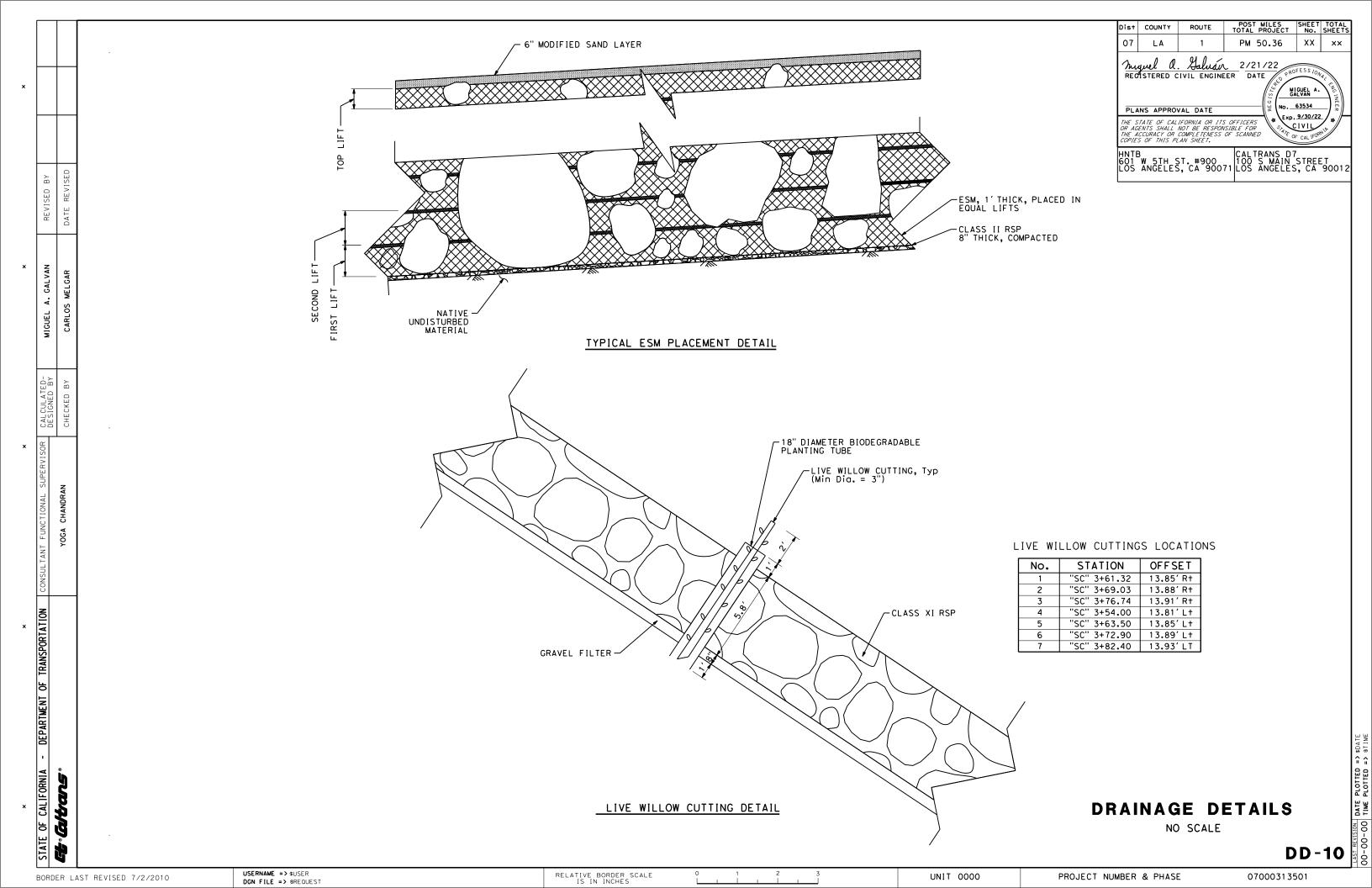


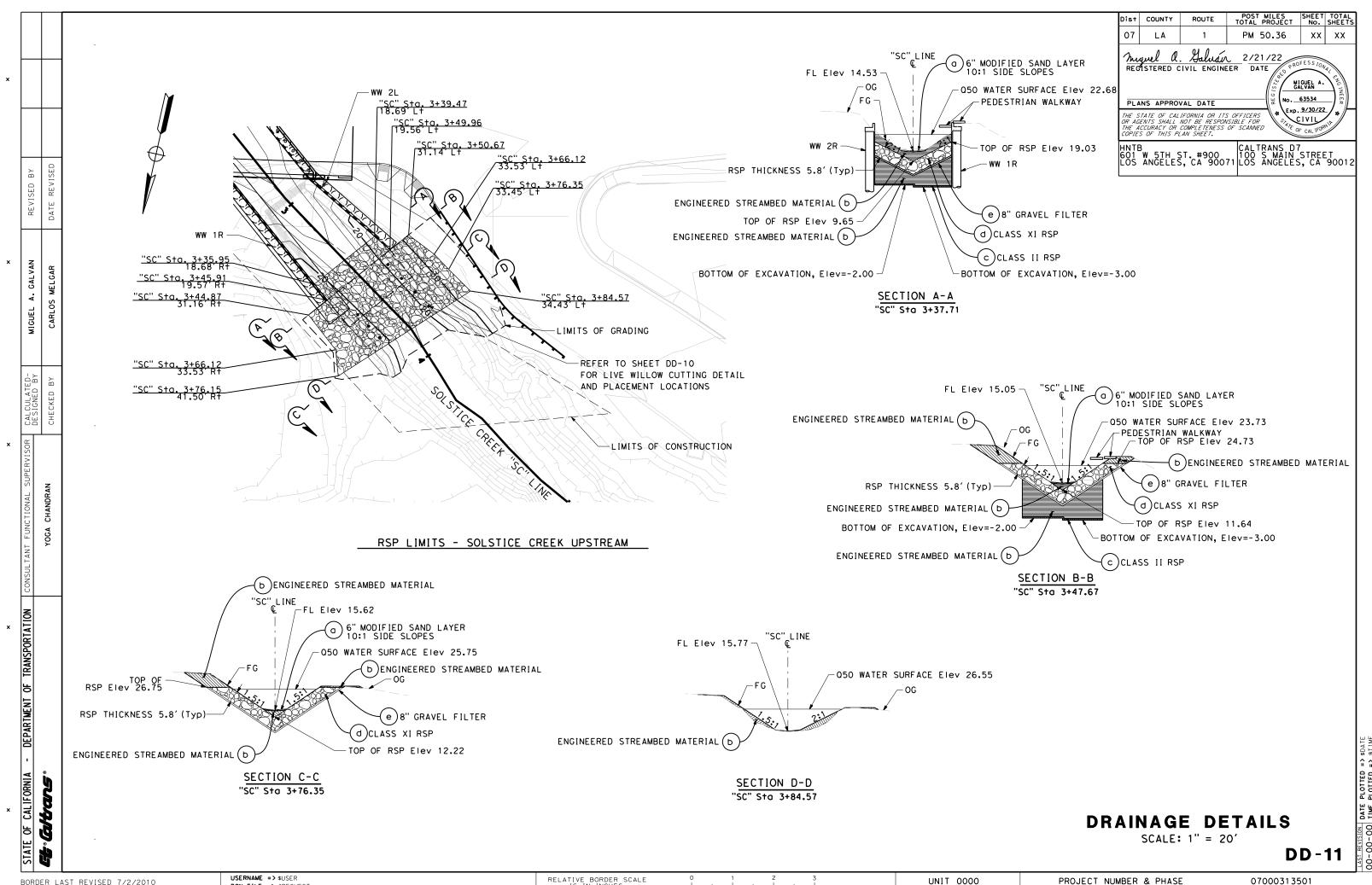
PROJECT NUMBER & PHASE











DGN FILE => \$REQUEST

RELATIVE BORDER SCALE
IS IN INCHES

PROJECT NUMBER & PHASE

x													
REVISED BY	DATE REVISED												
× MIGUEL A. GALVAN	CARLOS MELGAR		SHEET NO.	ON	SAND		RSP	RSP	- FILTER	PERFORATED PLANTING TUBE	CUTTINGS		
CALCULATED- DESIGNED BY	CHECKED BY		DRAINAGE PLAN SHEET NO.	DRAINAGE UNIT	A MODIFIED	E SM	CLASS II	CLASS XI	SO 8" GRAVEL	TA 18" PERFC	MILLOW CI	DESCRIPTION	STATION
x TIONAL SUPERVISOR				8 a	140.8	4750.0						6" MODIFIED SAND LAYER	"SC" Sta 1+06.86 TO 3+94.57
SUPEF	_		_	b c		4359.9	181.6					ENGINEERED STREAMBED MATERIAL (ESM) CLASS II RSP	"SC" Sta 1+06.86 TO 3+94.57 "SC" Sta 1+06.86 TO 3+94.57
NAL 8	CHANDRAN			d			101.0	601.0		7	7	CLASS XI RSP WITH WILLOW CUTTINGS	"SC" Sta 1+06.86 TO 3+94.57
FUNCTIO	CHAN			e				00110	3265.5	·	•	8" GRAVEL FILTER	"SC" Sta 1+06.86 TO 3+94.57
CONSULTANT	YOGA												
ITE OF	G.Citans	·										0 1 2 3	

Dist	COUNTY	ROUTE	POST MILES TOTAL PROJECT	SHEET No.	TOTAL SHEETS
07	LA	1	PM 50.36	xx	xx
REG		IVIL ENGINE	No	11GUEL A. ALVAN 63534	ENG INEER
OR AG THE A	ENTS SHALL	IFORNIA OR ITS NOT BE RESPON COMPLETENESS AN SHEET.	OFFICERS W 🕶 🕻	CIVIL OF CAL IFOR	/ *// I
HNTE 601 LOS	W 5TH S ANGELES	ST. #900 S, CA 900	CALTRANS D 100 S MAIN 71 LOS ANGELE	7 STREI S, CA	₹ 90012

DRAINAGE QUANTITIES

**DQ-4** 

USERNAME => \$USER
DGN FILE => \$REQUEST

PROJECT NUMBER & PHASE

NOTES: PIPE JOINTS ARE STANDARD OR AS NOTED ON PLANS (N) NOT A SEPARATE BID ITEM, FOR INFORMATION ONLY.

REVISED

CALCULATED-DESIGNED BY

FUNCTIONAL SUPERVISOR

STATE OF CALIFORNIA - DEPARTMENT OF TRANSPORTATION

HYDRAULICS

E Cutrans

COUNTY	ROUTE	POST MILES TOTAL PROJECT	SHEET No.	TOTAL SHEETS
LA Ven	1	37.7/62.9 0.0/0.9	13	81
New GISTERED C	IVIL ENGINE	ER DATE	NG-YAI LEE	CNO IN
	LA Ven	LA Ven 1	LA 1 37.7/62.9  Ven 1 0.0/0.9  SISTERED CIVIL ENGINEER DATE	LA 1 37.7/62.9 13  Ven 1 0.0/0.9 13  SISTERED CIVIL ENGINEER DATE WING-YAI LEE CLASS TO LEE CLAS

Exp.6-30-21

PLANS APPROVAL DATE

THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET.

### DRAINAGE QUANTITIES

ON			(N)	TE, DRAINAGE INLET	N AND STEEL	TE (HEADWALL)	TEEL			LF)		ACILITY (EA)	TEEL PIPE	(H=6.0′)	; III, METHOD B)	TON, METHOD B)	8)	EEL PIPE	CONCRETE PIPE	CONCRETE PIPE	CONCRETE PIPE	AND	PIPELINER	PIPELINER	. PIPELINER	AND GRATE Z	(N)		
DRAINAGE PLAN SHEET N	DRAINAGE SYSTEM NO	DRAINAGE UNIT NO	H OR V	STRUCTURAL CONCRE	MISCELLANEOUS IRON	STRUCTURAL CONCRE	BAR REINFORCING ST	REMOVE INLET	REMOVE PIPE	ABANDON CULVERT (L	SAND BACKFILL	REMOVE DRAINAGE F	48" CORRUGATED STE (.138" THICK)	DEBRIS RACK CAGE	RSP (150 LB, CLASS	CONCRETED RSP (1	RSP FABRIC (CLASS	18" CORRUGATED STE (.109" THICK)		REINFORCED CKED)	RFINFORCED	ANING, INSPE PARING CULVE	18" CURED-IN-PLACE	24" CURED-IN-PLACE	48" CURED-IN-PLACE	TYPE 24-12 FRAME	MAXIMUM COVER	DESCRIPTION	STATION
DR	- BR	DR	FT	CY	LB	CY	LB	EA	LF	LF	CY	ΕA	LF	EA	CY	CY	SQYI	D LF	LF	LF	LF	F LF	LF	LF	LF	EA	FT		
D-4	10	а							156																			REMOVE EXIST 18" CSP	
		Ь																20.0									20.0	18" CSP	
		С	10.0	6.16	326																					ı		TYPE G2 INLET; H=10.0'	69.8' R+ 2922+22.00 "CL-ALN1" LINE
		d							15																			REMOVE EXIST 18" RCP	
$\top$		е										ı																REMOVE EXIST JUNCTION STRUCTURE	
SHE	EET T			6,16	326				171						1	1		20.0											1

### DRAINAGE QUANTITIES DQ-1

BORDER LAST REVISED 7/2/2010

USERNAME => s146363 DGN FILE => 0731350id001.dgn RELATIVE BORDER SCALE IS IN INCHES

UNIT 1837

PROJECT NUMBER & PHASE

REVISED CALCULATED-DESIGNED BY FUNCTIONAL SUPERVISOR DEPARTMENT OF TRANSPORTATION HYDRAULICS STATE OF CALIFORNIA Et Gitans

NOTES: PIPE JOINTS ARE STANDARD

- (N) NOT A SEPARATE BID ITEM, FOR INFORMATION ONLY.
- (\*) QUANTITIES LISTED FOR INFORMATION ONLY.

### OR AS NOTED ON PLANS

# TEMPORARY DRAINAGE QUANTITIES (SEE SHEET SC-11 & SC-12)

TEMPORARY DRAINAGE PLAN		TEMPORARY DRAINAGE SYSTEM NO.	TEMPORARY DRAINAGE UNIT NO.	o RSP (300 LB, CLASS IV ≺ METHOD B)	RSP FABRIC (CLASS 8)	TEMPORARY GRAVEL BAG	r TEMPORARY CREEK O DIVERSION SYSTEM	T 78" CORRUGATED STEEL PIPE (.138" THICK)	DESCRIPTION	TEMPORARY DRAINAGE PLAN	TEMPORARY DRAINAGE SYSTEM No.	TEMPORARY DRAINAGE UNIT NO.	C RSP (300 LB, CLASS IV METHOD B)	S RSP FABRIC (CLASS 8)	TEMPORARY GRAVEL BAG	r TEMPORARY CREEK ν DIVERSION SYSTEM	다 78" CORRUGATED STEEL PIPE (.138" THICK)	DESCRIPTION
SC	C-II	1	а					29.5 <sup>(*)</sup>		SC-I	2 2	а					55.5	78" CMP
			Ь					18.9 (*)				Ь					18.8	78" CMP
			С					162.3 (*)	78" CMP			С					162.4	78" CMP
			d					23.2	78" CMP			d					22.2	78" CMP
			е					27.4 (*)	78" CMP			е					19.4	78" CMP
			f					I.4 (*)	78" CMP			f					7.3	78" CMP
			g					59.6 <sup>(*)</sup>	78" CMP			g					1.4	78" CMP
			h			27,648	(*)		GRAVEL BAG			h					59.6	78" CMP
			i	7.41	40.0	)			RSP			i			27,648	I		GRAVEL BAG
	•				•							j	7.41	40.0				RSP
									TEMPORARY DRAINA	AGE S	HEET '	TOTAL	7.41	40.0	27,648	I	346.6	,

POST MILES SHEET TOTAL TOTAL PROJECT No. SHEETS Dist COUNTY LA Ven 37.7/62.9 0.0/0.9 14 07 81 REGISTERED CIVIL ENGINEER DATE No. 61452 PLANS APPROVAL DATE Exp.6-30-21 THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET. OF CAL IFOR

Dist COUNTY 07 LA,Ven **LEGEND:** NOTES: 1. SIGN LOCATIONS SHOWN ARE APPROXIMATE. EXACT LOCATIONS WILL BE DETERMINED BY THE ENGINEER. LOCATION NUMBER STATIONARY MOUNTED CONSTRUCTION AREA SIGNS 2. SIGN POST LENGTH ARE APPROXIMATELY EXACT SIZE AND LENGTH WILL BE DETERMINED BY THE ENGINEER. No. OF SIGNS No. OF POST AND SIZE PANEL SIZE SIGN MESSAGE No. & DESIGNATION 3. SEE STAGE CONSTRUCTION AND TRAFFIC HANDLING SHEETS FOR ADDITIONAL STATIONARY MOUNTED AREA SIGNS. 1 - 6" X 6" Α 48" X 48" ROAD WORK AHEAD 2 W20-1 TRAFFIC FINES DOUBLED IN CONSTRUCTION ZONES В C40(CA) 144" X 60" 2 - 6" X 6" 2 G20-2 48" × 24" END ROAD WORK 1 - 4" X 6" 2 REVISED C3OA(CA)-2 48" × 24" SHOULDER CLOSED 1 - 4" X 6" 2 **MALIBU** ROUTE 1 MALIBU BEACH RV PARK 10 DEPARTMENT OF TRANSPORTATION DESIGN STATE OF CALIFORNIA altans

37.7/62.9 0.0/0.9 124

Madhan Cin REGISTERED CIVIL ENGINEER

05-28-21 DATE

NATHAN OUM ю. <u>С71418</u>

Exp.12-31-2

CIVIL

PLANS APPROVAL DATE

THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET.

**CONSTRUCTION AREA SIGNS** 

NO SCALE

CS-1

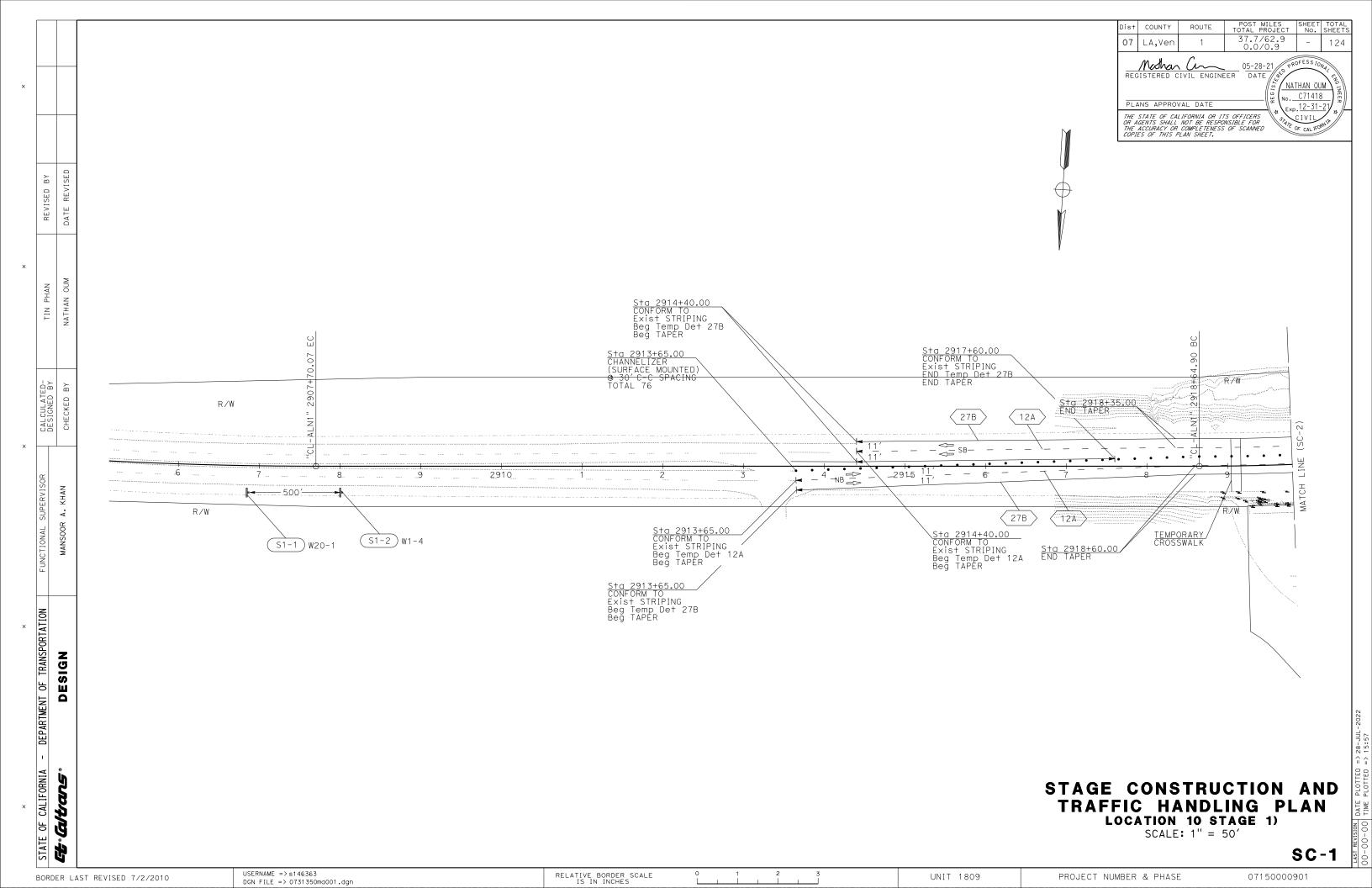
BORDER LAST REVISED 7/2/2010

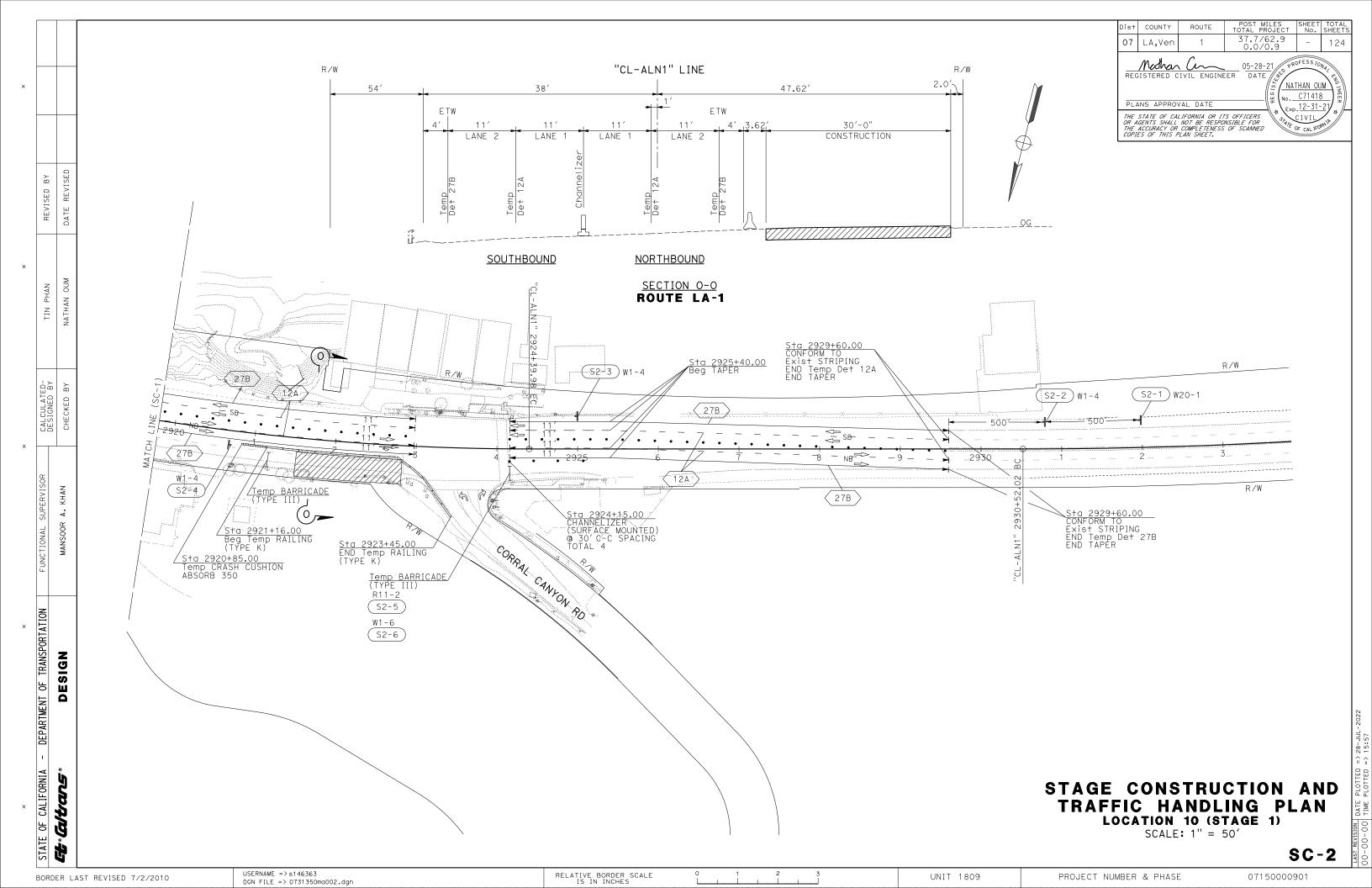
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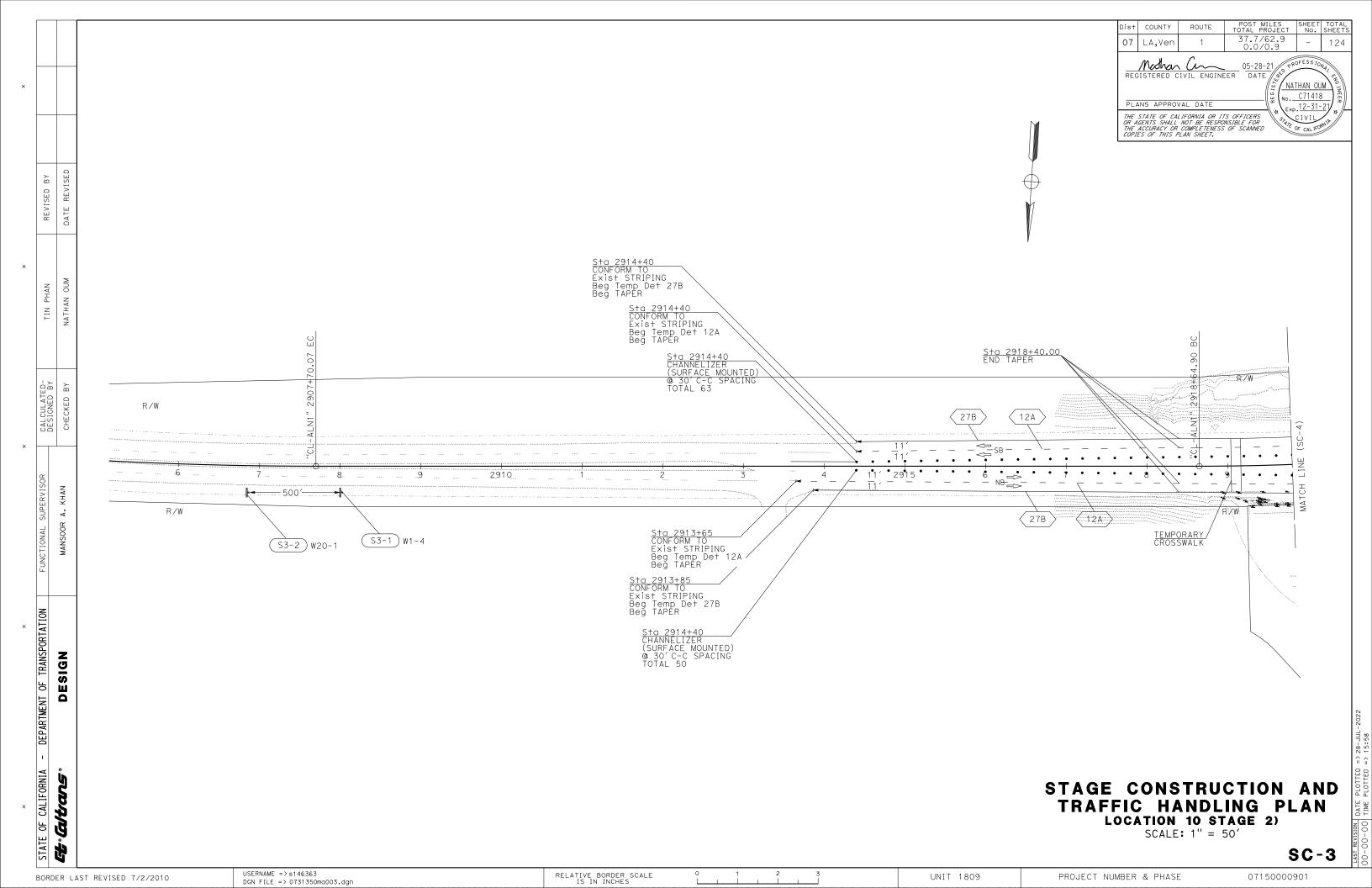
PROJECT NUMBER & PHASE

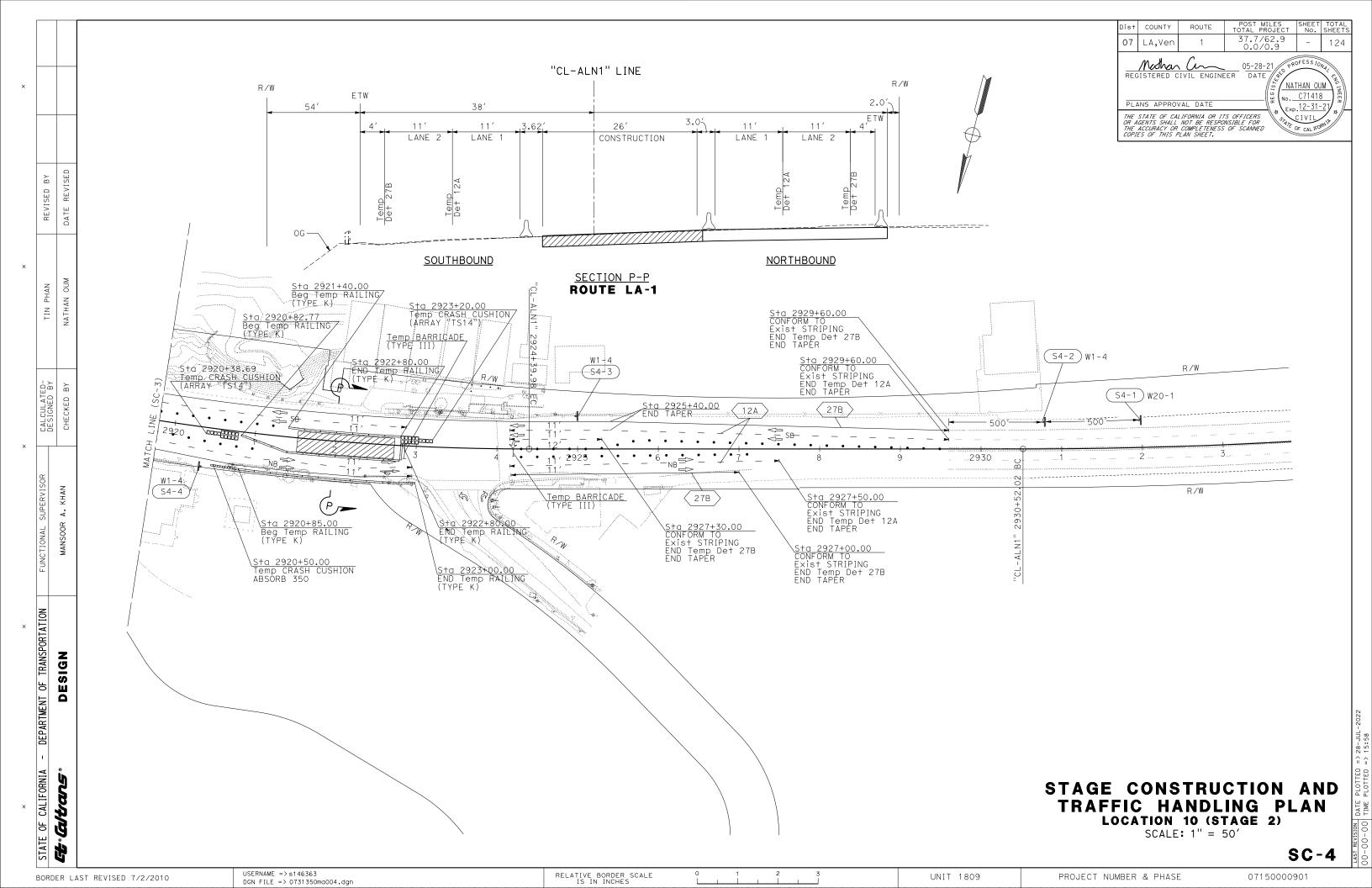
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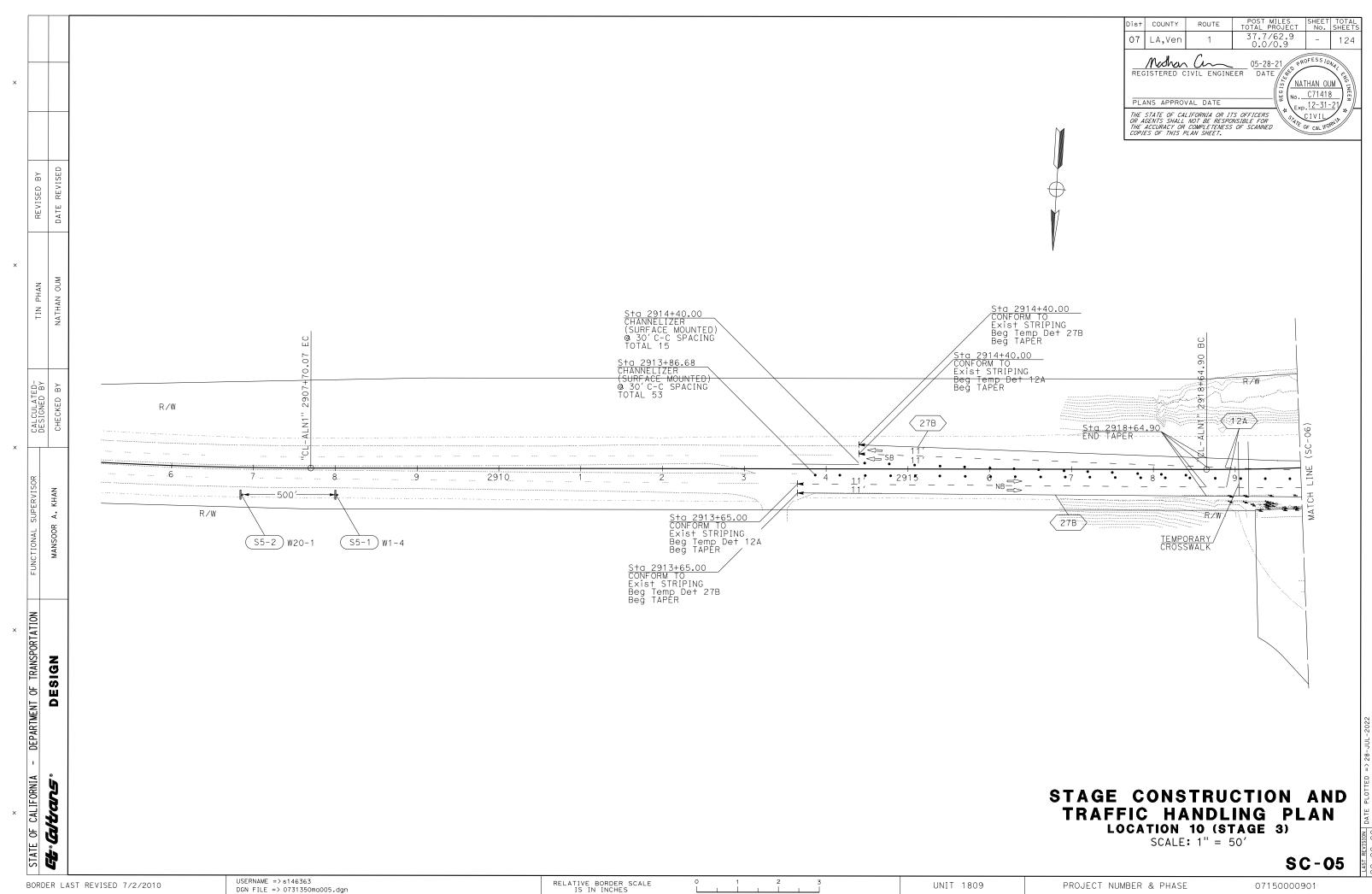
UNIT 1809



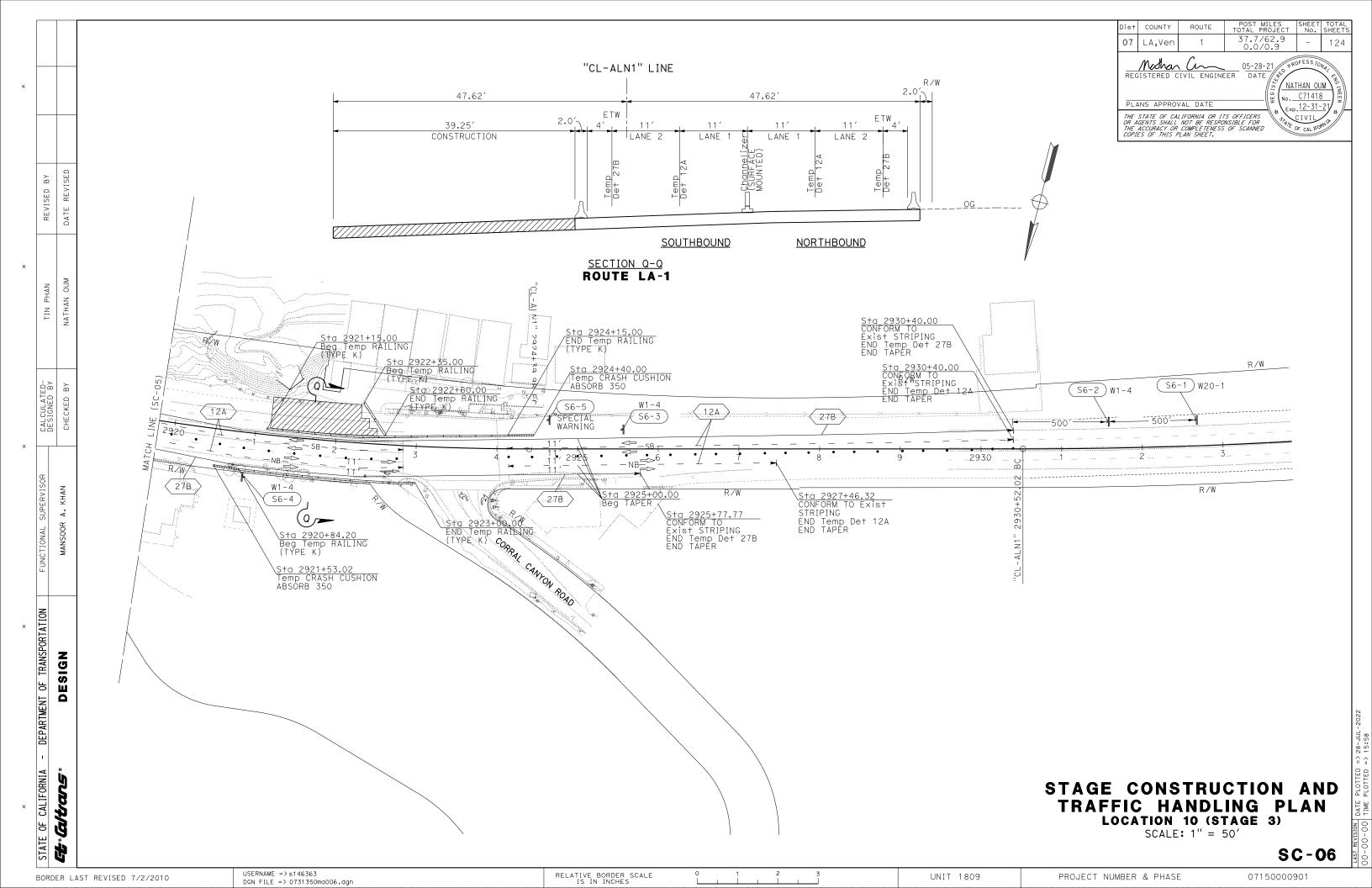


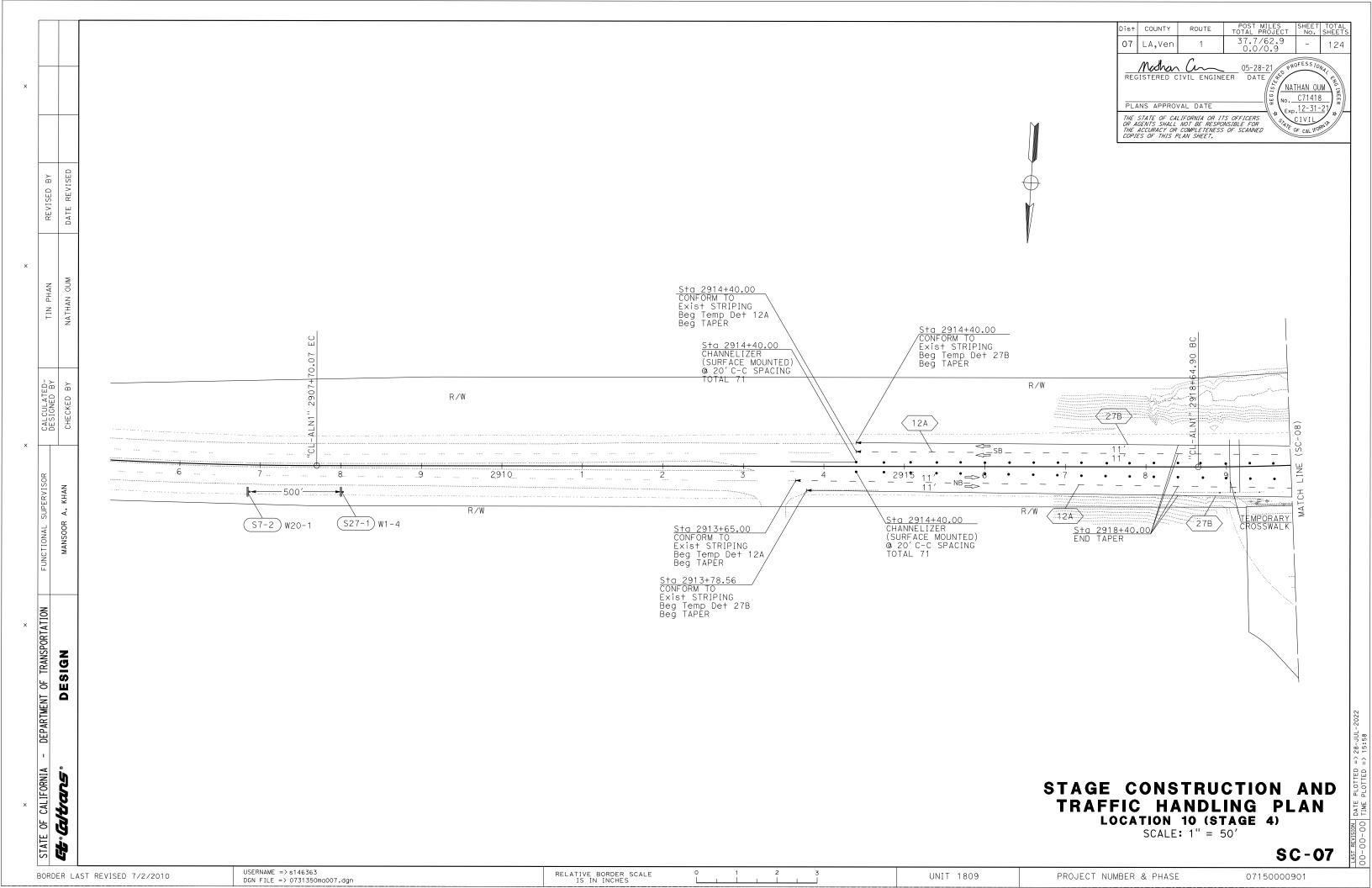


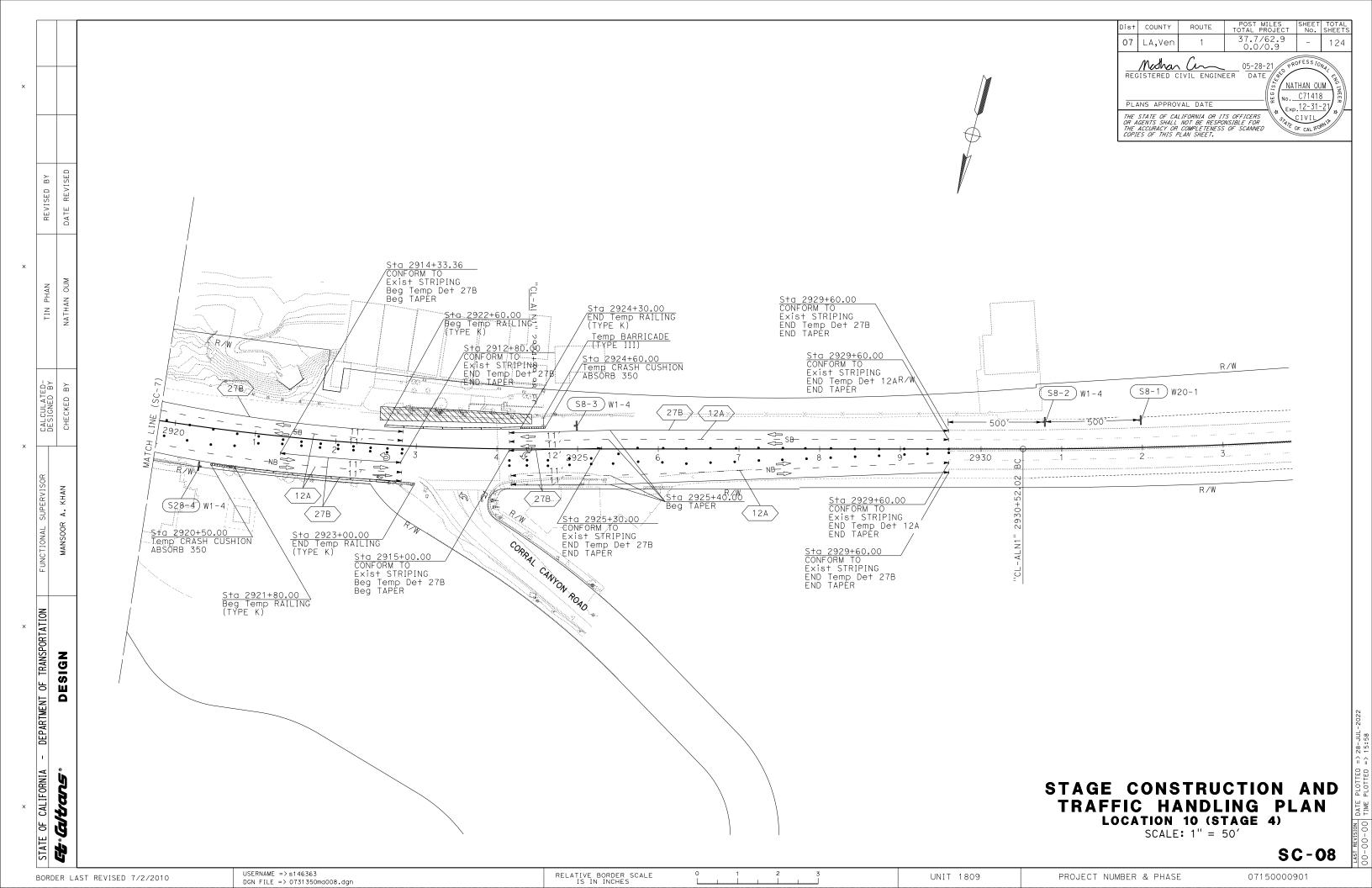


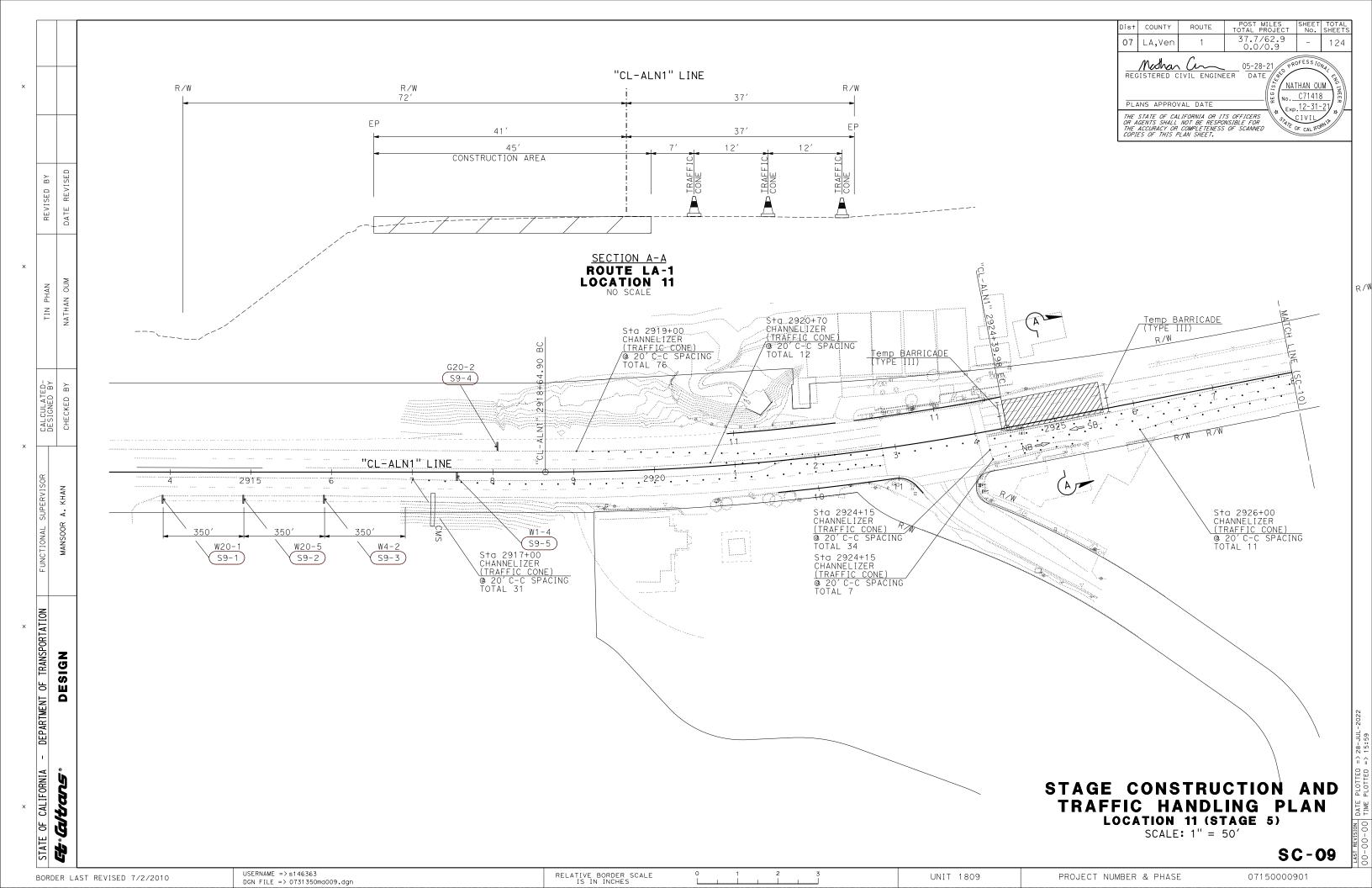


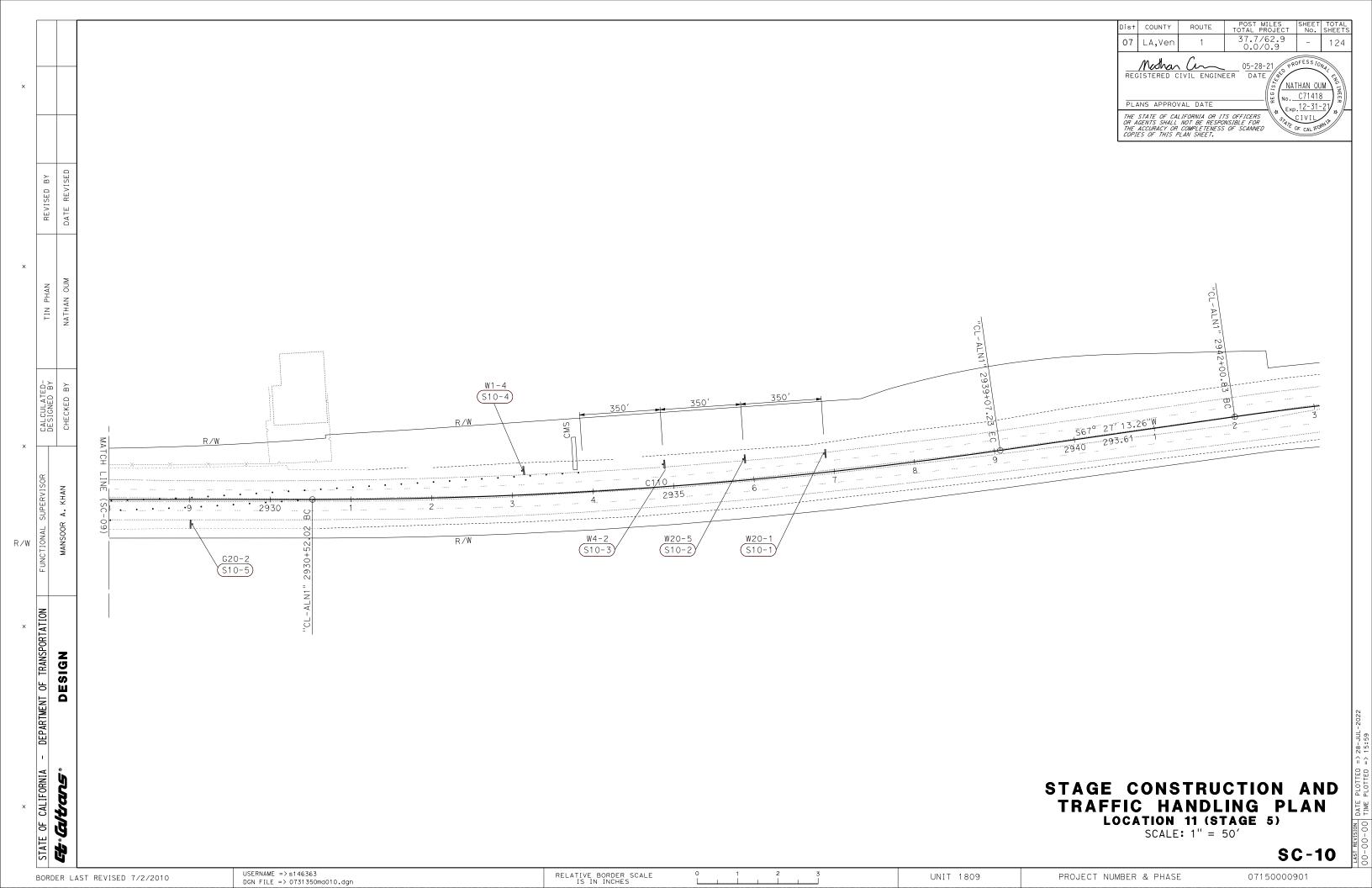
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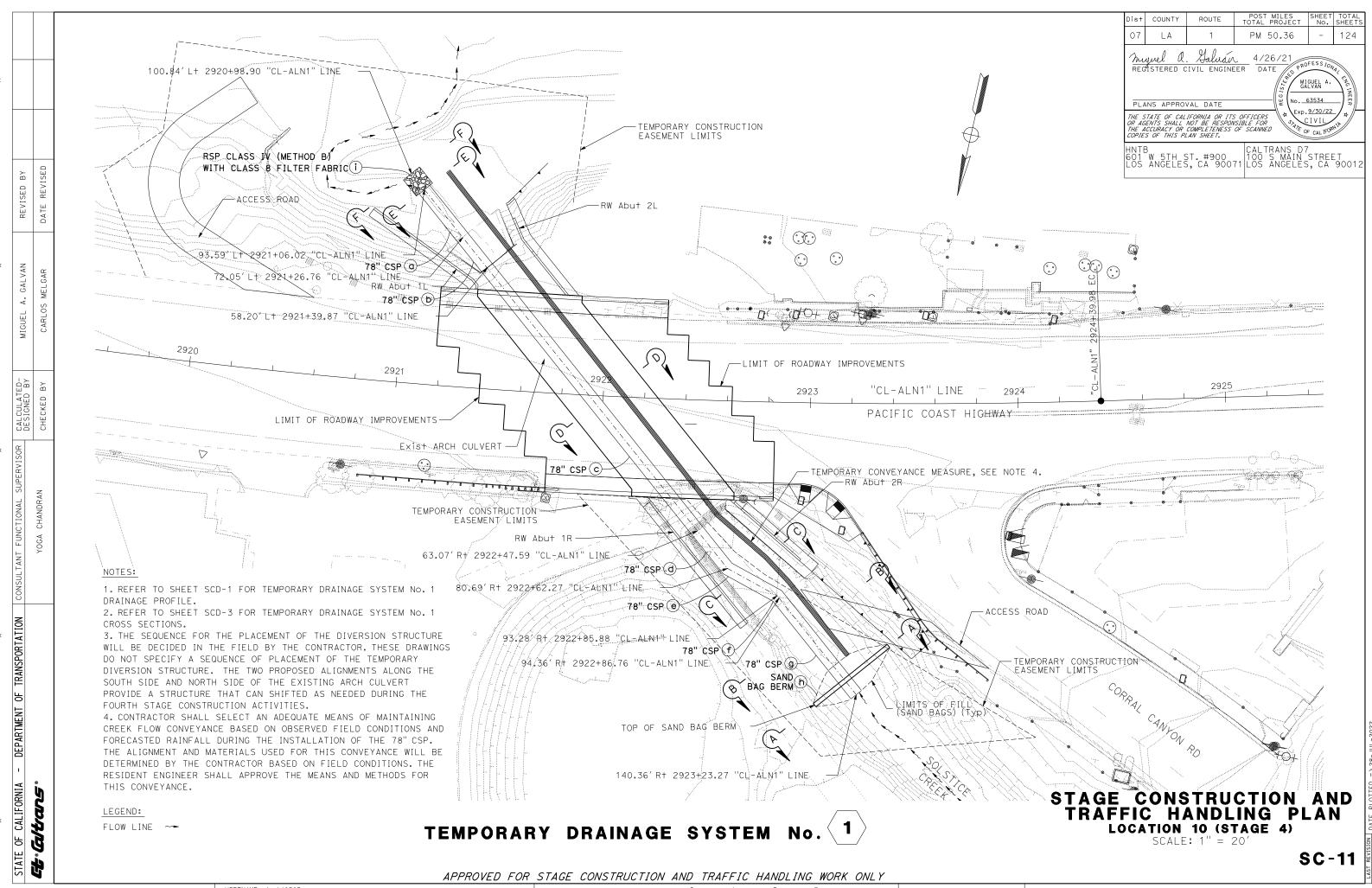










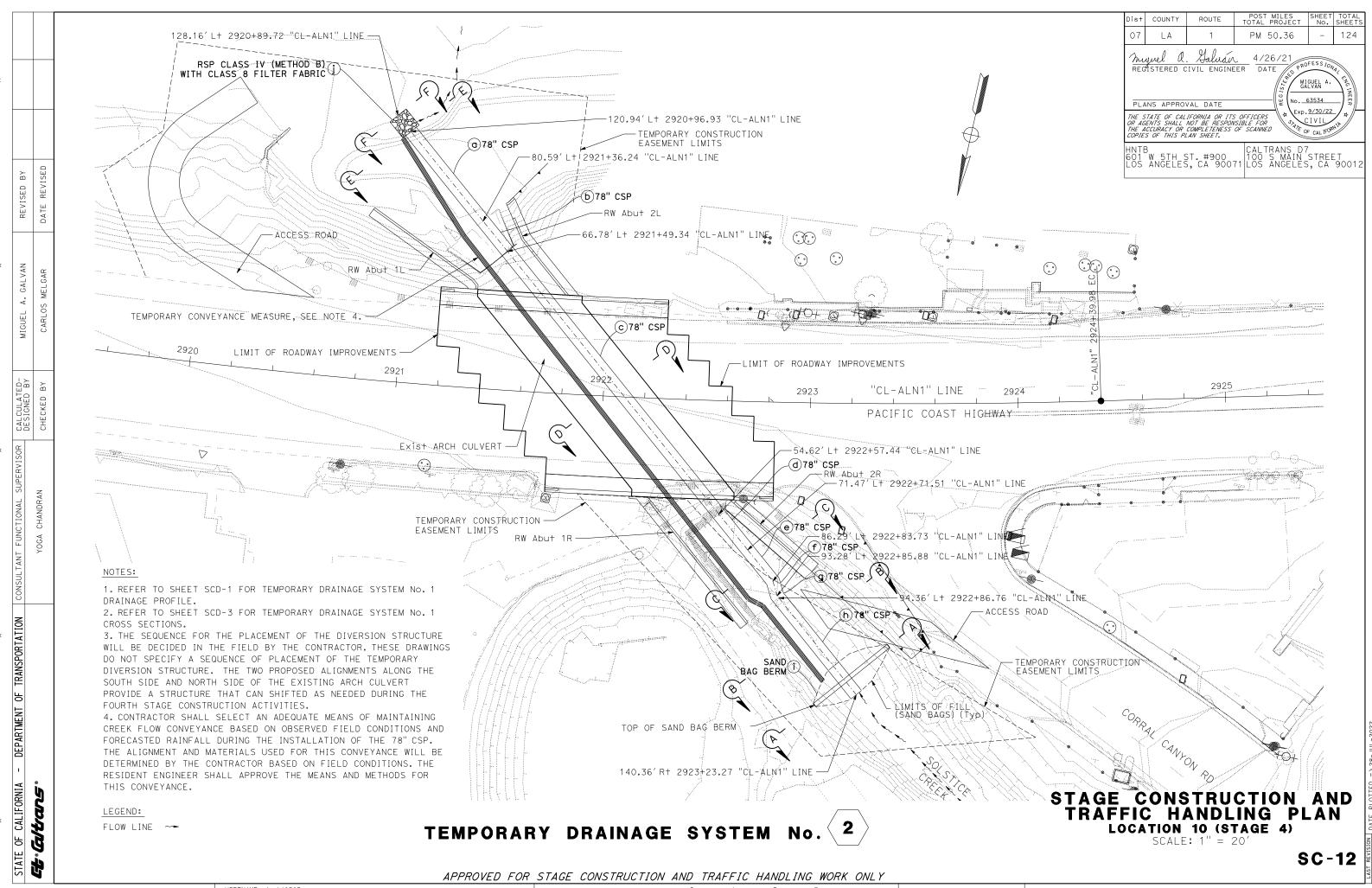


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PROJECT NUMBER & PHASE

07000313501

UNIT 0000

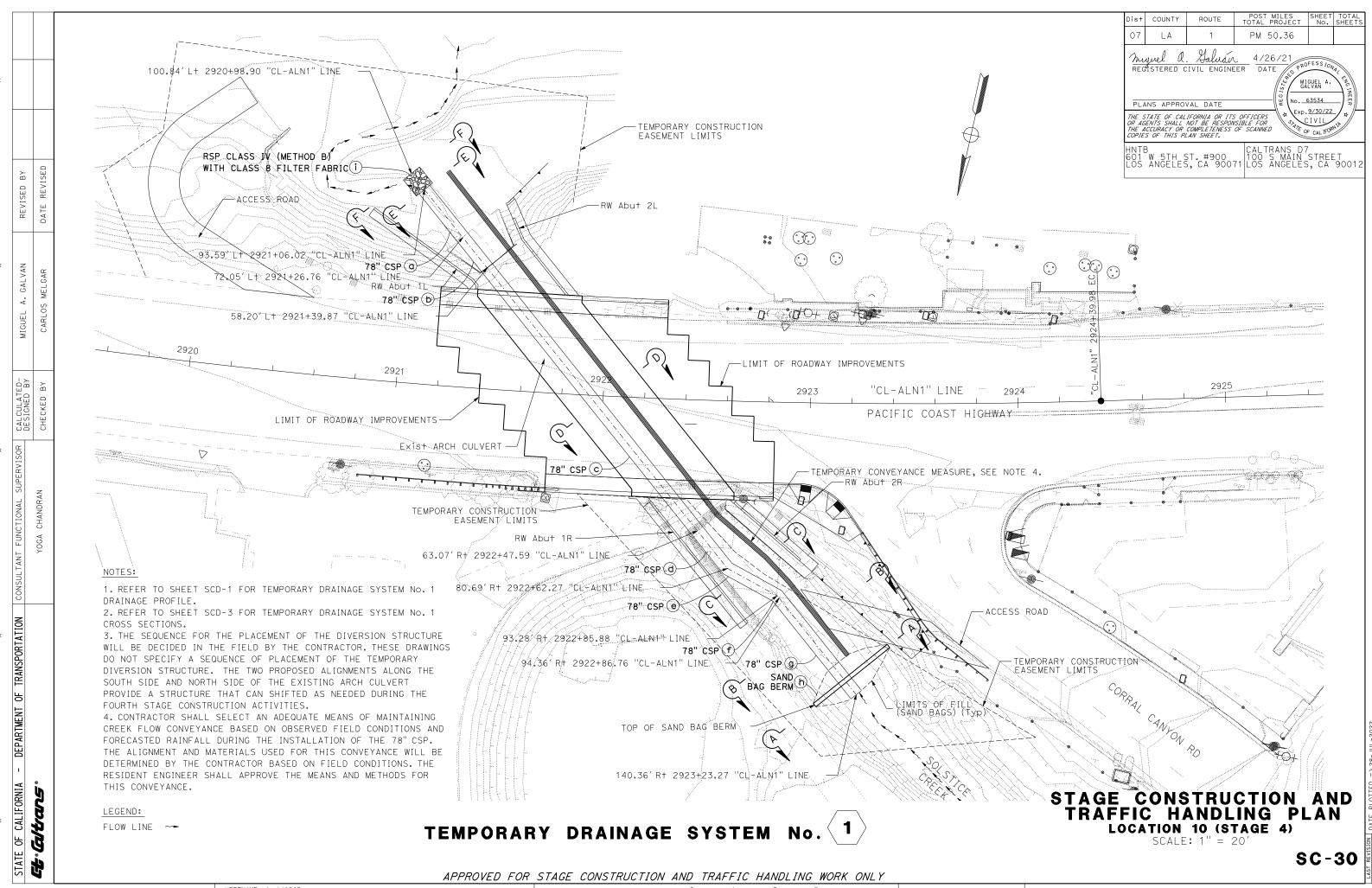


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UNIT 0000

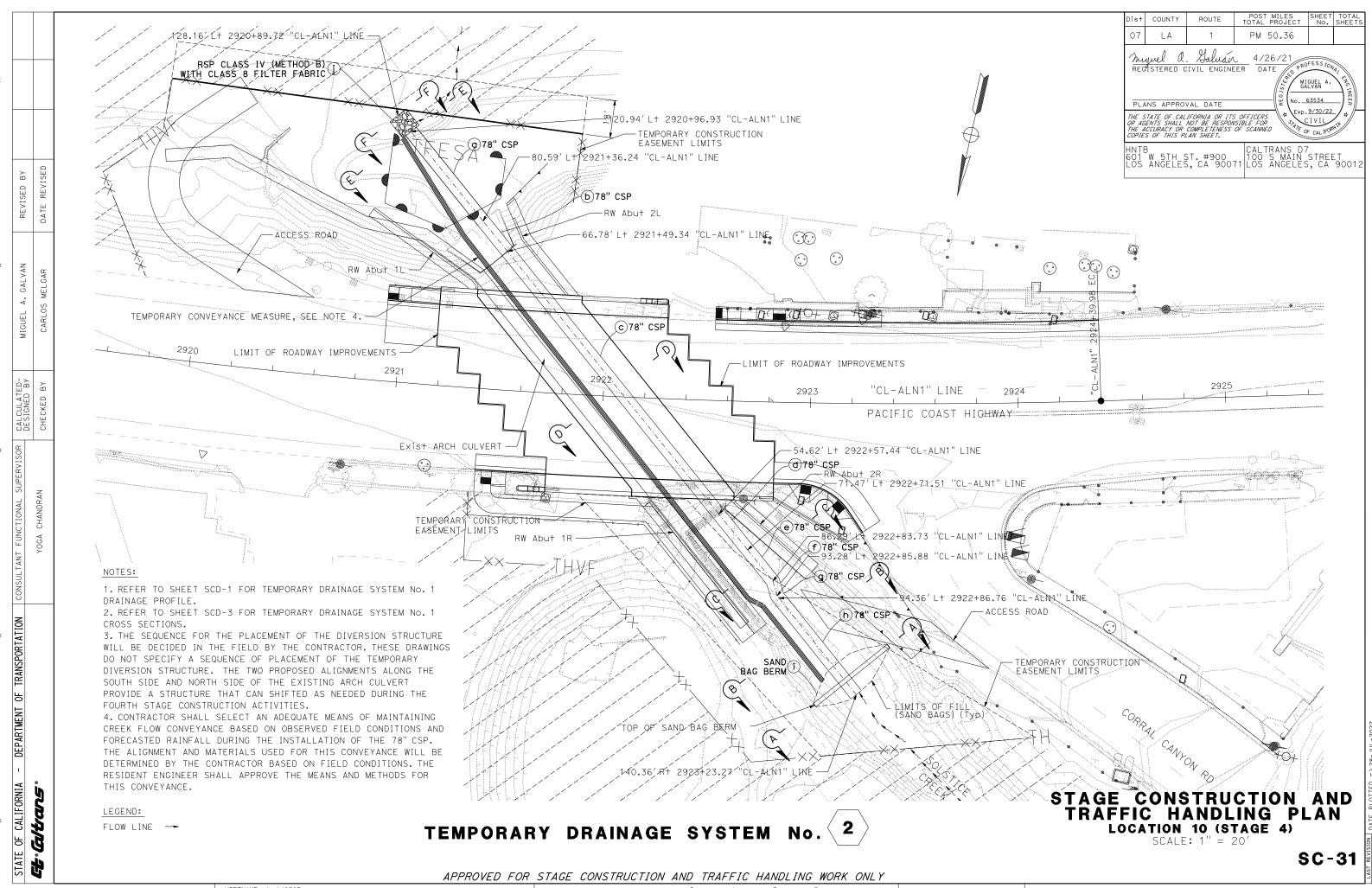
PROJECT NUMBER & PHASE



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PROJECT NUMBER & PHASE

UNIT 0000

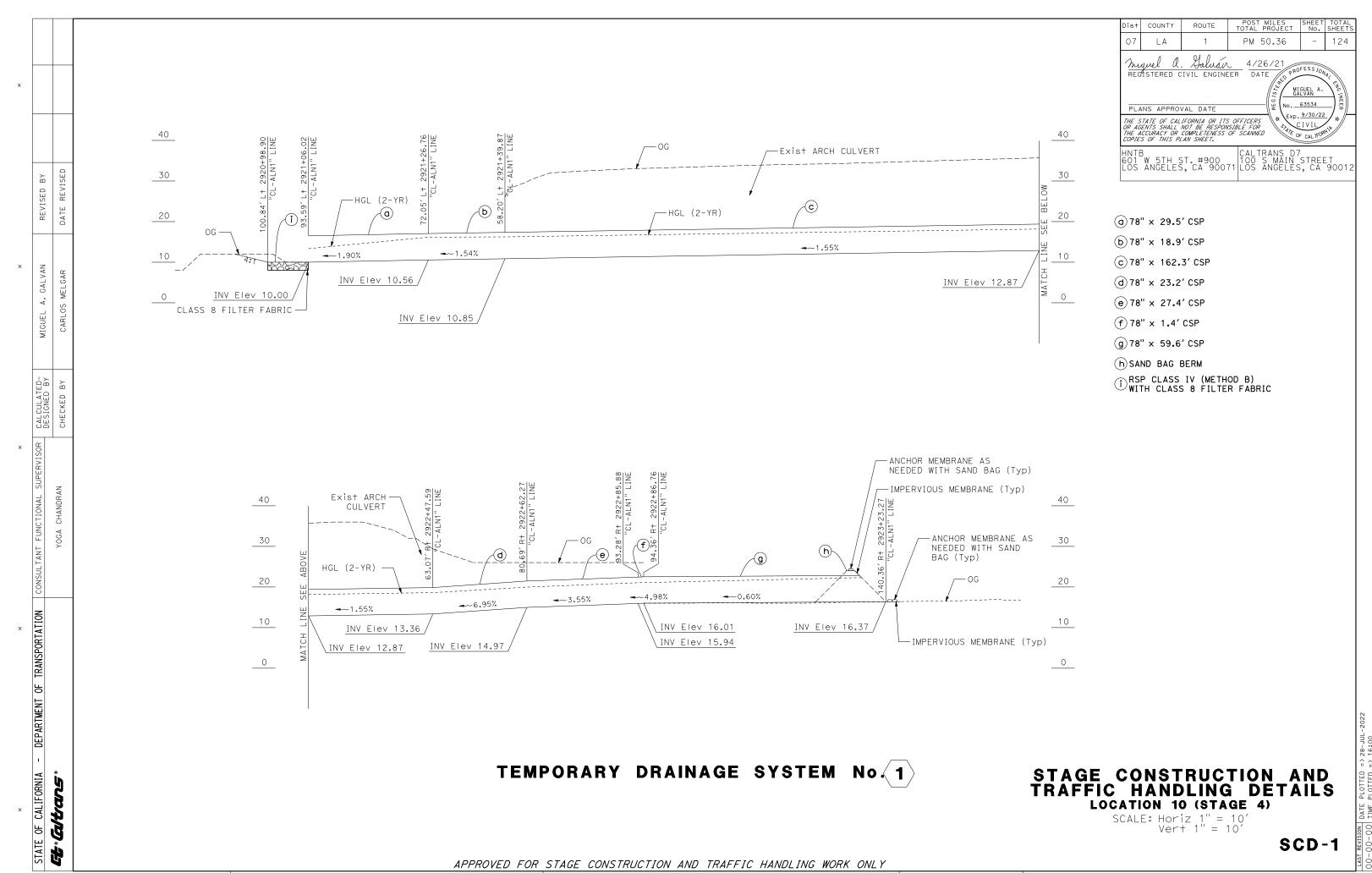


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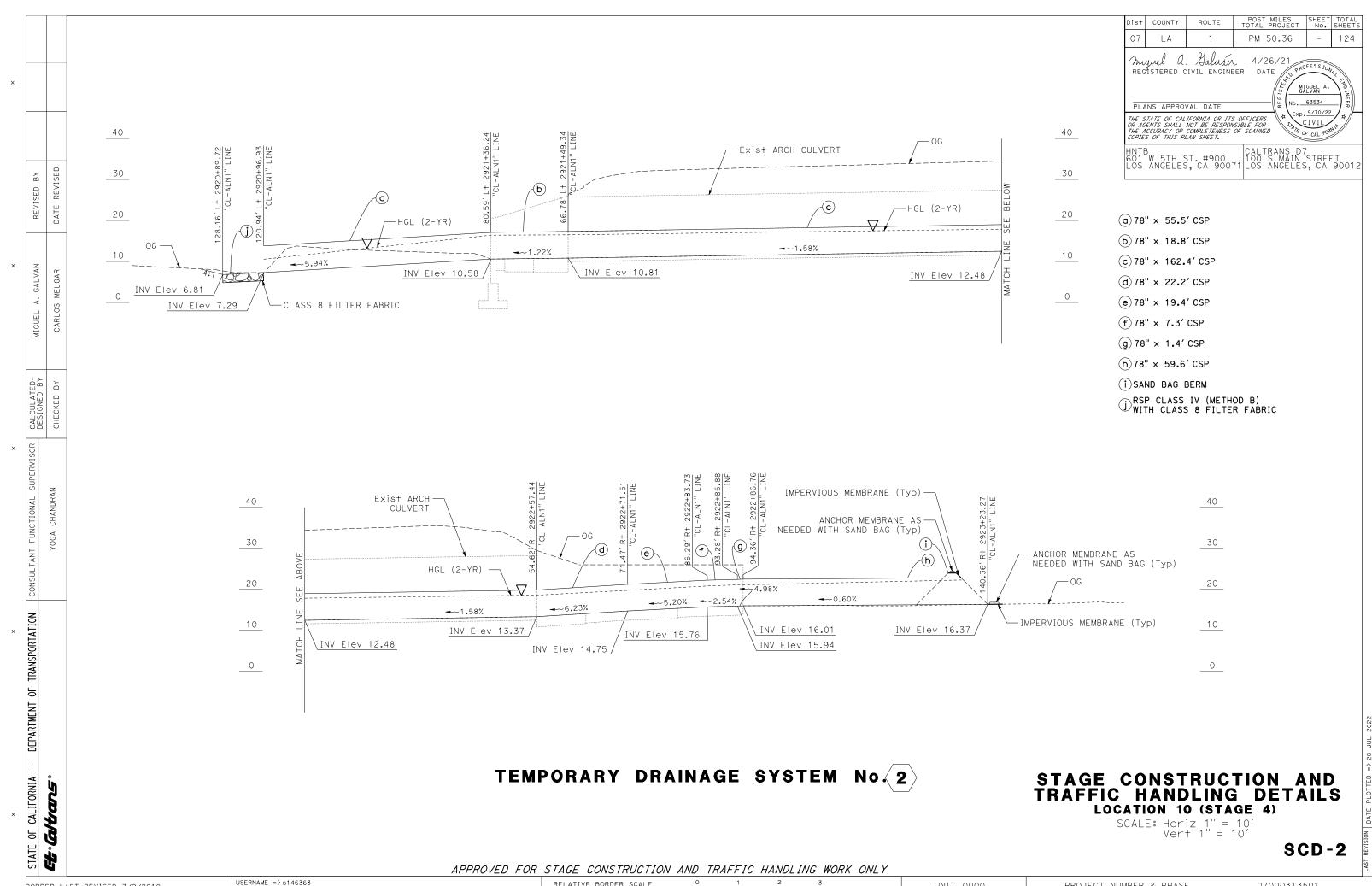
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UNIT 0000

PROJECT NUMBER & PHASE



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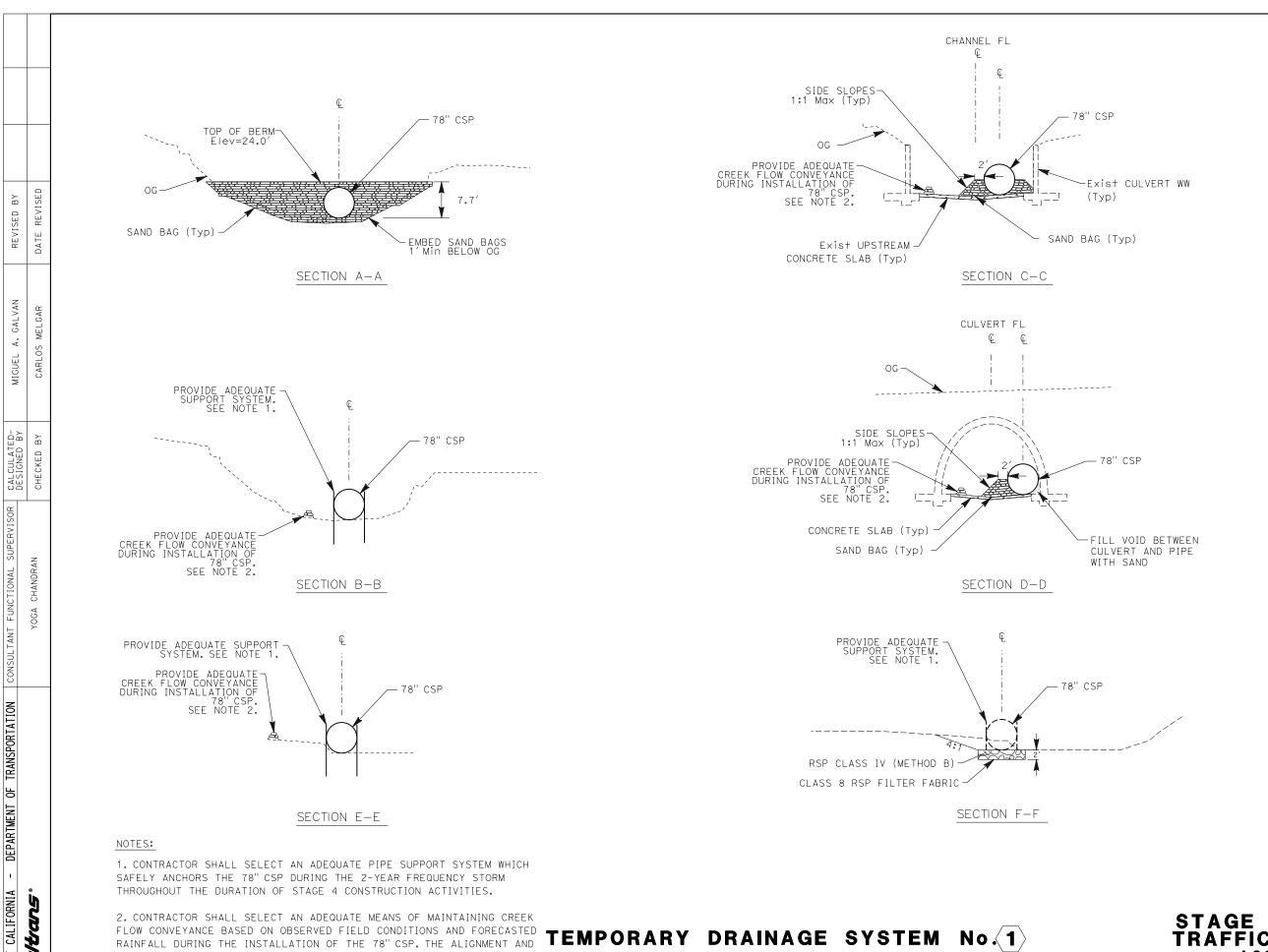


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RELATIVE BORDER SCALE IS IN INCHES

UNIT 0000

PROJECT NUMBER & PHASE



STAGE CONSTRUCTION AND TRAFFIC HANDLING DETAILS LOCATION 10 (STAGE 4)

NO SCALE

SCD-3

USERNAME => s146363 BORDER LAST REVISED 7/2/2010

PROJECT NUMBER & PHASE

07000313501

POST MILES SHEET TOTAL PROJECT No.

MIGUEL A. GALVAN

63534

Exp. 9/30/22

CIVIL

124

PM 50.36

3/18/21

07

LΑ

PLANS APPROVAL DATE

Myrel a. Haluan 3/18/ REGISTERED CIVIL ENGINEER DATE

THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET.

HNTB 601 W 5TH ST. #900 LOS ANGELES, CA 90071 LOS ANGELES, CA 9001

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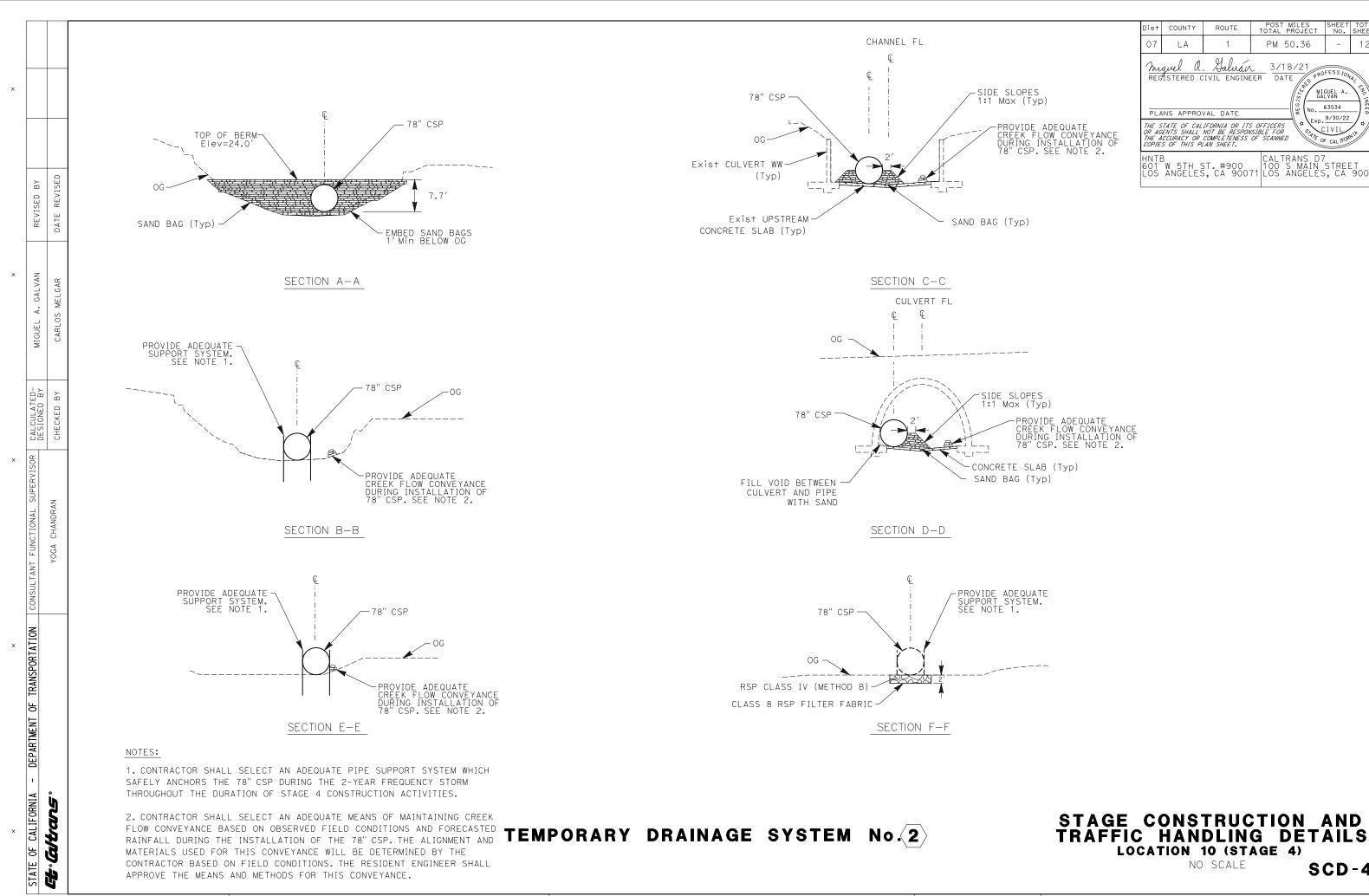
UNIT 0000

DEPARTMENT OF TRANSPORTATION | CONSULTANT FUNCTIONAL

OF CALIFORNIA

MATERIALS USED FOR THIS CONVEYANCE WILL BE DETERMINED BY THE CONTRACTOR BASED ON FIELD CONDITIONS. THE RESIDENT ENGINEER SHALL APPROVE THE MEANS AND METHODS FOR THIS CONVEYANCE.

RELATIVE BORDER SCALE IS IN INCHES



PM 50.36

3/18/21

124

MIGUEL A. GALVAN

63534

Exp. 9/30/22

SCD-4

USERNAME => s146363 RELATIVE BORDER SCALE IS IN INCHES UNIT 0000 PROJECT NUMBER & PHASE 07000313501 BORDER LAST REVISED 7/2/2010 DGN FILE => 0731350mb004.dgn

## STAGE CONSTRUCTION QUANTITIES

			STRIPE	Y TRAFFIC (TAPE)								ED A	
			6" V	VHITE							ы	SPE	
SHEET No.	LOCATION/STATION	IRECTION	DETAIL 12A	DETAIL 27B	TRAFFIC CONE	CHANNELIZER (SURFACE MOUNTED)	TYPE III BARRICADE	TEMPORARY RAILING (TYPE K)	TEMPORARY CRASH CUSHION MODULE (ABSORB 350)	TEMPORARY CRASH CUSHION MODULE	CHANGEABLE MESSAGE SIGN	TEMPORARY RADAR SPEED FEEDBACK SIGN SYSTEM	DESCRIPTION/ COMMENTS
			LF	LF	EA	EA	EA	LF	EA	EA	EA	EA	
STAGE 1													
SC-1 TO SC-12	"CL-ALN1" 2913+65 TO 2929+60	NB	1,480	1,480		80	2	260	10		1		
SC-1 TO SC-12	"CL-ALN1" 2914+40 TO 2929+60	SB	1,400	1,720							1		
STAGE 2			ı	1			ı	1			ı	1	
SC-3 TO SC-4	"CL-ALN1" 2913+65 TO 2927+50	NB	1,275	1,205		50		440	10	14	1		
SC-3 TO SC-4	"CL-ALN1" 2914+40 TO 2929+60	SB	1,505	1,515		63	2	140		14	1		
STAGE 3		1					I				1	1	
SC-5 TO SC-6	"CL-ALN1" 2913+65 TO 2930+40	NB	1,265	1,230		53		220	10		1		
SC-5 TO SC-6	"CL-ALN1" 2914+40 TO 2930+40	SB	1,480	1,605		15		340	10		1		
STAGE 4		110	4 475	1 010			ı		1.0				
SC-7 TO SC-8	"CL-ALN1" 2913+65 TO 2929+60	NB	1,475	1,610		71		220	10		1		
SC-7 TO SC-8	"CL-ALN1" 2914+40 TO 2929+60	SB	1,390	1,505		71	1	160	10		1		
STAGE 5	"OL ALNA" 0047.00 TO 0000.00	NE			0.7		4				1		
SC-9 TO SC-10	"CL-ALN1" 2917+00 TO 2928+00	NB			83		4				1		
SC-9 TO SC-10	"CL-ALN1" 2919+00 TO 2934+00	SB			88						1		
	SUB T	OTAL	11,270	11,870									
	GRAND T	OTAL	23,	140	171	403	9	1,780	60	28	10	12	

## SUMMARY OF QUANTITIES

SCQ-1

BORDER LAST REVISED 7/2/2010

REVISED

FUNCTIONAL SUPERVISOR

STATE OF CALIFORNIA - DEPARTMENT OF TRANSPORTATION

GEOGRAPION

DESIGN

USERNAME => s146363 DGN FILE => 0731350mc001.dgn

UNIT 1809

PROJECT NUMBER & PHASE

#### STATIONARY MOUNTED CONSTRUCTION AREA SIGNS

		ION					POST S	IZE AND	LENGTH	ROADSIG	GN SIGN	(N) DOADSIDE SION	
SHEET NUMBER	SIGN NUMBER	DIRECTION	SIGN DESIGNATION	SIGN MESSAGE	SIGN PANEL SIZE (INCHES)	"C" Dim	4"X4"	4"X6"	6"X6"	ONE POST	TWO POST	(N) ROADSIDE SIGN MOUNTED ON TYPE III BARRICADE	REMARKS
		DI				LF	LF	LF	LF	EA	EΑ	EA	
STAGE 1													
SC-1	S1-1	NB	W20-1	ROADWORK AHEAD	48 × 48	7		14		1			
SC-1	S1-2	NB	W1-4	REVERSE CURVE	48 × 48	7		14		1			
SC-2	S2-1	SB	W20-1	ROADWORK AHEAD	48 × 48	7		14		1			
SC-2	S2-2	SB	W1-4	REVERSE CURVE	48 × 48	7		14		1			
SC-2	S2-3	SB	W1-4	REVERSE CURVE	48 × 48	7		14		1			
SC-2	S2-4	NB	W1-4	REVERSE CURVE	48 × 48	7		14		1			
STAGE 2									1	T			
SC-3	S3-1	NB	W1 - 4	REVERSE CURVE	48 × 48	7		14		1			
SC-3	S3-2	NB	W20-1	ROADWORK AHEAD	48 × 48	7		14		1			
SC-4	S4-1	SB	W20-1	ROADWORK AHEAD	48 × 48	7		14		1			
SC-4	S4-2	SB	W1 - 4	REVERSE CURVE	48 × 48	7		14		1			
SC-4	S4-3	SB	W1 - 4	REVERSE CURVE	48 × 48	7		14		1			
SC-4	S4-4	NB	W1 - 4	REVERSE CURVE	48 × 48	7		14		1			
STAGE 3					10 10	_							
SC-5	S5-1	NB	W20-1	ROADWORK AHEAD	48 × 48	7		14		1			
SC-5	S5-2	NB	W1-4	REVERSE CURVE	48 × 48	7		14		1			
SC-6	S6-1	SB	W20-1	ROADWORK AHEAD	48 × 48	7		14		1			
SC-6	S6-2	SB	W1 - 4	REVERSE CURVE	48 × 48	7		14		1			
SC-6 SC-6	S6-3 S4-4	SB	W1 - 4	REVERSE CURVE	48 × 48	7		14		1			
STAGE 4	54-4	NB	W1-4	REVERSE CURVE	48 × 48	1		14		1			
	S7-1	ND	W1-4	REVERSE CURVE	10 11 10	7		14		1			
SC-7	S7-2	NB			48 × 48 48 × 48	7		14		1			
SC-7 SC-8	S8-1	NB SB	W20-1 W20-1	ROADWORK AHEAD ROADWORK AHEAD	48 × 48 48 × 48	7		14		1			
SC-8	S8-1	SB	W20-1 W1-4	REVERSE CURVE	48 × 48	7		14		1			
SC-8	S8-3	SB	W1 - 4	REVERSE CURVE	48 × 48	7		14		1			
SC-8	S8-4	NB	W1-4 W1-4	REVERSE CURVE	48 × 48	7		14		1			
STAGE 5	30 1	טאו	W1 7	MEVENSE CONVE	70 / 70	1		17		1			
SC-9	S9-1	SB	W20-1	ROADWORK AHEAD	48 × 48	7		14		1			
SC-9	S9-2	SB	W20-5	RIGHT LANE CLOSED	48 × 48	7		14		1			
SC-9	S9-3	SB	W4-2	LANE ENDS	48 × 48	7		14		1			
SC-9	S9-4	NB	G20-2	END ROAD WORK	48 × 24	7		14		1			
SC-9	S9-5	SB	W1 - 4	REVERSE CURVE	48 × 48	7		14		1			
SC-10	S10-1	SB	W20-1	ROADWORK AHEAD	48 × 48	7		14		1			
SC-10	S10-2	SB	W20-5	RIGHT LANE CLOSED	48 × 48	7		14		1			
SC-10	S10-3	SB	W4-2	LANE ENDS	48 × 48	7		14		1			
SC-10	S10-4	NB	W1-4	REVERSE CURVE	48 × 48	7		14		1			
SC-10	S10-5	SB	G20-2	END ROAD WORK	48 × 24	7		14		1			
					1			T.	TOTAL	71			

## SUMMARY OF QUANTITIES

SCQ-2

BORDER LAST REVISED 7/2/2010

STATE OF CALIFORNIA - DEPARTMENT OF TRANSPORTATION

DESIGN

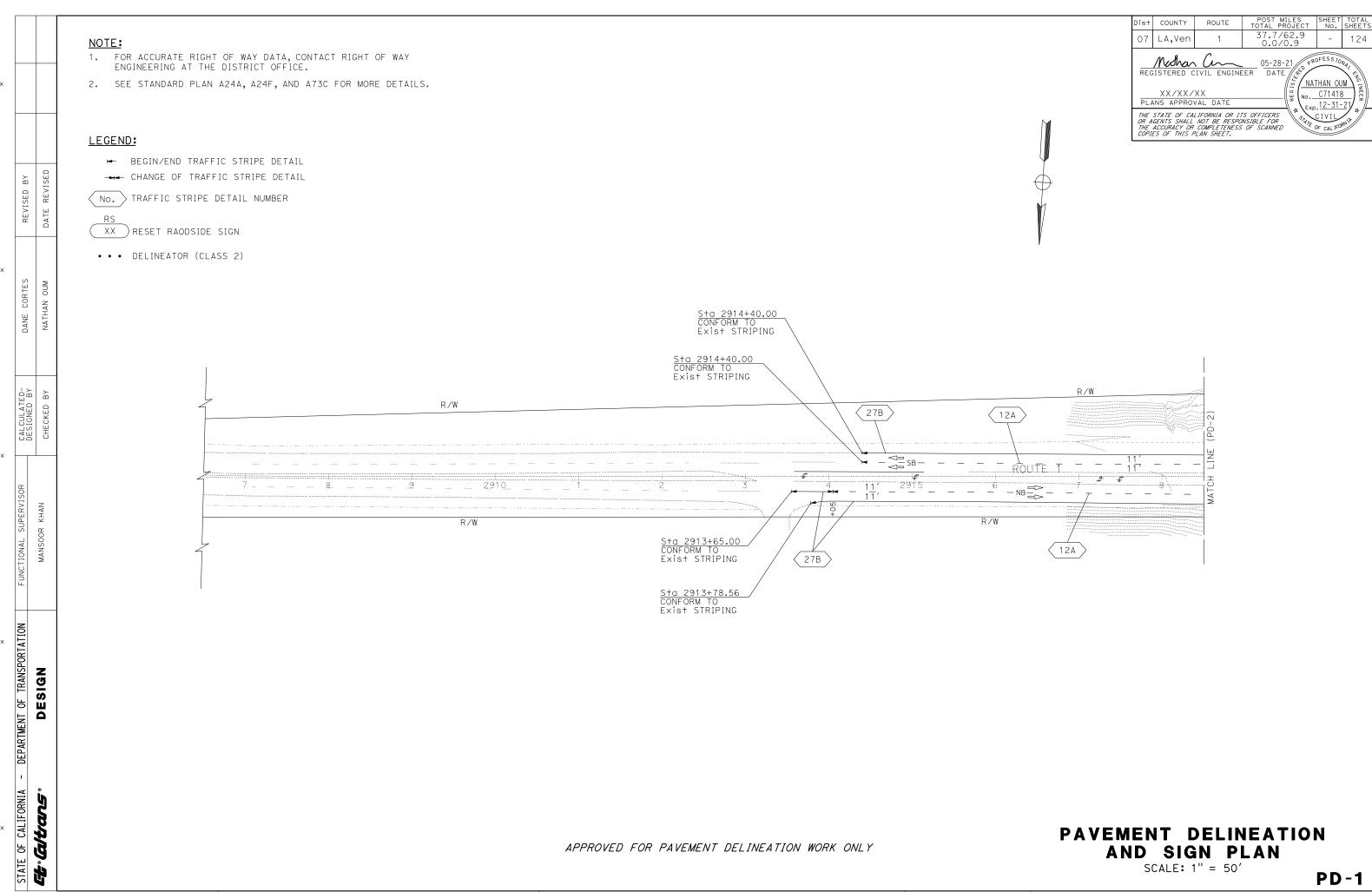
REVISED

USERNAME => s146363 DGN FILE => 0731350mc002.dgn RELATIVE BORDER SCALE IS IN INCHES

PROJECT NUMBER & PHASE

07150000900

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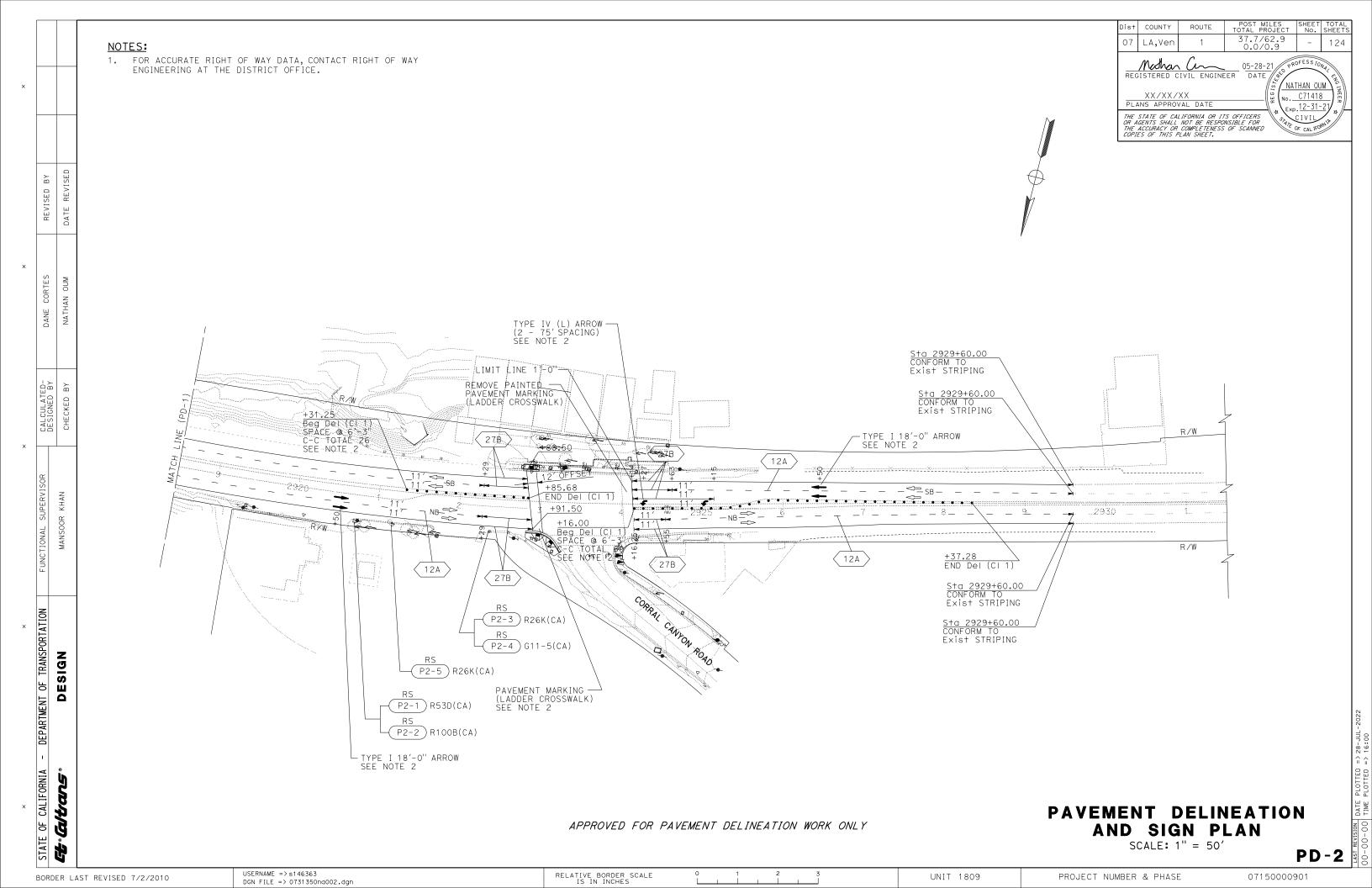


BORDER LAST REVISED 7/2/2010

DGN FILE => 0731350na001.dgn

RELATIVE BORDER SCALE IS IN INCHES

07150000901



	TAVENDERT DEFINEATION COARTITIES														
SHEET No.	LOCATION/STATION	NOI	E DETAIL	NEATOR (CLASS 1)	6" THERMOPLASTIC TRAFFIC STRIPE (BROKEN 18-12)	HERMOPLASTIC FFIC STRIPE	REMOVE THERMOPLASTIC PAVEMENT MARKING	HERMOPLASTIC ROSSWALK AND YAVEMENT MARKING		ENT MA ROFLEC		COMMENTS			
		IREC.	TRIP	DELINEA	6" T TRAF (BRC	6" TH TRAFI	REMC PAVE	THEF CROS PAVE	TYPE C	TYPE D	TYPE RY				
			S -	EΑ	LF	LF	SQFT	SQFT	EA	EΑ	EΑ				
PD-1 - PD-2	"CL-ALN1" 2914+40.00 TO 2929+60.00	SB	27B			1,595						SEE Std PLAN A20B.			
PD-2	"CL-ALN1" 2926+50.00	SB						50				TYPE I 18'-0" ARROW. SEE Std PLAN A24A.			
PD-1 - PD-2	"CL-ALN1" 2914+40.00 TO 2929+60.00	SB	12A		1,300				28			SEE Std PLAN A20A.			
PD-2	"CL-ALN1" 2924+27.00 TO 2925+15.00	SB						30				TYPE IV (L) ARROW. SEE Std PLAN A24A.			
PD-1 - PD-2	"CL-ALN1" 2913+65.00 TO 2929+60.00	NB	27B			1,622						SEE Std PLAN A20B.			
PD-2	"CL-ALN1" 2920+50.00	NB						50				TYPE I 18'-0" ARROW. SEE Std PLAN A24A.			
PD-1 - PD-2	"CL-ALN1" 2914+05.00 TO 2929+60.00	NB	12A		1,330				29			SEE Std PLAN A20A.			
PD-2	"CL-ALN1" 2912+77.50 TO 2914+14.00						844	844				PEDESTRIAN CROSSING (LADDER CROSSWALK). SEE S+d PLAN A24F.			
PD-2	"CL-ALN1" 2921+31.25 TO 2922+85.68			26											
PD-2	"CL-ALN1" 2924+16.00 TO 2928+37.28	NB/SB		69											

844

### **ROADSIDE SIGN QUANTITIES**

974

57

							POST S	IZE AND	LENGTH	ROADSI	DE SIGN	(N) ROADSIDE SIGN	
SHEET NUMBER			SIGN SIGN MESSAGE DESIGNATION		SIGN PANEL SIZE (INCHES)	"C" Dim	4"X4"	4"X6"	6"X6"	ONE POST	TWO POST	MOUNTED ON TYPE III BARRICADE	REMARKS
		DI				LF	LF	LF	LF	EA	EA	EA	
DD 3	D2 7	NB	SG28(L+)(CA)	COASTAL ACCESS	48 × 48	7		1.4		1	1		
PD-2 PD-2	P2-3 P2-4	NB	SG28(L+)(CA)	COASTAL ACCESS  COASTAL ACCESS	48 × 48 48 × 48	7		14		1			
PD-2	P2-5	SB	SG28(R+)(CA)	COASTAL ACCESS	48 × 48	7		1.4		1			
PD-2	P2-6	SB	SG28(R+)(CA)	COASTAL ACCESS	48 × 48	7		14		1			
				111111111111111111111111111111111111111	1 12 17 19				TOTAL	4			

## PAVEMENT DELINEATION **QUANTITIES**

PDQ-1

BORDER LAST REVISED 7/2/2010

REVISED

FUNCTIONAL SUPERVISOR

STATE OF CALIFORNIA - DEPARTMENT OF TRANSPORTATION

DESIGN

USERNAME => s146363 DGN FILE => 0731350nc001.dgn

TOTAL

95

2,630

3,217

UNIT 1809

PROJECT NUMBER & PHASE

07150000900

Dist	COUNTY	ROUTE	POST MILES TOTAL PROJECT	SHEET No.	TOTAL SHEETS
07	LA,Ven	1	37.7/62.9 0.0/0.9	-	124
PLA  THE OR A  THE	ANS APPROV STATE OF CA GENTS SHALL	IVIL ENGINE  VAL DATE  LIFORNIA OR II  NOT BE RESPO  COMPLETENESS	EER DATE	THAN OL C71418 .12-31- CIVIL OF CALIFO	JM B 21 ×

RO.	$\Delta$ D $V$	NAY	QUA	ΔΝΤΙ	TFS

SHEET No.	LOCATION/STATION	IRECTION	REMOVE GUARDRAIL	MIDWEST GUARDRAIL SYSTEM (WOOD POST)	TREATED WOOD WASTE	ALTERNATIVE IN-LINE TERMINAL SYSTEM	TRANSITION RAILING (TYBE WB-31)	VEGETATION CONTROL MAT (RUBBER)	RUBBERIZED HOT MIX ASPHALT (GAP GRADED)	HOT MIX ASPHALT (TYPE A)	LEAN CONCRETE BASE	CLASS 3 AGGREGATE BASE	MINOR CONCRETE (CURB, SIDEWALK AND CURB RAMP)	ROADWAY EXCAVATION	TACK COAT	ALTERNATIVE CRASH CUSHION TL-2	CABLE RAILING	STRUCTURE BACKFILL
			LF	LF	LB	EΑ	EA	SQYD	TON	TON	CY	CY	CY	CY	TON	EΑ	LF	CY
L-1	"CL-ALN1" 2920+92.66 TO 2921+18.75	SB	112.5		1,425				1.1	2.5	1.9	5.6	3.4	27.0	0.01	1.0	26.0	320
L-1	"CL-ALN1" 2920+85.18 TO 2921+75.00	NB		15.6		1	1	58.0	13.2	29.8	22.8	51.0	12.4	102.1	0.14			150
L-1	"CL-ALN1" 2922+83.00 TO 2923+37.00	NB	118.8		1,500				8.7	19.5	14.9	35.8	8.0	71.6	0.09			
L-1	"CL-ALN1" 2922+53.44 TO 2924+39.98	SB							13.9	31.2	23.9	51.9	4.8	126.4	0.13			
	SHEET TO	DTAL	231.3	15.6	2,925	1	1	58.0	36.9	83.0	63.5	144.3	28.6	327.1	0.37	1.0	26.0	470

### ADA CURB RAMP QUANTITIES

LAYOUT SHEET NO.	CONSTRUCTION DETAIL SHEET NO.	CURB RAMP No.	MINOR CONCRETE (CURB, SIDEWALK AND CURB RAMP)	DETECTABLE WARNING SURFACE	PRE/POST CONSTRUCTION SURVEYS
		ว	CY	SQFT	<b>EA</b>
L-1	C-1	1	7.0	58.0 40.0	1
L-1	C-2	2&3	8.6	40.0	2
	T(	DTAL	15.6	98.0	3

## SUMMARY OF QUANTITES

Q-1

STATE OF CALIFORNIA - DEPARTMENT OF TRANSPORTATION

Gt Caltans

DESIGN BORDER LAST REVISED 7/2/2010

USERNAME => s146363 DGN FILE => 0731350pa001.dgn

UNIT 1809

PROJECT NUMBER & PHASE

07150000900

REVISED BY

FUNCTIONAL SUPERVISOR

FOR ACCURATE RIGHT OF WAY DATA, CONTACT RIGHT OF WAY ENGINEERING AT THE DISTRICT OFFICE.

REVISED

STATE OF CALIFORNIA - DEPARTMENT OF TRANSPORTATION

## TEMPORARY WATER POLLUTION CONTROL QUANTITIES

ROUTE	SHEET NO.	LOCATION /STATION	DIRECTION	TEMPORARY COVER	TEMPORARY SILT FENCE	TEMPORARY DRAINAGE INLET PROTECTION	TEMPORARY CONSTRUCTION ENTRANCE
				SQYD	LF	EΑ	EΑ
	L-1	"LA1" 2920+00 TO 2924+00	NB	500	480	1	1
1	L-1	"LA1" 2919+00 TO 2925+00	SB	600	570	1	
	L-1	"CORCNYN" 8+40	WB			1	
SHEE	T TOTAL			1,100	1,050	3	1

<b>ENVIRONM</b>	ENTALLY	SENSITIVE
AREA	(ESA) QU	JANTITY

LOG	CATION	np HIGH-VISIBILITY ICE
SHEET No.	ROUTE	Temp FENCE
1,10.		LF
L-1	LA/VEN 1	661
	TOTAL	661

Dist COUNTY ROUTE POST MILES SHEET TOTAL No. SHEETS

07 LA, Ven 1 37.7/62.9 - 124

MATHAN OUM

PLANS APPROVAL DATE

THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET.

SUMMARY OF QUANTITIES

Q-2

BORDER LAST REVISED 7/2/2010 USERNAME => s146363
DGN FILE => 0731350pa002.dgn
RELATIVE BORDER SCALE 0 1 2 3
DGN FILE => 0731350pa002.dgn
UNIT 1809
PROJECT NUMBER & PHASE 07150000901



REVISED BY

SENIOR LANDSCAPE ARCHITECT

- DEPARTMENT OF TRANSPORTATION
LANDSCAPE ARCHITECTURE

STATE OF CALIFORNIA

EROSION CONTROL

### **EROSION CONTROL**

SEQUENCE	ITEM	MATERIAL	MATERIAL TYPE	APPLICATION RATE
STEP 1	HYDROSEED	SEED	MIX	15 LB/ACRE
SIEP I	HIDROSEED	FIBER	WOOD	500 LB/ACRE
STEP 2	HYDROMULCH	FIBER	WOOD	1,500 LB/ACRE
SIEF Z	HIDROWOLCH	TACKIFIER	GUAR	125 LB/ACRE

#### SEED MIX

BOTANICAL NAME (COMMON NAME)	PERCENT GERMINATION (MINIMUM)	POUNDS PURE LIVE SEED PER ACRE (SLOPE MEASUREMENT)
BROMUS CARINATUS (CALIFORNIA BROME)	80	8.0
ERIOGONUM FASCICULATUM (CALIFORNIA BUCKWHEAT)	5	2.0
ERIOPHYLLUM CONFERTIFLORUM (GOLDEN YARROW)	30	1.0
FESTUCA MICROSTACHYS (SMALL FESCUE)	70	4.0

#### PLANT LEGEND

					HOLE SIZE				픙	APPLICATION RATES							MINIMUM PLANTING DISTANCE FROM:							
PLANT GROUP (SIZE)		SYMBOL	BOTANICAL NAME	COMMON NAME	AETER	TH	SIN TYPE	NT AREA UP F/H/M	WOOD MULC	L ENDMENT	FATE	REL FERT:	OW- EASE ILIZER	ORG. FERTI	ANIC LIZER	KET TILIZER	CENTER CING			CE		ED CH	HT.	REMARKS
					DIAN	DEP	BAS	PLANT GROUP	BASIN	SOIL	IRON SULF	PLT	PLT ESTB	PLT	PLT ESTB	PAC	ON SPA	ETW	БР	P P E N	WAL	PAVI DIT(	EAR DIT(	
					INCH	INCH		SQFT	CY	CY	oz	oz	oz	oz	oz	EA	f†	f†	f†	f†	f†	f†	f†	
В	1	$(\circ)$	<u>Pla</u> tanus <u>Rac</u> emos	CALIFORNIA SYCAMORE	2×		I		0.03	0.04	4					3	1	_	15	15	15	15	15	TREE
(No. 5)	2	+	<u>SAL</u> IX <u>LAS</u> PIOLEPIS	ARROYO WILLOW	2×		I		0.03	0.04	4					3	1	_	10	8	8	8	10	SHRUB

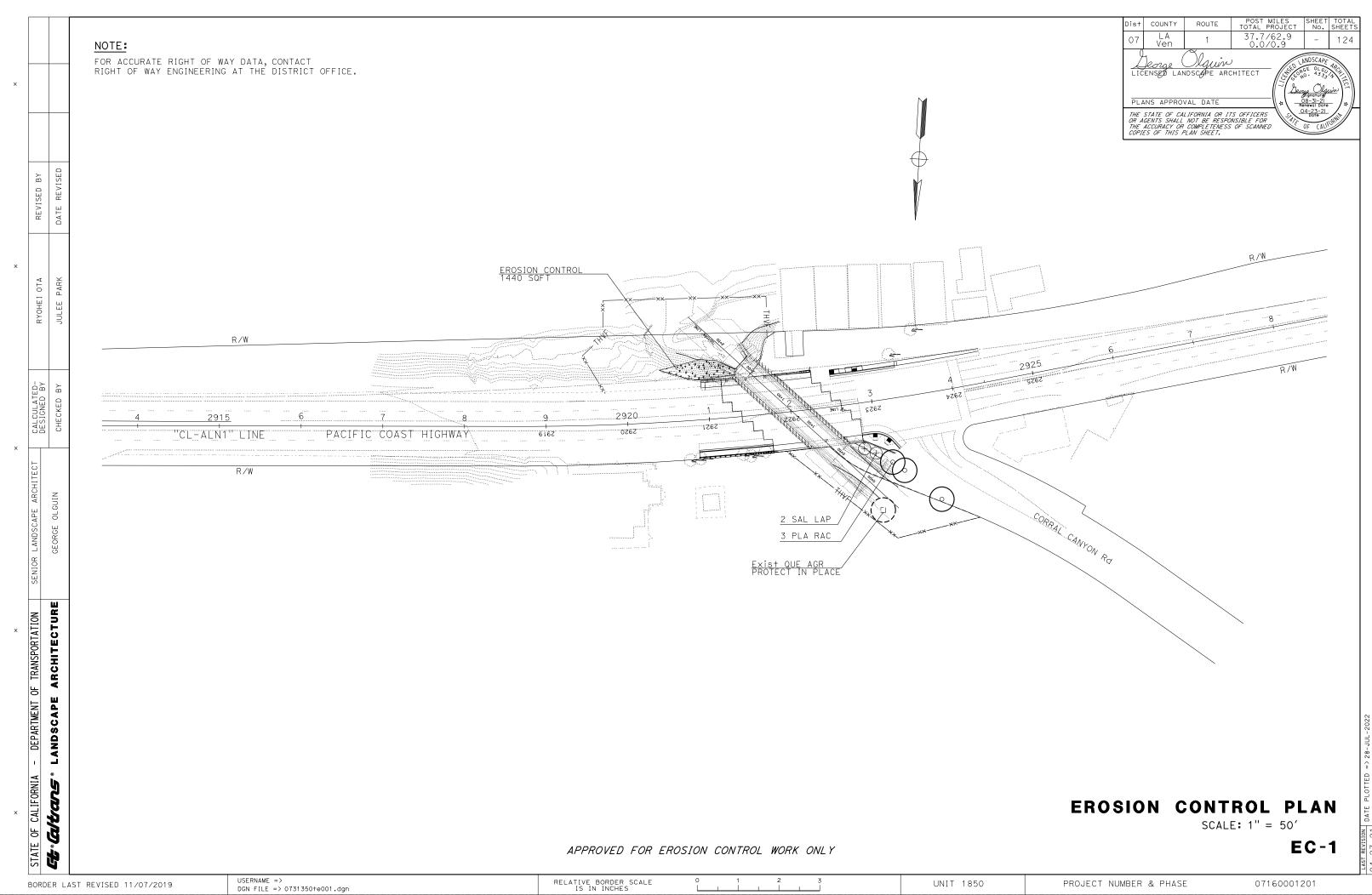
**EROSION CONTROL LEGEND** 

ECL-1

BORDER LAST REVISED 11/07/2019

USERNAME => DGN FILE => 0731350+c001.dgn

RELATIVE BORDER SCALE 0 1 2 3 UNIT 1850 PROJECT NUMBER & PHASE 07160001201



DGN FILE => 0731350+e001.dgn

Dist	COUNTY	ROUTE	POST MI TOTAL PR		SHEET No.	TOTAL SHEETS
07	LA Ven	1	37.7/6 0.0/0		-	124
	Jeoge (ENSED LAN	Slgwin Idscape arc		Dem	ge Olge Fignature 18-31-21 Inewal Date	uin ECI
OR A THE	GENTS SHALL	LIFORNIA OR IT NOT BE RESPO COMPLETENESS PLAN SHEET.	NSIBLE FOR		0F (ALL	GENT?

#### **EROSION CONTROL QUANTITIES**

			СН	MATE	EM RIALS N)	IT MATEF ()	
EC SHEET	DESCRIPTION	HYDROSEEC	HYDROMULO	SEED	FIBER	FIBER	TACKIFIER
		SQFT	SQFT	LB	LB	LB	LB
1	EROSION CONTROL	1440	1440	0.5	17	50	4
	TOTAL	92,550	92,550	32	1064	3188	268

(N) - NOT A SEPARATE BID ITEM

### PLANT QUANTITIES

		PLANT GROUP		PLA	NT B	ASIN
BOTANICAL NAME	COMMON NAME	В	PACKET FERTILIZER	WOOD MULCH	SOIL AMENDMENT	IRON SULFATE
		EA	EΑ	CY	CY	LB
PLATANUS RACEMOS	CALIFORNIA SYCAMORE	2	6	0.06	0.08	0.5
SALIX LASPIOLEPIS	ARROYO WILLOW	2	6	0.06	0.08	0.5
	SUBTOTAL	4	12	0.12	0.16	1
	TOTAL	4	4	0.12	0.16	1

**EROSION CONTROL QUANTITIES** 

ECQ-1

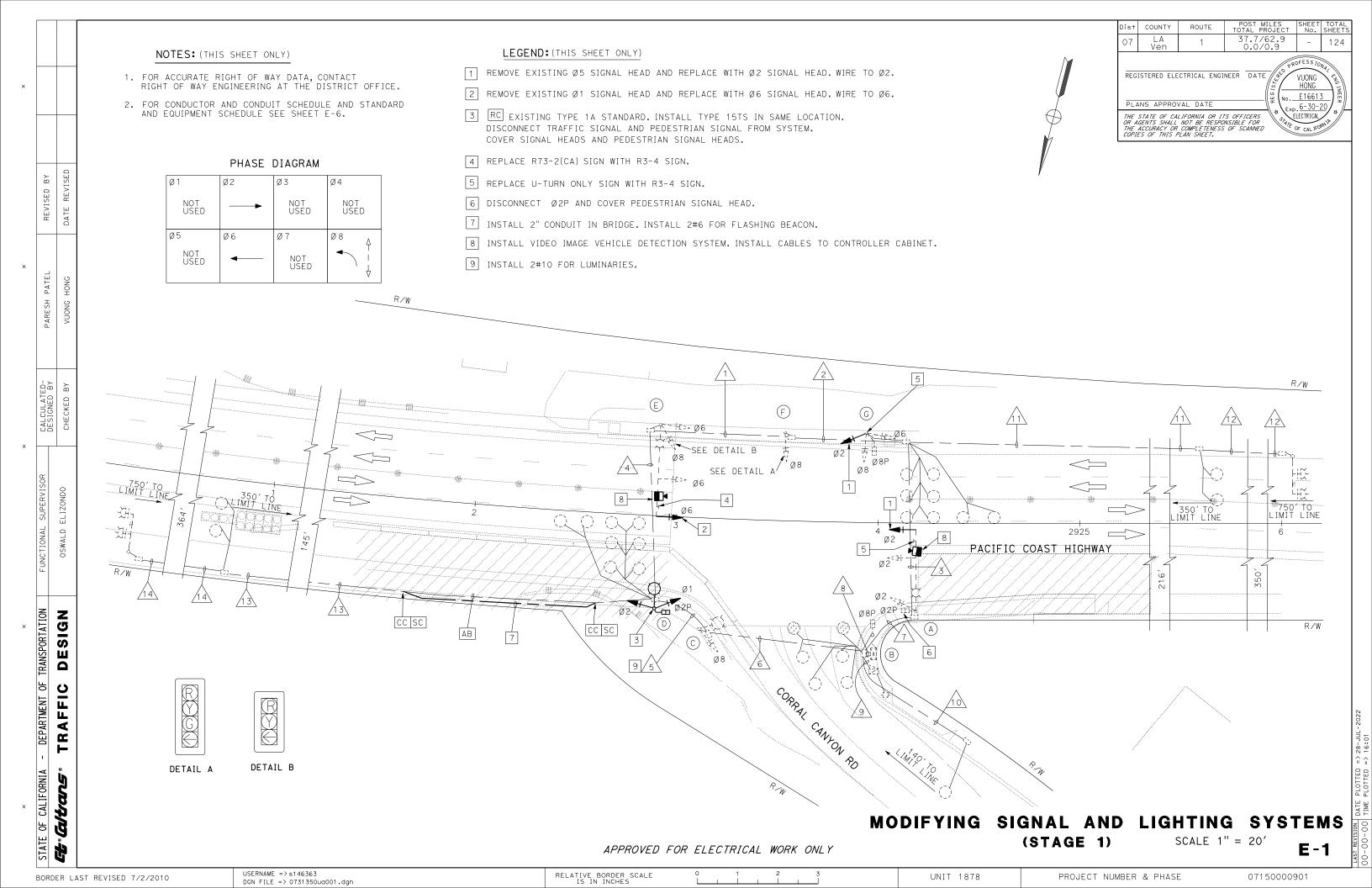
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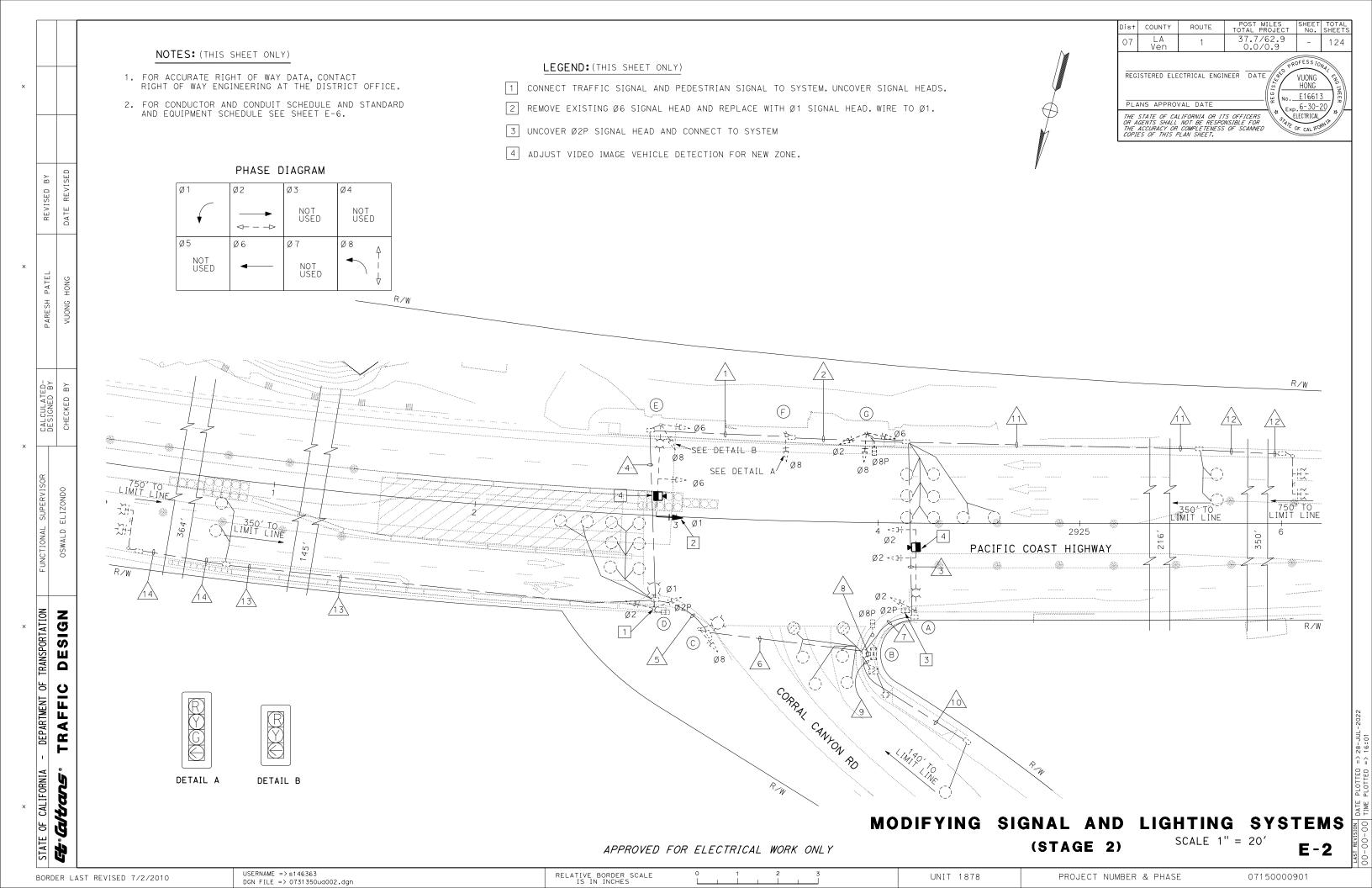
UNIT 1850

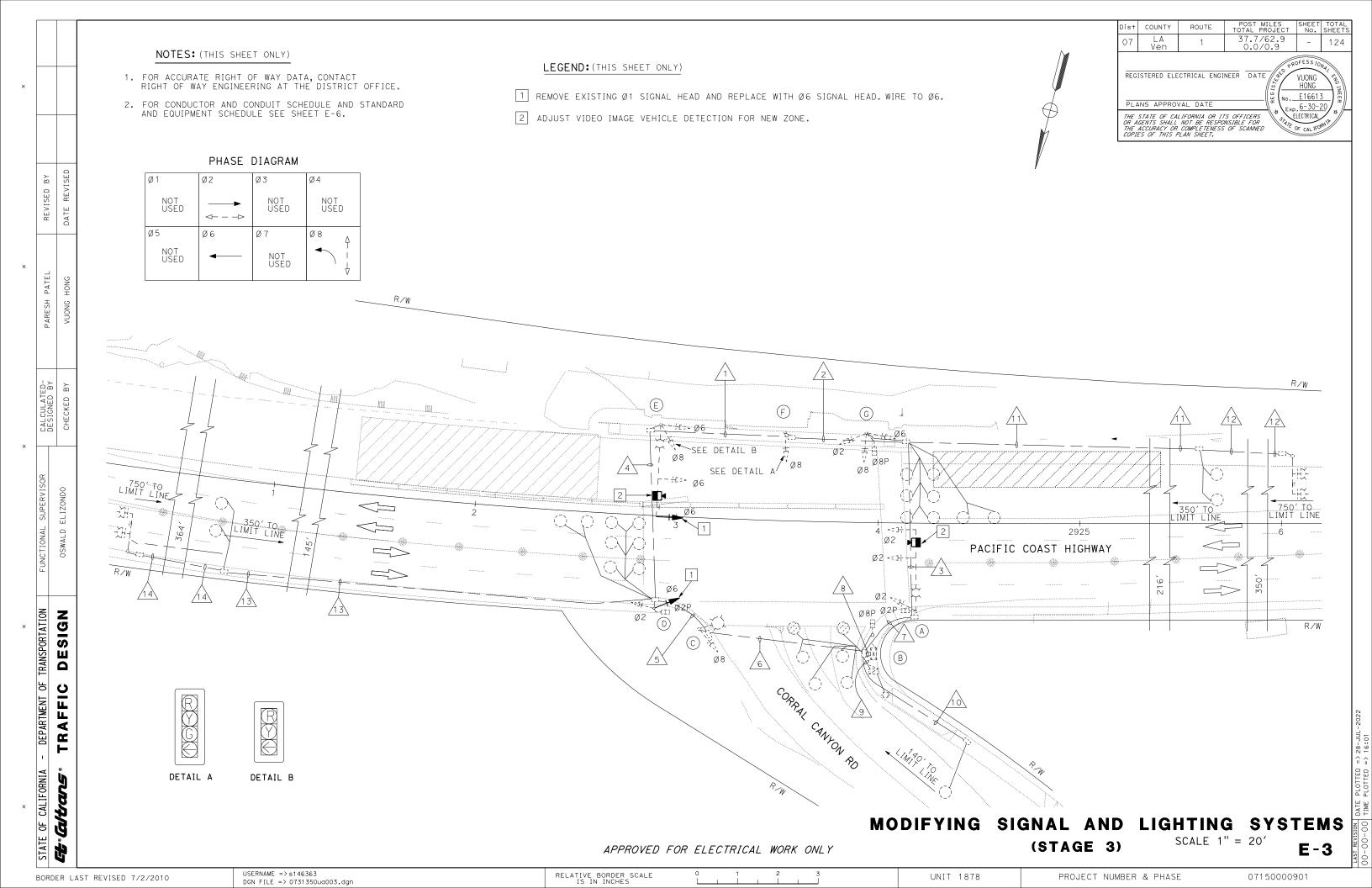
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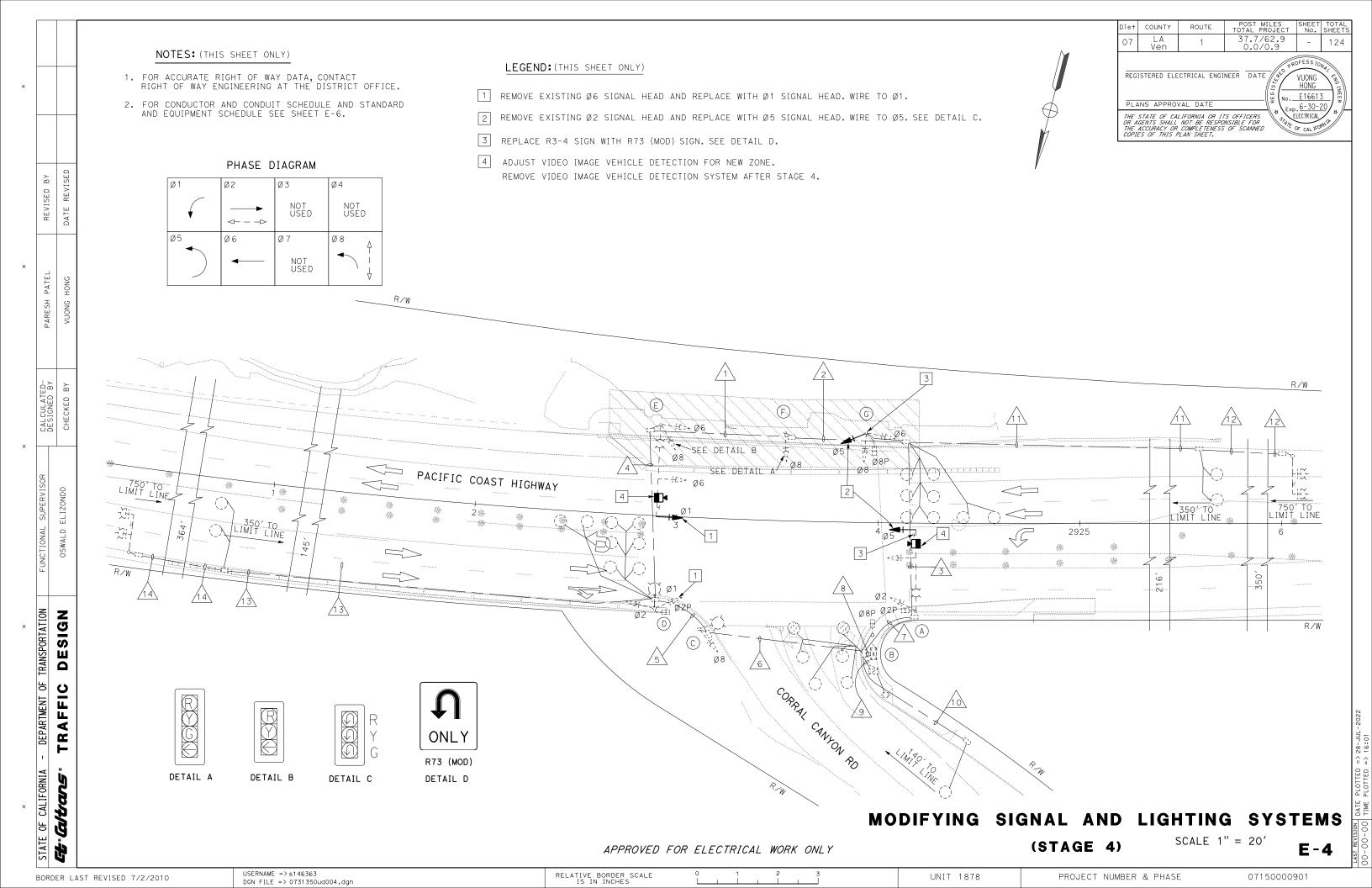
BORDER LAST REVISED 11/07/2019

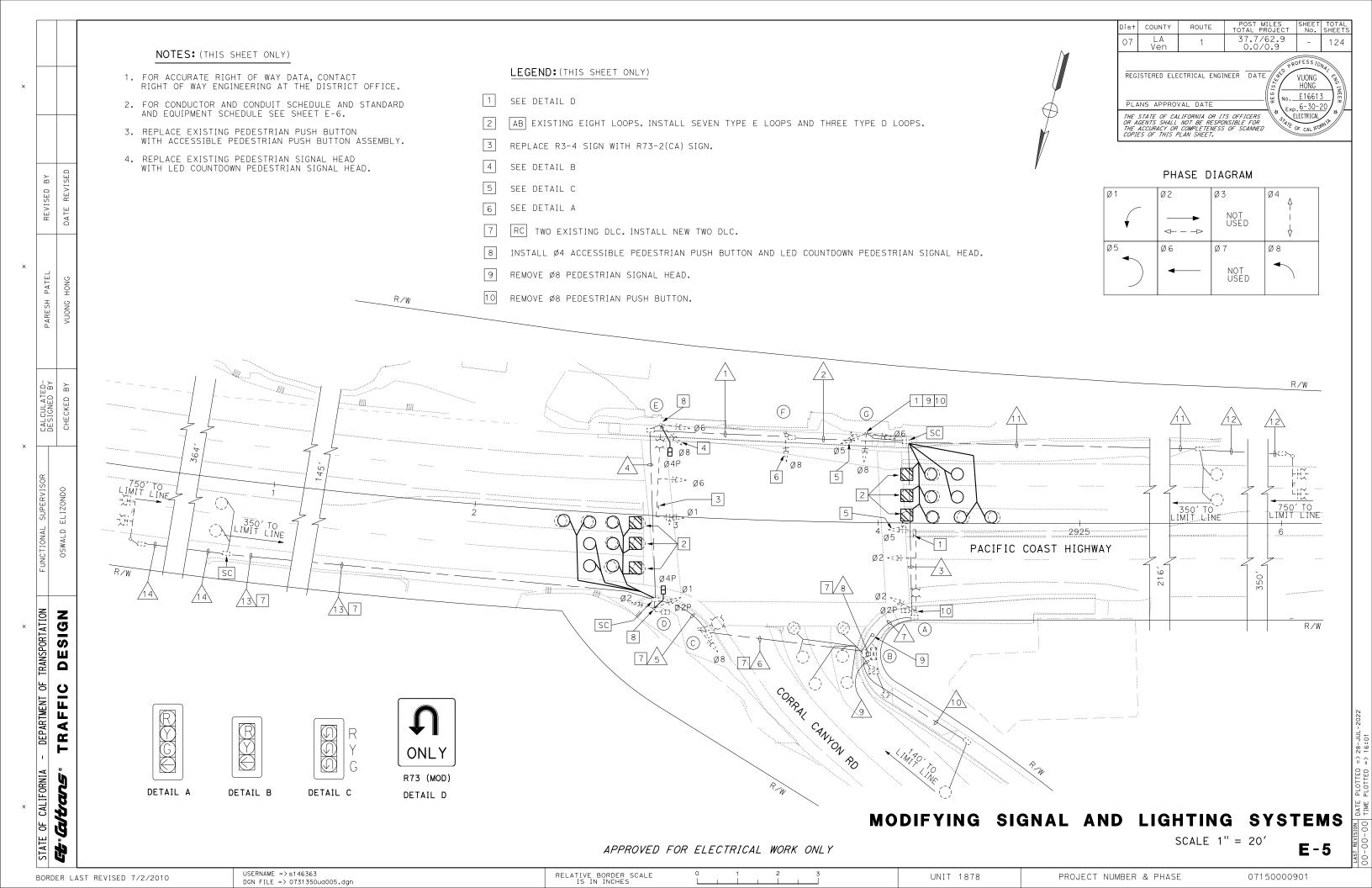
PROJECT NUMBER & PHASE











REGISTERED ELECTRICAL ENGINEER DATE PLANS APPROVAL DATE THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET. STANDARD AND EQUIPMENT SCHEDULE STANDARD Veh Sig Mtg Ped SIGNAL APS REFLECTIVE SNS LED LMA HEIGHT MAST ARM POLE M†g Ø ARROW LUMINAIRE SMA Ø (A) 29A-5-100 2 SP-1-T ROADWAY 2 40′ 15′ 35′ SV-1-T CORRAL CANYON Rd 2'MAS 15TS 12′ 30′ 2 ROADWAY 1 SP-2-T 15TS 12′ 30′ SV-2-T 15′ 35′ SV-2-T SP-1-T CORRAL CANYON Rd 2′MAS 4 ROADWAY 2 29A-5-100

#### CONDUCTOR AND CONDUIT SCHEDULE

TV-1-T

TV-3-T

15′

7′

1 – A

						RU	JN NUM	BER							
CONDUCTOR		NUMBER OF CONDUCTORS													
DESI	GNATION	1	2	3	4	5	6	7	8	9	10	11	12	13	14
28 CSC (C1)	SIGNAL		1	1		1	1	1	1						
28 CSC (C2)	SIGNAL	1	1	1			1	1	1						
#10 AWG	LUMINARIES	2	2	2		2	2	2							
#6 AWG	SERVICE									2					
#6 AWG	FLASHING BEACON			2		2	2	2	4			2	2	2	2
	EMERGENCY VEHICLE PREEMPTION	1	1					1	1						
	Ø1			1				1	1						
	Ø2					4	4		4					2	
DLC	Ø5					1	1		1						
	Ø6			4				4	4			2			
	Ø8								4		1				
CONDUIT	SIZE	2"	3"	3"	3''	2"	3"	3"	2-3"	2"	2"	2''	11/2"	2''	11/2"

MODIFYING SIGNAL AND LIGHTING SYSTEMS

NO SCALE

E-6

APPROVED FOR ELECTRICAL WORK ONLY

USERNAME => s146363 BORDER LAST REVISED 7/2/2010 DGN FILE => 0731350ua006.dgn

RELATIVE BORDER SCALE IS IN INCHES

UNIT 1878

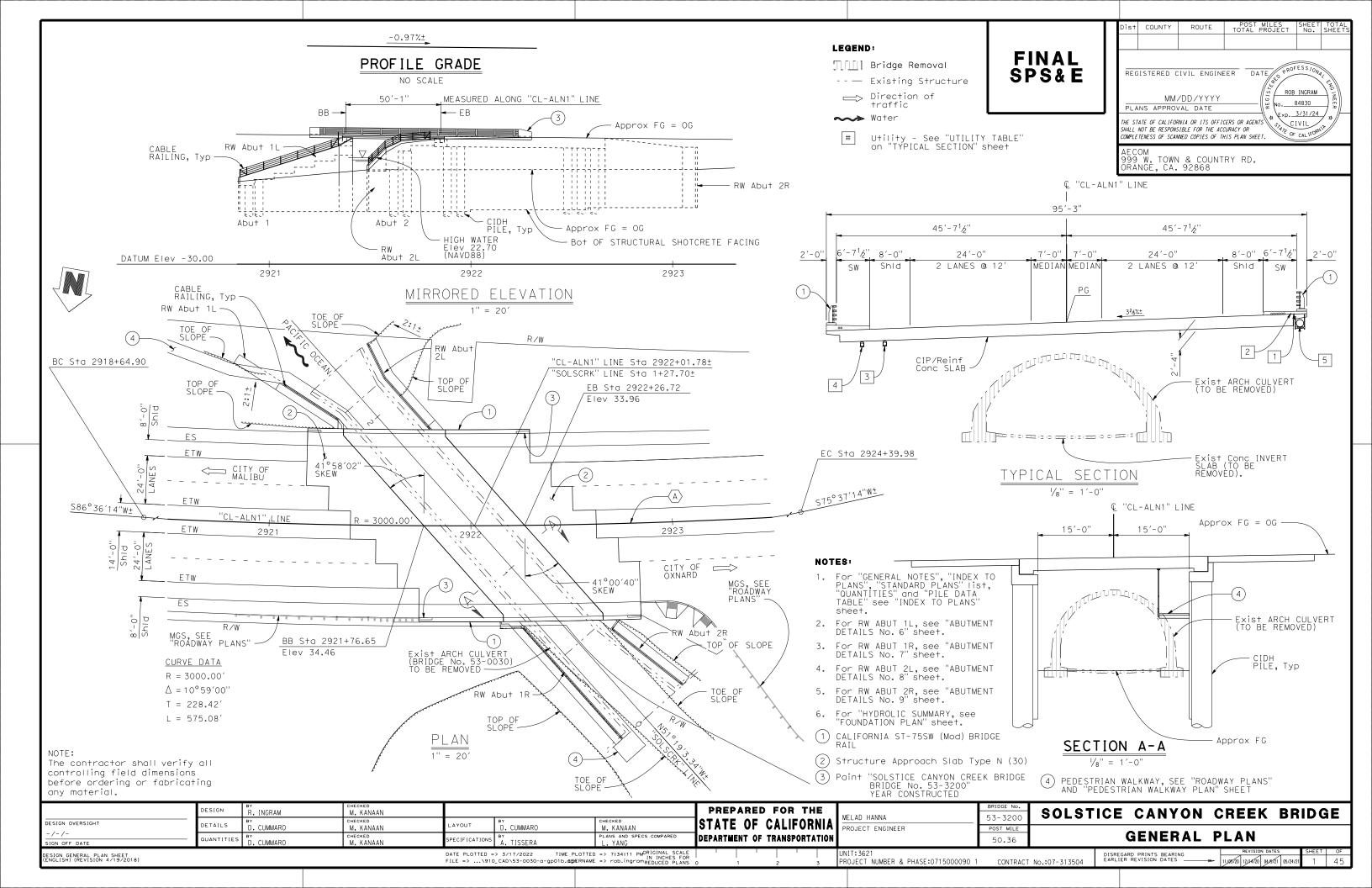
PROJECT NUMBER & PHASE

07150000901

37.7/62.9 0.0/0.9

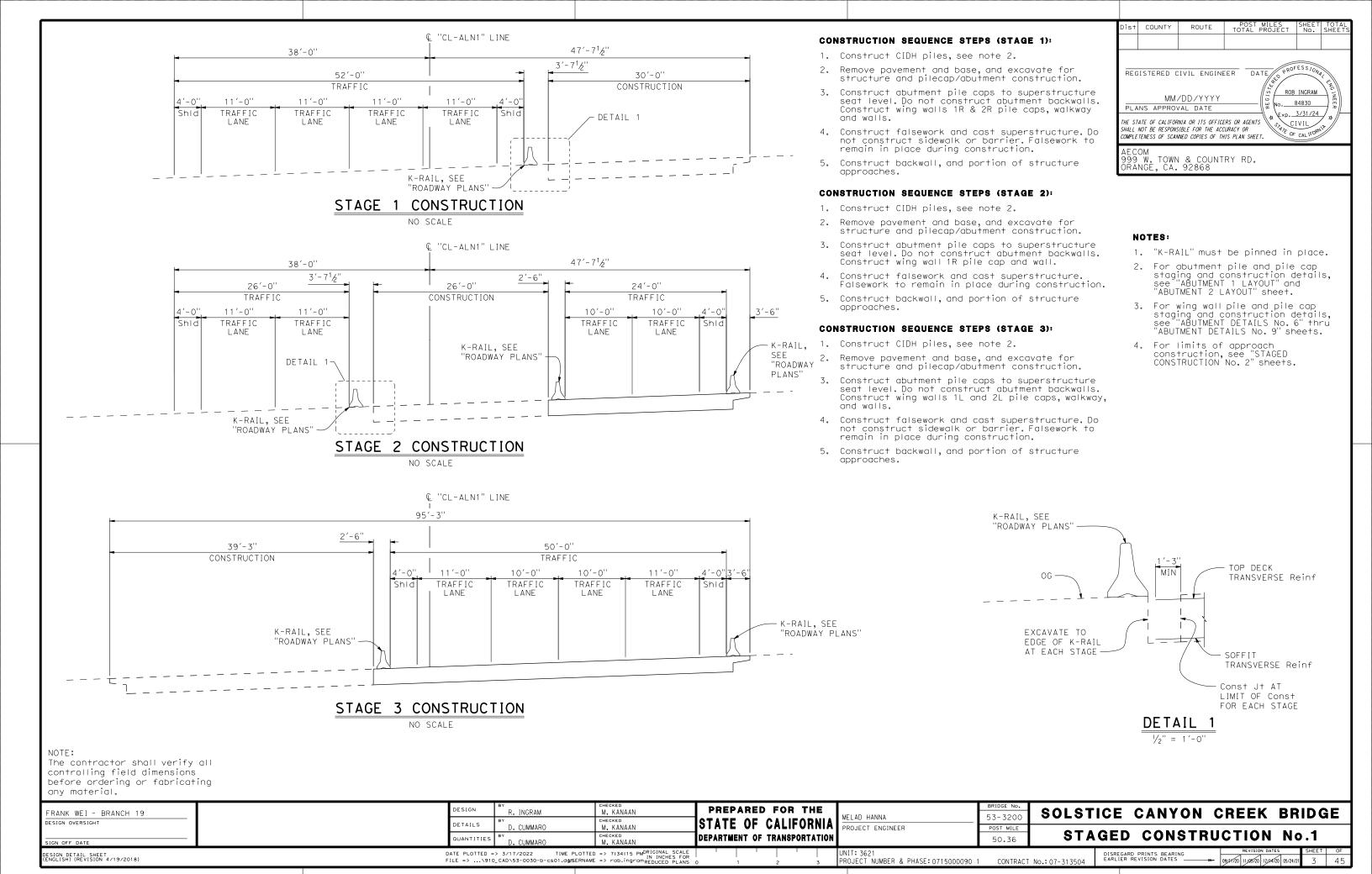
124

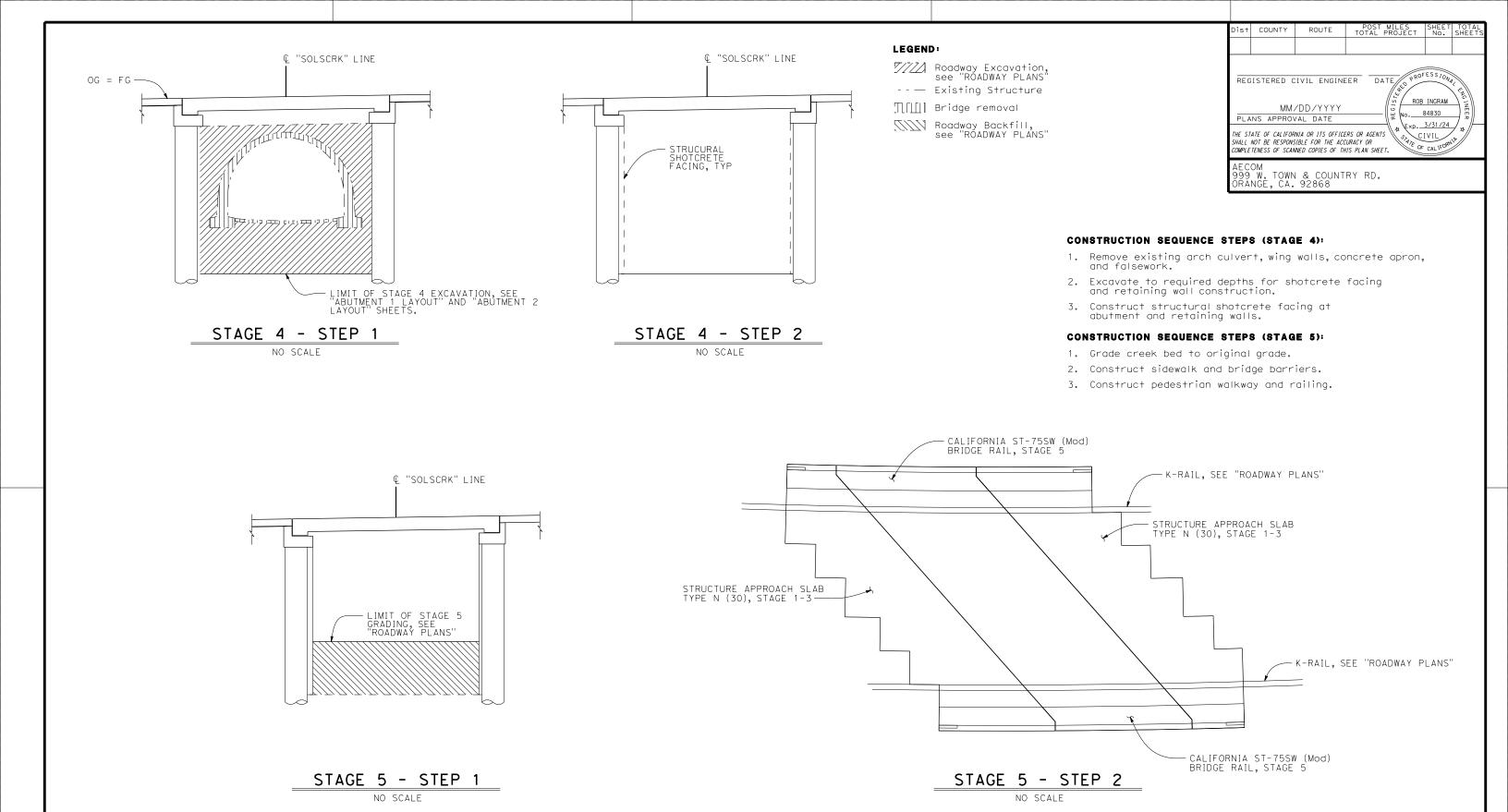
VUONG HONG No. <u>E16613</u> EXP. 6-30-20 ELECTRICAL OF CAL IFORM



#### INDEX TO PLANS GENERAL NOTES SHEET No. TITLE LOAD AND RESISTANCE FACTOR DESIGN REGISTERED CIVIL ENGINEER DATE GENERAL PLAN INDEX TO PLANS STAGED CONSTUCTION No.1 STAGED CONSTUCTION No.2 EXCAVATION LIMITS No.1 EXCAVATION LIMITS No.2 BACKFILL LIMITS BRIDGE REMOVAL No.1 BRIDGE REMOVAL No.2 DECK COUNTOURS FOUNDATION PLAN ABUTMENT 1 LAYOUT ABUTMENT 2 LAYOUT DESIGN: AASHTO LRFD Bridge Design Specifications, 8th edition with ROB INGRAM MM/DD/YYYY California Amendments, preface dated April 2019. 84830 PLANS APPROVAL DATE Caltrans Seismic Design Criteria (SDC), Version 2.0 Dated April 2019. xn. 3/31/24 SEISMIC DESIGN: THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS CIVIL SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR OF CALLFOR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET. LIVE LOAD: HL93 and permit design load. LIVE LOAD SURCHARGE: Varied surcharge on level ground surface. 999 W. TOWN & COUNTRY RD. DRANGE, CA. 92868 2 LAYOUT DETAILS NO. DETAILS NO. DETAILS NO. DETAILS NO. DEAD LOAD: Includes 35 psf for future wearing surface. ABUTMENT ABUTMENT SEISMIC LOAD: Shear wave velocity, Vs30 = 970.1 ft/sec for the top 100 ft of soil ABUTMENT ABUTMENT ABUTMENT PILE DATA TABLE Moment Magnitude: 6.64 Peak Rock Acceleration = 0.58g ABUTMENT ABUTMENT DETAILS No. 5 DETAILS No. 6 Nominal Resistance (kips) Specified Tip Fault Rupture; Vertical = 12.6 in, Horizontal = 4.6 in Design Tip ABUTMENT DETAILS NO. 6 ABUTMENT DETAILS NO. 7 ABUTMENT DETAILS NO. 8 ABUTMENT DETAILS NO. 9 TYPICAL SECTION SLAB REINFORCEMENT DETAILS PEDESTRIAN WALKWAY PLAN PEDESTRIAN WALKWAY LAYOUT NO. PEDESTRIAN WALKWAY LAYOUT NO. Location Pile Type Elevation Elevation $Ø = 29^{\circ}$ SOIL: (f+) Compression $\gamma = 114 \text{ pcf}$ REINFORCED (a) - 8.048" CIDH 160 0 -31.0 Abut 1 CONCRETE: fy = 60 ksi(c) -31.0f'c = 4.0 ksi, except as shown in "CONCRETE STRENGTH AND TYPE (a) -8.0LIMITS" diagram Abut 2 48" CIDH 160 0 -31.0 LAYOUT NO. 3 LAYOUT NO. 4 LAYOUT NO. 5 DETAILS NO. (c) -31.0STRIAN WALKWAY STRIAN WALKWAY = 8.0 (a) -8.0 (c) -31.0 TRIAN WALKWAY TRIAN WALKWAY RW 1L SHOTCRETE: fy = 60 ksi48" CIDH 160 0 -31.0 f'c = 4.0 ksiPEDESTRIAN WALKWAY DETAILS NO. 2 PEDESTRIAN WALKWAY DETAILS NO. 3 PEDESTRIAN WALKWAY DETAILS NO. 4 PEDESTRIAN WALKWAY DETAILS NO. 5 PEDESTRIAN WALKWAY DETAILS NO. 6 STRUCTURE APPROACH DRAINAGE DETAILS UTILITY PIPE SUPPORT DETAIL CALIFORNIA ST-75SW BRIDGE RAIL DETAILS NO. 1 CALIFORNIA ST-75SW BRIDGE RAIL DETAILS NO. 2 CALIFORNIA ST-75SW BRIDGE RAIL DETAILS NO. 3 CALIFORNIA ST-75SW BRIDGE RAIL DETAILS NO. 3 CALIFORNIA ST-75SW BRIDGE RAIL DETAILS NO. 4 JOINT ARMOR FOR PEDESTRIAN WALKWAYS LOG OF TEST BORINGS 1 OF 2 LOG OF TEST BORINGS 2 OF 2 n = 8.018" CIDH N/A N/A -31.0 N/A SECANT 5% DAMPING (b) 2.0 (a) - 8.0RW 1R 48" CIDH 160 0 -31.0 (c) - 31.0(a) - 8.0RW 2L 48" CIDH 160 0 -31.0 (c)' - 31.0(a) - 8.0RW 2R 48" CIDH 160 0 -31.0 (c)' - 31.0(a) -8.0 (c) -31.0 Ped 24" CIDH 160 -31.0 WALKWAY STANDARD PLANS DATED 2018 0.5 NOTES: 1) Design tip elevations are controlled by the following demands: (a) Compression, (b) Tension, (c) Lateral Load A3A A3B A3C ABBREVIATIONS ABBREVIATIONS (SHEET (SHEET (SHEET SHEET 2 OF 3) LEGEND - LINES AND SYMBOLS (SH LEGEND - LINES AND SYMBOLS (SHE LEGEND - LINES AND SYMBOLS (SHE LEGEND - SOIL (SHEET 1 OF 2) LEGEND - SOIL (SHEET 2 OF 2) BRIDGE DETAILS BRIDGE DETAILS 3RIDGE DETAILS OUNT OF 2) The specified tip elevations shall not be raised above the 0.0 design tip elevations. 0.0 1.0 2.0 3.0 Period (seconds) LEGEND: Structural Concrete, Bridge LEGEND - SOIL (SHEET 1 OF 2) LEGEND - SOIL (SHEET 2 OF 2) BRIDGE DETAILS BRIDGE DETAILS BRIDGE DETAILS BRIDGE DETAILS BRIDGE DETAILS BRIDGE DETAILS JOINT SEALS (MAXIMUM MOVEMENT RATING = 2") UTILITY OPENING BOX GIRDER STRUCTURE APPROACH - TYPE N (30) STRUCTURE APPROACH - SLAB DETAILS STRUCTURE APPROACH - DRAINAGE DETAILS CABLE RAILING MODIFIED ARS CURVE Structural Concrete, Bridge (Polymer Fiber) (f'c = 5.0 ksi) B0-3 B0-5 B0-13 Structural Concrete, Approach Slab (f'c = 3.6 ksi) B6-21 RSP B7-10 Structural Shotcrete B9-1 B9-5 Cast-In-Drilled-Hole Piling RSP B11-47 B14-3 ABLE RAILING COMMUNICATION AND SPRINKLER CONTROL CONDUITS (CONDUITS LESS THAN 4") STRUCTURE BACKFILL (BRIDGE) 48" CAST-IN-DRILLED-HOLE CONCRETE PILING 5,742 LF STRUCTURAL CONCRETE, BRIDGE 252 CY STRUCTURAL CONCRETE, BRIDGE (POLYMER FIBER) 483 CY STRUCTURAL CONCRETE, APPROACH SLAB (TYPE N) 293 CY STANDARD PLAN DRILL AND BOND DOWEL 3,604 LF SHEET NO. JOINT SEAL (MR 1/2") 254 LF BAR REINFORCING STEEL (BRIDGE) 733,609 LB STRUCTURAL SHOTCRETE 519 CY -DETAIL NO. BRIDGE REMOVAL LUMP SUM GEOCOMPOSITE DRAIN 1,037 SQFT MISCELLANEOUS METAL (BRIDGE) 176 LB CABLE RAILING 254 LF CALIFORNIA ST-75SW BRIDGE RAIL (MODIFIED) 220 LF 4" CONDUIT (BRIDGE) The contractor shall verify all CONCRETE STRENGTH AND TYPE LIMITS controlling field dimensions before ordering or fabricating NO SCALE RETAINING WALL any material. M. KANAAN PREPARED FOR THE SOLSTICE CANYON CREEK BRIDGE DESIGN R INGRAM FRANK WEI - BRANCH 19 53-3200 DESIGN OVERSIGH STATE OF CALIFORNIA DETAILS D. CUMMARO M. KANAAN PROJECT ENGINEER POST MILE INDEX TO PLANS DEPARTMENT OF TRANSPORTATION QUANTITIES 50.36 GN OFF DATE D CLIMMARC Μ. ΚΔΝΔΔΝ DATE PLOTTED => 3/17/2022 TIME PLOTTED => 7:34:12 PMORIGINAL SCALE FILE => ...\910\_CAD\53-0030-a-itp.dgrUSERNAME => rob.ingrampEDUCED PLANS ESIGN DETAIL SHEET ENGLISH) (REVISION 4/19/2018) UNIT: 3621 PROJECT NUMBER & PHASE: 0715000090 1 CONTRACT No.: 07-313504

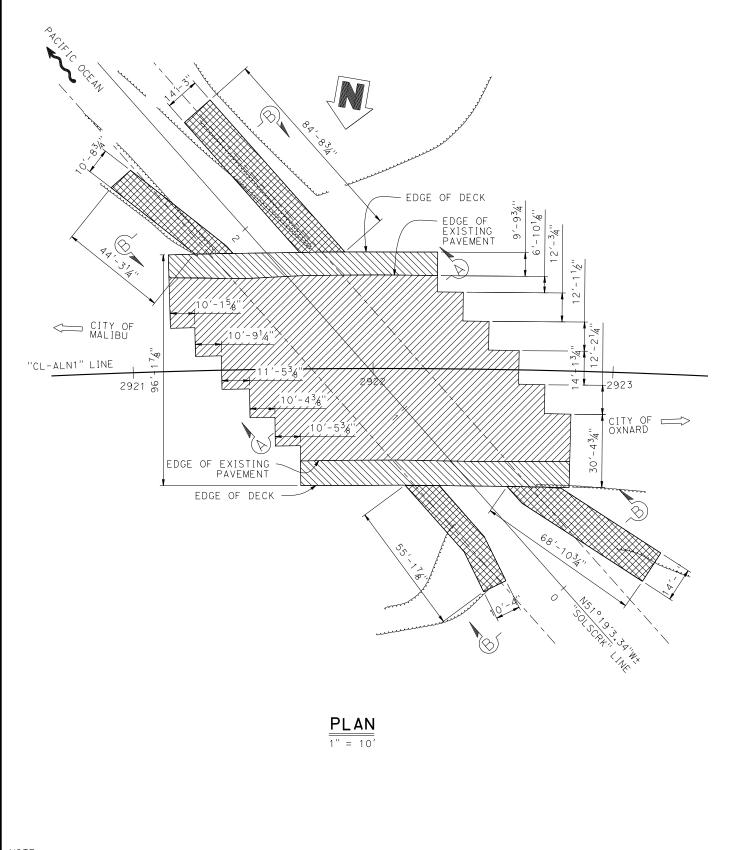
POST MILES TOTAL PROJECT





The contractor shall verify all controlling field dimensions before ordering or fabricating any material.

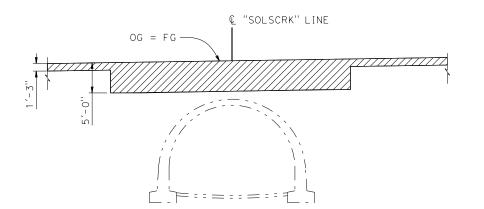
FRANK WEI - BRANCH 19		DESIGN	R. INGRAM BY	CHECKED M. KANAAN CHECKED		RED FOR TO		MELAD HANNA	BRIDGE No. 53-3200	SOLST	CE CANYON	CREEK	BRII	DGE
SIGN OFF DATE		QUANTITIES	D. CUMMARO  BY  D. CUMMARO			OF TRANSPOR			POST MILE 50.36	STAG	ED CONSTRU	CTION	No.	2
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#### LEGEND:

Roadway Excavation (TPH), see "ROADWAY PLANS"

Roadway Excavation (Type Z-2 - ADL and TPH), see "ROADWAY PLANS"



<u>SECTION</u> A-A

Dis+	COUNTY	ROUTE	POST MILES TOTAL PROJECT	SHEET No.	TOTAL SHEETS

REGISTERED CIVIL ENGINEER

MM/DD/YYYY

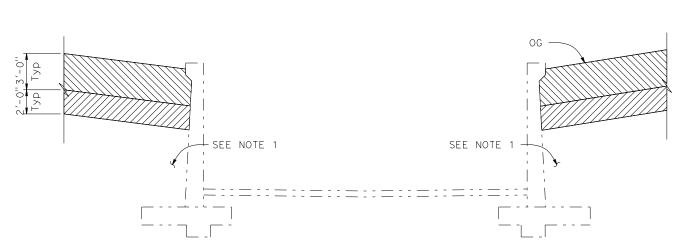
PLANS APPROVAL DATE

THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS
SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR
COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET.

AECOM 999 W. TOWN & COUNTRY RD. ORANGE, CA. 92868

#### NOTES:

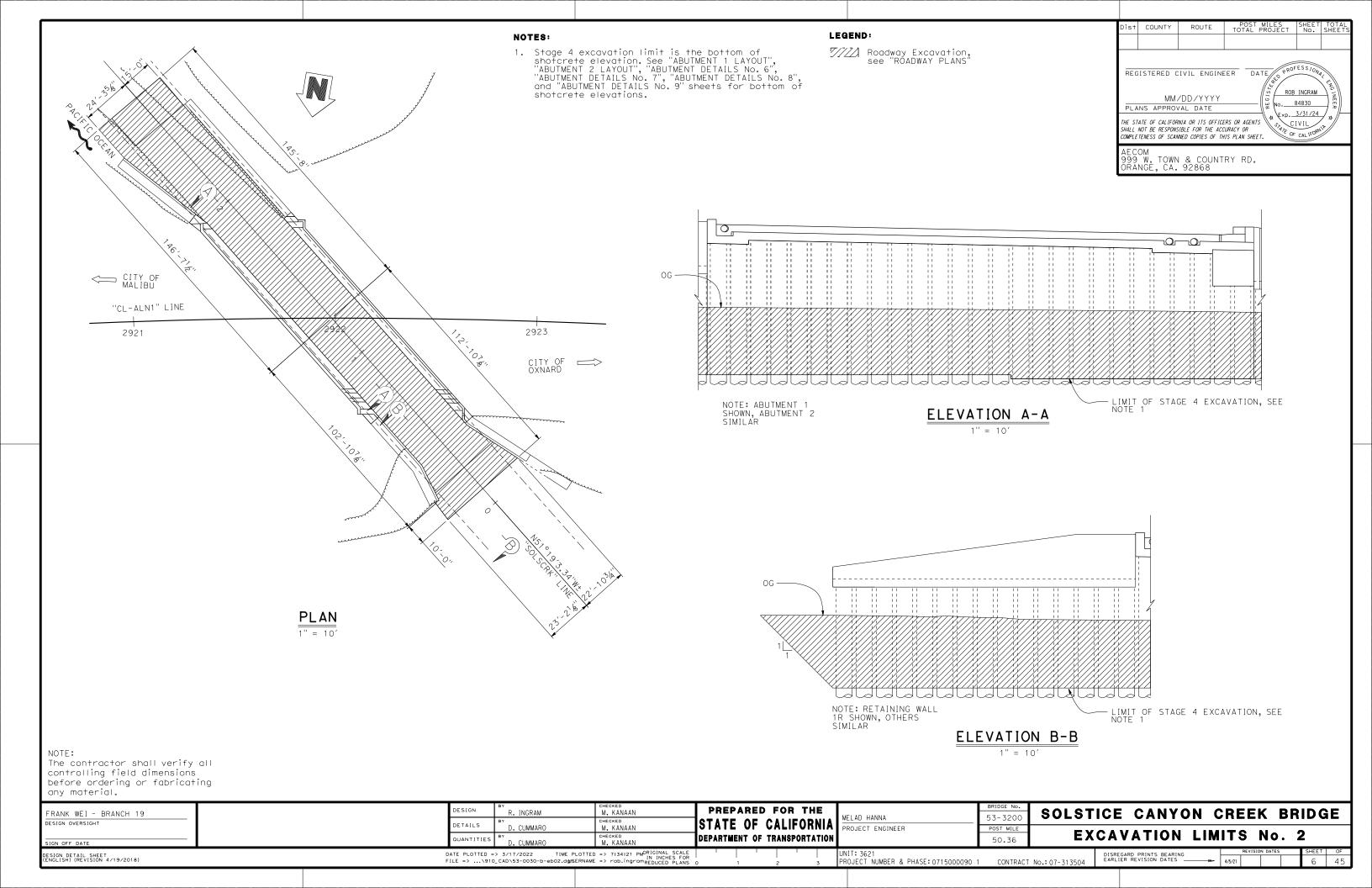
 Remaining excavation for Existing Wing Wall removal to be classified under "ROADWAY EXCAVATION", see "EXCAVATION LIMITS No.2" sheet.

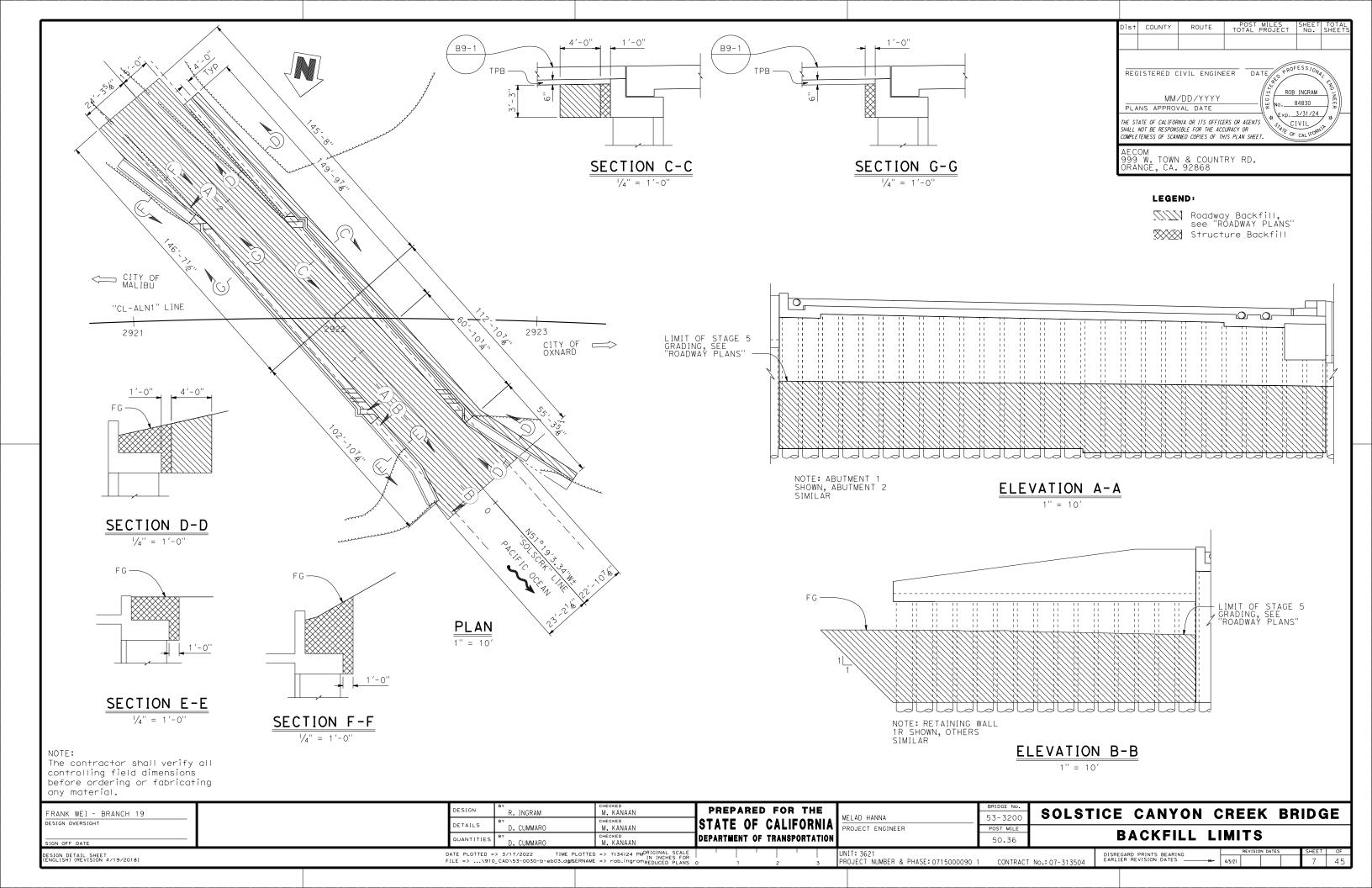


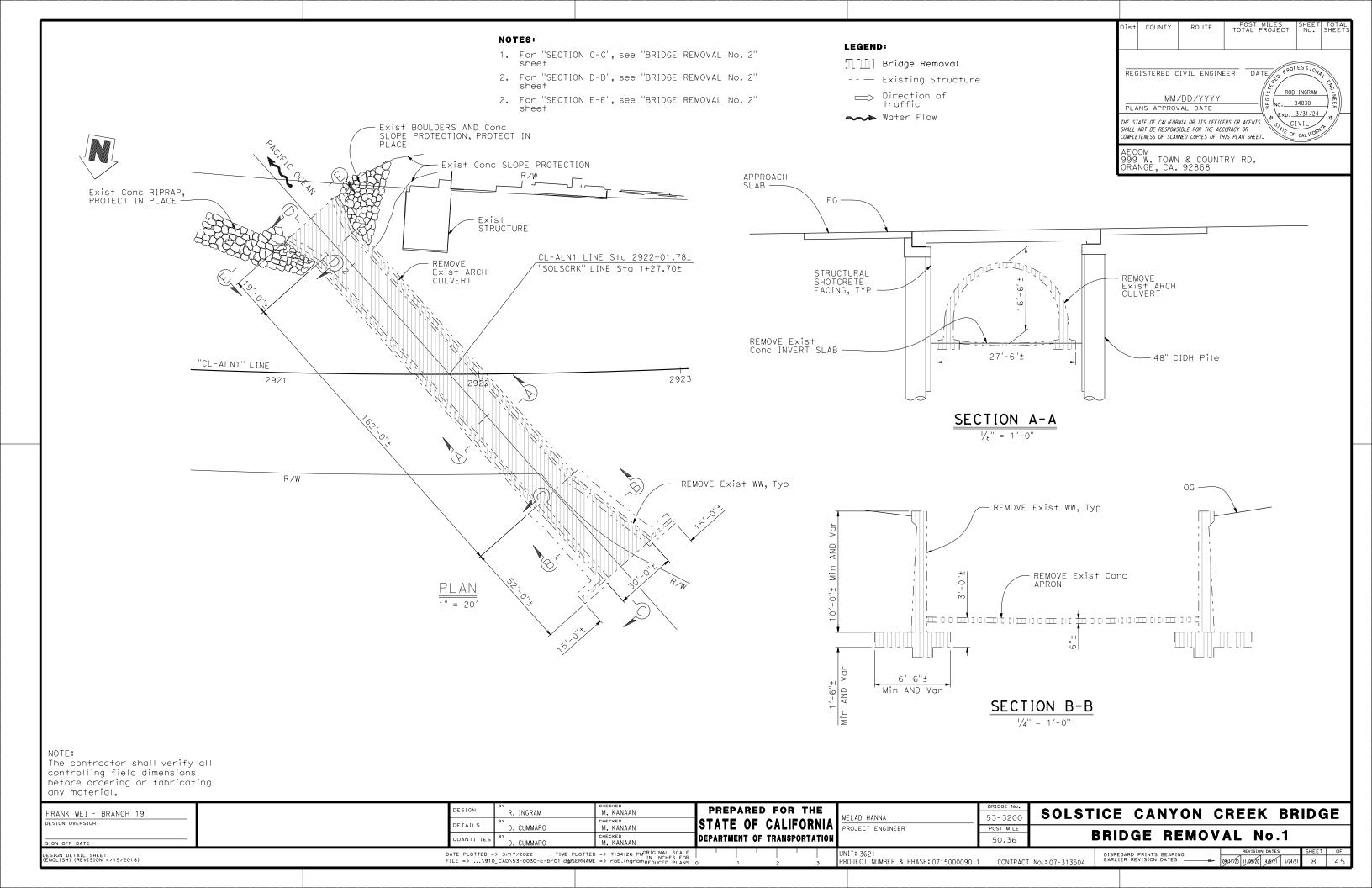
 $\frac{SECTION B-B}{\frac{1}{4}" = 1'-0"}$ 

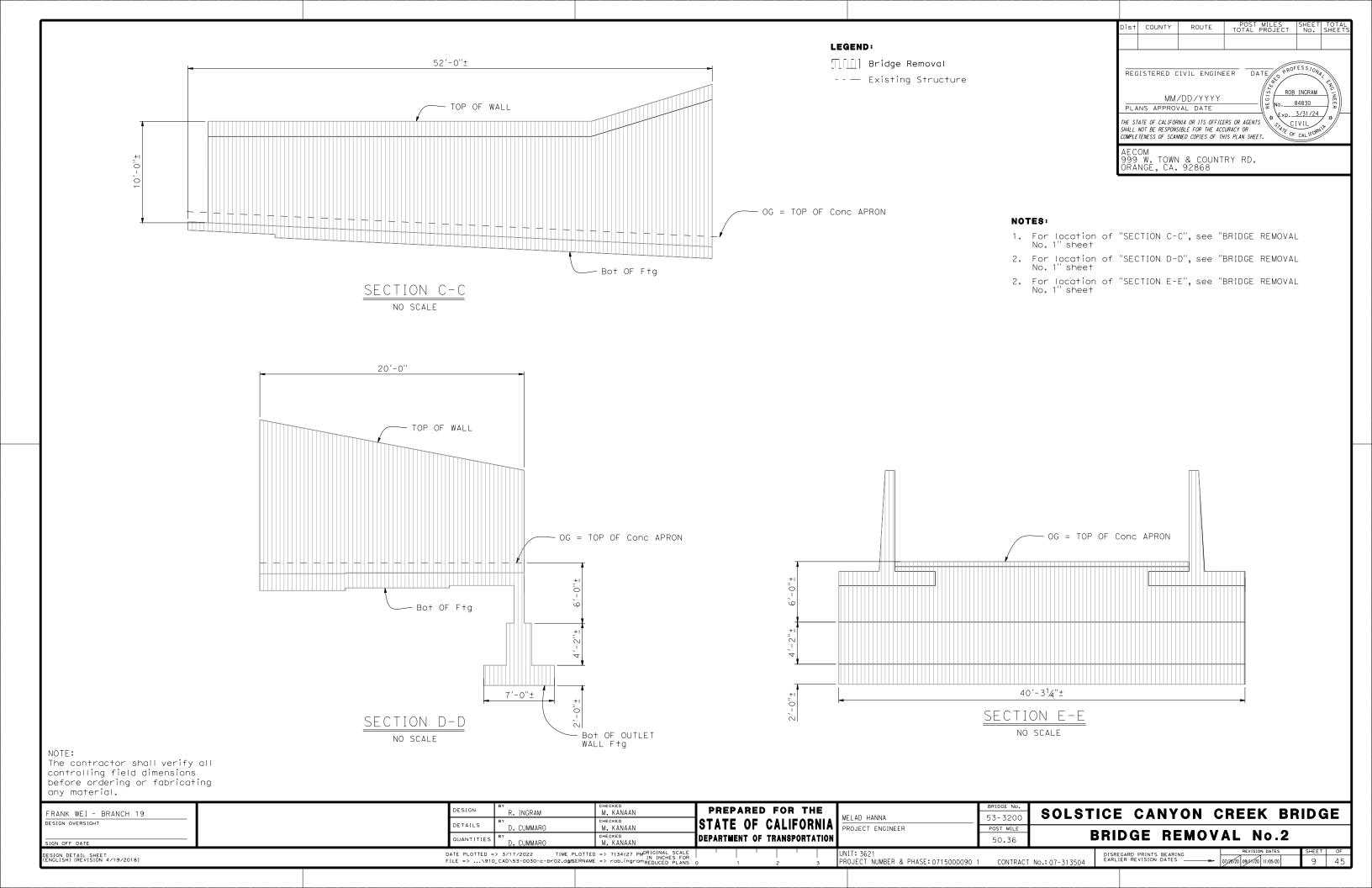
NOTE:
The contractor shall verify al
controlling field dimensions
before ordering or fabricating
any material.

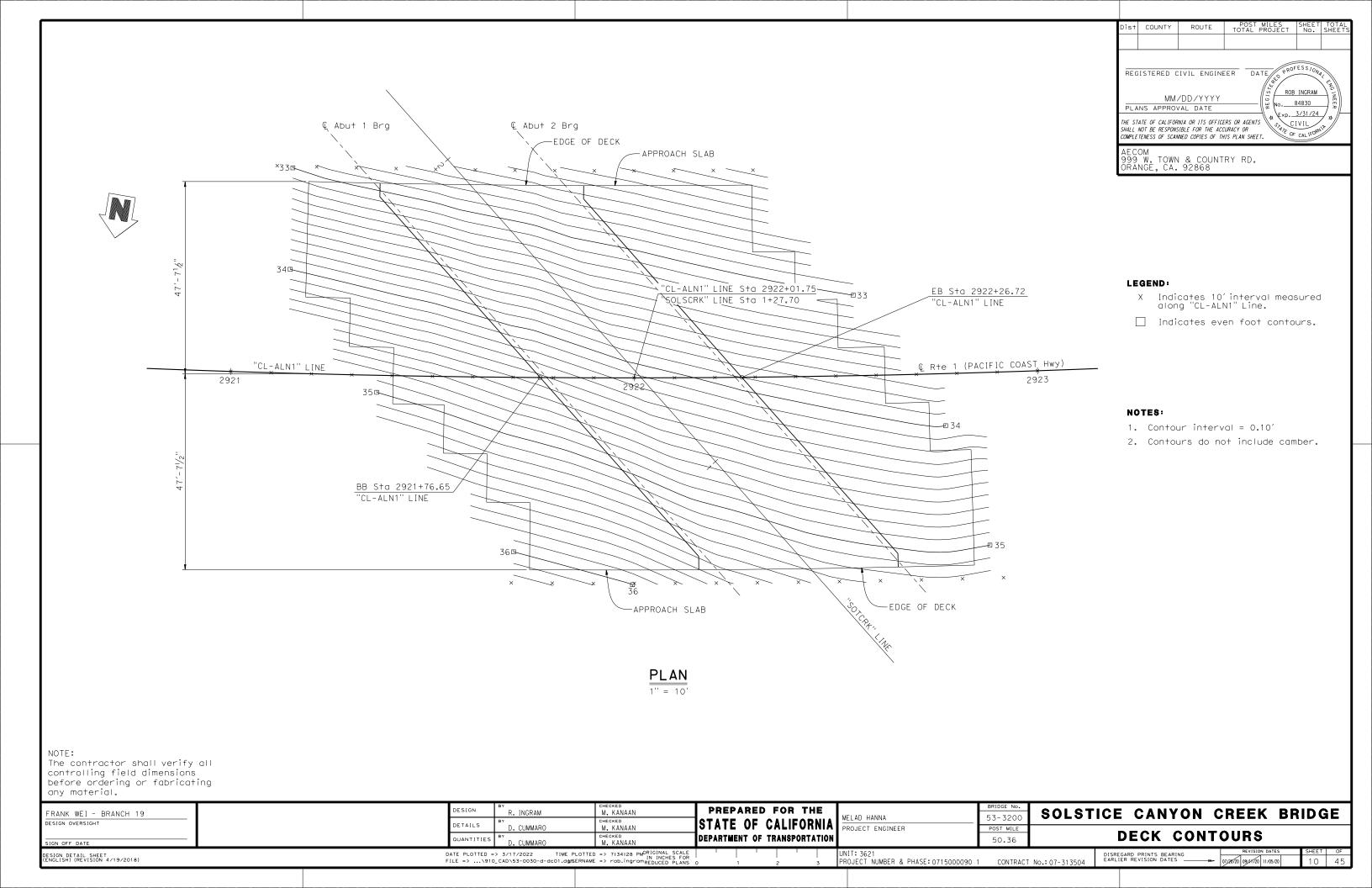
	DESIGN	ВҮ	(	CHECKED	PREPARED FO	D TUE		BRIDGE No.	00105			
FRANK WEI - BRANCH 19	DESIGN	BY	R. INGRAM	M. KANAAN			MELAD HANNA	53-3200	SOLST	CE CANYO	N CREEK	BRIDGE
DESIGN OVERSIGHT	DETAIL	LS	D. CUMMARO	M. KANAAN	STATE OF CAL	IFUKNIA	PROJECT ENGINEER	POST MILE	Ev	OAVATION	LIMITON	
SIGN OFF DATE	QUANTI	ITIES BY	D. CUMMARO	CHECKED M. KANAAN	DEPARTMENT OF TRANS	SPORTATION		50.36	EX	CAVATION	LIMITS NO	7.1
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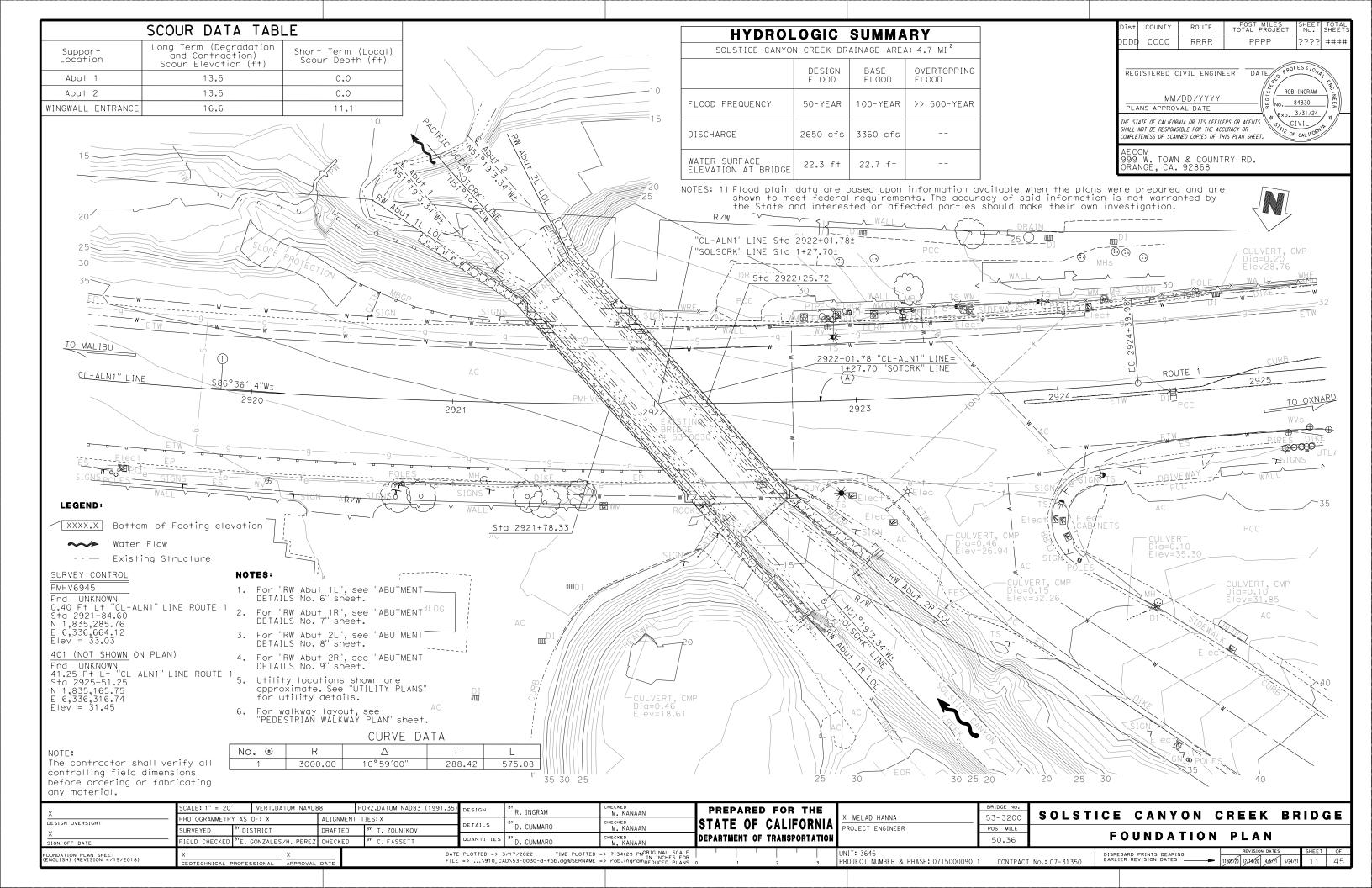


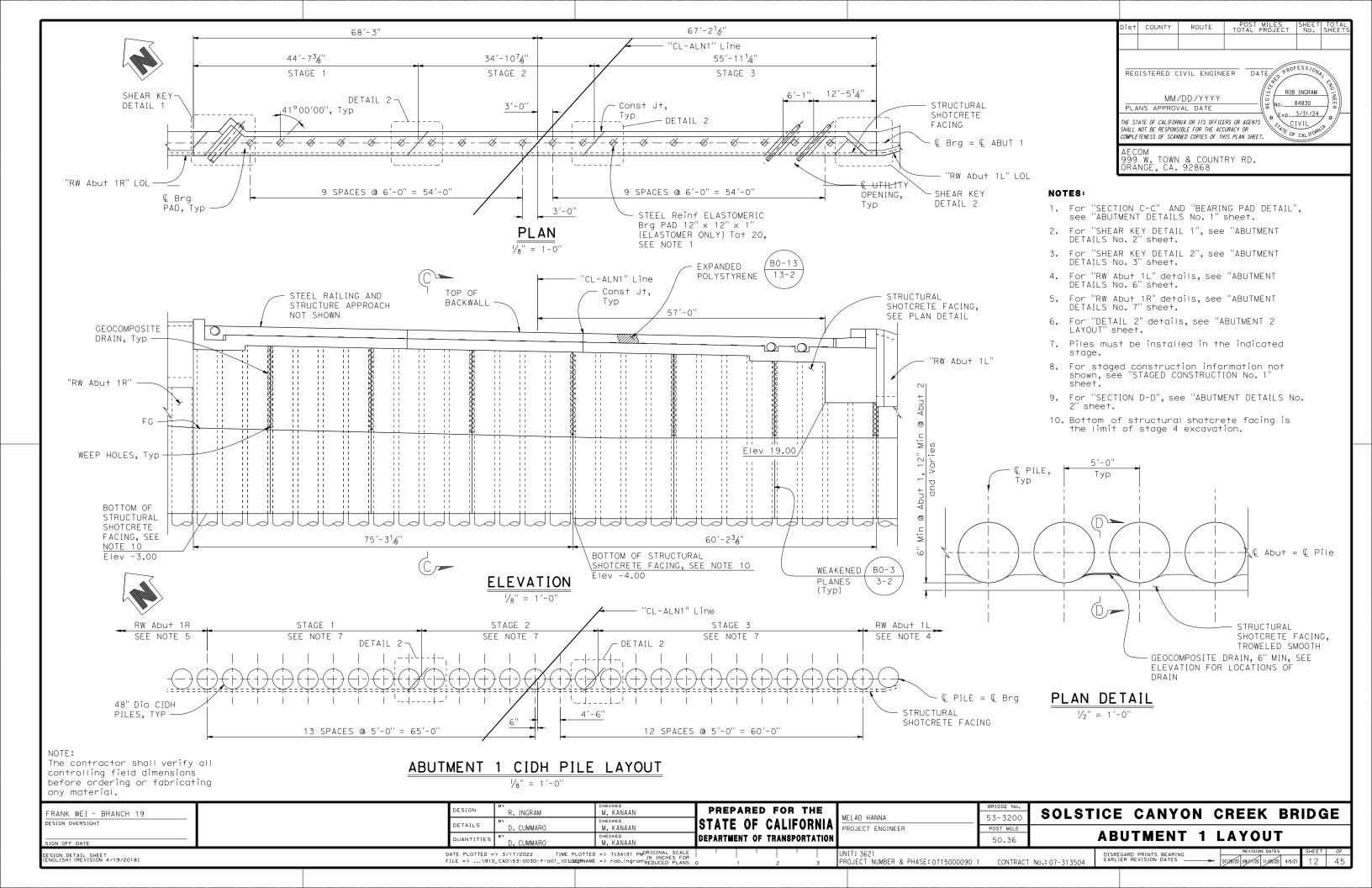


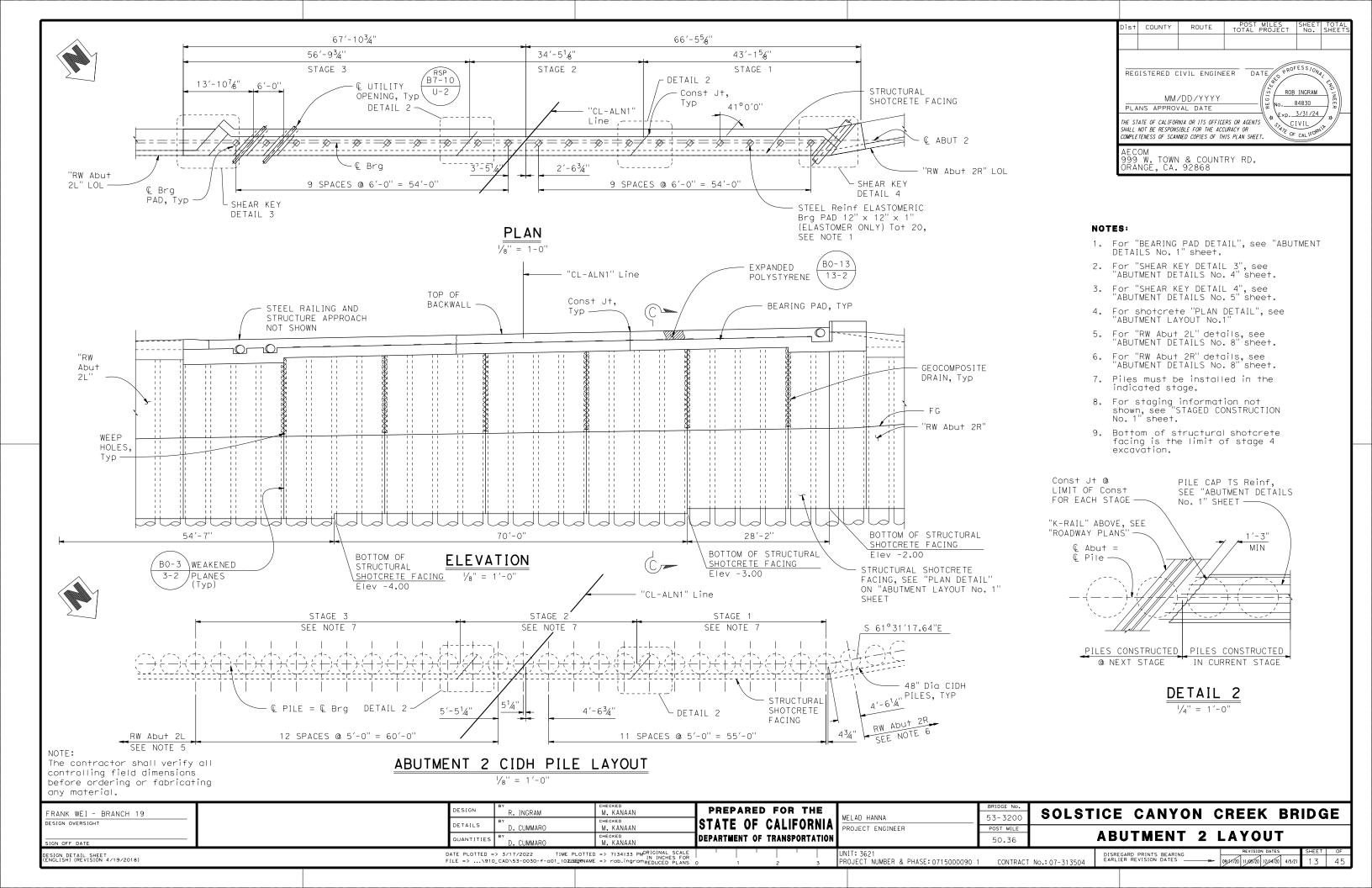


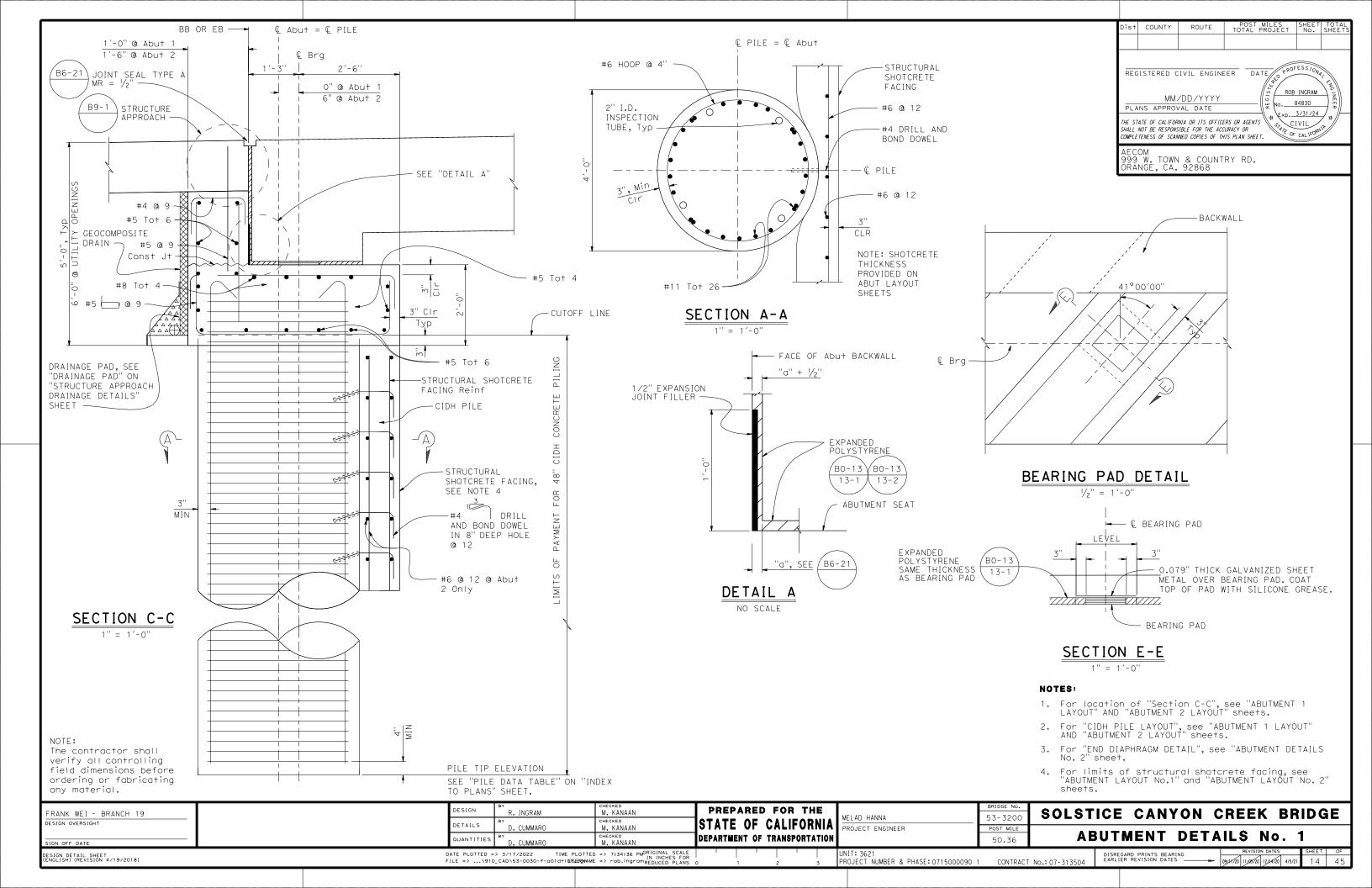


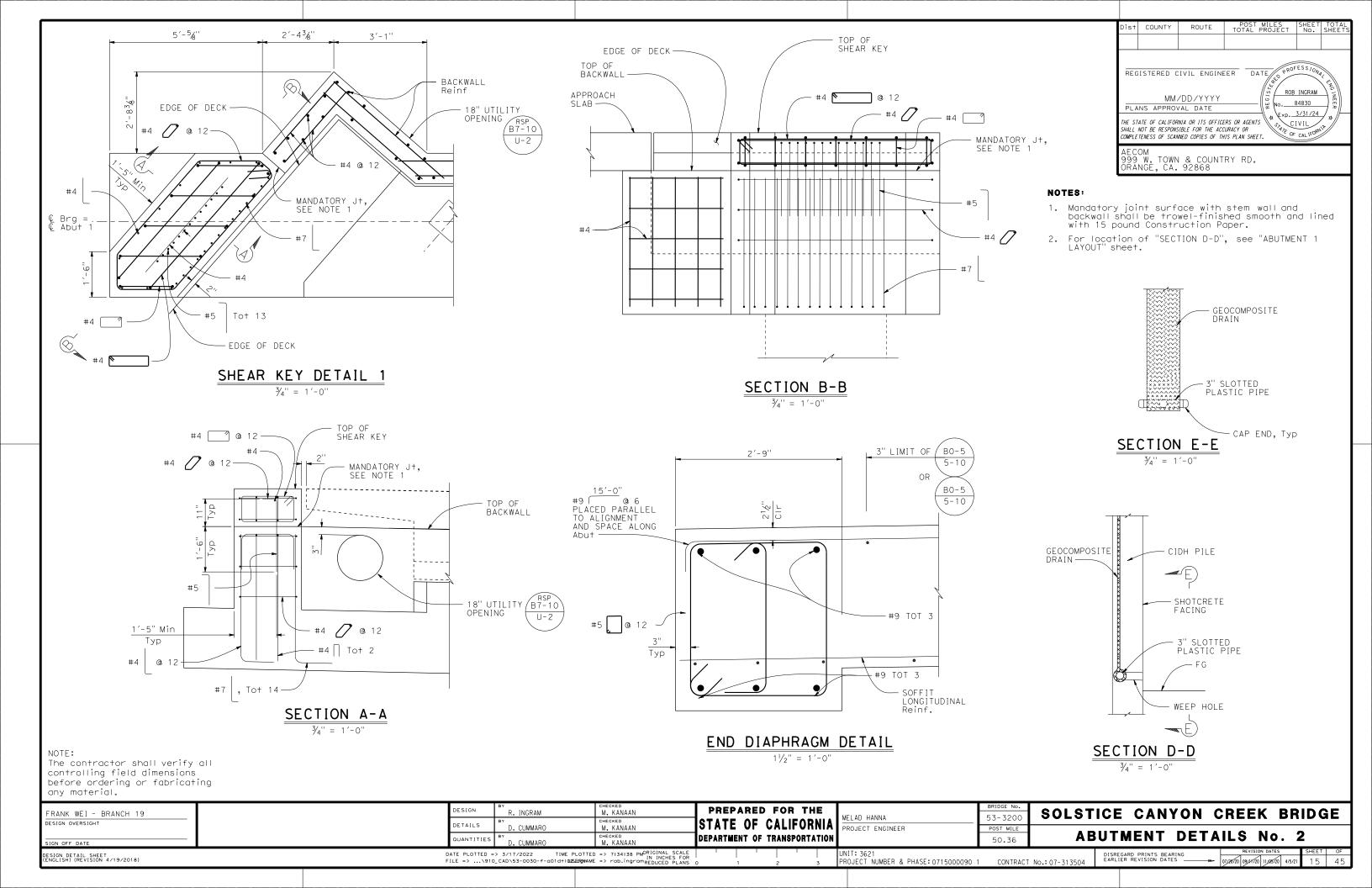


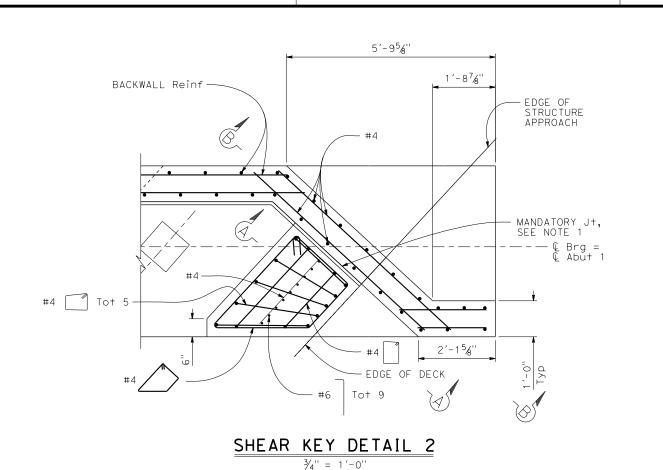


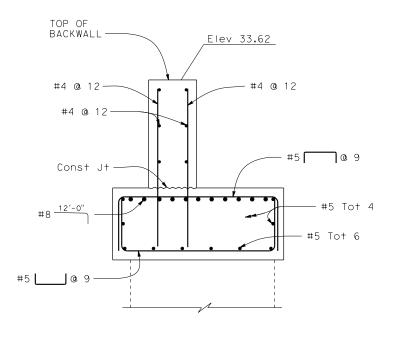




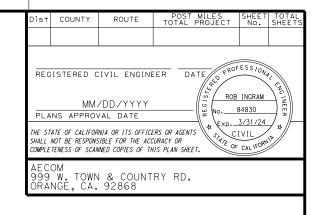






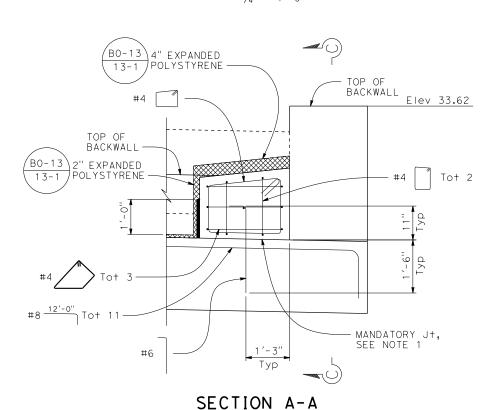


SECTION C-C  $\frac{3}{4}$ " = 1'-0"



#### NOTES:

 Mandatory joint surface with stem wall and backwall shall be trowel-finished smooth and lined with 15 pound Construction Paper.



 $\frac{3}{4}$ " = 1'-0"

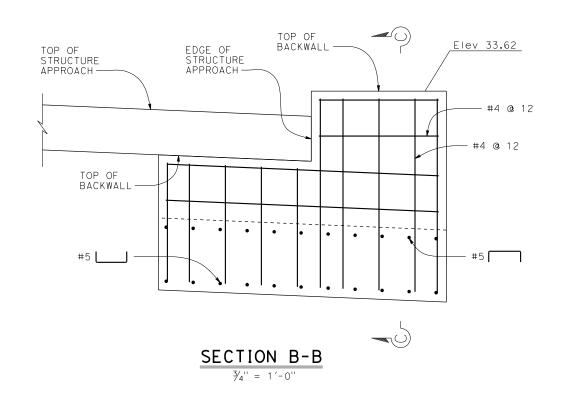
The contractor shall verify all controlling field dimensions before ordering or fabricating

any material.

DESIGN OVERSIGHT

FRANK WEI - BRANCH 19

ESIGN DETAIL SHEET ENGLISH) (REVISION 4/19/2018)

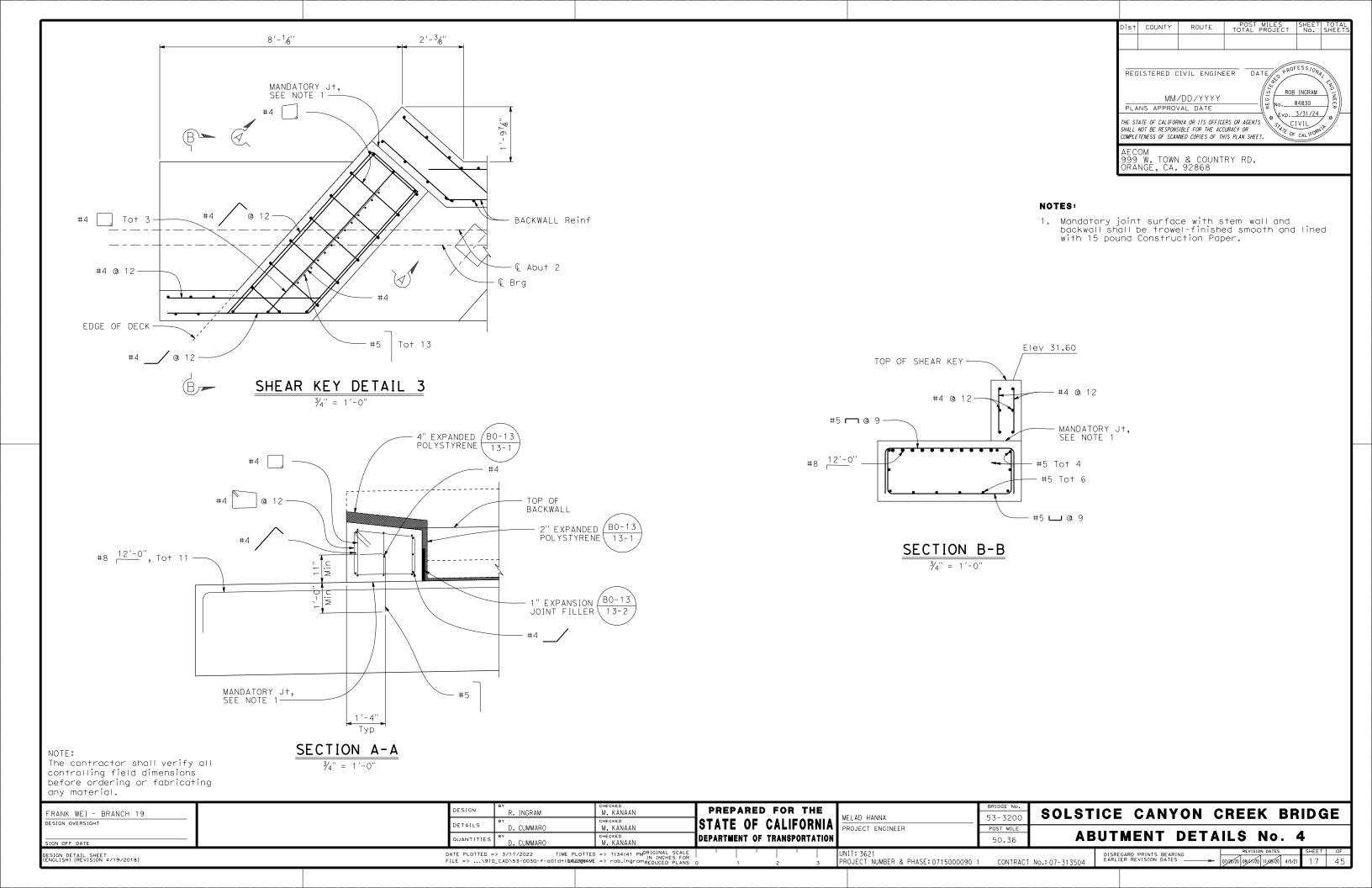


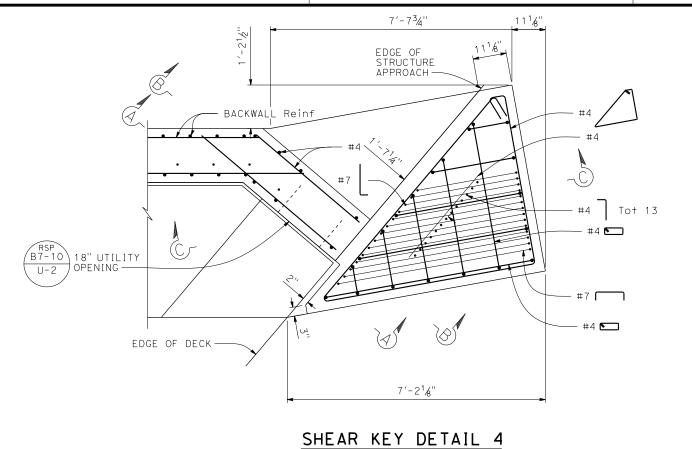
M. KANAAN PREPARED FOR THE DESIGN SOLSTICE CANYON CREEK BRIDGE R. INGRAM MELAD HANNA 53-3200 STATE OF CALIFORNIA PROJECT ENGINEER M. KANAAN DETAILS D. CUMMARO POST MILE ABUTMENT DETAILS No. 3 CHECKED M. KANAAN DEPARTMENT OF TRANSPORTATION QUANTITIES 50.36

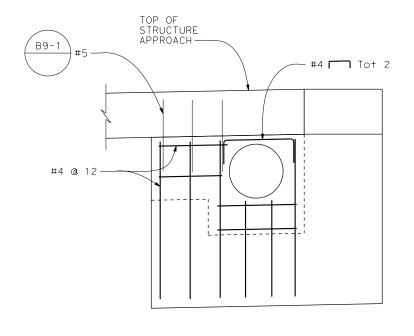
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UNIT: 3621 PROJECT NUMBER & PHASE: 0715000090 1

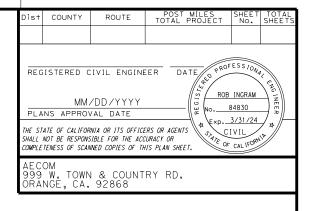
CONTRACT No.: 07-313504







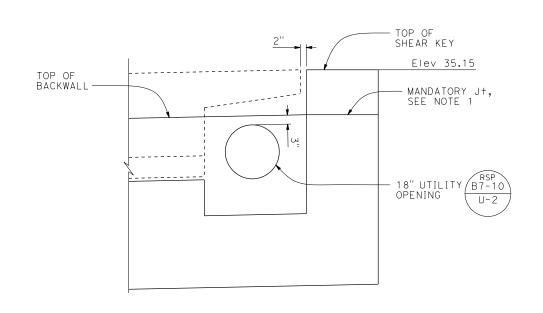
SECTION B-B 3/4" = 1'-0"

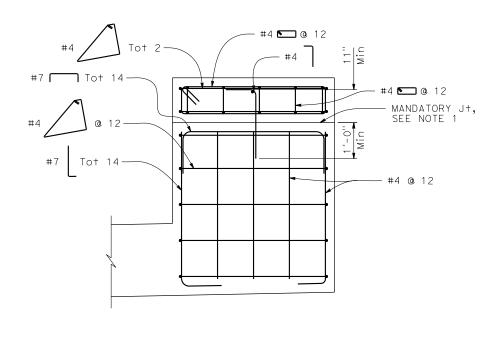


#### NOTES:

Mandatory joint surface with stem wall and backwall shall be trowel-finished smooth and lined with 15 pound Construction Paper.

# 3/4'' = 1'-0''



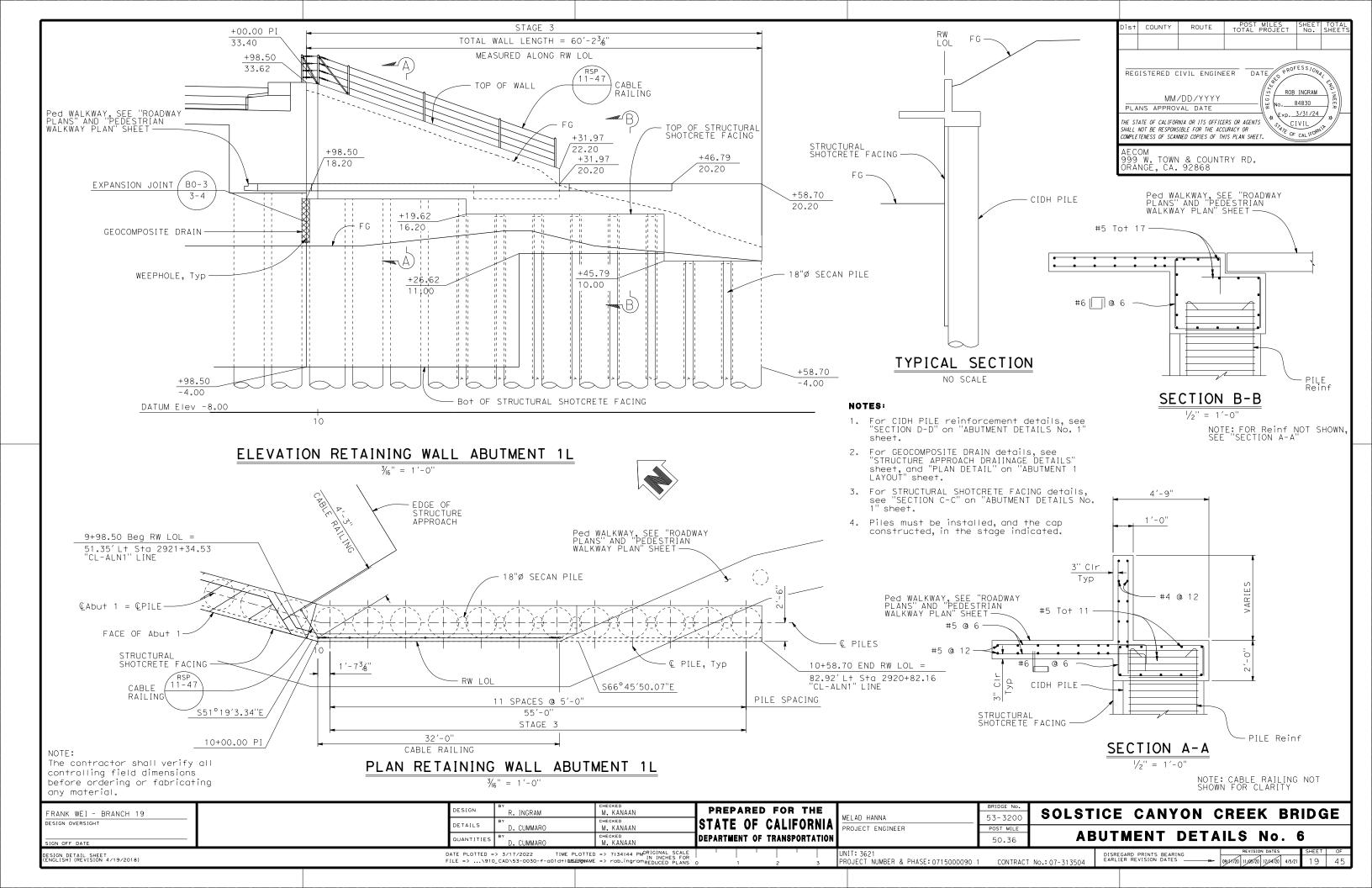


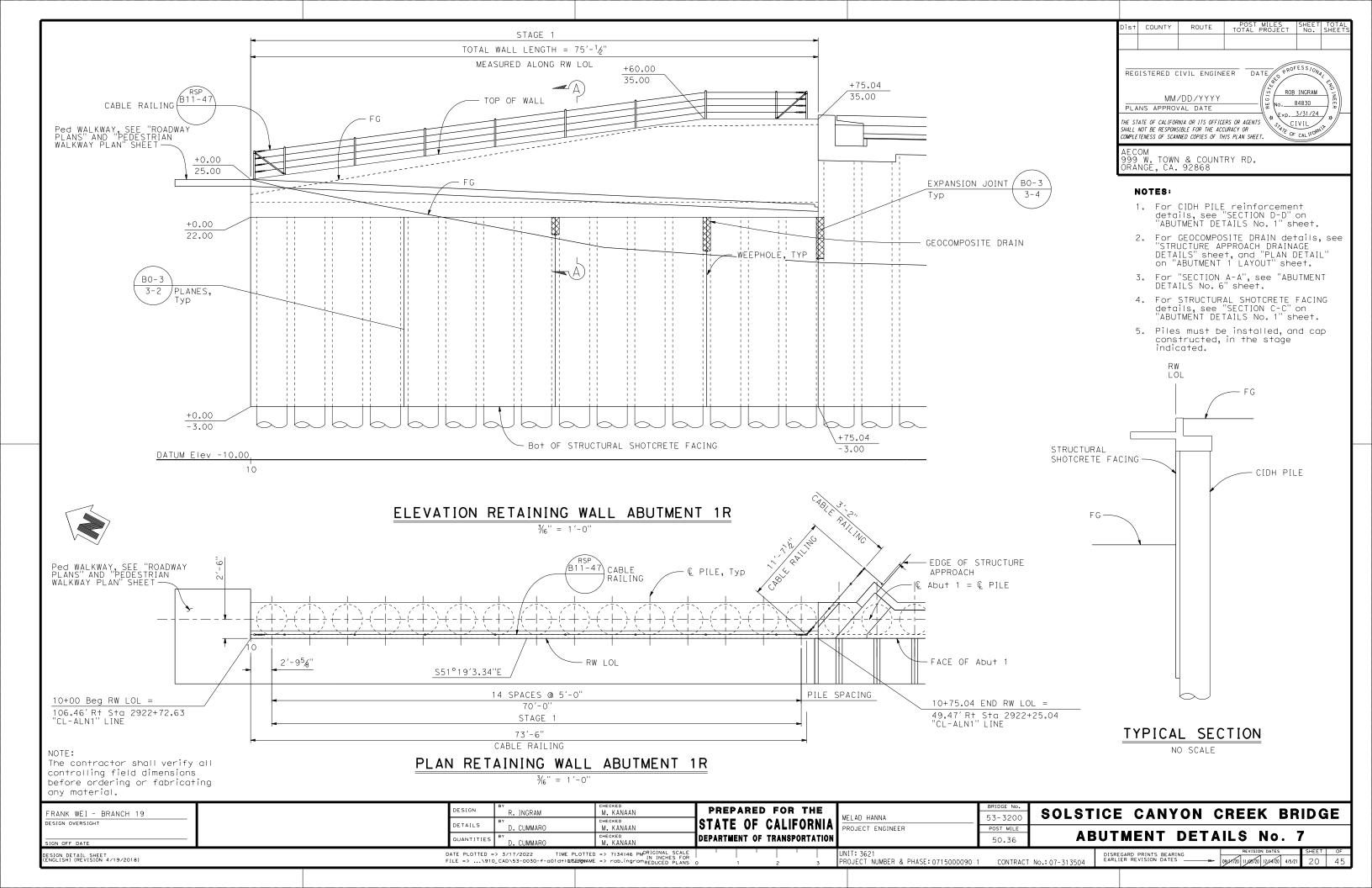
SECTION C-C

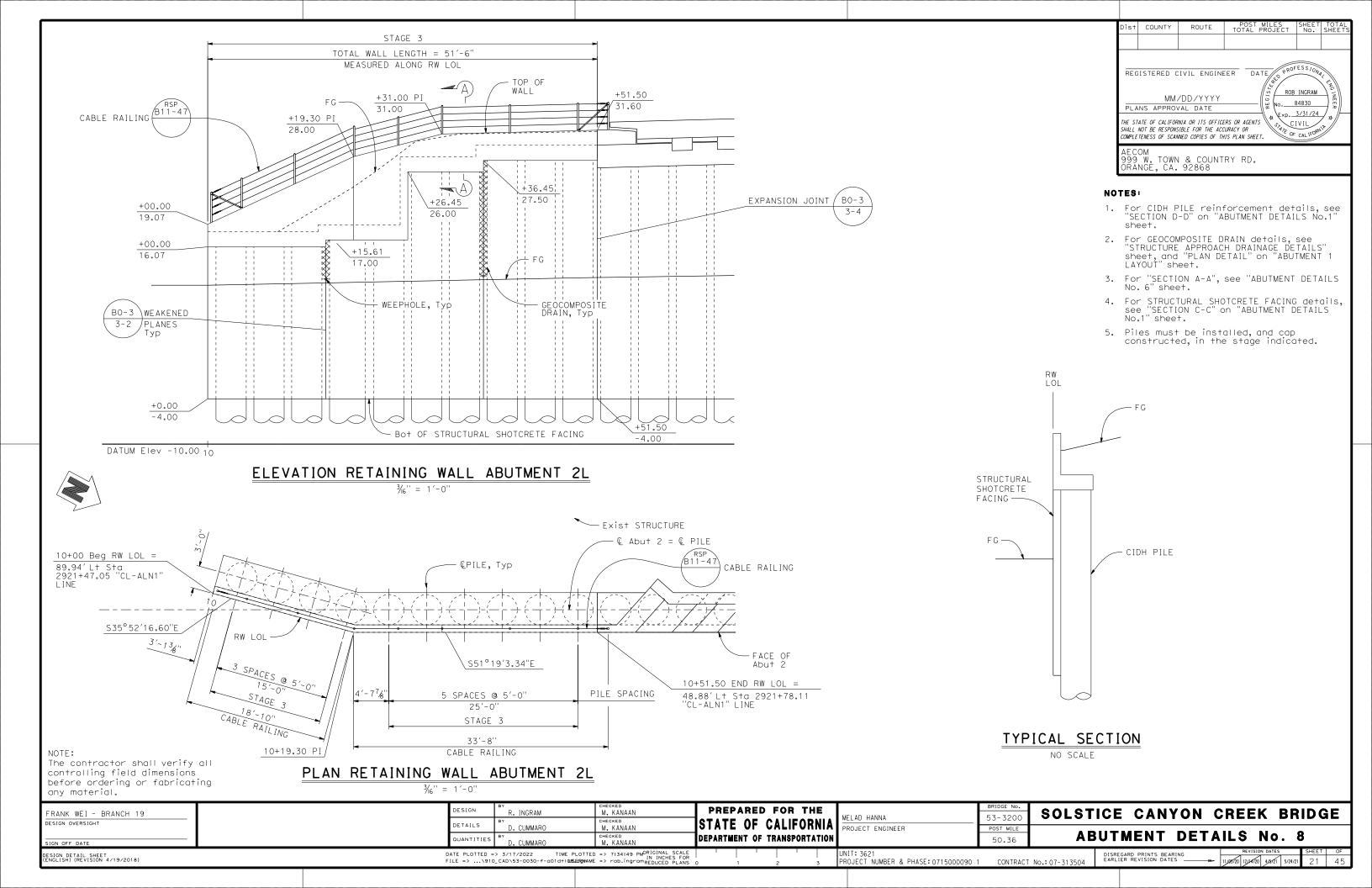
## SECTION A-A

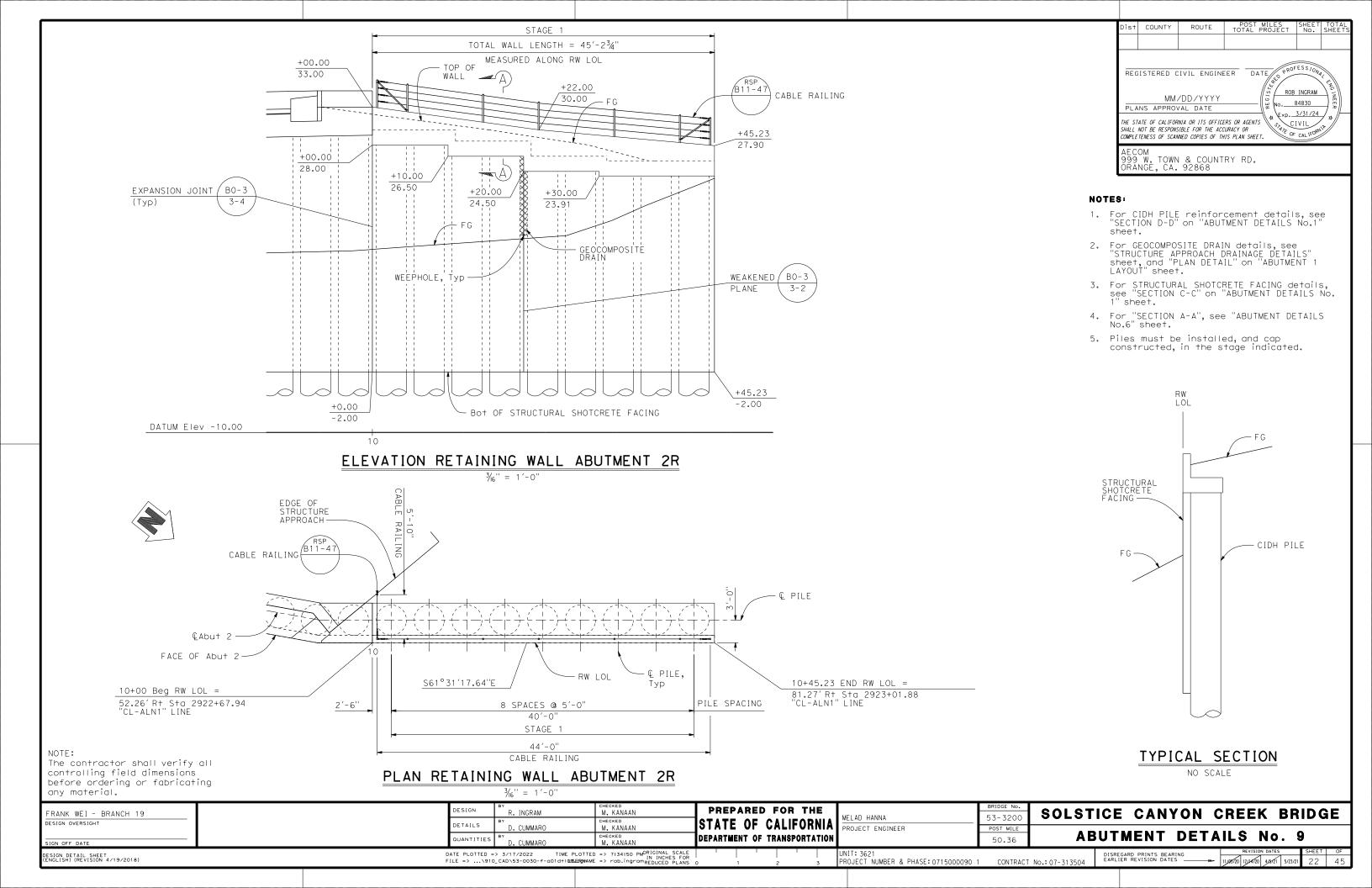
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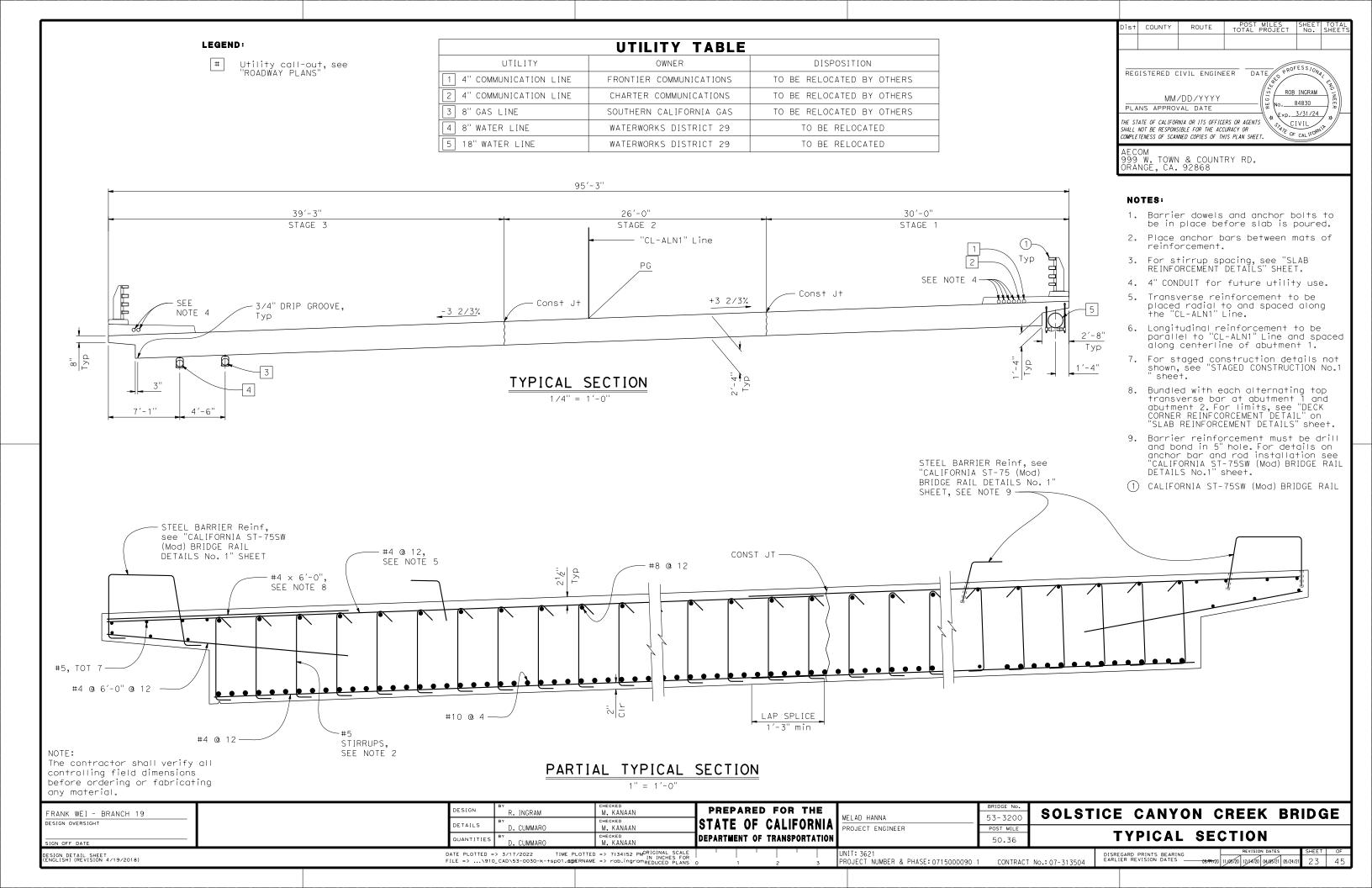
FRANK WEI - BRANCH 19	DESIGN	R. INGRAM	M. KANAAN	PREPARED FOR THE	MELAD HANNIA	BRIDGE No.	SOLST	TICE CANYON CREEK BRIDGE
DESIGN OVERSIGHT	DETAILS	D. CUMMARO	CHECKED M. KANAAN	STATE OF CALIFORNIA	PROJECT ENGINEER	POST MILE		
SIGN OFF DATE	QUANTITIES	D. CUMMARO	CHECKED M. KANAAN	DEPARTMENT OF TRANSPORTATION		50.36	AE	BUTMENT DETAILS No. 5
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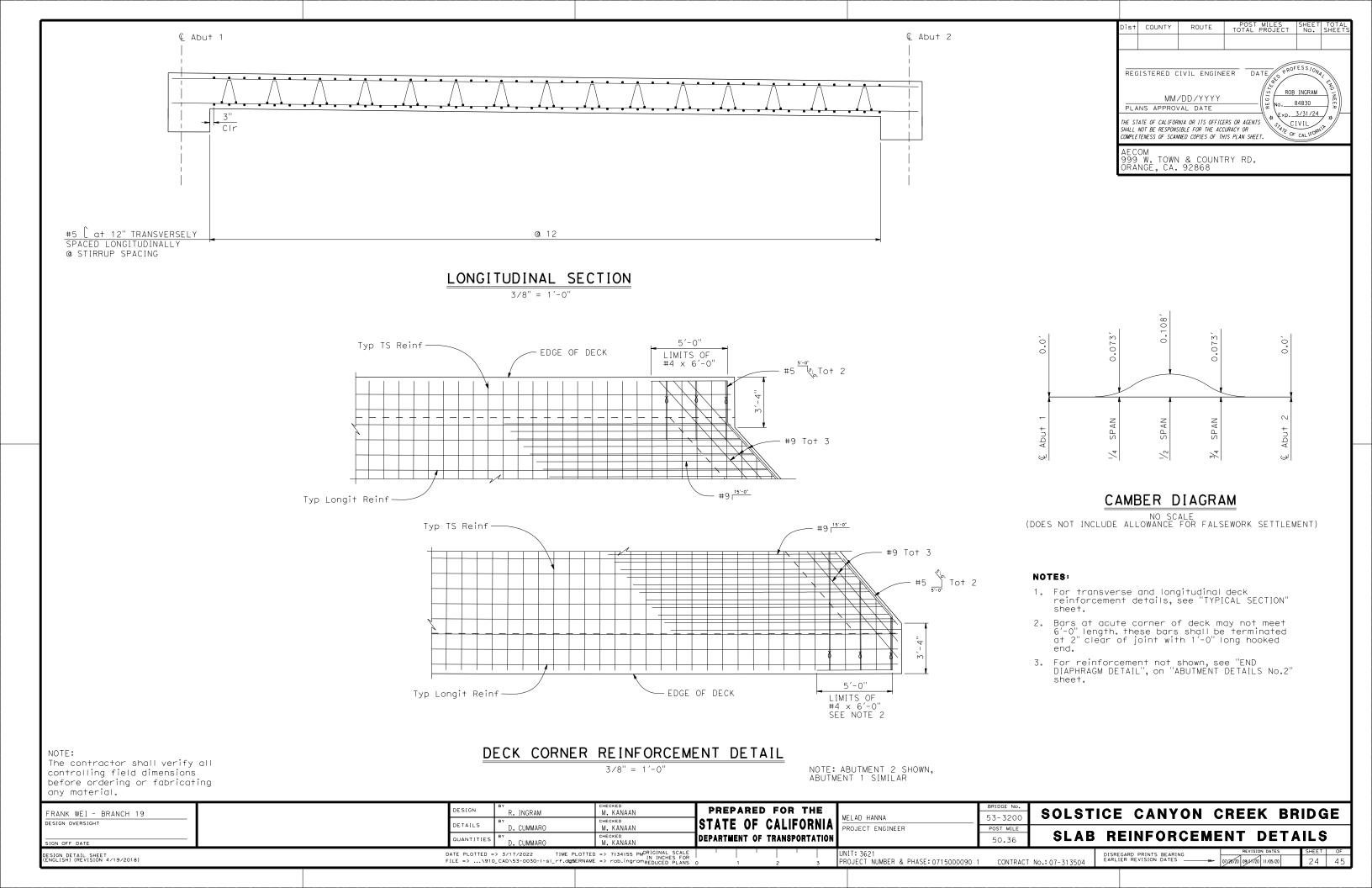


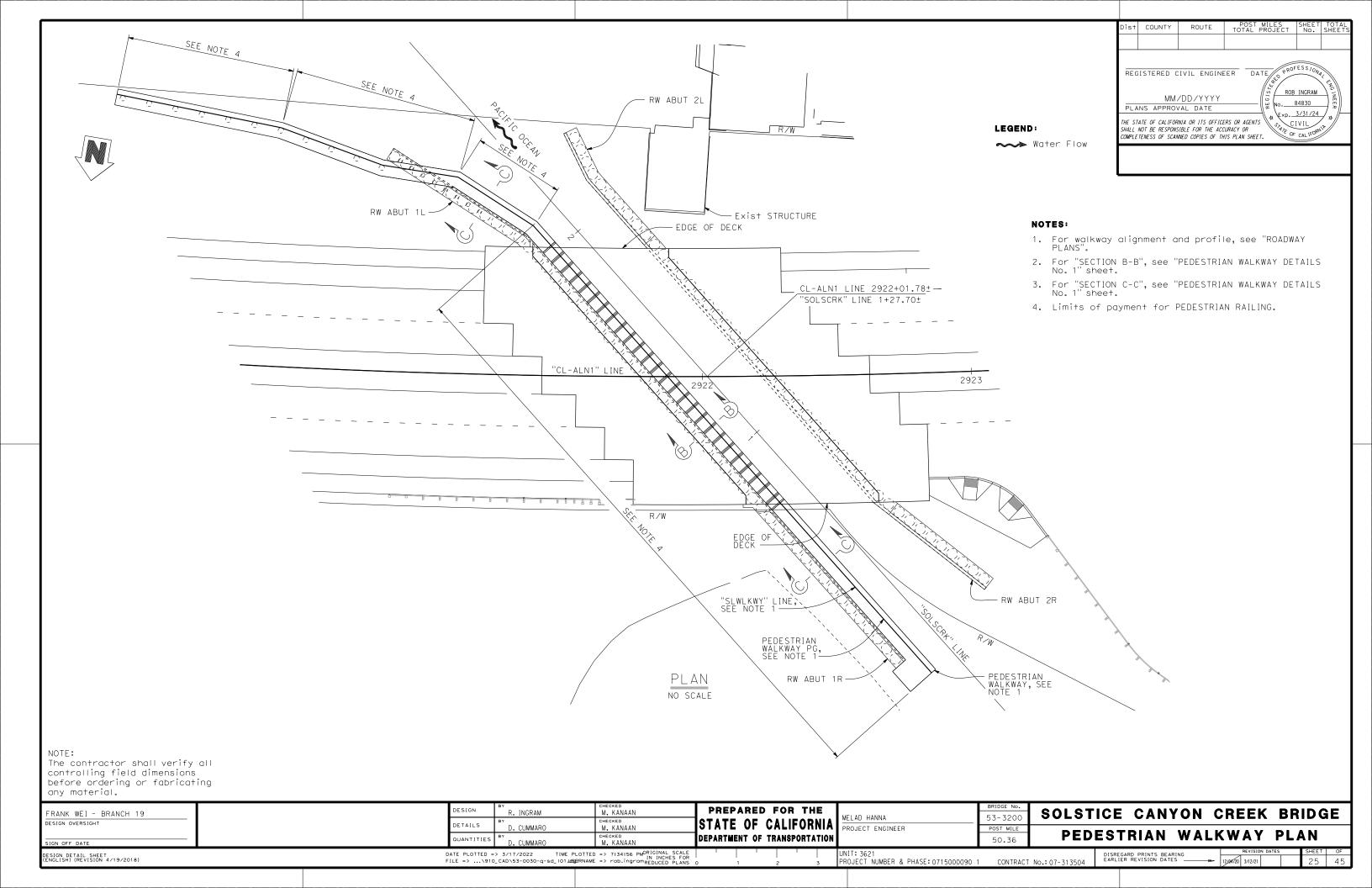


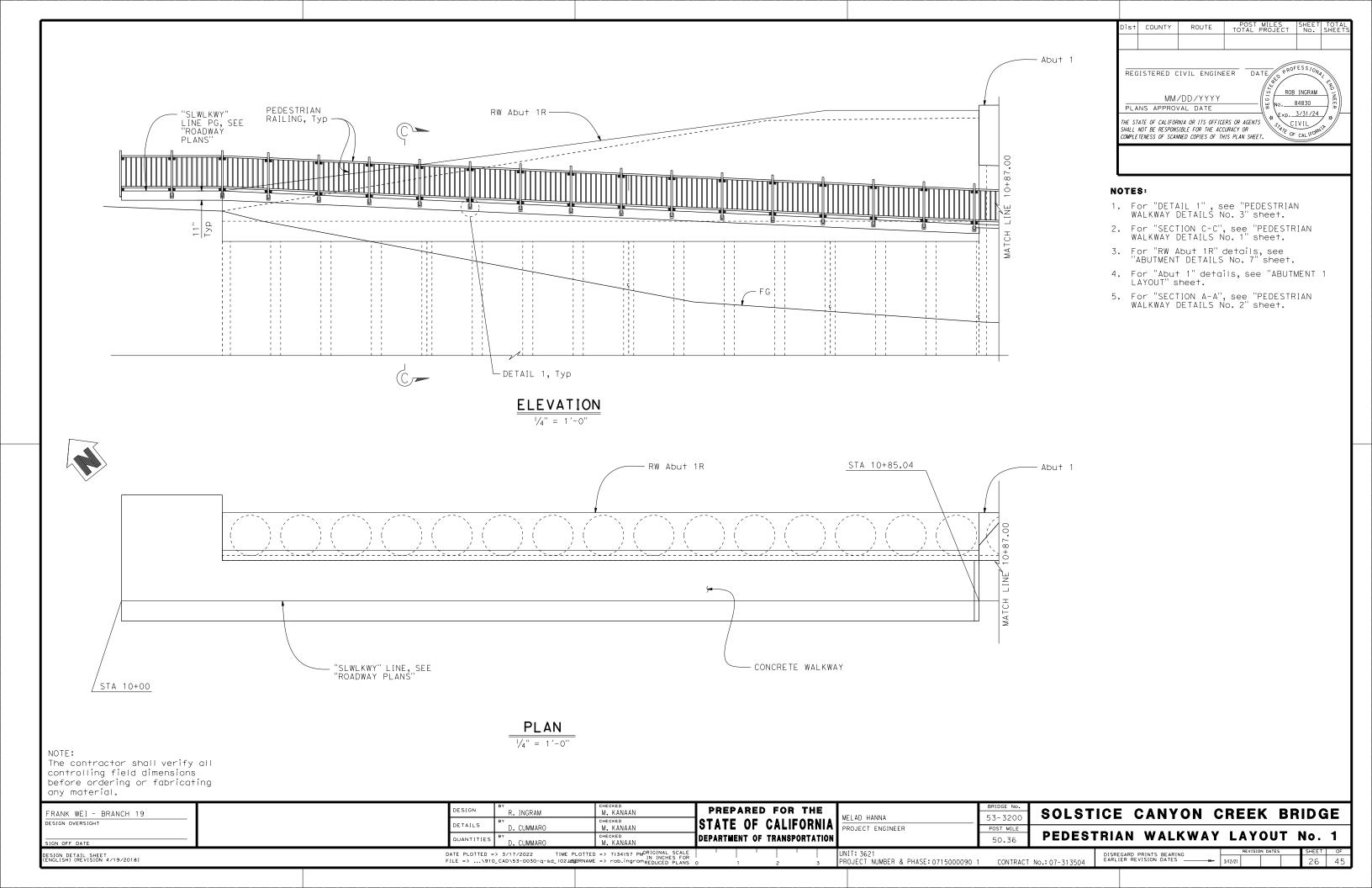


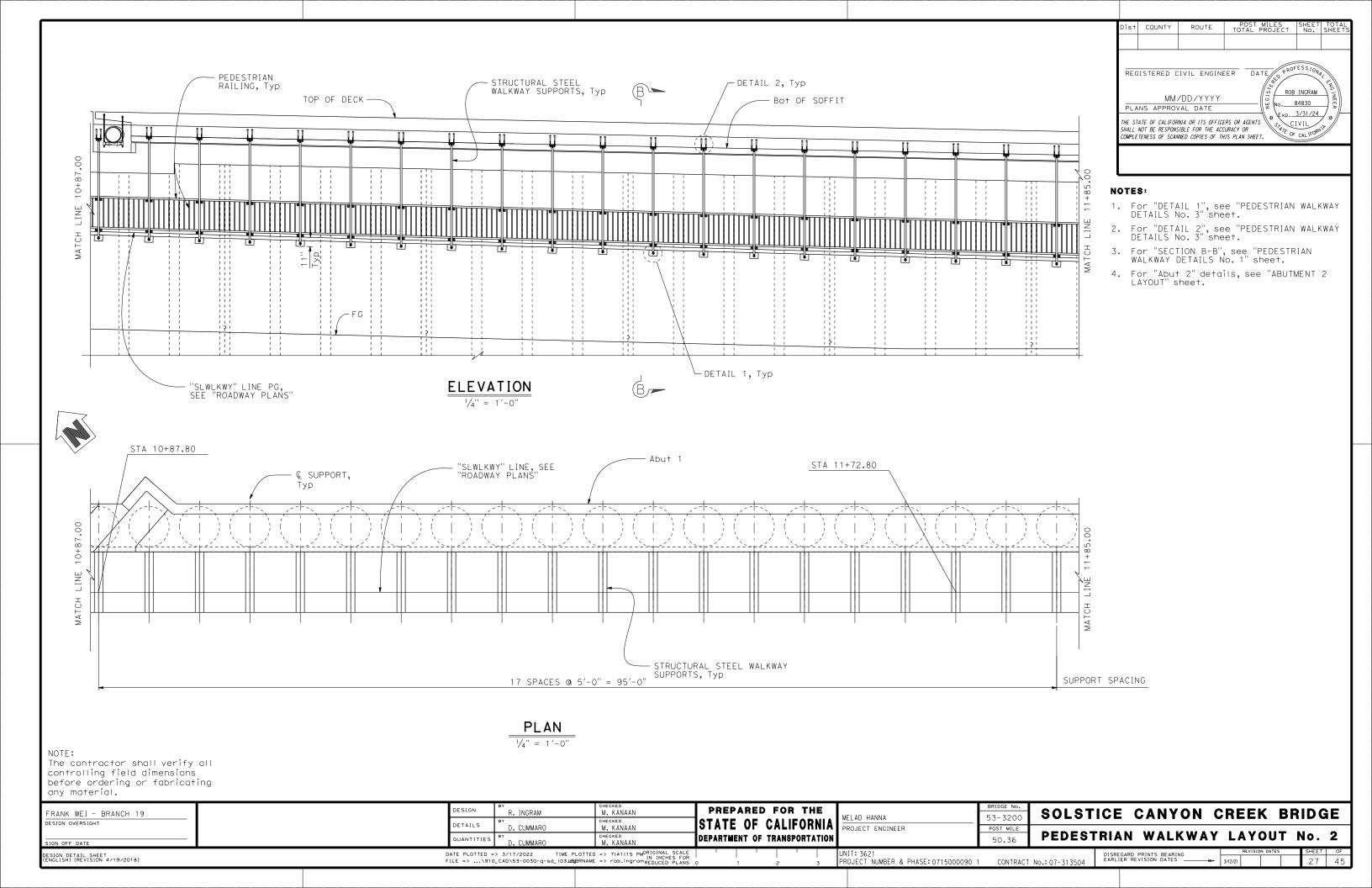


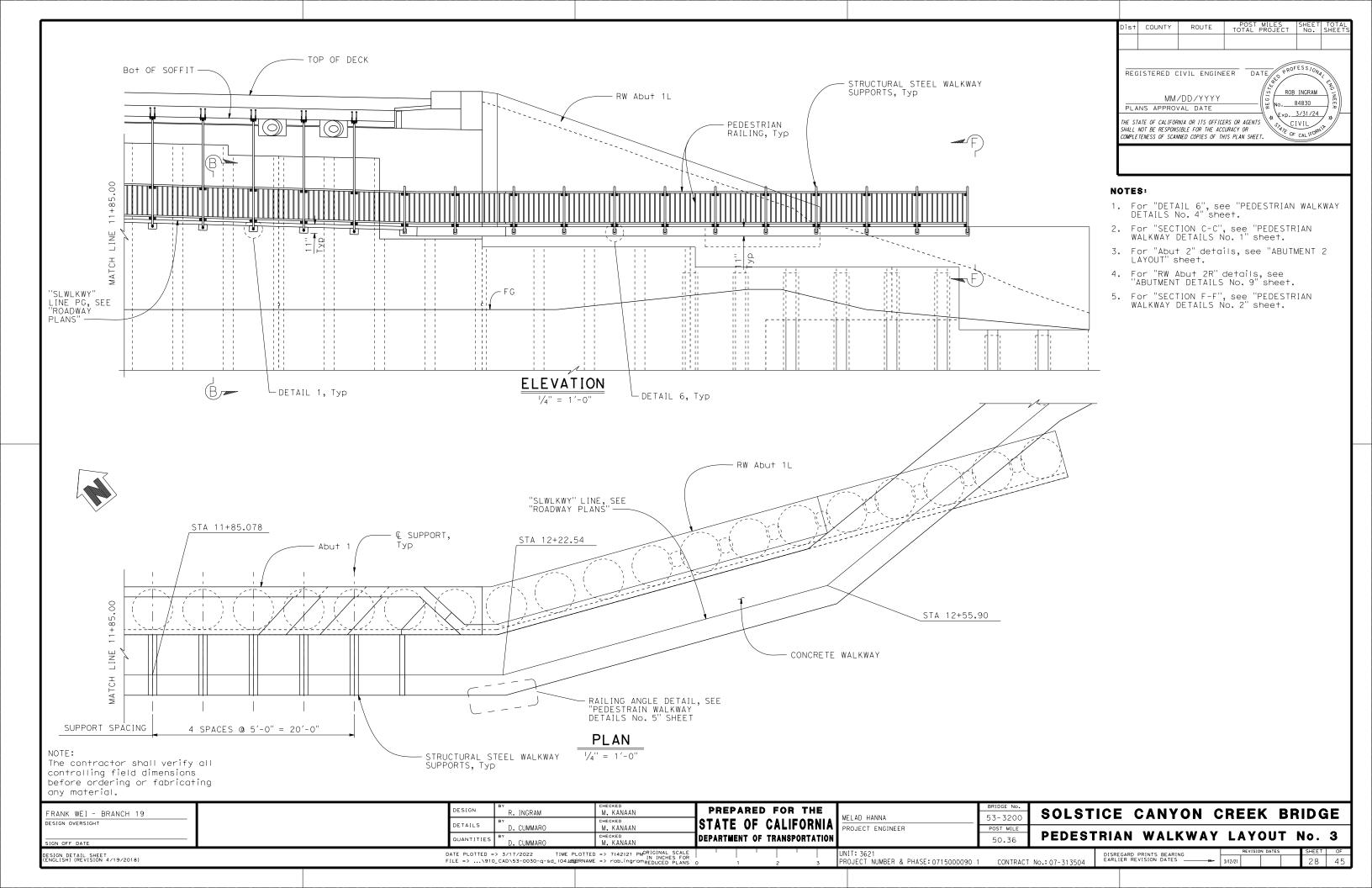


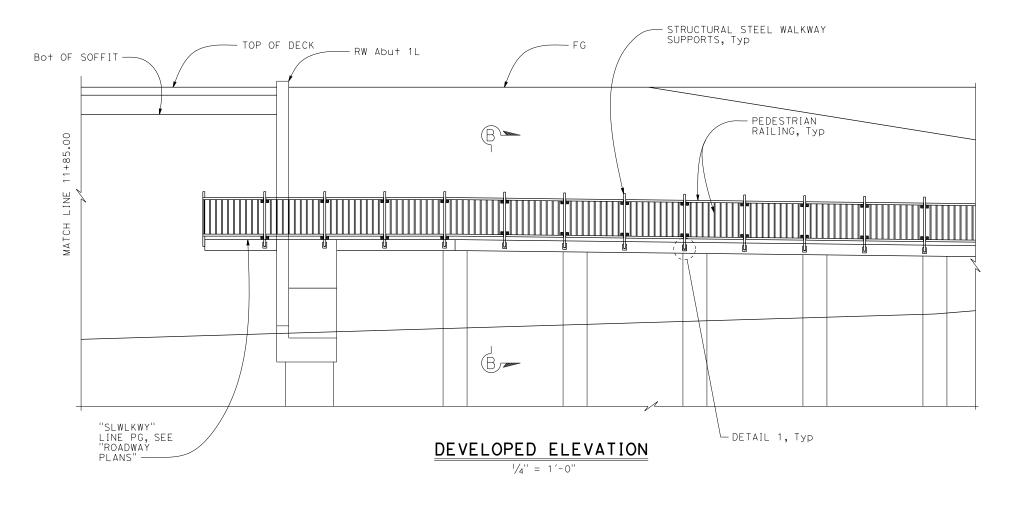


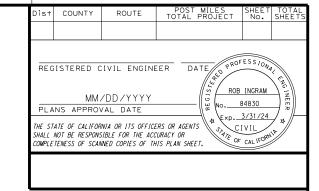






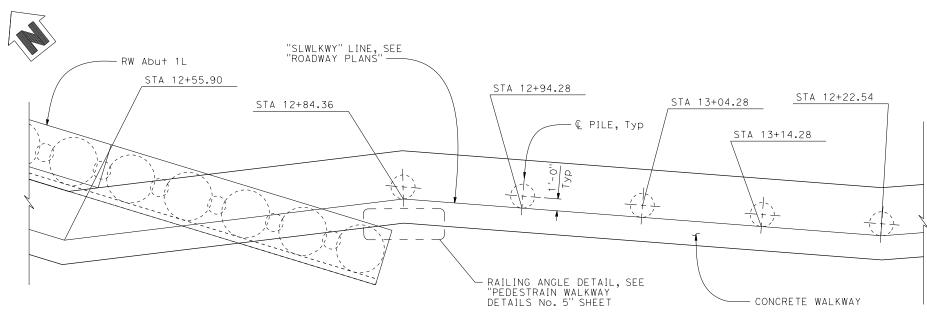






# NOTES:

- 1. For "DETAIL 6", see "PEDESTRIAN WALKWAY DETAILS No. 4" sheet.
- 2. For "SECTION C-C", see "PEDESTRIAN WALKWAY DETAILS No. 1" sheet.
- 3. For "Abut 1" details, see "ABUTMENT 1 LAYOUT" sheet.
- 4. For "RW Abut 1L" details, see "ABUTMENT DETAILS No. 9" sheet.

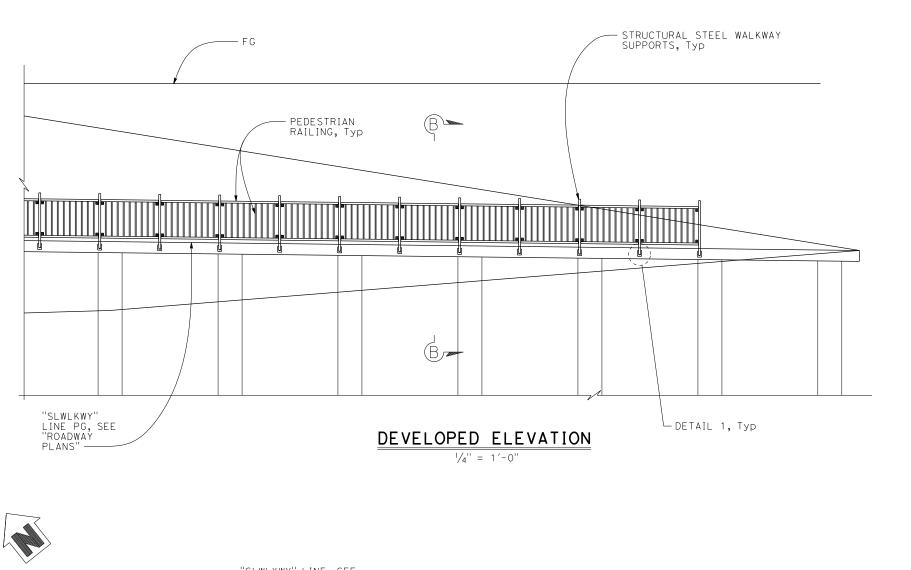


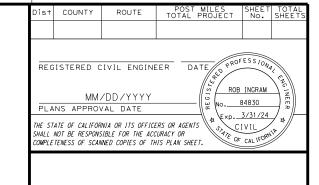
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PLAN

1/4" = 1'-0"

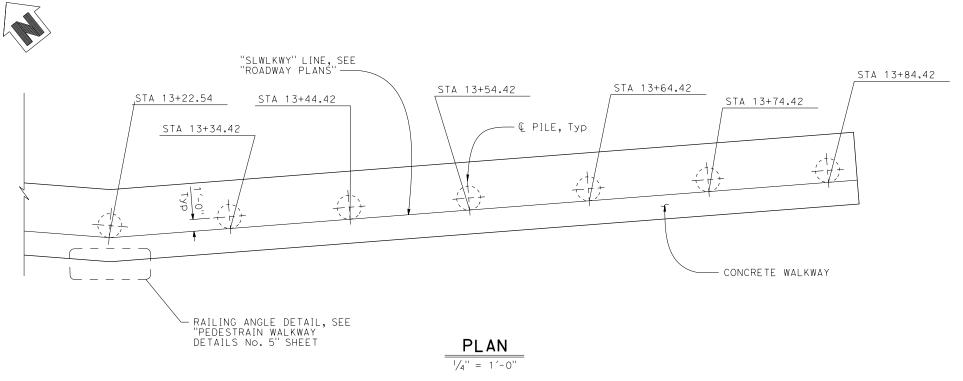
M. KANAAN PREPARED FOR THE SOLSTICE CANYON CREEK BRIDGE DESIGN R. INGRAM FRANK WEI - BRANCH 19 53-3200 MELAD HANNA STATE OF CALIFORNIA PROJECT ENGINEER DESIGN OVERSIGHT M. KANAAN DETAILS D. CUMMARO POST MILE PEDESTRIAN WALKWAY LAYOUT No. 4 CHECKED M. KANAAN DEPARTMENT OF TRANSPORTATION QUANTITIES 50.36 D CLIMMARO DATE PLOTTED => 3/17/2022 TIME PLOTTED => 7:35:01 PMORIGINAL SCALE IN INCHES FOR FILE => ...\910\_CAD\53-0030-q-sd\_105148ftRNAME => rob.ingramREDUCED PLANS UNIT: 3621 PROJECT NUMBER & PHASE: 0715000090 1 ESIGN DETAIL SHEET ENGLISH) (REVISION 4/19/2018) DISREGARD PRINTS BEARING EARLIER REVISION DATES CONTRACT No.: 07-313504





# NOTES:

- 1. For "DETAIL 6", see "PEDESTRIAN WALKWAY DETAILS No. 4" sheet.
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M. KANAAN

M. KANAAN

CHECKED M. KANAAN

PREPARED FOR THE

DEPARTMENT OF TRANSPORTATION

STATE OF CALIFORNIA PROJECT ENGINEER

SOLSTICE CANYON CREEK BRIDGE PEDESTRIAN WALKWAY LAYOUT No. 5

DATE PLOTTED => 3/17/2022 TIME PLOTTED => 7:35:02 PMORIGINAL SCALE IN INCHES FOR FILE => ...\910\_CAD\53-0030-q-sd\_106148ftRNAME => rob.ingramREDUCED PLANS

R. INGRAM

D. CUMMARO

D CLIMMARO

DESIGN

DETAILS

QUANTITIES

The contractor shall verify all controlling field dimensions

before ordering or fabricating

any material.

DESIGN OVERSIGHT

FRANK WEI - BRANCH 19

DESIGN DETAIL SHEET ENGLISH) (REVISION 4/19/2018)

UNIT: 3621 PROJECT NUMBER & PHASE: 0715000090 1

MELAD HANNA

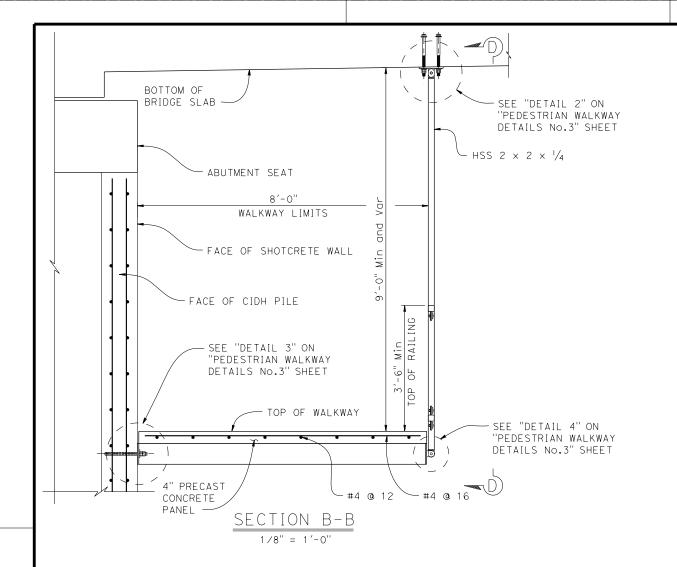
CONTRACT No.: 07-313504

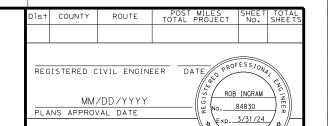
53-3200

POST MILE

50.36

DISREGARD PRINTS BEARING EARLIER REVISION DATES





STATE OF CALIFORNI

# MATERIAL NOTES

ANCHOR BOLTS: fy = 105 ksi
STRUCTURAL STEEL: fy = 36 ksi
BOLTS: fy = 90 ksi
HSS: fy=46 ski
COATING: All steel members shall be hot dip
galvanized after fabrication.

THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS
SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR
COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET.

# NOTES:

1. Precast concrete panel reinforcement must be epoxy coated.

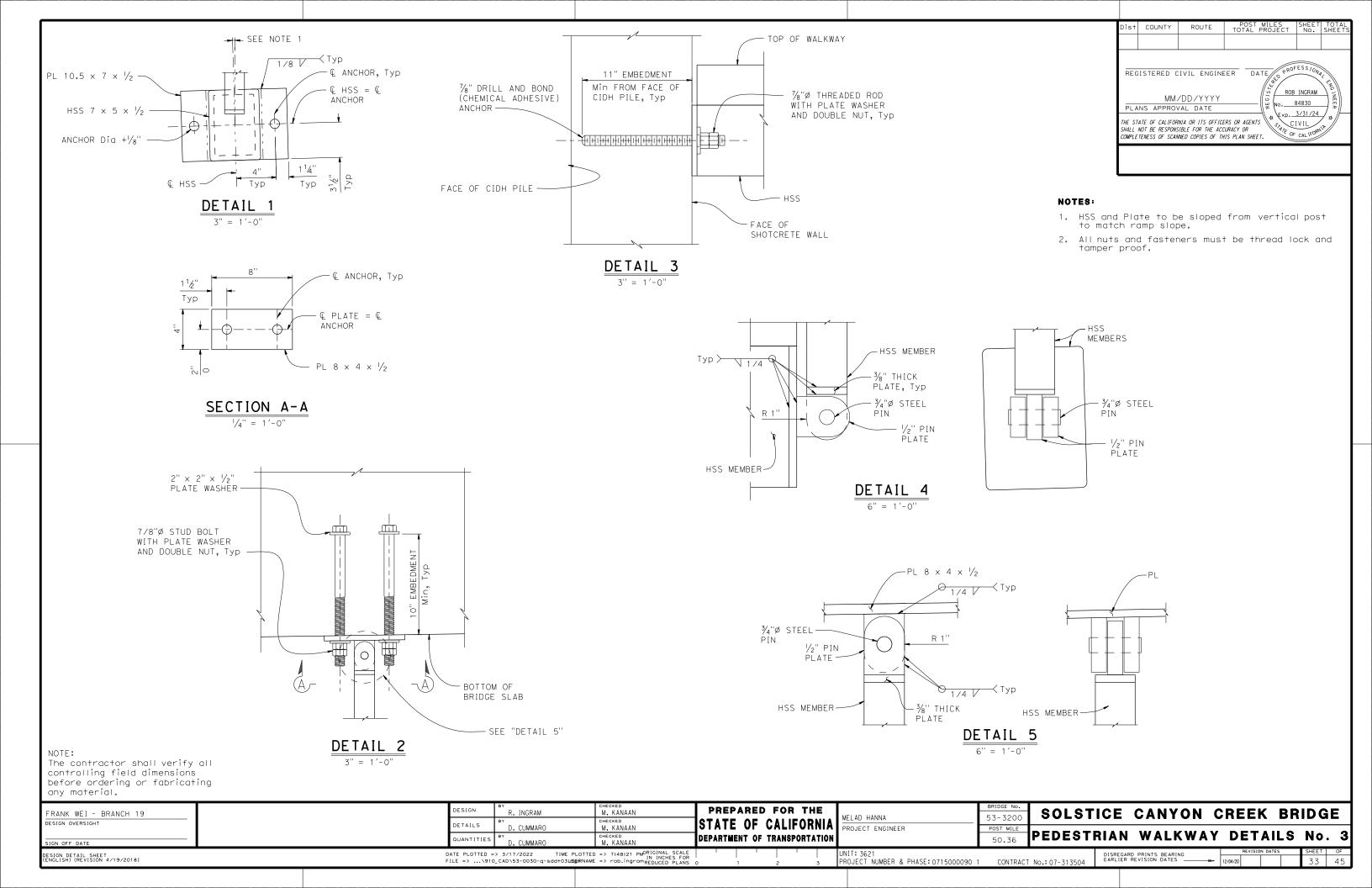
FRANK WEI - BRANCH 19	DESIGN	R. IN	NGRAM	M. KANAAN		RED FOR		MELAD HANNA	BRIDGE No.	SOLST	ICE	CANYON C	REEK BE	RIDGE
DESIGN OVERSIGHT	DETAILS	BY D. Cl	UMMARO	M. KANAAN	STATE	OF CALI	FORNIA	PROJECT ENGINEER	POST MILE					
SIGN OFF DATE	QUANTITI	ES BY D. CU	UMMARO	M. KANAAN	DEPARTMEN	T OF TRANS	PORTATION		50.36	PEDEST	RIAN	WALKWAY	DETAILS	5 No. 1
DESIGN DETAIL SHEET (ENGLISH) (REVISION 4/19/2018)	DATE PLOTTE  FILE =>	ED => 3/17/20 \910_CAD\53-0		) => 7:35:04 PMORIGINAL SCALE IN INCHES FOR E => rob.ingromREDUCED PLANS	, ,	3	3	UNIT: 3621 PROJECT NUMBER & PHASE: 0715000090 1	CONTRACT	No : 07-313504		D PRINTS BEARING REVISION DATES	REVISION DATES	31 45

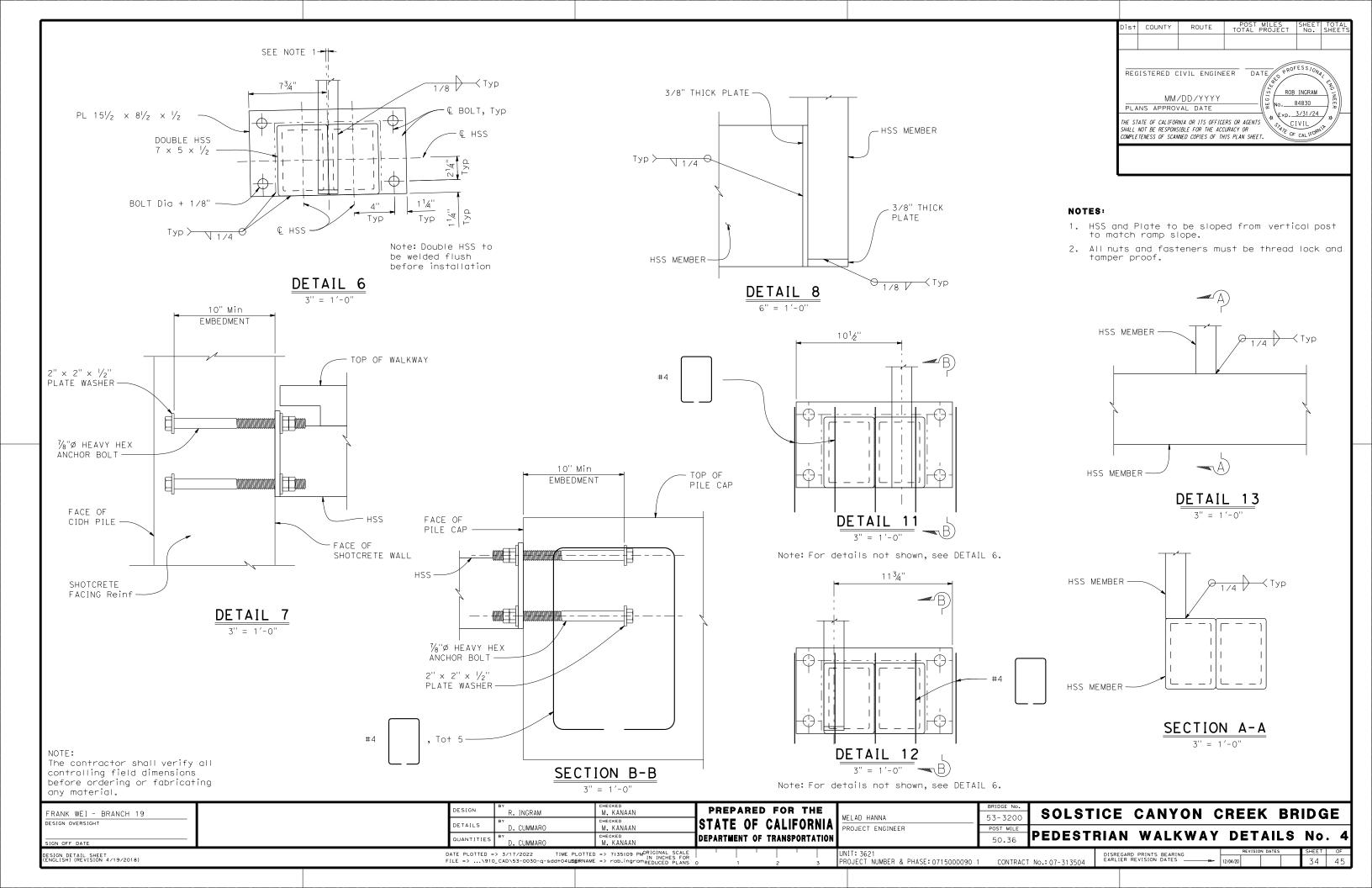
	Dist COUNTY ROUTE POST MILES SHEET TOTAL SHEETS
	REGISTERED CIVIL ENGINEER DATE
	MM/DD/YYYY  PLANS APPROVAL DATE  MM/DD/YYYY  PLANS APPROVAL DATE  MO. 84830  Exp. 3/31/24
	THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET.

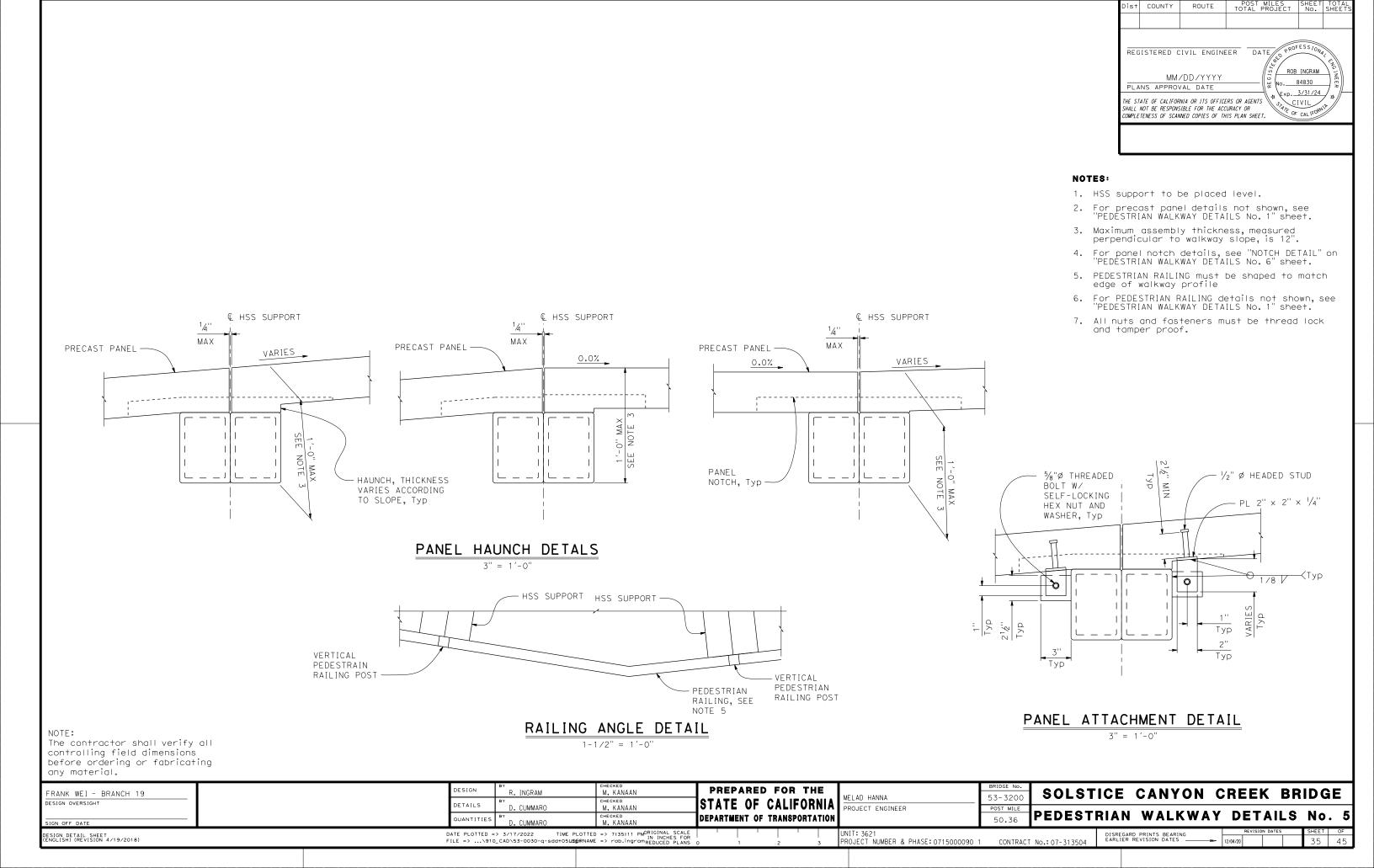
### NOTES

- For PEDESTRIAN RAILING details not shown, see "PEDESTRIAN WALKWAY DETAILS No. 1" sheet.
- For limits of cable railing, and retaining wall details, see "ROADWAY PLANS".

FRANK WEI - BRANCH 19	DESIGN	BY R.	INGRAM	CHECKED M. KANAAN			FOR TH		MEL AD LIANNIA	BRIDGE No.	SOLST	ICE	CANYON C	REEK BE	RIDGE	<u>:</u> ]
DESIGN OVERSIGHT	DETAILS	BY D.	CUMMARO	CHECKED M. KANAAN	STATI	E OF	CALIFOR	NIA	PROJECT ENGINEER	POST MILE						-
SIGN OFF DATE	QUANTITI	IES BY D.	CUMMARO	CHECKED M. KANAAN	DEPARTM	MENT OF	TRANSPORTA	ATION		50.36	PEDEST	RIAN	WALKWAY	DETAILS	No.	2
DESIGN DETAIL SHEET (ENGLISH) (REVISION 4/19/2018)	DATE PLOTTE  FILE =>	ED => 3/17.	7/2022 TIME PLOTTED	=> 7:35:06 PMORIGINAL SCALE IN INCHES FOR	1		3	7	UNIT: 3621	CONTRAC	「No ° 07-313504	DISREGAR EARLIER	RD PRINTS BEARING REVISION DATES	REVISION DATES	SHEET 32	)F 15



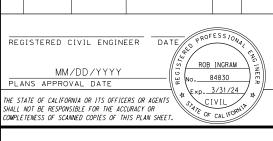


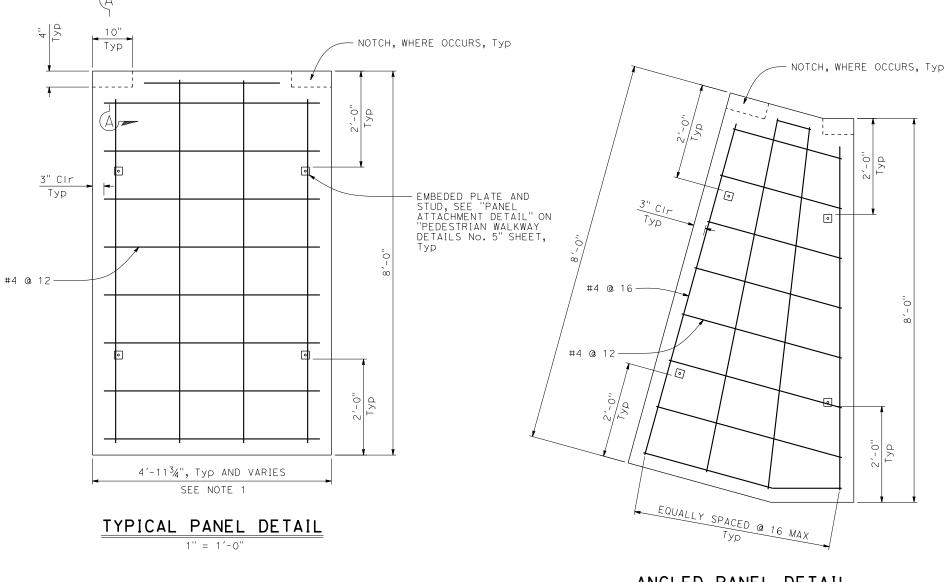


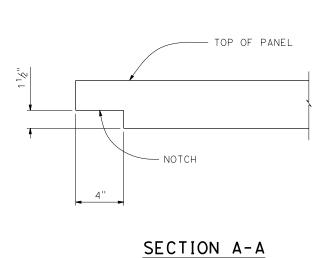
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# NOTES:

 Panel width varies with support spacing and must account for the required gap between panels.



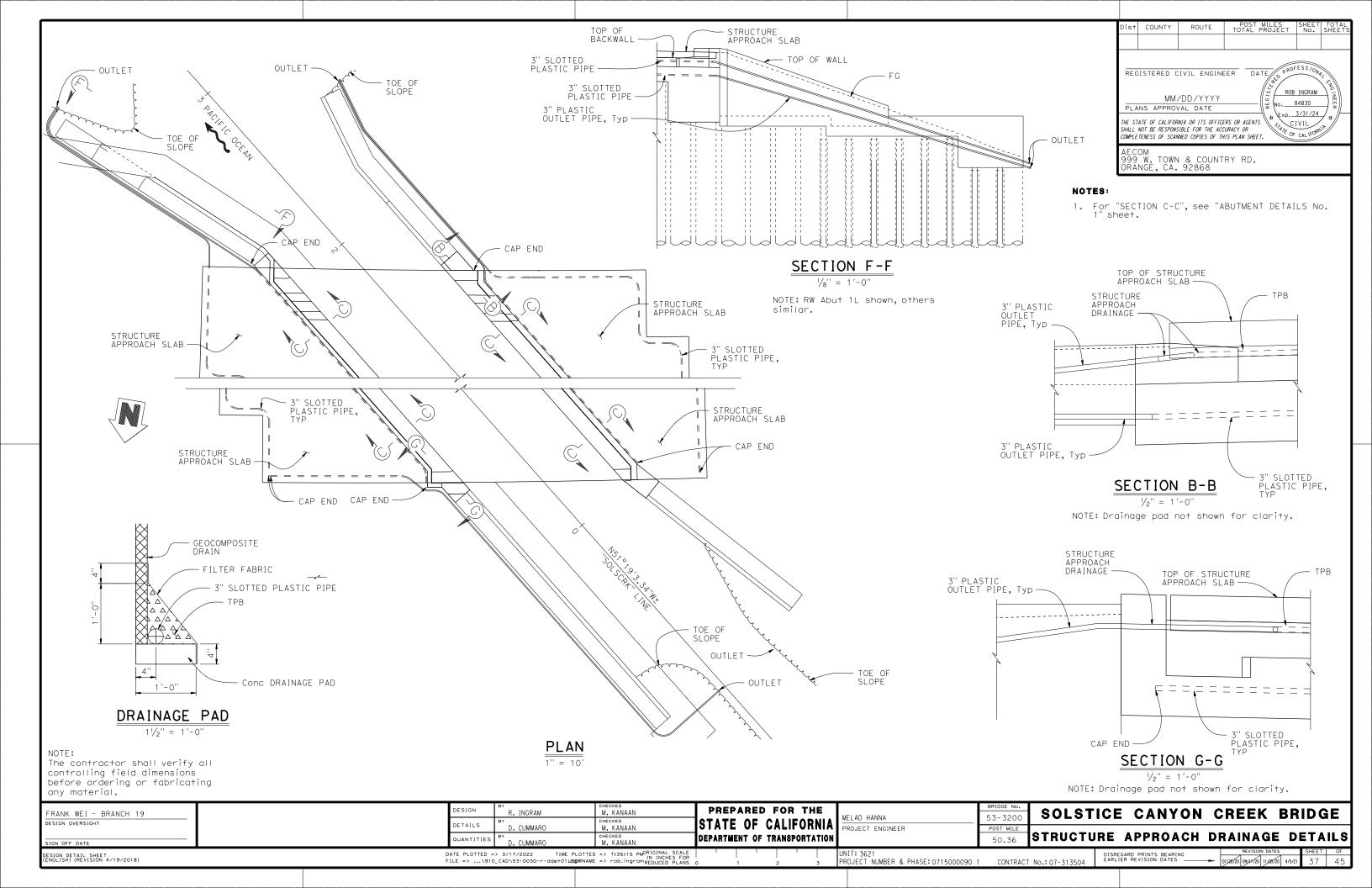


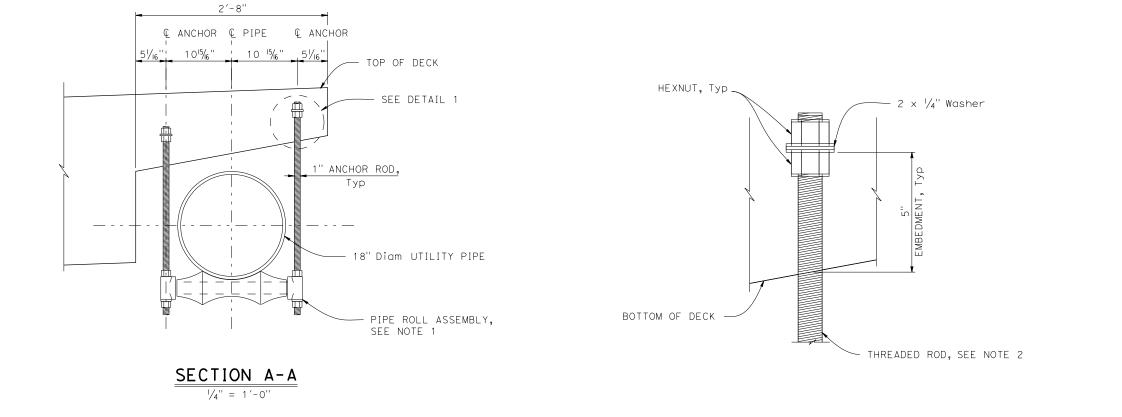


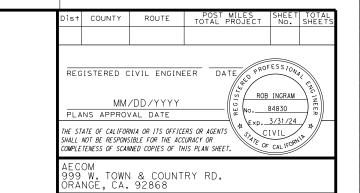
1/4" = 1'-0"

ANGLED PANEL DETAIL

FRANK WEI - BRANCH 19	DESIGN	R. INGRAM	M. KANAAN	PREPARED FOR THE		BRIDGE No.	SOLST	ICE CANYON C	CREEK	BRII	DGE
DESIGN OVERSIGHT				STATE OF CALLEDDINA	MELAD HANNA	53-3200	SULSI	ICE CANTON (	NEEK	DNII	<i>D</i> G E
DESIGN OVERSIGHT	DETAILS	D. CUMMARO	M. KANAAN	STATE OF CALIFORNIA	PROJECT ENGINEER	POST MILE					
SIGN OFF DATE	OUANTITI	ES D. CUMMARO	CHECKED M. KANAAN	DEPARTMENT OF TRANSPORTATION		50.36	PEDEST	RIAN WALKWAY	DETA	ILS I	NO. 6
DESIGN DETAIL SHEET	DATE PLOTTE	ED => 3/17/2022 TIME PLOTTE	D => 7:35:12 PMORIGINAL SCALE		UNIT: 3621	-		DISREGARD PRINTS BEARING	REVISION DAT	TES S	SHEET OF
DESIGN DETAIL SHEET (ENGLISH) (REVISION 4/19/2018)	FILE =>	\910_CAD\53-0030-q-sdd+06\tagranam	E => rob.ingromREDUCED PLANS	0 1 2 3	PROJECT NUMBER & PHASE: 0715000090 1	CONTRACT	No.: 07-313504	EARLIER REVISION DATES	12/04/20		36 45

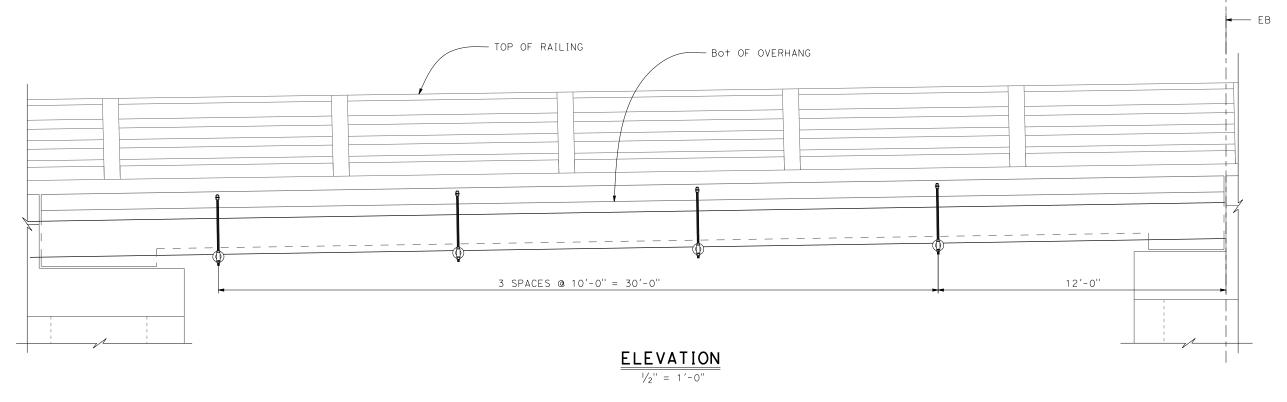






# NOTES:

- 1. Pipe Roll Assembly to be made by manufacturer. Must be fitted for 18" diameter utility pipe with a maximum outside diameter of 197%". Must be load rated for minimum 4,200 lbs.
- 2. Threaded Rod to be cast in place with concrete.
- For pipe location, profile, limits of payment including utility pipe support, and details not shown, see "ROADWAY PLANS".



FRANK WEI - BRANCH 19	DESIGN	R. INGRAM	M. KANAAN	PREPARED FOR THE	MELAD HANNA	BRIDGE No.	SOLST	ICE CANYON	CREEK BR	RIDGE
DESIGN OVERSIGHT	DETAILS QUANTITIES	D. CUMMARO	CHECKED	STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION		POST MILE	UT	LITY PIPE SUPF		
SIGN OFF DATE  DESIGN DETAIL SHEET (ENGLISH) (REVISION 4/19/2018)	DATE PLOTTED	D. CUMMARO => 3/17/2022 TIME	M. KANAAN  PLOTTED => 7:35:17 PMORIGINAL SCALE IN INCHES FOR USSFRNAME => rob.ingrammeDuceD PLANS (		UNIT: 3621 PROJECT NUMBER & PHASE: 0715000090 1	CONTRACT		DISREGARD PRINTS BEARING EARLIER REVISION DATES	REVISION DATES	SHEET OF

