Calculation of Base Flood Elevation

Kidder Creek

Approximately 2.5-mi above Br 02-052 SR003 PM 27.03

Kidder Creek Orchard Camp

Kidder Creek Orchard Camp Planned Development

2700 S Kidder Creek Rd, Etna, CA 96027

Hydrologic and Hydraulic Calculations

By: Chris Gaido, P.E.

February 20,2014

These hydrologic and hydraulic calculations and related information have been prepared under the direction of the following Professional Engineer. The Engineer attests, to the best of his knowledge, to the accuracy of the technical information contained herein and has judged appropriate the engineering data which recommendations, conclusions, and decisions are based.

Chris Gaido PE Civil Engineer Date

Summary

Kidder Creek Orchard Camp is currently planning an expansion to their existing facility. Kidder Creek Orchard Camp development plans are preliminary, details found under separate cover. A portion of the planned development includes construction of a natural recreational water feature within and near the potential floodplain of Kidder Creek. This report endeavors to establish a draft base flood elevation (BFE) for Kidder Creek for the impacted areas of the development plan.

Kidder Creek Orchard Camp has a physical address of 2700 South Kidder Creek Road, Etna CA. Located 6.25-mi northwest of the town of Etna, the area of interest along Kidder Creek is centered at approximately, 41.530873° N latitude, 122.95298741° W Longitude, NAD 1983. The estimated 100-yr Return Period (RP) baseflood elevation (BFE) for the approximate project area immediately upstream of the water feature is <u>3023.9-ft</u>. This elevation does not correlate with the Zone A delineation found on the January 19, 2011 FEMA Flood Insurance Rate Map, FIRM (Map Number 06093C200D). The upper limit of the Zone A found on this FIRM map appears to represent the study limits and not the actual floodplain for all of Kidder Creek (Exhibit B). There is no detailed floodplain study available for Kidder Creek.

The calculated BFE is considered a pre-project condition. The position and orientation of the final embankments for the water feature will have minimal impact to upstream BFE. The download stream impacts are limited to the potential streambed lateral movements caused by the new embankment.

The property owner and developers are advised to construct all living space dwellings and attached outbuildings at a minimum finished floor elevation of the estimated BFE plus 2-ft or as required by local building code/regulations. The actual BFE varies significantly with location along Kidder Creek. See subsequent section for detailed analysis. The estimated baseflood elevation does not consider or account for localized flooding caused by obstructions and backwater that may be present in the overbank areas of the river during flood flows.

Although survey data and sound engineering principles were employed in the calculation of the 100-yr BFE, the engineering field of hydrology and hydraulics of natural stream/river systems is an inexact science. The calculation of 100-yr BFE is only an estimate and should not be considered a detailed study of the hydrologic and hydraulic characteristics of Kidder Creek above project area. Other indeterminate factors do and will impact BFE. Debris accumulation, landsides, sediment load, bedload, channel lateral instabilities, and climatic changes all impact the transient 100-BFE of a river or creek. Therefore, even with the construction of the property above the 100-yr BFE the property owner should consider appropriate flood insurance to protect against unforeseen adverse changes to the creek's BFE.

Hydrology

The watershed of interest is Kidder Creek, above project area at 41.530873° N latitude, 122.95298741° W Longitude, NAD 1983. The watershed area, above this point, is approximately 24.866-mi² (15,914acres). The elevation ranges from 3010-ft at the centerline of the Kidder Creek downstream of the property to approximately 7,190-ft in the upper watershed. The upper watershed is rather complicated hydrologic system because of the volcanic soil, large winter/spring snowpack, irrigation diversions and convoluted natural drainage courses. The watershed has two predominant tributaries, a west-to-east draining sub-watershed and a south-to-north draining sub-watershed. A watershed map of the Kidder Creek above the project area is provided, (Exhibit A).

There are a few smaller lakes within the upper watershed but none of significant storage potential. The watershed is predominantly forest land with steep to very steep gradients. There is little development within the watershed. Portions of the upper watershed are most likely ineffective and contribute little to the overall discharge from the watershed (predominately contributing to groundwater). Nevertheless this area is considered in the calculation of Return Period discharges for watershed. Stream bed slope is quite steep in the immediate and upper watershed. At or near the lower limit of this study area the stream bed slope flattens significantly. The stream channel changes from an incised stable channel to an alluvial transient basin. The alluvial basin allows the stream to meander and change course, as is evident from aerial photography (Google maps). The upper watershed produces significant sediment load in the form of cobbles, boulders and coarse sands. This sediment load is deposited in the alluvial basin and hence causes the meandering nature of the stream in the lower watershed.

Annual precipitation for watershed was obtained from nearby gages and synthetic climatological mapping. The gage data period of record is 1955 – present. The average annual precipitation for Etna California is approximately 18-in. The upper watershed experiences significantly more precipitation. The predominance of precipitation occurs during the winter months of October – March, with January and February being the historic period of highest monthly snowfall and rainfall totals. The 100-yr Return Period, 24-hr precipitation estimate is 8.00-in. No detailed historic precipitation data gathering was required. The methods used to estimate runoff were not contingent on detailed analysis of climatological factors.

Estimated RP discharge for the watershed Kidder Creek was calculated by two methods, (USGS Regional Method, and the Log Pearson III statistical analysis of stream gage data. Both methods are common to hydrologic calculations for Northern California and are well documented in the appropriate technical reference(s). Detailed data results provided in Exhibit C. Summary table below:

Method	Q 100-yr (cfs)	Q 50-yr (cfs)	Q 25-yr (cfs)	Q 10-yr (cfs)	Note
Regional Method (S)	2830	2270	1680	1180	North Coast Regional Method Equation (reasonable approximation)
LPIII – s11518050	4500	3600	2700	1700	USGS gaging station s11518050 Scott River with basin comparison method employed. (1960-74) short period of record
LPIII – s11516900	1850	1300	880	500	USGS gaging station s11516900 Little Shasta R, NR Montague (1957-77) short record
Project	3050	2400	1750	1200	Rounded average of all values

Table 1 Summary of Return Period Discharges for Kidder Creek Above Project Area (24.866-mi²)

Hydraulics

Survey data, cross sections and proposed development limits taken from photogrammetry mapping provided by Kidder Creek Orchard Camp management. Alignment and cross section locations found on development planning map, (Exhibit D).

Cross sectional data incorporated into a one-dimensional, steady-state hydraulic model (HEC-RAS 4.01). Overbank and flowline limits from survey and geo-referenced aerial photographic image. Manning N-values from field investigation and comparative review of similar studies. Manning N-values ranged from 0.045 to 0.070.

Hydraulic modeling boundary conditions used the critical depth calculation for all RP flows. Detailed cross sectional data and result tabular data found in Exhibit D.

Hydraulics Discussion

The controlling water surface elevation for the 100-yr Return Period discharge is considered the upstream cross section above station "k" line STA 36+00 (XS – 4800). Review of the tabular data in Exhibit D finds the calculated water surface elevation (BFE) for the 100-yr event as 2023.9-ft. The

average channel velocity for the 100-yr event is near 9-fps. The in-channel velocity is significantly higher and has the ability to move large cobbles and boulders (200-lbs or more) as bedload down the stream channel.

All return period discharges produced flow regimes at or near super critical flow. Froude numbers range for 0.6 to 1.0, though field observations suggest transient supercritical flow conditions and unstable flow regime expected throughout the river reach. Large amounts of bedload are expected during medium to large discharge events. The super critical flow analysis was not endeavored and all resultant cross sections with high Froude numbers were limited to their critical depth calculation. This result giving the conservative answer for BFE calculations.

The transient lateral condition of downstream Kidder Creek is disconcerting. The landuse patterns in this area and the continued encroachment into the floodplain were not considered. The meandering nature of the downstream channel and its inevitable impact to BFE are difficult to analyze in preliminary study. Further, downstream encroachments and landuse changes will ultimately impact BFE in the general area. There is no cost effective method at present to estimate the streambed and channel movements with respect to time, landuse and future hydrologic events.

EXHIBIT A

USGS WATERSHED MAP

Kidder Creek Orchard Camp Planned Development



EXHIBIT B

FEMA FLOOD INSURANCE RATE MAP FIRM MAP NUMBER 06093C200D JAN 19, 2011

Kidder Creek Orchard Camp Planned Development



To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FISM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table in management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolate between cross sections. The floodways were based on hydraulic consideration with regard to requirements of the National Flood Insurance Program. Floodwe vidths and other pertinent floodway data are provided in the Flood Insuranc Study report for this jurisdiction. Study

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 10. The **horizontal datum** was NAD 83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

conversion שבושכים North American Ver website at <u>http://ww</u> the following address:

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242, or visit its website at

Road centerline information shown on this FIRM was provided in digital format by the Siskiyou County Department of Public Works. These data were developed in 2001 using vehicle mounted sub-meter GPS equipment. Railroad centerlines were derived from 2006 Second Edition TIGER/Line files published by the U.S. Census Bureau.

This map reflects more detailed and up-to-date **stream channel configuration**: than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to confirm to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances tha differ from what is shown on this map.

For infor Service include Report, a

If you have **questions about this map**, how to order products or the National Flood Insurance Program in general, please call the FEMA Map Information eXchange (FMIX) at 1-877-FEMA-MAP (1-877-336-2627) or visit the FEMA website at <u>http://www.fema.gov/business/nfip</u>.

EXHIBIT C

HYDROLOGY

REGIONAL METHOD AND USGS STREAM GAGE DATA LPIII STATISTICAL ANALYSIS

Kidder Creek Orchard Camp Planned Development

Hydrologic Estimate REGIONAL METHOD North Coast

INPUT

 NAME:
 Kidder Creek Camp

 DATE:
 1/30/2014

 CREEK/RIVER NAME:
 Kidder Creek

 QUAD MAP NAME:
 Etna

 LOCATION:
 2700 S Kidder Creek Rd, Etna, CA

A = Drainage area in square miles

P = Mean annual precipitation in inches

H = Altitude index in thousands of feet (min. value is 1)

A =	24.87 sq. mi.
P =	20.50 inches
H =	4.2000

OUTPUT

⁸⁷ = 270 cfs	$\mathbf{Q}_2 = 3.52 * A^{0.90} * P^{0.89} * H^{-0.87} =$
³⁵ = 830 cfs	$\mathbf{Q}_{5} = 5.04 * A^{0.89} * P^{0.91} * H^{-0.35} =$
²⁷ = 1180 cfs	$\mathbf{Q}_{10} = 6.21 * A^{0.88} * P^{0.93} * H^{-0.27} =$
¹⁷ = 1680 cfs	$\mathbf{Q}_{25} = 7.64 * A^{0.87} * P^{0.94} * H^{-0.17} =$
⁰⁸ = 2270 cfs	$\mathbf{Q_{50}} = 8.57 * A^{0.87} * P^{0.96} * H^{-0.08} =$
^{.97} = 2830 cfs	$\mathbf{Q}_{100} = 9.23 * A^{0.87} * P^{0.97} =$

Summary Table LPIII and Log Normal Distribution of Stream flow data Basin Comparison Method for ungaged watersheds

Gaged basin name	EF SCOTT R CALAHAN CA	۱.	Yrs of Re	ecord	15
Latitude gaged basin	41.3	Decimal Degrees	Begin Re	ecord Feb-19	960
Longitude gaged basin	122.7666	Decimal Degrees	End Re	ecord Jan-19	974
					- 00
Ungaged basin name	Kidder Creek Above Camp		Min Q reco	orded 825	0.00
Latitude ungaged basin	41.53102	Decimal Degrees	Max Q reco	orded 12500).00
Longitude ungaged basin	122.95294	Decimal Degrees	Q _{MPD} ^A	539	962
Distance Between Gages	18.8	miles			
Bearing from Gaged to Ungaged Basin	NorthWest				
Gaged basin Area	110.000	mi ²			
Ungaged Basin Area	24.866	mi ²			
Hydrologic Region	North Coast	Select Hydrologic R	egion from pulldown menu (See HDM pg 810-19)		

			EF SCOTT R CALAHAN CA		Kidder Creek Above Camp		
Return Period (yr)	Gaged Log-Normal Q (cfs)	Ungaged Log-Normal Q (cfs)	Gaged LP-3 Q (cfs)	Gaged Unit Q (cfs/mi ²)	Ungaged LP-3 Q (cfs)	Ungaged Unit Q (cfs/mi ²)	
2	2095.58	549.66	2040.45	18.5	535.20	21.5	
2.5	2606.64	685.41	2541.31	23.1	668.23	26.9	
3	3292.14	869.24	3226.56	29.3	851.93	34.3	
5	4326.61	1151.86	4288.18	39.0	1141.62	45.9	
10	6320.08	1707.77	6420.04	58.4	1734.78	69.8	
25	9467.26	2596.51	9989.31	90.8	2739.69	110.2	
50	12291.27	3371.03	13377.99	121.6	3669.07	147.6	
100	15544.42	4263.24	17474.29	158.9	4792.53	192.7	
200	19270.94	5285.29	22395.46	203.6	6142.22	247.0	
500	25003.59	6857.53	30394.71	276.3	8336.11	335.2	
1000	30014.85	8231.93	37773.37	343.4	10359.80	416.6	

A. Q _{MPD} = Maximum Peak Discharge (cfs). Estimate as per Creager enveloping curve. From reference 2 below. pg. 9

References:

1. Regional Flood Frequency Equation. HDM May 2001, pg. 810-19

2. Magnitude and Frequency of Floods in California, USGS Water-Resources Investigation 77-21 1977 pg 4-6

3. Guidelines for Determing Flood Flow Frequency, Bulletin No. 17B, USGS, office of Water Data Coordination, 1982

4. Design Hydrology and Sedimentology for Small Catchments, Hydrologic Frequency Analysis, pg 8-18, 1994

Summary Table LPIII and Log Normal Distribution of Stream flow data Basin Comparison Method for ungaged watersheds

Gaged basin name	LITTLE SHASTA R NR MO	NTAGUE CA	Yrs of Record	21
Latitude gaged basin	41.75305	Decimal Degrees	Begin Record	Nov-1957
Longitude gaged basin	122.032778	Decimal Degrees	End Record	Dec-1977
Ungaged basin name	Kidder Creek Above Camp		Min Q recorded	41.00
Latitude ungaged basin	41.53102	Decimal Degrees	Max Q recorded	5910.00
Longitude ungaged basin	122.95294	Decimal Degrees	Q _{MPD} ^A	53962
Distance Between Gages	51.1	miles		
Bearing from Gaged to Ungaged Basin	SouthWest			
Gaged basin Area	48.200	mi ²		
Ungaged Basin Area	24.866	mi ²		
Hydrologic Region	North Coast	Select Hydrologic R	egion from pulldown menu (See HDM pg 810-19)	

			LITTLE SHASTA R NR	MONTAGUE CA	Kidder Creek Above Camp		
Return Period (yr)	Gaged Log-Normal Q (cfs)	Ungaged Log-Normal Q (cfs)	Gaged LP-3 Q (cfs)	Gaged Unit Q (cfs/mi ²)	Ungaged LP-3 Q (cfs)	Ungaged Unit Q (cfs/mi ²)	
2	213.92	117.91	202.01	4.2	111.35	4.5	
2.5	280.77	154.93	265.75	5.5	146.64	5.9	
3	375.58	207.63	359.32	7.5	198.64	8.0	
5	527.93	292.93	517.06	10.7	286.89	11.5	
10	846.56	472.83	873.64	18.1	487.96	19.6	
25	1400.72	787.55	1568.16	32.5	881.69	35.5	
50	1939.21	1090.31	2321.23	48.2	1305.10	52.5	
100	2598.36	1460.92	3335.27	69.2	1875.24	75.4	
200	3396.23	1909.51	4685.06	97.2	2634.16	105.9	
500	4698.19	2641.54	7146.82	148.3	4018.27	161.6	
1000	5899.14	3316.76	9677.25	200.8	5440.99	218.8	

A. Q MPD = Maximum Peak Discharge (cfs). Estimate as per Creager enveloping curve. From reference 2 below. pg. 9

References:

1. Regional Flood Frequency Equation. HDM May 2001, pg. 810-19

2. Magnitude and Frequency of Floods in California, USGS Water-Resources Investigation 77-21 1977 pg 4-6

3. Guidelines for Determing Flood Flow Frequency, Bulletin No. 17B, USGS, office of Water Data Coordination, 1982

4. Design Hydrology and Sedimentology for Small Catchments, Hydrologic Frequency Analysis, pg 8-18, 1994

EXHIBIT D

MAPPING AND STREAM ALIGNMENT

Kidder Creek Orchard Camp Planned Development







EXHIBIT E

PRELIMINARY HYDRAULIC MODELING RESULTS

Kidder Creek Orchard Camp Planned Development

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
	7250	100yr RP	3050.00	3059.50	3062.89	3062.89	3064.24	0.021716	9.33	326.92	122.43	1.01
	7250	50vr RP	2400.00	3059,50	3062,45	3062,45	3063.64	0.022738	8,75	274.43	117.27	1.01
	7250	25vr RP	1750.00	3059.50	3061.97	3061.97	3062.96	0.024092	8.00	218.65	111.53	1.01
	7250	10vr RP	1200.00	3059.50	3061.49	3061.49	3062.29	0.025826	7.17	167.25	105.97	1.01
		1.000.00										
	7000	100vr BP	3050.00	3050.25	3057.62	3056 17	3058 28	0.006640	6.55	465 64	121 68	0.59
	7000	50vr RP	2400.00	3050.25	3056 95	3055.65	3057 54	0.006755	6.19	387 94	111 90	0.59
	7000	25vr PD	1750.00	3050.25	3056 18	3055.03	3056 69	0.006876	5.72	305.86	100.54	0.58
	7000	10ur PD	1200.00	3050.25	3055 30	3054 35	3055.81	0.006000	5.12	231.06	88.94	0.50
	7000		1200.00	3030.25	3033.39	3034.33	3055.61	0.000990	5.19	231.00	00.34	0.57
e el publicario de la composición de la El composición de la c	6700	100m PP	3050.00	3049 50	3054 43	3053 88	3055 54	0.012846	8.46	360 72	104 54	0.80
1	6700	50ur PP	2400.00	3049.50	2052.05	3053.88	3053.54	0.012040	7.60	312 10	09.03	0.30
	6700		2400.00	3049.50	3053.95	3053.34	3054.67	0.011940	7.09	057.00	90.93	0.70
	0700	20yr RP	1750.00	3049.50	3053.39	3052.72	3054.10	0.010923	6.79	257.09	92.24	0.72
	6700	TUYFRP	1200.00	3049.50	3062.79	3052.12	3053.32	0.009930	0.86	204.88	83.21	0.67
	0500			0010.00	0050.00		0050.04	0.000.000	0.50	004 70		0.00
	6500	100yr RP	3050.00	3046.80	3050.89	3050.89	3052.31	0.020428	9.59	321.79	115.77	0.99
1	6500	SUYL RP	2400.00	3046.80	3050.43	3050.43	3051.68	0.021914	8.98	269.25	111.98	1.00
	6500	25yr RP	1750.00	3046.80	3049.93	3049.93	3050.97	0.023657	8.19	214.02	107.08	1.01
	6500	10yr RP	1200.00	3046.80	3049.42	3049.42	3050.27	0.025531	7.40	162.24	97.55	1.01
2	1											
	6100	100yr RP	3050.00	3039.50	3044.35	3043.32	3044.87	0.007324	5.77	528.73	185.62	0.59
1	6100	50yr RP	2400.00	3039.50	3043.99	3042.91	3044.41	0.006701	5.18	463.50	174.45	0,56
1	6100	25yr RP	1750.00	3039.50	3043.52	3042.42	3043.84	0.005910	4.55	384.57	159.89	0.52
l	6100	10yr RP	1200.00	3039.50	3043.03	3041.93	3043.26	0.005017	3.88	309.47	144.68	0.47
-	-											
	5900	100yr RP	3050.00	3039.50	3041.54	3041.54	3042.37	0.025225	7.32	416.63	252.33	1.00
	5900	50yr RP	2400.00	3039.50	3041.28	3041.28	3042.00	0.026615	6.82	351.87	246.73	1.01
	5900	25yr RP	1750.00	3039.50	3040.99	3040.99	3041.59	0.028306	6.19	282.92	240.62	1.01
	5900	10yr RP	1200.00	3039.50	3040.72	3040.72	3041.19	0.031013	5.52	217.30	234.66	1.01
	5650	100yr RP	3050.00	3032.00	3036.26	3035.37	3036.80	0.007671	5.90	522.31	195.84	0.61
	5650	50yr RP	2400.00	3032.00	3035.81	3034.98	3036.28	0.008227	5.50	437.03	180.38	0.61
	5650	25yr RP	1750.00	3032.00	3035.28	3034.53	3035.68	0.008511	5.04	347.04	162.48	0.61
	5650	10yr RP	1200.00	3032.00	3034.80	3034.07	3035.10	0.007733	4.40	272.94	146.11	0.57
1	5250	100yr RP	3050.00	3029.50	3031.38	3031.36	3032.02	0.021065	6.67	527.49	430.87	0.92
	5250	50yr RP	2400.00	3029.50	3031.28	3031.14	3031.74	0.016605	5.67	483.19	419.05	0.81
	5250	25yr RP	1750.00	3029.50	3031.11	3030.90	3031.44	0.013422	4.72	415.09	400.21	0.71
	5250	10yr RP	1200.00	3029.50	3030.86	3030.63	3031.11	0.013255	4.09	317.12	371.44	0.68
	1								- Forester			
1	5000	100yr RP	3050.00	3024.00	3026.95	3026.72	3027.61	0.014991	6.63	493.32	310.57	0.81
1	5000	50vr RP	2400.00	3024.00	3026,53	3026,42	3027,20	0.019781	6,62	373.01	262.77	0.89
1	5000	25vr RP	1750.00	3024.00	3026.09	3026.09	3026 75	0.027716	6.54	268.21	213.43	1.00
1	5000	10vr RP	1200.00	3024.00	3025 77	3025 77	3026.31	0.029944	5.88	204 15	195.61	1 01
	0000		1200.00	0021.00	0020.17	0020.77	0020.01	0.020011	0.00		100.01	
1	4800	100vr PD	3050.00	3010 50	3023 00	3023 60	3024 04	0.011020	8 08	450.76	180 27	0.70
	4800	FOr PD	2400.00	2010.50	2022.50	2022.00	2024.34	0.010557	7.06	201.07	168.01	0.74
	4000	DOYI RF	1750.00	3019.50	2022.00	2022.13	2024.35	0.010557	6 79	224 70	152.26	0.74
1	4000	20yr RP	1750.00	3019.30	3023.13	3022.00	3023.77	0.000905	0.70 E.Ct	324.15	103.20	0.00
	4800	TUYF RP	1200.00	3019.50	3022.70	3022.03	3023.14	0.007470	5.01	259.40	137.17	0,59
	4000	100 - 00	2050.00	2010.00	2004 02	2004 40	2004 70	0.040070	0.04	FET OF	200.00	0.00
	4600	TUUYF RP	3050,00	3019.00	3021.22	3021.10	3021.79	0.019672	0.01	007.35	389.92	0.89
	4600	SUYF RP	2400.00	3019.00	3021.02	3020.90	3021.49	0.019330	6.02	4/9.30	304.13	0.87
	4600	25yr RP	1750.00	3019.00	3020.78	3020.69	3021.17	0.019058	5.36	390.89	3/7.45	0.84
1	4600	10yr RP	1200.00	3019.00	3020.56	3020.46	3020.86	0.018539	4.65	306.38	370.96	0.80
1	4200	100yr RP	3050.00	3010.00	3012.14	3012.14	3013.11	0.023934	7.92	385.18	199.92	1.00
1	4200	50yr RP	2400.00	3010.00	3011.84	3011.84	3012.68	0.025200	7.38	325.04	194.23	1.01
1	4200	25yr RP	1750.00	3010.00	3011.50	3011.50	3012.20	0.026655	6.71	260.98	187.98	1.00
	4200	10yr RP	1200.00	3010.00	3011.18	3011.18	3011.73	0.028720	5.97	200.84	181.91	1.00







1 in Horiz. = 100 ft 1 in Vert. = 40 ft





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