APPENDIX N

Sewer Study (March 31, 2021, as Revised June 23, 2021)

SEWER STUDY FOR EL CAMINO REAL ASSISTED LIVING FACILITY

PTS No. 675732

Prepared By: LEPPERT ENGINEERING CORPORATION 5190 Governor Drive, Suite 205 San Diego, CA 92122 Job No. SDR 05.01-09.16

> Prepared: March 31, 2021 Revised: June 23, 2021

John D Leppert

RCE 26283 Exp. 3-31-22



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Introduction

The purpose of this sewer study is to evaluate the sewer demands generated by the proposed development for the purposes of validating the design criteria assumed for the subject property in the construction of the private onsite sewer. The existing private sewer was constructed in conjunction with the approved Church project to the north of the subject site.

Project Location

The subject property is a rectangular parcel of land with a gross area of approximately 4 acres. It is located south and east of El Camino Real and west of San Dieguito Rd (see Exhibit "A" - Location Map).

Project Description

The project proposes one building to provide assisted living and memory care, as well as the associated support services. The project to the north is also being evaluated with this study and proposes four buildings for religious gatherings and their supporting services.

-Building 1:

Proposed 1-level church sanctuary building.

-Building 2:

Proposed 2-level events 'hall' for hosting religious events and functions.

-Building 3: Proposed 2-level cultural center and history archive.

-Building 4:

Proposed 1-level gym facility for sports and youth events.

-ECR Assisted Living Facility:

Proposed 3-level assisted and memory care facility providing onsite care and support for residents.

Onsite sewer flows are collected by a private PVC sewer which discharges to the existing public 8" PVC sewer main within El Camino Real. This public sewer was installed by 28175-D, and the private 6" PVC lateral connection to that line was completed by 41200-D.

Project Sewer Demand

The sewer flow generated by the proposed development site was estimated by determining the total number of fixture units for each building within the development, based on the designs by the building MEPs (See Appendix II). A total fixture unit count of 1,286 was estimated for the combined flows from both project sites. A conversion factor of 20 fixture units per EDU was then applied to the estimated total, resulting in approximately 64.3 EDUs for the proposed project.

Design Summary

The existing and proposed onsite sewer has been designed with a minimum 1% slope and a 2.0 Ft/s cleansing velocity. The following calculation shows the proposed sewer flow anticipating **840 DFU** or **42 EDU** for the proposed development:

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Equivalent Dwelling Units = 42 EDU Sewer Generation Rate = 280 gal/EDU, (See Appendix III) Peak Factor = 4.0, (See Appendix III) Peak Flow = 47,040 GPD = <u>0.07 cfs</u>, (See Appendix III)

The combined sewer flow from the northerly church property and the proposed project totals **0.11 cfs**. The Sewer Summary Tables included as Exhibit "B" illustrates the individual flow rates from node to node, all of which which meet the City's Sewer Design criteria.

Conclusion

The proposed development of the 4 acres site does not exceed the expected sewer flows estimated during the construction of the sewer for the Church parcel on the North, and will not have an adverse impact on those facilities. At full project completion the private on site sewer will be operating at less than half of its maximum, and therefor it is not anticipated that there

EXHIBIT "A" Location Map

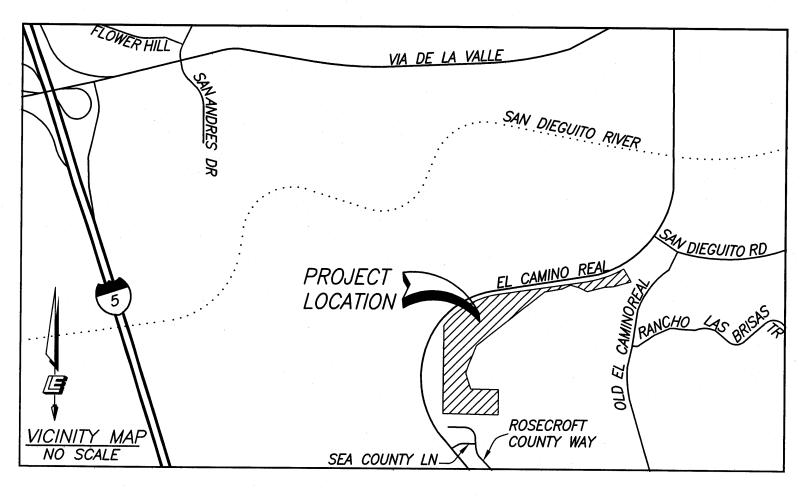


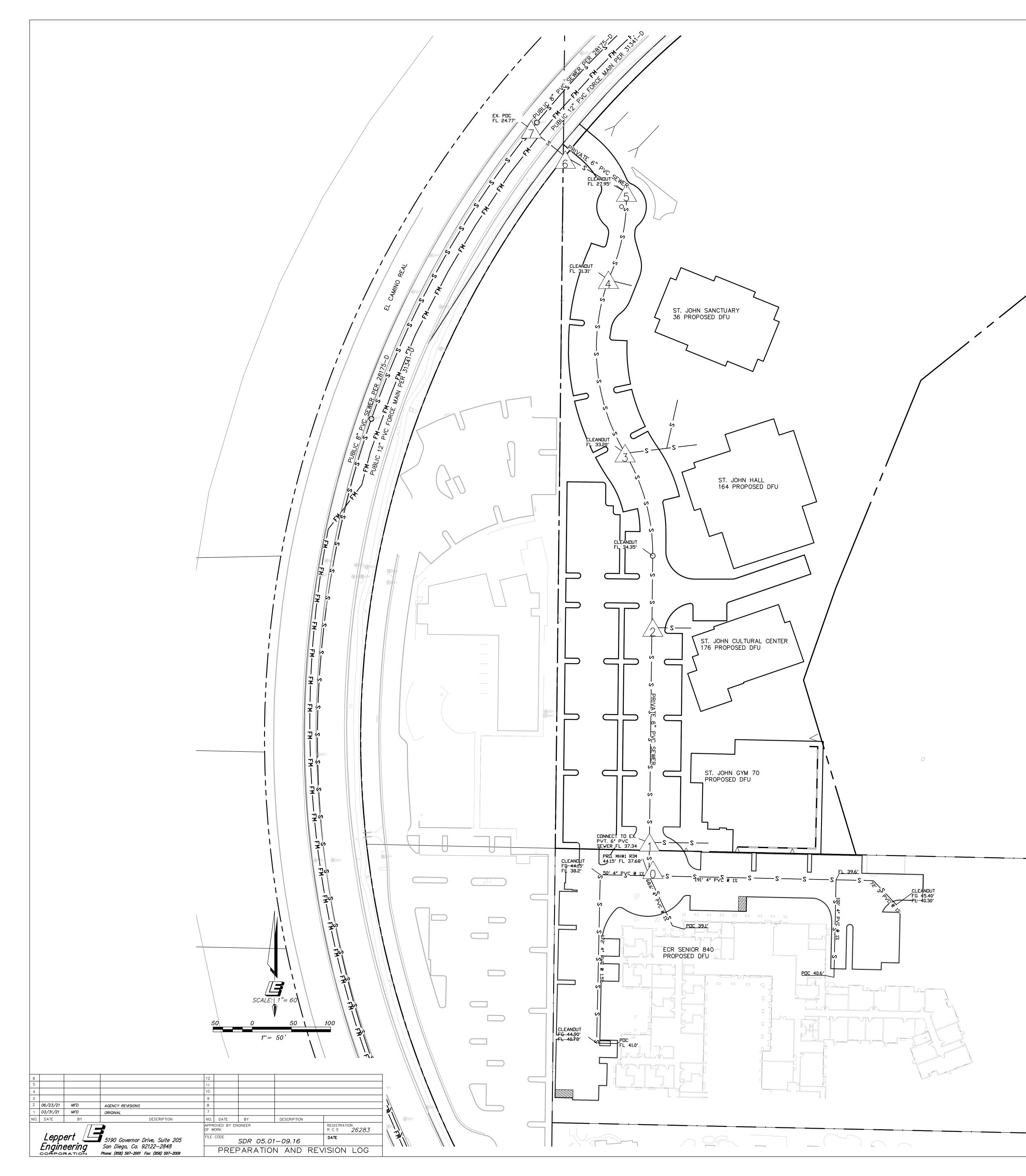
EXHIBIT "B" Sewer Summary Tables EL CAMINO REAL ASSISTED LIVING FACILITY SEWER STUDY -

LINE SUMMARY TABLE 1

ADDITIONAL	FROM	ТО	IN-LINE	IN-LINE	CUMLATIVE	SEWAGE	DRY	PEAK	PEAK	LINE	DESIGN	DEPTH (d)	d/D	VELOCIT
FLOW FROM	NODE	NODE	UNITS	POP.	TOTAL	PER DAY	PEAK	DRY WEATHER	DRY WEATHER	DIAM	SLOPE	(FT)		(FPS)
			DFU	EDU	POP	(GAL)	FACTOR	FLOW (GPD)	FLOW (CFS)	(IN)	%			
ECR Senior	0	1	840	42.0	42.0	11,760	4.00	47,040	0.07	6	1.00	0.12	0.24	2.0
Gym	1	2	70	3.5	45.5	12,740	4.00	50,960	0.08	6	1.00	0.13	0.26	2.0
Cultural	2	3	176	8.8	54.3	15,204	4.00	60,816	0.09	6	1.00	0.14	0.27	2.1
Hall	3	4	164	8.2	62.5	17,500	4.00	70,000	0.11	6	1.00	0.15	0.30	2.2
Sanctuary	4	5	36	1.8	64.3	18,004	4.00	72,016	0.11	6	3.60	0.11	0.22	3.5
	5	6	0	0.0	64.3	18,004	4.00	72,016	0.11	6	1.70	0.13	0.26	2.7
	6	7	0	0.0	64.3	18,004	4.00	72,016	0.11	6	2.00	0.13	0.25	2.8



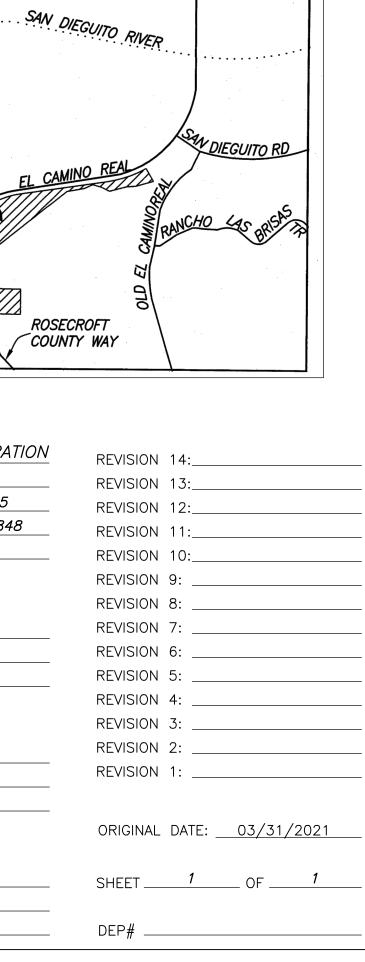
EXHIBIT "C" Sewer Site Plan



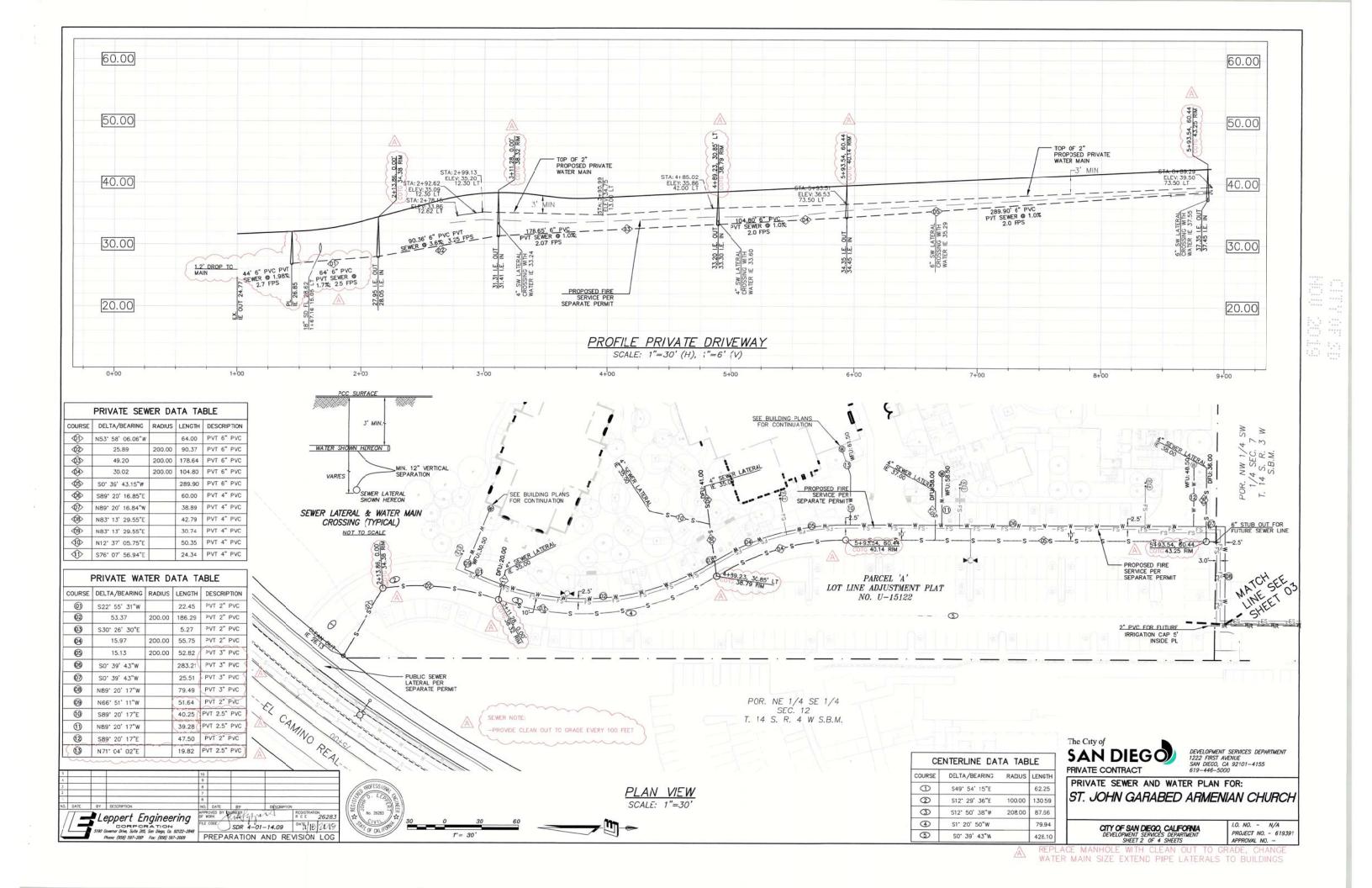
FLOWERH	IIL VIA DE LA VALLE
A C	SANANDRES DR
5	PROJECT LOCATION
VICINITY MAP NO SCALE	
PRE	PARED BY: <i>LEPPERT ENGINEERING CORPORATIO</i>
NAM	DRESS: 5190 GOVERNOR DRIVE, SUITE 205 SAN DIEGO, CALIFORNIA 92122–2848
PHC	DNE #: (858) 597-2001
	DJECT ADDRESS: <i>3860 El camino real</i>
<u>_</u>	AN DIEGO, CALIFORNIA 92130
	DJECT NAME: <i>E camino real senior</i>
	ET TITLE: Sewer study exhibit

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APPENDIX I Approved plumbing plan for site sewer



APPENDIX II Fixture Unit Calculations

Matthew DeVincenzo

From:James Christie <JPChristie@cox.net>Sent:Monday, March 18, 2019 4:35 PMTo:Matthew DeVincenzoSubject:RE: Garabad

I have come up with; Hall 163.75 F.U., 85 gpm Church 35.5 F.U., 22 GPM Cultural 176 F.U., 86 GPM Youth 70 F.U., 58 GPM

Total F.U. 445.25 or 133 GPM

A main line size according to the 2016 appendix A Tables is 3" on a 2" meter.

Call me if you'd like to discuss this.

Jim Christie (760) 525-6701 JpChristie@cox.net

From: Alex Moore [mailto:AMoore@kdsarch.com] Sent: Monday, March 18, 2019 2:26 PM To: James Christie Subject: FW: Garabad Hall

From: Ken Smith Sent: Monday, March 18, 2019 2:26 PM To: Alex Moore Subject: RE: Garabad Hall

Flush valves



From: Alex Moore Sent: Monday, March 18, 2019 12:51 PM To: Ken Smith Subject: FW: Garabad Hall

Any ideas?

Matthew DeVincenzo

То:	Rosales, Roberto
Cc:	Berg, Jennifer
Subject:	RE: El Camino Real - Water Quality

From: Rosales, Roberto <RRosales@glumac.com>
Sent: Monday, March 29, 2021 6:12 PM
To: Matthew DeVincenzo <matt@leppertengineering.com>
Cc: Berg, Jennifer <jberg@glumac.com>
Subject: RE: El Camino Real - Water Quality

Hi Matt,

A quick calc on my end shows 840 DFU for the units (estimated about 8 DFU per unit at 105 total unit count. Please let me know if you'd like an in depth calculation.

Sorry I didn't have an easy answer for you, but I am happy to help and lend ideas for possible solutions.

Roberto Rosales

APPENDIX III Sewer Design Reference Documents

Zone	Maximum Density (DU / Net Ac)	Population Per DU	Equivalent Population (Pop/Net Ac)
Schools/Public	8.9	3.5	31.2
Offices	10.9	3.5	38.2*
Commercial/Hotels	12.5	3.5	43.7*
Industrial	17.9	3.5	62.5*
Hospital	42.9	3.5	150.0*

TABLE 1-1 CITY OF SAN DIEGO SEWER DESIGN GUIDE DENSITY CONVERSIONS (Continued)

Figures with asterisk (*) represent equivalent population per floor of the building.

Definitions:

DU = Dwelling UnitsAc = Acreage Pop = Population

Net Acreage is the developable lot area excluding areas that are dedicated as public streets in acres. Gross Area is the entire area in acres of the drainage basin, including lots, streets, etc.

For undeveloped areas, assume Net Acreage = $0.8 \times \text{Gross}$ Area in Acres

For developed areas, calculate actual Net Acreage.

Tabulated figures are for general case. <u>The tabulated figures shall not be used if more accurate figures are available.</u>

Population is based on actual equivalent dwelling units (EDU) or the maximum estimate obtained from zoning.

Conversion of Fixture Units to Equivalent Dwelling Units (EDU): The Water Meter Data Card, maintained by the Development Services Department, contains a table of plumbing fixtures that should be used for determining the equivalent dwelling units (EDU's) for the purpose of estimating the rate of wastewater generation in residential, commercial, or industrial areas. Currently, the basis for conversion is: 20 fixtures = 1 EDU and 1 EDU = 280 gallons of wastewater per day.

In high rise building areas, flow rates shall be based on the most current, adopted edition of the applicable Plumbing Code, assuming one lateral per area. The most conservative flow rate shall govern.

PUBLIC UTILITIES DEPARTMENT

PEAKING FACTOR FOR SEWER FLOWS (Dry Weather)

Ratio of Peak to Average Flow* <u>Versus Tributary Population</u>

	Ratio of Peak to		<u>Ratio of Peak to</u>
Population	Average Flow	Population	Average Flow
200	4.00	4 900	2.01
<mark>200</mark> 500	<mark>4.0</mark> 0 3.00	4,800 5,000	2.01 2.00
			2.00 1.99
800 900	2.75 2.60	5,200 5,500	1.99
900 1,000	2.50	5,500 6,000	1.97
	2.50	6,200	1.95
1,100	2.47 2.45		1.94
1,200 1,300	2.45	6,400 6,900	1.95
	2.45		1.91
1,400 1,500	2.40	7,300 7,500	1.90
1,600	2.38	8,100	1.89
1,700	2.30	8,400	1.86
	2.34		1.84
1,750 1,800	2.33	9,100 9,600	1.83
	2.32		1.82
1,850		10,000	
1,900	2.30 2.29	11,500	1.80 1.78
2,000		13,000	
2,150 2,225	2.27 2.25	14,500 15,000	1.76 1.75
	2.25	16,000	1.75
2,300	2.24		1.74
2,375 2,425	2.25 2.22	16,700 17,400	1.73
	2.22		1.72
2,500		18,000	
2,600	2.20 2.19	18,900	1.70 1.69
2,625 2,675	2.19	19,800 21,500	1.69
	2.18		1.00
2,775 2,850	2.17	22,600 25,000	1.65
3,000	2.10	26,500	1.64
	2.14 2.13	-	1.63
3,100 3,200	2.15	28,000	1.61
3,200 3,500	2.12 2.10	32,000 36,000	1.61
	2.10		1.59
3,600 3,700	2.09	38,000	1.58 1.57
3,700		42,000	
3,800	2.07	49,000 54,000	1.55 1.54
3,900	2.06	54,000	
4,000	2.05	60,000 70,000	1.53
4,200	2.04	70,000	1.52
4,400	2.03	90,000 100,000	1.51
4,600	2.02	100,000+	1.50

*Based on formula:

Peak Factor = 6.2945 x (pop)^{-0.1342} (Holmes & Narver, 1960)

FIGURE 1-1