CHURCH OF THE WOODS EARTHWORK ANALYSIS REPORT

JUNE 12, 2017

Prepared by:

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CHURCH OF THE WOODS EARTHWORK ANALYSIS REPORT JUNE 12, 2017

Site Description

The site consists of 36.7 acres on the north side of State Hwy 18 in the northwest ¼ of Section 29 T 2 N, R 3 W. The site is located on the easterly edge of the area known as Rim Forest in the Lake Arrowhead area of San Bernardino County.

The site is bounded on the south by State Highway 18; on the west by an existing subdivision known as the Arrowview Tract (Tract No. 2015) consisting of $4,000 \pm S$. F. residential lots; on the north and east by vacant land owned by the U. S. Forest Service. State Highway 18 on the south boundary is a paved, two lane highway. Daley Canyon Road, an existing two-lane road, cuts across the northeast corner of the site.

The site itself is a typical wooded mountain site. Since the original developed plan was prepared, dead tree removal has occurred primarily in the area of the site proposed for development. There is an existing natural drainage course running diagonally through the site from the southwest corner to the northeast corner. There is an existing 8" sewer main located in the natural drainage course. This sewer is part of the Lake Arrowhead Community Services District's Wastewater Collection System.

There are no existing drainage or storm drain improvements located on the site.

Proposed Project

The proposed project consists of a church, athletic field, and appurtenant facilities. A proposed site plan is included as Exhibit "A" of this report. The proposed facilities are located generally in the south area of the site.

The total site consists of approximately 36.9 acres. Approximately 6.5 acres will be impervious area, buildings, parking lots, sidewalk and paved assembly areas. Approximately 7.1 acres will be landscaped areas, sodded recreation areas, water quality basin and landscaped slopes. Approximately 23.3 acres will not be developed as part of this project.

The proposed project will require a fill in the natural drainage course in the southwest corner of the site. This fill will require the construction of approximately 750' of storm drain. This storm drain is shown on the site plan. See attached Site Plan included in Exhibit "A" of this report.

Soils Report

A Soils Report titled "Engineering Geology and Soils Engineering Investigation Church of the Woods, Rimforest Area, San Bernardino County" was prepared by LOR Geotechnical Group Inc. dated 11/27/01 and an update letter dated 3/31/17, which states that the site is suitable for intended use provided the recommendations in previous reports are adhered to. A copy of pages 18 through 20, inclusive, of the 11/27/01 report are contained in Exhibit "B" of this report. A copy of the 3/31/17 update letter is included in Exhibit "C" of this report.

Earthwork Calculations

Earthwork calculations were prepared using the "Church of the Woods Conditional Use Permit Site Plan & Preliminary Grading Plan", prepared by W.J. McKeever Inc. dated 4/24/17 and AutoCad Civil 3D Version 2016 software.

A preliminary structural section of 4" of asphalt concrete over 6" of Class II aggregate base was used for the onsite drive aisles and parking spaces. This criteria was incorporated into the raw earthwork cut and fill quantities.

The raw cut and fill quantities are shown on the results of the calculations and is contained in Exhibit "F" of this report. A graphic printout showing the areas and depths of the cuts and fills is contained in Exhibit "G" of this report.

Earthwork Analysis

The Earthwork Analysis consists of adjustments made to the raw cut and fill numbers to account for clearing losses, unsuitable material, over excavation quantities, subsidence of natural ground and shrinkage of material when compacted. These factors are all considered in the determination of final quantities in relation to the balance of the site design. Balance determines if import or export of material is necessary to accomplish final grading of the site.

Clearing losses refer to that volume that is occupied by tree roots, chafe and other vegetation that cannot be included in engineered fill onsite. As mentioned in the Soils Report, the highly organic portions of the top soil, 1 to 2 feet in depth, are not suitable for engineered fill. This material will be exported from the site to a suitable disposal area. Subsidence of natural ground occurs when the surfaces of area to receive fills are scarified to a depth of 6" and recompacted to 90 percent.

Shrinkage occurs when existing in place material which, on this particular site, has a relative density ranging from 64 to 79% of maximum is removed, has water introduced as a lubricant, and is recompacted as fill to 90% of maximum density to support the loads imposed by the

development. This compaction loss is also experienced by the required over excavation and recompaction.

The analysis of the earthwork quantities using the above mentioned data is included in Exhibit "H" of this report.

Conclusion

Preliminary analysis of the raw cut and fill quantities considering the losses, indicates that an excess of 14,553 cubic yards of material would remain after the grading operation was completed. This amount represents approximately 7.2% of the total material to be moved.

Given the variation that may be expected in the behavior of the different materials on site, this variance is within normally expected limits. These quantities will be refined during the final engineering phase of the project and adjustments will be made, if necessary to eliminate exporting of cut material from the site.

EXHIBIT "A" SITE PLAN

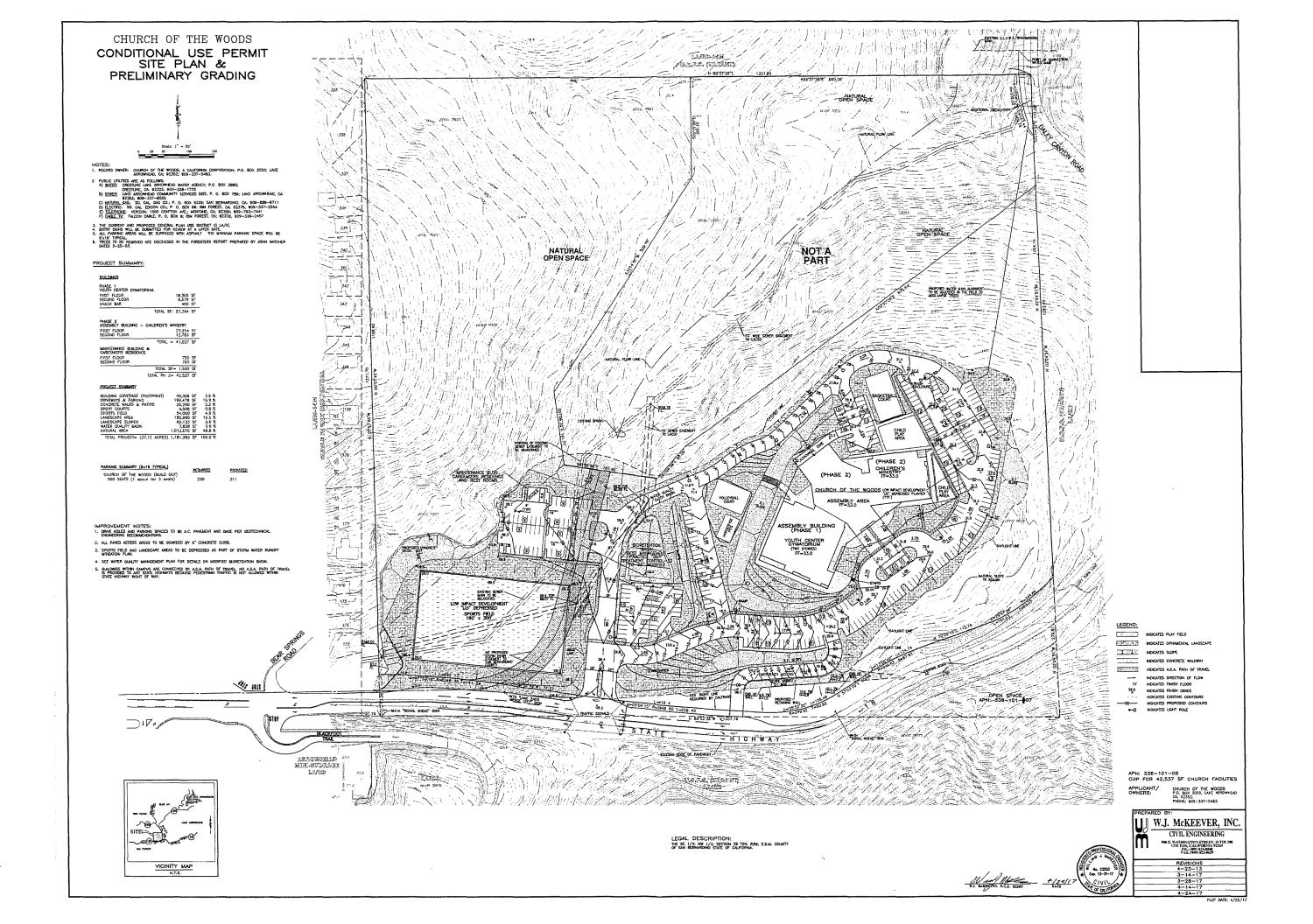


EXHIBIT "B"

SOILS REPORT (Pages 18-20)

ICON November 27, 2001

Foundation Support

Based upon our field investigation and test data, it is our opinion that the thin layers of topsoil, colluvium, and fill found across the site, will not, in the present condition, provide uniform and/or adequate support for the future structures.

Therefore, to provide adequate support for the proposed structures, we recommend that these materials be completely removed from all structural fill areas and a compacted fill mat be constructed beneath footings and slabs. This compacted fill mat will provide a dense, high strength, soil layer to uniformly distribute the anticipated foundation loads.

If the final grade of a particular building site includes deep cuts into the existing materials so as the entire building would be placed on competent bedrock materials, then the construction of a fill mat may not be required.

In addition, care should be exercised along any removals in the southwest portion of the site where the capped water well is believed to exist. If this structure is encountered it may need to be recapped.

Conventional spread foundations, either individual spread footings and/or continuous wall footings, will provide adequate support for the anticipated downward and lateral loads when utilized in conjunction with the recommended fill mat.

RECOMMENDATIONS

General Site Grading

It is imperative that no clearing and/or grading operations be performed without the presence of a certified geotechnical engineer and certified engineering geologist. An on-site, pre-job meeting with the owner, contractor, geotechnical engineer, and engineering geologist, should occur prior to all grading related operations.

Grading of the subject site should be performed in accordance with the following recommendations as well as applicable portions of Appendix Chapter 33 of the Uniform Building Code, and/or applicable local ordinances.

All excavations at the site should be inspected by the project engineering geologist and soils engineer to ensure that the encountered conditions are in conformance with anticipated conditions stated within this report. The soils engineer should provide sufficient observation during grading to ensure that the work is being conducted in accordance with the conditions of the report.

Initial Site Preparation

All previously existing topsoil, loose colluvial, and fills should be removed from all structural areas or areas to receive engineered compacted fill, cleaned of significant deleterious materials, and may be reused as compacted fill. However, the highly organic portions of the topsoil materials (1 to 2 feet) are not considered to be suitable for re-use as engineered fill. The data developed during this investigation indicates that moderate removals on the order of 3 to 16 feet will be required within the anticipated structural areas. The actual depths of removal should be verified during the grading operation by observation and in-place density testing.

Preparation of Fill Areas

Cavities created by removal of subsurface obstructions should be thoroughly cleaned of loose soil, organic matter and other deleterious materials, shaped to provide access for construction equipment, and backfilled as recommended in the following Engineered Compacted Fill section of this report.

Prior to placing fill, the surfaces of all areas to receive fill should be scarified to a depth of at least 6 inches. The scarified soil should be brought to near optimum moisture content and recompacted to a relative compaction of at least 90 percent (ASTM D 1557).

Preparation of Foundation Areas

All footings should rest entirely upon competent bedrock materials or entirely upon at least 18 inches of properly compacted fill material. In areas where the required thickness is not accomplished by site rough grading, the footing areas should be further subexcavated to a depth of at least 18 inches below the proposed footing base grade, with the subexcavation extending at least 5-feet beyond the footing lines. The

bottom of this excavation should then be scarified to a depth of at least 6 inches. brought to near optimum moisture content, and recompacted to at least 90 percent relative compaction (ASTM D 1557) prior to refilling the excavation to grade as properly compacted fill.

Engineered Compacted Fill

Except for the topsoil materials, the on-site soils should provide adequate quality fill material, provided they are free from trash, organic matter, and/or other deleterious materials. Unless approved by the geotechnical engineer, rock or similar irreducible material with a maximum dimension greater than 6-inches should not be buried or placed in fills.

Import fill is not anticipated at this time. However if any fills are imported they should be inorganic, non-expansive granular soils free from rocks or lumps greater than 6inches in maximum dimension. Sources for import fill should be approved by the geotechnical engineer prior to their use.

Fill should be spread in maximum 8 inch loose lifts, each lift brought to near optimum moisture content, and compacted to a relative compaction of at least 90 percent in accordance with ASTM D 1557.

Based upon the relative compaction of the near surface soils determined during this investigation and the relative compaction anticipated for compacted fill soil, we estimate that the compaction shrinkage will vary widely, perhaps ranging from approximately 10 to 20 percent. Deeper excavations encountering competent bedrock should be expected to vield-lesser-shrinkage values on the order of 0 to 5 percent. In addition, we would anticipate subsidence of approximately 0.10 feet. These values are for estimating purposes only, and are exclusive of losses due to stripping or the removal of subsurface obstructions. These values may vary due to differing conditions within the project boundaries and the limitations of this investigation. Shrinkage should be monitored during construction. If percentages vary, provisions should be made to revise final grades or adjust quantities of borrow or export.

EXHIBIT "C"

SOILS UPDATE LETTER

March 31, 2017

ICON General Contractors 1814 Commercenter West, Suite A San Bernardino, California 92408

Project No. 51497.16

Attention: Mr. Patrick Hopkins

Subject:

Geotechnical Update, Church of the Woods, Rimforest Area, San

Bernardino County, California.

At your request, we have prepared this letter report providing a geotechnical update for the subject proposed church development.

A representative from this firm visited the site on March 28, 2017 to observe the current condition of the property. At the time of our visit, the site area generally consisted of forested, gently rolling to steep hillside land in a relatively natural condition and essentially the same as described within our referenced geotechnical investigation report.

The property is considered suitable for use as intended provided the recommendations contained in our previous reports are adhered to.

Although a Conditional Use Permit Site Plan and Preliminary Grading plan were provided at this time, precise grading plans were not yet available for our review. Such plans should be reviewed by this firm prior to construction and additional recommendations can be provided at that time, as necessary.

Section 1613 of Chapter 16 of the 2016 California Building Code (CBC) contains the procedures and definitions for the calculations of the earthquake loads on structures and non structural components that are permanently attached to structures and their supports and attachments. The following earthquake design criteria have been formulated for the site. However, these values should be reviewed and the final design should be performed by a qualified structural engineer familiar with the region.

CBC 2016 SEISMIC DESIGN SUMMARY Site Location: (USGS WGS84) 34.2305, -117.2179, Occupancy Category II				
Site Class Definition (Table 1613.5.2)	D			
S _s Mapped Spectral Response Acceleration at 0.2s Period, (Figure 1613.5(3))	2.942			
S ₁ Mapped Spectral Response Acceleration at 1s Period, (Figure 1613.5(4))	0.960			
F _a Short Period Site Coefficient at 0.2s Period, (Table 1613.5.3(1))	1.0			
F _v Long Period Site Coefficient at 1s Period,(Table 1613.5.3(2))	1.5			
S _{MS} Adjusted Spectral Response Acceleration at 0.2s Period, (eq .16-36)	2.942			
S _{M1} Adjusted Spectral Response Acceleration at 1s Period, (eq .16-37)	1.439			
S _{DS} Design Spectral Response Acceleration at 0.2s Period,(eq .16-38)	. 1.961			
S _{D1} Design Spectral Response Acceleration at 1s Period, (eq .16-39)	0.960			
Seismic Design Category, Short Period (Table 1613.5.6(1))	E			
Seismic Design Category, Long Period (Table 1613.5.6(2))	E			

We trust this information is desired at this time, if you have any questions please contact this firm at your convenience.

Respectfully submitted, LOR Geotechnical Group, Inc.

John P. Leuer, GE 2030

President

RMM:JPL/ss

Addressee (2) and via email: patrick@icongc.net Distribution:

REFERENCES

- 1. California Building Standards Commission and International Conference of Building Officials, 2016, California Building Code, 2016 edition.
- 2. LOR Geotechical Group, Inc., 2001, Engineering Geology and Soils Engineering Investigation, Church of the Woods, Rimforest Area, San Bernardino County, California, Project No. 51497.1, Revision dated November 27, 2001.
- 3. San Bernardino County Land Use Services, Geologic Hazards Map FH23 C, Rimforest.
- 4. W. J. McKeever, Inc., 2017, Church of the Woods, Conditional Use Permit Site Plan & Preliminary Grading, 50-scale, dated March 14, 2017.

EXHIBIT "D"

NATURAL GROUND MODEL



EXHIBIT "E"

DESIGN MODEL

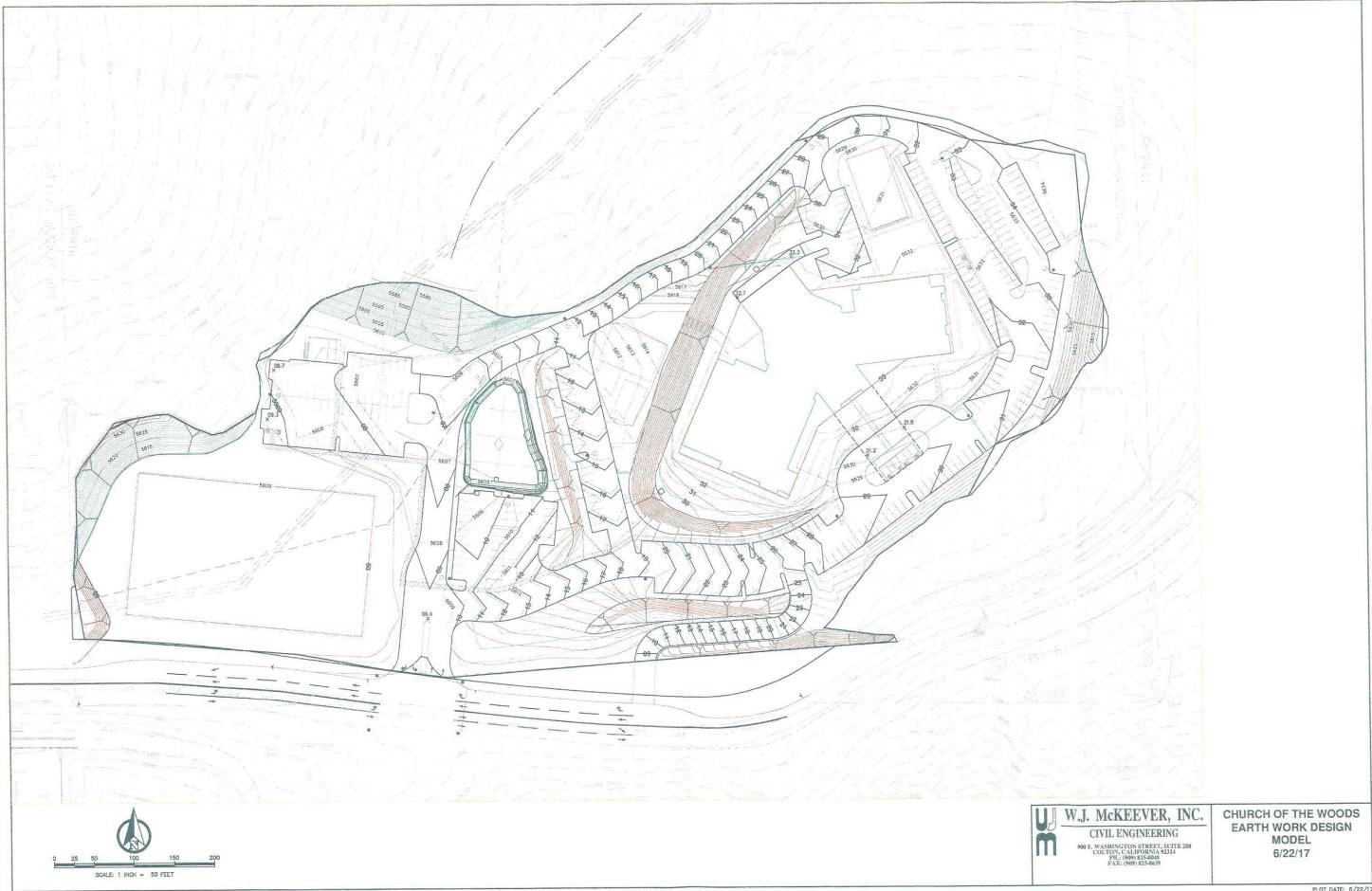


EXHIBIT "F"

EARTHWORK CALCULATIONS

Cut/Fill Report

Generated:

2017-06-21 08:03:51

By user:

admin

Drawing:

 $G: \A CADDWGS \C -241\ Church\ of\ the\ Woods \C 241_Earthwork \G: \A CADDWGS \C -241$

Church of the Woods\C241_Earthwork\C241_Surface_Earthwork_6-21-17.dwg

Volume Summary							
Name	Туре	Cut Factor	Fill Factor	2d Area (Sq. Ft.)	Cut (Cu. Yd.)	Fill (Cu. Yd.)	Net (Cu. Yd.)
C241_Earthwork_Edits	full	1.000	1.000	556978.14	195296.68	119312.98	75983.70 <cut></cut>

Totals				
	2d Area (Sq. Ft.)	Cut (Cu. Yd.)	Fill (Cu. Yd.)	Net (Cu. Yd.)
Total	556978.14	195296.68	119312.98	75983.70 <cut></cut>

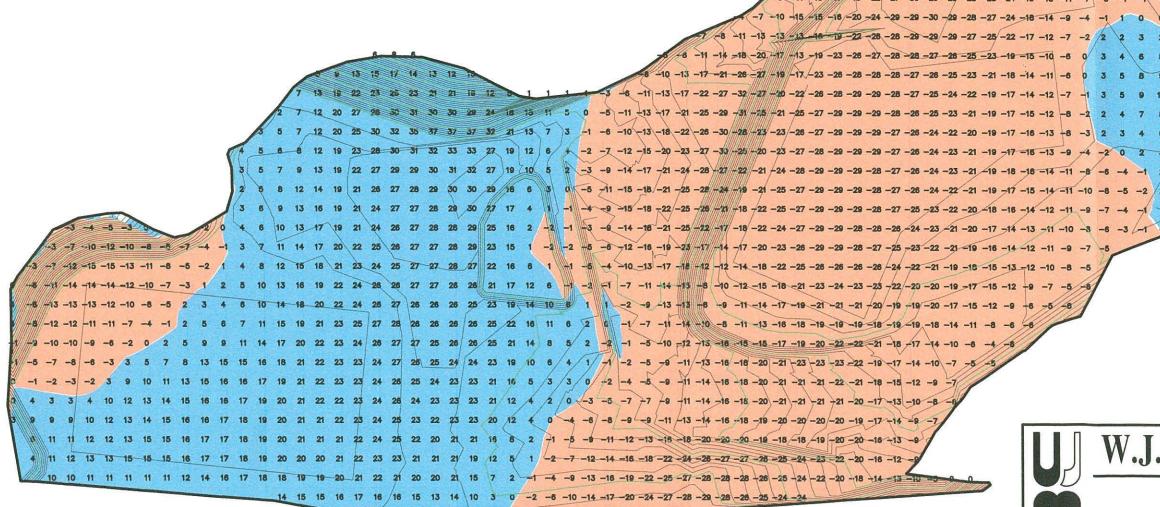
^{*} Value adjusted by cut or fill factor other than 1.0

EXHIBIT "G"

EARTHWORK GRAPHIC



	ELE	ATIONS TABI	LE	
NUMBER	MIN. ELEVATION	MAXI. ELEVATION	AREA	COLOR
1	-33.00	-0.05	339814.61 SF	
2	-0.05	0.05	1532.73 SF	
3	0.05	38.04	215630.80 SF	



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EXHIBIT "H"

EARTHWORK ANALYSIS

CHURCH OF THE WOODS EARTHWORK ANALYSIS June 21, 2017

Raw cut & fill numbers per W.J. McKeever Inc. 6/21/17 (Attached).

Unsuitable material, subsidence and shrinkage information per LOR Geotechnical Group Inc., Engineering Geology and Soils Engineering Investigation, Church of the Woods, Rimforest Area, San Bernardino County, California dated 11/27/2001 and update letter dated 3/31/17 (Attached).

Cut Over Exc Unsuitable Material Subsidence Subtotal	195,297 CY - 5,655 CY -42,367 CY - 1,270 CY 157,315 CY
Shrinkage 50% of Subtotal @ 15% = 50% of Subtotal @ 5% =	11,799 CY 3,933 CY 15,732 CY
Cut Available for Fill	141,583 CY
Fill = Over Exc Subsidence Total Fill Required	119,313 CY 5,655 2,062 CY 127,030 CY
Net	14,553 CY - Excess